City of Venice
Building Department
401 W. Venice Ave
Venice, Fl. 34285

Phone (941) 486-2626 Fax (941) 486-2448 Inspections (941) 483-5907 Apply Online https://trakit.venicegov.com/eTRAKiT/

RESIDENTIAL DATA SUMMARY WORKSHEET

This form shall be completed and submitted with Application Documents

Owners Name	D R HORTON INC		P.I.D.		
Project Address	277 VINADIO BLVD. NOKOMIS , FL, 34275				
Design Professional	Structural Systems	Phone	239-549-4554	Fax	
Contractor	DR HORTON INC	Phone	239-225-600	Fax	

Applicable Codes

Building Code	Florida Building Code	2020 Residential Volume
Mechanical Code	Florida Building Code	2020 Residential Volume
Plumbing Code	Florida Building Code	2020 Residential Volume
Electrical Code	NFPA 70 / NEC 2020	
Accessibility Code	Florida Building Code	FACBC 2020
Energy Code	Florida Building Code	Residential Energy Efficiency 2020

Manufacturer / FL Product Approval / NOA #

Doors / SGD	MI Window FL22401.3-FL22401.4
Windows	SH MI Window-Impact FL21637.7
Overhead Doors	Wayne Dalton FL9174.1/9174.3
Mitered Glass	N/A
Shutters	ALL AMERICAN - FL17869.1
Roof Coverings	Eagle Roofing - FL7473.1 (R9)
Soffit	KAYCAN LTD - FL24564.3 (R4)
Sentricon Bait	BORA CARE

Method of Design per R301 / Residential Volume

_____ AF&PA (WFCM) _____ ASCE 7 _____ AISI (COFS/PM) _____ ICC 600

_____ MAF Guide Other _____

X FBC 2020 / Residential

Volume **Construction Type** IV V (circle one) Other VB

Design Wind Speed 160 m.p.h. R301.2 (4)

Importance Factor 1.0

Wind Debris Area (Yes) No Exposure B or (C) (circle one)

Structural Forces Section R301.4 / R301.5 / R301.6

Floor Design Live Load 40 p.s.f
Dead Load Slab On Grade p.s.f

Roof Design Live Load 20 p.s.f
Dead Load TC=20 BC=10 p.s.f

WINDOW & DOOR WIND PRESSURE DESIGN LOADING

Mean Roof Height 15 feet

Windows +33.5, -44.8 psf

Doors +33.5, -44.8 psf

Garage Doors +39.20, -43.70 psf

Please Show Design Pressure for Worst Case ONLY

Components and Cladding Design Pressures: R301.2 (7)

Z1 +24.9, -44.8 p.s.f. Z3 +24.9, -61.7 p.s.f. Z5 +33.5, -44.8 p.s.f.

Z2 +24.9, -61.7 p.s.f. Z4 +33.5, -36.3 p.s.f. a= edge distance 4 ft.

Misc. Notes

For Specific window and door pressures, see Sheet A3 or S-2, whichever one is sealed.

Area Tabulation

Living	2,000	sf / Conditioned Space
Garage	552	sf 167
Lanai	167	sf
Entry	98	sf
Storage		sf
Other		sf
	2,817	**Total square footage**

I certify to the best of my knowledge and belief, these plans and specifications have been designed to comply with the structural portion of the Building Code for wind and gravity loads as amended and enforced by the permitting jurisdiction.

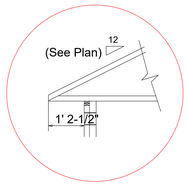
Signature _____ **Date** _____
Architect / Engineer

RAUL REYES LICENSE No. 88925 STATE OF FLORIDA PROFESSIONAL ENGINEER

Seal Residential Data Summary Worksheet

This item has been digitally signed by Raul Reyes on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be validated on any electronic copies.

Hatch Legend
13' 11" BRG HGT
12" TRAY CEILING



CANTILEVER DETAIL
1X

Wind Importance Factor	1
Occupancy/Risk Category	II
Lanai, Entry, Porch areas	EXPOSED TO WIND

TRUSSES DESIGNED FOR TILE OR SHINGLE APPLICATIONS
BUILDING CODE: FBC2020 / TPI 2014
9' 4" WALL HEIGHT TYP.
1X

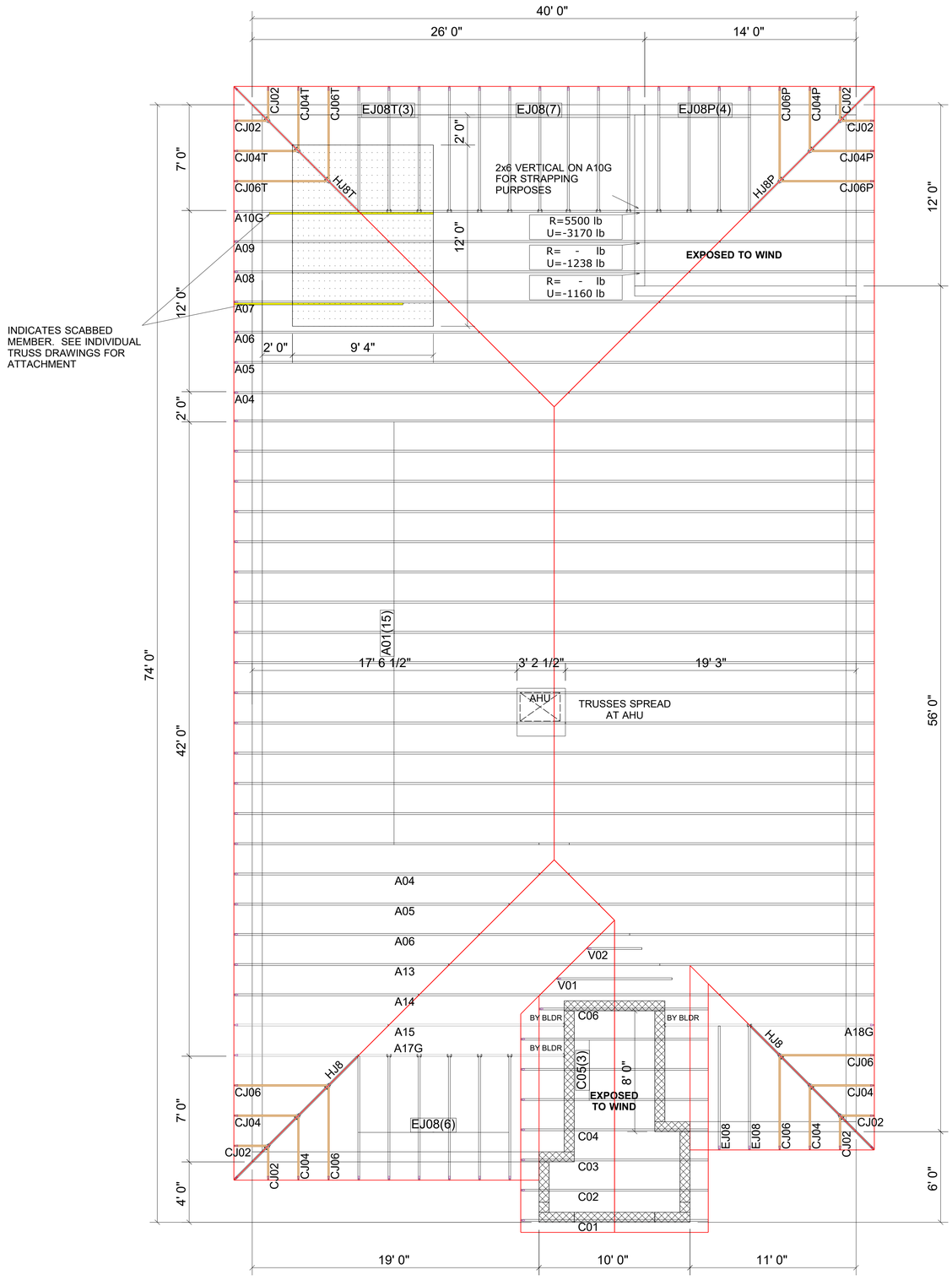
Truss List of <5000# reaction & <-1000# uplift

Truss	Qty	Span	Reactions		
A08	1	42' 5"	1201.62 lb -439.43 lb	2678.14 lb -1159.01 lb	467.03 lb -463.58 lb
A09	1	42' 5"	1154.57 lb -411.56 lb	2799.18 lb -1237.65 lb	386.86 lb -412.82 lb
A10G	1	42' 5"	2097.32 lb -864.07 lb	5499.09 lb -3169.87 lb	450.47 lb -586.91 lb

Engineer of Record for the Structure
Structural Systems of N. Fl, Inc.
Raul Reyes, PE 88925
1634 SE 47th Street #3
Cape Coral, FL 33904

This document has been reviewed for conformance with the design intent of the structure and specified design criteria.

☒ Accepted As-Is ☐ Accepted As Noted ☐ Revise and Resubmit



Hanger Notes

* Refer to Simpson Strong-Tie website (www.strongtie.com/products/connectors), or the USP website (www.uspconnectors.com/us/products/connectors), for proper use and attachment of the specified hangers.

General Notes

*** Required interior bearing walls shown @ heights noted**
*** Trusses may not be cut or altered in any way without prior authorization from ABS, Inc.**
*** Any trusses that are cut or altered without authorization will be repaired or replaced at the customers expense**
*** No backcharges of any kind will be accepted without prior review and written consent from ABS, Inc.**
*** For proper truss handling and bracing, refer to the "TPI" documents "BCSI-B1 through B4"**
*** Any multi-ply trusses must be attached together per the engineering specifications prior to installation**

*** Permanent and temporary bracing is the responsibility of the truss installer. The "Engineer of Record" for the project is responsibile for the design of the permanent bracing, the diaphram system, shear walls, and structural elements to resist lateral loads from wind and or seismic activity. The "EOR" is also responsible to call out the required strapping materials to sufficiently attach the trusses to the load bearing structure below, to verify truss design specifications (pitch, span, profiles, applied loading, wind application, etc.), and for the overall design and placement plan of the truss system.**

*** If any job site accidents occur involving trusses, the installer must immediately stop work on the project and notify a representative of ABS, Inc.. All trusses involved in an accident must be inspected by a licensed structural engineer to determine the cause of the accident. The builder assumes all liability if trusses involved in an accident are altered or moved in any way before an inspection is completed. All decisions regarding necessary repairs or replacement of trusses will be based on the recommendation of the report submitted by the structural engineer.**

MULTI-PLY ATTACHMENT

* For 4-ply or 5-ply attachment, refer to the Detail Packet Sheet:
"STANDARD BOLT TO SCREW TRUSS CONNECTION DETAIL" -
"T-4PLY OR 5PLY SCREW"

* * * C R I T I C A L * * *
ATTN: FRAMER
For multi-ply girder attachments, refer to engineering for specific instructions for attaching plies. Each ply must be applied in layers per the nailing specifications.

2-ply trusses may be nailed from one face.

For 3-ply trusses, the first two plies are nailed together from one face, then third ply is attached to either face of first two plies.

For 4 ply trusses, after assembling the first three plies, attach fourth ply to either face.

For 5 ply trusses, after assembling the first four plies, attach fifth ply to either face.

(Refer to engineering for additional bolts or screw rqrmts and the "STANDARD BOLT TO SCREW TRUSS CONNECTION DETAIL" for substituting screws for bolts, located in the engineering detail pkg.

NOTE: Bolts/Screws are intended to provide clamping force to aid in allowing the mult-ply assembly to act as a unit and are not included in the calculation of ply-to-ply load transfer.

Designed Per ASCE 7-16

Loading and Design Criteria

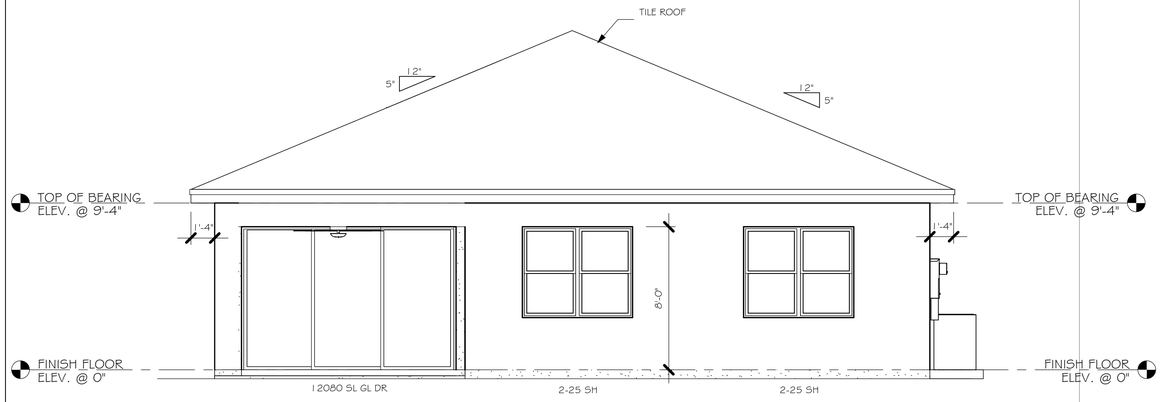
	Roof	Floor	BLDG CODE	FBC2020
TC LL	20	40	Mean Hgt	15'
TC DL	20	10	Wind Speed	160
BC LL	0	0	Exposure	C
BC DL	10	5		
Duration	1.25	1.00		

AMERICAN BUILDERS SUPPLY

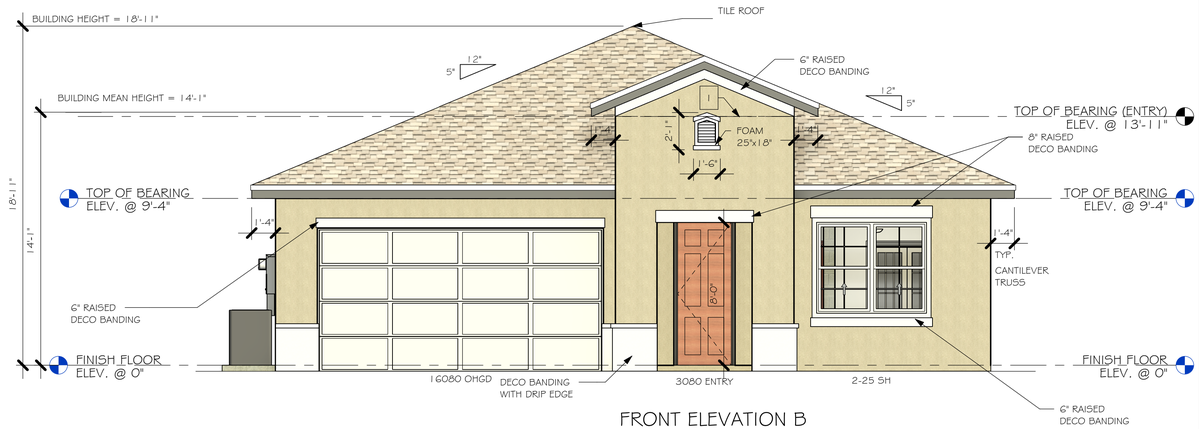
Builder:	DR Horton Ft Myers
Model:	1962 B,F
Options:	Base
Location:	location

Date:	Sales Rep:	Designer:	Job Number:
12/7/2020	Carl F	Joe D	M2001623-20BX

Revision: 2x6 vertical over brg on A10G 7/6/21 Joe D

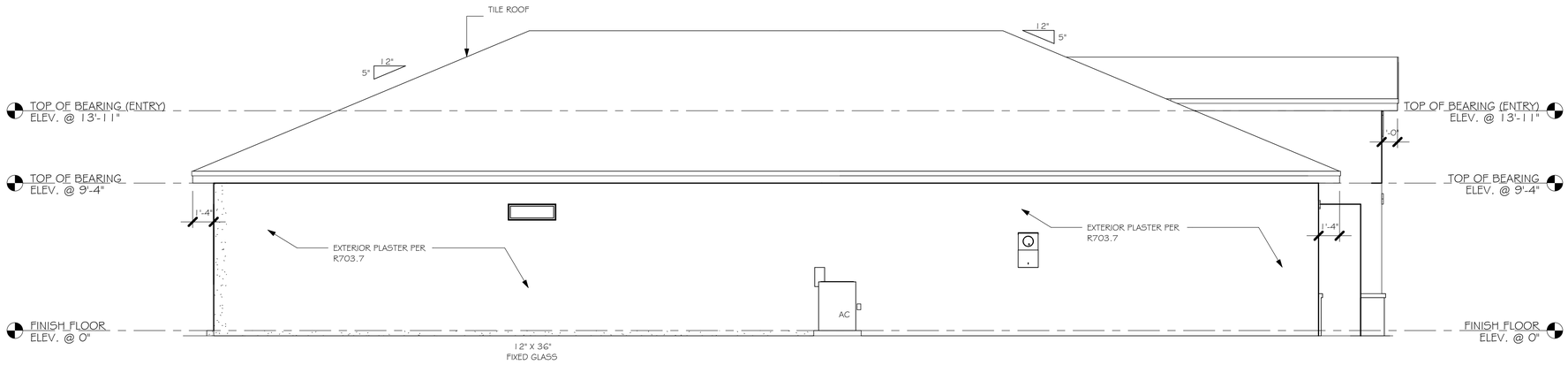


REAR ELEVATION B
1/4" = 1'-0"

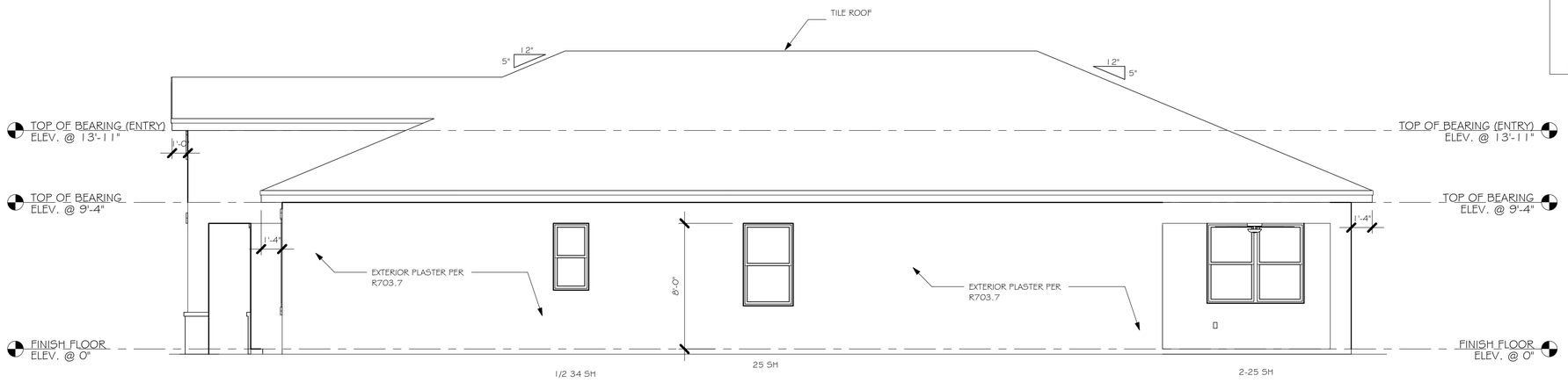


FRONT ELEVATION B
1/4" = 1'-0"

1	MID-WALL WEEP SCREED AT WOOD MASONRY INTERFACE. INSTALL STRICTLY PER MFG. INSTRUCTIONS
2	ROOF / WALL SCREED INSTALL STRICTLY PER MFG. INSTRUCTIONS



LEFT ELEVATION B
1/4" = 1'-0"

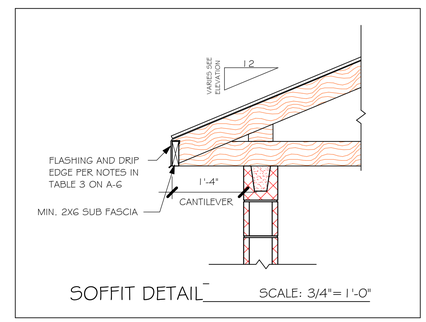


RIGHT ELEVATION B
1/4" = 1'-0"

FLORIDA BUILDING CODE 7TH EDITION

OCCUPANCY: FBC 310.5 RESIDENTIAL GROUP R-3
CONSTRUCTION TYPE: V-B (FIRE RESISTANCE RATING 0 HOURS, NOT SPRINKLED)

CODES TO BE USED BY OTHER DESIGN PROFESSIONALS AND LICENSED CONTRACTORS:
2020 FLORIDA BUILDING CODE, 7TH EDITION: RESIDENTIAL; ACCESSIBILITY; ENERGY CONSERVATION; PLUMBING; MECHANICAL; AND FUEL GAS.
ELECTRICAL IS CONTAINED BY REFERENCE WITHIN FBC RESIDENTIAL CHAPTER 34: NFPA 70-17 NATIONAL ELECTRICAL CODE.



SOFFIT DETAIL SCALE: 3/4"=1'-0"

DESIGN IN ACCORDANCE WITH THE RESIDENTIAL FLORIDA BUILDING CODE 2020 - 7TH EDITION

D·R·HORTON® America's Builder
DHI NYSE

Gulf Coast Drafting & Design, Inc.
EMAIL: PLANS@GULFCOASTDRAFTING.COM
PHONE: 239-540-1822
1515 SE 47TH ST. CAPE CORAL, FL 33904

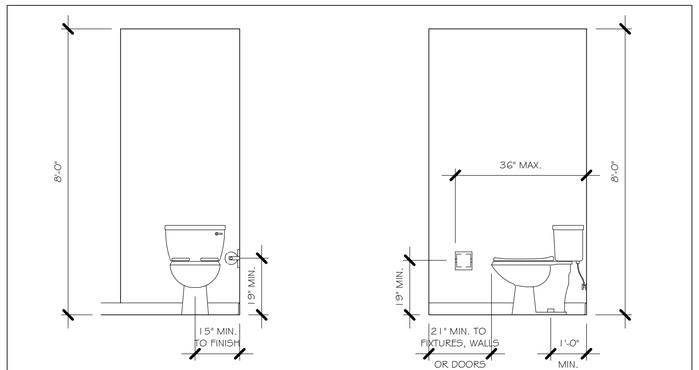
LOT: 699
SUBDIVISION: TOSCANA III & IV 50s
ADDRS: 277 VINADIO BLVD
D.R.H. #: 5795801 11

MODEL # 1962 B

GCD JOB # 13514

DATE: 10/4/21
DRAWN BY: CWL
CHECKED BY: JWC
REVISED:
PLAN: ELEVATION
SCALE: As indicated

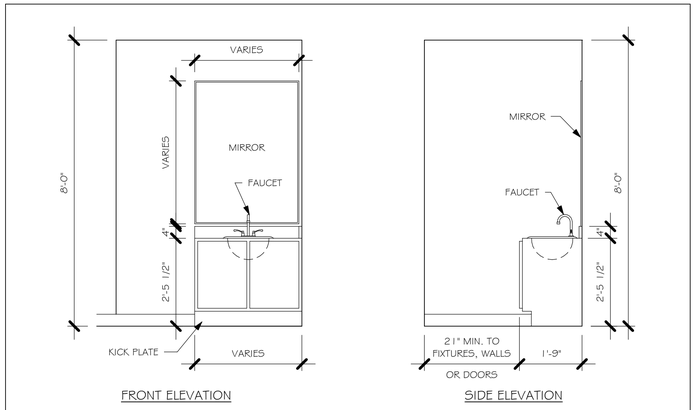
A-1



FRONT ELEVATION

SIDE ELEVATION

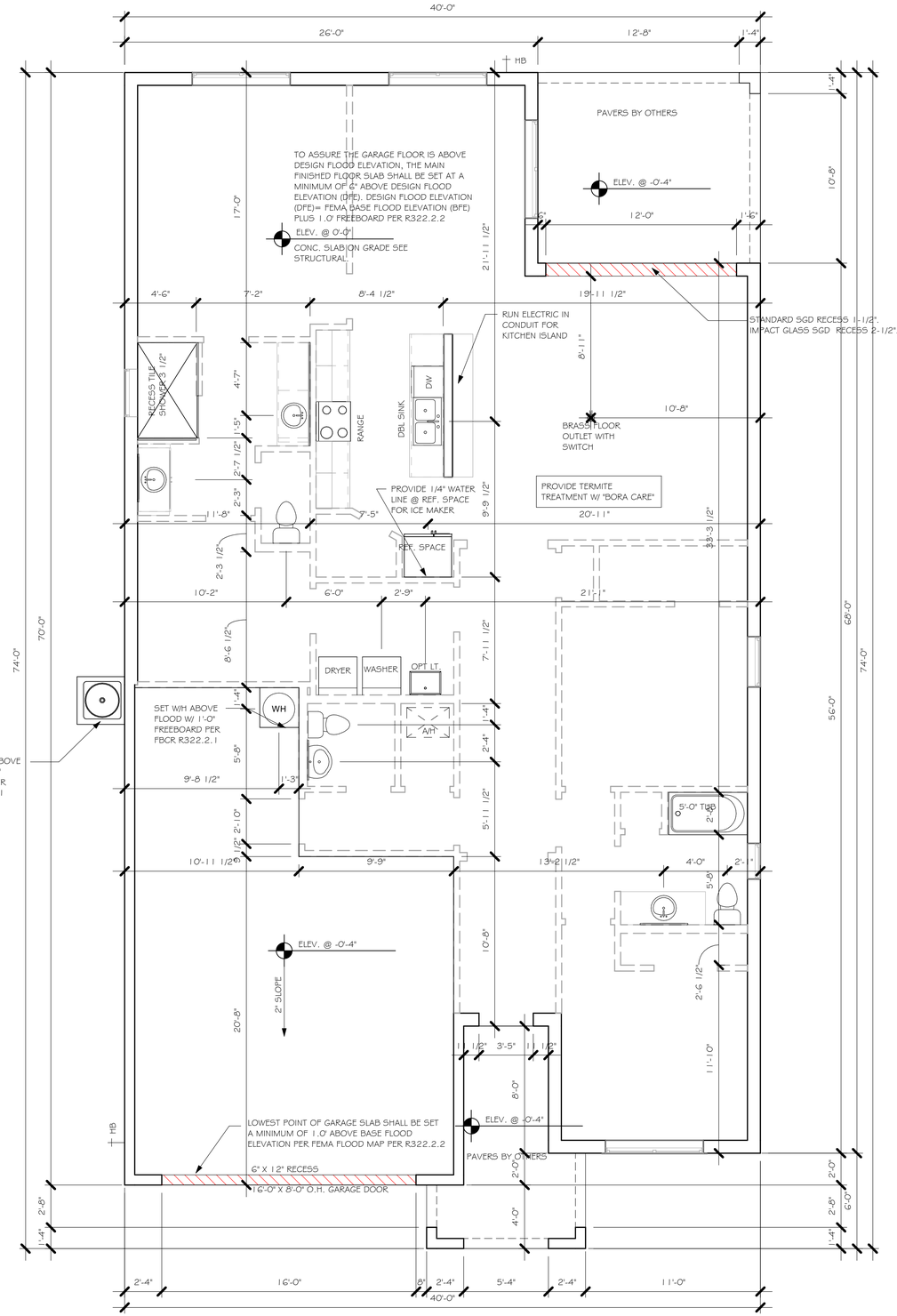
WATER CLOSET CLEARANCE DETAIL



FRONT ELEVATION

SIDE ELEVATION

VANITY SINK CLEARANCE DETAIL



SLAB PLUMBING PLAN

1/4" = 1'-0"

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LOT: 699
SUBDIVISION: TOSCANA III & IV 50s
ADDRS: 277 VINADIO BLVD
D.R.H. #: 5795801111

MODEL # 1962 B

GCD JOB # 13514

DATE: 10/4/21
DRAWN BY: CWL
CHECKED BY: JWC
REVISED:
PLAN: FOUNDATION
SCALE: As indicated

A-2

DOOR SCHEDULE

TYPE MARK	DESCRIPTION	MANUFACTURER	HEIGHT	WIDTH	COMMENTS	QTY
1	3080 ENTRY	DISTINCTION	8'-0"	3'-0"		1
2	(3)-4080 SL. GL. DR.	DISTINCTION	8'-0"	12'-0"	IMPACT	1
3	16080 OHGD	GARAGE DOOR	8'-0"	16'-0"		1

WINDOW SCHEDULE

MARK	DESCRIPTION	WIDTH	HEIGHT	COMMENTS	QTY
A	1/2 34 SH	2'-5"	4'-5"	IMPACT	1
B	25 SH	3'-4"	5'-5"	IMPACT	1
C	2-25 SH	6'-4"	5'-3"	IMPACT	4
D	12" X 36" FIXED GLASS	3'-2"	1'-2"	IMPACT	1

OPT. IMPACT GLASS MAY BE INSTALLED IN LIEU OF SHUTTERS VERIFY W/ CONTRACT

DOOR HEADERS

6'-8" BI-FOLD	HEADER HEIGHT	82" A.F.F.
6'-8" SWING	HEADER HEIGHT	82 1/2" A.F.F.
8'-0" SWING	HEADER HEIGHT	98 1/2" A.F.F.

PLAN NOTES

1) VERIFY ALL ROUGH OPENING DIMENSIONS FOR ALL WINDOWS AND DOORS

2) PROVIDE SAFETY GLAZING WITHIN 24" FROM EXIT PER FLORIDA BUILDING CODE R 308.4.2.

3) PROVIDE SAFETY GLAZING AT BATH/ SHOWER PER FLORIDA BUILDING CODE R 308.4.5.

4) NON BEARING INTERIOR FRAME WALLS SHALL BE FRAMED W/ WOOD OR METAL STUDS, SPACING SHALL NOT EXCEED 24" O.C. (NON BEARING WALLS ONLY)

5) PROVIDE DEAD WOOD IN ATTIC FOR OVERHEAD GARAGE DOOR HARDWARE

6) KITCHEN KNEE WALL TO BE FRAMED W/ TOP @ 34 1/2" A.F.F.

