

1. THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND CIVIL DRAWINGS AND SPECIFICATIONS FORM A PART OF THESE DRAWINGS AND SHOULD BE USED ACCORDINGLY.
2. SPECIFICATIONS FOR INFORMATION TO BE SHOWN ON DRAWINGS.
3. CONTRACTOR TO VERIFY ALL DIMENSIONS AND CONDITIONS IN FIELD PRIOR TO FABRICATION AND CONSTRUCTION. ALL DIMENSIONS AND CONDITIONS TYPED INTO OR GOVERNED BY EXISTING CONSTRUCTION ARE APPROXIMATE AND ARE NOT CLAIMED TO BE CORRECT. ALL SUCH DIMENSIONS AND CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO THE PREPARATION OF SHOP DRAWINGS. IF CONDITIONS AND DIMENSIONS VARY FROM THOSE SHOWN, THE CONTRACTOR SHALL REPORT THE VARIATION TO THE ARCHITECT IMMEDIATELY.
4. NO PIPING SHALL PASS THROUGH BEAMS WITHOUT THE PERMISSION OF THE ARCHITECT. PIPES THAT PASS-THROUGH BEAMS SHALL PASS WITHIN THE MIDDLE THIRD OF THE BEAM LENGTH AND DEPTH.
5. TYPICAL DETAILS: DETAILS LABELED "TYPICAL DETAILS" ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS SHALL APPLY WHERE SHOWN AND WHERE NOT SHOWN AT EACH LOCATION. QUESTIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE DETERMINED BY THE ENGINEER.
6. DRAWING CONFLICTS: THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN EACH SET OF DRAWINGS AND WITHIN EACH SET OF DRAWINGS TO THE ARCHITECT AND ENGINEER PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.
7. ALL STRUCTURAL ELEMENTS OF THE PROJECT HAVE BEEN DESIGNED BY THE STRUCTURAL ENGINEER TO RESIST THE REQUIRED CODE SEISMIC AND LATERAL LOADS THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL REQUIRED BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE STRUCTURE IS TIED TOGETHER AND COMPLETED.
8. THE CONTRACTOR SHALL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND, EXCEPT WHERE SPECIFICALLY NOTED, DOES NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE.
9. INTERMITTENT SITE OBSERVATION BY FIELD REPRESENTATIVES OF SCHENK, ENDOM & FLANAGAN, LLC, IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN GENERAL ACCORDANCE WITH THE STRUCTURE CONSTRUCTION. THE OBSERVATION IS NOT TO BE CONSTRUED AS AN EXHAUSTIVE OR EXHAUSTING OR CONTINUOUS OR TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.
10. THE CONTRACTOR SHALL INVESTIGATE THE EXISTING ADJACENT BUILDING, SEWERS AND OTHER UTILITIES AND SHALL TAKE PROPER AND NECESSARY PRECAUTIONS TO PROTECT SAME FROM DAMAGE DUE TO THE EXECUTION OF NEW WORK, SUCH AS BRACING, SHORING, OR SHIELDING, TO MAINTAIN THE EXISTING STRUCTURE'S RESPONSIBILITY FOR REPAIRING OR REPLACING THE WORK IN ITS ORIGINAL CONDITION SHALL BE BORNE BY THE CONTRACTOR AT NO COST TO THE OWNER.
11. CONTRACTOR SHALL COORDINATE LOCATIONS, SIZES, EXTENT, ETC. OF CURBS, DEPRESSIONS, SLOPES, RECESSES, EMBEDDED ITEMS, OPENINGS IN SLAB, ETC. WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL AND OTHER DRAWINGS. THE INFORMATION SHOWN ON THESE DRAWINGS FOR INDICATED ITEMS IS SCHEMATIC IN NATURE AND MAY BE INCOMPLETE. SEE FIELD FOR EXISTING HAVING PLUMBING, ETC.) FOR THE INSTALLATION OF PIPES AND DUCTS. SLEEVES, THESE SLEEVES SHALL BE STEEL AND SHALL NOT INTERFERE WITH THE STRUCTURAL FRAMING, NOR SHALL THEY IMPAIR THE STRENGTH OF THE STRUCTURE.
12. PIPE SIZES INDICATED ARE NOMINAL DIAMETERS (TYP).

1. CONSTRUCTION Dewatering: The Contractor shall determine the extent of construction dewatering required for the excavation. The Contractor shall submit to the Engineer for review the proposed plan for construction dewatering prior to beginning the excavation. Provide adequate drainage to drain surface water away from the construction area. Motorized equipment shall not be allowed on exposed clay surfaces when wet.
2. All pile caps are to be centered under beams and columns unless otherwise noted.
3. PILING:
 - a. SHALL BE 14" DIAMETER CONCRETE AUGER CAST PILES. PILE CAPACITY IS 49 TONS. SEE SPECS FOR ADDITIONAL INFORMATION.
 - b. EXPANDED POLYSTYRENE (EPS) LIGHTWEIGHT FILL TO MEET ASTM D6817. EPS SHALL BE TREATED WITH TERMITICIDE TREATMENT FOR BELOW GRADE APPLICATIONS. BASIS-OF-DESIGN PRODUCT IS GEOFOAM EPS15 BY FOAM CONTROL. SEE FOUNDATION SECTIONS FOR TYPICAL APPLICATION. CONTRACTOR IS RESPONSIBLE FOR CONTROLLING EPS FLOTATION DURING CONSTRUCTION AND STABILIZING THE EPS INSTALLATION UNTIL COMPLETE WITH FIRST FLOOR SLAB INSTALLATION AND FINAL EXTERIOR GRADING AT THE PERIMETER OF THE BUILDING. ALL EPS FILL MATERIAL SHALL BE OVERBURDENED WITH SOIL FILL AS INDICATED BY THE DRAWINGS. UNDER SLAB PILING WITHIN EPS FILL MATERIAL MAY BE INSTALLED BY TAPPING THE EPS AND BACK FILLING WITH SPECIFIED SOIL MATERIAL. EPS FILL IS INDICATED 3'-0" BELOW TOP OF BUILDING SLAB DOWN TO EXISTING GRADE. THE EPS SHALL EXTEND DOWN TO EXISTING GRADE IN ALL CASES. SPECIFIED SOIL FILL MATERIAL SHALL BE PLACED IN ACCORDANCE WITH 02200-EARTHWORK REQUIREMENTS AT EXISTING GRADES BELOW BOTTOM OF EPS.

1. CONCRETE (SEE SPECS. FOR ADD'L INFORMATION)
 - a. 3000 PSI 28 DAY COMP. STRENGTH (NORMAL WEIGHT) U.O.N.
 - b. 3000 PSI 28 DAY COMP. STRENGTH (NORMAL WEIGHT) PEA-GRAVEL MIX AT REINFORCED CMU CELLS.
 - c. 4000 PSI 28 DAY COMP. STRENGTH (NORMAL WEIGHT) AT COMPOSITE MEAL DECK.
 - d. MAX. WATER/CEMENT RATIO AT FIRST FLOOR SLAB = 0.45
 - e. SUBMIT PROPOSED CONCRETE MIX DESIGNS TO ARCHITECT AND TESTING LABORATORY CONCURRENTLY FOR REVIEW AND APPROVAL.
 - f. COORDINATE ALL SLAB FINISHES WITH ARCHITECTURAL DOCUMENTS.
 - g. ALL PUMPED CONCRETE MUST CONTAIN SUPERPLASTICIZER.
 - h. FOR CONCRETE EXPOSED-TO-VIEW A SHRINKAGE INHIBITING ADMIXTURE SHALL BE USED.
2. ALL CONCRETE TO CONFORM WITH THE LATEST A.C.I. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE. (A.C.I. 308-11)
 - a. THE DESIGN OF CONCRETE MIXES, LOCATING OF CONSTRUCTION JOINTS IN SLABS, STAGGERING OF FOUR PLACEMENTS LOCATION OF POUR STRIPS, AND PLACEMENT AND CURING PROCEDURES ARE TO BE PERFORMED BY THE CONTRACTOR IN A MANNER THAT WILL MINIMIZE SHRINKAGE CRACKING OF THE SLABS. IN ADDITION TO MATERIALS SPECIFICALLY IDENTIFIED TO BE USED IN THE CONCRETE MIXES, FIBER REINFORCING STEEL MAY BE ADDED TO THE SLAB AND/OR FIBER REINFORCING MAY BE ADDED TO THE CONCRETE MIX AT THE CONTRACTOR'S OPTION. THE CONTRACTOR SHALL REPAIR ALL SHRINKAGE CRACKS DESIGNATED AS UNACCEPTABLE BY THE ARCHITECT BY INJECTION GROUTING WITH NO ADDITIONAL COST TO THE CONTRACTOR. REPAIR MATERIAL SHALL BE APPROPRIATE FOR THE APPLICATION AS RECOMMENDED BY THE MANUFACTURER. PRODUCTS SHALL BE BY SIKKA CORPORATION, OR MASTER BUILDERS, THE CONTRACTOR SHALL BE RESPONSIBLE TO REVIEW PRIOR TO THE DEVELOPMENT OF THE SLAB REINFORCING SHOP DRAWINGS, A PROPOSED SLAB CONSTRUCTION JOINT LAYOUT PLAN, ALONG WITH PROPOSED METHODS FOR CONTROLLING SHRINKAGE CRACKING IN THE SLABS.
3. FOR FLOOR COPS AND GRADE BEAMS, USE CONCRETE WEDGES FOR REINFORCING STEEL SUPPORTS. (BRICK OR MASONRY BLOCK IS NOT ACCEPTABLE) FIRST FLOOR SLAB REINF. TO BE SUPPORTED BY SAND CHAIRS @ 48"oc. MAX. E.W.
4. SECTIONS 2000 AND 2001 INDICATE ALL CONCRETE REINFORCING. CHECK SCHEDULES AND NOTES FOR FIVE COPS, BEAM, COLUMN AND SLAB REINFORCING.
 - a. FIRST FOUR FLOOR SLAB AREAS ON VAPOR RETARDER (15 MIL) OVER COMPACTED FILL.
 - b. ALL REINFORCING STEEL TO CONFORM WITH REQUIREMENTS OF A.S.T.M. A-615 GRADE 60.
 - c. PROVIDE REINFORCING BARS AS SHOWN ON DRAWINGS, OR GREATER IN SLABS AS SHOWN ON THE DRAWINGS.
 - d. PROVIDE BAR SUPPORTS AND SPACERS IN ACCORDANCE WITH REQUIREMENTS OF A.C.I. 315 UNLESS NOTED OTHERWISE.
 - e. CONDUIT SHALL NOT BE PLACED WITHIN THE FORMED SLABS. CONDUITS AT THE FIRST FLOOR SLAB SHALL BE PLACED BENEATH THE SLAB AS SHOWN ON THE DRAWINGS. LIKEWISE, CONDUITS SHALL NOT BE PLACED IN GRADE BEAMS.

1. DESIGN CRITERIA (2021 IBC)
- a. WIND DESIGN PARAMETERS
 - ULTIMATE DESIGN WIND SPEED = 145 MPH
 - EXPOSURE B
 - b. SEISMIC DESIGN PARAMETERS
 - RISK CATEGORY IV
 - IMPORTANCE FACTOR = 1.25
 - $S_s = 0.090g$, $S_{DS} = 0.096g$
 - $S_1 = 0.059g$, $S_{D1} = 0.094g$
 - SITE CLASS D
 - SEISMIC DESIGN CATEGORY C
 - $R=3.5$ (MOST CONSERVATIVE VALUE)
 - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
 - c. LATERAL SYSTEM: STEEL OMF, STEEL ECCENTRICALLY BRACED FRAME, CONCRETE SHEAR WALLS.
2. LOADS
- a. ROOF = 20 PSF
 - b. FIRST FLOOR (U.N.O.) = 100 PSF
 - c. OFFICE (U.N.O.) = 50 PSF
 - d. MECHANICAL ROOMS = 150 PSF
 - e. CORRIDOR (ABOVE FIRST FLOOR) = 80 PSF
 - f. PARTITIONS = 15 PSF (STEEL STUD FORMED)
3. GROUND SNOW LOAD (P_g) = 0 PSF
4. HELICOPTER LOADS
- a. THE DESIGN HELICOPTER IS AN AIRBUS H135 (FORMERLY EUROCOPTER EC135) WITH A MAXIMUM TAKEOFF WEIGHT OF 6,250 LBS.
- b. THE STATIC AND DYNAMIC DESIGN LOADS SHALL BE AS FOLLOWS:
- | | | | |
|---------------|------------|----------------|------------|
| 3,125 LBS. | 3,125 LBS. | 7,032 LBS. | 7,032 LBS. |
| ↓ | ↓ | ↓ | ↓ |
| <u>STATIC</u> | | <u>DYNAMIC</u> | |
- THESE LOADS ARE UNFACTORED AND SHALL BE USED WITH THE LOAD FACTORS DEFINED IN ASCE-7 SECTION 2.3 (LRFD OR 2.4 DYNAMIC LOADS SHALL BE ASSUMED TO OCCUR OVER A 0.2 SECOND DURATION).

1. DESIGN CRITERIA (2021 IBC)
 - a. WIND DESIGN PARAMETERS
 - ULTIMATE DESIGN WIND SPEED = 127 MPH
 - EXPOSURE B
 - b. SEISMIC DESIGN PARAMETERS
 - RISK CATEGORY II
 - IMPORTANCE FACTOR = 1.00
 - $S_{DS} = 0.120g$, $S_{D5} = 0.110g$
 - $S_1 = 0.067g$, $S_0 = 0.085g$
 - SITE CLASS D
 - SEISMIC DESIGN CATEGORY B
 - $R=3.5$ (MOST CONSERVATIVE VALUE)
 - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
 - LATERAL SYSTEM: STEEL OMRF, STEEL ECCENTRICALLY BRACED FRAME, CONCRETE SHEAR WALLS.
 - c. LIVE LOADS
 - ROOF = 20 PSF
 - FIRST FLOOR (U.N.O.) = 100 PSF
 - OFFICE (U.N.O.) = 50 PSF
 - MECHANICAL ROOMS = 150 PSF
 - CORRIDOR (ABOVE FIRST FLOOR) = 80 PSF
 - PARTITIONS = 15 PSF (STEEL STUD FORMED)
 - d. GROUND SNOW LOAD (P_g) = 0 PSF

1. MECHANICAL EQUIPMENT LOADS: THE GENERAL CONTRACTOR SHALL SUBMIT ACTUAL WEIGHTS OF EQUIPMENT TO BE USED IN THE PROJECT TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOADS USED IN THE DESIGN, AND SHALL PROVIDE CHANGES IN LOADS, NUMBER OF PIECES, AND WEIGHTS TO THE STRUCTURAL ENGINEER, AS SHOWN ON THE MECHANICAL/ELECTRICAL/PLUMBING DRAWINGS AT LEAST TWO WEEKS PRIOR TO FABRICATION AND CONSTRUCTION OF THE SUPPORTING STRUCTURE.
2. SPECIAL NOTES REGARDING ELEVATORS AND OTHER MECHANICAL OR ELECTRICAL EQUIPMENT:
 - a. PRIOR TO THE DETAILING OF ANY STRUCTURAL MATERIALS INVOLVED IN THE SUPPORT OF ELEVATORS AND OTHER MECHANICAL OR ELECTRICAL EQUIPMENT, THE GENERAL CONTRACTOR SHALL FURNISH TO THE ARCHITECT ALL INFORMATION RELATIVE TO LOADS AND DIMENSIONS, ETC. OF THE ACTUAL EQUIPMENT WHICH IS TO BE USED. ALL DETAILS SHOWN ON THESE DRAWINGS ARE TENTATIVE UNTIL SUCH TIME THAT THIS INFORMATION IS REVIEWED BY THE ARCHITECT.
 - b. THE CONTRACTOR SHALL PROVIDE AND COORDINATE THE INSTALLATION OF ELEVATOR SILL ANGLES AND OTHER ACCESSORY ATTACHMENTS WITH ELEVATOR MANUFACTURER'S REQUIREMENTS. ANCHORS SHALL BE CAST-IN WELD STUDS OR APPROVED EQUAL. ALL OTHER ELEVATOR EMBEDMENTS REQUIRED SHALL BE PROVIDED AND COORDINATED BY THE GEN. CONTRACTOR.

1. ALL STRUCTURAL STEEL TO CONFORM WITH LATEST REQUIREMENTS OF ASTM AND AISC FOR FABRICATION AND ERECTION OF STRUCTURAL STEEL.
2. STRUCTURAL STEEL
 - a. WIDE FLANGE STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, GRADE 50. ALL OTHER STRUCTURAL SHAPES SHALL CONFORM TO A36, UON.
 - b. STEEL TUBE - ASTM A500, GRADE B ($F_y = 42KSI$)
 - c. STEEL PIPE - ASTM A500, GRADE B ($F_y = 42KSI$)
 - d. PLATES AND BARS - ASTM A36, UON
 - e. ALL STRUCTURAL BOLTS SHALL BE A-325 UNLESS OTHERWISE NOTED.
 - f. ALL ANCHOR BOLTS SHALL BE A-307 UNLESS OTHERWISE NOTED.
3. ALL EMBEDDED STEEL ITEMS EXCLUDING COLUMN ANCHOR BOLTS (U.N.O.) SHALL BE FABRICATED IN ACCORDANCE WITH CRIPPER/STANLEY'S CO. STANDARD SPECIFICATIONS AND SHALL BE A.S.T.M. A-36 U.N.O. ALL EMBEDDED STEEL SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION.
4. ALL WELDING SHALL CONFORM WITH THE LATEST EDITION OF AWS CODE. ALL WELDERS (SHOP & FIELD) SHALL BE AWS CERTIFIED FOR THE TYPE OF WELDING BEING PERFORMED.
5. WELDING INDICATED ON THESE DWGS. MAY BE SHOP OR FIELD PERFORMED AT CONTRACTOR'S OPTION AND AS REQ'D. TO OBTAIN THE BEST QUALITY AND FIT-UP. FIELD WELDING MUST BE PERFORMED WHERE SPECIFICALLY NOTED. ALL WELDS ON SLAB AND ROOF BENT PLATES, ANGLES AND OTHER MISC. EXTERIOR OR INTERIOR WALL ALIGNMENT ITEMS SHALL BE FIELD WELDED TO BEAMS. PROVIDE FABRICATION AND ERECTION TOLERANCES MORE STRINGENT THAN SPECIFIED BY AISC WHERE REQUIRED TO ACHIEVE THE DETAILS INDICATED.
6. ALL WELDING SHALL BE DONE USING EXXON ELECTRODES, UON
 - a. UNLESS OTHERWISE INDICATED AT THE STRUCTURAL DETAILS PROVIDE THE ANGLES, TUBES, PLATES, CHANNELS, AND OTHER STEEL MEMBERS/PIECES THROUGH AN ARCHITECTURAL DRAWING AND CONNECT WITH 1/4" FILLET WELDED AT ALL MATERIAL PIECE INTERFACES. UNLESS OTHERWISE INDICATED THE FOLLOWING ASSUMPTIONS SHALL BE MADE REGARDING SPACING AND MEMBER SIZE IN ORDER TO ESTABLISH AN ALL-INCLUSIVE STRUCTURAL STEEL BID PRICE.
 - a. PLATE = 5x16
 - b. CONT. BENT PLATE = 1/4"
 - c. ANGLE = 1.5x5x5/16
 - d. CHANNEL (HORIZONTALLY ORIENTED) = C12x30
 - e. CHANNEL (VERTICALLY ORIENTED) = C6x8.2
 - f. WIDE FLANGE = W14x22
 - g. TUBE = HSS6x1/4"
 - b. MEMBERS/PIECES SHOWN IN SECTION SHALL BE ASSUMED AS CONTINUOUS UNLESS OTHERWISE DETAILED.
7. SPACING INTERVALS OF STIFFENERS, HANGERS, AND KICKERS = 32" O.C. U.N.O.
8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATION AND INSTALLATION OF ALL MISCELLANEOUS STEEL ITEMS (ANGLES, BRACKETS, TUBES, CHANNELS, RODS, PLATE, BAR ETC.) INDICATED, DESCRIBED, OR IMPLIED IN THE DRAWINGS WHETHER SHOWN ON THE ARCHITECTURAL AND/OR STRUCTURAL DRAWINGS.
9. FOR WORK WHICH WILL BE EXPOSED TO VIEW, USE ONLY MATERIALS WHICH ARE SMOOTH AND FREE OF SURFACE BLEMISHES INCLUDING PITTING, RUST AND SCALE, SEAM MARKS, ROLLER MARKS, ROLLED TRADE NAMES AND ROUGHNESS REMOVE SUCH BLEMISHES BY GRINDING, OR BY WELDING AND GRINDING PRIOR TO CLEANING, TREATING AND APPLICATION OF FINISH. FINISH OF STRUCTURAL STEEL EXPOSED TO VIEW SHALL BE AS SPECIFIED IN FINAL PRODUCT SHEET WITH AN "ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS)" SPECIFICATION, CATEGORY AESS 3, INCLUDING THE REMOVAL OF BACKER-BARS AND BACK-GOUGING AT FULL-PENETRATION WELDS, UNLESS NOTED OTHERWISE. ALSO SEE AESS CATEGORY MATRIX.
10. ALL FASTENERS JOINING AND/OR CONNECTING HOT DIP GALVANIZED STRUCTURAL ELEMENTS INCLUDING BUT NOT LIMITED TO ANCHOR BOLTS, NUTS, WASHERS, & EMBEDDED PLATES SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A153. ALL DAMAGED COATING, INCLUDING DAMAGE DUE TO WELDING, SHALL BE REPAIRED WITH ZINC BASED SOLDER OR STICK GALVANIZING IN ACCORDANCE WITH ASTM A780.

2. ALL BOLTS INTO EXISTING CONCRETE USE HILTI-HY 200 ADHESIVE ANCHOR OR APPROVED EQUAL.

1. PROVIDE AN ALLOWANCE FOR 6 TONS OF MISCELLANEOUS STEEL DETAILED, DELIVERED, AND INSTALLED. THIS STEEL WILL BE USED TO PROVIDE ANY EQUIPMENT SUPPORT REVISIONS THAT MAY BE REQUIRED BASED ON THE FINAL EQUIPMENT SELECTIONS, FOR FRAMING OR REFRAMING ROOF & SLAB OPENINGS NEEDED FOR THE INSTALLATION OF UPGRADED MEP EQUIPMENT AND OTHER REVISIONS AND ITEMS.
2. PROVIDE AN ALLOWANCE FOR 48 YARDS OF REINFORCED CONCRETE DETAILED, DELIVERED, AND INSTALLED TO ADDRESS REQUIRED REVISIONS OF CONCRETE WORK IN THE FIELD. THE ALLOWANCE SHALL BE USED AS DIRECTED BY THE DESIGN TEAM.
3. PROVIDE A UNIT COST FOR INSTALLED JOB PILES (AUGERCAST) PER PILE AND PER 20 PILES.
4. PROVIDE A UNIT COST FOR INSTALLED COMPACTED STRUCTURAL FILL PER CU. YARD.