7) INSTALL SMOOTH WALLS IN KITCHEN AND ALL BATHROOM AREAS

8) WHERE DRYWALL CEILING IS APPLIED TO TRUSSES @ 24" O.C. USE 5/8" DRYWALL OR 1/2" SAG RESISTANT PER SEC. R702.3.5

9) THE GARAGE SHALL BE SEPARATED FROM THE RESIDENCE & ATTIC BY NOT LESS THEN 1/2" GYPSUM BOARD APPLIED TO THE GARAGE SIDE. GARAGES BENEATH HABITABLE ROOMS SHALL BE SEPARATED WITH NOT LESS THAN 5/8" TYPE "X" GYPSUM BOARD OR EQUIVALENT. WHERE THE SEPARATIION IS A FLOOR - CEILING ASSEMBLY, THE STRUCTURE SUPPORTING THE SEPARTION SHALL ALSO BE PROTECTED BY NOT LESS THAN 1/2" GYPSOM BOARD OR EQUIVALENT

10) INSTALL 1 3/8" THICK SOLID WOOD DOOR BETWEEN LIVING AND GARAGE PER FLORIDA BUILDING CODE R302.5.1.

11) ALL WINDOWS INSTALLED 72" ABOVE GRADE MUST COMPLY WITH R312.2 MIN 24" SILL HEIGHT OR PROVIDED WITH AN APPROVED WINDOW FALL PREVENTION DEVICE

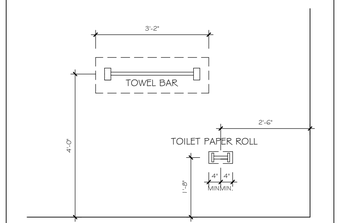
12) ALL CLOSET SHELVES TO BE 12", ALL PANTRY & LINEN TO BE (4)-16" SHELVES 18" O.F.F. W/ 15" INCREMENT.

13) ALL MECHANICAL AND ELECTRICAL EQUIPMENT TO BE INSTALLED AT OR ABOVE FLOOD PLUS 1'-0" FREEBOARD.

CABINET BACKING

KITCHEN	UPPER TOP @ 84"	BASE TOP @ 35"
MASTER BATH	UPPER	BASE TOP @ 35"
GUEST BATH	UPPER	BASE TOP @ 31"
LAUNDRY ROOM	UPPER TOP @ 84"	BASE

	BATHROOM NOTES
TB TOWEL BAR	ALL TUB DECKS @ 21" A.F.F
TP TOILET PAPER	ALL BLOCKING TO BE PT IN SHOWERS



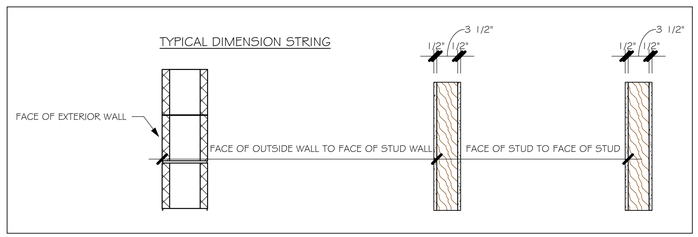
SQUARE FOOTAGE

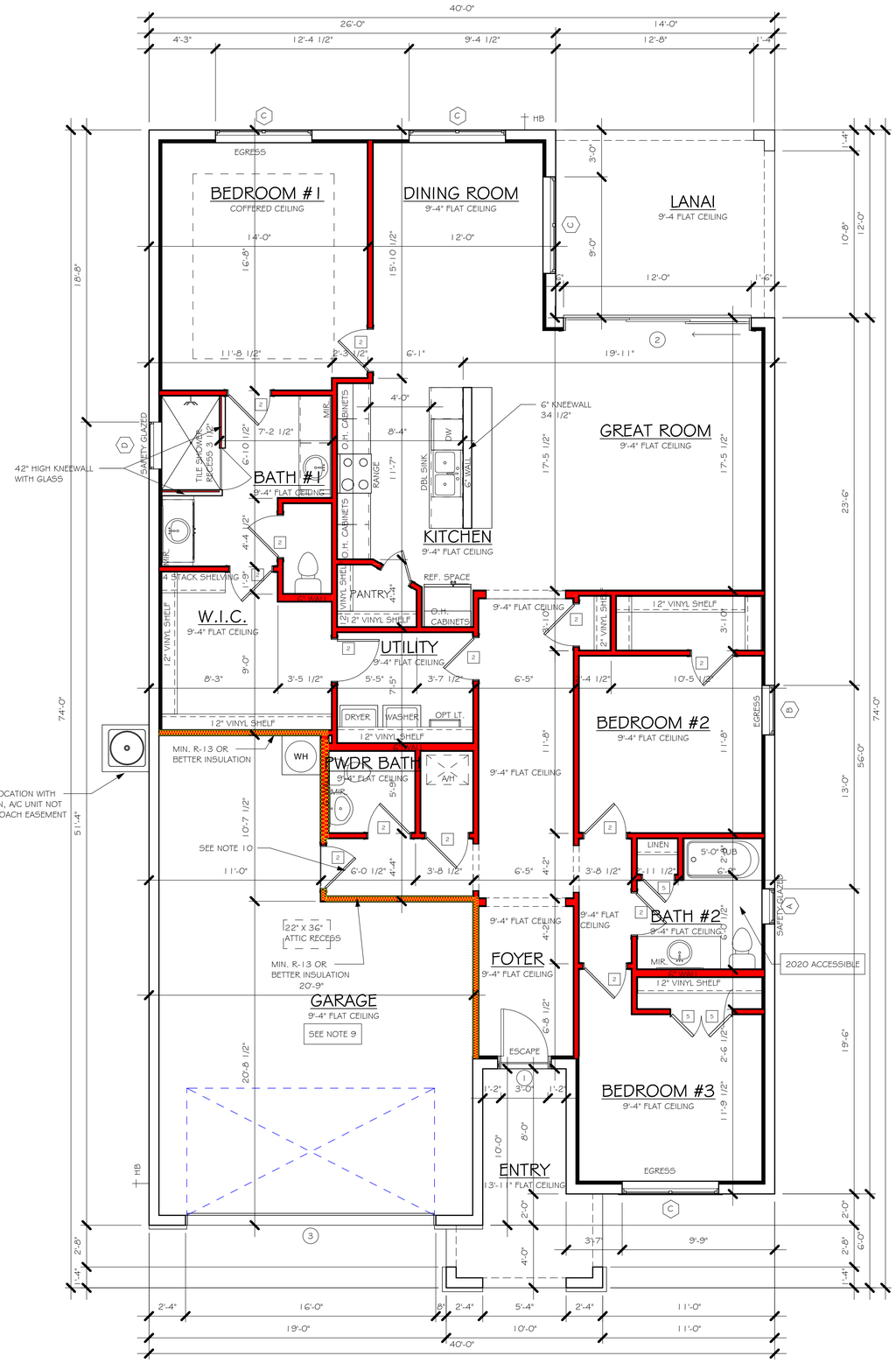
ENTRY AREA	98 SF
LANAI AREA	167 SF
GARAGE AREA	552 SF
LIVING AREA	2000 SF
TOTAL SQAURE FOOTAGE	2817 SF

INTERIOR DOOR SCHEDULE

MARK	DOOR WIDTH
1	3'-0"
2	2'-8"
3	2'-6"
4	2'-4"
5	2'-0"
6	1'-8"
7	1'-6"
8	2'-11"

P.K. = POCKET DOOR
B.F. = BI-FOLD DOOR
B.P. = BI-PASS DOOR
L.V. = LOUVERED DOOR





DESIGN IN ACCORDANCE WITH THE RESIDENTIAL FLORIDA BUILDING CODE 2020 - 7TH EDITION

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America's Builder

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LOT: 699
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ADDRS: 277 VINADIO BLVD
D.R.H. #: 5795801111

MODEL # 1962 B

GCD JOB # 13514

DATE: 10/4/21
DRAWN BY: CWL
CHECKED BY: JWC
REVISED:
PLAN: FLOOR
SCALE: As indicated

A-3

MODEL 1962 B : ATTIC VENTILATION FBCR R806

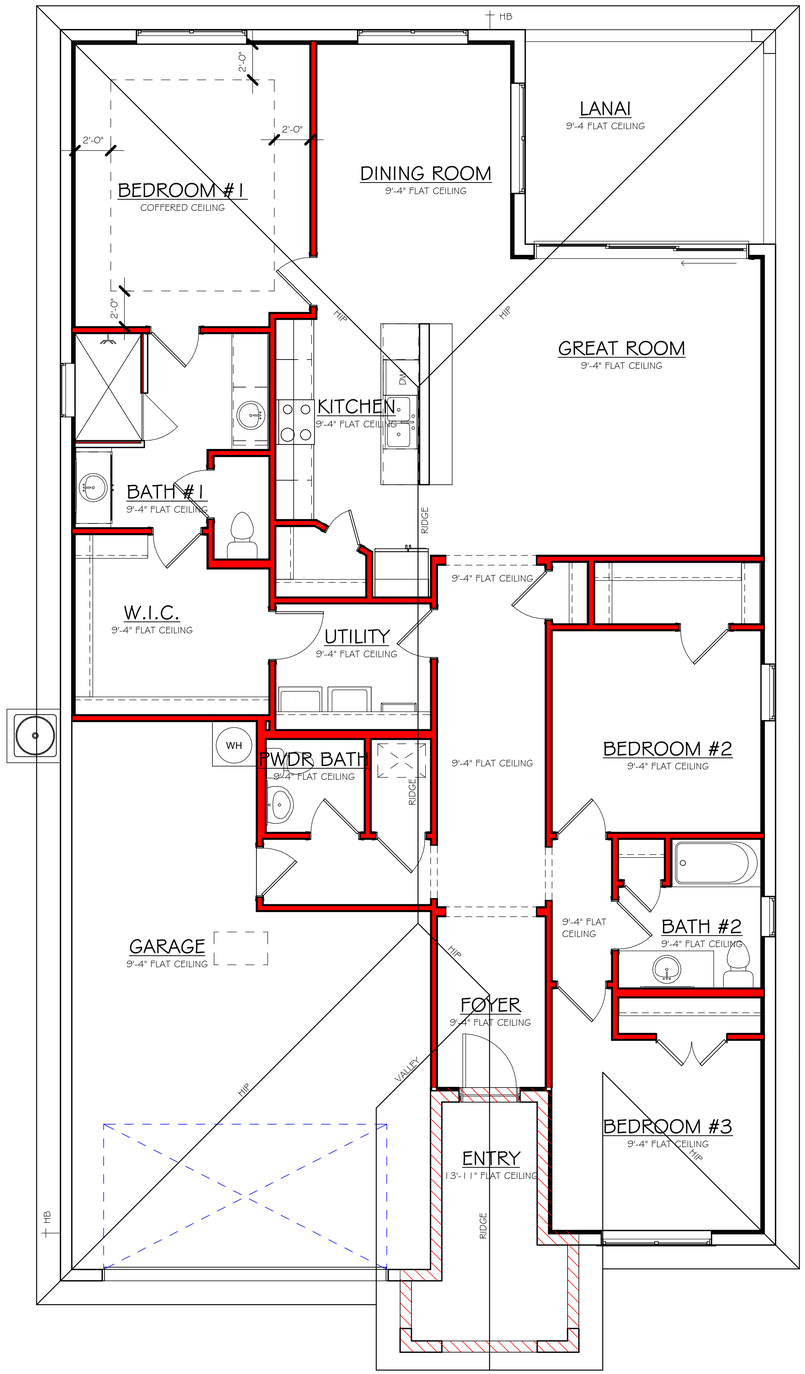
COORDINATE VENTING REQUIREMENTS WITH ENERGY CALCULATIONS

			SOFFIT ONLY (1/150) (NO ROOF VENTS)			WITH ROOF VENTS (1/300) (R.V.)		
AREAS (SQ. FT.)			ATTIC VENTILATION REQUIRED			ATTIC VENTILATION REQUIRED		
MARK	ATTIC	SOFFIT	ATTIC AREA/150	REQ'D AIR FLOW OF SOFFIT	QUAD 4 SOFFIT HAS	ATTIC AREA/300	QUANTITY OF ROOF VENTS	MIN AIR FLOW OF SOFFIT
1st STORY	3125.0 SQ. FT.	308.0 SQ. FT.	20.83 SQ.FT.	6.76%	8.15%	-.-- SQ. FT.	-	-.-%
			"SOFFIT ONLY" QUALIFIES			ROOF VENTS ARE NOT REQUIRED		
			SOFFIT MODEL ACM QUAD 4, FULL VENT, NARROW PATTERN, 8.15% FREE AIR FLOW			ROOF VENT MODEL 32" BASE 22 3/8" BASE LOMANCO 770-D 0.97 SQ. FT. FREE AIR		

BEARING HEIGHT

= BEARING @ 9'-4"

= BEARING @ 13'-11"



ROOF PLAN

1/4" = 1'-0"

DESIGN IN ACCORDANCE WITH THE RESIDENTIAL FLORIDA BUILDING CODE 2020 - 7TH EDITION

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REVISED:
PLAN: ROOF
SCALE: As indicated

A-4

ELECTRICAL LEGEND

	ELECTRICAL METER
	ELECTRICAL PANEL
J	120 V JUNCTION BOX
	SINGLE RECEPTACLE OUTLET
	220 V RECEPTACLE OUTLET
	4-PLEX RECEPTACLE OUTLET
	DUPLEX RECEPTACLE OUTLET
	1/2 SWITCHED DUPLEX OUTLET
AFF	DUPLEX RECEPTACLE AT ELEV. A.F.F.
	DUPLEX RECEPTACLE - ABOVE COUNTER
S	SINGLE POLE SWITCH
S 3	3 WAY SWITCH
S D	DIMMER SWITCH
S MS	MOTION SENSOR SWITCH
SD SCD	AC/DC SMOKE DETECTOR TO BE INTERCONNECTED ANY RESIDENT HAVING A FOSSIL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR AN ATTACHED GARAGE SHALL HAVE AN OPERATIONAL CARBON MONOXIDE ALARM INSTALLED WITHIN 10 FEET OF EACH ROOM USED FOR SLEEPING PERPOSES. PER RULE 9B-3.04.72 SD (SMOKE DETECTOR) SCD (CARBON MONOXIDE/ SMOKE DETECTOR)
T	TELEPHONE OUTLET
TV	TELEVISION RECEPTION OUTLET
	SURFACE MOUNTED CEILING LIGHT
	FLUSH MOUNTED LIGHT
	WALL MTD. BRACKET LIGHT
	DUPLEX FLOOD LIGHT
	EXHAUST FAN
	TRACK MTD. LIGHTS
	A/C DISCONNECT
PB	PUSH BUTTON (PB) / DOOR BELL (DB)
IC	INTERCOM
K	KEYPAD
	4' FLUORESCENT LIGHT
	2' UNDER COUNTER LIGHT

NOTE: NOT ALL SYMBOLS ARE USED FOR THIS PROJECT.

ELECTRICAL NOTES:

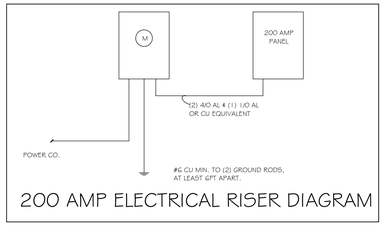
ARC-FAULT CIRCUIT-INTERRUPTERS AND TAMPER RESISTANT RECEPTACLES SHALL BE INSTALLED IN DWELLING UNITS PER N.E.C 210.12 AND 406.11

ALL ELECTRIC, ELECTRICAL EQUIPMENT AND APPLIANCES TO BE SET AT OR ABOVE BASE FLOOD ELEVATION PLUS 1'-0" FREEBOARD.

ALL OUTLETS IN WET AREAS AND ALL EXTERIOR OUTLETS TO BE GFI'S.

INSTALL PHONE AND T.V PER CONTRACT.

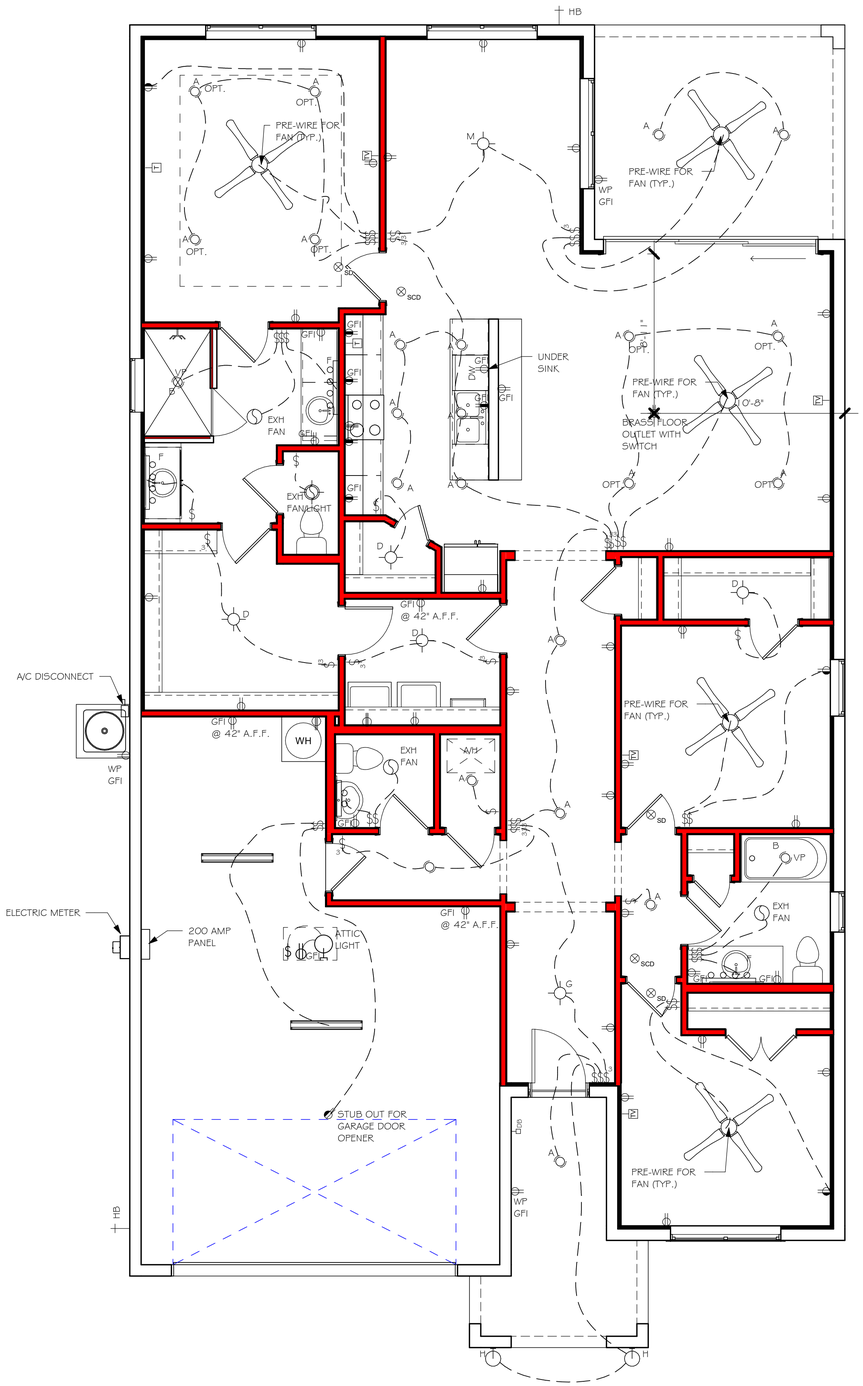
INSTALL ALL ELECTRICAL PER NEC 2017



200 AMP ELECTRICAL RISER DIAGRAM

ELECTRICAL PLAN 1962 "B"

200 AMP SERVICE		
TAG	QUANTITY	PRODUCT
A	(37)	(FLUSH MOUNTED LT)
B	(3)	(VAPORS)
C	(5)	(PENDANT LIGHT
D	(X)	(10" MUSHROOMS)
E	(5)	(24" 3 LT)
F	(X)	(36" 4 LT)
G	(X)	(NOT USED)
H	(3)	(COACH LIGHTS)
I	(X)	(COACH LIGHTS)
J	(1)	(J BOX)
K	(4)	(4' FLUORESCENT)
L	(3)	(2' FLUORESCENT)
M	(X)	(5LT CHANDELIER)
N	(X)	(3 LT)
O	(X)	(PENDANT/ NOOK)
P	(X)	(X)
Q	(X)	(X)



ELECTRIC FLOOR PLAN

1/4" = 1'-0"

DESIGN IN ACCORDANCE WITH THE RESIDENTIAL FLORIDA BUILDING CODE 2020 - 7TH EDITION

D·R·HORTON® America's Builder

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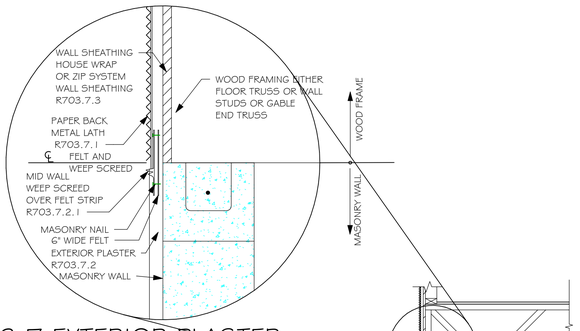
D.R.H. #: 5795801 1 1

MODEL # 1962 B

GCD JOB # 13514

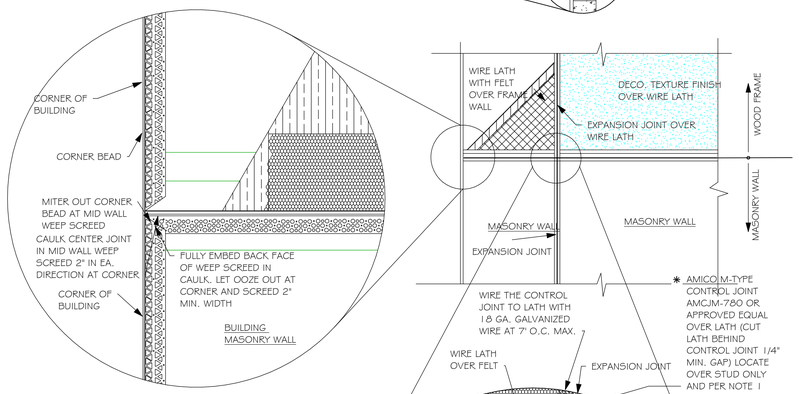
DATE: 10/4/21
DRAWN BY: CWL
CHECKED BY: JWC
REVISED:
PLAN: ELECTRICAL
SCALE: As indicated

A-5

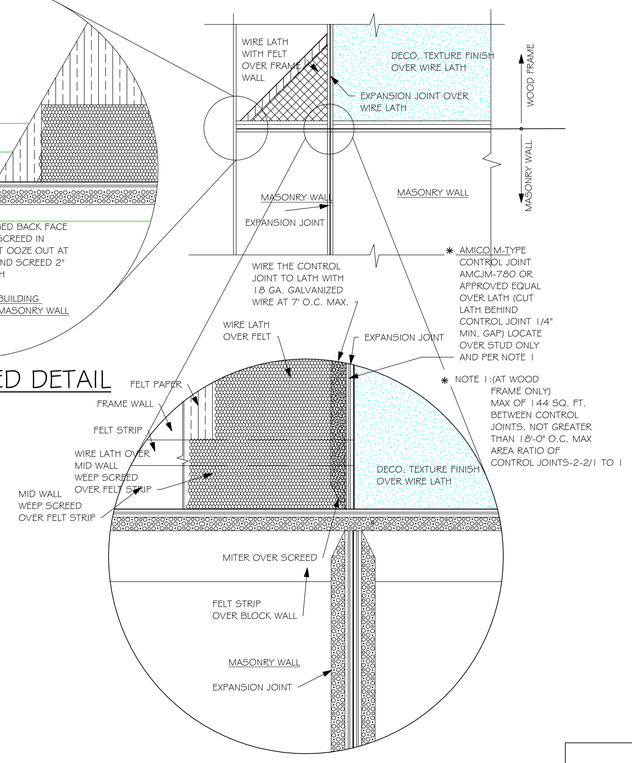


R703.7 EXTERIOR PLASTER

ASTM C926 AND ASTM C1063



MID WALL WEEP SCREED DETAIL



WEEP SCREED DETAIL

INSTALL AT ALL EXTERIOR WALL LOCATIONS WHERE WOOD STUD FRAMING IS ABOVE MASONRY WALLS.

1

RESIDENTIAL SPECIFICATIONS

GENERAL NOTES

1. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE JOB SITE PRIOR TO COMMENCING WORK. THE CONTRACTOR SHALL REPORT ALL DISCREPENCIES BETWEEN THE DRAWINGS AND EXISTING CONDITIONS TO THE DESIGNER PRIOR TO COMMENCING WORK.

2. THE CONTRACTOR SHALL SUPPLY, LOCATE AND BUILD INTO THE WORK ALL INSERTS, ANCHORS, ANGLES, PLATES, OPENINGS, SLEEVES, HANGERS, SLAB DEPRESSIONS AND PITCHES AS MAY BE REQUIRED TO ATTACH AND ACCOMMODATE OTHER WORK.

3. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUCTED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE IN THE WORK EXCEPT WHERE A DIFFERENT DETAIL IS SHOWN.

4. SUBSURFACE SOIL CONDITION INFORMATION IS NOT AVAILABLE FOUNDATIONS ARE DESIGNED FOR A SOIL BEARING CAPACITY OF 2,000 PSF. THE CONTRACTOR SHALL REPORT ANY DIFFERING CONDITIONS TO THE DESIGNER PRIOR TO COMMENCING WORK.

5. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUCTION WITH JOB SPECIFICATION AND HOUSE PLANS, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

6. ALL SPECIFIED FASTENERS MAY ONLY BE SUBSTITUTED IF APPROVED BY THE ENGINEER IN WRITING, THE INSTALLATION OF THE FASTENERS SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS. SIMPSON FASTENERS SPECIFIED MAY BE SUBSTITUTED WITH THE SAME QUANTITY AND EQUIVALENT STRENGTH PRODUCT.
ALL BOLTS, NUTS, WASHERS, STRAPS AND FASTENERS INCLUDING NAILS, SHALL BE HOT MOPED DIPPED GALVANIZED OR STAINLESS STEEL CONTINUOUS ANCHORAGE SHALL BE PROVIDED BETWEEN ALL TRUSSES, WALL SECTIONS, BEAMS, POSTS AND FOOTINGS WITH USE OF STRAPS AND CONNECTORS AS SPECIFIED HEREIN.

7. TREATED WOOD REQUIREMENTS:-
ALL TREATED WOOD EXPOSED TO WEATHER SHALL BE PROTECTED, PRESSURE TREATED, OR NATURALLY RESISTANT TO DECAY.
ALL WOOD TOUCHING MASONRY OR CONCRETE SHALL BE ISOLATED, OR PRESSURE TREATED.

8. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILTY TO DETERMINE ERECTION PROCEDURES AND SEQUENCES TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, OR TIE DOWNS.

9. CEILING DRYWALL INSTALLED WITHIN THE HOUSE TO TRUSSES SPACED 24" O.C. SHALL BE 5/8" DRYWALL OR 1/2" SAG RESISTANT PER SEC. 702.3.5

10. LANAI CEILINGS & COVERED ENTRY CEILINGS
1X4 STRIPPING @ 16" O.C. FASTENED WITH 2-8d NAILS TO EACH TRUSS. 5/8" EXTERIOR GYP. BOARD CEILING FASTENED WITH 8d NAILS OR 1-5/8" DRYWALL SCREWS @ 6" O.C. EDGE AND FIELD.

2

GENERAL ROOF ASSEMBLY

ROOF SHEATHING FBCR R803.2.2
SHALL BE 19/32 APA RATED SHEATHING, EXPOSURE 1, SPAN RATING 40/20 OR BETTER. INSTALL PANELS WITH LONG DIMENSION PLACED PERPENDICULAR TO TRUSSES. A 1/8" SPACE BETWEEN ADJACENT SHEETS SHALL BE MAINTAINED . INSTALL "H" CLIPS AT UNSUPPORTED PANEL EDGES. THE ROOF SHEATHING SHALL BE NAILED WITH 2 1/2" x 0.131 OR 3" x 0.120 RING SHANK NAILS @ 6" O.C. EDGE AND 6" O.C. FIELD. FOR WIND SPEED/EXPOSURE 160/B, 160/C AND 170/B. FOR 170/C, DECREASE NAIL SPACING TO 4" O.C. EDGE AND 4" O.C. FIELD. ENSURE THAT ALL NAILS PENETRATE THE TOP CHORD OF THE TRUSSES WITHOUT SPLITTING.

FLASHING
FLASHING SHALL BE ALUMINUM, ALUMINUM ZINC COATED STEEL 0.0179" THICK, 26 GAUGE AZ50 ALUM ZINC, OR GALVANIZED STEEL 0.0179" THICK, 26 GAUGE ZINC COATED G90. FLASHING SHALL BE INSTALLED IN ACCORDANCE WITH THE ZIP SYSTEM ROOF SHEATHING MANUFACTURES PUBLISHED REQUIREMENTS. ALL FLASHING AND INSTALLATION SHALL CONFORM TO SECTION R905.2.8 (1 TO 5).

DRIP EDGE
DRIP EDGE SHALL BE PROVIDED AT ALL EAVES AND GABLES OF SHINGLES ROOFS, LAPPED A MINIMUM OF 3" @ JOINTS. THE OUTSIDE EDGE SHALL EXTEND A MINIMUM OF 1/2" BELOW SHEATHING AND THE INSIDE EDGE SHALL EXTEND BACK A MINIMUM OF 2". DRIP EDGE SHALL BE FASTENED AT NO MORE THAN 4" CENTERS. THERE SHALL BE A MINIMUM OF 4" WIDTH OF ROOF CEMENT INSTALLED OVER THE DRIP EDGE FLANGE.

3

ASPHALT SHINGLE ROOF SPEC'S

SHINGLES
30# FELT SHALL BE INSTALLED UNDER ASPHALT SHINGLES. ALL ASPHALT SHINGLES SHALL HAVE SELF-SEALING STRIPS OR BE INTERLOCKING AND COMPLY WITH ASTM D 225 OR D 3462, AND SHALL BE SECURED TO THE ROOF WITH NO LESS THAN 6 FASTENERS PER SHINGLE STRIP, OR A MINIMUM OF 2 FASTENERS PER SHNGLE TAB, AND SHALL IN NO CASSE BE FASTENED WITH LESS FASTENERS THAN THAT REQUIRED BY THE MANUFACTURE. INSTALLATION SHALL COMPLY WITH MANUFACTURES REQUIREMENTS FOR INSTALLATION IN THE GIVEN FLORIDA WIND ZONE, AS DETERMINED BY ASTM D 3161.