	BELOW	FF	FAR FACE	OC	ON CENTER
(N)	NEW	FDN	FOUNDATION	OD	OUTSIDE DIAMETER
Ø	DIAMETER	FIN	FINISH	OH	OPPOSITE HAND
&	AND	FF	FINISH FLOOR	OPNG	OPENING
@	AT	FLR	FLOOR	OPP	OPPOSITE
		FO	FACE OF		
AA	ADHESIVE ANCHOR	FOC	FACE OF CONCRETE	P	PILE
AB	ANCHOR BOLT	FOM	FACE OF MASONRY	PAF	POWDER ACTUATED FASTENER
ABV	ABOVE	FOS	FACE OF STUDS	PC	PIPE COLUMN, (STEEL)
AC	ASPHALT CONCRETE	FOW	FACE OF WALL	PCS	PIECES
ADDL	ADDITIONAL	FP	FIREPROOFING	PL	PLATE, (STEEL)
ADJ	ADJACENT	FS	FAR SIDE	PERP	PERPENDICULAR
AGGR	AGGREGATE	FT	FOOT or FEET	PP	PARTIAL PENETRATION
AL	ALUMINUM	FTG	FOOTING	PR	PAIR
ALT	ALTERNATE	FJ	FLOOR JOIST	PT	POINT
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE			PTN	PARTITION
		GA	GAUGE	PSF	POUNDS PER SQ. FT.
APPROX	APPROXIMATE	GB	GRADE BEAM	PSI	POUNDS PER SQ. IN.
ARCH	ARCHITECTURAL	GALV	GALVANIZED		
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	GL	GLASS or GLAZING		
		GRND	GROUND	RO	ROUGH OPENING
ATR	ALL THREAD ROD	GR	GRADE	REF	REFERENCE
AWG	AMERICAN WIRE GAUGE	GTS	GIRDER TRUSS	REINF	REINFORCED or REINFORCING
		GYP BD	GYPSPUM BOARD	REM	REMAINDER
				REQ	REQUIRED
BTWN	BETWEEN			REV	REVISION
BLDG	BUILDING	HDG	HOT DIPPED GALVANIZED	RFG	ROOFING
BM	BEAM	HDR	HEADER		
BOF	BOTTOM OF FOOTING	HP	HIGH POINT		
BOTT	BOTTOM	HSB	HIGH STRENGTH BOLT	SAD	SEE ARCHITECTURAL DRAWINGS
BP	BASE PLATE	HT	HEIGHT	SCHED	SCHEDULE
BRG	BEARING	HK	HOOK	SECT	SECTION
BS	BOTH SIDES	HORIZ	HORIZONTAL	SHT	SHEET
				SIM	SIMILAR
C	CENTER LINE	ID	INSIDE DIAMETER	SL	SLOPE
CHANNEL		INT	INTERIOR	SMS	SHEET METAL SCREW
CANT	CANTILEVER	in	INCH	SW	SHEAR WALL
CBR	CAMBER	INFO	INFORMATION	SG	SLAB ON GRADE
CIP	CAST IN PLACE			SPECS	SPECIFICATIONS
CL	CENTER LINE	JST	JOIST	SQ	SQUARE
CJ	CONSTRUCTION JOINT (COLD JOINT)	JT.	JOINT	SS	STAINLESS STEEL
				STAGG	STAGGER or STAGGERED
CLG	CEILING	K	KIP	STD	STANDARD
CLR	CLEAR	KO	KNOCK OUT	STIFF	STIFFENER
CMU	CONCRETE MASONRY UNIT			STR	STIRRUP
COL	COLUMN	L	ANGLE STEEL	STL	STEEL
CONC	CONCRETE	LAT	LATERAL	STRUCT	STRUCTURAL
CONN	CONNECTION	LBS	POUNDS	SUBST	SUBSTITUTE
CONST	CONSTRUCTION	LD	DEVELOPMENT LENGTH	SUSP	SUSPENDED
CONT	CONTINUOUS	LDH	HOOK DEVELOPMENT LENGTH	SYM	SYMMETRICAL
CP	COMPLETE PENETRATION	LEV	LEVEL		
CNT	CENTER	LLB	LONG LEG BACK / BACK	T&B	TOP AND BOTTOM
		LLH	LONG LEG HORIZONTAL	THK	THICK
DBL	DOUBLE	LLV	LONG LEG VERTICAL	THRD	THREADED
DECK	DECK, DECKING	LOC	LOCATION	THRU	THROUGH
DEMO	DEMOLITION	LONG	LONGITUDINAL	TO	TOP OF
DET	DETAIL	LP	LOW POINT	TOC	TOP OF CONCRETE
DP	DEEP	LPS	LAP SPLICE LENGTH	TOF	TOP OF FOOTING
DL	DEEP LEG	LT	LIGHT	TOS	TOP OF STEEL
DIAG	DIAGONAL	LGS	LIGHT GAUGE STEEL	TOSLAB	TOP OF STRUCTURAL SLAB
DIA	DIAMETER	LWC	LIGHT WEIGHT CONCRETE	TOW	TOP OF WALL
DIM	DIMENSION			TR	TRIPLE
DIST	DISTANCE	MAS	MASONRY	TW	TREAD
DN	DOWN	MANUF	MANUFACTURER	TS	TUBE STEEL
DO	DITTO "THE SAME"	MAX	MAXIMUM	TRANS	TRANSVERSE
DR	DROP	MB	MACHINE BOLT	TPY	TYPICAL
DWL	DOWEL	MEZZ	MEZZAINE		
DWG	DRAWING	MECH	MECHANICAL	UNO	UNLESS NOTED OTHERWISE
		MEP	MECHANICAL, ELECTRICAL PLUMBING DOCUMENTS	UON	UNLESS OTHERWISE NOTED
EA	EACH	MTL	METAL	VERT	VERTICAL
E.A.	EXPANSIVE ANCHOR	MF	MOMENT FRAME	VIF	VERIFY IN FIELD
EE	EACH END	MIN	MINIMUM		
EF	EACH FACE	MISC	MISCELLANEOUS	WF	WIDE FLANGE
EQ	EQUAL	MTD	MOUNTED	W/	WITH
EQUIP	EQUIPMENT			W/O	WITHOUT
ES	EACH SIDE	N	NORTH	WP	WORK POINT
EW	EACH WAY	NF	NEAR FACE	WT	WEIGHT
ELEV	ELECTRICAL	NIC	NOT IN CONTRACT	WWF	WELDED WIRE FABRIC
EMBED	EMBEDMENT	NS	NEAR SIDE		
EOS	EDGE OF SLAB	NTS	NOT TO SCALE	X HVY	EXTRA HEAVY
EJ	EXPANSION JOINT	No or #	NUMBER	XX HVY	DOUBLE EXTRA HVY
EV	EVERY	NOM	NOMINAL (DIAMETER)	X STR	EXTRA STRONG
EPS	EXPANDED POLYSTYRENE	NWC	NORMAL WEIGHT CONCRETE	XX STR	DOUBLE EXTRA STRONG
EXCAV	EXCAVATION				
EXP	EXPANSION				
EXT	EXTERIOR				

[illegible]

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AND

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AT

ST. TAMMANY PARISH HOSPITAL

Date
05.NOV.2025

Job No. 222058.01

Drawn by _____ Checked by _____
Author Checker

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Sheet No.

Sheet No.

AESS CATEGORY MATRIX					
CATEGORY		AESS 3	AESS 2	AESS 1	SSS
ID	CHARACTERISTICS	FEATURE ELEMENTS IN CLOSE VIEW	FEATURE ELEMENTS NOT IN CLOSE VIEW	BASIC ELEMENTS	STANDARD STRUCTURAL STEEL
1.1	SURFACE PREPARATION TO SSPC-SP 6	X	X	X	
1.2	SHARP EDGES GROUND SMOOTH	X	X	X	
1.3	CONTINUOUS WELD APPEARANCE	X	X	X	
1.4	STANDARD STRUCTURAL BOLTS	X	X	X	
1.5	WELD SPATTERS REMOVED	X	X	X	
2.1	VISUAL SAMPLES	X	OPTIONAL		
2.2	ONE-HALF STANDARD FABRICATION TOLERANCES	X	X		
2.3	FABRICATION MARKS NOT APPARENT	X	X		
2.4	WELDS UNIFORM AND SMOOTH	X	X		
3.1	MILL MARKS REMOVED	X			
3.2	BUTT AND PLUG WELDS GROUND SMOOTH AND FILLED	X			
3.3	HSS WELD SEAM ORIENTED FOR REDUCED VISIBILITY	X			
3.4	CROSS SECTIONAL ABUTTING SURFACE ALIGNED	X			
3.5	JOINT GAP TOLERANCES	X			
3.6	ALL WELDED CONNECTIONS	OPTIONAL			

LEGEND			
SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
	DETAIL NO. SHEET LOCATION		INDICATES STEP IN SLAB, S.A.D. FOR DIFFERENTIAL AND LOCATION
	VIEW DIRECTION (SHADED) SECTION/ELEVATION NO. SHEET LOCATION		INDICATES SHEAR STUD LAYOUT PATTERN
	ELEVATION RELATIVE TO 1ST FLOOR TOP OF SLAB ELEVATION 0'-0" NOTE		INDICATES UPSET (+) OR DEPRESS (-) FROM REFERENCE ELEV.
	TOP OF PILE CAP ELEVATION		INDICATES UPWARD/POSITIVE CAMBER AT MIDSPAN OF BEAM
	TOP OF GRADE BEAM ELEVATION		WORK POINT
	TOP OF PILE ELEVATION		INDICATES SLOPED BEAM, SLAB, OR DECK WHERE ARROWHEAD INDICATES DOWNWARD (-) OR UPWARD (+) DIRECTION
	INDICATES FIELD WELD, SHOP WELD WHEN NOT SHOWN		SHEET NOTE CALLOUT
	WELD NOTES CENTER TO CENTER WELD SPACING (INCHES) WELD LENGTH (INCHES) WELD TYPE WELD SIZE (REFER TO AISC, LATEST EDITION, FOR ALL WELD TYPES & SYMBOLS)		REVISION NO.
(E)	EXISTING ELEMENT		AFFECTED REGION DUE TO CURRENT REVISION
			BEAM TO COLUMN MOMENT CONNECTION PER X/XXX
			BEAM TO BEAM MOMENT CONNECTION PER X/XXX

FOUNDATION PLAN LEGEND/NOTES	
MARK	DESCRIPTION
	8" CONCRETE SLAB. REINF. W/ #5 @ 10"o.c. EA. WAY TOP. & #4 @ 10"o.c. EA. WAY BOTT.
	5" SOIL SUPPORTED CONCRETE SLAB W/ STIFFENING BMS. REINF. W/ #4 @ 10"o.c. EA. WAY @ MID-DEPTH.
	FORMED SONOTUBE SLAB SUPPORT.
	6" TALL x 6" WIDE CONCRETE CURB. REINF. W/ #4 CONT. & #3 x 0'-9" HOOKED BARS DRILL & EPOXY GROUT @ 16"o.c. W. HILTI HY 150 OR EQUAL.
	1'-4" TALL x 7 1/4" WIDE CONCRETE CURB. REINF. W/ (2) #4 CONT. & (2) #3 x 1'-7" HOOKED BARS DRILL & EPOXY GROUT @ 16"o.c. W. HILTI HY 150 OR EQUAL.

FRAMING PLAN LEGEND/NOTES	
MARK	DESCRIPTION
	6 1/4" (TOTAL) SLAB SYSTEM. 3 1/4" LT.WT. CONCRETE ATOP 3"-18 GA. GALV. COMPOSITE METAL DECK.
	3"-18 GA. GALV. TYPE B ROOF DECK.
	1 1/2"-20 GA. GALV. TYPE B ROOF DECK.

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STRUCTURAL LEGENDS

Job No. 222058.01

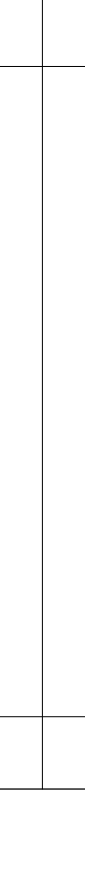
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SPECIAL INSPECTION PER THE 2021 IBC:					
1. THE OWNER WILL EMPLOY THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE SPECIAL INSPECTIONS DURING CONSTRUCTION FOR THE REQUIRED SPECIAL INSPECTION ITEMS. 2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON FROM AN APPROVED AGENCY WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. 3. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH: a. THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAIVE ANY OF THE REQUIREMENTS OF THE DOCUMENTS. b. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE PROFESSIONAL-OF-RECORD, AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. THEN, IF UNCORRECTED, SUBMIT A COMPLETE LIST OF ALL OUTSTANDING DISCREPANCIES ON A WEEKLY BASIS TO THE OWNER, THE BUILDING OFFICIAL, AND THE PROFESSIONAL-OF-RECORD, UNTIL ALL CORRECTIONS HAVE BEEN COMPLETED. c. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE BUILDING CODE. 4. WHERE SPECIAL INSPECTION REQUIREMENTS DUPLICATE THE REQUIREMENTS OF OTHER SPECIFIED TESTING, DUPLICATE INSPECTIONS SHALL NOT BE REQUIRED. 5. STRUCTURAL OBSERVATION (AS DEFINED IN CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE) IS NOT REQUIRED, UNLESS SPECIFICALLY REQUIRED BY THE BUILDING OFFICIAL.					
LIST OF SPECIAL INSPECTIONS		CONT.	PERIODIC	N/A	REFERENCED STANDARD
SOIL					IBC TABLE 1705.6
	1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	-	X	-	
	2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	-	X	-	
	3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	-	X	-	
	4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X	-	-	
	5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	-	X	-	
CONCRETE					IBC TABLE 1705.3
	1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	-	X	-	ACI 318 CH. 20, 25.2, 25.3, 26.6.1-26.6.3
	2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706 b. INSPECT SINE-WAVE FILLET WELDS, MAXIMUM 5/16" c. INSPECT ALL OTHER WELDS	- - X	X X -	- - -	AWS D14; ACI 318: 26.6.4
	3. INSPECT ANCHORS CAST IN CONCRETE	-	X	-	ACI 318: 17.8.2
	4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED LOAD TENSION LOADS b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a	X - -	- X -	- - -	ACI 318: 17.8.2.4 ACI 318: 17.8.2
	5. VERIFYING USE OF REQUIRED DESIGN MIX	-	X	-	ACI 318: CH. 19, 26.4.3, 26.4.4
	6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	-	-	ASTM C172; ASTM C31; ACI 318: 26.5, 26.12
	7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	-	-	ACI 318: 26.5
	8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	-	X	-	ACI 318: 26.5.3-26.5.5
	9. INSPECT PRESTRESSED CONCRETE FOR: a. APPLICATION OF PRESTRESSING FORCES b. GROUTING OF BONDED PRESTRESSING TENDONS	X X	- -	- -	ACI 318: 26.10
	10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	-	X	-	ACI 318: CH. 26.9
	11. FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MODE OR HIDE) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C,D,E, OR F, INSPECT SUCH CONNECTIONS AND REINFORCEMENT IN THE FIELD FOR: a. INSTALLATION OF THE EMBEDDED PARTS b. COMPLETION OF THE CONTINUITY OF REINFORCEMENT ACROSS JOINTS c. COMPLETION OF CONNECTIONS IN THE FIELD	X X X	- - -	- - -	ACI 550.5
	11. INSPECT INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNECTIONS FOR COMPLIANCE WITH ACI 550.5.	X	-	-	ACI 318: 26.13.1.3
	13. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	-	X	-	ACI 318: 26.11.2
	14. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	-	X	-	ACI 318: 26.11.1.2(b)
	FOUNDATIONS				
1. DRIVEN DEEP FOUNDATION ELEMENTS a. VERIFY ELEMENT MATERIALS, SIZES, AND LENGTHS COMPLY WITH THE REQUIREMENTS b. DETERMINE CAPACITIES OF TEST ELEMENTS AND CONDUCT ADDITIONAL LOAD TESTS, AS REQUIRED c. INSPECT DRIVING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT d. VERIFY PLACEMENT LOCATIONS AND PLUMBNESS; CONFIRM TYPE AND SIZE OF HAMMER, RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION, DETERMINE REQUIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP AND BUTT ELEVATIONS, AND DOCUMENT ANY DAMAGE TO FOUNDATION ELEMENT e. FOR CONCRETE ELEMENTS AND CONCRETE-FILLED ELEMENTS, PERFORM TESTS ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.3		- X X X -	X - - - X	- - - -	IBC TABLE 1705.8
	2. CAST-IN-PLACE DEEP FOUNDATION ELEMENTS a. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT b. VERIFY PLACEMENT LOCATIONS AND PLUMBNESS; CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE) AND ADEQUATE END-BEARING STRATA CAPACITY. RECORD CONCRETE OR GROUT VOLUMES c. FOR CONCRETE ELEMENTS, PERFORM ADDITIONAL SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.3	X X -	- - X	- - -	
OTHER STEEL CONSTRUCTION					IBC 1705.2.2, APPLICABLE ASTM STANDARD
	1. MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS b. MANUFACTURER'S CERTIFIED TEST REPORTS	- -	X X	- -	
	2. INSPECTION OF WELDING a. COLD-FORMED STEEL DECK • FLOOR AND ROOF DECK WELDS b. REINFORCING STEEL • VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706 • REINFORCING STEEL RESISTING FLUXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT c. SHEAR REINFORCEMENT d. OTHER REINFORCING STEEL	- - - X X -	X X - X X -	- - - - - -	AWS D1.3 AWS D14.4; ACI 318: 3.5.2
	3. VERIFY THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE FOR COLD-FORMED STEEL TRUSSES SPANNING 60' OR GREATER	X	-	-	IBC 1705.2.4

[illegible]

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SPECIAL INSPECTIONS	Job No. 222058.01	Drawn by Checked by Author Checker	NO. ISSUE	DATE
TYLER STREET PARKING GARAGE 619 WEST 17TH AVENUE, COVINGTON, LA 70433 AND ST. TAMMANY HEALTH SYSTEM NORTHLAKE SURGICAL ASSOCIATES 1028 SOUTH TYLER STREET, COVINGTON, LA 70433 AT ST. TAMMANY PARISH HOSPITAL	CONSTRUCTION DOCUMENTS Date 05 NOV 2025	STATE OF LOUISIANA OFFICE OF THE ARCHITECT JOHN E. GIBSON REGISTERED ARCHITECT NO. 10000 IN THE STATE OF LOUISIANA EXPIRATION DATE 12/31/2025		
Sheet No.	S0.2			



FLAT ROOF COMPONENTS AND CLADDING DESIGN PRESSURES (PSF) - 2016																
ZONE EWA (FT²)	1p (ROOF FIELD INTERIOR)	1 (ROOF FIELD)	2 (ROOF EDGE)	3 (ROOF CORNER)	4 (WALL FIELD)	5 (WALL EDGE)	2 (OVERHANG EDGE)	3 (OVERHANG CORNER)								
≤10	-35	-	-58	-	-58	-	-87	-	-38	35	-46	35	-94	-	-94	-
20	-33	-	-44	-	-44	-	-53	-	-34	31	-39	31	-94	-	-94	-
50	-33	-	-44	-	-44	-	-53	-	-34	31	-39	31	-94	-	-94	-
100	-32	-	-38	-	-38	-	-38	-	-32	29	-36	29	-94	-	-94	-
200	-32	-	-38	-	-38	-	-38	-	-31	29	-34	29	-94	-	-94	-
≥500	-32	-	-38	-	-38	-	-38	-	-29	26	-29	26	-94	-	-94	-

NOTES:

1. WIND PRESSURES ARE BASED ON THE ULTIMATE DESIGN WIND SPEED AND SHOULD BE USED WITH LRFD LOAD COMBINATIONS. TO CONVERT TO ALLOWABLE PRESSURES, MULTIPLY THE TABULATED PRESSURES BY 0.6.

2. EWA IS THE EFFECTIVE WIND AREA OF A STRUCTURAL COMPONENT.

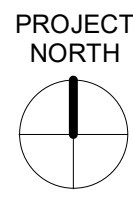
3. FOR ZONE DEFINITIONS, SEE ASCE 7-16 FIGURES 30.4-1 AND 30.4-2B.

4. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACES, RESPECTIVELY.

5. PARAPET DESIGN PRESSURES SHALL BE XX PSF ON THE WINDWARD FACE AND -XX/-XX PSF ON THE LEEWARD FACE IN ZONES 2/3, RESPECTIVELY. COMBINED PARAPET DESIGN PRESSURE SHALL BE XXX PSF IN ZONES 2/3 RESPECTIVELY.

6. OVERHANG DESIGN PRESSURES ACTING ON THE SOFFIT ALONE SHALL BE TAKEN AS +XX/-XX PSF IN ZONE 4 AND +XX/-XX IN ZONE 5.

7. WALL EDGE PRESSURES (ZONE 5) OCCUR WITHIN XX' OF A BUILDING CORNER.



1
S0.3

WIND UPLIFT PLAN - HELIPAD, GARAGE, BRIDGE, & STAIR TOWERS
1/16" = 1'-0"

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ST. TAMMANY PARISH HOSPITAL

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Date
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WIND UPLIFT PLAN

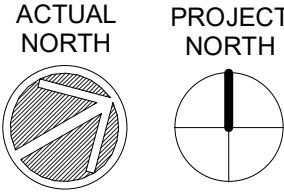
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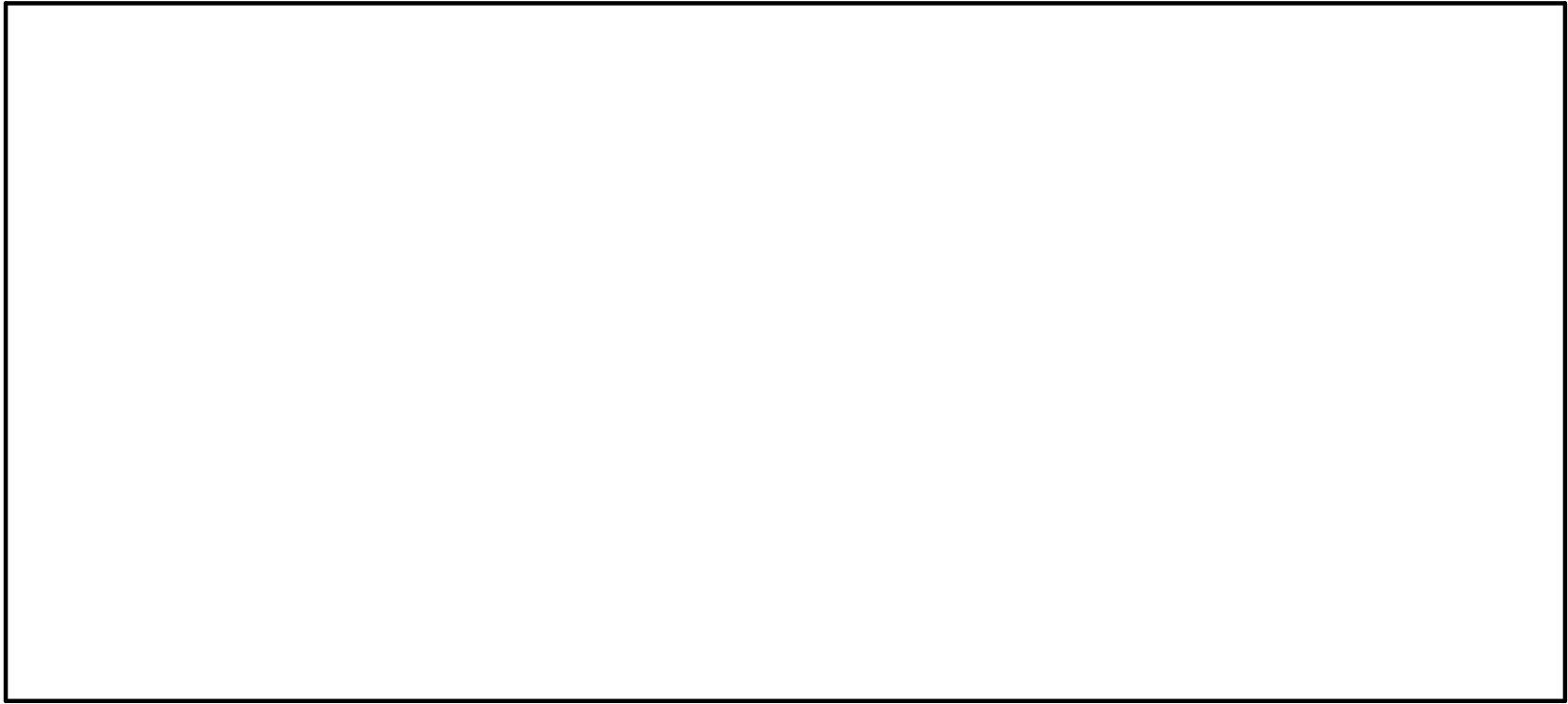
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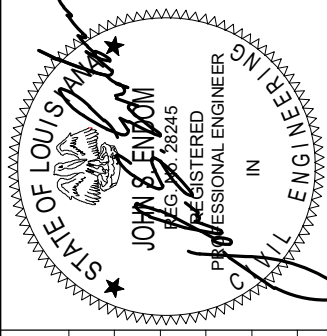


1
S0.4
1/16" = 1'-0"

WIND UPLIFT PLAN - NLSA BUILDING



FLAT ROOF COMPONENTS AND CLADDING DESIGN PRESSURES (PSF) - 2016														
ZONE (EWA) (FT²)	1p (ROOF FIELD INTERIOR)		1 (ROOF FIELD)		2 (ROOF EDGE)		3 (ROOF CORNER)		4 (WALL FIELD)		5 (WALL EDGE)		2 (OVERHANG EDGE)	3 (OVERHANG CORNER)
≤10	-35	-	-58	-	-58	-	-87	-	-38	35	-46	35	-94	-
20	-33	-	-44	-	-44	-	-53	-	-34	31	-39	31	-94	-
50	-33	-	-44	-	-44	-	-53	-	-34	31	-39	31	-94	-
100	-32	-	-38	-	-38	-	-38	-	-32	29	-36	29	-94	-
200	-32	-	-38	-	-38	-	-38	-	-31	29	-34	29	-94	-
≥500	-32	-	-38	-	-38	-	-38	-	-29	26	-29	26	-94	-
NOTES: 1. WIND PRESSURES ARE BASED ON THE ULTIMATE DESIGN WIND SPEED AND SHOULD BE USED WITH LRFD LOAD COMBINATIONS. TO CONVERT TO ALLOWABLE PRESSURES, MULTIPLY THE TABULATED PRESSURES BY 0.6. 2. EWA IS THE EFFECTIVE WIND AREA OF A STRUCTURAL COMPONENT. 3. FOR ZONE DEFINITIONS, SEE ASCE 7-16 FIGURES 30.4-1 AND 30.4-2B. 4. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACES, RESPECTIVELY. 5. PARAPET DESIGN PRESSURES SHALL BE XX PSF ON THE WINDWARD FACE AND -XX-XX PSF ON THE LEeward FACE IN ZONES 2/3, RESPECTIVELY. COMBINED PARAPET DESIGN PRESSURE SHALL BE XXXX PSF IN ZONES 2/3, RESPECTIVELY. 6. OVERHANG DESIGN PRESSURES ACTING ON THE SOFFIT ALONE SHALL BE TAKEN AS +XX-XX PSF IN ZONE 4 AND +XX-XX IN ZONE 5. 7. WALL EDGE PRESSURES (ZONE 5) OCCUR WITHIN XX' OF A BUILDING CORNER.														



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Date
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WIND UPLIFT PLAN

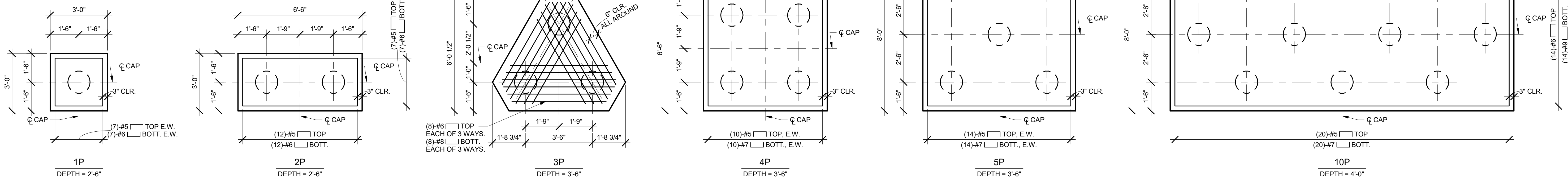
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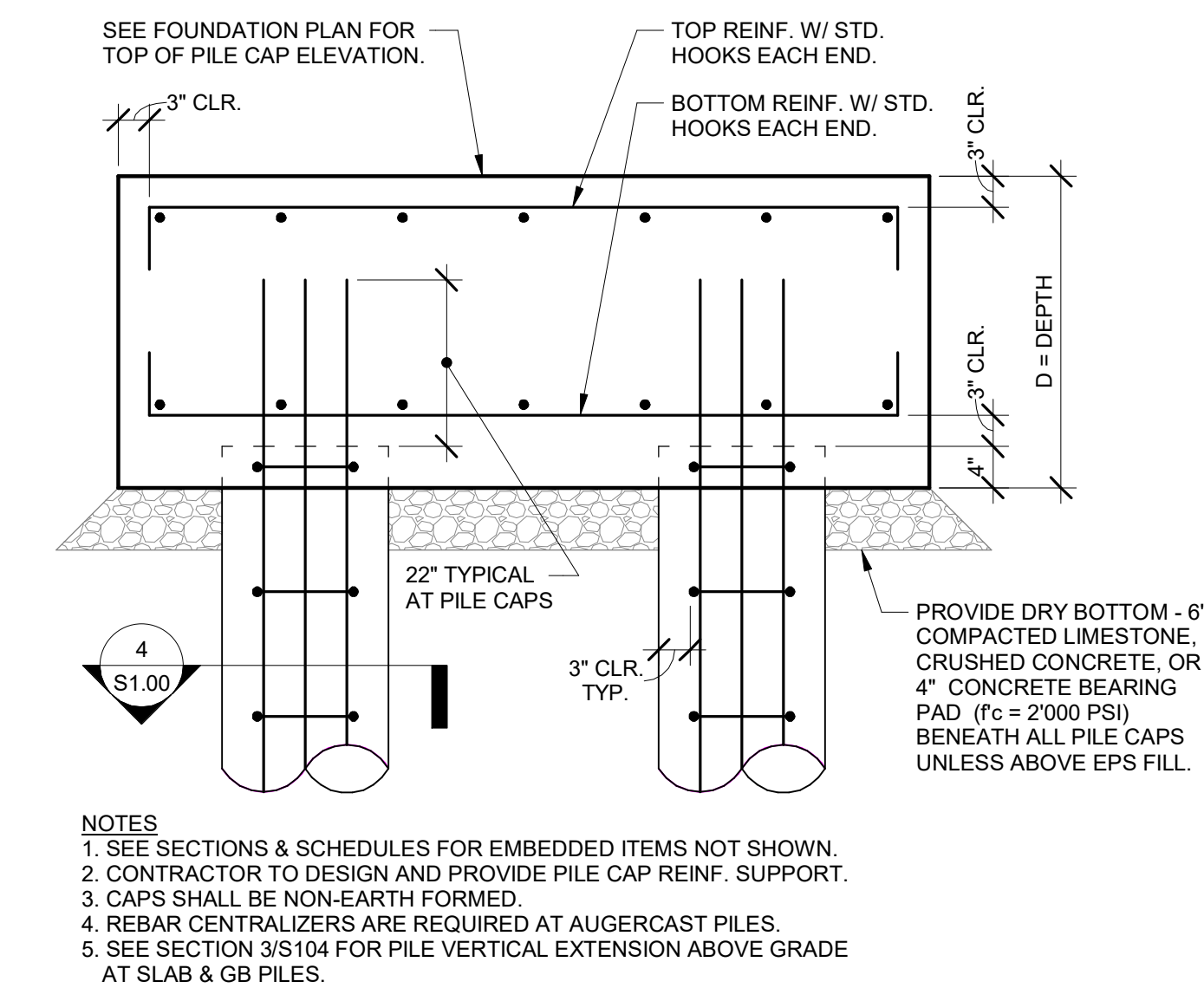
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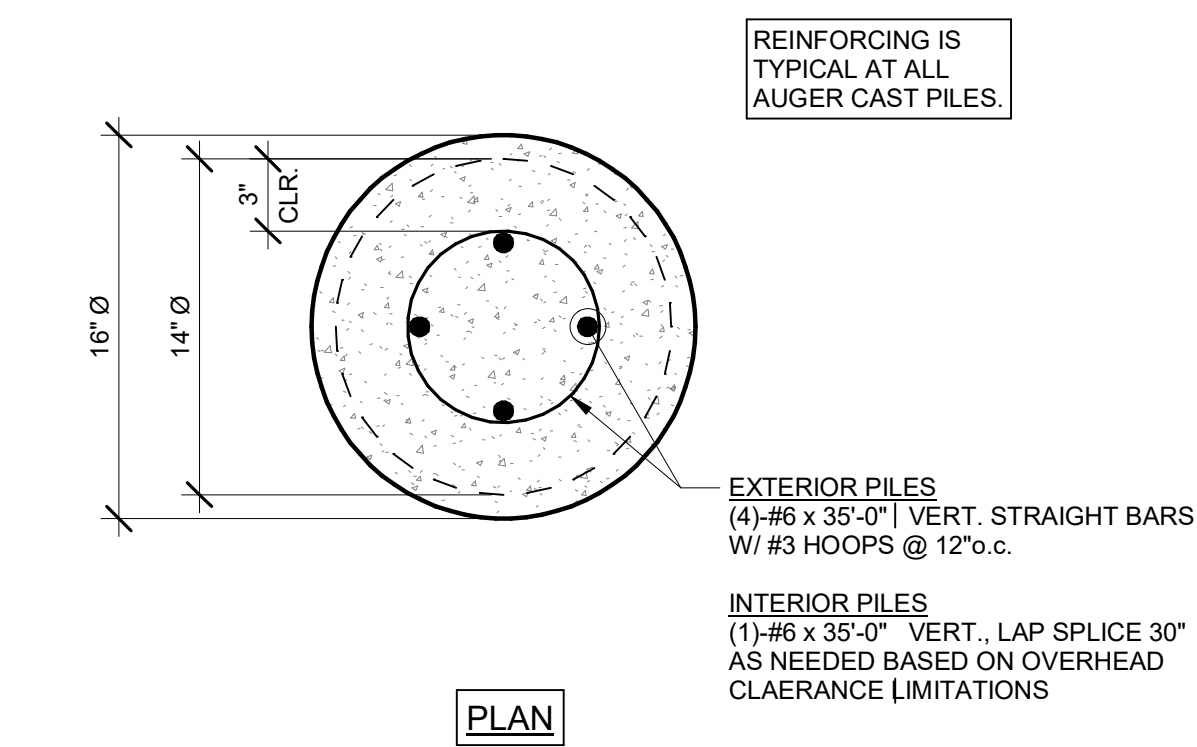
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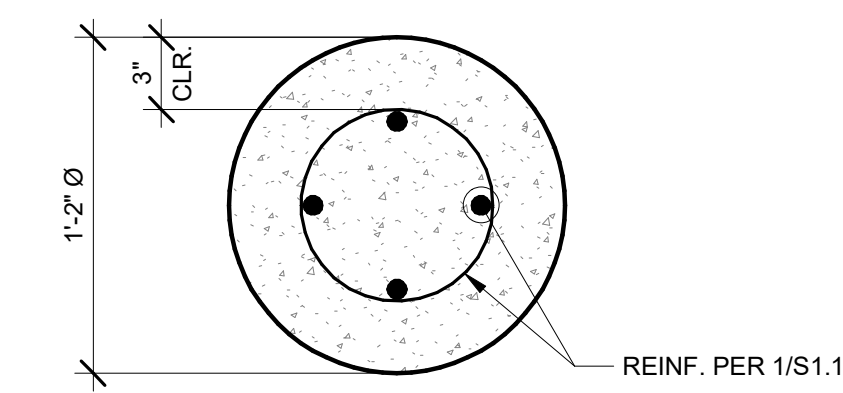
1 PILE CAP DETAILS
3/8" = 1'-0"



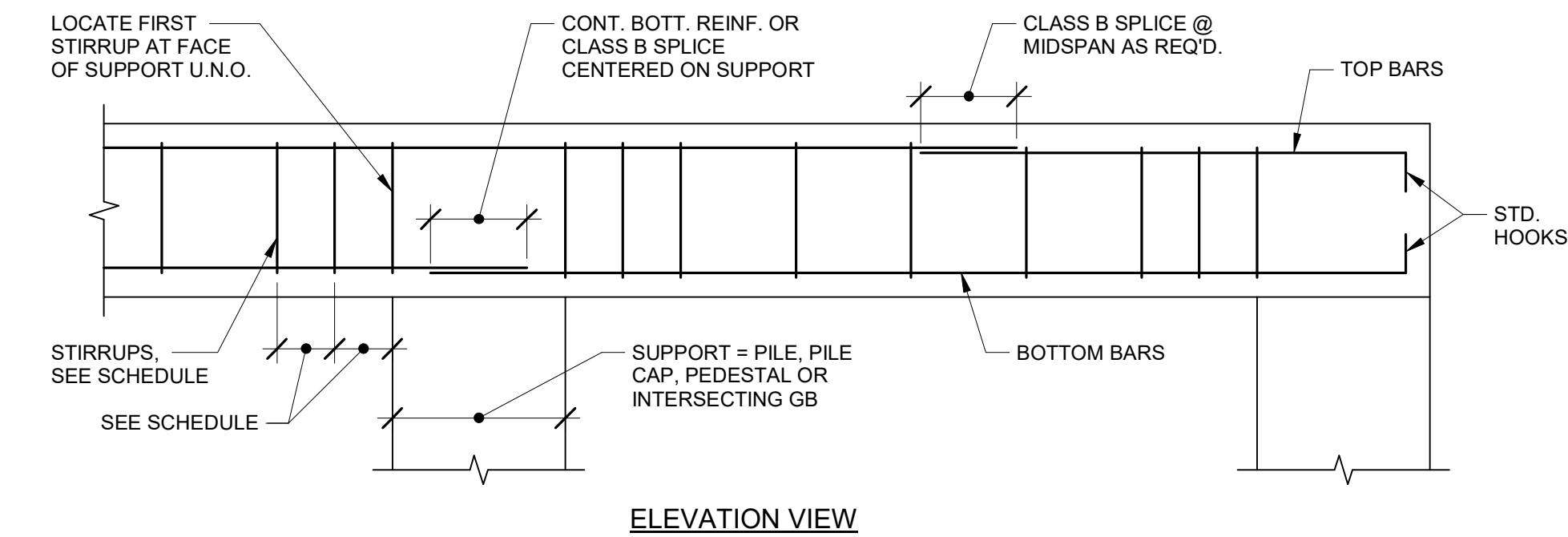
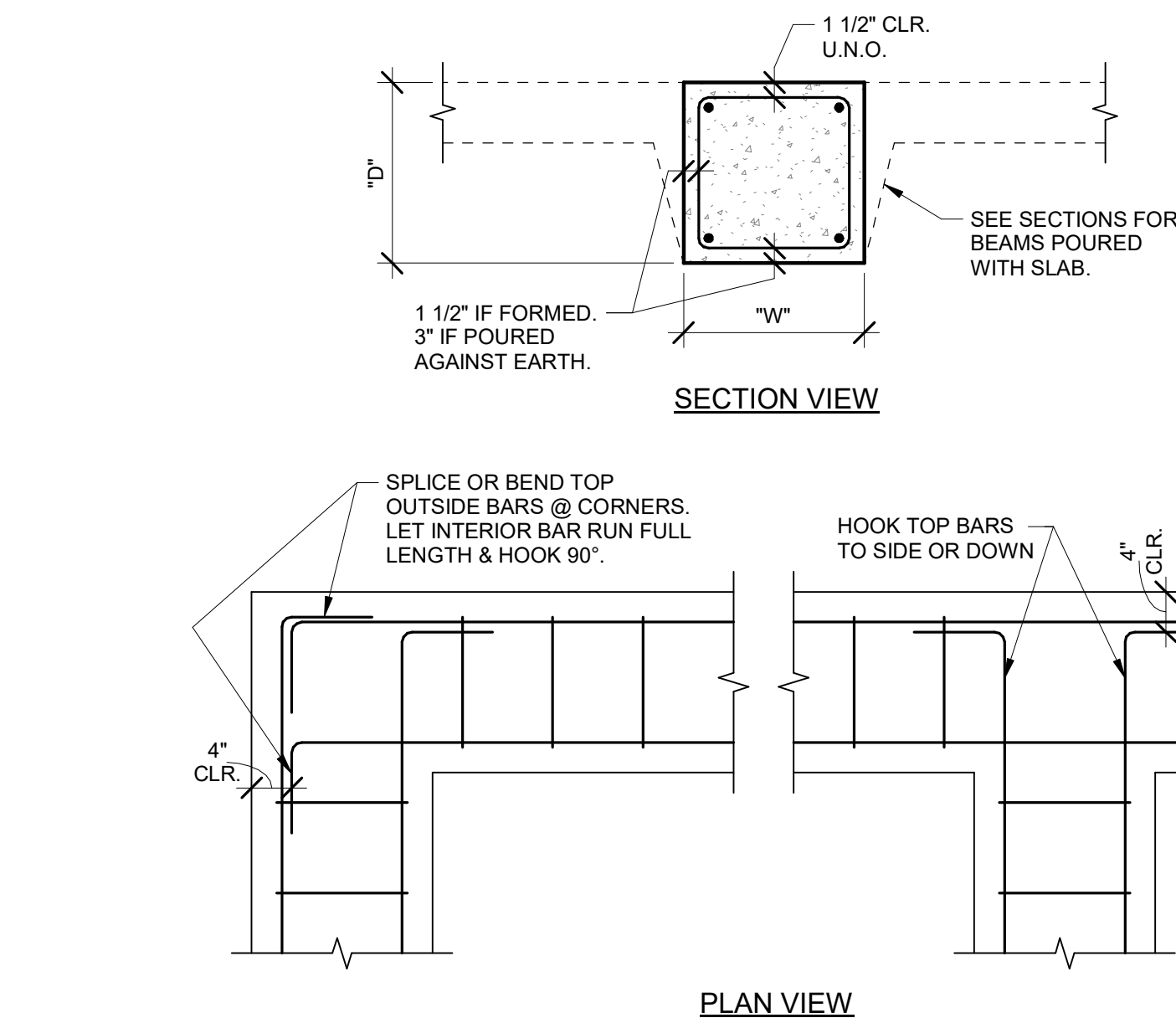
2 TYPICAL PILE CAP ELEVATION
3/4" = 1'-0"



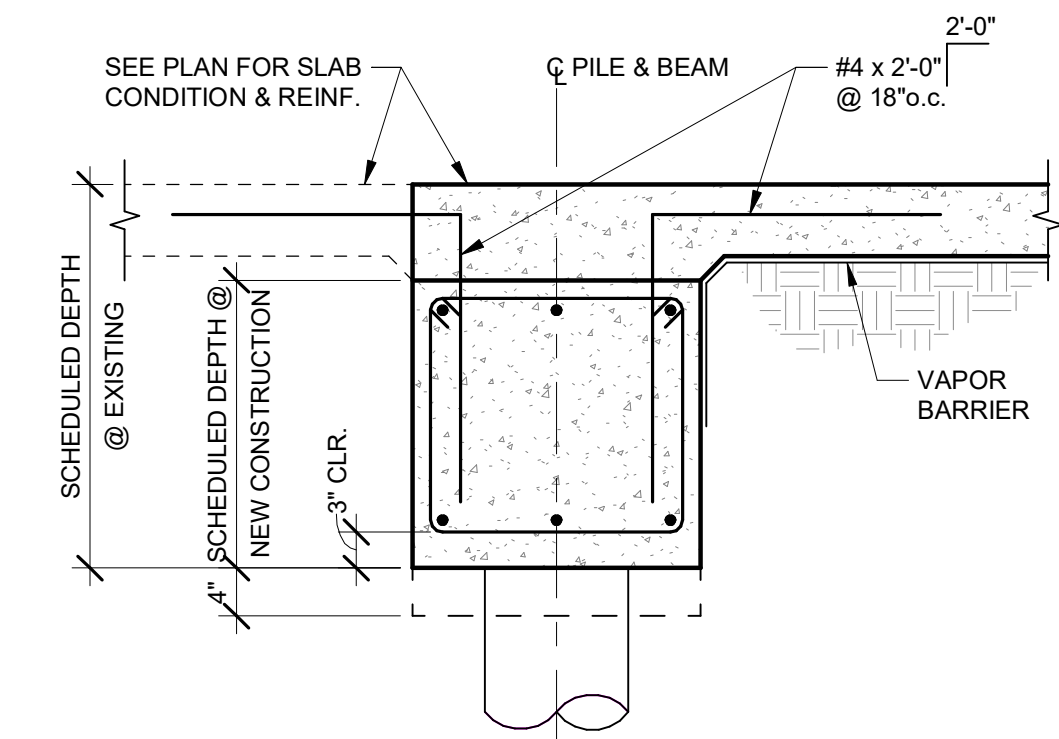
3 TYPICAL VERTICAL EXTENSION AT AUGER CAST SLAB & GB PILES
1 1/2" = 1'-0"



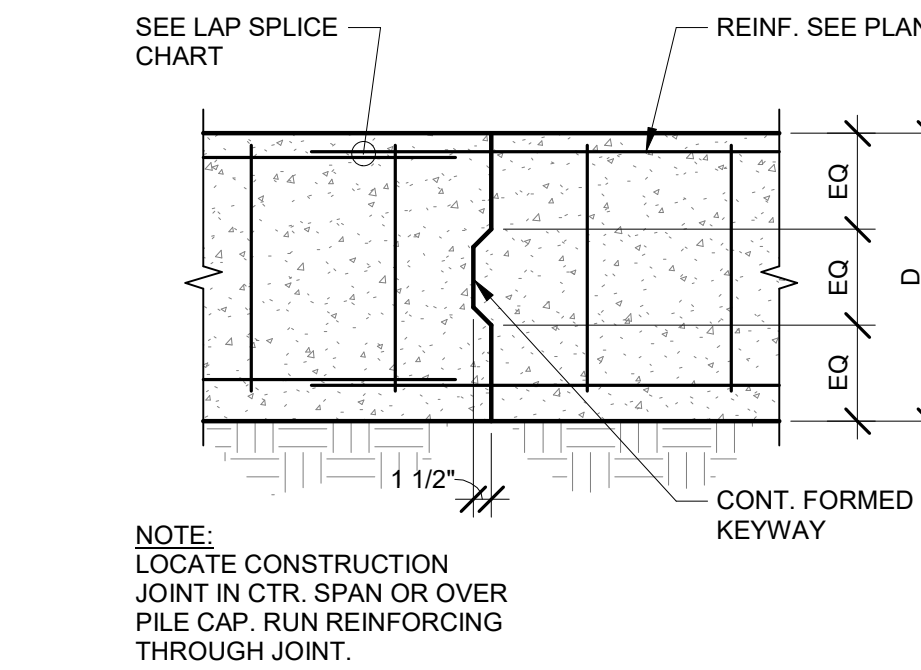
4 TYPICAL AUGER CAST PILE DETAIL
1 1/2" = 1'-0"



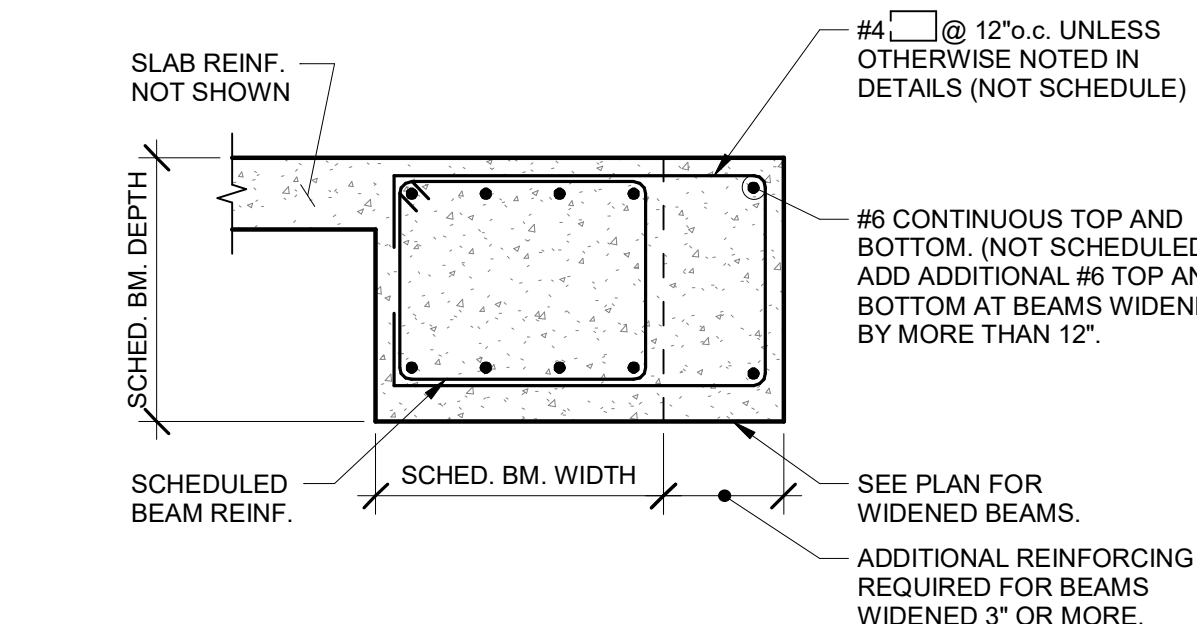
1 TYPICAL GRADE BEAM REINF. DETAIL
S1.01 3/4" = 1'-0"