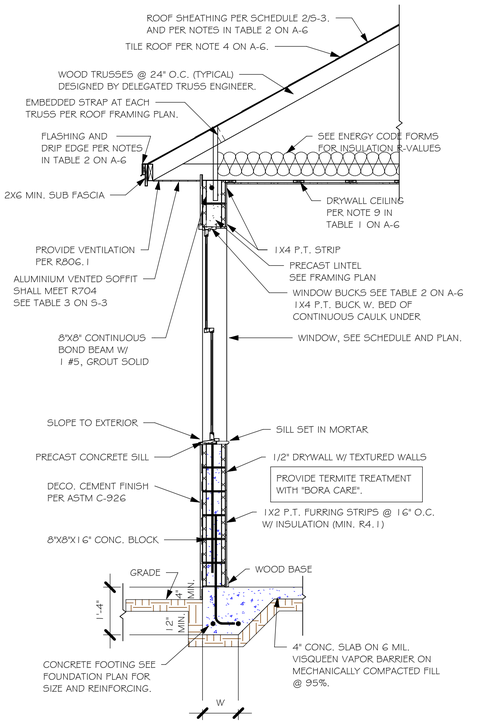
FASTENERS
FASTENERS FOR ASPHALT SHINGLES SHALL COMPLY WITH ASTM F 1667, AND SHALL BE MADE WITH GALVANIZED STEEL, STAINLESS STEEL OR ALUMINUM WITH A MINIMUM SHANK SIZE OF 12 GAUGE (0.105") WITH A MINIMUM 3/8" DIAMETER HEAD SHANK AND SHALL BE A LENGTH TO PENTRATE THE SHEATHING

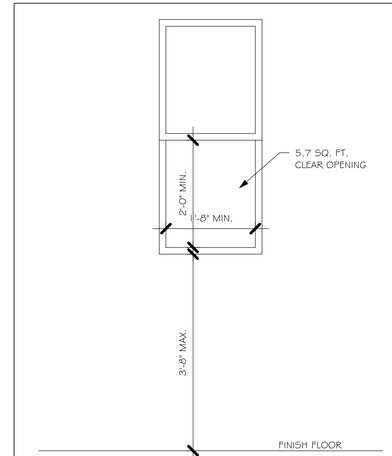
THE NAIL COMPONENT OF PLASTIC CAP NAILS SHALL MEET OR EXCEED THE REQUIREMENTS OF ASTM A 641, CLASS 1, OR EQUAL, AND SHALL BE CORROSION RESTITANT BY ELECTRO GALVANIZATION, MECHANICAL GALVANIZATION, HOT DIPPED GALVANIZATION OR SHALL BE MADE OF STAINLESS STEEL, NON-FERROUS METAL

4

CLAY AND CONCRETE ROOF TILE SPECS

INSTALL PEEL AND STICK UNDERLAYMENT APPROVED FOR SINGLE LAYER APPLICATION UNDER TILE ROOF.
THE INSTALLATION OF CLAY AND CONCRETE TILE SHALL COMPLY WITH THE PROVISIONS OF R905.3 F.B.C.
MARKING: EACH ROOF TILE SHALL HAVE A PERMANENT MANUFACTURER'S IDENTIFICATION MARK.
APPLICATION SPECIFICATIONS: THE TILE MANUFACTURER'S WRITTEN APPLICATION SPECIFICATIONS SHALL BE AVAILABLE AND SHALL INCLUDED BUT NOT BE LIMITED TO THE FOLLLOWING:
1. TILE PLACEMENT AND SPACING,
2. ATTACHMENT SYSTEM NECESSARY TO COMPLY WITH CURRENT WIND CODE,
A. AMOUNT AND PLACEMENT OF MORTAR.
B. AMOUNT AND PLACEMENT OF ADHESIVE
C. TYPE, NUMBER, SIZE AND LENGTH OF FASTENERS AND CLIPS.
3. UNDERLAYMENT
4. SLOPE REQUIREMENT.





R310.1.1 MINIMUM OPENING AREA- ALL EMERGENCY ESCAPE AND RESCUE OPENINGS SHALL HAVE A MINIMUM NET CLEAR OPENING OF 5.7 SQUARE FEET (0.530 m^2).

EXCEPTION- GRADE FLOOR OPENINGS SHALL HAVE A MINIMUM NET CLEAR OPENING OF 5 SQUARE FEET (0.465 m^2).

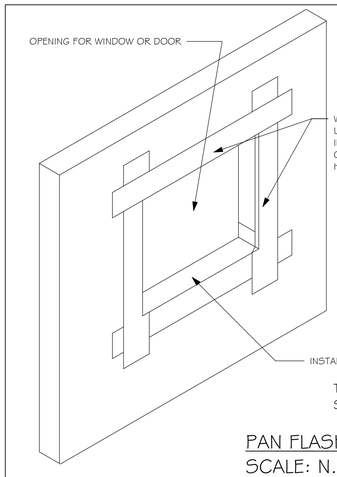
R310.1.2 MINMUM OPENING HEIGHT- THE MINIMUM NET CLEAR OPENING HEIGHT SHALL BE 24 INCHES (610mm).

R310.1.3 MINMUM OPENING WIDTH- THE MINIMUM NET CLEAR OPENING WIDTH SHALL BE 20 INCHES (508mm).

R310.1.4 OPERATIONAL CONSTRAINTS- EMERGENCY ESCAPE AND RESCUE OPENINGS SHALL BE OPERATIONAL FROM THE INSIDE OF THE ROOM WITHOUT THE USE OF KEYS OR TOOLS.

R310.2 WINDOW WELLS- THE MINIMUM HORIZONTAL AREA OF THE WINDOW WELL SHALL BE 9 SQUARE FEET (0.84 m^2), WITH A MINIMUM HORIZONTAL PROJECTION AND WIDTH OF 36 INCHES (914mm). THE AREA OF THE WINDOW WELL SHALL ALLOW THE EMERGENCY ESCAPE AND RESCUE OPENING TO BE FULLY OPENED.

MINIMUM EGRESS WINDOW DETAIL



R703.4 - WHERE FLASHING INSTRUCTIONS OR DETAILS ARE NOT PROVIDED BY THE WINDOW OR DOOR MANUFACTURER OR BY THE FLASHING MANUFACTURER, "PAN FLASHING" AHLL BE INSTALLED AT THE SILL OF EXTERIOR WINDOW AND DOOR OPENINGS. PAN FLASHING SHALL BE SEALED OR SLOPED IN SUCH A MANNER AS TO DIRECT WATER TO THE SURFACE OF THE EXTERIOR WALL FINISH OR TO THE WATER-RESISTIVE BARRIER FOR SUBSEQUENT DRAINAGE. OPENINGS USING PAN FLASHING SHALL ALSO INCORPORATE FLASHING OF PROTECTION AT THE HEAD AND SIDES.

"PAN FLASHING" IS A GENERIC TERM THAT USED TO REFER TO "METAL PAN FLASHING". HOWEVER MANY MODERN MATERIALS HAVE BEEN DEVELOPED FOR THE SAME FUNCTION SUCH AS:
- FLEXIBLE PEEL AND STICK FLASHING MEMBRANE
-FLUID APPLIED FLASHING
FOR SUCH PRODUCTS FOLLOW THE MANUFACTURER'S INSTALLATION REQUIREMENTS

FOR IN-DEPTH FLASHING INSTRUCTIONS, REFER TO THE FOLLOWING PUBLICATIONS:
FMA/AAMA 100
FMA/AAMA 200
FMA/WDMA 250
FMA/AAMA/WDMA 300

THE FLASHING INSTRUCTIONS FROM THE WINDOW/ DOOR MFR., OR THE FLASHING MFR., SHALL SUPERCEDE THIS DETAIL

PAN FLASHING PER R703.4

SCALE: N.T.S.

DESIGN IN ACCORDANCE WITH THE RESIDENTIAL FLORIDA BUILDING CODE 2020 - 7TH EDITION

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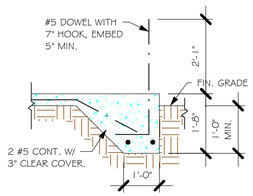
SUBDIVISION: TOSCANA III & IV 50s
ADDRS: 277 VINADIO BLVD
D.R.H. #: 5795800111

MODEL # 1962 B

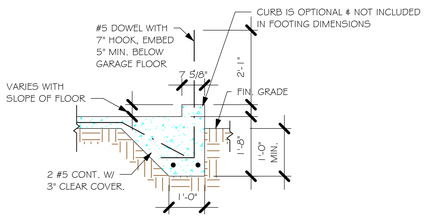
GCD JOB # 1351.4

DATE: 10/4/21
DRAWN BY: CWL
CHECKED BY: JWC
REVISED:
PLAN: SECTIONS
SCALE: As indicated

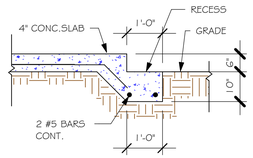
A-6



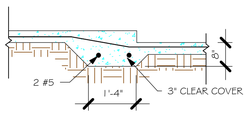
"F3" FOOTING
1/2" = 1'-0"



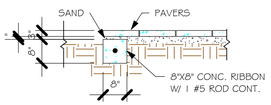
"F3" WITH CURB AT GARAGE
1/2" = 1'-0"



"GARAGE DOOR RECESS
1/2" = 1'-0"



"F6" STEP DOWN
1/2" = 1'-0"



"P" PAVERS DETAIL ENTRY/ LANAI
1/2" = 1'-0"

WALL FOOTING SCHEDULE

USED	TYPE	LENGTH	WIDTH	DEPTH	BOTTOM REINFORCING	SHAPE	
	F1	CONT.	1'-4"	0'-8"	2-#5		
	F2	CONT.	1'-8"	0'-10"	2-#5		
X	F3	CONT.	1'-0"	1'-8"	2-#5		ADD CURB TO GARAGE, SEE DETAIL
	F4	CONT.	1'-4"	1'-8"	2-#5		
	F5	CONT.	1'-4"	1'-0"	2-#5		
	F6	CONT.	1'-4"	1'-0"	2-#5		
X	F6A	CONT.	0'-8"	0'-8"	1-#5		
	TE	CONT.	0'-8"	0'-8"	1-#5		

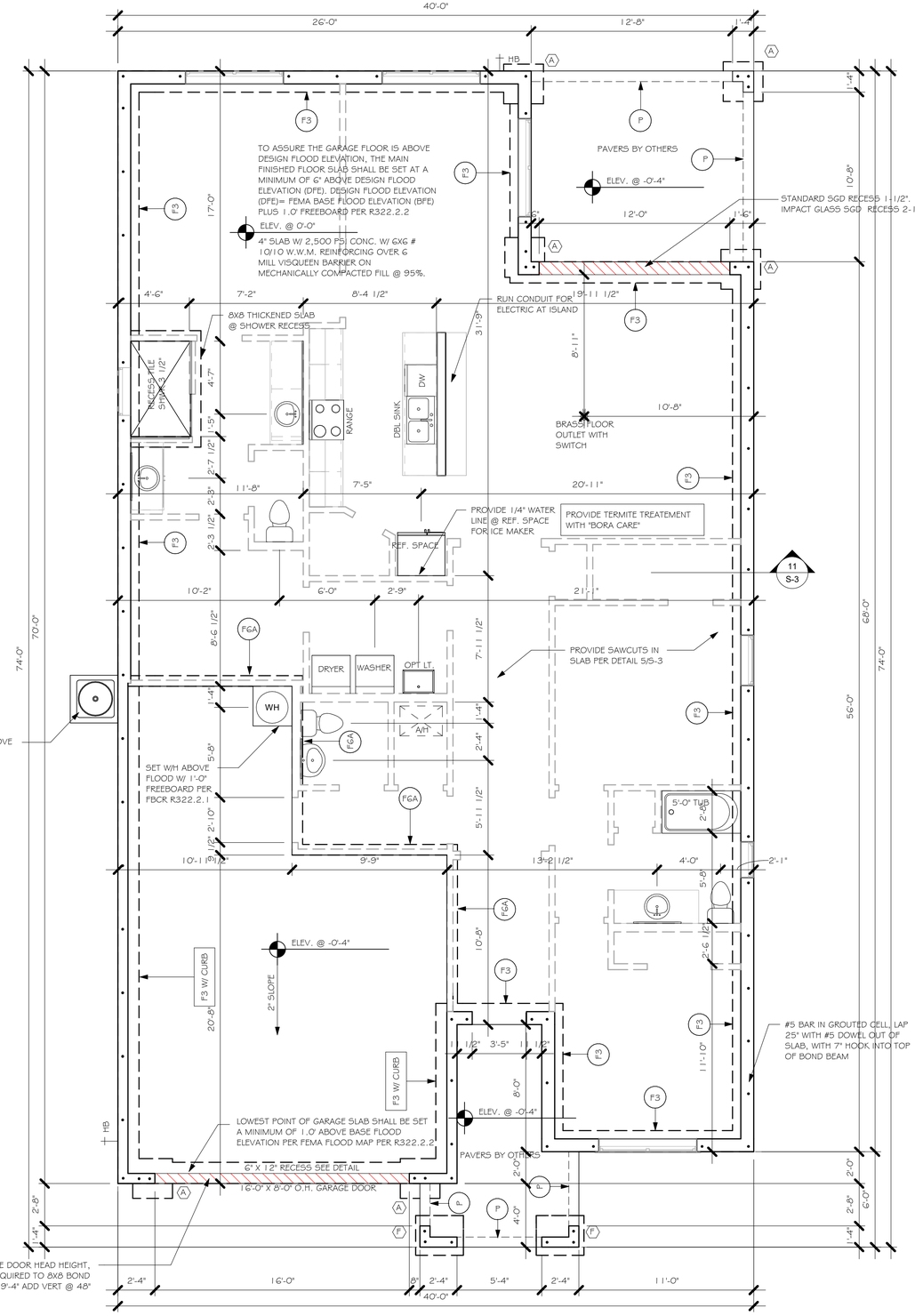
PROVIDE CORNER BARS PER 6/S-3

PAD FOOTING SCHEDULE

USED	TYPE	LENGTH	WIDTH	DEPTH	BOTTOM REINF. LONG WAY	BOTTOM REINF. SHORT WAY	REMARKS
X	Ⓐ	2'-6"	2'-6"	1'-0"	3-#5	3-#5	-
	Ⓑ	3'-0"	3'-0"	1'-0"	4-#5	4-#5	-
	Ⓒ	3'-6"	3'-6"	1'-0"	4-#5	4-#5	-
	Ⓓ	4'-0"	4'-0"	1'-2"	5-#5	5-#5	-
	Ⓔ	5'-0"	5'-0"	1'-2"	6-#5	6-#5	-
X	Ⓕ	3'-0"	2'-6"	1'-0"	3-#5	4-#5	-

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
PLAN NOTES:
1. **TOP OF GROUND FLOOR SLAB DATUM ELEVATION 0'-0"**
2. **"F#" DENOTES CONTINUOUS WALL FOOTING TYPE PER SCHEDULE THIS SHEET.**
3. **PROVIDE #5 VERTICAL REINFORCING AT DOT LOCATIONS SHOWN ON PLAN FROM FOOTING TO BOND BEAM.**
4. **ALL DIMENSIONS ARE TO OUTSIDE FACE OF MASONRY WALLS. SOME SLAB EDGES MAY EXTEND BEYOND FACE OF WALL.**
5. **FOR DIMENSIONS OF ROUGH OPENINGS IN MASONRY WALLS, COORDINATE WITH WINDOW/ DOOR SUPPLIER.**
6. **PROVIDE PRESSURE TREATED BUCKS AT WINDOWS/ DOORS PER DETAIL 7/S-3.**



FOUNDATION PLAN
1/4" = 1'-0"

DESIGN IN ACCORDANCE WITH THE RESIDENTIAL FLORIDA BUILDING CODE 2020 - 7TH EDITION

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RAUL REYES LICENSE No. 88925 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been digitally signed by Raul Reyes on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be validated on any electronic copies.

This signature and seal is for work performed by the Structural Engineer of Record only and does not include any responsibility for any work performed by others, other disciplines such as architectural, mechanical, plumbing, electrical, fire, life safety, accessibility, energy, site work, civil, or geotechnical.

STRUCTURAL SYSTEMS OF NORTH FLORIDA
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STRUCTURAL ENGINEERING

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SUBDIVISION: TOSCANA III & IV 50s
ADDRS: 277 VINADIO BLVD
D.R.H. #: 5795801 | 1

MODEL # 1962 B

GCD JOB # 1351 4

DATE: 10/4/21
DRAWN BY: CWL
CHECKED BY: JWC
REVISED:
PLAN: FOUNDATION PLAN
SCALE: As indicated

S-1

Y:\0-New Data\1-MASTER 2019\2019-BUILDERS\DR HORTON 2019\SUBDIVISIONS\TOSCANA ISLES 60'S\1351 4 LOT 699 1962 BLUREVIT\1351 4 1962 BL.rvt

	TRUSS STRAPPING TO MASONRY		
	MAX TRUSS UPLIFT (LBS)	STRAP/ANCHOR Valid lengths x/x/x	FASTENER
INSTALL META1G AT ALL TRUSSES TO 1450 lb UPLIFT. FOR HIGHER UPLIFTS, SEE NOTES ON PLAN.	1450 (1 PLY)	(1) META1G/18/20	(8) 0.148x1-1/2 ", EMBED 4"
	1810 (1 PLY)	(1) META1G/20	(9) 0.148x1-1/2", EMBED 4"
	1875 (1 PLY)	(2) META1G/18/20	(10) 0.148x1-1/2", EMBED 4"
	1920 (1 PLY)	(2) META1G/20	(10) 0.148x1-1/2", EMBED 4"
	2120 (1 PLY)	(2) MMETA1G/20	(10) 0.148x1-1/2", EMBED 4"
	1795 (2 OR 3 PLY)	(2) META1G/18/20	(14) 0.162x3-1/2", EMBED 4"
	2365 (2 OR 3 PLY)	(2) HETA1G/20	(12) 0.162x3-1/2", EMBED 4"
	3965 /DF /SP (2 PLY)	MGT	(22) 0.148x3" ATR, EPOXY 12"
	3000 /DF /SP (1 PLY 2x4)	HTT4	(18) 0.148x1-1/2", 5/8" ATR, EPOXY 12"
	4455 /DF /SP (1 PLY 2x6)	HTT4	(18) SD#10x1-1/2", 5/8" ATR, EPOXY 12"
	4235 /DF /SP (2 PLY 2x4)	HTT4	(18) 0.162x2-1/2", 5/8" ATR, EPOXY 12"
	4555 /DF /SP (1 PLY 2x6)	HTT5	(26) SD#10x1-1/2", 5/8" ATR, EPOXY 12"
	4670 /DF /SP (2 PLY 2x4)	HTT5	(26) 0.148x3", 5/8" ATR, EPOXY 12"
	5445 /DF /SP (2 PLY 2x4)	HTT5KT	(26) SD#10x2-1/2", 5/8" ATR, EPOXY 18"
	10690 /DF /SP (2 PLY)	(1)HGT - 2	(16) 0.148x3", (2) 3/4" ATR, EPOXY 12"
	10790 /SYP (3PLY)	(1)HGT - 3	(16) 0.148x3", (2) 3/4" ATR, EPOXY 12"

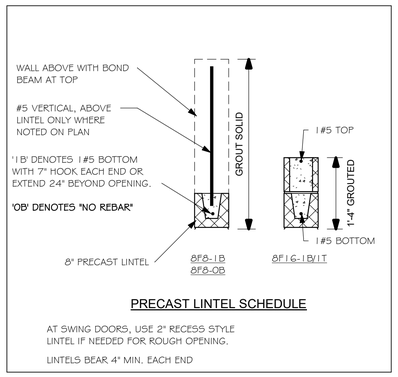
NOTES:
1. PROVIDE A STRAP FROM THE ABOVE LIST AT EACH ROOF TRUSS BEARING POINT, BASED ON THE TRUSS UPLIFT VALUES IN THE SIGNED AND SEALED TRUSS DESIGN PACKAGE AND SUITABLE FOR THE GEOMETRY. EMBED STRAP ON CENTERLINE OF WALL.
2. ANY OF THE VALID LENGTHS SHOWN MAY BE USED IN PLACE OF THE LENGTH SPECIFIED ON PLAN.
3. CONNECTORS ARE SIMPSON STRONG TIE. ALL CONNECTORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH SIMPSON PRINTED INSTUCTIONS. SUBSTITUTIONS MUST BE APPROVED IN WRITING BY THE ENGINEER OF RECORD.
4. WHERE EMBEDDED STRAPS ARE MISSING, OR MIS-LOCATED, INSTALL RETROFIT STRAP PER 10/S-3. PER UPLIFT IN TRUSS ENGINEERING.

SIMPSON CATALOG C-C- 2019

	TRUSS STRAPPING TO STUDWALL/ WOOD BEAM		
	MAX TRUSS UPLIFT (LBS)	STRAP(S) Valid lengths x/x/x	FASTENER
INSTALL AT ALL TRUSSES TO 850 lb UPLIFT. FOR HIGHER UPLIFTS, SEE NOTES ON PLAN.	850	(1) MTS1G/20/30	(14) 0.148x1-1/2" or 3" EACH STRAP
	1700	(2) MTS1G/20/30	
	2550	(3) MTS1G/20/30	
	1125	(1) HTS20/24/30	(24) 0.148x1-1/2" OR (20) 0.148x3" EACH STRAP
	2250	(2) HTS20/24/30	
	3375	(3) HTS20/24/30	
	4500	(4) HTS20/24/30	

NOTES:
1. PROVIDE A STRAP FROM THE ABOVE LIST AT EACH ROOF TRUSS BEARING POINT, BASED ON THE TRUSS UPLIFT VALUES IN THE SIGNED AND SEALED TRUSS DESIGN PACKAGE.
2. ANY OF THE VALID LENGTHS SHOWN MAY BE USED IN PLACE OF THE LENGTH SPECIFIED ON PLAN.
3. 1-1/2" NAIL SHALL BE USED IN 1 PLY LUMBER, 2 PLY LUMBER IS REQUIRED FOR 3" NAILS.
4. CONNECTORS ARE SIMPSON STRONG TIE. ALL CONNECTORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH SIMPSON PRINTED INSTUCTIONS.

SIMPSON CATALOG C-C- 2019



PRECAST LINTEL SCHEDULE

AT SWING DOORS, USE 2" RECESS STYLE LINTEL IF NEEDED FOR ROUGH OPENING.

LINTELS BEAR 4" MIN. EACH END

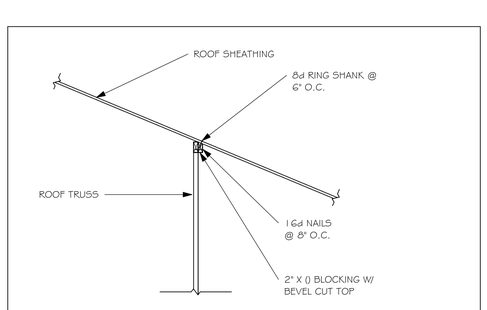
PLAN NOTES:

1. ROOF AND FLOOR TRUSS BEARING ELEVATION VARIES, SEE LEGEND.
2. ROOF AND FLOOR FRAMING SHALL BE WOOD TRUSSES DESIGNED BYA DELEGATED TRUSS ENGINEER PER DESIGN CRITERIA ON SHEET S-3.
3. PROVIDE STRAPPING AT TRUSSES PER NOTES ON THIS SHEET.
4. FOR NAILING OF ROOF AND FLOOR DECK, SEE 1 AND 2 ON S-3.
5. 8F8-1B, etc., DENOTES PRECAST LINTEL ABOVE DOOR/WINDOW OPENING PER SCHEDULE THIS SHEET.
6. AT TRUSS BEARING, PROVIDE 8x8 MASONRY BOND BEAM W/ 1 #5 CONTINUOUS, SEE DETAIL 11/S-3.
7. "SW" DENOTES PLYWOOD SHEARWALL PER SCHEDULE THIS SHEET.

BEARING HEIGHT

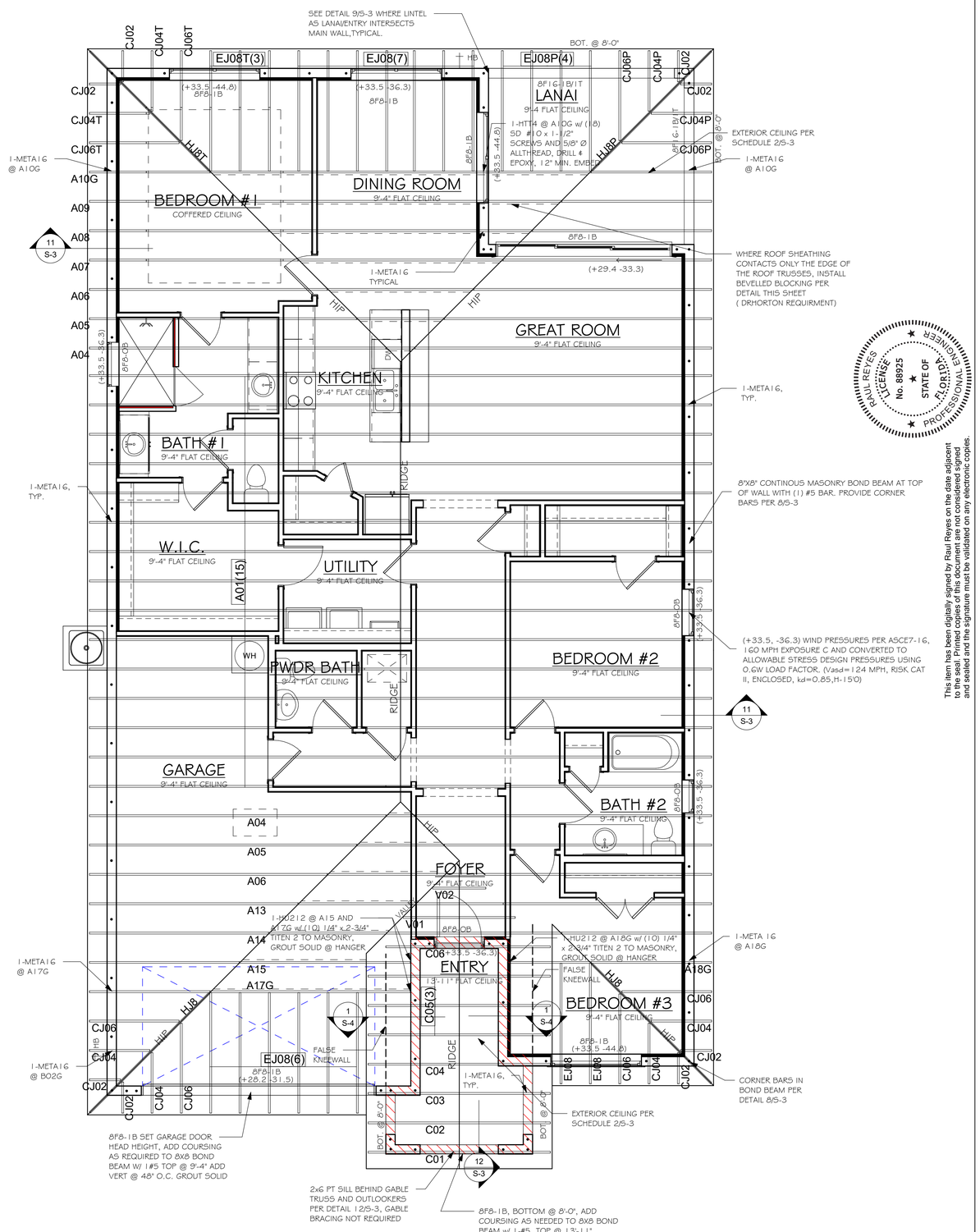
= BEARING @ 9'-4"

= BEARING @ 13'-11"



BEVELLED BLOCKING DETAIL

TRUSS BEARING CONDITIONS AND STRAPPING IS BASED ON TRUSS LAYOUT PREPARED BY AMERICAN BUILDER SUPPLY JOB# M20016223-20BX DATED 12/07/20 REVISED: NONE



ROOF FRAMING PLAN
1/4" = 1'-0"

DESIGN IN ACCORDANCE WITH THE RESIDENTIAL FLORIDA BUILDING CODE 2020 - 7TH EDITION

D·R·HORTON® America's Builder

Gulf Coast Drafting & Design, Inc.
EMAIL: PLANS@GULFCOASTDRAFTING.COM
PHONE: 239-540-1822
1515 SE 47TH ST. CAPE CORAL, FL 33904

STRUCTURAL SYSTEMS OF NORTH FLORIDA
1034 SE 47th ST SUITE #3
CAPE CORAL, FLORIDA
(239) 549-4554
CA 8859

RAUL REYES LICENSE No. 88925 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been digitally signed by Raul Reyes on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be validated on any electronic copies.

LOT: 699
SUBDIVISION: TOSCANA III # IV 50s
ADDRS: 277 VINADIO BLVD
D.R.H. #: 579580111

MODEL # 1962 B

GCD JOB # 13514

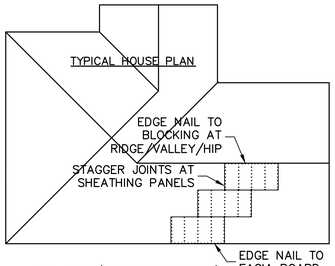
DATE: 10/4/21
DRAWN BY: CWL
CHECKED BY: JWC
REVISED:
PLAN: ROOF FRAMING PLAN
SCALE: As indicated

S-2

L:\0-New Data\1-MASTER 2019\2019-BUILDERS\DR HORTON 2019\SUBDIVISIONS\TOSCANA 15LE5 605\13514 LOT 699 1962 B\REVIT\13514 1962 BL.rvt

TABLE R803.2.3.1 – NAIL SPACING BASED ON SPECIFIC GRAVITY OF RAFTER/TRUSS: ALL TRUSS TOP CHORDS AND FIELD ROOF FRAMING SHALL BE SOUTHERN PINE, SPECIFIC GRAVITY=0.55 (EXCEEDS SG=0.42 AND 0.49 OF TABLE R803.2.3.1).