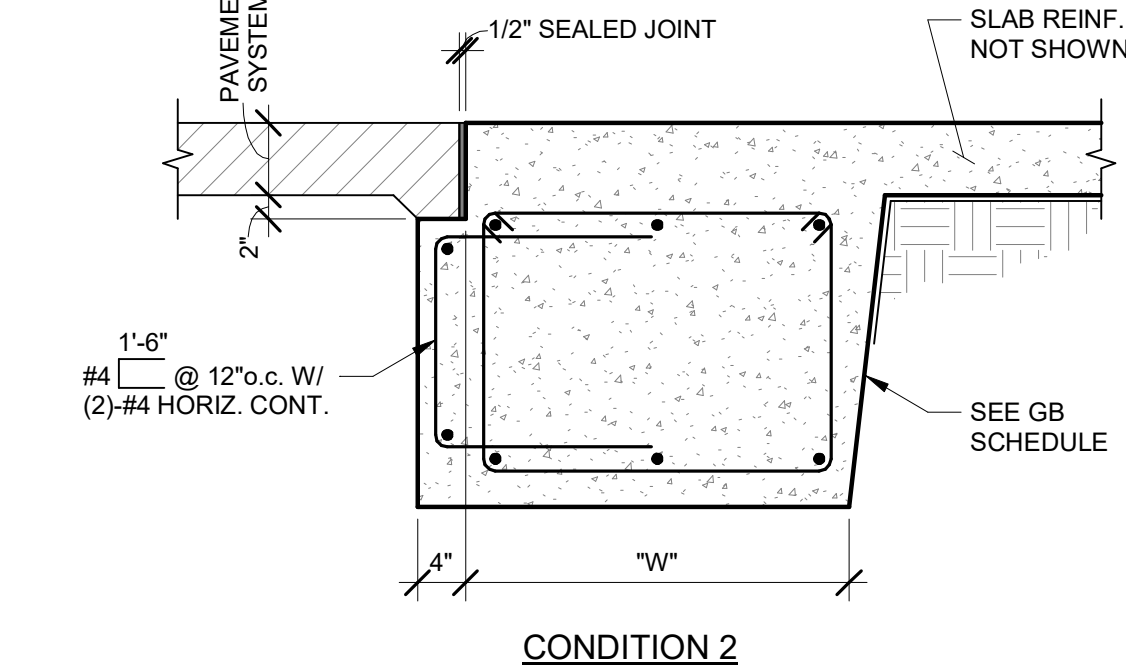
2 TYPICAL PILE SUPPORTED BEAM DETAIL
S1.01 3/4" = 1'-0"



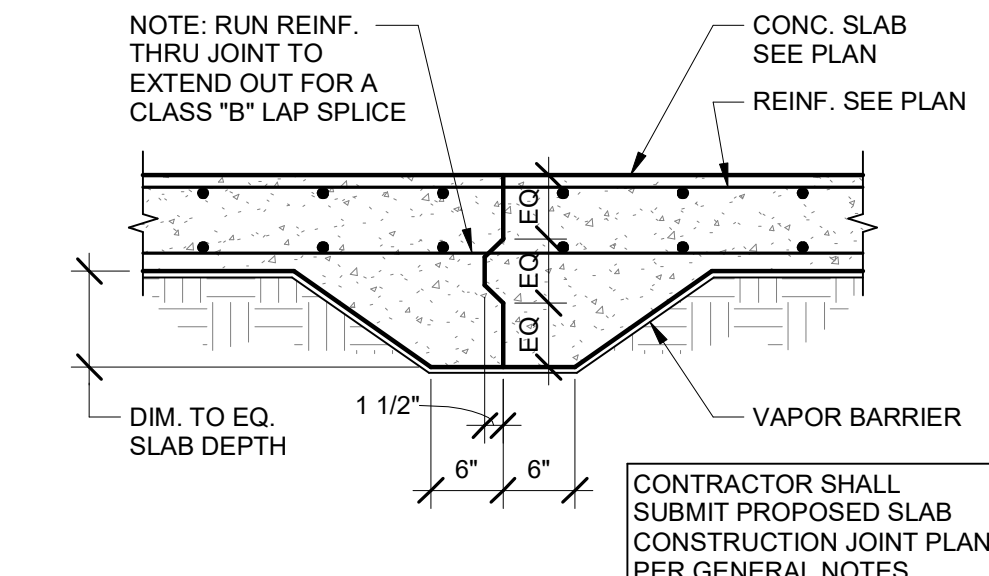
3 CONSTRUCTION JOINT FOR GRADE BEAMS
S1.01 3/4" = 1'-0"



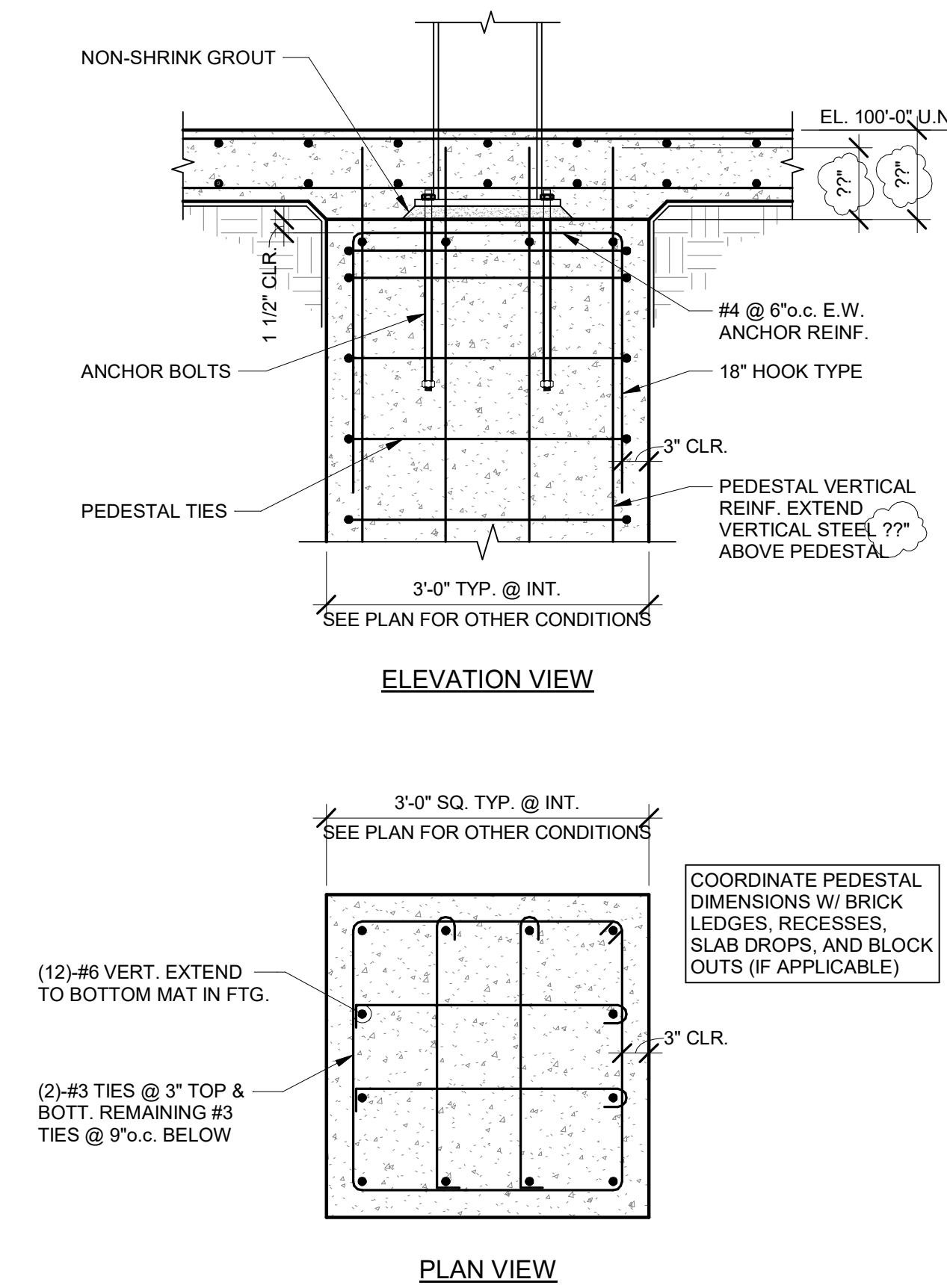
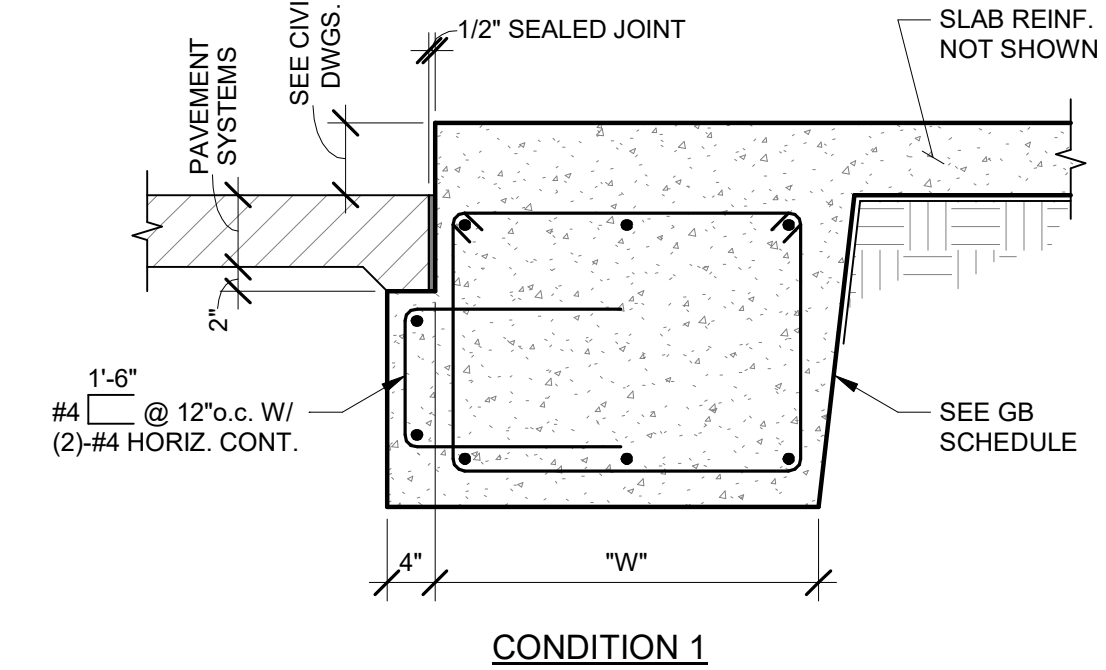
4 TYPICAL DETAIL AT WIDENED CONCRETE GRADE BEAM
S1.01 3/4" = 1'-0"



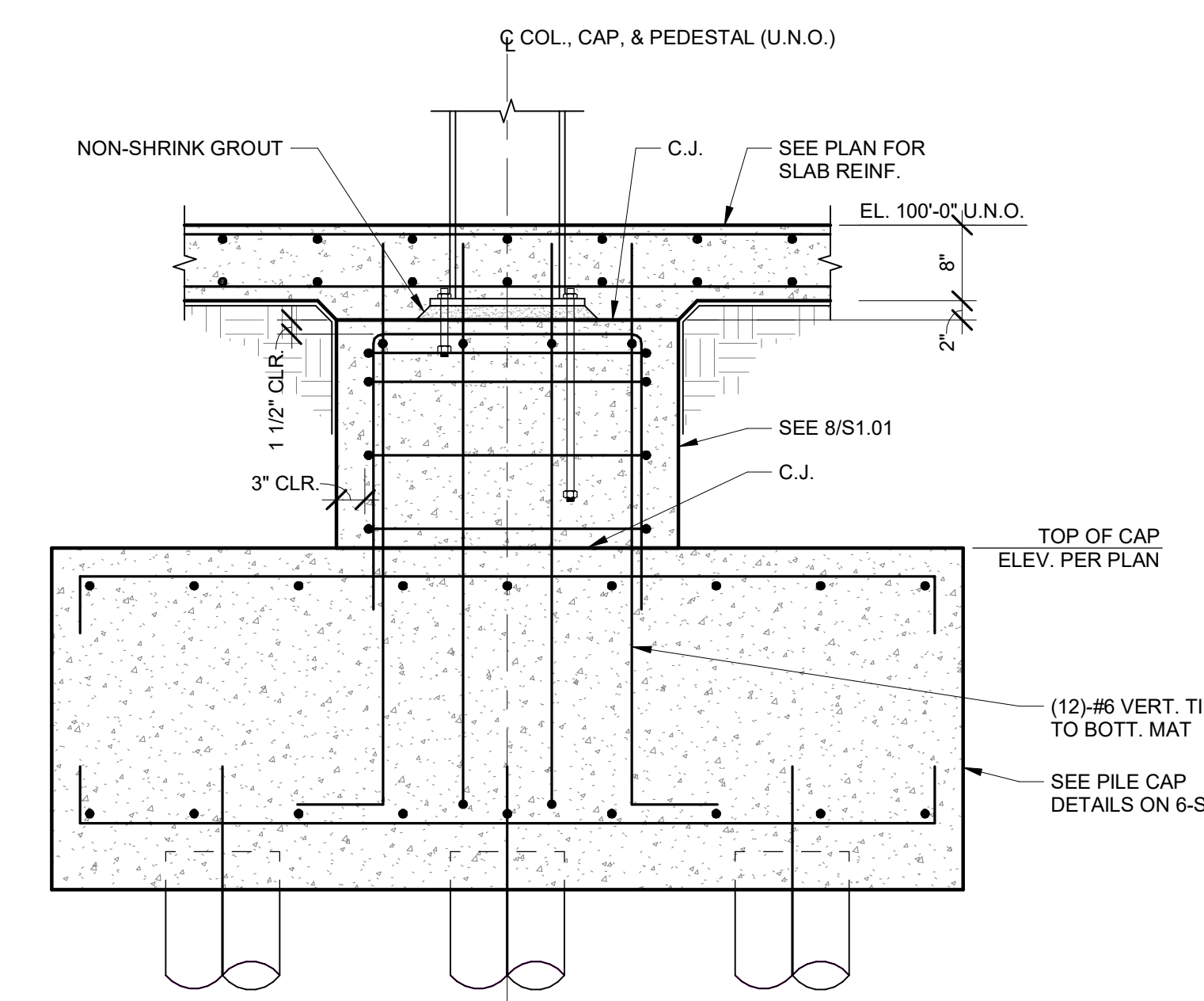
5 PAVEMENT SUPPORT LEDGE
S1.01 3/4" = 1'-0"



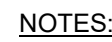
6 SLAB CONSTRUCTION JOINT ON GRADE
S1.01 3/4" = 1'-0"



7 FORMED CONCRETE PEDESTAL
S1.01 3/4" = 1'-0"



8 PEDESTAL ATOP PILE CAP DETAIL
S1.01 3/4" = 1'-0"

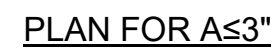


1. GENERAL CONTRACTOR TO COORDINATE LOCATION, SIZE AND ELEVATION AND INCLUDE IN HIS CONTRACT PRICE ALL REQUIRED HORIZONTAL PENETRATIONS THROUGH CONCRETE BEAMS AN JOISTS, WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT. WHERE BEAM PENETRATIONS ARE REQUIRED, BUT ARE NOT SPECIFICALLY DETAILED ON STRUCTURAL DRAWINGS, SUBMIT TO ARCHITECT FOR REVIEW AND APPROVAL.
3. "D" DENOTES THE DEPTH OF BEAM OR JOIST.
4. CLEAR SPACING BETWEEN PENETRATIONS SHALL BE 24" MINIMUM UNLESS DESIGNED OTHERWISE BY THE ENGINEER.
5. PENETRATIONS SHALL BE LOCATED ACCORDING TO THE FOLLOWING CRITERIA:
- a. FOR BEAMS (AND JOISTS) NOT SUPPORTING INTERSECTING BEAMS (OR JOISTS):
 - PENETRATIONS WITHIN TWO FEET EITHER SIDE OF BEAM MIDSPAN.
 - FOR BEAMS (AND JOISTS) SUPPORTING INTERSECTING BEAMS (OR JOISTS) CHECK WITH STRUCTURAL ENGINEER.
6. FOR LOCATIONS AND/OR SIZES OF PENETRATIONS NOT CONFORMING TO THE ABOVE CRITERIA AND NOT OTHERWISE DETAILED ON STRUCTURAL DRAWINGS, GENERAL CONTRACTOR SHALL COORDINATE THE REQUIRED ADDITIONAL REINFORCEMENT WITH THE ENGINEER ON THE SHOP DRAWINGS.

1
S1.02



1. WHERE CLEAR SPACING BETWEEN ADJACENT SLEEVES IS LESS THAN 3". THE SLEEVE GROUP SHALL BE TREATED AS AN EQUIVALENT RECTANGULAR OPENING WITH LENGTH "L" AND WIDTH "W" AS SHOWN ABOVE.
2. WHERE CLEAR SPACING BETWEEN ADJACENT SLEEVES IS GREATER THAN OR EQUAL TO 3". SCHEDULE SLAB BAR REINFORCEMENT SHALL BE OFFSET AS REQUIRED TO MISS SLEEVES.
3. REINFORCEMENT SHOWN IS ADDITIONAL TO SCHEDULED SLAB REINFORCEMENT.
4. SCHEDULED SLAB MESH REINFORCEMENT, MAY BE CUT AS REQUIRED TO MISS PIPE SLEEVES.
5. REFER TO MECHANICAL DRAWINGS FOR LOCATION AND SIZE OF SLEEVES.
6. ISOLATED PIPE SLEEVES THAT ARE SMALLER THAN 5" AND DO NOT INTERRUPT REINFORCING, DO NOT REQUIRE THE USE OF THIS DETAIL.
7. THIS DETAIL SHOULD NOT BE USED FOR OPENING GROUPS WITH SPACINGS LARGER THAN 12". CONSULT ENGINEER FOR FRAMING OF SUCH CONDITIONS.



4
S1 02

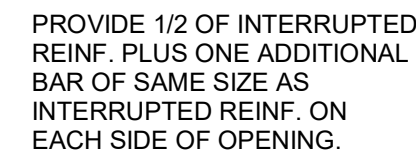


2
S1.02



- NOTES:**
1. COORDINATE OPENING SIZE AND LOCATION WITH ARCH. AND MECHANICAL DRAWINGS.
 2. MAXIMUM DIMENSION "A" SHALL BE 4'-0" FOR "A" DIMENSION GREATER THAN 4'-0" OPENING SHALL BE REFERRED TO ENGINEER FOR FRAMING.
 3. WWF SLAB REINFORCEMENT SHALL BE CUT AROUND SLAB OPENING. WWF SHALL EXTEND TO WITHIN 2" OF OPENING ON ALL SIDES.
 4. OPENINGS 10" AND SMALLER DO NOT REQUIRE ADDITIONAL REINFORCEMENT.
 5. THIS DETAIL SHOWS TYPICAL CONDITIONS. VERIFY REINFORCING STEEL PLACEMENT WITH ENGINEER FOR SPECIAL CASES.
 6. PROVIDE REINFORCEMENT AS SHOWN FOR SQUARE, RECTANGULAR, OR ROUND OPENING.

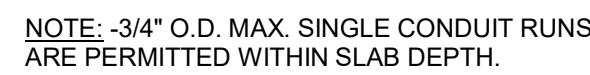
3	S1.02
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5
S1.02

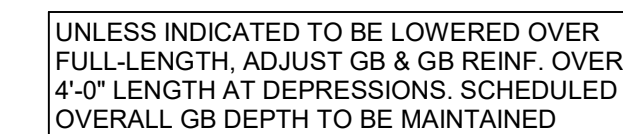


- NOTES:**
1. REQUIRED BEAM SLEEVES ARE TO BE COORDINATED WITH M.E.P. CONTRACTORS. REQUIRED SLEEVES MAY OR MAY NOT BE SHOWN ON THE STRUCTURAL DRAWINGS. GENERAL CONTRACTOR SHALL SUBMIT PLAN SHOWING LAYOUT OF ALL SLEEVES W/ FORM WORK SHOP DRAWINGS SUBMITTAL.
 2. SLEEVES SHALL BE LOCATED ON THE BEAM CENTERLINE OR AT LEAST WITHIN THE MIDDLE THIRD OF THE SCHEDULED BEAM WIDTH.
 3. CONTINUOUS BEAM REINFORCING MAY BE SLIGHTLY DISPLACED (3" MAX.) OR ADJACENT BARS BUNDLED (2 BAR BUNDLES MAX.) TO FACILITATE SLEEVE INSTALLATION. DO NOT CUT, OFFSET, OR BEND REINFORCING.
 4. SLEEVES OCCURRING ON OPPOSITE SIDES OF A COLUMN MUST BE IN LINE.
 5. THE OUTSIDE DIAMETER OF A SLEEVE MAY NOT EXCEED 15% OF THE SCHEDULE WIDTH OF THE BEAM THROUGH WHICH IT MUST PASS.
 6. THE CONTRACTOR SHALL CONTACT THE ENGINEER OF RECORD FOR DIRECTIONS WHEN A SLEEVE SIZE OR LOCATION DOES NOT MEET THE CONDITIONS ESTABLISHED ABOVE.



8
S1.0

* STIRRUPS SHALL CONTINUE @ THE SPACING INDICATED INTO CANTILEVERED BEAM ENDS
(DO NOT STOP STIRRUPS @ EITHER SIDE OF SUPPORT).

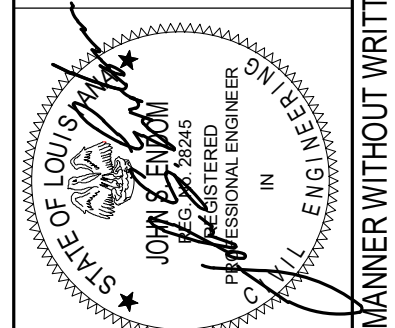


7
\$1.02

TENSION LAP SPLICE LENGTH CHART CLASS B					
BAR SIZE	F'c=4,000psi				
	TOP BARS		OTHER BARS		
	1" COVER	1.5" COVER	1" COVER	1.5" COVER	
#3	16	16	16	16	
#4	25	20	19	16	
#5	38	26	29	20	
#6	55	37	42	28	
#7	93	62	71	48	
#8	121	81	93	62	
#9	153	102	118	79	
#10	198	126	145	97	
#11	228	152	175	117	

- NOTES**
1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE.
 2. TENSION LAP SPlice LENGTHS ARE BASED ON ACI 318. TABULATED VALUES FOR BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE OVER MEETING MINIMUM CODE REQUIREMENTS. LENGTHS ARE IN FEET.
 3. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
 4. FOR LIGHTWEIGHT AGGREGATE CONCRETE, MULTIPLY THE TABULATED VALUES BY 1.3.
 5. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED VALUES BY ONE OF THE FOLLOWING FACTORS:
- | COVER AND SPACING | TOP BARS | OTHER BARS |
|----------------------------------|----------|------------|
| COVER ≥ 3d, OR C-C SPACING < 6d, | 1.7 | 1.50 |
| COVER ≥ 3d, OR C-C SPACING ≥ 6d, | 1.6 | 1.20 |

6
S1.02

[illegible]

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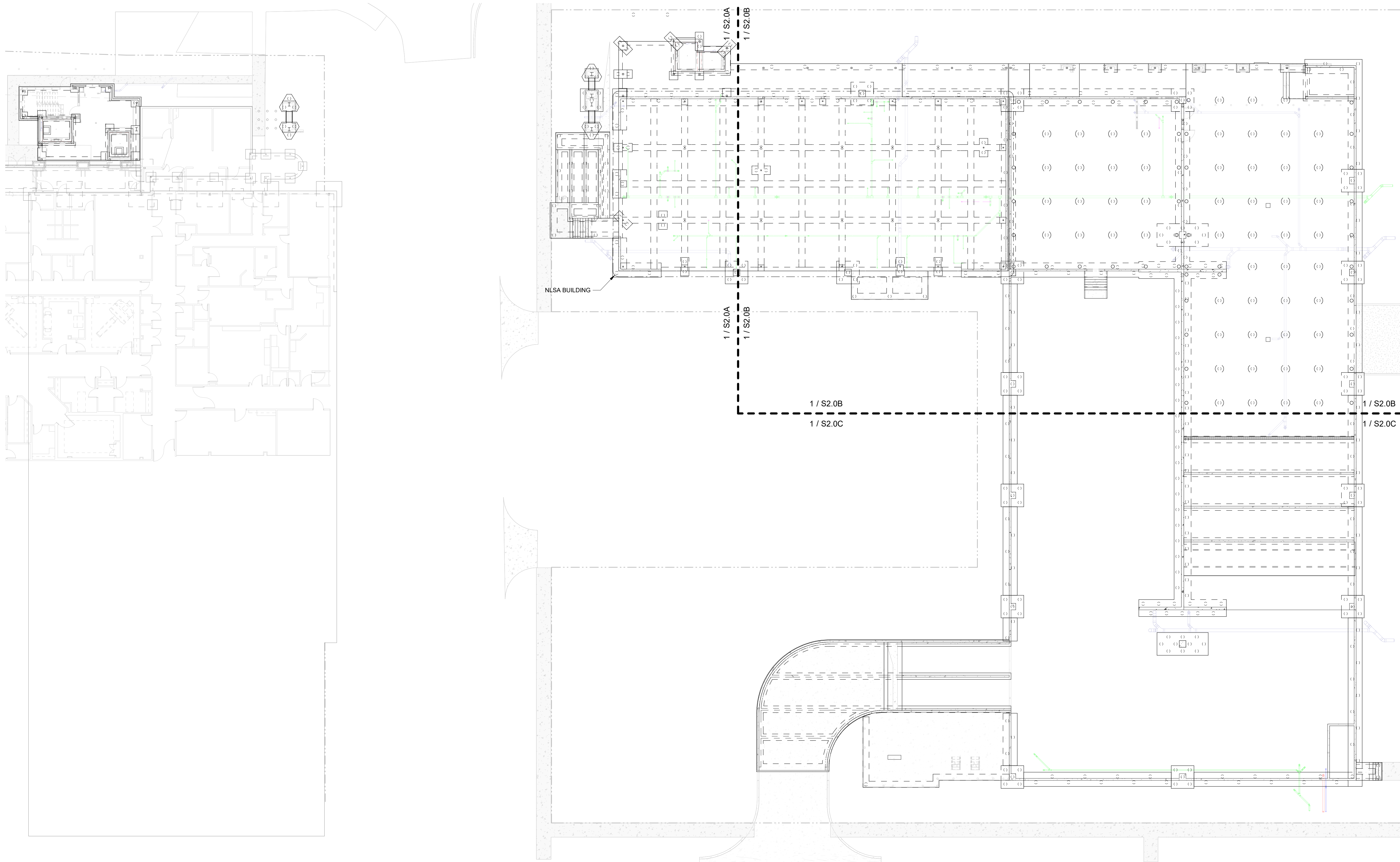
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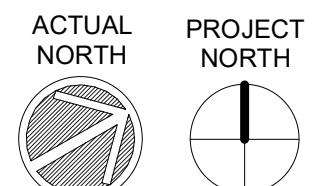
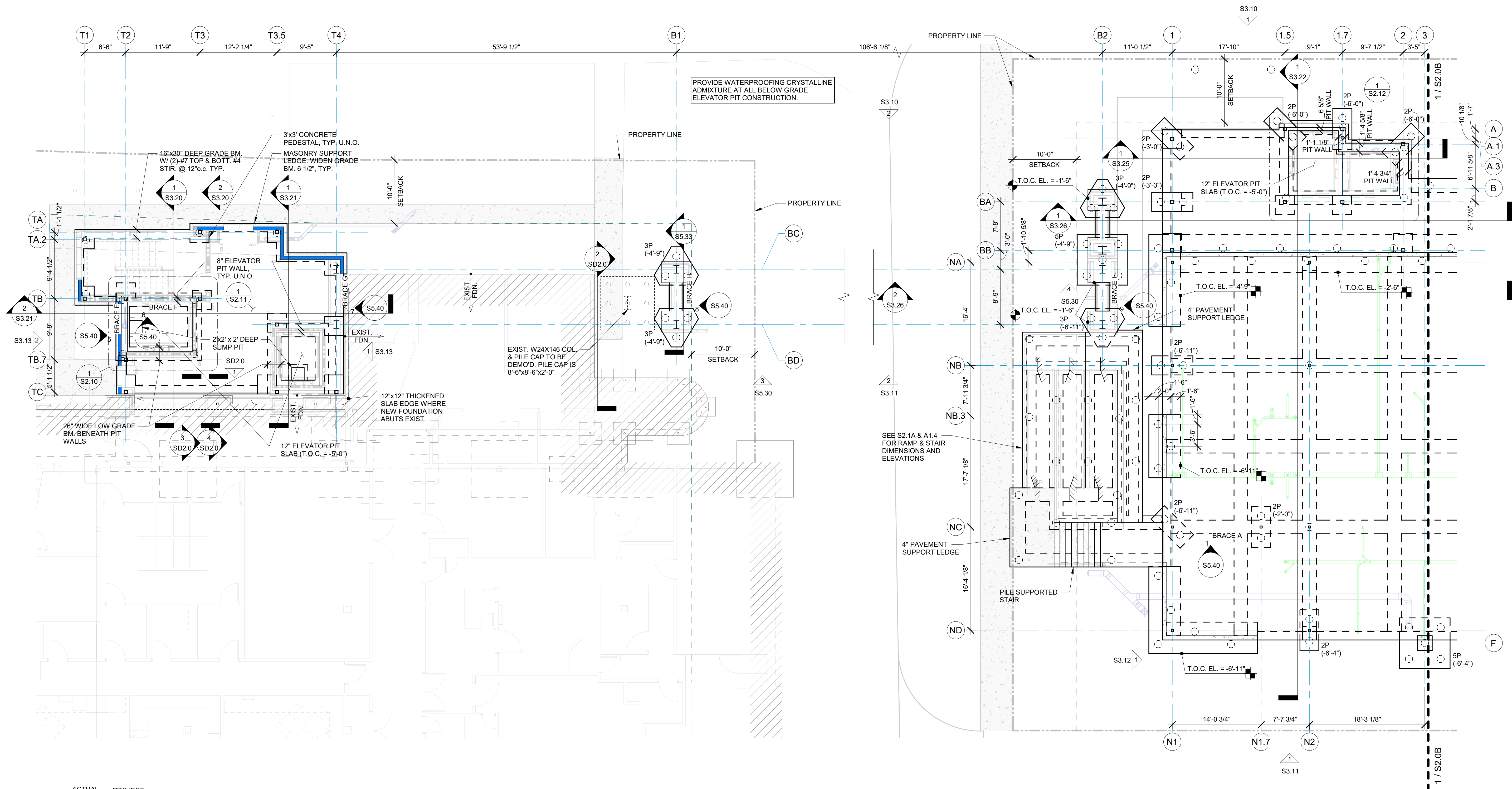
1
S2.0

FOUNDATION PLAN - OVERALL

1/16" = 1'-0"

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1 FOUNDATION PLAN - AREA A
S2.0A 1/8" = 1'-0"

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S0.1 FOR FOUNDATION LEGEND.

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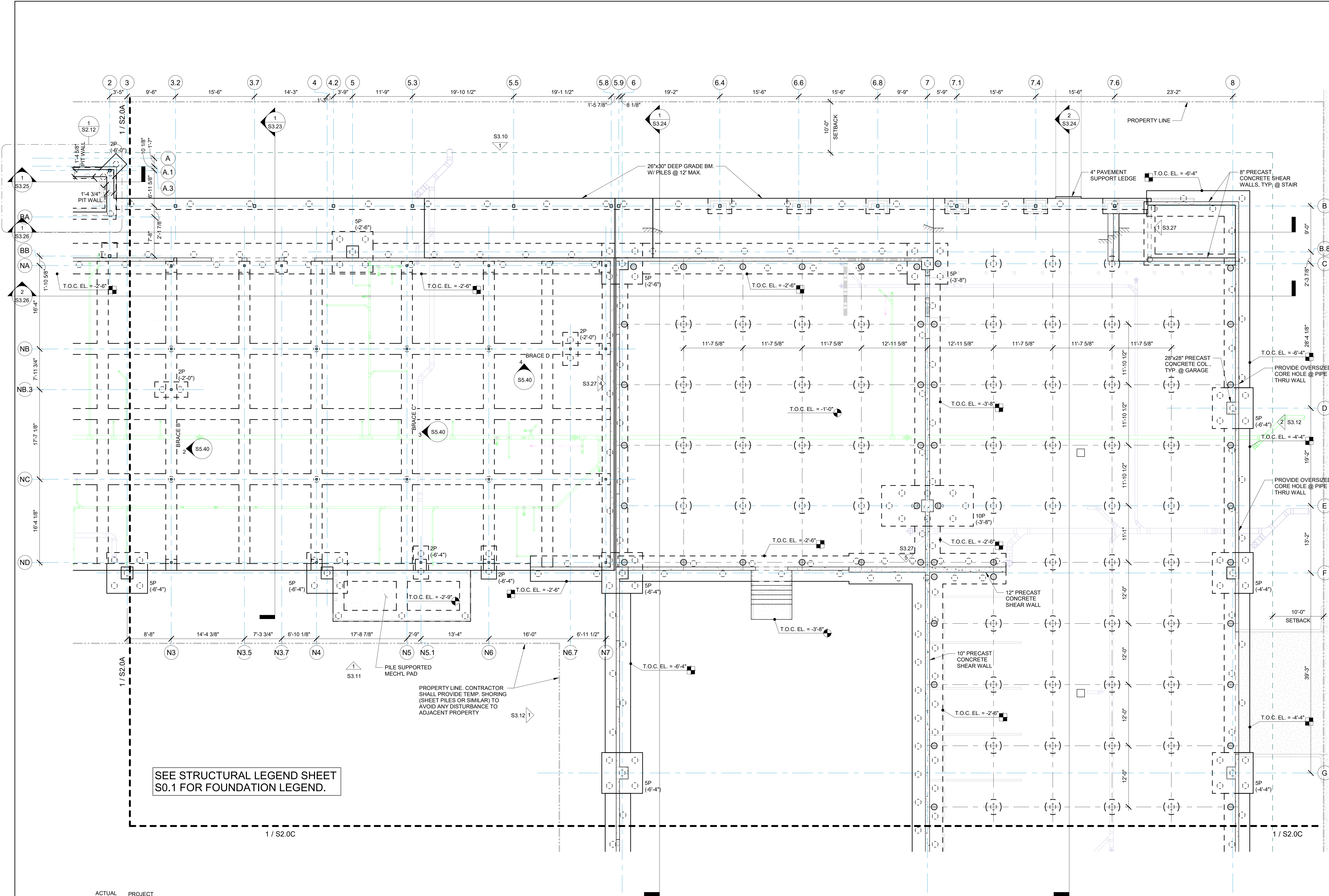
FOUNDATION PLAN - AREA A

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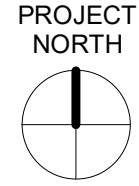
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S2.0A
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SEE STRUCTURAL LEGEND SHEET
S0.1 FOR FOUNDATION LEGEND.



1
S2.0B

FOUNDATION PLAN - AREA B
1/8" = 1'-0"

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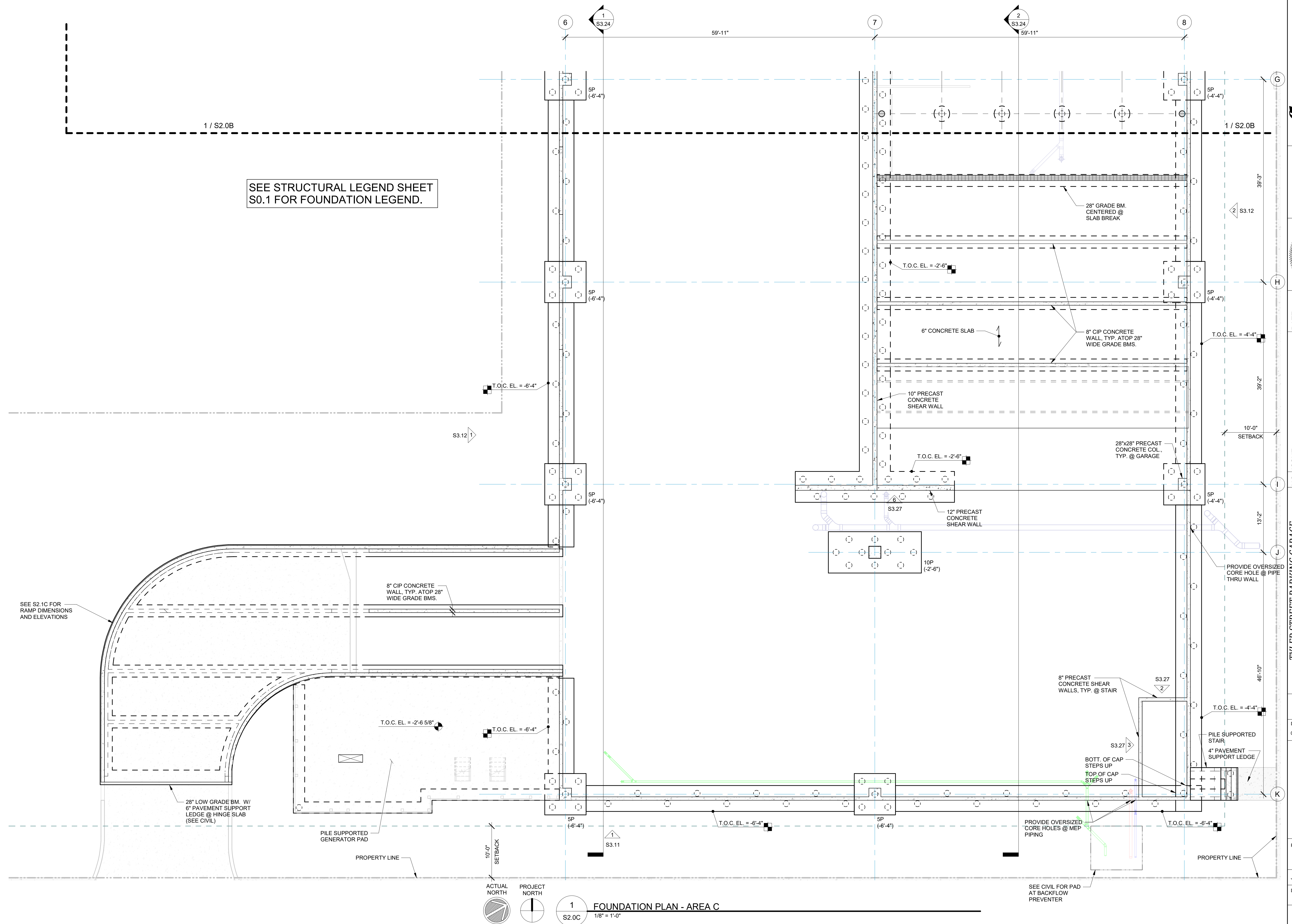
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SEE S2.1C FOR —
RAMP DIMENSIONS
AND ELEVATIONS

28" LOW GRADE BM. W/
6" PAVEMENT SUPPORT
LEDGE @ HINGE SLAB
(SEE CIVIL)

PILE SUPPORTED GENERATOR PAD

PROPERTY LINE -

T.O.C. EL. = -2'-6 5/8"

T.O.C. EL. = -6'-4"

8" CIP CONCRETE
WALL, TYP. ATOP 28"
WIDE GRADE BMS.

S3.12 1

T.O.C. EL. = -6'-4"

5P
(-6'-4")

5P
(-6°-4°)

3F
(-6°-4°)

*) — — —

S3.1

FOUNDATION PLAN - AREA C

$$1/8'' = 1'-0''$$

DC EL = -2'-6"

T.O.C. EL. = -2'-6"

10P
(-2'-6")

8" PRECAST ———
CONCRETE SHEAR
WALLS, TYP. @ STAIR

BOTT. OF C.
STEPS UP

TOP OF CAP
STEPS UP

A diagram of a road with a dashed line. A car is shown on the left side of the road, and a dashed line is drawn across the road. An arrow points to the dashed line.

OVERSIZED
S @ MEP

FOR PAD —
LOW
ER

PROPERTY LINE ————

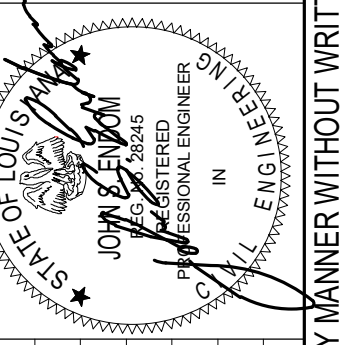
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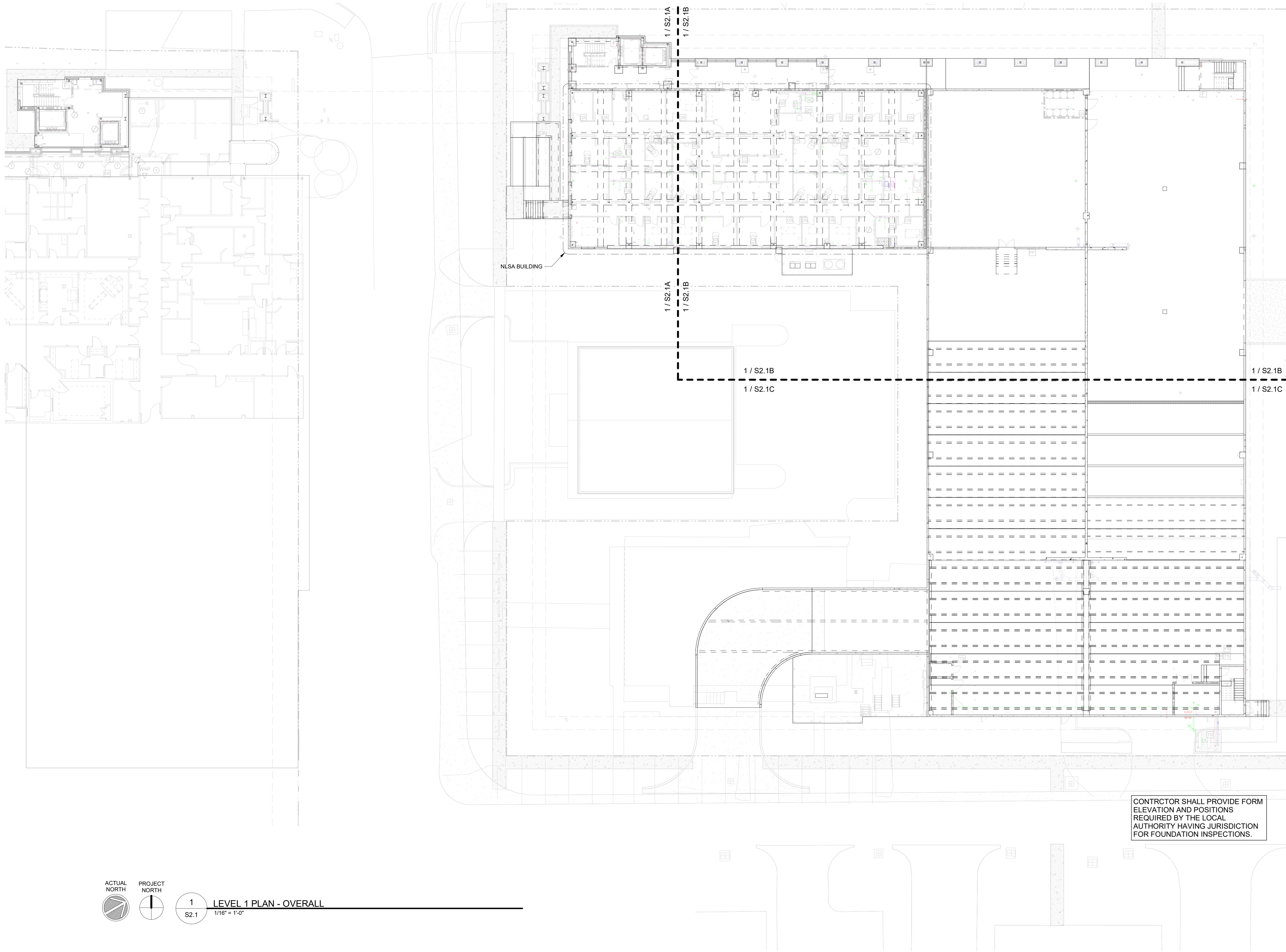
FOUNDATION PLAN - AREA C

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S2.0C

Sheet No.	



1
S2.1

LEVEL 1 PLAN - OVERALL
1/16" = 1'-0"

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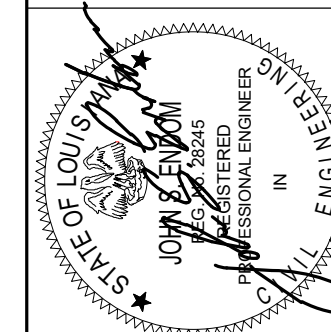
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LEVEL 1 PLAN - OVERALL

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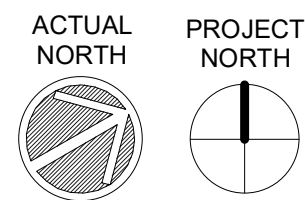
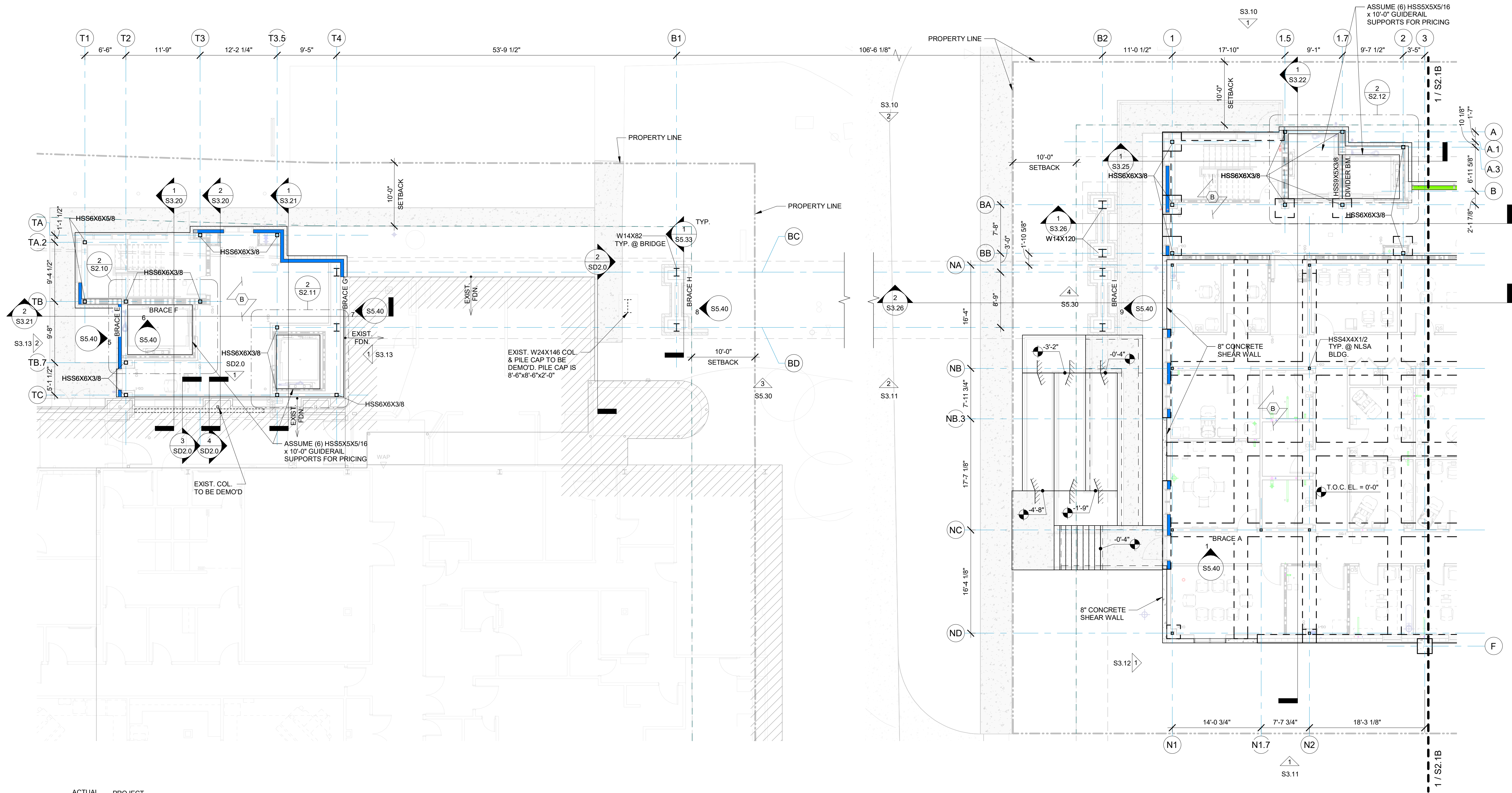
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1
S2.1A
LEVEL 1 PLAN - AREA A
1/8" = 1'-0"

SEE STRUCTURAL LEGEND SHEET
S0.1 FOR FOUNDATION LEGEND.