ENSURE THAT ALL NAILS PENETRATE THE TOP CHORD OF THE TRUSS WITHOUT SPLITTING.



NAIL SPACING (TABLE R803.2.3.1) WIND SPEED / EXPOSURE

160/B, 160/C, 170/B	170/C
NAIL SPACING: 6" O.C. EDGE 6" O.C. FIELD	NAIL SPACING: 4" O.C. EDGE 4" O.C. FIELD

NAIL TYPE (SECTION R803.2.3.1) 19/32 SHEATHING

2½" x 0.131" RING SHANK OR 3" x 0.120" RING SHANK (PER ASTM F1667 RSRS–03 & 04)

1 NAILING OF ROOF SHEATHING
SCALE: NTS

SHEATHING SCHEDULE

EXTERIOR STUD WALL	FLOOR
7/16" ZIP SYSTEM WALL SHEATHING BY HUBER ENGINEERED WOODS LLC, NAILED W/ 8d COMMON WIRE @ 6" O.C. EDGE AND 6" O.C. FIELD. PROVIDE 2x4 BLOCKING AT ALL JOINTS. INSTALL SHEATHING AND SEAM TAPE IN STRICT ACCORDANCE WITH MFR. WRITTEN INSTRUCTIONS.	N/A
ROOF – PER FBCR TABLE 803.2.2	**EXTERIOR CEILING**
19/32 CLASS A.P.A. RATED SHEATHING, EXPOSURE 1, SPAN RATING 40/20. FASTEN WITH RING SHANK NAILS PER DETAIL 1/S–3 (WHEN ZIP BRAND ROOF SHEATHING IS USED, H–CLIPS ARE NOT REQUIRED)	1) 1x4 STRIPPING @ 16"OC w/ 2–8d NAILS TO EACH TRUSS, 5/8" EXTERIOR GYPBOARD CEILING, FASTEN w/8d NAILS OR 1 5/8" DRYWALL SCREWS @ 6"OC EDGE & FIELD. 2) 3/8" BC PLYWOOD NAILED w/ 6d COMMON @ 6" OC EDGE & FIELD.
	SOFFIT
	ALUMINUM PERFORATED SOFFIT INSTALLED PER MANUFACTURER INSTRUCTIONS TO MEET WIND PRESSURES PER R704.

2
NOTE: EXTERIOR CEILINGS SPECIFIED ABOVE MEET THE DESIGN WIND PRESSURES PER R703.1.2

WINDOW/DOOR/SOFFIT DESIGN WIND PRESSURES

WIND PRESSURES PER ASCE7–16, 160 MPH, EXPOSURE C, AND CONVERTED TO ALLOWABLE STRESS DESIGN PRESSURES USING 0.6W LOAD FACTOR. (Vasd=124 MPH, RISK CAT II, ENCLOSED, kd=0.85, h=15')

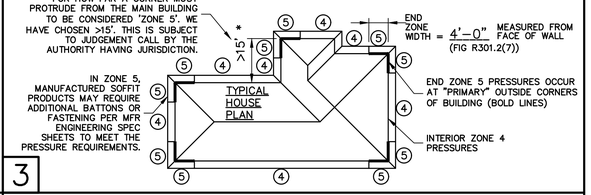
TYPE	INTERIOR ZONE 4	END ZONE 5
SOFFIT (10 SQ. FT.)	+33.5 –36.3	+33.5 –44.8
WINDOWS & DOORS (10 SQ. FT.)	+33.5 –36.3	+33.5 –44.8
8' OR 9' GARAGE DOORS	+29.4 –33.3	
16' OR 18' GARAGE DOORS	+28.2 –31.5	

(SEE PLAN FOR OTHER SPECIFIC PRESSURES)

1) TABLE MAY BE USED FOR ANY SIZE WINDOW OR DOOR IN EACH TYPE.
2) USE "INTERIOR ZONE 4" PRESSURES UNLESS WINDOW OR DOOR IS LOCATED WITHIN THE "END ZONE 5" (SEE DIAGRAM BELOW), THEN USE THE HIGHER PRESSURES UNDER THE "END ZONE 5" COLUMN.
3) ALL GLASS / GLAZING SHALL BE IMPACT RATED OR USE IMPACT RATED SHUTTERS.
4) SUBMIT PRODUCT APPROVALS TO THE BUILDING DEPARTMENT AS REQUIRED BY THE LOCAL JURISDICTION.
5) MANUFACTURED SOFFIT PRODUCTS SHALL BE INSTALLED PER MFR ENGINEERING SPEC SHEETS.

* ON IRREGULAR SHAPED BUILDINGS, THERE IS NO GUIDANCE IN THE CODE FOR HOW FAR A CORNER MUST PROTRUDE FROM THE MAIN BUILDING TO BE CONSIDERED 'ZONE 5'. WE HAVE CHOSEN >15'. THIS IS SUBJECT TO JUDGEMENT CALL BY THE AUTHORITY HAVING JURISDICTION.

IN ZONE 5, MANUFACTURED SOFFIT PRODUCTS MAY REQUIRE ADDITIONAL BATTONS OR FASTENING PER MFR ENGINEERING SPEC SHEETS TO MEET THE PRESSURE REQUIREMENTS.



3

DESIGN CRITERIA:

STRUCTURAL ENGINEERING PER:
FLORIDA BUILDING CODE 7th EDITION (2020) RESIDENTIAL
Occupancy: FBC 310.5 Residential Group R–3
Construction Type: V–B (fire resistance rating 0 hours, not sprinkled)

Codes to be used by other design professionals and licensed contractors:
2020 Florida Building Code, 7th Edition; Residential; Accessibility;
Energy Conservation; Plumbing; Mechanical; and Fuel Gas.
Electrical is contained by reference within FBC Residential Chapter 34:
NFPA 70–17 National Electrical Code.

1. FLOOR & ROOF UNIFORM LOADS:
   ELEVATED FLOORS: LIVE LOAD 40 PSF, DEAD LOAD 20 PSF
   ROOF: LIVE TOP CHORD 20 PSF
   LIVE BOTTOM CHORD 10 PSF (NON–CONCURRENT w/ TCLL)
   CEMENT ROOF TILE DEAD LOAD 25 PSF TOTAL
   SHINGLE/METAL ROOFING DEAD LOAD 15 PSF TOTAL
   MINIMUM DEAD LOAD FOR WIND: TC 5 PSF, BC 5 PSF
   DEFLECTION CRITERIA:
   FLOOR L/480 LIVE, L/360 TOTAL
   ROOF L/240 LIVE, L/180 TOTAL

2. WIND DESIGN PER ASCE7–16
   BASIC WIND SPEED (ASCE7–16) 160 MPH
   NOMINAL WIND SPEED (Vasd TABLE R301.2.1.3) 124 MPH
   BUILDING CATEGORY II
   IMPORTANCE FACTOR 1.00
   EXPOSURE C
   MEAN ROOF HEIGHT = 15 FT
   HEIGHT & EXPOSURE COEFFICIENT λ TABLE R301.2(3) = 1.21
   ROOF PITCH 5/12
   ENCLOSURE CLASS. ENCLOSED
   INTERNAL PRES. COEFF. +/– 0.18
   WINDOW/DOOR DESIGN WIND PRESSURE PER TABLE R301.2(2), R301.2(3) AND R301.2(4), SEE DETAIL 3/S–3.
   SOFFITS – PER R704, ALL SOFFITS & THEIR ATTACHMENTS SHALL BE CAPABLE OF RESISTING THE DESIGN PRESSURES SPECIFIED IN TABLE R301.2(2) FOR WALLS USING 10 SQ. FT

3. REINFORCED CONCRETE: DESIGN PER ACI 318–14
   REQUIRED COMPRESSIVE STRENGTH AT 28 DAYS:
   SLAB ON GRADE f'c = 2500 PSI
   3½" MINIMUM THICKNESS REINFORCED WITH 6x6 w1.4xw1.4 WWF OR FIBERMESH.
   CONVENTIONAL SHALLOW FOOTINGS f'c = 2500 PSI
   BEAMS AND COLUMNS f'c = 3000 PSI
   ALL OTHER CONCRETE (U.N.O.) f'c = 3000 PSI
   UNLESS OTHERWISE SHOWN ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:
   FOOTINGS 3"
   SLAB ON GRADE CENTERED
   BEAMS 1½"
   COLUMNS 1½"
   ALL REINFORCING STEEL SHALL BE PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAMS AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. ALL REINFORCING STEEL SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE.
   REINFORCING STEEL – ASTM A615 GRADE 40 FOR #3
   GRADE 60 FOR #4 TO #11
   WELDED WIRE FABRIC – ASTM A185
   SPLICES IN REINFORCING, SHALL BE 40 BAR DIAMETERS. NON–CONTACT LAP SPLICES MAY BE USED PROVIDED REINFORCING IS NOT SPACED MORE THAN 5" APART FOR #5 BARS.
   FORMWORK AND SHORING SHALL REMAIN IN PLACE UNTIL CONCRETE HAS REACHED AT LEAST 2/3 OF THE REQUIRED 28 DAY STRENGTH.

4. REINFORCED MASONRY: DESIGN PER TMS 402/602–16
   REQUIRED COMPRESSIVE STRENGTHS:
   MASONRY WALLS f'm = 1500 PSI
   REINFORCING STEEL – ASTM A615 GRADE 60.
   SPLICES IN REINFORCING, SHALL BE 48 BAR DIAMETERS.
   ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90, GRADE N–1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE 'S' MORTAR. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. ALL CELLS BELOW FINISHED GRADE SHALL BE GROUTED SOLID. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT AT DOT LOCATIONS ON PLAN.

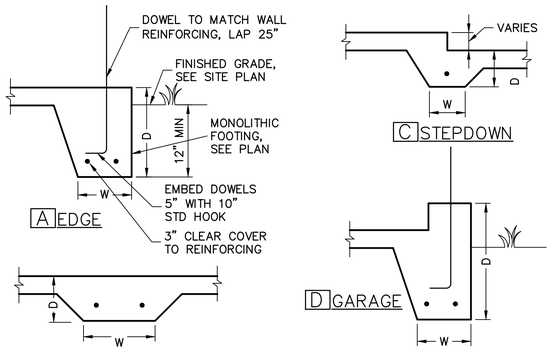
5. DELEGATED–ENGINEERED WOOD ROOF & FLOOR TRUSSES:
   ALL WOOD ROOF AND FLOOR TRUSSES SHALL BE DESIGNED BY A DELEGATED TRUSS ENGINEER PER RULE 61G15–31.003 OF THE FLORIDA ADMINISTRATIVE CODE. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB–91." FOR OTHER BRACING REQUIREMENTS, NOTIFY ENGINEER. PROVIDE PERMANENT BRACING PER TRUSS MFR. SHOP DRAWINGS. IF PERMANENT BRACING IS NOT SPECIFIED, CONTACT ENGINEER.

6. FOUNDATION: CONVENTIONAL SHALLOW CONCRETE FOOTINGS
   SOIL BEARING CAPACITY 2000 PSF
   THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SOIL CONDITIONS FOR THE INTENDED STRUCTURE AND ASSUMED SOIL BEARING CAPACITY. IT IS RECOMMENDED THAT A GEOTECHNICAL FIRM BE HIRED TO PERFORM A SITE EVALUATION.

7. DIMENSIONS: VERIFY ALL DIMENSIONS WITH HOUSE PLANS. SEE HOUSE PLANS, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. WHICH ARE NOT SHOWN ON STRUCTURAL DRAWINGS.

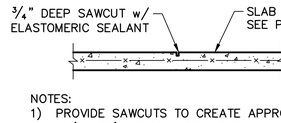
8. MEANS AND METHODS: THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO SUPPORT STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE OF ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

9. SHOP DRAWINGS: SHOP DRAWINGS SHALL BE PREPARED AND SUBMITTED TO THE ENGINEER FOR REVIEW FOR ALL STRUCTURAL ELEMENTS UTILIZING PREFABRICATED COMPONENTS. ONE SET OF SIGNED & SEALED TRUSS ENGINEERING SHALL BE DELIVERED TO THE ENGINEER OF RECORD FOR THE STRUCTURE PER FLORIDA ADMINISTRATIVE CODE 61G15–30.005 AND 61G15–31.003.



MONOLITHIC FOOTINGS
SCALE: 3/4" = 1'–0"

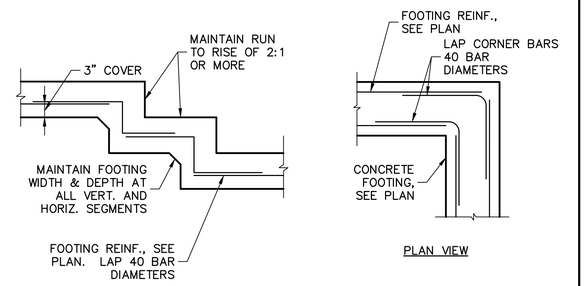
4



NOTES:
1) PROVIDE SAWCUTS TO CREATE APPROXIMATE 20' X 20' MAXIMUM SQUARES.
2) SAWCUT CONCRETE SLAB WITHIN 4 TO 12 HOURS OF CONCRETE PLACEMENT.

SLAB SAWCUT DETAIL
SCALE: NTS

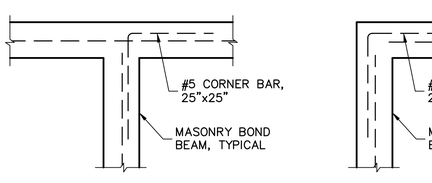
5



STEP FOOTING
SCALE: NTS

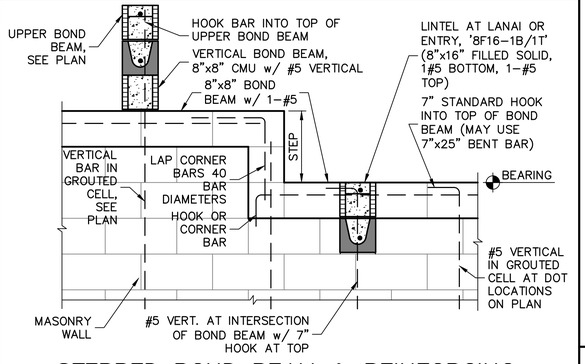
FOOTING CORNER BARS
SCALE: NTS

6



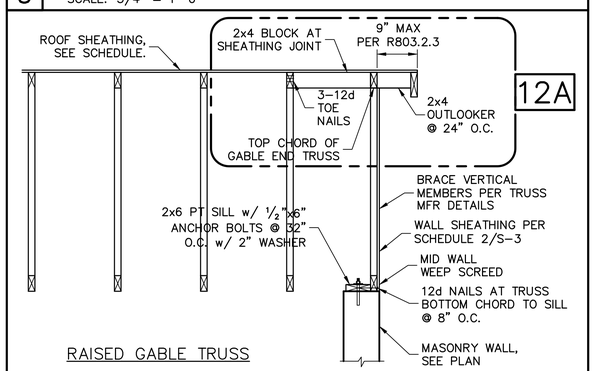
CORNER BAR DETAIL IN BOND BEAMS
SCALE: 3/4" = 1'–0"

8



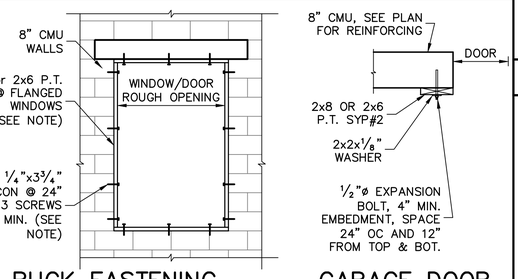
STEPPED BOND BEAM & REINFORCING
SCALE: 3/4" = 1'–0"

9



GABLE END DETAIL
SCALE: N.T.S.

12A



BUCK FASTENING

GARAGE DOOR

NOTE: THIS BUCK FASTENING DETAIL IS INTENDED FOR FLANGED WINDOW/DOOR PRODUCTS THAT FASTEN THRU THE FLANGE WITH WOOD SCREWS TO THE BUCK. FOR WINDOW/DOOR PRODUCTS THAT DO NOT HAVE A FLANGE AND FASTEN INSTEAD OUTWARD THRU THE FRAME, USE MASONRY SCREWS PER MFR. THAT ARE LONG ENOUGH TO PENETRATE 2–1/4" INTO THE MASONRY. IN THIS CASE, THE BUCK MATERIAL IS SIMPLY A SPACER AND MAY BE 1x4 OR 1x6 OR OMITTED ENTIRELY AND THE SPACER MAY BE TACKED IN PLACE WITH MASONRY NAILS OR PINS.

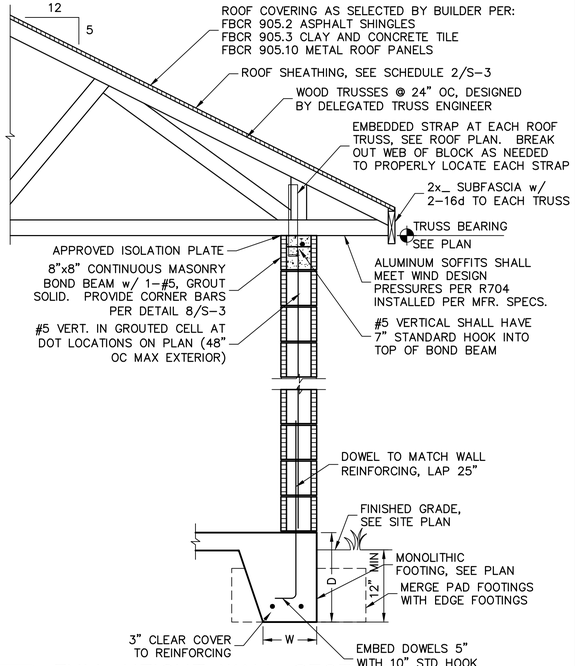
7

RETROFIT STRAPS TO CONCRETE/MASONRY

TRUSS UPLIFT (LBS) @ 24" OC	CONNECTOR	
TO 840	1–MTSM16 or 20	7–10dx1½"; 4–¼x2¼ TITEN
TO 1045	1–HTSM16 or 20	8–10dx1½"; 4–¼x2¼ TITEN
TO 2090	2–HTSM16 or 20	8–10dx1½"; 4–¼x2¼ TITEN
TO 4300	2–LGT2	16–16d, 7–¼"x2¼" TITEN
TO 3480	HTT16	18–16d, ⅝"ø ALLTHREAD, DRILL & EPOXY 10" EMBED w/ SIMPSON SET.
TO 10530	HGT–2/3	TWO ¾"ø ALTHREAD, DRILL & EPOXY 12" EMBED WITH SIMPSON SET.

NOTES:
1) WHERE EMBEDDED STRAP IS MISSING OR MIS–LOCATED, PROVIDE A STRAP FROM THE ABOVE LIST AT EACH ROOF TRUSS BEARING POINT, BASED ON THE TRUSS UPLIFT VALUES IN THE SIGNED AND SEALED TRUSS DESIGN PACKAGE.
2) CONNECTORS ARE SIMPSON STRONG TIE. ALL CONNECTORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH SIMPSON PRINTED INSTRUCTIONS.

10 RETROFIT UPLIFT CONNECTOR SCHEDULE



FULL HEIGHT WALL SECTION
SCALE: 3/4" = 1'–0"

11

FBC R703.7 EXTERIOR PLASTER
ASTM C926 AND ASTM C1063

THE CODE SECTIONS REFERENCED BELOW ARE FOR SUMMARY PURPOSES. SEE THE FLORIDA BUILDING CODE AND THE ASTM STANDARDS FOR FULL DESCRIPTIONS AND REQUIREMENTS.

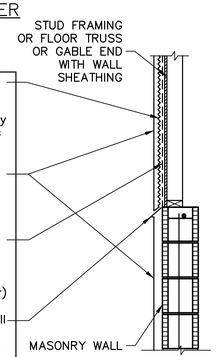
R703.7.1 Lath; Where required by the wall framing type, install metal lath per ASTM C1063 or non metallic lath per ASTM C1787. Use self furring lath as required by the ASTM spec. Use paper backed lath as required per Water Resistive Barrier specs.

R703.7.2 Plaster; Install portland cement based plaster and number of coats per ASTM C926 and thickness per Table R702.1(1).

R703.7.3 Water Resistive Barriers; Install water resistive barriers per R703.2 and water resistive vapor–permeable barrier over stud walls. (Note: ZIP wall sheathing with seam tape qualifies as the first layer)

R703.7.2.1 Weep Screed; Weep screed shall be installed at the bottom edge of all exterior wood stud framed walls (including gable end trusses) receiving lath and plaster.

Note: Exterior Stud Walls includes Gable End Trusses or Floor Trusses with Wall Sheathing.



EXTERIOR WALL WITH PLASTER
THICKNESSES ARE EXAGGERATED FOR DRAWING PURPOSES

REVISIONS	BY

STRUCTURAL ENGINEERING:
STRUCTURAL SYSTEMS OF NORTH FLORIDA
1634 S.E. 47th STREET, SUITE #3
CAPE CORAL, FL 33904
(239) 549–4554
CA# 8829

This item has been digitally signed by Raul Reyes on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be validated on any electronic copies.

RAUL REYES LICENSE No. 88925 STATE OF FLORIDA PROFESSIONAL ENGINEER

DESIGNED IN ACCORDANCE WITH FLORIDA BUILDING CODE 7th EDITION (2020) RESIDENTIAL

BUILDER: D·R HORTON America's Builder

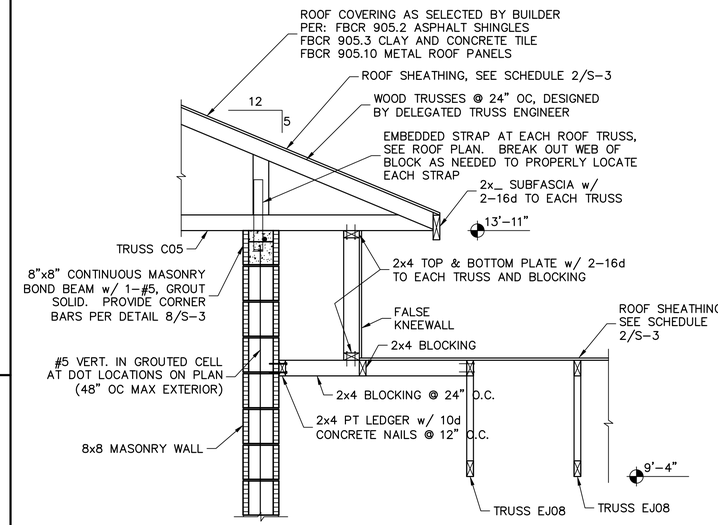
FOR AMERICAN BUILDERS SUPPLY TRUSSES, MODEL 1962, ELEVATION B & F, JOB # M2001623–20BX, DATED: 12/07/20, REVISED: 07/06/21

STRUCTURAL DETAILS MODEL 1962 B

277 VINADIO BLVD.
VENICE, FLORIDA 34275
LOT: 699 SUBDIVISION: TOSCANA

DESIGN/DRAWN	DWB/RR
CHECKED	DWB
DATE	10/06/21
SCALE	VARIES
JOB NO.	DR 13514
SHEET	S–3

SHEET 3 OF 4



1 FALSE KNEEWALL AT ENTRY
SCALE: 3/4" = 1'-0"

THIS DETAIL ONLY USED
FOR ELEVATIONS B & F

FOR AMERICAN BUILDERS SUPPLY TRUSSES, MODEL 1962, ELEVATION B & F, JOB # M2001623-20BX, DATED: 12/07/20, REVISED: 07/06/21

DESIGNED IN ACCORDANCE WITH FLORIDA BUILDING CODE 7th EDITION (2020) RESIDENTIAL

REVISIONS	BY

STRUCTURAL ENGINEERING:
STRUCTURAL SYSTEMS OF NORTH FLORIDA
1634 S.E. 47th STREET, SUITE #3
CAPE CORAL, FL 33904
(239) 549-4554
CA# 8829

RAUL REYES LICENSE No. 88925 STATE OF FLORIDA PROFESSIONAL ENGINEER

BUILDER:

D·R·HORTON America's Builder

STRUCTURAL DETAILS
MODEL 1962 B

277 VINADIO BLVD.
VENICE, FLORIDA, 34275
LOT: 699 SUBDIVISION: TOSCANA

DESIGN/DRAWN	DWB/RR
CHECKED	DWB
DATE	10/06/21
SCALE	VARIES
JOB NO.	DR 13514
SHEET	S-4

SHEET 4 OF 4

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: M2001623-20BX - 1962-Roof

MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: D.R. Horton- Ft. Myers Project Name: - Model: 1962 B,F 160 C TILE/SHINGLE
Lot/Block: - Subdivision: -
Address: -, -
City: - State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Design Program: MiTek 20/20 8.4
Wind Code: ASCE 7-16 Wind Speed: 160 mph
Roof Load: 50.0 psf Floor Load: N/A psf

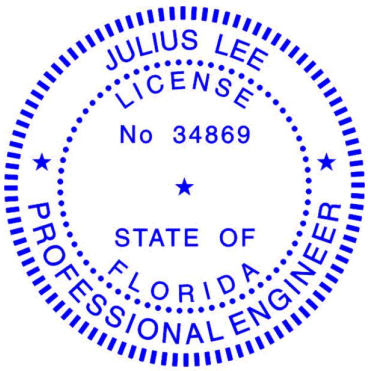
This package includes 34 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T24659402	A01	7/13/21	15	T24659416	C02	7/13/21
2	T24659403	A04	7/13/21	16	T24659417	C03	7/13/21
3	T24659404	A05	7/13/21	17	T24659418	C04	7/13/21
4	T24659405	A06	7/13/21	18	T24659419	C05	7/13/21
5	T24659406	A07	7/13/21	19	T24659420	C06	7/13/21
6	T24659407	A08	7/13/21	20	T24659421	CJ02	7/13/21
7	T24659408	A09	7/13/21	21	T24659422	CJ04	7/13/21
8	T24659409	A10G	7/13/21	22	T24659423	CJ04P	7/13/21
9	T24659410	A13	7/13/21	23	T24659424	CJ04T	7/13/21
10	T24659411	A14	7/13/21	24	T24659425	CJ06	7/13/21
11	T24659412	A15	7/13/21	25	T24659426	CJ06P	7/13/21
12	T24659413	A17G	7/13/21	26	T24659427	CJ06T	7/13/21
13	T24659414	A18G	7/13/21	27	T24659428	EJ08	7/13/21
14	T24659415	C01	7/13/21	28	T24659429	EJ08P	7/13/21

This item has been electronically signed and sealed by Lee, Julius, PE using a Digital Signature.
Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by American Builders Supply, Inc..

Truss Design Engineer's Name: Lee, Julius
My license renewal date for the state of Florida is February 28, 2023.