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LEVEL 1 PLAN - AREA A

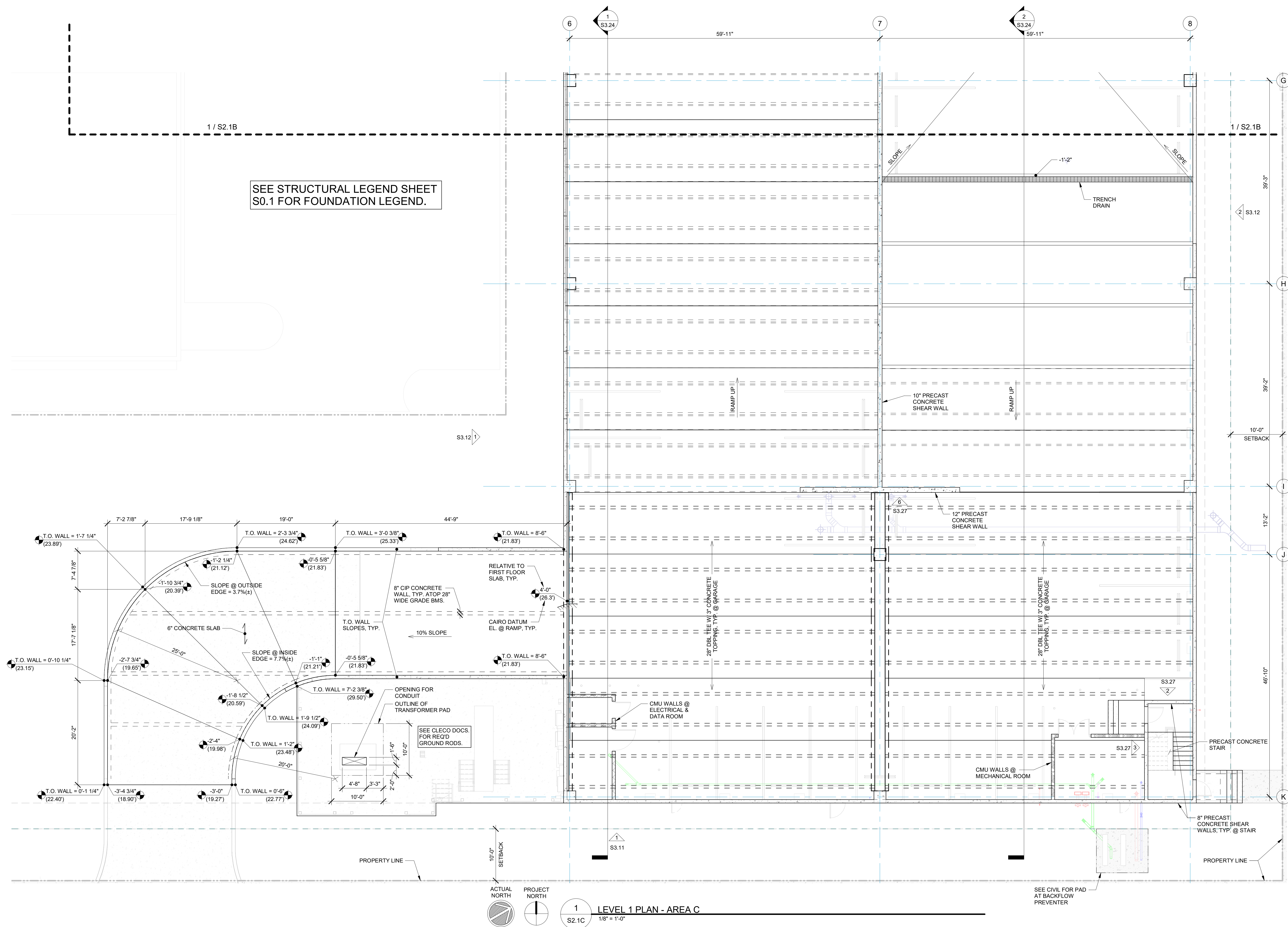
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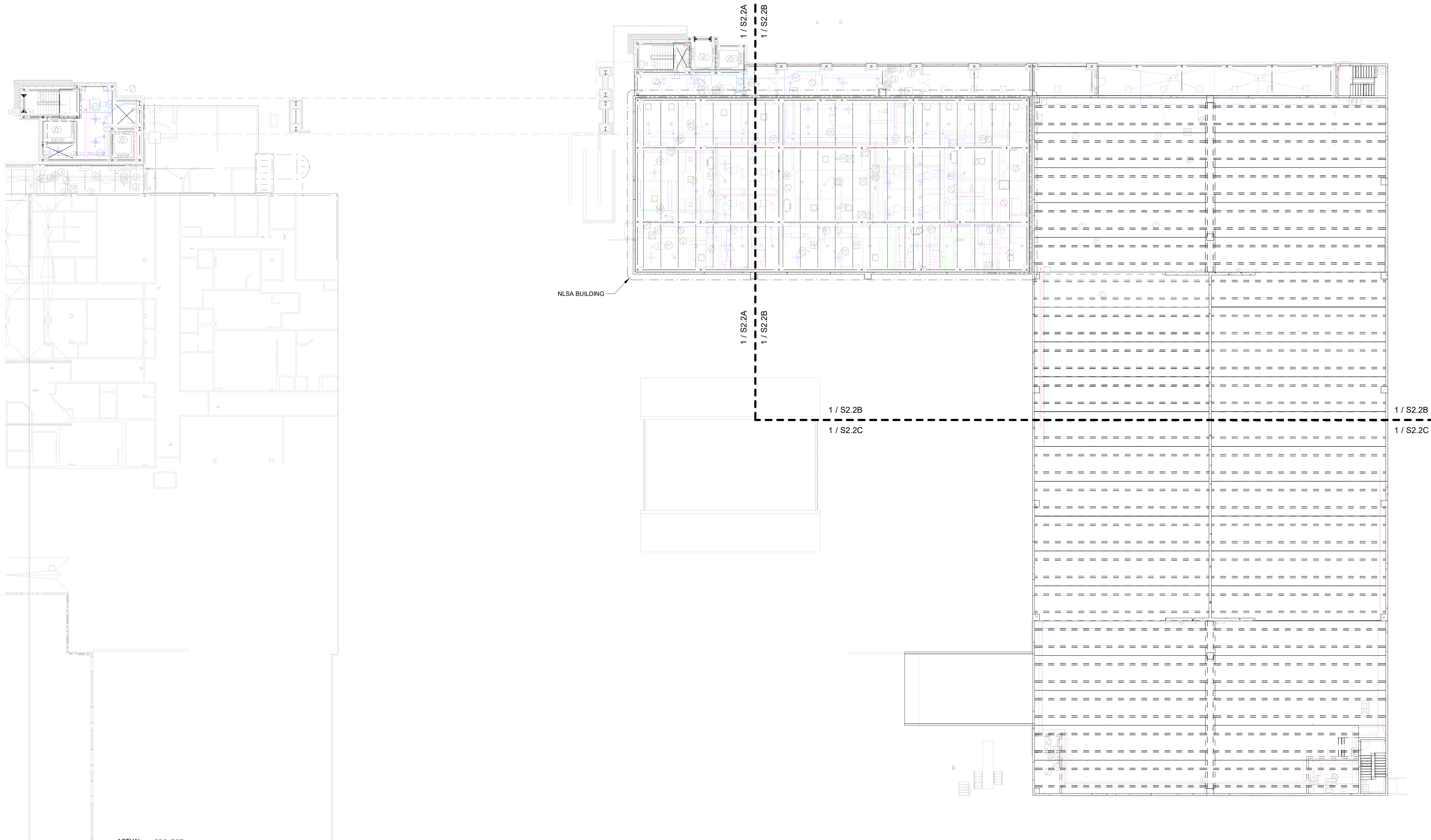
LEVEL 1 PLAN - AREA C

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S2.1C

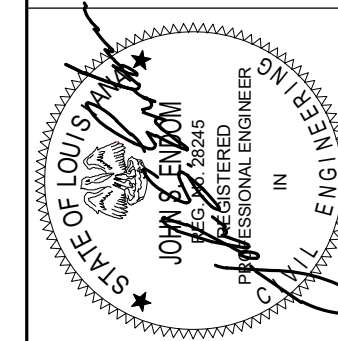
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1
S2.2

LEVEL 2 PLAN - OVERALL

1/16" = 1'-0"



DATE

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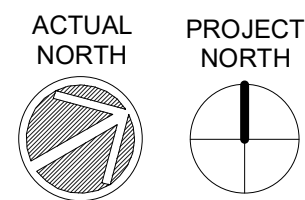
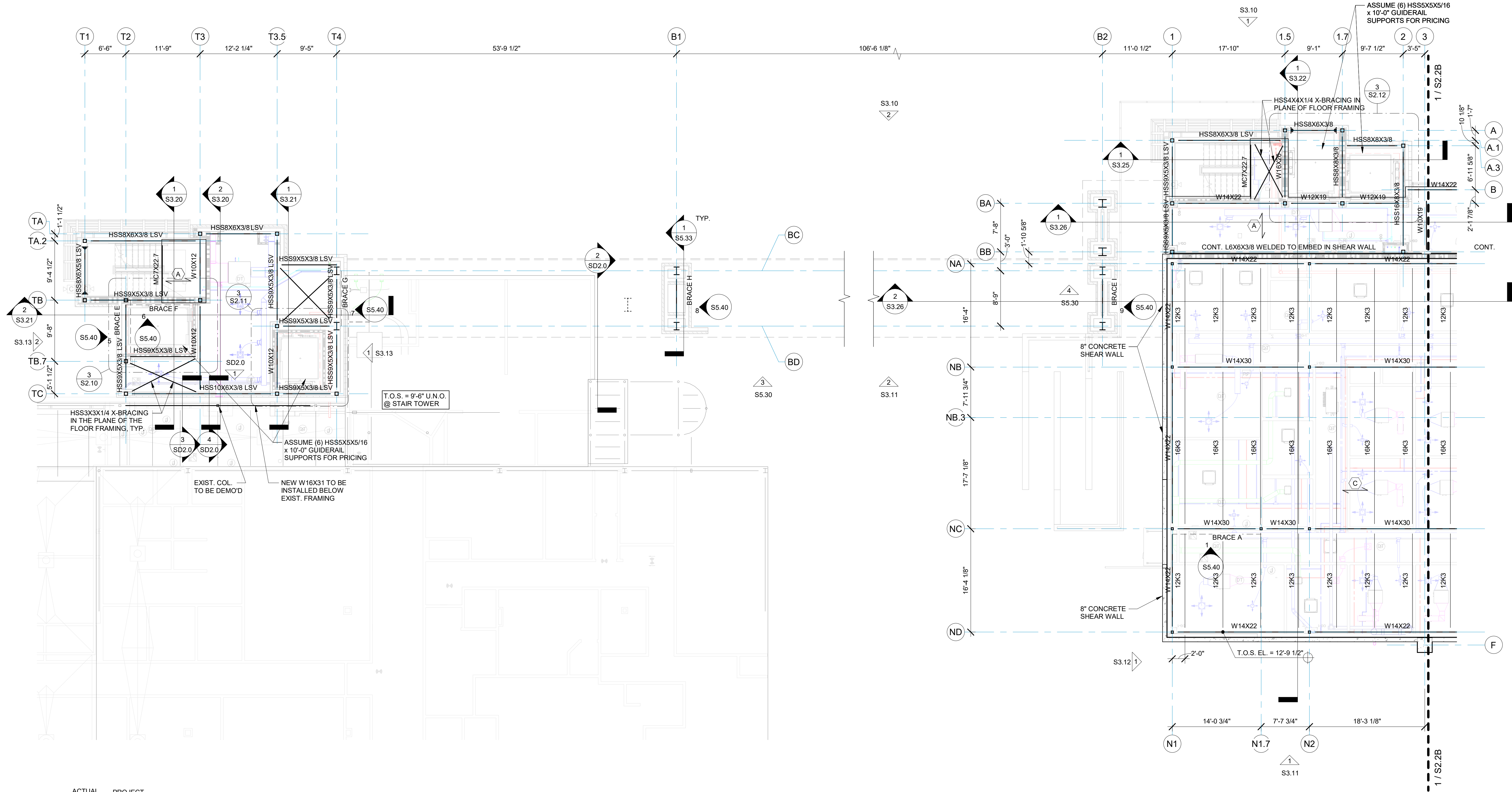
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1
S2.2A
LEVEL 2 PLAN - AREA A
1/8" = 1'-0"

SEE STRUCTURAL LEGEND SHEET
S0.1 FOR FRAMING LEGEND.

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LEVEL 2 PLAN - AREA A

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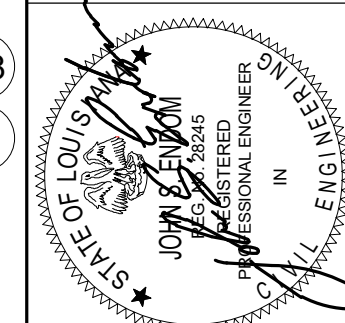
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S0.1 FOR FRAMING LEGEND.



1
S2.2B

LEVEL 2 PLAN - AREA B

1/8" = 1'-0"

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PROJECT
NORTH

A circular symbol with a vertical line through the center, indicating North.

1
S2.2C

LEVEL 2 PLAN - AREA C

1/8" = 1'-0"

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905-883-4100
985-883-4100

CHIECTS, APC.

[illegible]

TYLER STREET PARKING GARAGE
 619 WEST 12TH AVENUE, COVINGTON, LA 70433
 AND
ST. TAMMANY HEALTH SYSTEM
NORTHLAKE SURGICAL ASSOCIATES
 1028 SOUTH TYLER STREET, COVINGTON, LA 70433
 AT
ST. TAMMANY PARISH HOSPITAL

CONSTRUCTION
DOCUMENTS

Date
05.NOV.2025

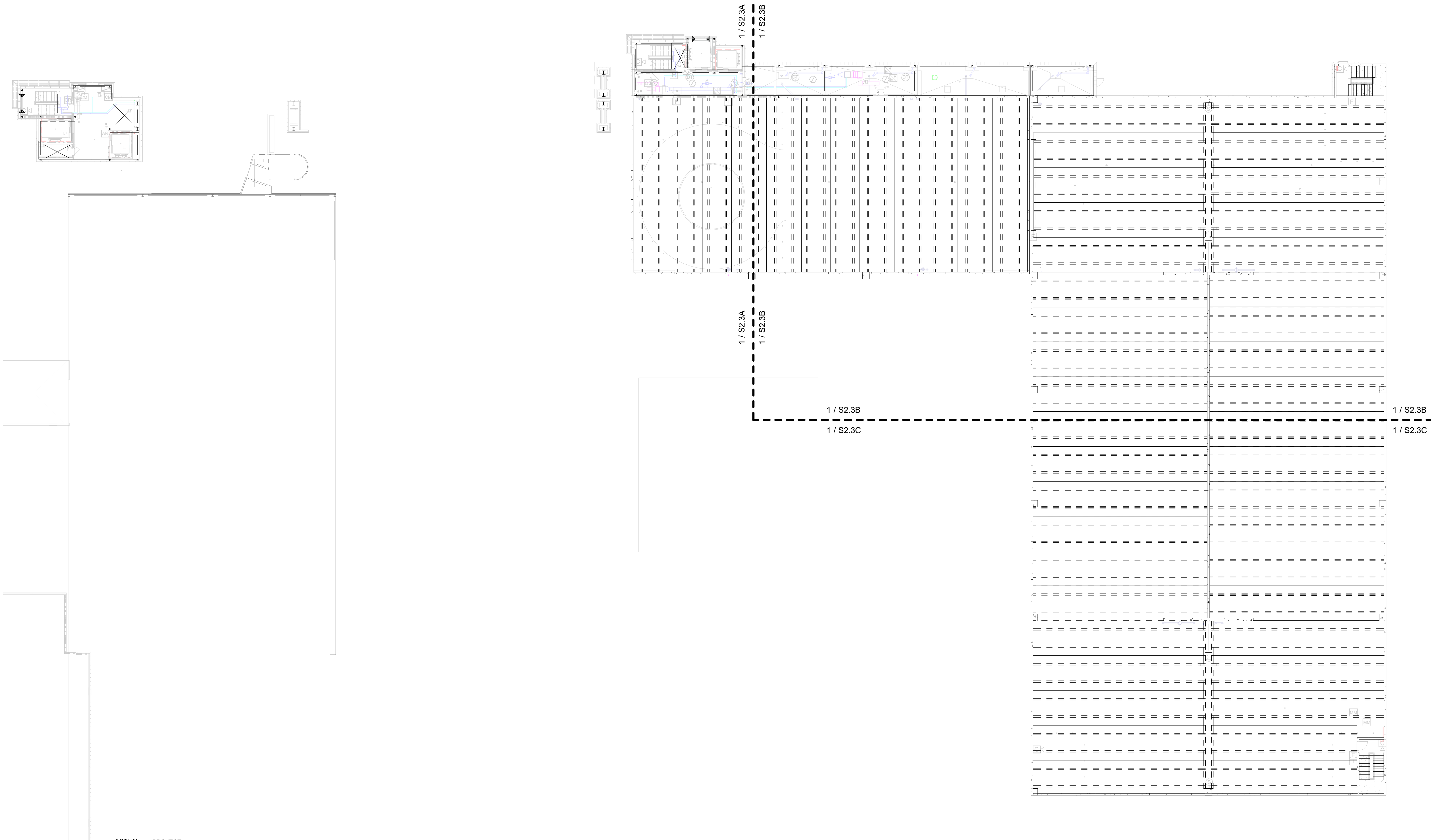
LEVEL 2 PLAN - AREA C

Job No. 222058.01

Drawn by	Checked by
<u>Author</u>	<u>Checker</u>

S2.2C

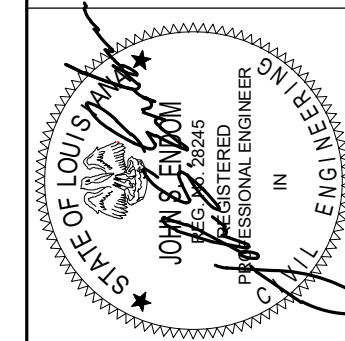
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1
S2.3

LEVEL 3 PLAN - OVERALL

1/16" = 1'-0"



DATE

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DOCUMENTS

Date
05 NOV 2025

LEVEL 3 PLAN - OVERALL

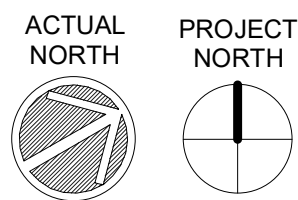
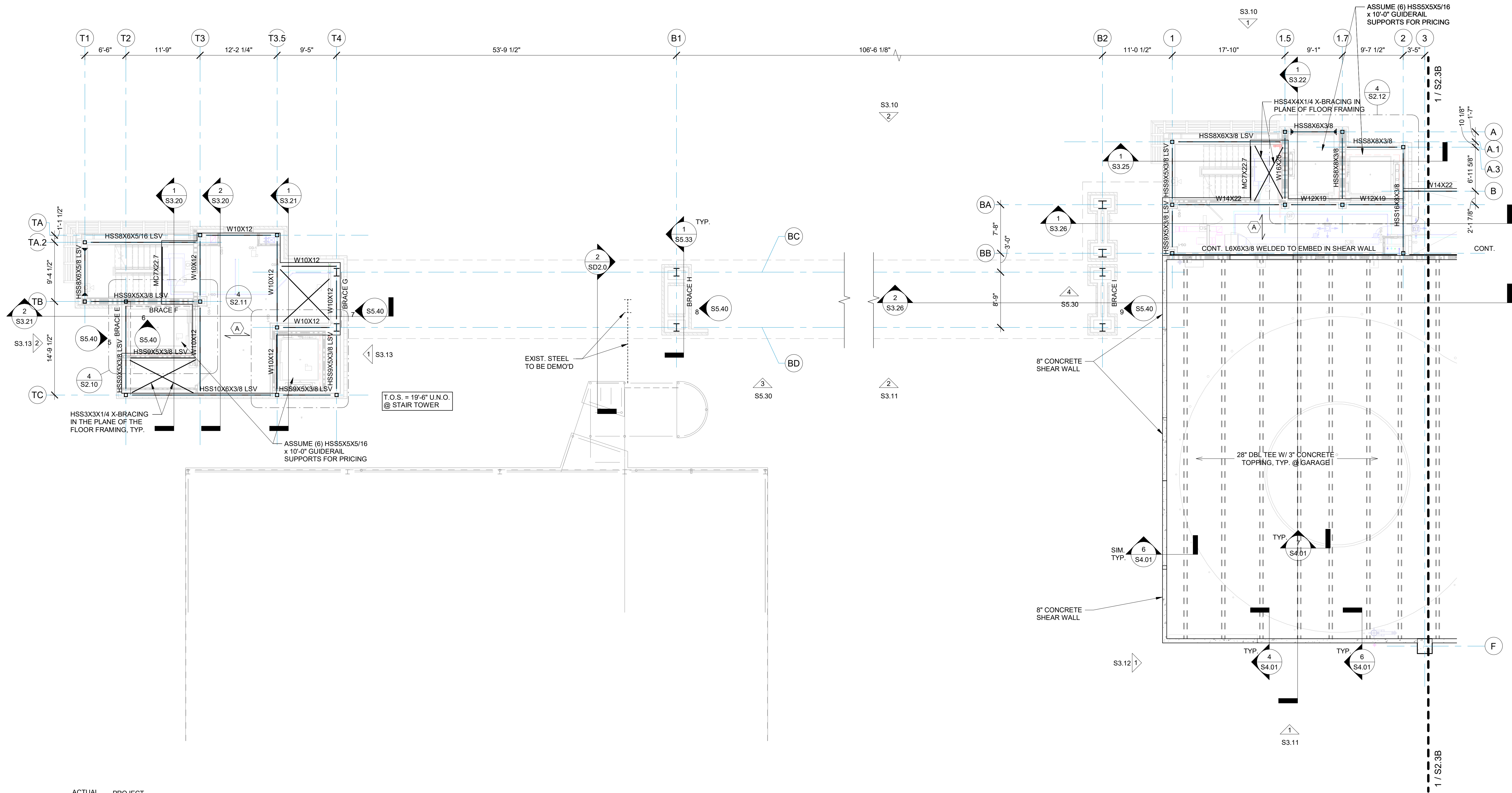
Job No. 222058.01

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S2.3
Sheet No.

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1
S2.3A
LEVEL 3 PLAN - AREA A
1/8" = 1'-0"

SEE STRUCTURAL LEGEND SHEET
S0.1 FOR FRAMING LEGEND.

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consulting

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INDEPENDENT
SINCE 1981

STATE OF LOUISIANA
Professional Engineer
JOHN J. BROWN
No. 12345
Exp. 12/31/2025

DATE

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TYLER STREET PARKING GARAGE
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LEVEL 3 PLAN - AREA A

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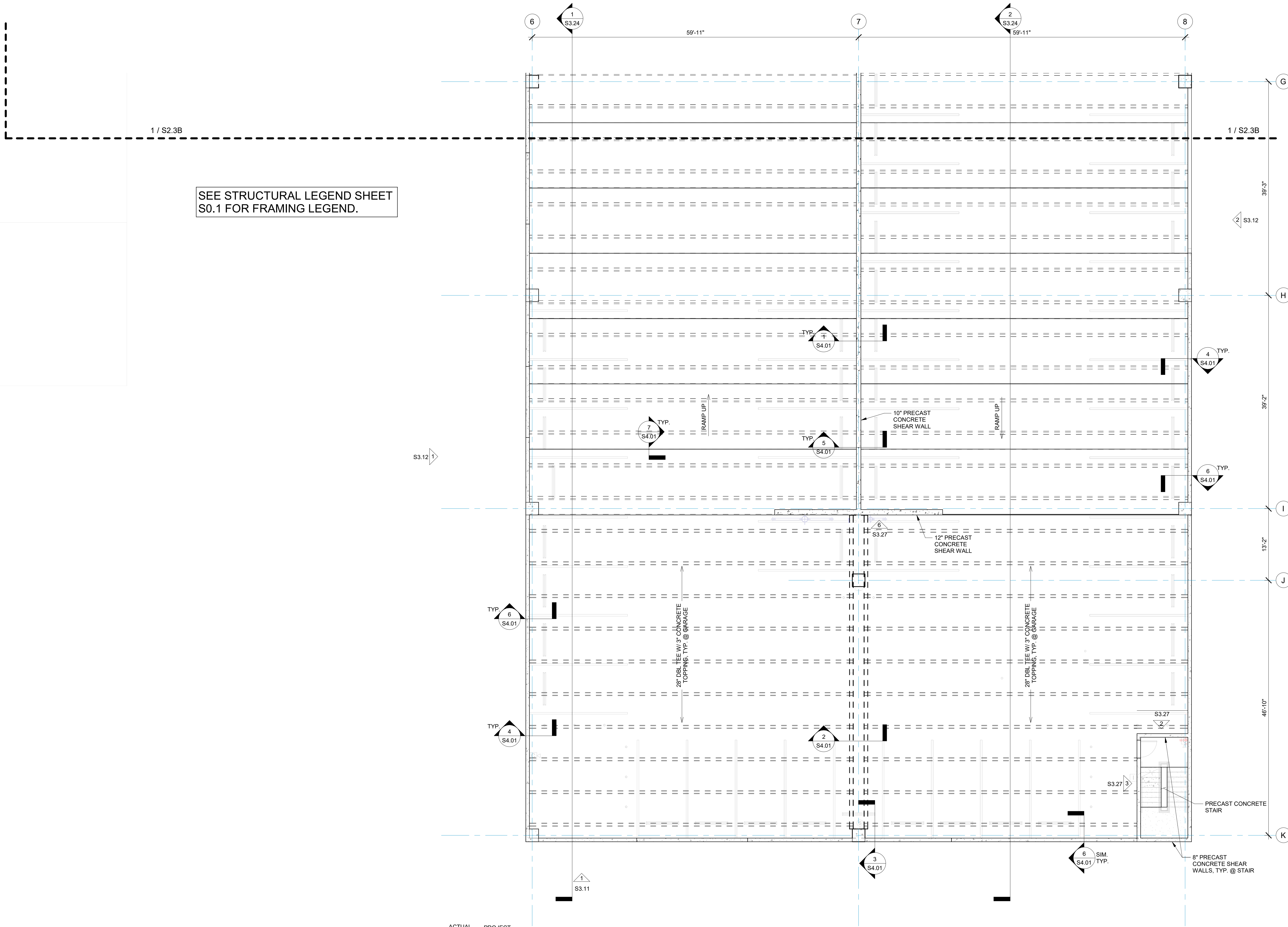
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ST. TAMMANNI LAMBERTI HOJII HALL

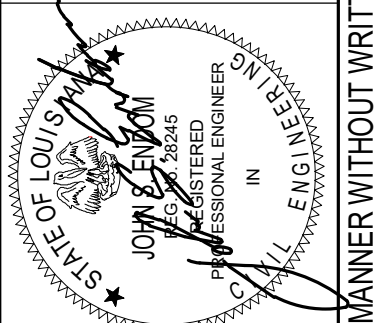
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SEE STRUCTURAL LEGEND SHEET
S0.1 FOR FRAMING LEGEND.

1 / S2.3B

1 / S2.3B



NO.	ISSUE	DATE

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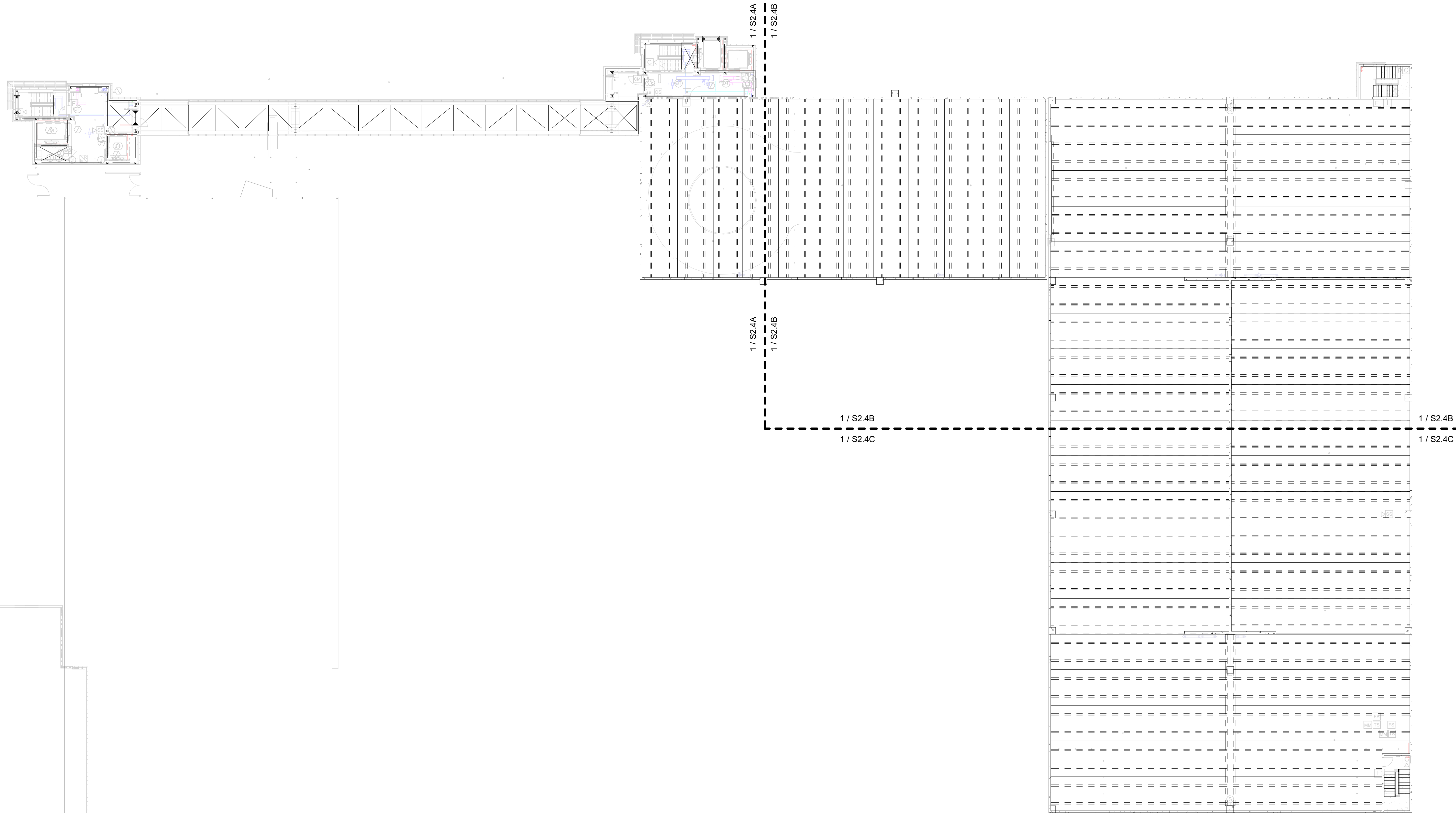
LEVEL 3 PLAN - AREA C

Job No. 222058.01

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S2.3C
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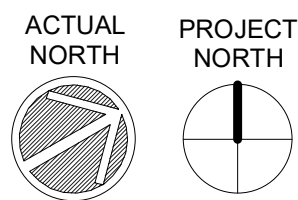
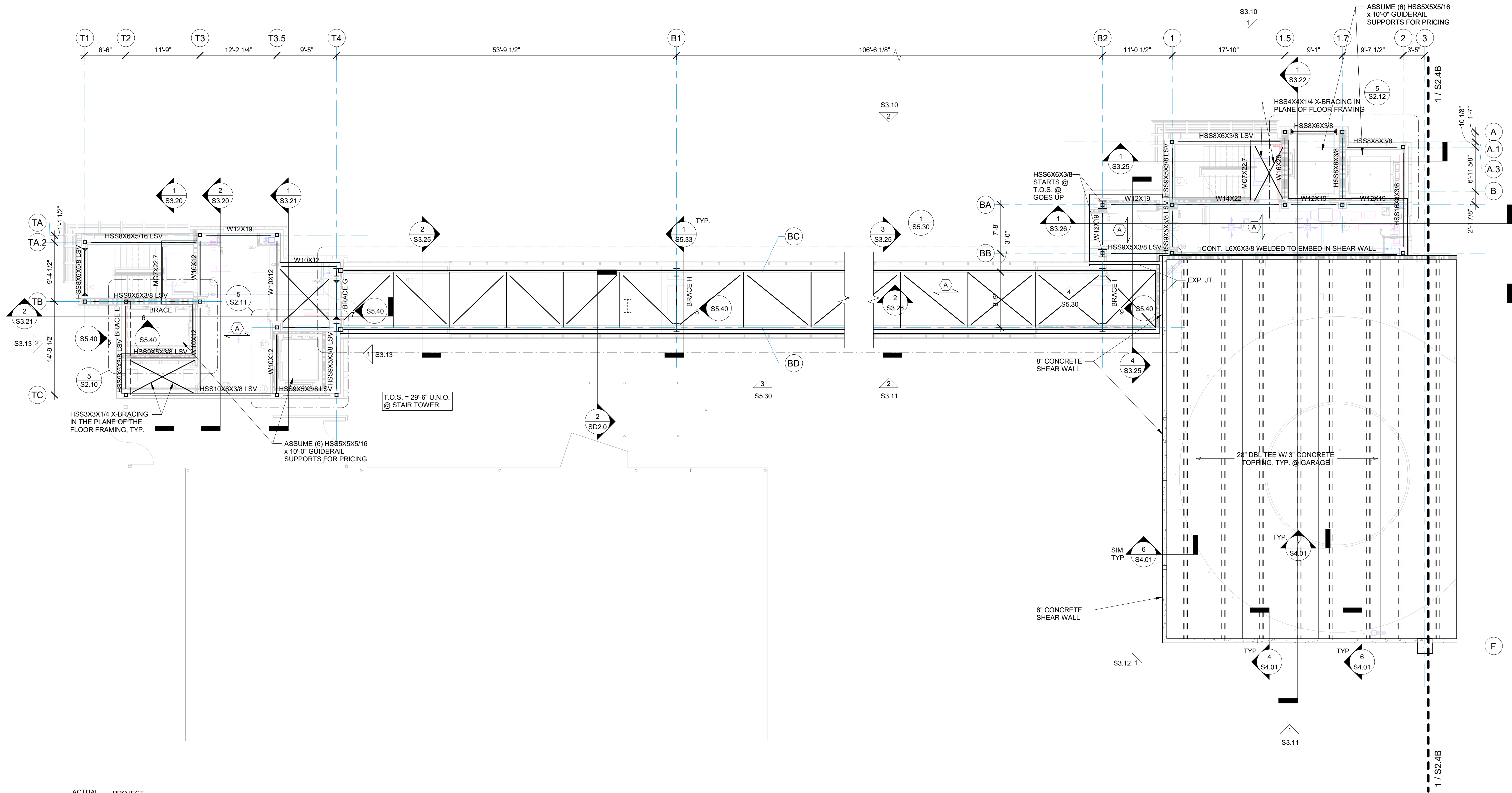


1
S2.4

LEVEL 4 PLAN - OVERALL

1/16" = 1'-0"

architecture planning interior design sustainable design consulting		fl+ WB architects a professional corporation		90 LOUIS PRIMA DRIVE, SUITE A COVINGTON, LA 70433 885-893-4100		a professional corporation in continuous practice since 1981	
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CONSTRUCTION DOCUMENTS		Date 05 NOV 2025					
LEVEL 4 PLAN - OVERALL							
Job No. 222058.01							
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S2.4							
Sheet No.							



1
S2.4A
LEVEL 4 PLAN - AREA A
1/8" = 1'-0"

SEE STRUCTURAL LEGEND SHEET
S0.1 FOR FRAMING LEGEND.

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ENGINEER
IN
LOUISIANA

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Date
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LEVEL 4 PLAN - AREA A

Job No. 222058.01

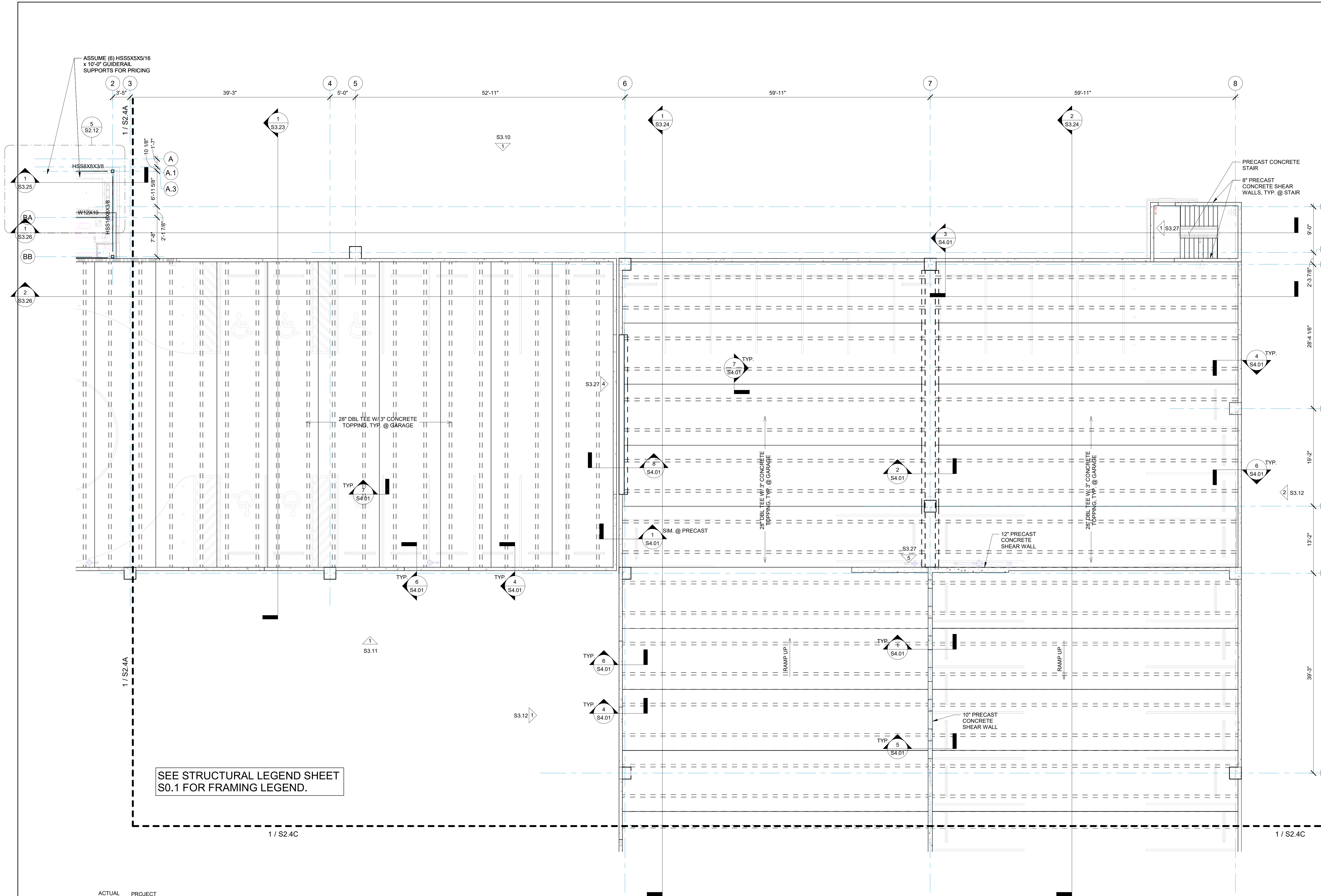
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1
S2.4B

LEVEL 4 PLAN - AREA B
1/8" = 1'-0"

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interior design
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865-893-4100

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Professional Engineer
JOHN B. WILSON
No. 10000
Exp. 12/31/2025

NO.	ISSUE	DATE

TYLER STREET PARKING GARAGE
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1028 SOUTH TYLER STREET, CONVINGTON, LA 70433
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Date
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LEVEL 4 PLAN - AREA B

Job No. 222058.01

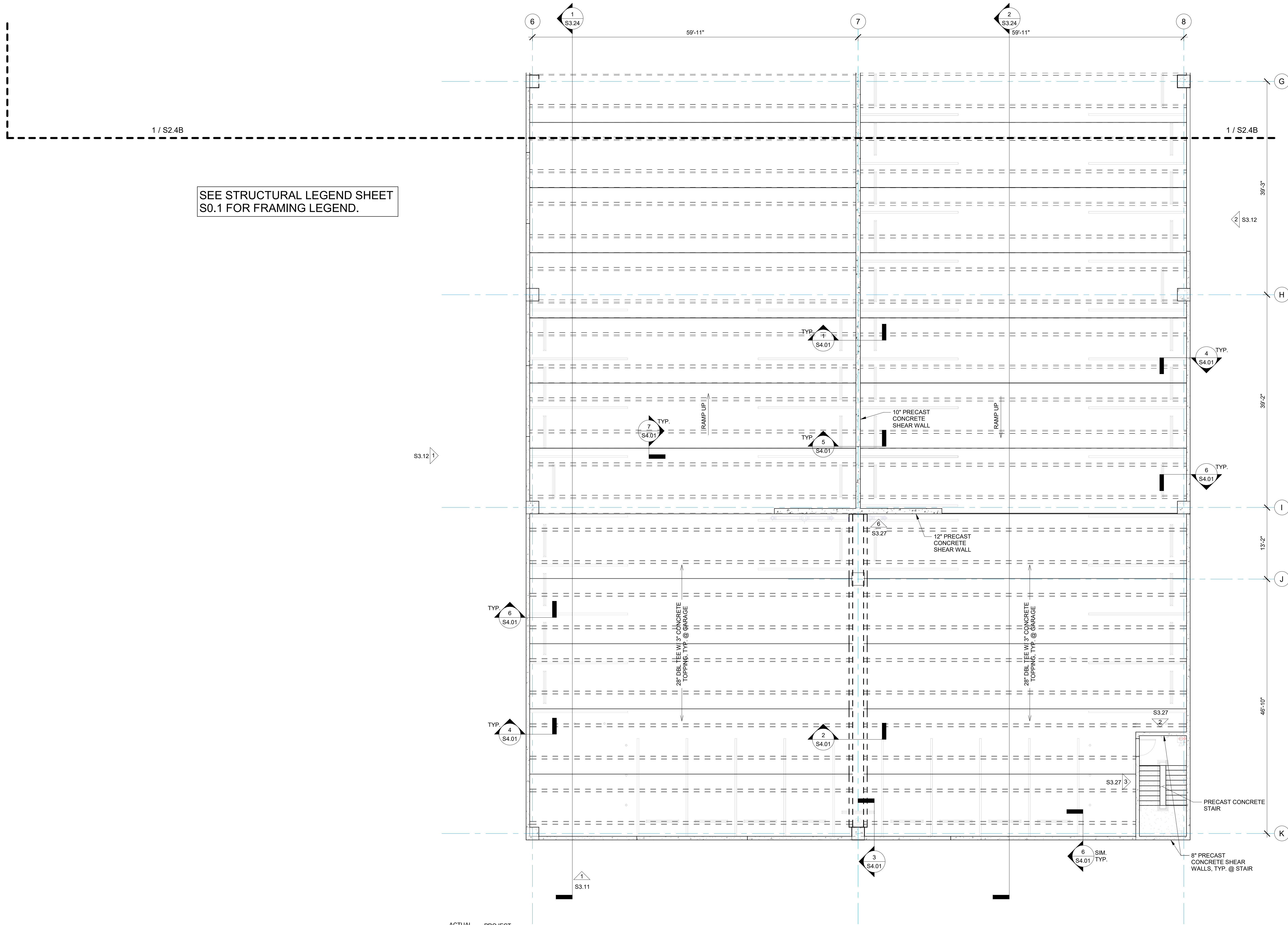
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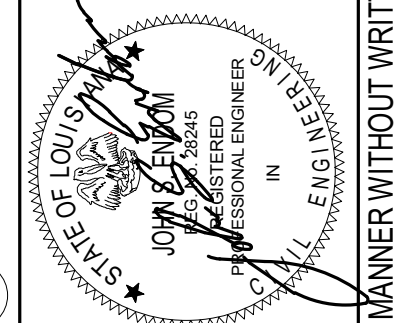


SEE STRUCTURAL LEGEND SHEET
S0.1 FOR FRAMING LEGEND.



1
S2.4C

LEVEL 4 PLAN - AREA C
1/8" = 1'-0"



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LEVEL 4 PLAN - AREA C

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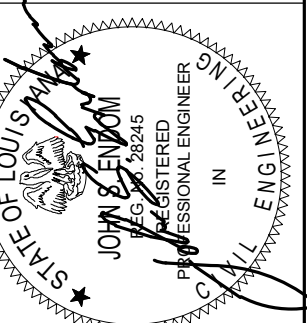

$$1/8'' = 1'-0''$$

SEE STRUCTURAL LEGEND SHEET
S0.1 FOR FRAMING LEGEND.