Julius Lee PE No.34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: July 13,2021

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

RE: M2001623-20BX - 1962-Roof

MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: D.R. Horton- Ft. Myers Project Name: - Model: 1962 B,F 160 C TILE/SHINGLE
Lot/Block: - Subdivision: -
Address: -, -
City: - State: FL

No.	Seal#	Truss Name	Date
29	T24659430	EJ08T	7/13/21
30	T24659431	HJ8	7/13/21
31	T24659432	HJ8P	7/13/21
32	T24659433	HJ8T	7/13/21
33	T24659434	V01	7/13/21
34	T24659435	V02	7/13/21

Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	A01	Common	15	1	Job Reference (optional)	T24659402

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:37 Page: 1
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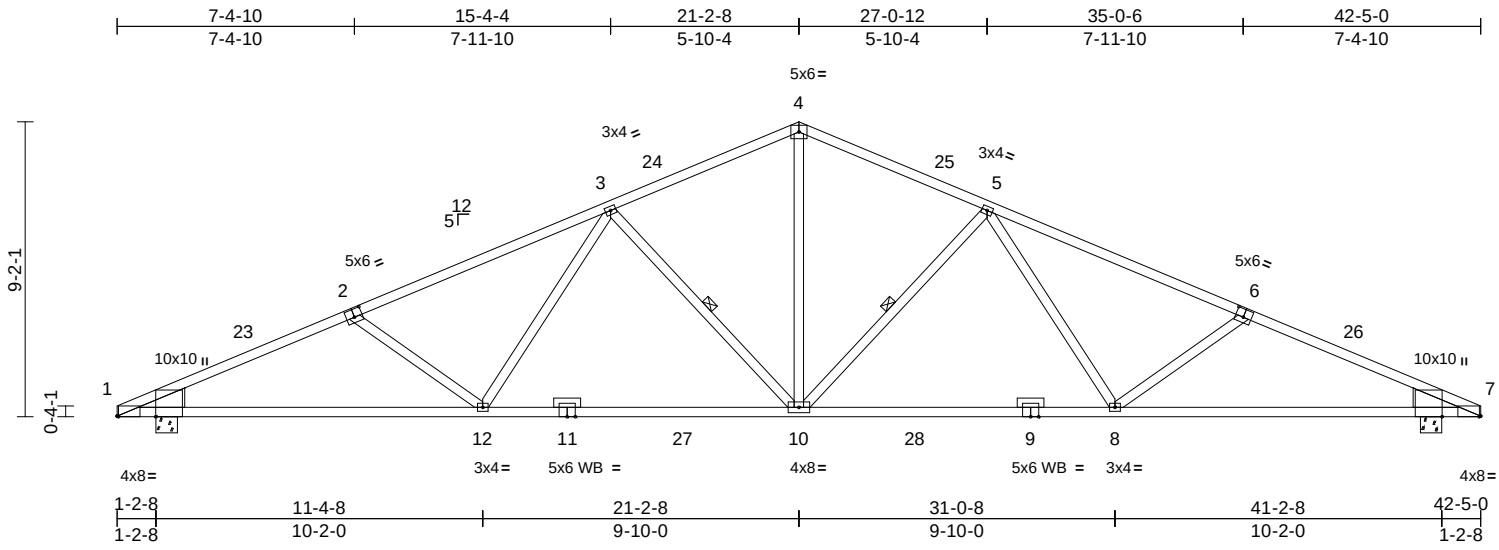

Scale = 1:71.7

Plate Offsets (X, Y): [1:0-0-8,Edge], [1:0-0-4,Edge], [2:0-3-0,0-3-0], [6:0-3-0,0-3-0], [7:0-0-8,Edge], [7:0-0-4,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.85	Vert(LL)	-0.45	10-12	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.86	Vert(CT)	-0.92	10-12	>553	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	1.00	Horz(CT)	0.16	7	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.36	10-12	>999	240	Weight: 217 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP M 30
BOT CHORD 2x4 SP M 31 *Except* 11-9:2x4 SP M 30
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3
WEDGE Left: 2x8 SP No.2
Right: 2x8 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 5-10, 3-10

REACTIONS (lb/size) 1=2121/0-8-0, 7=2121/0-8-0
Max Horiz 1=-337 (LC 8)
Max Uplift 1=-705 (LC 10), 7=-705 (LC 10)
Max Grav 1=2419 (LC 15), 7=2419 (LC 16)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-3=-4221/1622, 3-4=-2848/1311, 4-5=-2848/1311, 5-7=-4221/1622
BOT CHORD 1-12=-1339/3998, 10-12=-1075/3371, 8-10=-1075/3144, 7-8=-1339/3746
WEBS 4-10=-660/1808, 5-10=-1090/511, 5-8=-45/607, 6-8=-387/311, 3-10=-1090/511, 3-12=-45/607, 2-12=-387/311

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-2-14, Interior (1) 4-2-14 to 16-11-10, Exterior(2R) 16-11-10 to 25-5-6, Interior (1) 25-5-6 to 38-2-2, Exterior (2E) 38-2-2 to 42-5-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 705 lb uplift at joint 1 and 705 lb uplift at joint 7.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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Julius Lee PE No. 34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	A04	Hip	2	1	Job Reference (optional)	T24659403

American Builders Supply, Inc., Winter Haven, FL - 33880,

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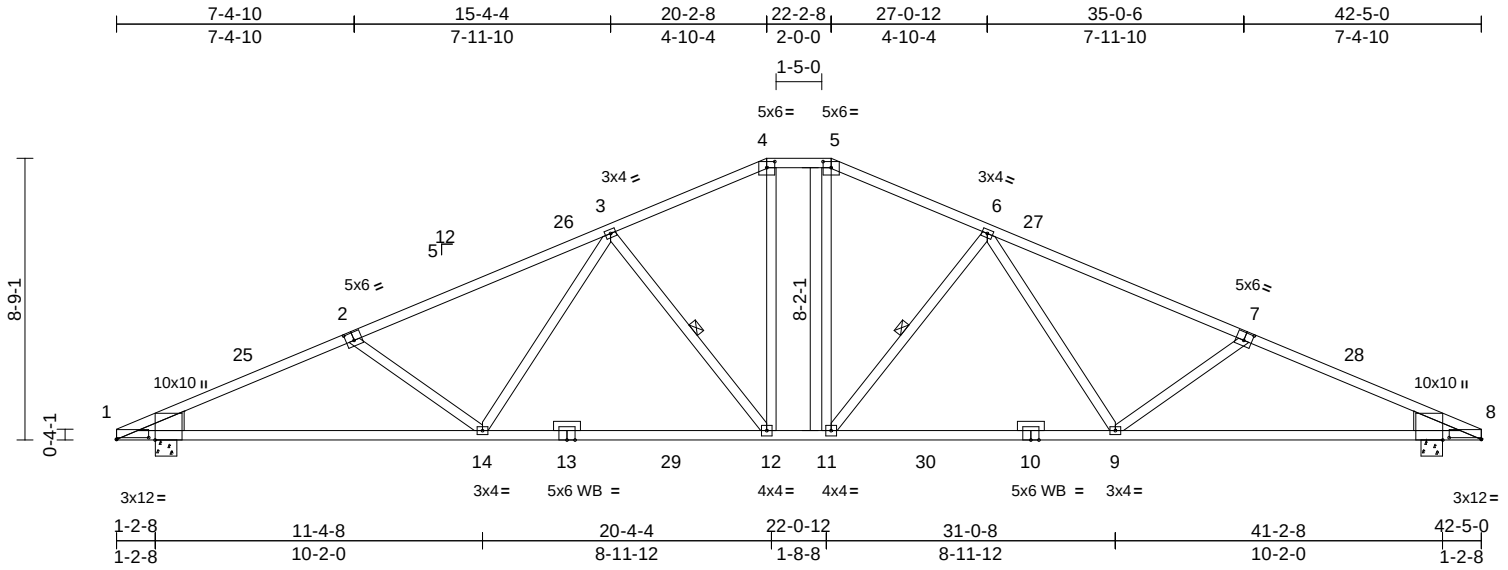

Scale = 1:71.6

Plate Offsets (X, Y): [1:1-0-0,0-0-10], [1:0-0-4,Edge], [2:0-3-0,0-3-0], [4:0-3-0,0-2-4], [5:0-3-0,0-2-4], [7:0-3-0,0-3-0], [8:1-0-0,0-0-10], [8:0-0-4,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.84	Vert(LL)	-0.42	12-14	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.77	Vert(CT)	-0.87	12-14	>584	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.48	Horz(CT)	0.16	8	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.36	12-14	>999	240	Weight: 227 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP M 30 *Except* 4-5:2x4 SP No.2
BOT CHORD	2x4 SP M 31 *Except* 13-10:2x4 SP M 30
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
WEDGE	Left: 2x8 SP No.2
	Right: 2x8 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 3-12, 6-11

REACTIONS (lb/size) 1=2121/0-8-0, 8=2121/0-8-0
Max Horiz 1=322 (LC 9)
Max Uplift 1=-705 (LC 10), 8=-705 (LC 10)
Max Grav 1=2417 (LC 15), 8=2417 (LC 16)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-3=-4218/1589, 3-4=-2914/1302, 4-5=-2616/1248, 5-6=-2914/1302, 6-8=-4218/1589
BOT CHORD	1-14=-1310/3984, 12-14=-1033/3352, 11-12=-730/2712, 9-11=-1033/3132, 8-9=-1310/3743
WEBS	2-14=-389/318, 3-14=-48/615, 3-12=-1069/503, 4-12=-348/972, 5-11=-348/973, 6-11=-1069/503, 6-9=-48/615, 7-9=-389/318

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-2-14, Interior (1) 4-2-14 to 14-2-8, Exterior(2R) 14-2-8 to 28-2-8, Interior (1) 28-2-8 to 38-2-2, Exterior(2E) 38-2-2 to 42-5-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 705 lb uplift at joint 1 and 705 lb uplift at joint 8.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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Julius Lee PE No. 34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	A05	Hip	2	1	Job Reference (optional)	T24659404

American Builders Supply, Inc., Winter Haven, FL - 33880,

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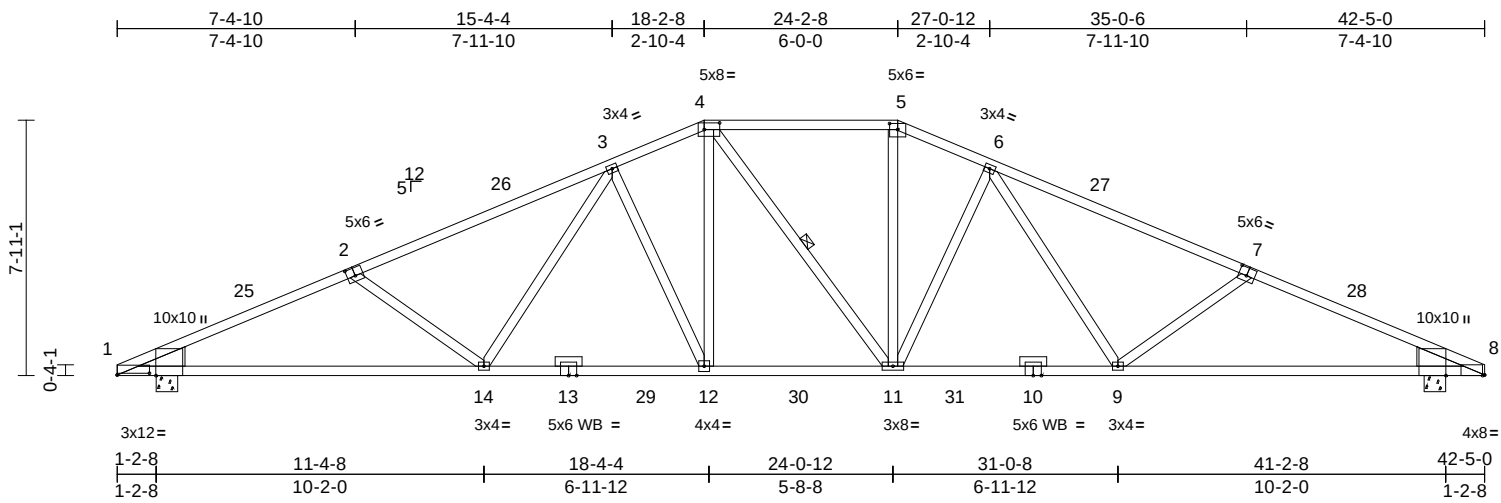

Scale = 1:71.5

Plate Offsets (X, Y): [1:1-0-0,0-0-10], [1:0-0-4,Edge], [2:0-3-0,0-3-0], [4:0-5-12,0-2-8], [5:0-3-0,0-2-4], [7:0-3-0,0-3-0], [8:0-0-12,Edge], [8:0-0-4,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.82	Vert(LL)	-0.34	12-14	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	1.00	Vert(CT)	-0.73	12-14	>697	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.87	Horz(CT)	0.18	8	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.36	12-14	>999	240	Weight: 234 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP M 30 *Except* 4-5:2x4 SP No.2
BOT CHORD 2x4 SP M 31 *Except* 13-10:2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3
WEDGE Left: 2x8 SP No.2
Right: 2x8 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 4-11

REACTIONS (lb/size) 1=2121/0-8-0, 8=2121/0-8-0
Max Horiz 1=-290 (LC 8)
Max Uplift 1=-705 (LC 10), 8=-705 (LC 10)
Max Grav 1=2431 (LC 15), 8=2428 (LC 16)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-3=-4238/1645, 3-4=-3192/1449, 4-5=-2887/1353, 5-6=-3185/1449, 6-8=-4232/1645
BOT CHORD 1-14=-1361/3983, 12-14=-1069/3368, 11-12=-881/3002, 9-11=-1069/3157, 8-9=-1361/3760
WEBS 2-14=-409/327, 3-14=-61/550, 3-12=-916/460, 4-12=-361/1120, 4-11=-203/207, 5-11=-360/983, 6-11=-914/459, 6-9=-61/550, 7-9=-410/328

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-2-14, Interior (1) 4-2-14 to 12-2-8, Exterior(2R) 12-2-8 to 30-2-8, Interior (1) 30-2-8 to 38-2-2, Exterior(2E) 38-2-2 to 42-5-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 705 lb uplift at joint 1 and 705 lb uplift at joint 8.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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Julius Lee PE No. 34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	A06	Hip	2	1	Job Reference (optional)	T24659405

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:40 Page: 1
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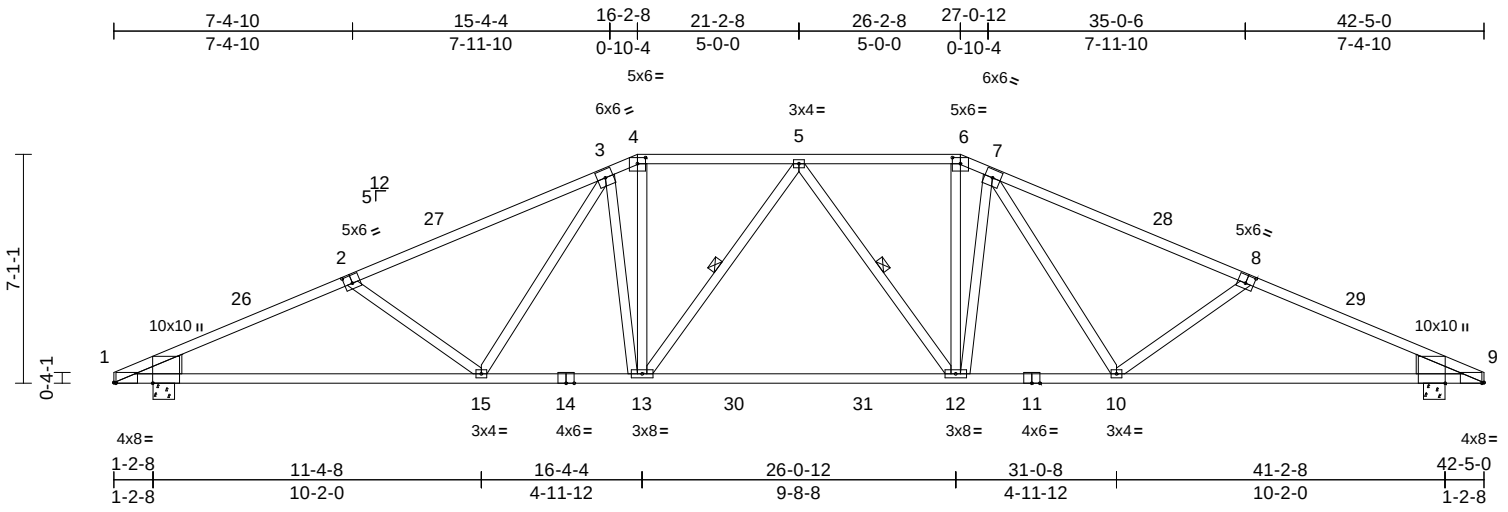

Scale = 1:71.3

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.80	Vert(LL)	-0.44	12-13	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.80	Vert(CT)	-0.91	12-13	>557	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.85	Horz(CT)	0.16	9	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.33	13-15	>999	240	Weight: 237 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP M 30 *Except* 4-6:2x4 SP No.2
BOT CHORD 2x4 SP M 31 *Except* 14-11:2x4 SP M 30
WEBS 2x4 SP No.3
WEDGE Left: 2x8 SP No.2
Right: 2x8 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 5-13, 5-12

REACTIONS (lb/size) 1=2121/0-8-0, 9=2121/0-8-0
Max Horiz 1=258 (LC 9)
Max Uplift 1=-705 (LC 10), 9=-705 (LC 10)
Max Grav 1=2402 (LC 15), 9=2402 (LC 16)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-3=-4157/1695, 3-4=-3508/1660, 4-5=-3108/1441, 5-6=-3108/1441, 6-7=-3508/1660, 7-9=-4158/1695
BOT CHORD 1-15=-1406/3889, 13-15=-1105/3337, 12-13=-1125/3260, 10-12=-1105/3154, 9-10=-1406/3696
WEBS 2-15=-419/328, 3-15=-76/417, 3-13=-1064/674, 4-13=-749/1488, 5-13=-392/187, 5-12=-392/187, 6-12=-749/1488, 7-12=-1064/674, 7-10=-76/417, 8-10=-419/328

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-2-14, Interior (1) 4-2-14 to 10-2-8, Exterior(2R) 10-2-8 to 32-2-8, Interior (1) 32-2-8 to 38-2-2, Exterior(2E) 38-2-2 to 42-5-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 705 lb uplift at joint 1 and 705 lb uplift at joint 9.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659406
M2001623-20BX	A07	Hip	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:40 Page: 1

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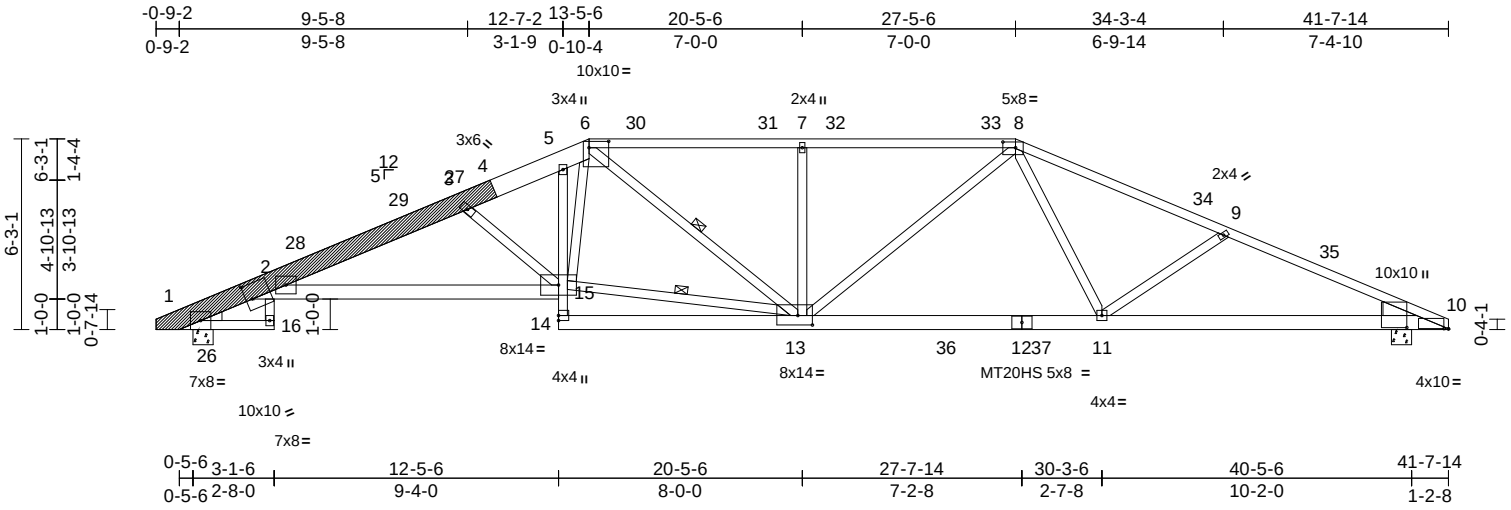

Scale = 1:75.6

Plate Offsets (X, Y): [2:0-2-8,0-5-15], [6:0-7-12,0-2-8], [8:0-5-0,0-2-4], [10:0-1-13,0-0-1], [10:0-0-8,1-4-6], [13:0-5-12,0-3-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.70	Vert(LL)	-0.35	11-13	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.97	Vert(CT)	-0.78	11-13	>640	240	MT20HS	187/143
BCLL	0.0*	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.34	10	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.36	11-13	>999	240	Weight: 311 lb	FT = 20%

LUMBER

TOP CHORD	2x8 SP 2400F 2.0E *Except* 6-8:2x4 SP No.2, 8-10:2x4 SP M 31
BOT CHORD	2x4 SP No.3 *Except* 1-16:2x4 SP No.2, 2-15,12-10:2x6 SP 2400F 2.0E, 14-12:2x6 SP No.2
WEBS	2x4 SP No.3 *Except* 13-15:2x4 SP No.2
LBR SCAB	1-4 SP 2400F 2.0E one side
WEDGE	Left: 2x4 SP No.3 Right: 2x6 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 13-15, 6-13

REACTIONS (lb/size) 1=2028/0-8-0, 10=2140/0-8-0
Max Horiz 1=222 (LC 9)
Max Uplift 1=-671 (LC 10), 10=-711 (LC 10)
Max Grav 1=2277 (LC 15), 10=2420 (LC 16)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-956/409, 2-3=-5658/2283, 3-5=-4672/1936, 5-6=-4307/1870, 6-7=-3689/1685, 7-8=-3708/1700, 8-9=-3730/1518, 9-10=-4101/1684
BOT CHORD	1-16=-165/486, 2-16=-61/215, 2-15=-2037/5597, 14-15=0/135, 5-15=-86/424, 13-14=-143/339, 11-13=-1112/3116, 10-11=-1406/3650
WEBS	3-15=-1820/799, 13-15=-1201/3602, 6-15=-484/1431, 6-13=-233/159, 7-13=-603/406, 8-13=-271/873, 8-11=-2/493, 9-11=-409/360

NOTES

1) Attached 12-0-0 scab 1 to 4, front face(s) 2x8 SP 2400F 2.0E with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 0-8-8 from end at joint 1, nail 2 row(s) at 2" o.c. for 4-11-15; starting at 7-3-13 from end at joint 1, nail 2 row(s) at 3" o.c. for 3-11-13.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-9-2 to 4-11-2, Interior (1) 4-11-2 to 8-3-13, Exterior(2R) 8-3-13 to 20-1-3, Interior (1) 20-1-3 to 22-3-13, Exterior(2R) 22-3-13 to 34-1-3, Interior (1) 34-1-3 to 38-3-0, Exterior (2E) 38-3-0 to 42-5-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 671 lb uplift at joint 1 and 711 lb uplift at joint 10.
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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Julius Lee PE No. 34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659407
M2001623-20BX	A08	Hip	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:40 Page: 1

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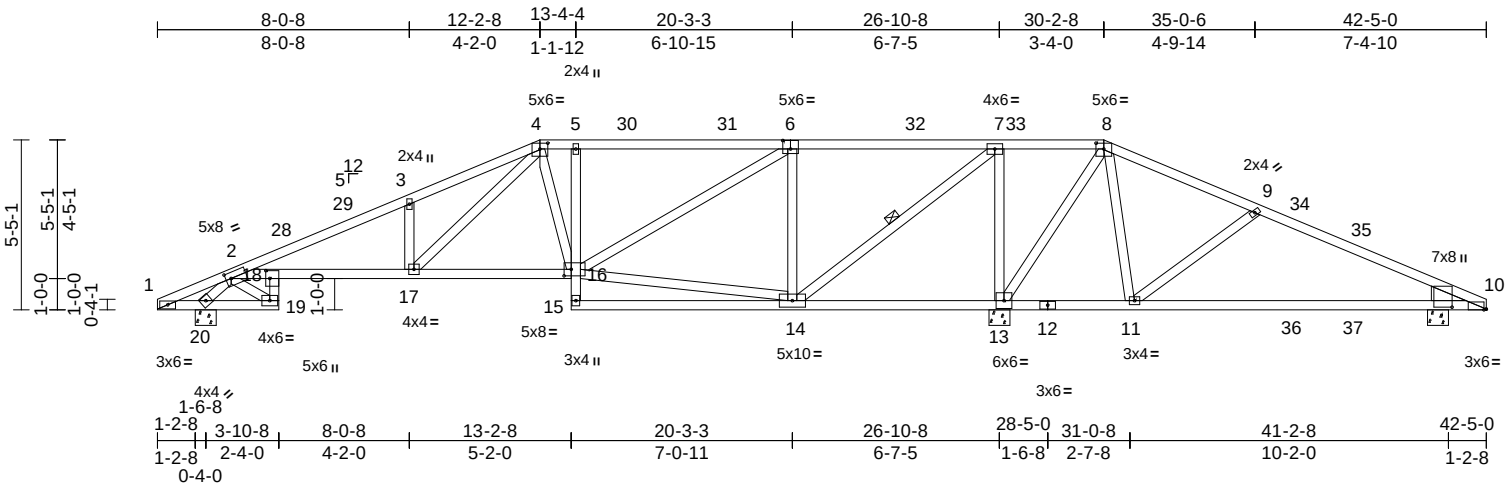

Scale = 1:73.5

Plate Offsets (X, Y): [2:0-2-0,0-2-4], [4:0-3-0,0-2-4], [6:0-3-0,0-3-4], [8:0-3-0,0-2-4], [10:0-0-14,Edge], [10:0-0-12,1-1-2], [16:0-2-12,0-2-8], [18:0-3-0,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.75	Vert(LL)	0.57	11-27	>325	240	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	1.00	Vert(CT)	-0.43	17-18	>708	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.86	Horz(CT)	0.19	13	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS							Weight: 235 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except* 2-16,12-10:2x4 SP M 30, 5-15:2x4 SP No.3

WEBS 2x4 SP No.3

WEDGE Right: 2x6 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied.

BOT CHORD Rigid ceiling directly applied.

WEBS 1 Row at midpt 7-14

REACTIONS (lb/size) 10=401/0-8-0, 13=2678/0-8-0, 20=1163/0-8-0

Max Horiz 20=-195 (LC 8)

Max Uplift 10=-464 (LC 10), 13=-1159 (LC 10), 20=-439 (LC 10)

Max Grav 10=467 (LC 20), 13=2678 (LC 1), 20=1202 (LC 19)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-195/108, 2-3=-2413/1002, 3-4=-2462/1162, 4-5=-1360/729, 5-7=-1368/742, 7-8=-268/1212, 8-9=0/865, 9-10=-154/866

BOT CHORD 1-20=-97/224, 19-20=-370/1114, 18-19=-256/852, 2-18=-700/1975, 17-18=-782/2225, 16-17=-393/1345, 15-16=0/130, 5-16=-462/307, 14-15=0/137, 13-14=-1206/590, 11-13=-720/188, 10-11=-688/65

WEBS 2-20=-1488/796, 4-16=-86/181, 8-13=-1133/1296, 7-13=-1650/786, 9-11=-578/474, 8-11=-1166/536, 14-16=-49/396, 6-14=-1109/582, 6-16=-424/1072, 7-14=-787/1958, 3-17=-566/380, 4-17=-547/1301, 2-19=-1147/393

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-2-14, Interior (1) 4-2-14 to 6-2-8, Exterior(2R) 6-2-8 to 18-2-8, Interior (1) 18-2-8 to 24-2-8, Exterior(2R) 24-2-8 to 36-2-8, Interior (1) 36-2-8 to 38-2-2, Exterior(2E) 38-2-2 to 42-5-0 zone; cantilever left and right exposed ; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 439 lb uplift at joint 20, 1159 lb uplift at joint 13 and 464 lb uplift at joint 10.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659408
M2001623-20BX	A09	Hip	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880,

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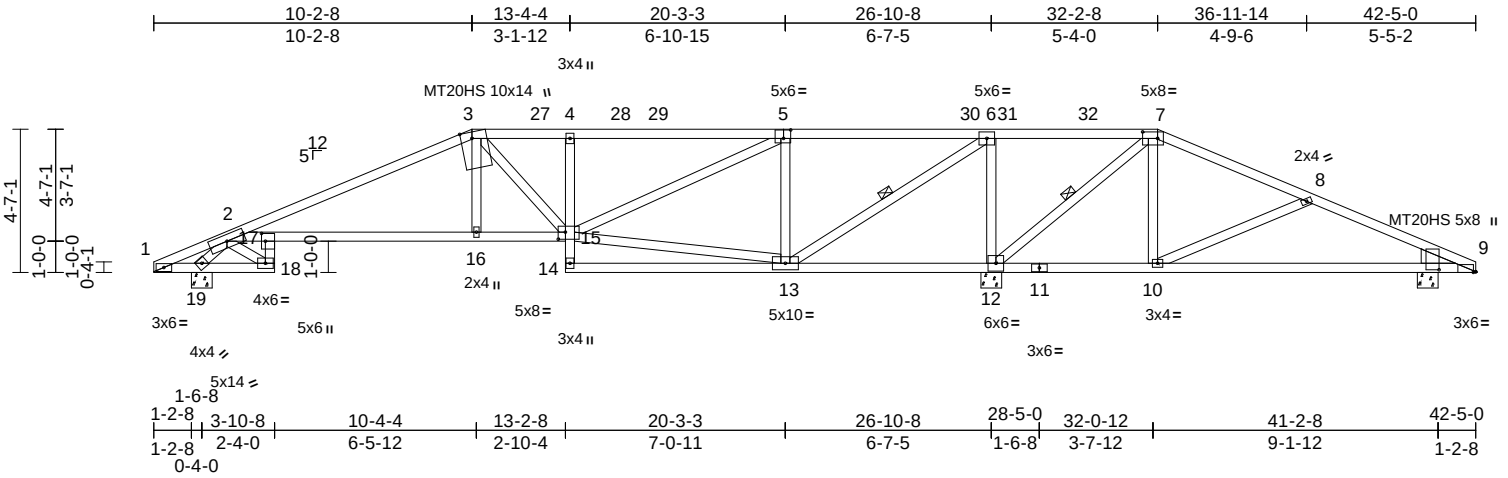

Scale = 1:73.9

Plate Offsets (X, Y): [3:0-2-7,Edge], [5:0-2-12,0-3-4], [7:0-5-12,0-2-8], [9:0-0-14,Edge], [9:0-0-12,1-2-2], [15:0-2-12,0-2-12], [17:0-3-0,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.78	Vert(LL)	-0.26	16-17	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.81	Vert(CT)	-0.68	16-17	>452	240	MT20HS	187/143
BCLL	0.0*	Rep Stress Incr	YES	WB	0.86	Horz(CT)	0.24	12	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.37	10-26	>509	240	Weight: 222 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP M 30 *Except* 3-5,5-7:2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 2-15:2x4 SP M 31, 4-14:2x4 SP No.3, 11-9:2x4 SP M 30
WEBS 2x4 SP No.3
WEDGE Right: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 7-12, 6-13

REACTIONS (lb/size) 9=323/0-8-0, 12=2799/0-8-0, 19=1120/0-8-0
Max Horiz 19=163 (LC 9)
Max Uplift 9=-413 (LC 10), 12=-1238 (LC 10), 19=-412 (LC 10)
Max Grav 9=387 (LC 20), 12=2799 (LC 1), 19=1155 (LC 19)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-207/111, 2-3=-1928/700, 3-4=-1470/606, 4-6=-1513/638, 6-7=-667/1675, 7-8=0/871, 8-9=-122/845
BOT CHORD 1-19=-95/220, 18-19=-373/1068, 17-18=-209/782, 2-17=-316/1468, 16-17=-456/1713, 15-16=-458/1732, 14-15=0/132, 4-15=-367/225, 13-14=0/219, 12-13=-1675/953, 10-12=-698/100, 9-10=-688/63
WEBS 2-19=-1442/796, 3-16=-46/495, 3-15=-512/200, 13-15=0/221, 7-12=-1495/1643, 7-10=-965/432, 8-10=-535/430, 6-12=-1749/838, 5-13=-1119/588, 5-15=-576/1268, 6-13=-913/2248, 2-18=-1049/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-2-14, Exterior(2R) 4-2-14 to 16-2-8, Interior (1) 16-2-8 to 26-2-8, Exterior(2R) 26-2-8 to 38-2-2, Exterior(2E) 38-2-2 to 42-5-0 zone; cantilever left and right exposed ; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 412 lb uplift at joint 19, 1238 lb uplift at joint 12 and 413 lb uplift at joint 9.
9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659409
M2001623-20BX	A10G	Hip Girder	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:42 Page: 1
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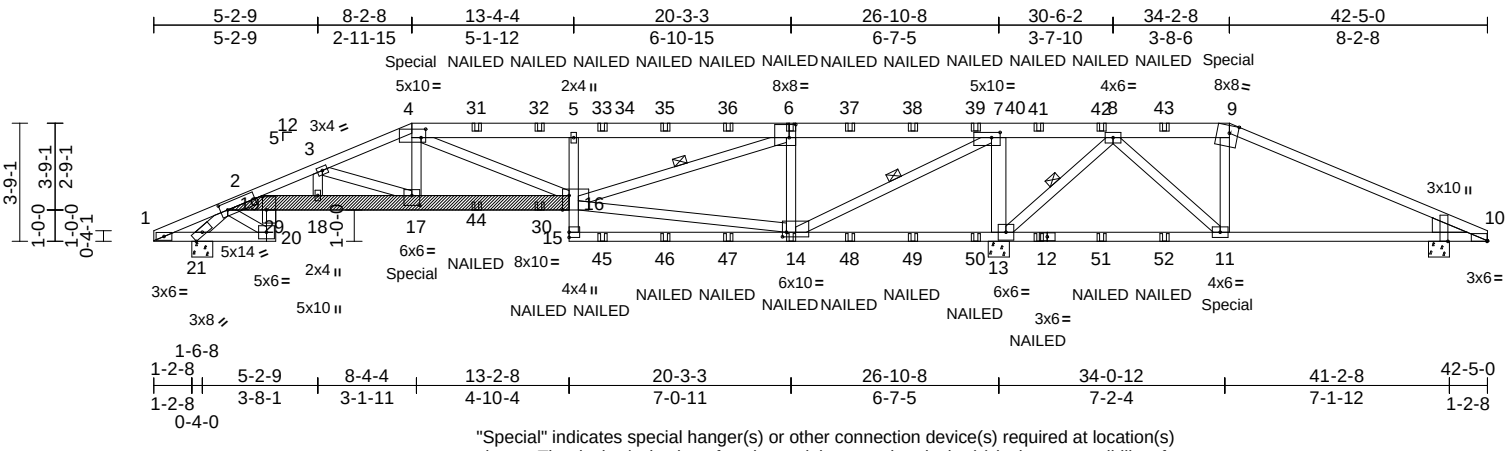

"Special" indicates special hanger(s) or other connection device(s) required at location(s) shown. The design/selection of such special connection device(s) is the responsibility of others. This applies to all applicable truss designs in this job.

Scale = 1:73.3

Plate Offsets (X, Y): [2:0-2-12,0-2-12], [4:0-1-12,0-3-4], [6:0-2-8,0-5-0], [7:0-3-4,0-2-0], [9:0-3-4,Edge], [10:0-0-2,Edge], [10:0-0-4,Edge], [14:0-1-8,0-1-12], [16:0-2-8,Edge], [17:0-3-0,0-3-12], [19:0-5-0,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.74	Vert(LL)	-0.22	5	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.87	Vert(CT)	-0.57	14-15	>536	240		
BCLL	0.0*	Rep Stress Incr	NO	WB	0.82	Horz(CT)	0.18	13	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MSH		Wind(LL)	0.32	5	>962	240	Weight: 275 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 4-6:2x6 SP No.2, 6-9:2x6 SP 2400F 2.0E

BOT CHORD 2x4 SP No.2 *Except* 20-19:2x4 SP M 30, 2-16:2x6 SP 2400F 2.0E

WEBS 2x4 SP No.3 *Except* 7-13:2x6 SP No.2, 6-16:2x4 SP No.2, 14-7:2x4 SP M 30

LBR SCAB 2-16 SP 2400F 2.0E one side

WEDGE Right: 2x6 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-1-9 oc purlins.

BOT CHORD Rigid ceiling directly applied or 2-8-15 oc bracing.

WEBS 1 Row at midpt 8-13, 6-16, 7-14

REACTIONS (lb/size) 10=461/0-8-0, 13=5458/0-8-0, 21=2019/0-8-0

Max Horiz 21=-130 (LC 6)

Max Uplift 10=-572 (LC 23), 13=-3168 (LC 8), 21=-865 (LC 8)

Max Grav 10=475 (LC 18), 13=5458 (LC 1), 21=2116 (LC 13)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-173/51, 2-3=-5961/2227, 3-4=-5249/2019, 4-5=-5435/1954, 5-7=-5370/1942, 7-8=-1884/3302, 8-9=-13/562, 9-10=-159/547

BOT CHORD 1-21=-16/172, 20-21=-788/2259, 19-20=-944/2751, 2-19=-1979/5577, 18-19=-1966/5560, 17-18=-1966/5560, 16-17=-1742/4964, 15-16=0/265, 5-16=-978/607, 14-15=-73/476, 13-14=-3258/2014, 11-13=-1541/648, 10-11=-378/42

WEBS 2-21=-2789/1123, 4-17=-475/1303, 4-16=-49/691, 9-11=-910/305, 2-20=-3405/1202, 7-13=-3577/1725, 8-13=-2503/1865, 8-11=-1398/1906, 6-16=-1743/3848, 6-14=-2421/1312, 14-16=-38/1318, 7-14=-2300/5493, 3-18=-66/300, 3-17=-718/268

NOTES

1) Attached 10-10-5 scab 2 to 16, front face(s) 2x6 SP 2400F 2.0E with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 0-5-5 from end at joint 2, nail 2 row(s) at 2" o.c. for 3-5-13.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.00
4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 865 lb uplift at joint 21, 3168 lb uplift at joint 13 and 572 lb uplift at joint 10.
9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 358 lb down and 177 lb up at 8-2-8, and 463 lb down and 316 lb up at 34-2-8 on top chord, and 513 lb down and 297 lb up at 8-2-8, and 279 lb down and 357 lb up at 34-1-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (lb/ft)
Vert: 1-4=-80, 4-9=-80, 9-10=-80, 20-22=-20, 16-19=-20, 15-25=-20
Concentrated Loads (lb)

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Date:

July 13,2021

Continued on page 2

MiTek
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Tampa, FL 36610

Vert: 4=-139 (B), 9=-300 (B), 12=-53 (B), 17=-439 (B), 11=-271 (B), 6=-162 (B), 14=-51 (B), 30=-118 (B), 31=-94 (B), 32=-94 (B), 33=-162 (B), 35=-162 (B), 36=-162 (B), 37=-162 (B), 38=-162 (B), 39=-162 (B), 41=-159 (B), 42=-159 (B), 43=-159 (B), 44=-118 (B), 45=-51 (B), 46=-51 (B), 47=-51 (B), 48=-51 (B), 49=-51 (B), 50=-51 (B), 51=-53 (B), 52=-53 (B)

Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	A13	Hip	1	1	Job Reference (optional)	T24659410

American Builders Supply, Inc., Winter Haven, FL - 33880,

Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:43 Page: 1
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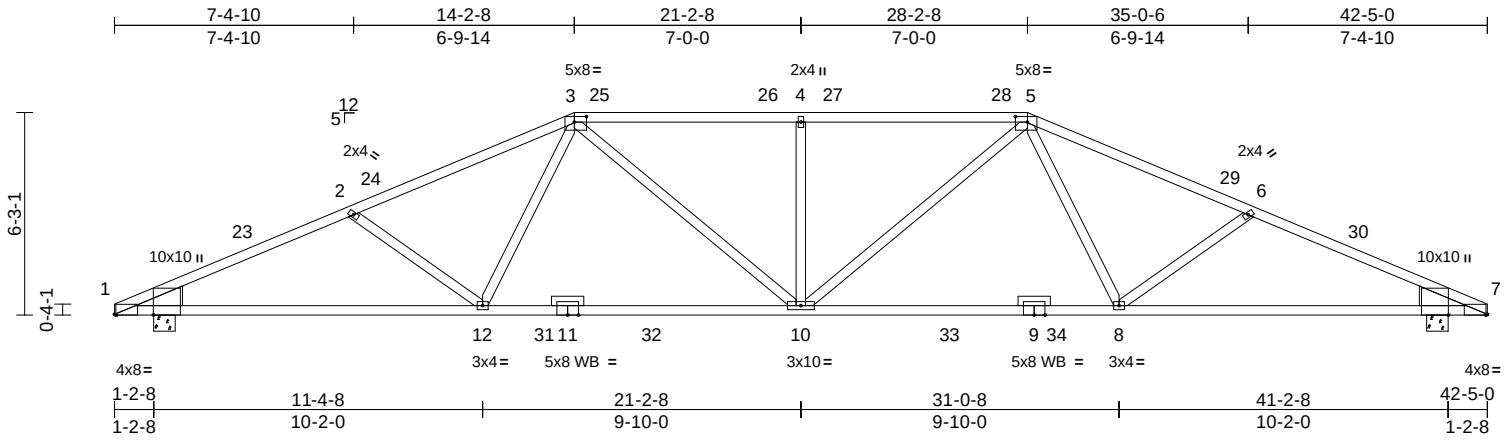

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Plate Offsets (X, Y): [1:0-0-8,Edge], [1:0-0-4,Edge], [3:0-4-8,0-2-0], [5:0-4-8,0-2-0], [7:0-0-8,Edge], [7:0-0-4,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.85	Vert(LL)	-0.45	10-12	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.84	Vert(CT)	-0.94	10-12	>540	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.41	Horz(CT)	0.17	7	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.37	10-12	>999	240	Weight: 210 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP M 30 *Except* 3-5:2x4 SP No.2
BOT CHORD	2x4 SP M 31 *Except* 11-9:2x4 SP M 30
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
WEDGE	Left: 2x8 SP No.2 Right: 2x8 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.

REACTIONS (lb/size) 1=2121/0-8-0, 7=2121/0-8-0
Max Horiz 1=226 (LC 9)
Max Uplift 1=-705 (LC 10), 7=-705 (LC 10)
Max Grav 1=2419 (LC 15), 7=2419 (LC 16)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-4206/1710, 2-3=-3873/1569, 3-4=-3778/1700, 4-5=-3778/1700, 5-6=-3873/1569, 6-7=-4206/1710
BOT CHORD	1-12=-1419/3898, 10-12=-1147/3387, 8-10=-1147/3223, 7-8=-1419/3728
WEBS	3-10=-229/837, 4-10=-587/394, 5-10=-229/837, 3-12=-36/548, 2-12=-343/319, 5-8=-36/549, 6-8=-343/319

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-2-14, Interior (1) 4-2-14 to 8-2-8, Exterior(2R) 8-2-8 to 20-2-8, Interior (1) 20-2-8 to 22-2-8, Exterior(2R) 22-2-8 to 34-2-8, Interior (1) 34-2-8 to 38-2-2, Exterior(2E) 38-2-2 to 42-5-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 705 lb uplift at joint 1 and 705 lb uplift at joint 7.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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Julius Lee PE No. 34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659411
M2001623-20BX	A14	Roof Special	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880,

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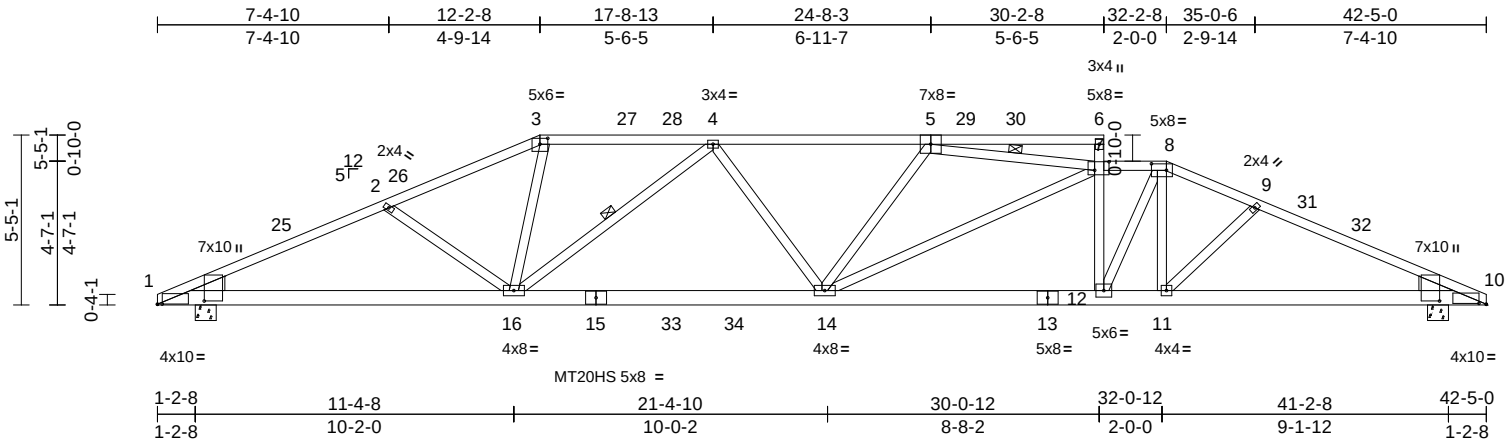

Scale = 1:73.5

Plate Offsets (X, Y): [1:0-1-13,0-0-1], [1:0-1-4,1-5-14], [3:0-3-0,0-2-4], [7:0-5-8,0-3-4], [8:0-5-12,0-2-8], [10:0-2-13,0-0-5], [10:0-1-4,1-5-14]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.90	Vert(LL)	-0.33	14-16	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	1.00	Vert(CT)	-0.73	14-16	>693	240	MT20HS	187/143
BCLL	0.0*	Rep Stress Incr	YES	WB	0.85	Horz(CT)	0.11	10	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MSH		Wind(LL)	0.34	14-16	>999	240	Weight: 260 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 1-3,8-10:2x4 SP M 30
BOT CHORD 2x6 SP 2400F 2.0E *Except* 15-13:2x6 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x6 SP No.2
Right: 2x6 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-7, 4-16

REACTIONS (lb/size) 1=2121/0-8-0, 10=2121/0-8-0
Max Horiz 1=-220 (LC 8)
Max Uplift 1=-694 (LC 10), 10=-715 (LC 10)
Max Grav 1=2388 (LC 15), 10=2357 (LC 16)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-3998/1725, 2-3=-3683/1582, 3-4=-3196/1491, 4-6=-4130/1817, 7-12=-876/472, 6-7=-187/140, 7-8=-3923/1849, 8-9=-3600/1664, 9-10=-3821/1746
BOT CHORD 1-16=-1533/3718, 14-16=-1610/4098, 12-14=-1537/3876, 11-12=-1281/3251, 10-11=-1454/3391
WEBS 4-14=0/315, 5-14=-17/212, 5-7=-3699/1677, 7-14=-132/421, 8-12=-599/1447, 8-11=-274/117, 9-11=-191/235, 3-16=-259/1015, 4-16=-1178/470, 2-16=-316/305

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=42ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-2-14, Interior (1) 4-2-14 to 7-11-10, Exterior(2R) 7-11-10 to 16-5-6, Interior (1) 16-5-6 to 25-9-14, Exterior (2E) 25-9-14 to 30-0-12, Exterior(2R) 30-0-12 to 36-5-6, Interior (1) 36-5-6 to 38-2-2, Exterior(2E) 38-2-2 to 42-5-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 694 lb uplift at joint 1 and 715 lb uplift at joint 10.

LOAD CASE(S) Standard

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Julius Lee PE No. 34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	A15	Half Hip	1	1	Job Reference (optional)	T24659412

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:43 Page: 1
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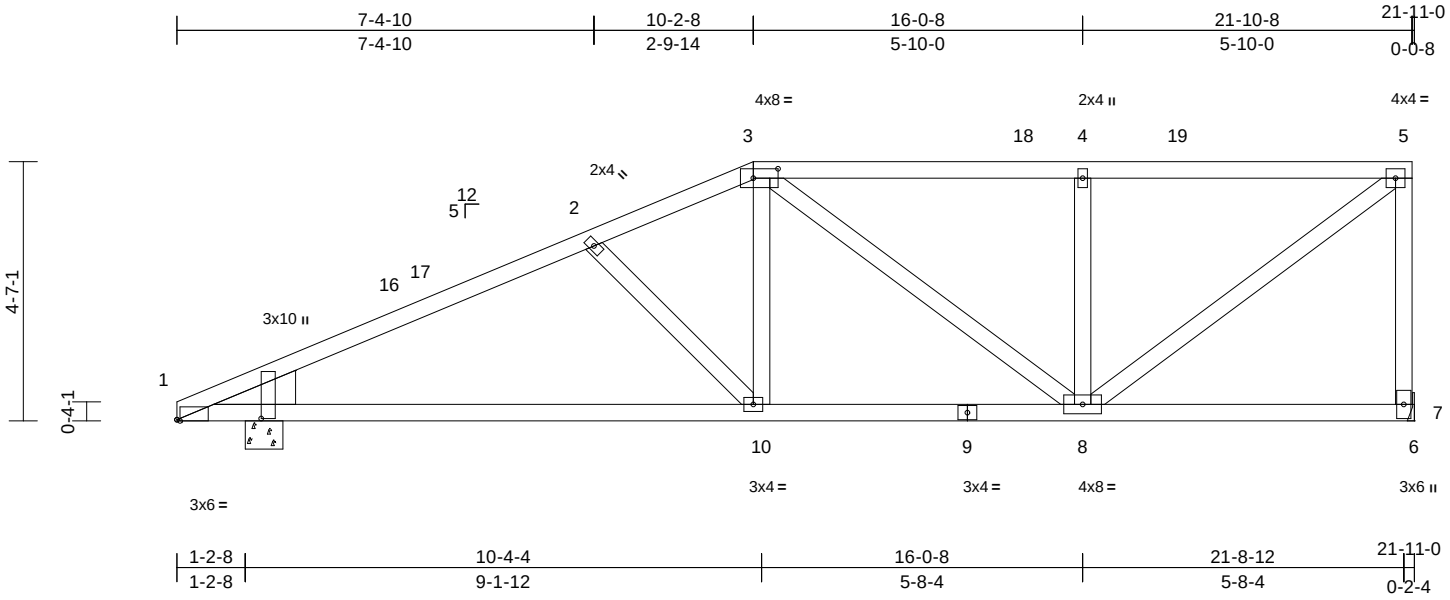

Scale = 1:40.8

Plate Offsets (X, Y): [1:0-0-10,Edge], [1:0-0-4,1-5-14], [3:0-5-4,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.52	Vert(LL)	-0.10	10-15	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.70	Vert(CT)	-0.20	10-15	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.64	Horz(CT)	0.03	7	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.06	8-10	>999	240	Weight: 115 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x8 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=1150/0-8-0, 7=1026/ Mechanical
Max Horiz 1=229 (LC 10)
Max Uplift 1=-358 (LC 10), 7=-362 (LC 10)
Max Grav 1=1150 (LC 1), 7=1026 (LC 19)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1626/783, 2-3=-1368/690, 3-4=-1050/555, 4-5=-1050/555, 5-7=-964/558
BOT CHORD 1-10=-868/1419, 8-10=-680/1238, 7-8=-13/32, 6-7=0/0
WEBS 2-10=-305/270, 3-10=-100/384, 3-8=-270/155, 4-8=-522/388, 5-8=-680/1276

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-0-0, Interior (1) 4-0-0 to 4-6-10, Exterior(2R) 4-6-10 to 16-0-8, Interior (1) 16-0-8 to 17-8-12, Exterior(2E) 17-8-12 to 21-8-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 7 and 358 lb uplift at joint 1.
9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

This item has been electronically signed and sealed by Lee, Julius, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Julius Lee PE No. 34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	A17G	Half Hip Girder	1	1	Job Reference (optional)	T24659413

American Builders Supply, Inc., Winter Haven, FL - 33880,

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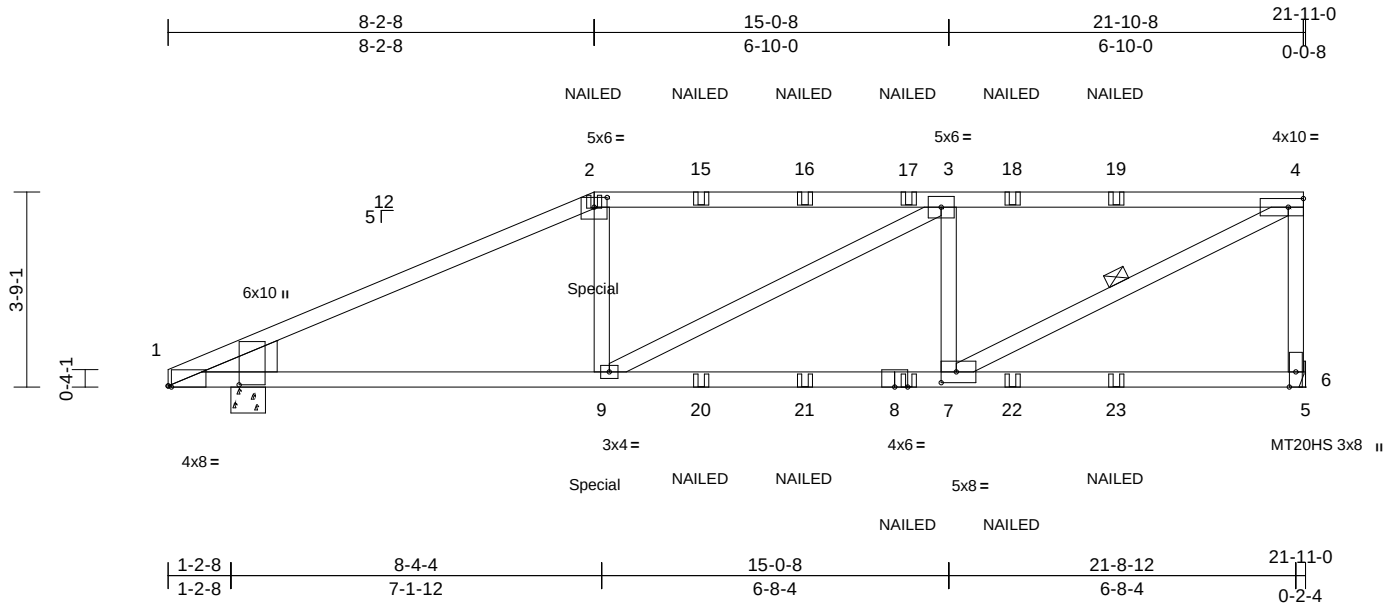

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Plate Offsets (X, Y): [1:0-0-12,Edge], [1:0-0-4,1-4-6], [2:0-3-0,0-2-4], [7:0-3-8,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.96	Vert(LL)	-0.17	7-9	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.97	Vert(CT)	-0.43	7-9	>605	240	MT20HS	187/143
BCLL	0.0*	Rep Stress Incr	NO	WB	0.77	Horz(CT)	0.05	6	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MSH		Wind(LL)	0.22	7-9	>999	240	Weight: 106 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP M 30 *Except* 2-4:2x4 SP M 31
BOT CHORD 2x4 SP M 31 *Except* 8-5:2x4 SP M 30
WEBS 2x4 SP No.3 *Except* 3-9,4-7:2x4 SP No.2
WEDGE Left: 2x8 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-5-8 oc bracing.
WEBS 1 Row at midpt 4-7

REACTIONS (lb/size) 1=1973/0-8-0, 6=1929/ Mechanical
Max Horiz 1=184 (LC 23)
Max Uplift 1=-757 (LC 8), 6=-779 (LC 8)
Max Grav 1=2075 (LC 13), 6=1996 (LC 13)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3582/1354, 2-3=-3228/1295, 3-4=-3228/1281, 4-6=-1908/822
BOT CHORD 1-9=-1297/3213, 7-9=-1281/3228, 6-7=-20/65, 5-6=0/0
WEBS 2-9=0/527, 3-7=-1276/816, 3-9=-166/47, 4-7=-1420/3562

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
8) Refer to girder(s) for truss to truss connections.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 779 lb uplift at joint 6 and 757 lb uplift at joint 1.
10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 313 lb down and 103 lb up at 8-2-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (lb/ft)
Vert: 1-2=-80, 2-4=-80, 5-10=-20
Concentrated Loads (lb)
Vert: 2=-403 (F), 8=-51 (F), 9=-262 (F), 15=-162 (F), 16=-162 (F), 17=-162 (F), 18=-162 (F), 19=-162 (F), 20=-51 (F), 21=-51 (F), 22=-51 (F), 23=-51 (F)

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MiTek USA, Inc. FL Cert 6634
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Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659414
M2001623-20BX	A18G	Half Hip Girder	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880,

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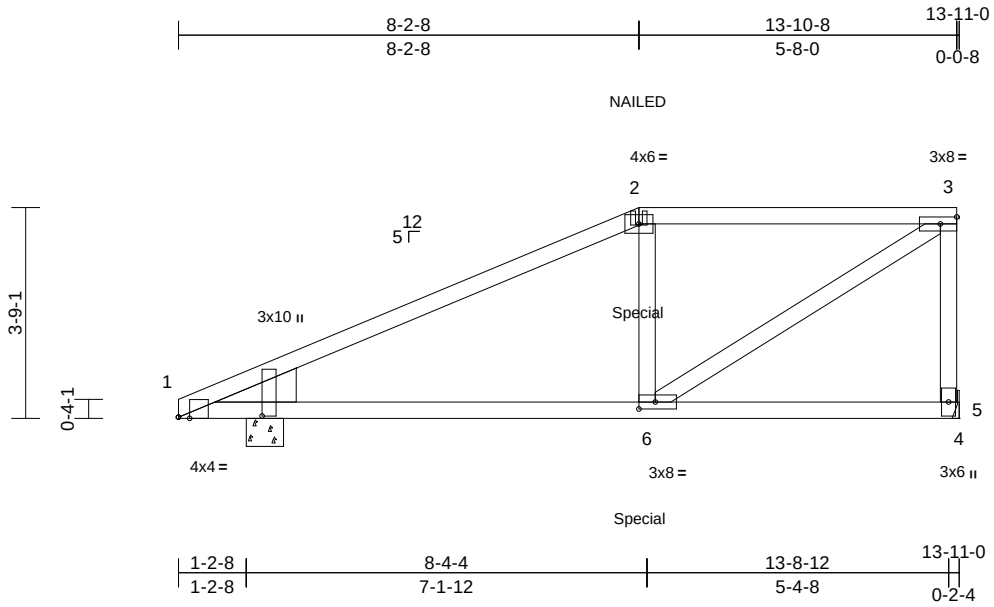

Scale = 1:41.1

Plate Offsets (X, Y): [1:0-2-6,Edge], [1:0-0-4,1-5-14], [6:0-3-8,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.70	Vert(LL)	0.11	6-11	>999	240	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.74	Vert(CT)	-0.07	6-11	>999	240		
BCLL	0.0*	Rep Stress Incr	NO	WB	0.51	Horz(CT)	0.01	5	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MSH							Weight: 65 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x8 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-11-13 oc bracing.

REACTIONS (lb/size) 1=1043/0-8-0, 5=998/ Mechanical
Max Horiz 1=184 (LC 8)
Max Uplift 1=-365 (LC 8), 5=-411 (LC 8)
Max Grav 1=1087 (LC 13), 5=1022 (LC 13)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1436/510, 2-3=-1242/514, 3-5=-965/440
BOT CHORD 1-6=-526/1256, 5-6=-18/45, 4-5=0/0
WEBS 2-6=-355/288, 3-6=-597/1435

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 411 lb uplift at joint 5 and 365 lb uplift at joint 1.
9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 313 lb down and 103 lb up at 8-2-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (lb/ft)
Vert: 1-2=-80, 2-3=-80, 4-7=-20
Concentrated Loads (lb)
Vert: 2=-403 (B), 6=-262 (B)

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Julius Lee PE No. 34869
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659415
M2001623-20BX	C01	Common Supported Gable	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880,

Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:44
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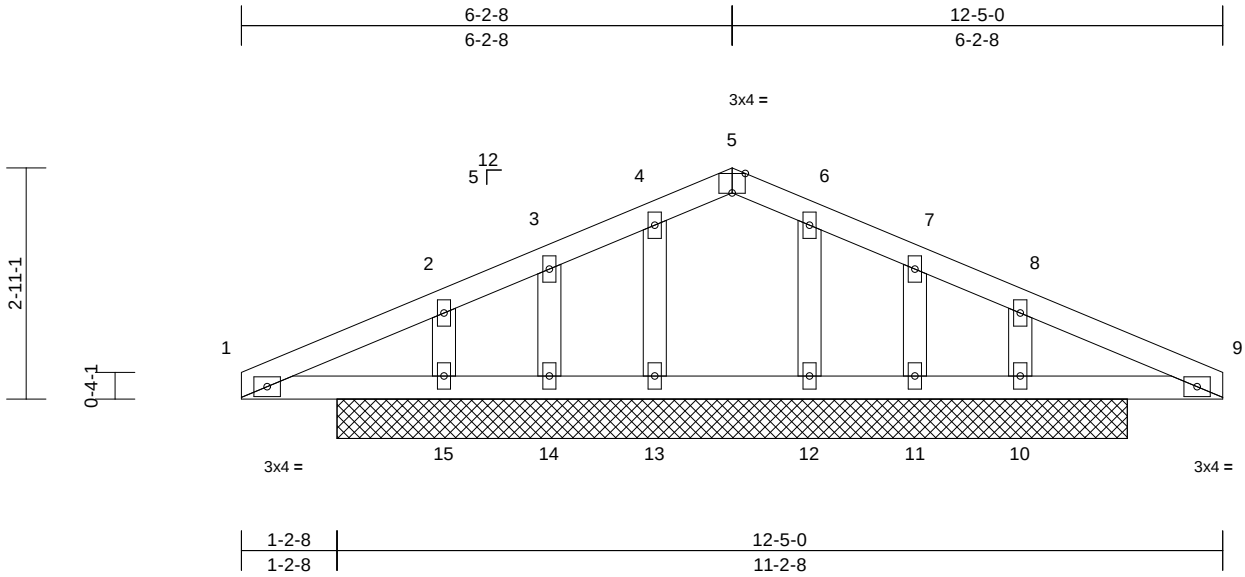

Scale = 1:29.2

Plate Offsets (X, Y): [5:0-2-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.19	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.15	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.08	Horiz(TL)	0.00	10	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS							Weight: 51 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 10=327/10-0-0, 11=-13/10-0-0, 12=307/10-0-0, 13=307/10-0-0, 14=-13/10-0-0, 15=327/10-0-0
Max Horiz 15=97 (LC 9)
Max Uplift 10=-123 (LC 10), 11=-56 (LC 20), 12=-57 (LC 10), 13=-57 (LC 10), 14=-56 (LC 19), 15=-123 (LC 10)
Max Grav 10=385 (LC 20), 11=35 (LC 19), 12=307 (LC 1), 13=307 (LC 1), 14=35 (LC 20), 15=385 (LC 19)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-202/269, 2-3=-91/221, 3-4=-46/259, 4-5=0/188, 5-6=0/188, 6-7=-46/259, 7-8=-91/221, 8-9=-202/267
BOT CHORD 1-15=-221/247, 14-15=-219/247, 13-14=-219/247, 12-13=-219/247, 11-12=-219/247, 10-11=-219/247, 9-10=-219/247
WEBS 4-13=-262/193, 6-12=-262/193, 3-14=-31/105, 2-15=-238/274, 7-11=-31/105, 8-10=-238/274

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=2ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Corner(3E) 0-0-0 to 3-10-12, Corner(3R) 3-10-12 to 8-6-4, Corner(3E) 8-6-4 to 12-5-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 13, 57 lb uplift at joint 12, 56 lb uplift at joint 14, 123 lb uplift at joint 15, 56 lb uplift at joint 11 and 123 lb uplift at joint 10.
10) Non Standard bearing condition. Review required.
11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659416
M2001623-20BX	C02	Common	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:45 Page: 1
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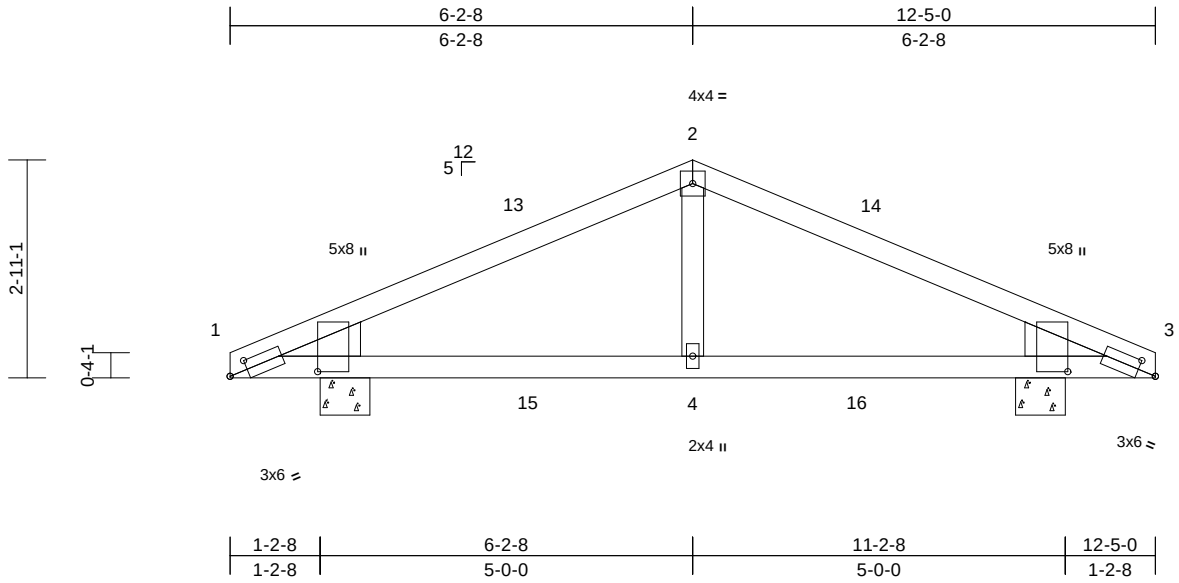

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Plate Offsets (X, Y): [1:0-3-0,0-1-8], [1:0-0-12,1-2-2], [3:0-3-0,0-1-8], [3:0-0-12,1-2-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.73	Vert(LL)	0.08	4-8	>999	240	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.65	Vert(CT)	-0.05	4	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.08	Horz(CT)	-0.01	3	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS							Weight: 47 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x6 SP No.2
Right: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=621/0-8-0, 3=621/0-8-0
Max Horiz 1=-97 (LC 8)
Max Uplift 1=-446 (LC 10), 3=-446 (LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-497/1011, 2-3=-497/1011
BOT CHORD 1-4=-751/372, 3-4=-751/372
WEBS 2-4=-437/119

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-0-0, Exterior(2R) 4-0-0 to 8-5-0, Exterior(2E) 8-5-0 to 12-5-0 zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 1 and 446 lb uplift at joint 3.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659417
M2001623-20BX	C03	Common	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880,

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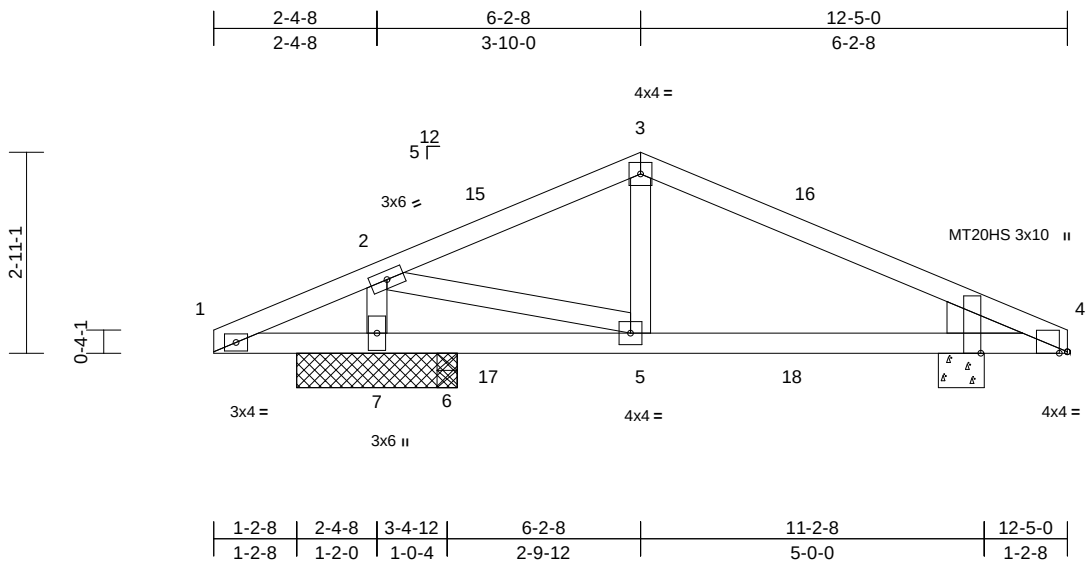

Scale = 1:33.5

Plate Offsets (X, Y): [4:0-1-6,Edge], [4:0-0-4,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	0.05	5-14	>999	240	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.52	Vert(CT)	-0.02	11	>999	240	MT20HS	187/143
BCLL	0.0*	Rep Stress Incr	YES	WB	0.31	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS							Weight: 51 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
WEDGE	Right: 2x6 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.

REACTIONS (lb/size) 4=548/0-8-0, 6=102/0-3-8, 7=592/2-4-0
Max Horiz 7=97 (LC 9)
Max Uplift 4=-394 (LC 10), 6=-75 (LC 7), 7=-412 (LC 10)
Max Grav 4=548 (LC 1), 6=110 (LC 3), 7=592 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-428/240, 2-3=-342/728, 3-4=-371/719
BOT CHORD	1-7=-171/406, 6-7=-199/406, 5-6=-199/406, 4-5=-491/259
WEBS	2-5=-928/445, 2-7=-596/1046, 3-5=-189/25

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-0-0, Exterior(2R) 4-0-0 to 8-5-0, Exterior(2E) 8-5-0 to 12-5-0 zone; cantilever left and right exposed ; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 412 lb uplift at joint 7, 394 lb uplift at joint 4 and 75 lb uplift at joint 6.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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Date:

July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659418
M2001623-20BX	C04	Common	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880,

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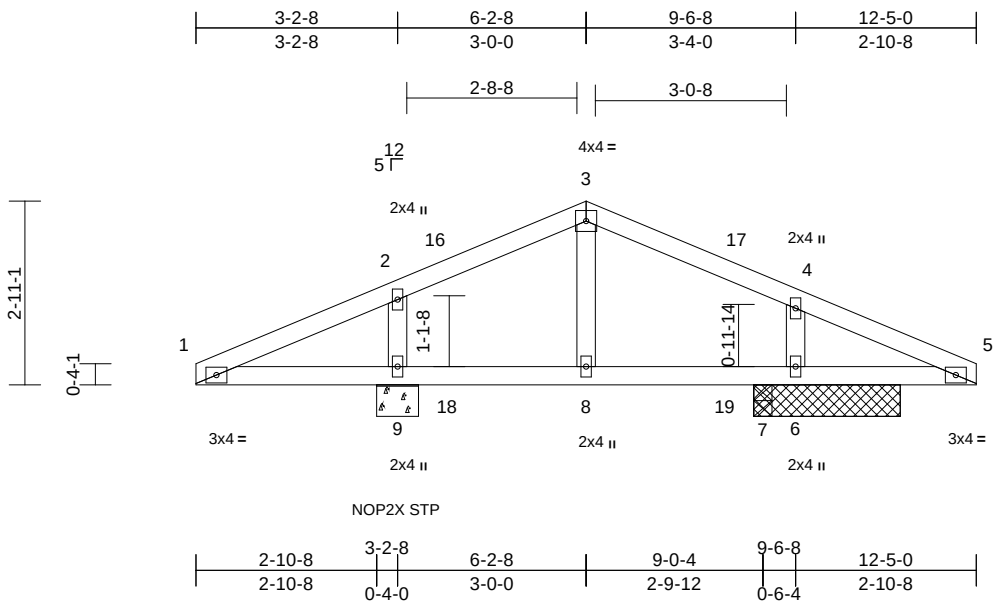

Scale = 1:36.7

Plate Offsets (X, Y): [1:0-1-14,0-0-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.63	Vert(LL)	-0.03	8	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.82	Vert(CT)	-0.07	8	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.00	6	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.09	8	>817	240	Weight: 45 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 6=294/2-4-0, 7=320/0-3-8, 9=627/0-8-0
Max Horiz 9=97 (LC 9)
Max Uplift 6=-11 (LC 19), 7=-363 (LC 10), 9=-464 (LC 10)
Max Grav 6=471 (LC 20), 7=392 (LC 19), 9=627 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-195/199, 2-3=0/146, 3-4=-13/153, 4-5=-191/222
BOT CHORD 1-9=-131/198, 8-9=-158/198, 7-8=-158/198, 6-7=-158/198, 5-6=-158/198
WEBS 2-9=-379/609, 4-6=-395/508, 3-8=-209/61

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-0-0, Exterior(2R) 4-0-0 to 8-5-0, Exterior(2E) 8-5-0 to 12-5-0 zone; cantilever left and right exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 464 lb uplift at joint 9, 11 lb uplift at joint 6 and 363 lb uplift at joint 7.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	C05	Common	3	1	Job Reference (optional)	T24659419

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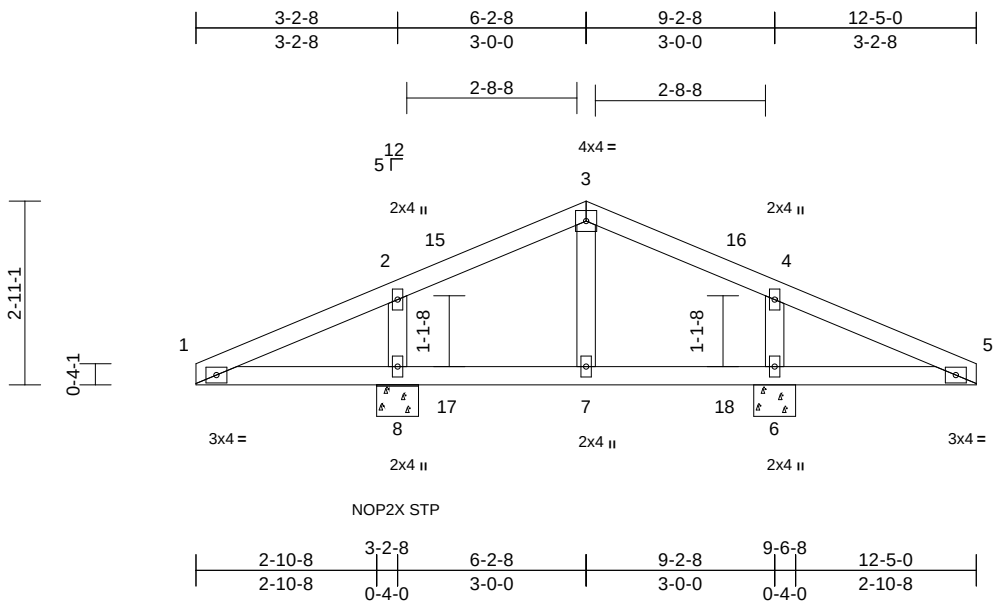

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Plate Offsets (X, Y): [1:0-1-14,0-0-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	-0.03	7	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.73	Vert(CT)	-0.07	7	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.00	6	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.10	7	>714	240	Weight: 46 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 6=621/0-8-0, 8=621/0-8-0
Max Horiz 8=97 (LC 9)
Max Uplift 6=-305 (LC 7), 8=-479 (LC 10)
Max Grav 6=625 (LC 20), 8=625 (LC 19)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-275/195, 2-3=-34/134, 3-4=-34/134, 4-5=-275/194
BOT CHORD 1-8=-142/275, 7-8=-142/275, 6-7=-142/275, 5-6=-142/275
WEBS 2-8=-392/605, 4-6=-392/605, 3-7=-178/26

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 4-0-0, Exterior(2R) 4-0-0 to 8-5-0, Exterior(2E) 8-5-0 to 12-5-0 zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 479 lb uplift at joint 8 and 305 lb uplift at joint 6.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659420
M2001623-20BX	C06	Common	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:45 Page: 1

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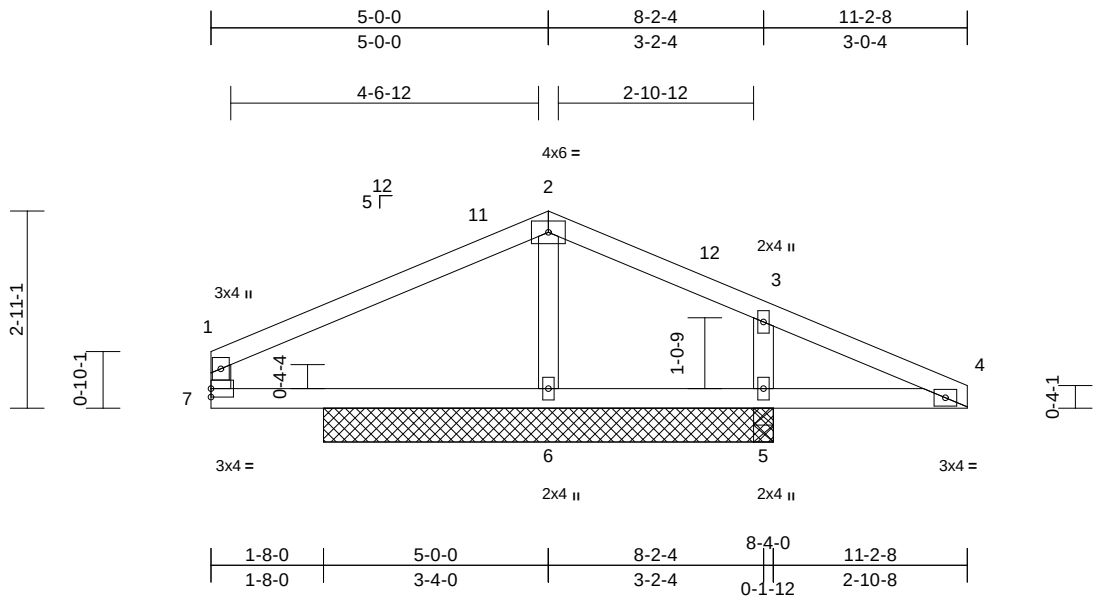

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.52	Vert(LL)	0.01	5-6	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.43	Vert(CT)	0.01	5-6	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	-0.02	5-6	>999	240	Weight: 41 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 5=235/0-3-8, 6=871/6-8-0
Max Horiz 6=87 (LC 9)
Max Uplift 5=-180 (LC 11), 6=-292 (LC 10)
Max Grav 5=383 (LC 20), 6=873 (LC 15)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-318/454, 2-3=-259/427, 3-4=-447/410, 1-7=-16/65
BOT CHORD 6-7=-355/373, 5-6=-344/437, 4-5=-344/437
WEBS 2-6=-686/610, 3-5=-289/469

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 4-1-12, Exterior(2R) 4-1-12 to 7-2-8, Exterior(2E) 7-2-8 to 11-2-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 6 and 180 lb uplift at joint 5.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	CJ02	Corner Jack	8	1	Job Reference (optional)	T24659421

American Builders Supply, Inc., Winter Haven, FL - 33880,

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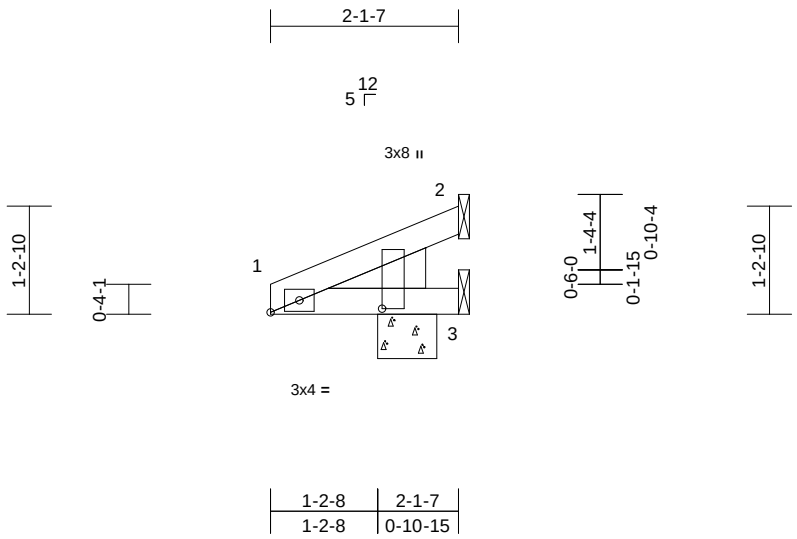

Scale = 1:26

Plate Offsets (X, Y): [1:0-1-14,0-0-2], [1:0-0-8,1-3-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.18	Vert(LL)	0.00	4	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.31	Vert(CT)	-0.01	4	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.01	2	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MP		Wind(LL)	0.02	4	>999	240	Weight: 9 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-1-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=410/0-8-0, 2=-37/ Mechanical, 3=-168/ Mechanical
Max Horiz 1=46 (LC 10)
Max Uplift 1=-254 (LC 10), 2=-37 (LC 1), 3=-168 (LC 1)
Max Grav 1=410 (LC 1), 2=14 (LC 10), 3=92 (LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-102/58
BOT CHORD 1-3=-43/106

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 2, 168 lb uplift at joint 3 and 254 lb uplift at joint 1.

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659422
M2001623-20BX	CJ04	Corner Jack	4	1	Job Reference (optional)	

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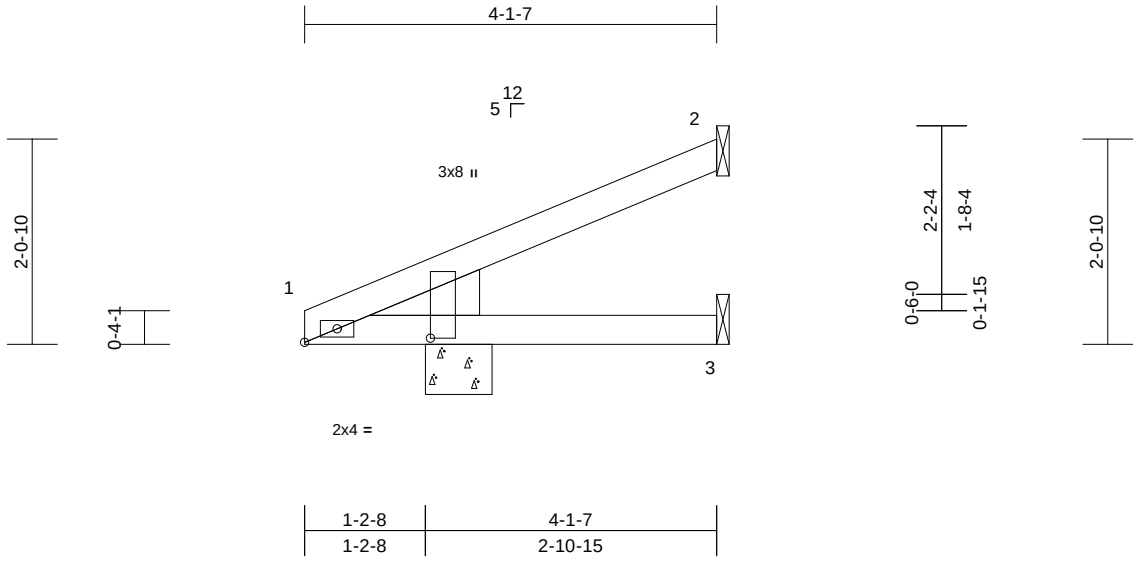

Scale = 1:23

Plate Offsets (X, Y): [1:0-1-14,0-0-10], [1:0-0-8,1-3-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.19	Vert(LL)	-0.01	4	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.20	Vert(CT)	-0.01	4	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	2	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	-0.01	4	>999	240	Weight: 16 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=327/0-8-0, 2=77/ Mechanical, 3=1/ Mechanical
Max Horiz 1=91 (LC 10)
Max Uplift 1=-77 (LC 10), 2=-67 (LC 10)
Max Grav 1=332 (LC 15), 2=87 (LC 15), 3=27 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-72/66
BOT CHORD 1-3=-84/209

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 2 and 77 lb uplift at joint 1.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659423
M2001623-20BX	CJ04P	Corner Jack	2	1	Job Reference (optional)	

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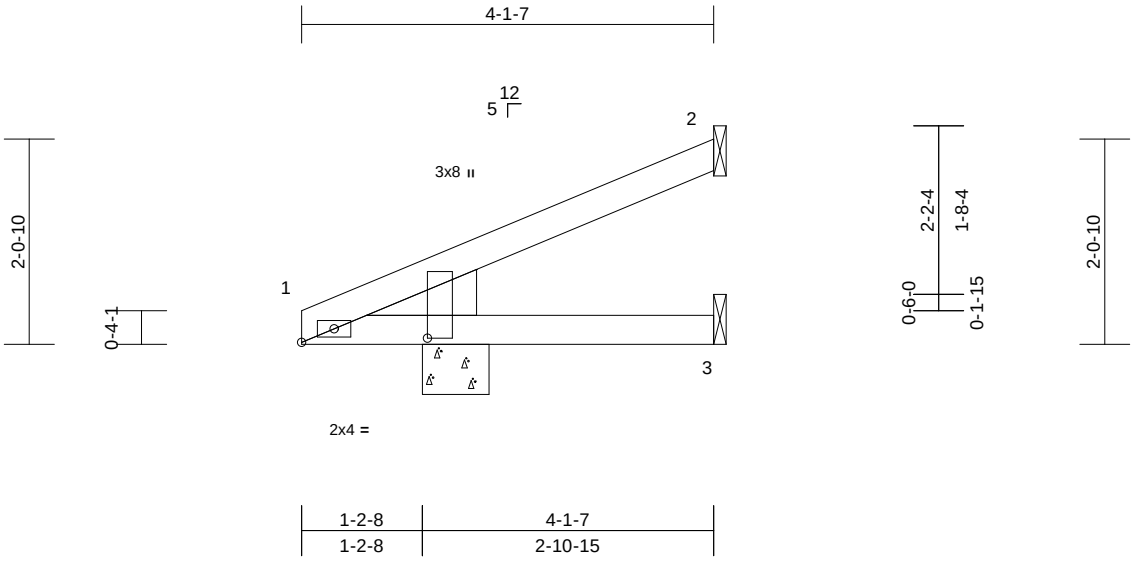

Scale = 1:23

Plate Offsets (X, Y): [1:0-1-14,0-0-10], [1:0-0-8,1-3-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.19	Vert(LL)	-0.01	4	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.21	Vert(CT)	-0.01	4	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	2	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.01	4	>999	240	Weight: 16 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=327/0-8-0, 2=77/ Mechanical, 3=1/ Mechanical
Max Horiz 1=91 (LC 10)
Max Uplift 1=-203 (LC 10), 2=-62 (LC 10), 3=-26 (LC 10)
Max Grav 1=327 (LC 1), 2=90 (LC 15), 3=27 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-119/64
BOT CHORD 1-3=-84/209

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 2, 26 lb uplift at joint 3 and 203 lb uplift at joint 1.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659424
M2001623-20BX	CJ04T	Corner Jack	2	1	Job Reference (optional)	

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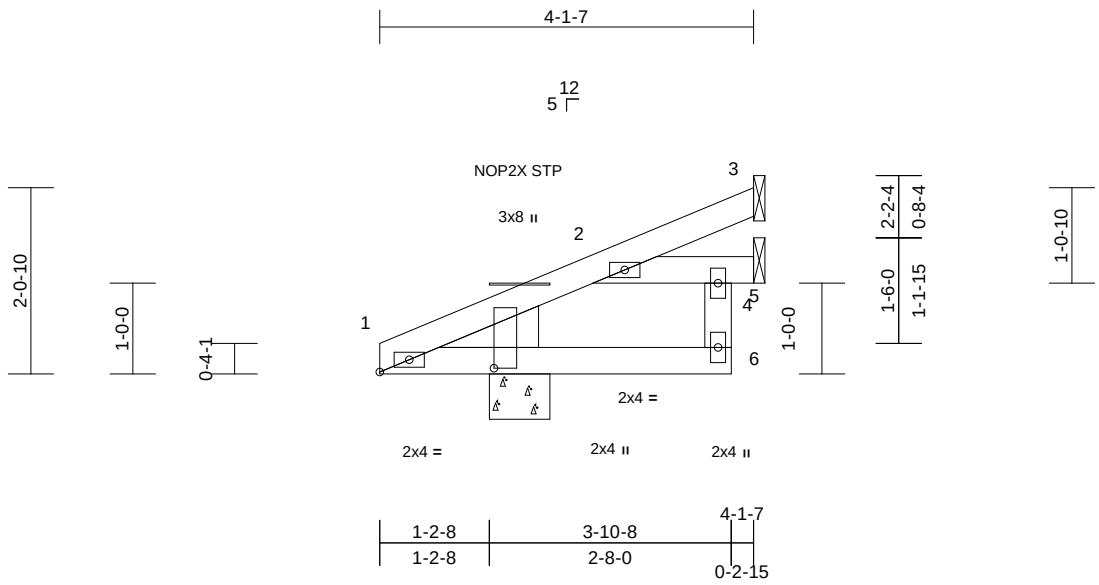

Scale = 1:25.4

Plate Offsets (X, Y): [1:0-1-14,0-0-10], [1:0-0-8,1-3-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	0.00	10	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.14	Vert(CT)	-0.01	10	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.00	7	>999	240	Weight: 19 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 6-5:2x4 SP No.3
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=338/0-8-0, 3=60/ Mechanical, 4=35/ Mechanical
Max Horiz 1=91 (LC 10)
Max Uplift 1=-70 (LC 10), 3=-27 (LC 10), 4=-21 (LC 10)
Max Grav 1=343 (LC 15), 3=63 (LC 15), 4=56 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-57/64, 2-3=-66/35
BOT CHORD 1-6=-75/162, 5-6=-19/24, 2-5=-52/67, 4-5=0/0

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 21 lb uplift at joint 4 and 70 lb uplift at joint 1.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659425
M2001623-20BX	CJ06	Corner Jack	4	1	Job Reference (optional)	

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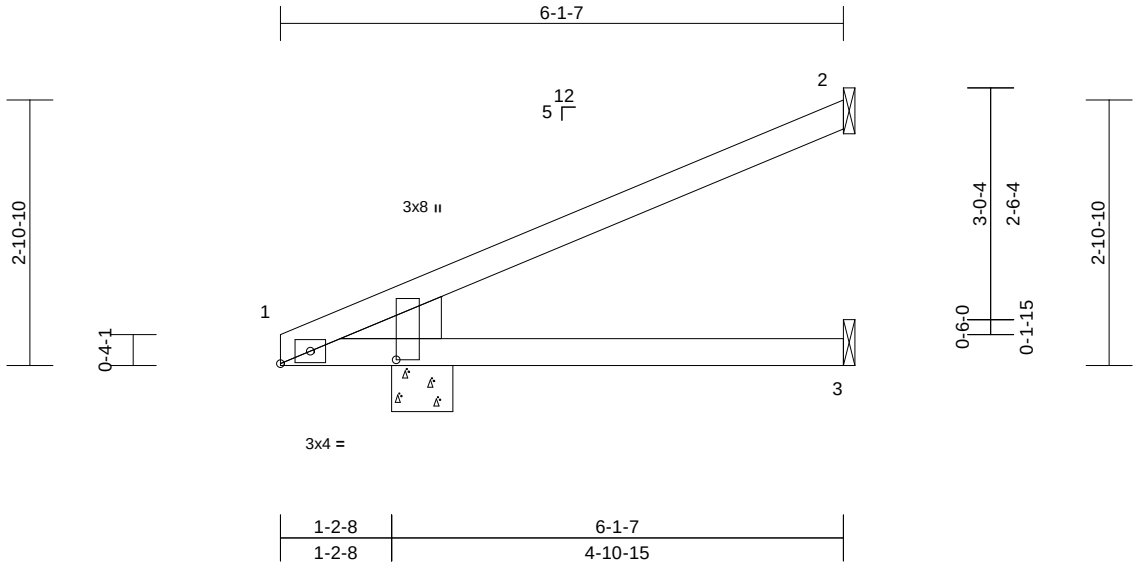

Scale = 1:25

Plate Offsets (X, Y): [1:0-1-14,0-0-2], [1:0-0-8,1-3-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.52	Vert(LL)	-0.02	3-7	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.47	Vert(CT)	-0.03	3-7	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.03	2	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	-0.07	4	>999	240	Weight: 22 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=406/0-8-0, 2=158/ Mechanical, 3=41/ Mechanical
Max Horiz 1=135 (LC 10)
Max Uplift 1=-96 (LC 10), 2=-115 (LC 10)
Max Grav 1=413 (LC 15), 2=174 (LC 15), 3=78 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-123/79
BOT CHORD 1-3=-121/302

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 2 and 96 lb uplift at joint 1.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659426
M2001623-20BX	CJ06P	Corner Jack	2	1	Job Reference (optional)	

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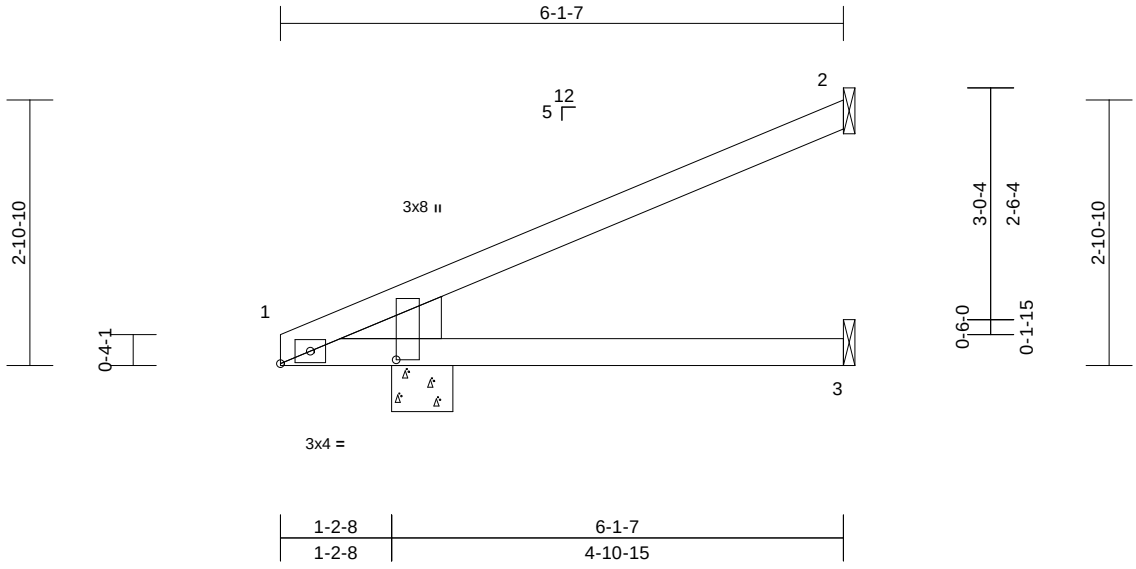

Scale = 1:25

Plate Offsets (X, Y): [1:0-1-14,0-0-2], [1:0-0-8,1-3-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.57	Vert(LL)	0.10	3-7	>714	240	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.55	Vert(CT)	-0.03	3-7	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.04	2	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS							Weight: 22 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=406/0-8-0, 2=158/ Mechanical, 3=41/ Mechanical
Max Horiz 1=135 (LC 10)
Max Uplift 1=-253 (LC 10), 2=-119 (LC 10), 3=-63 (LC 10)
Max Grav 1=406 (LC 1), 2=171 (LC 15), 3=78 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-131/72
BOT CHORD 1-3=-121/302

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 2, 63 lb uplift at joint 3 and 253 lb uplift at joint 1.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659427
M2001623-20BX	CJ06T	Corner Jack	2	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:47 Page: 1
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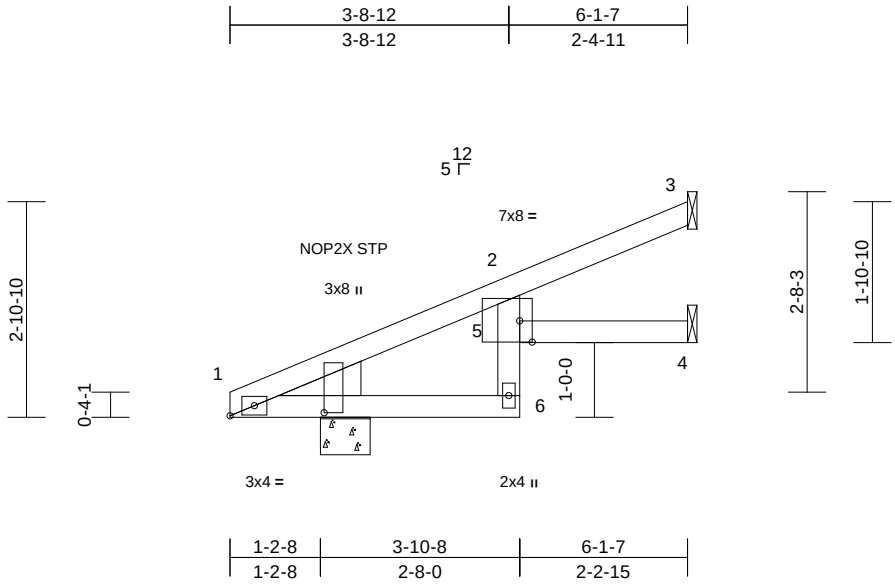

Scale = 1:30.8

Plate Offsets (X, Y): [1:0-1-14,0-0-2], [1:0-0-8,1-3-2], [2:0-2-0,0-3-7]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.24	Vert(LL)	-0.02	4-5	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.50	Vert(CT)	-0.04	4-5	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.02	4	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.05	4-5	>999	240	Weight: 24 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 6-2:2x4 SP No.3
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=406/0-8-0, 3=125/ Mechanical, 4=75/ Mechanical
Max Horiz 1=135 (LC 10)
Max Uplift 1=-96 (LC 10), 3=-79 (LC 10), 4=-26 (LC 10)
Max Grav 1=413 (LC 15), 3=136 (LC 15), 4=82 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-149/63, 2-3=-84/56
BOT CHORD 1-6=-214/136, 5-6=-28/15, 2-5=-74/148, 4-5=0/0

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 3, 26 lb uplift at joint 4 and 96 lb uplift at joint 1.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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Date:

July 13,2021

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659428
M2001623-20BX	EJ08	Jack-Open	15	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:47 Page: 1
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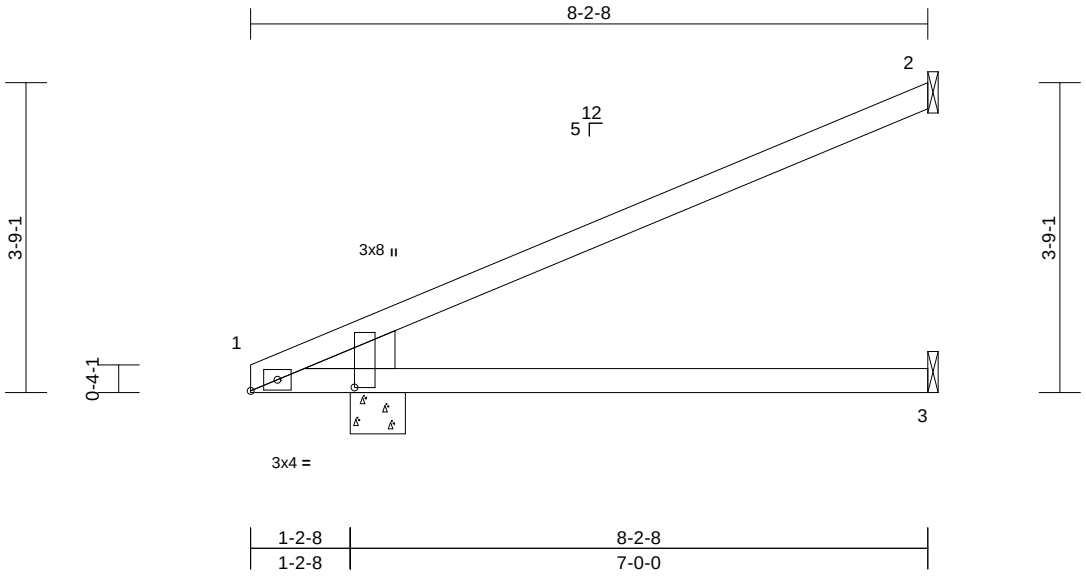

Scale = 1:27.9

Plate Offsets (X, Y): [1:0-1-14,0-0-2], [1:0-0-8,1-3-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	-0.07	3-7	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.72	Vert(CT)	-0.18	3-7	>548	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.08	2	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	-0.17	4	>568	240	Weight: 28 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP M 30
BOT CHORD 2x4 SP No.2
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=502/0-8-0, 2=242/ Mechanical, 3=71/ Mechanical
Max Horiz 1=182 (LC 10)
Max Uplift 1=-119 (LC 10), 2=-168 (LC 10)
Max Grav 1=510 (LC 15), 2=264 (LC 15), 3=120 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-164/96
BOT CHORD 1-3=-142/368

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 2 and 119 lb uplift at joint 1.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659429
M2001623-20BX	EJ08P	Jack-Open	4	1	Job Reference (optional)	

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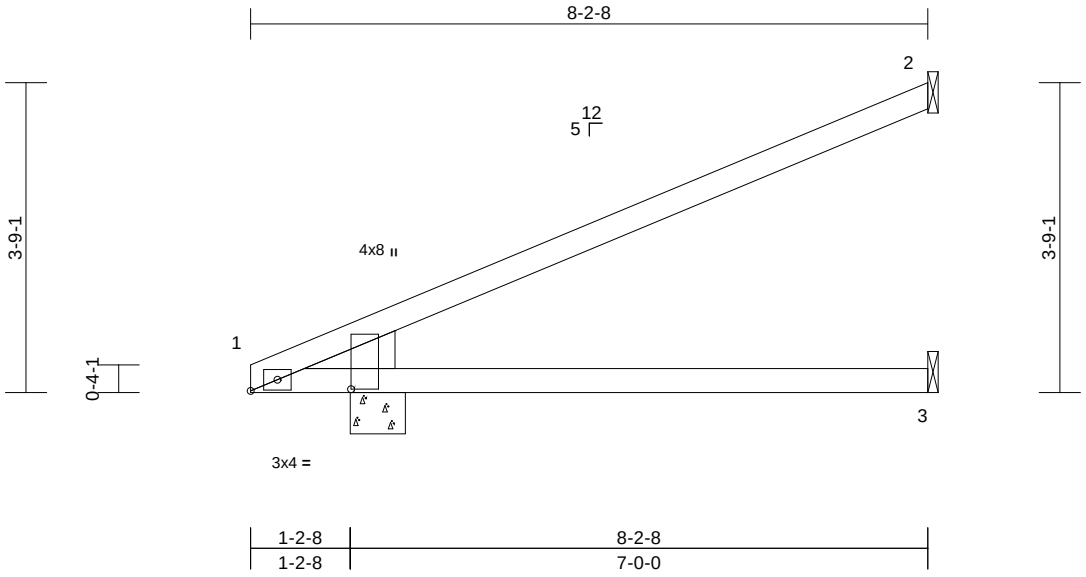

Scale = 1:27.9

Plate Offsets (X, Y): [1:0-1-14,0-0-2], [1:0-0-4,1-2-10]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.67	Vert(LL)	0.40	3-7	>245	240	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.68	Vert(CT)	-0.15	3-7	>637	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.12	2	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS							Weight: 28 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP M 30
BOT CHORD 2x4 SP M 30
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=502/0-8-0, 2=239/ Mechanical, 3=73/ Mechanical
Max Horiz 1=182 (LC 10)
Max Uplift 1=-313 (LC 10), 2=-176 (LC 10), 3=-95 (LC 10)
Max Grav 1=502 (LC 1), 2=253 (LC 15), 3=122 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-174/98
BOT CHORD 1-3=-142/368

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 2, 95 lb uplift at joint 3 and 313 lb uplift at joint 1.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659430
M2001623-20BX	EJ08T	Jack-Open	3	1	Job Reference (optional)	

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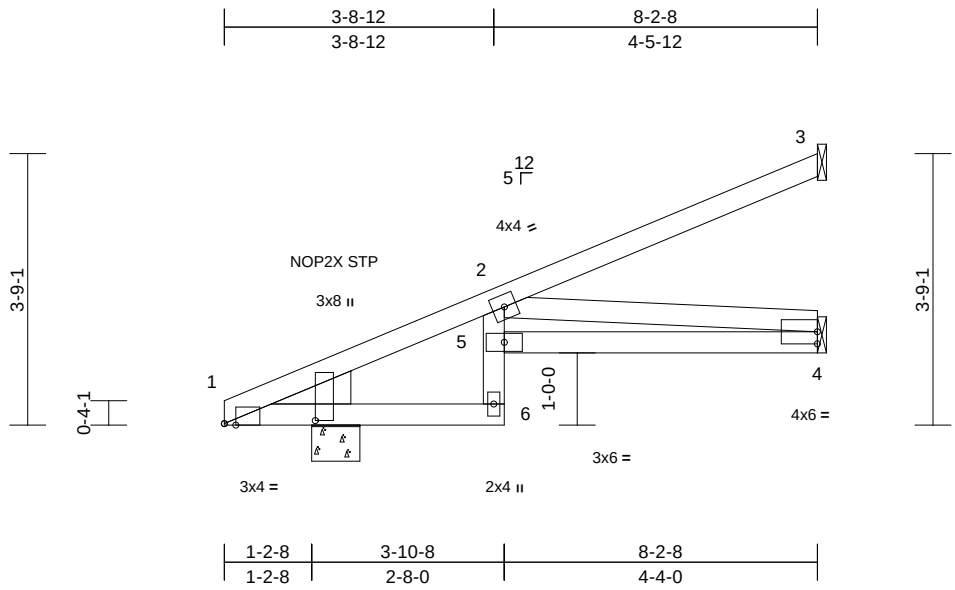

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Plate Offsets (X, Y): [1:0-1-14,Edge], [1:0-0-8,1-3-2]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.45	Vert(LL)	-0.03	5	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.72	Vert(CT)	-0.08	4-5	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.38	Horz(CT)	-0.05	4	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS		Wind(LL)	0.08	5	>999	240	Weight: 37 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=504/0-8-0, 3=174/ Mechanical, 4=138/ Mechanical
Max Horiz 1=182 (LC 10)
Max Uplift 1=-120 (LC 10), 3=-113 (LC 10), 4=-37 (LC 10)
Max Grav 1=512 (LC 18), 3=188 (LC 18), 4=157 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-350/186, 2-3=-110/68
BOT CHORD 1-6=-415/330, 5-6=-53/52, 2-5=-9/79, 4-5=-1121/893
WEBS 2-4=-900/1131

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 3, 37 lb uplift at joint 4 and 120 lb uplift at joint 1.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659431
M2001623-20BX	HJ8	Diagonal Hip Girder	2	1	Job Reference (optional)	

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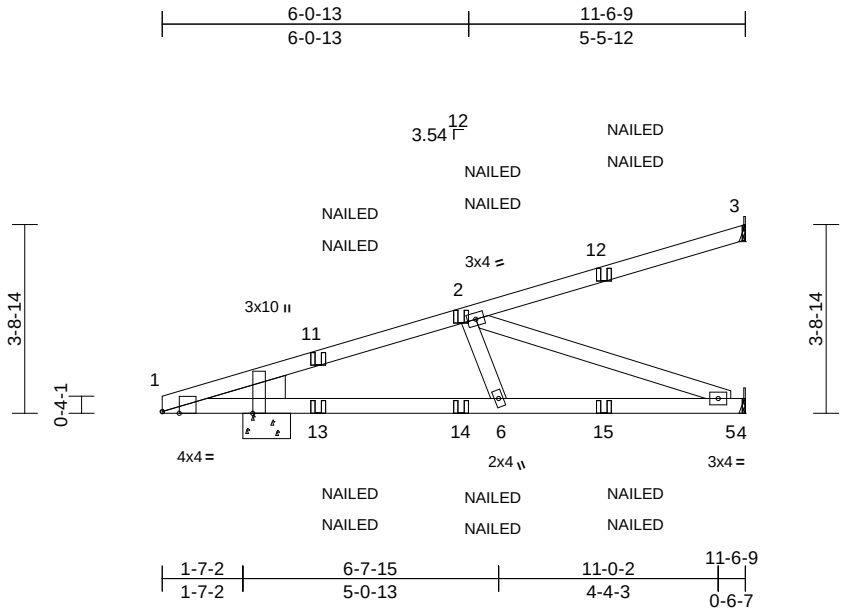

Scale = 1:45.6

Plate Offsets (X, Y): [1:0-4-0,Edge], [1:0-0-6,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.55	Vert(LL)	-0.07	7	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.62	Vert(CT)	-0.18	7	>760	240		
BCLL	0.0*	Rep Stress Incr	NO	WB	0.40	Horz(CT)	-0.06	3	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MSH		Wind(LL)	0.08	7	>999	240	Weight: 49 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP M 30
BOT CHORD 2x4 SP M 30
WEBS 2x4 SP No.3
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=356/0-11-5, 3=241/ Mechanical, 4=239/ Mechanical
Max Horiz 1=181 (LC 8)
Max Uplift 1=-395 (LC 8), 3=-158 (LC 8), 4=-109 (LC 8)
Max Grav 1=555 (LC 13), 3=249 (LC 13), 4=287 (LC 13)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-716/368, 2-3=-108/60
BOT CHORD 1-6=-432/664, 5-6=-408/627, 4-5=0/0
WEBS 2-6=-79/119, 2-5=-663/431

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 3, 109 lb uplift at joint 4 and 395 lb uplift at joint 1.
7) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (lb/ft)
Vert: 1-3=-80, 4-7=-20
Concentrated Loads (lb)
Vert: 11=162 (F=81, B=81), 12=-90 (F=-45, B=-45), 13=253 (F=126, B=126), 14=12 (F=6, B=6), 15=-26 (F=-13, B=-13)

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659432
M2001623-20BX	HJ8P	Diagonal Hip Girder	1	1	Job Reference (optional)	

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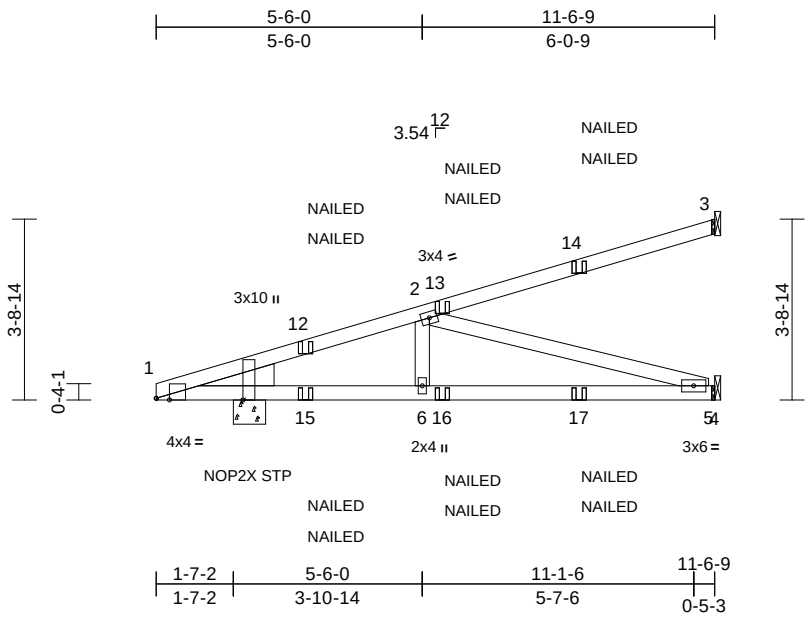

Scale = 1:47.6

Plate Offsets (X, Y): [1:0-3-4,Edge], [1:0-0-6,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.61	Vert(LL)	0.20	5-6	>670	240	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.96	Vert(CT)	-0.19	5-6	>730	240		
BCLL	0.0*	Rep Stress Incr	NO	WB	0.48	Horz(CT)	-0.03	3	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MSH							Weight: 50 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP M 30
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
WEDGE	Left: 2x6 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-9-3 oc bracing.

REACTIONS (lb/size) 1=338/0-8-0, 3=254/ Mechanical, 5=246/ Mechanical

Max Horiz 1=181 (LC 22)

Max Uplift 1=-558 (LC 8), 3=-179 (LC 8), 5=-326 (LC 8)

Max Grav 1=405 (LC 13), 3=255 (LC 13), 5=254 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-649/760, 2-3=-182/60
BOT CHORD	1-6=-829/620, 5-6=-829/620, 4-5=0/0
WEBS	2-6=-155/122, 2-5=-644/861

NOTES

1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 3, 326 lb uplift at joint 5 and 558 lb uplift at joint 1.
9) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (lb/ft)
   Vert: 1-3=-80, 4-7=-20
   Concentrated Loads (lb)
   Vert: 12=162 (F=81, B=81), 14=-90 (F=-45, B=-45), 15=253 (F=126, B=126), 16=12 (F=6, B=6), 17=-26 (F=-13, B=-13)

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Date:

July 13,2021

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	
M2001623-20BX	HJ8T	Diagonal Hip Girder	1	1	Job Reference (optional)	T24659433

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:48 Page: 1
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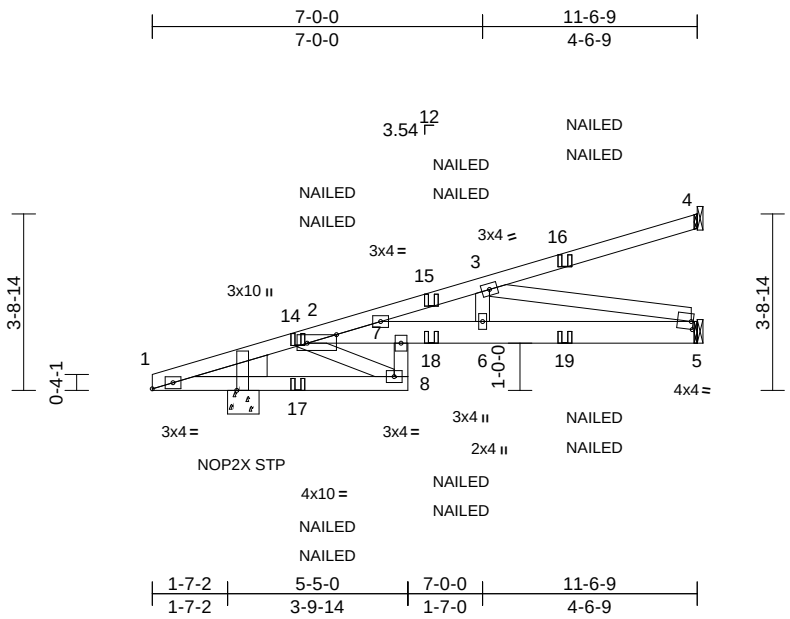

Scale = 1:48.8

Plate Offsets (X, Y): [1:0-3-4,0-0-1], [1:0-0-6,Edge], [2:0-7-7,Edge], [5:0-0-8,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.42	Vert(LL)	-0.04	6-7	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.43	Vert(CT)	-0.08	6-7	>999	240		
BCLL	0.0*	Rep Stress Incr	NO	WB	0.61	Horz(CT)	-0.03	5	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MSH		Wind(LL)	0.08	6-7	>999	240	Weight: 61 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 8-7:2x4 SP No.3, 2-5:2x6 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=356/0-8-0, 4=158/ Mechanical, 5=350/ Mechanical
Max Horiz 1=181 (LC 8)
Max Uplift 1=-426 (LC 8), 4=-104 (LC 8), 5=-230 (LC 8)
Max Grav 1=549 (LC 13), 4=162 (LC 13), 5=412 (LC 13)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-672/494, 2-3=-1507/935, 3-4=-105/37
BOT CHORD 1-8=-592/486, 7-8=-330/287, 2-7=-895/1346, 6-7=-1001/1435, 5-6=-1001/1435
WEBS 3-6=-247/315, 3-5=-1470/1025, 2-8=-464/574

NOTES
1) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.00
2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 4, 230 lb uplift at joint 5 and 426 lb uplift at joint 1.
8) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (lb/ft)
Vert: 1-4=-80, 8-9=-20, 5-7=-20
Concentrated Loads (lb)
Vert: 14=162 (F=81, B=81), 16=-23 (F=-12, B=-12), 17=253 (F=126, B=126), 18=-13 (F=-7, B=-7), 19=-93 (F=-46, B=-46)

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659434
M2001623-20BX	V01	Valley	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:48 Page: 1
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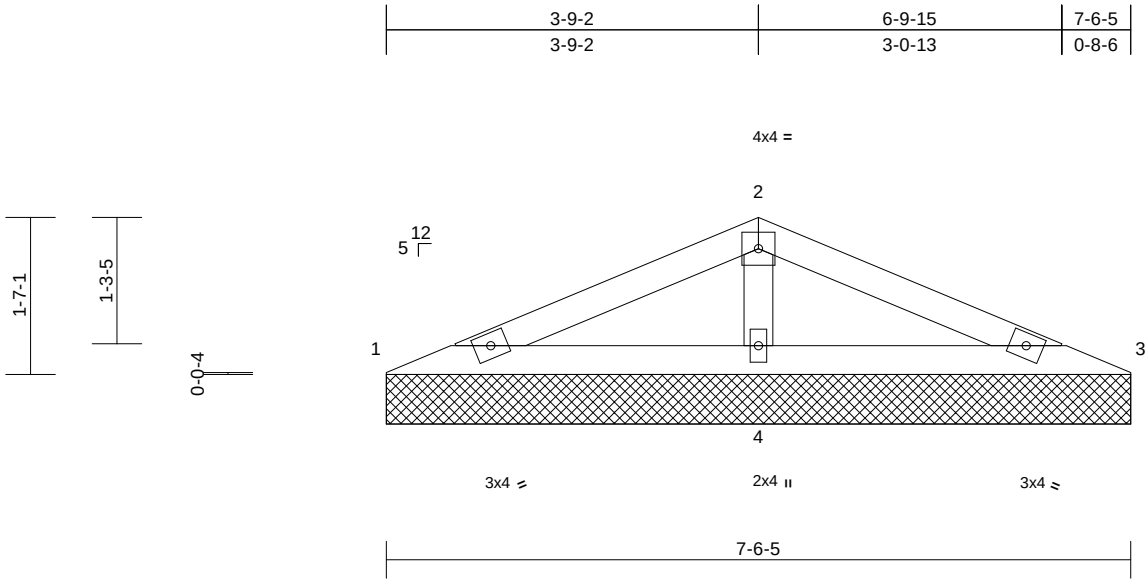

Scale = 1:23.3

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.24	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.12	Horiz(TL)	0.00	4	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-AS							Weight: 23 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 1=83/7-6-5, 3=83/7-6-5, 4=587/7-6-5
Max Horiz 1=-54 (LC 8)
Max Uplift 1=-27 (LC 10), 3=-27 (LC 10), 4=-197 (LC 10)
Max Grav 1=104 (LC 19), 3=104 (LC 20), 4=587 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-231/294, 2-3=-231/294
BOT CHORD 1-4=-287/317, 3-4=-287/317
WEBS 2-4=-407/410

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 1, 27 lb uplift at joint 3 and 197 lb uplift at joint 4.
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	1962-Roof	T24659435
M2001623-20BX	V02	Valley	1	1	Job Reference (optional)	

American Builders Supply, Inc., Winter Haven, FL - 33880, Run: 8.43 S Jun 2 2021 Print: 8.430 S Jun 2 2021 MiTek Industries, Inc. Tue Jul 13 07:12:48 Page: 1

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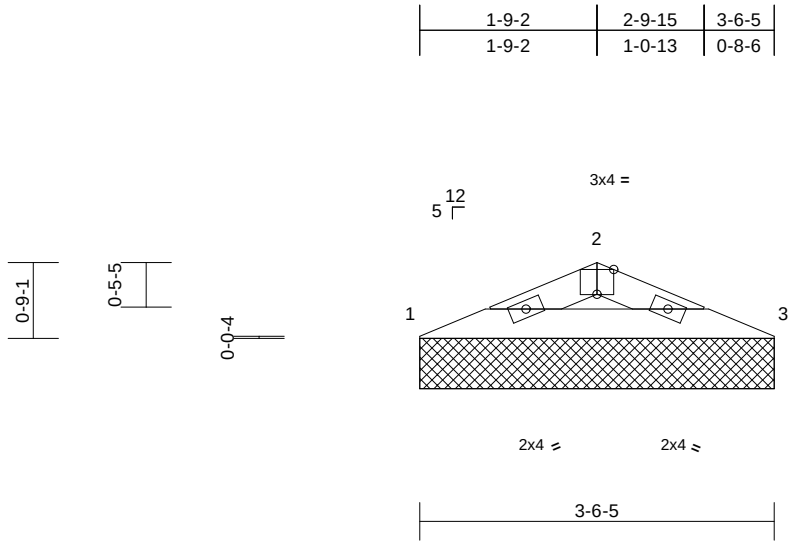

Scale = 1:22.9

Plate Offsets (X, Y): [2:0-2-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	20.0	Lumber DOL	1.25	BC	0.13	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MP							Weight: 9 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=176/3-6-5, 3=176/3-6-5
Max Horiz 1=22 (LC 9)
Max Uplift 1=-59 (LC 10), 3=-59 (LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-350/325, 2-3=-350/325
BOT CHORD 1-3=-276/310

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=160mph (3-second gust) Vasd=124mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=70ft; L=40ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.00
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 1 and 59 lb uplift at joint 3.

LOAD CASE(S) Standard

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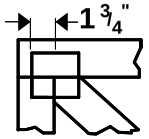
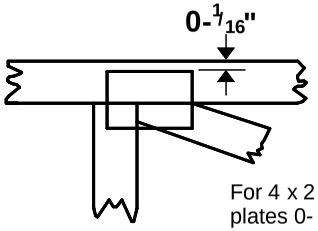
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

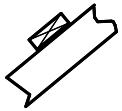
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

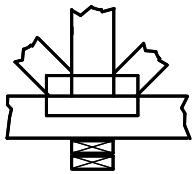
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

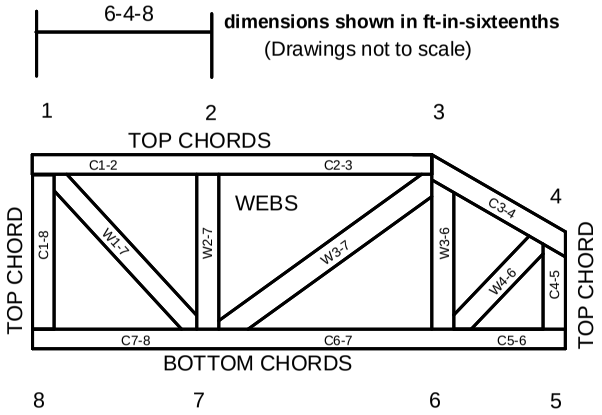


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

21.The design does not take into account any dynamic or other loads other than those expressly stated.