1
S2.5B

LEVEL 5 PLAN - AREA B

$$1/8'' = 1'-0''$$


DATE	NO.	ISSUE

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AT
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CONSTRUCTION
DOCUMENTS

6 NOV 2025

LEVEL 5 PLAN - AREA B

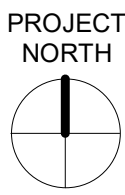
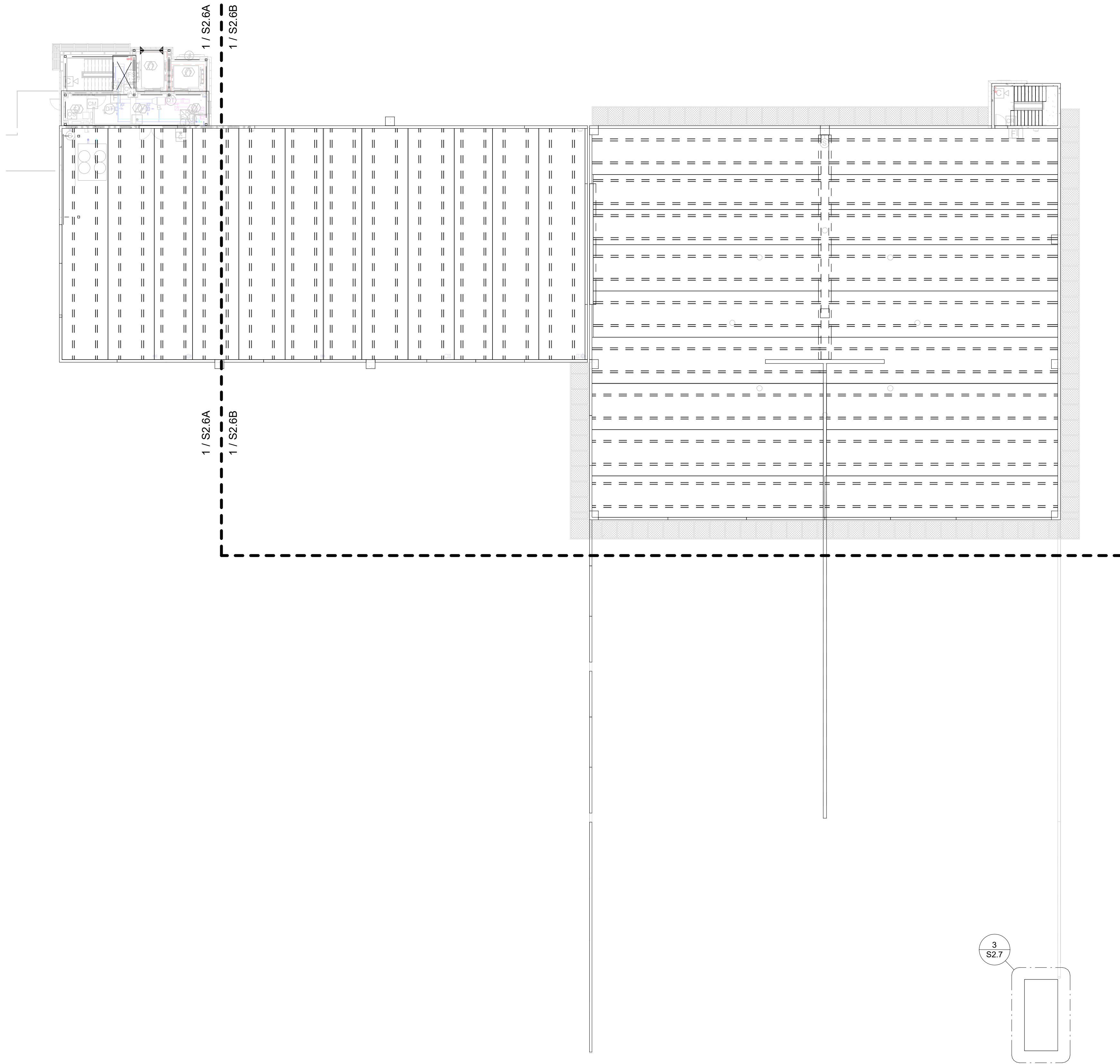
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S2.5B

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1
S2.6

ROOF PLAN - OVERALL

1/16" = 1'-0"

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619 WEST 12TH AVENUE, COVINGTON, LA 70433
AND
ST. TAMMANY HEALTH SYSTEM
NORTHLAKE SURGICAL ASSOCIATES
1028 SOUTH TYLER STREET, COVINGTON, LA 70433
AT
ST. TAMMANY PARISH HOSPITAL

CONSTRUCTION
DOCUMENTS

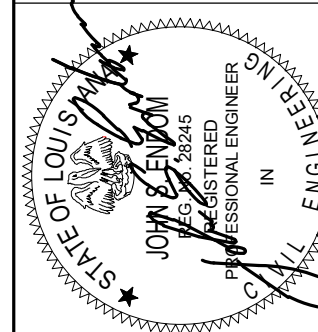
Date
05 NOV 2025

ROOF PLAN - OVERALL

Job No. 222058.01

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S2.6
Sheet No.



DATE

NO. ISSUE

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ROOF PLAN - OVERALL

Job No. 222058.01

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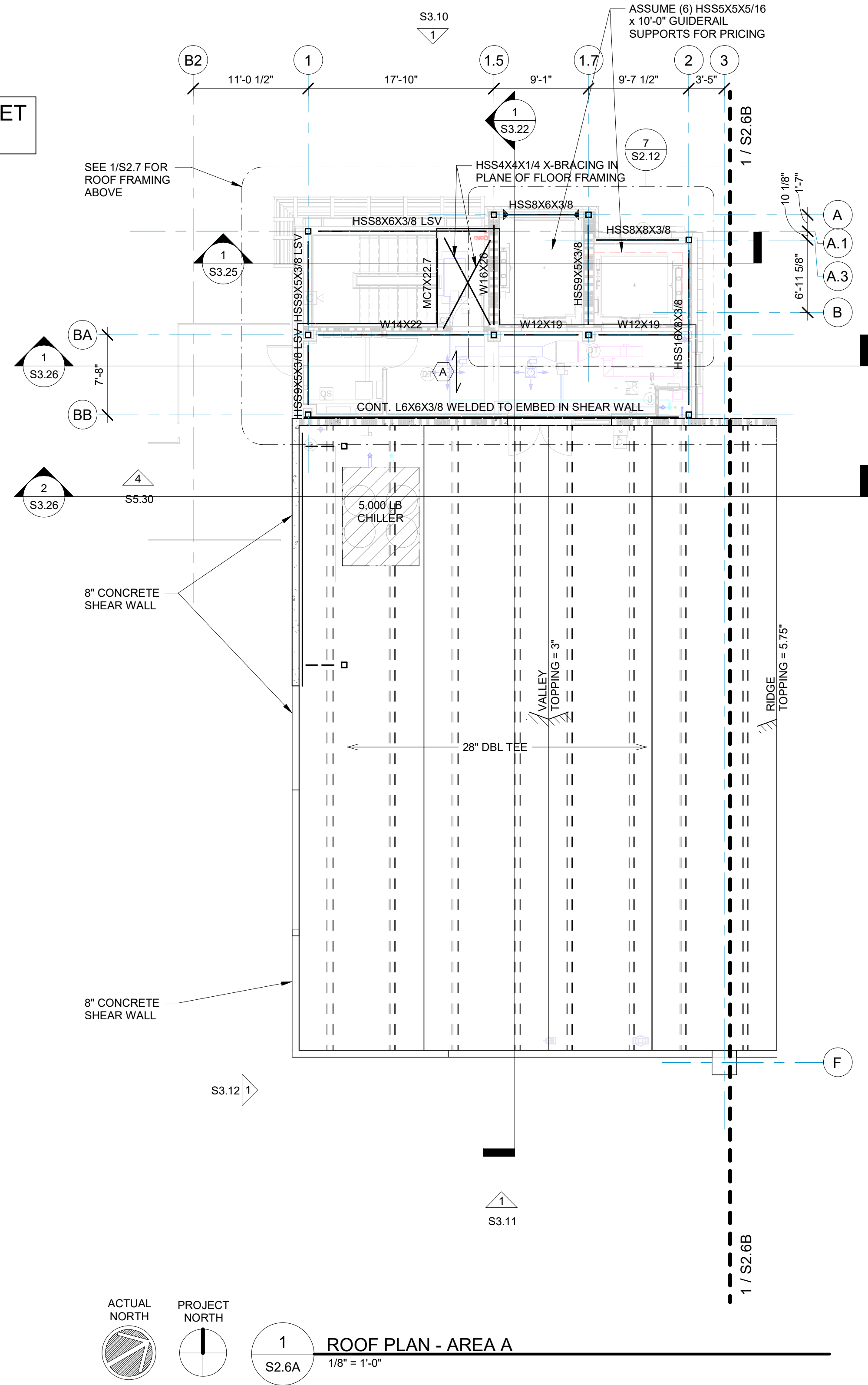
S2.6
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ACTUAL
NORTH

PROJECT
NORTH



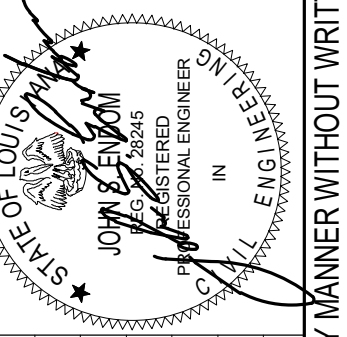
1 ROOF PLAN - AREA A
S2.6A 1/8" = 1'-0"

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CHITECTS, APC.



DATE	DESCRIPTION	AMOUNT	CHECK NO.	BANK	REMARKS
10/1/2023	Initial deposit	1000.00		Bank of America	Starting balance
10/5/2023	Deposit	500.00	101	Bank of America	Monthly deposit
10/10/2023	Withdrawal	250.00	102	Bank of America	ATM withdrawal
10/15/2023	Deposit	750.00	103	Bank of America	Monthly deposit
10/20/2023	Withdrawal	300.00	104	Bank of America	ATM withdrawal
10/25/2023	Deposit	600.00	105	Bank of America	Monthly deposit
10/30/2023	Withdrawal	400.00	106	Bank of America	ATM withdrawal
10/31/2023	Final balance	1000.00		Bank of America	Month-end balance

NO. 1330E	
-----------	--

619 WEST 12TH AVENUE, COVINGTON, LA 70433
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ST. TAMMANY HEALTH SYSTEM
NORTHLAKE SURGICAL ASSOCIATE,
1028 SOUTH TYLER STREET, COVINGTON, LA 70433
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ST. TAMMANY PARISH HOSPITAL
ECTS, APC AND ARE ISSUED AS INSTRUMENTS OF SUR

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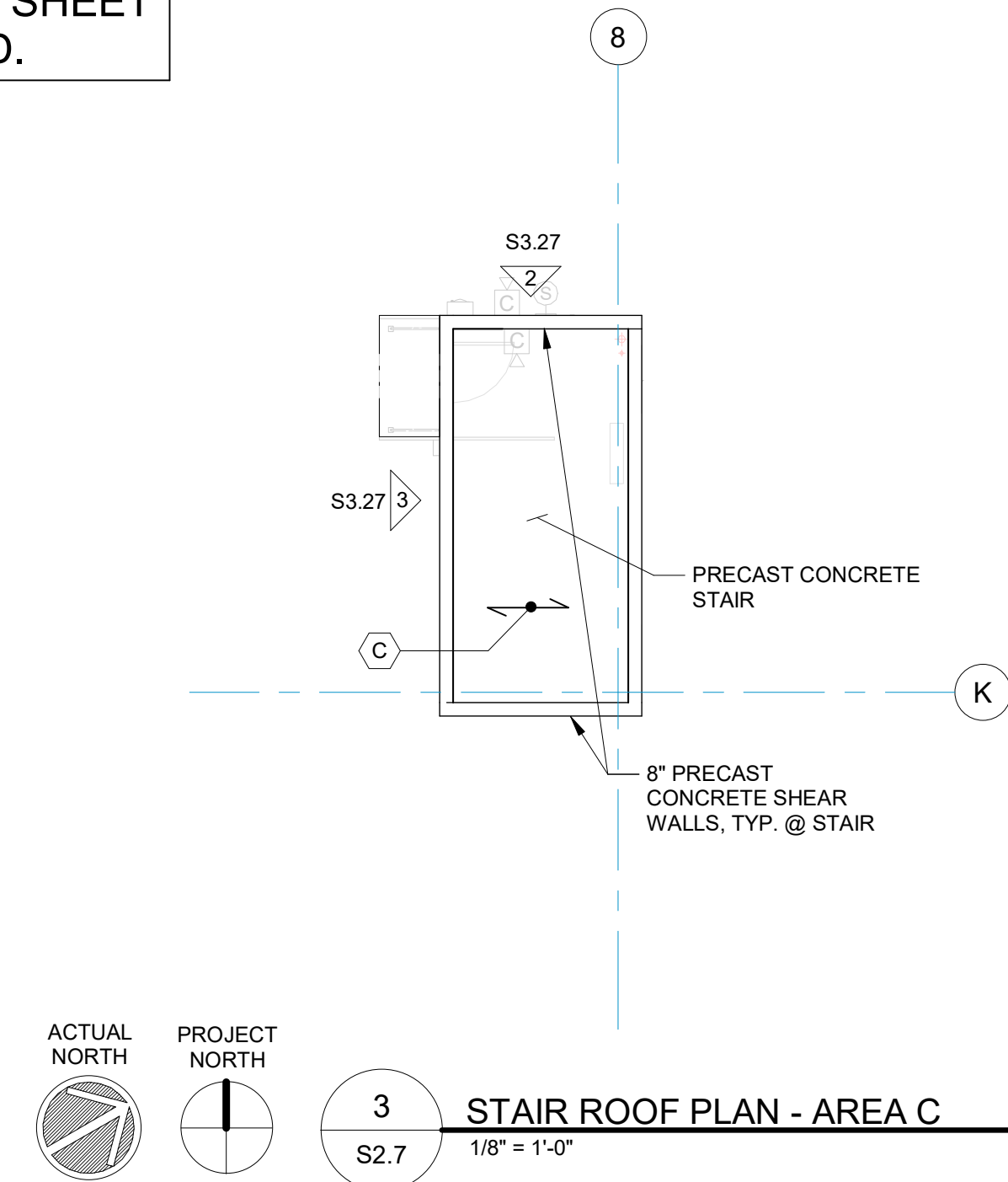
OF PLAN - AREA A

No. 222058.01

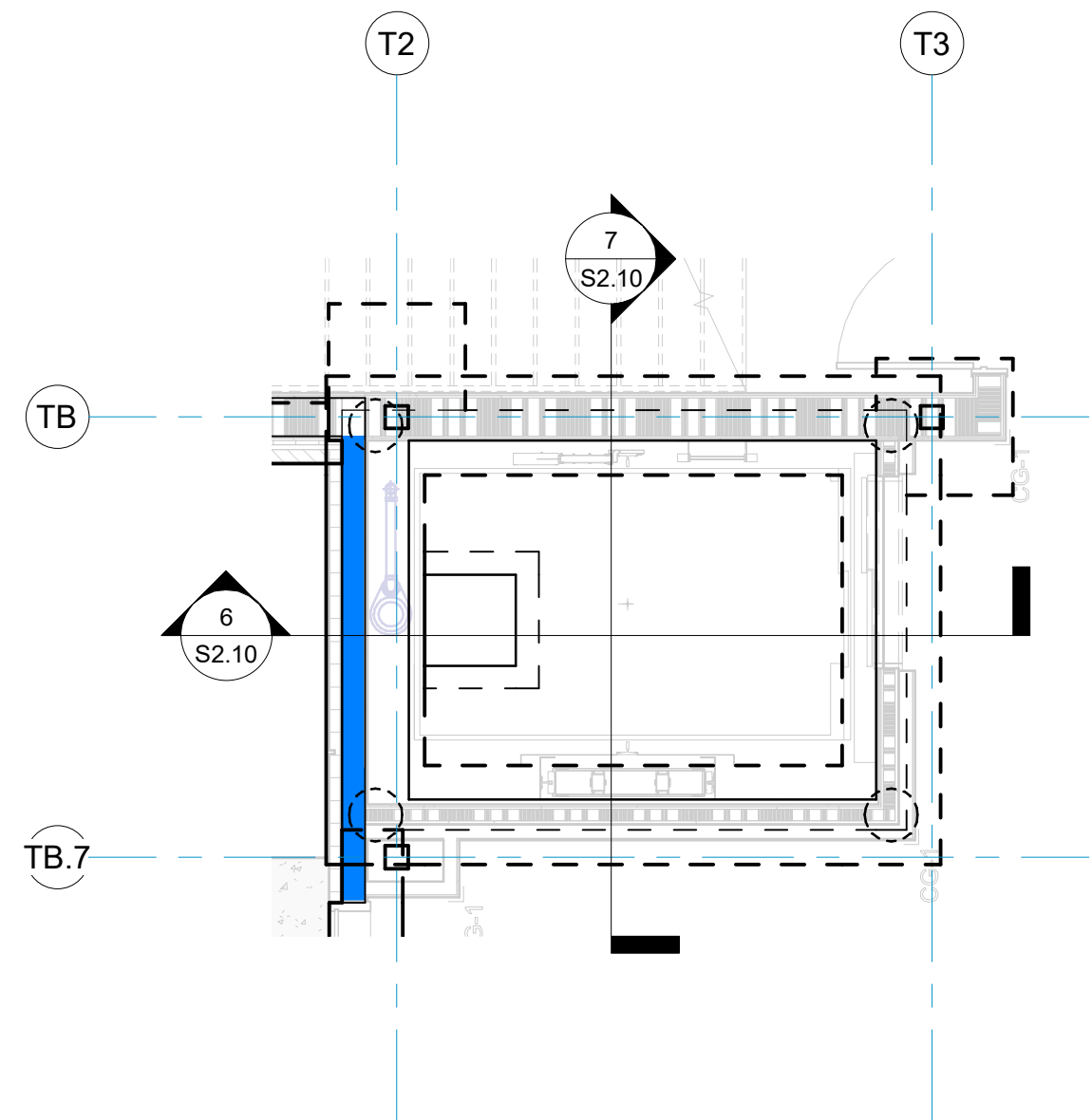
Written by _____
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 Author _____
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S2.6A

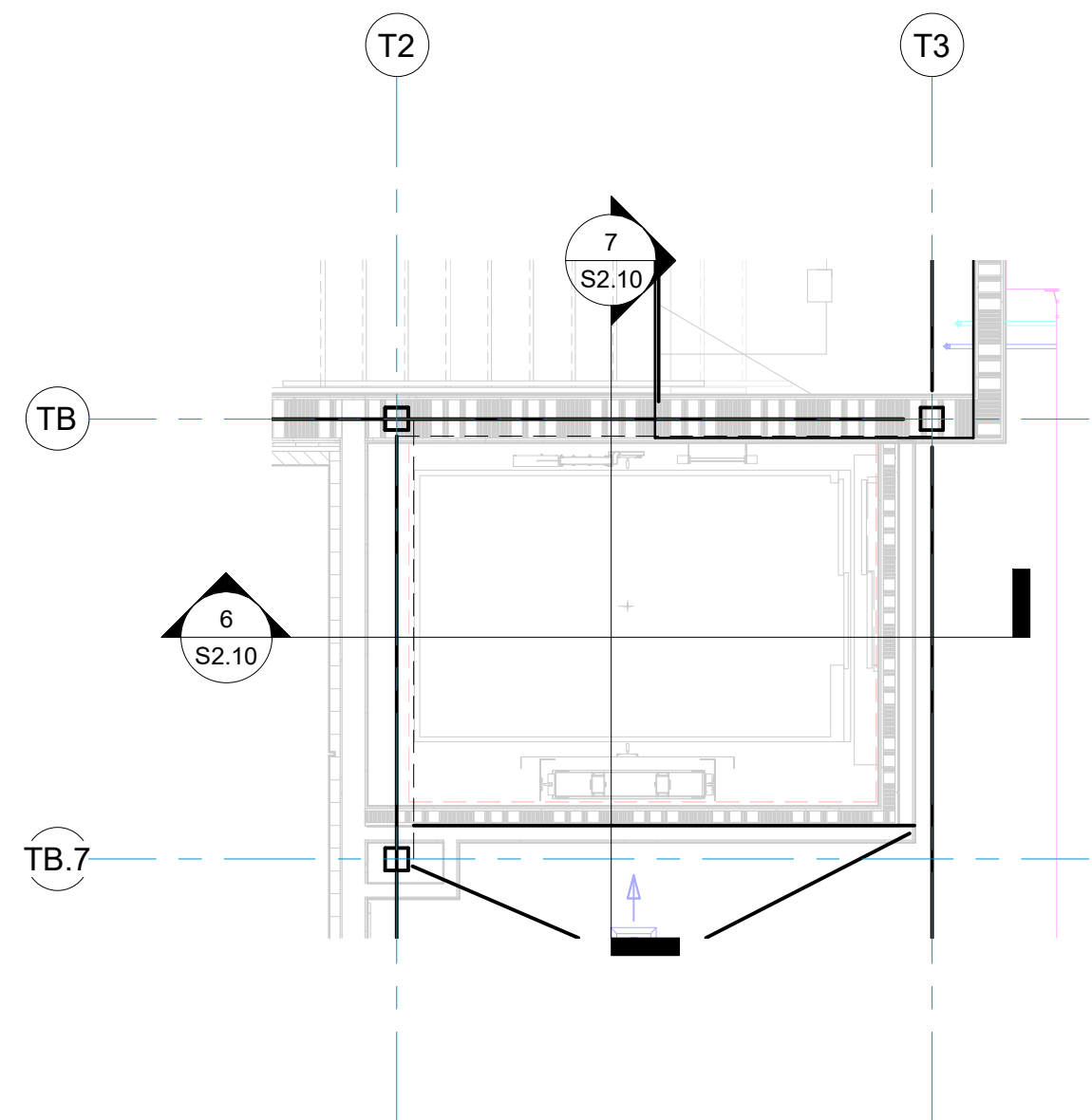
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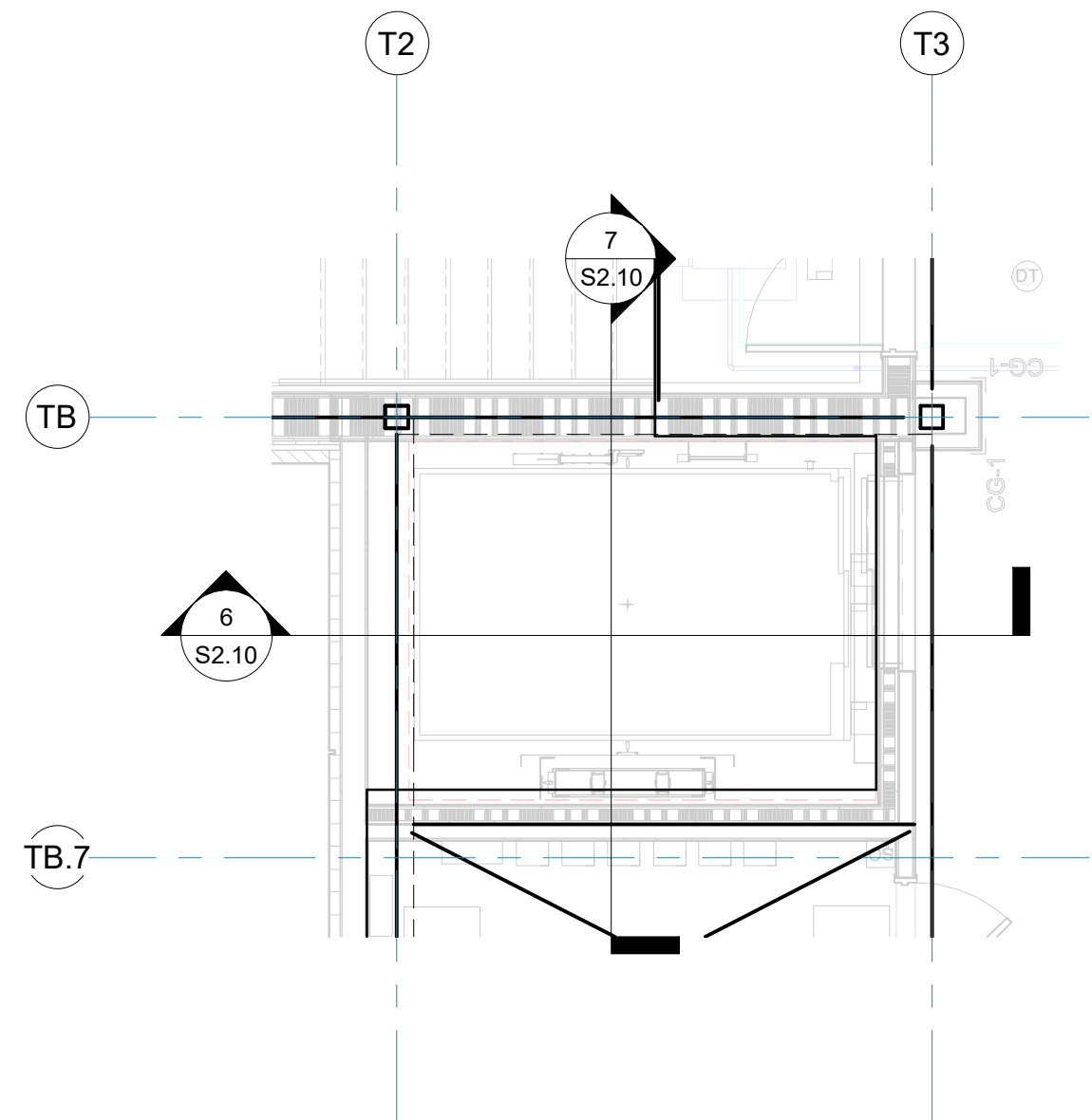
architecture planning interiors sustainable design consulting 90 LOUIS PRIMA DRIVE, SUITE A COVINGTON, LA 70433 985-882-4100					
TYLER STREET PARKING GARAGE 619 WEST 12TH AVENUE, COVINGTON, LA 70433 AND ST. TAMMANY HEALTH SYSTEM NORTHLAKE SURGICAL ASSOCIATES 1028 SOUTH TYLER STREET, COVINGTON, LA 70433 AT ST. TAMMANY PARISH HOSPITAL		NO. ISSUE _____ _____ _____ _____ _____		DATE _____ _____ _____ _____ _____	
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Job No. 222058.01					
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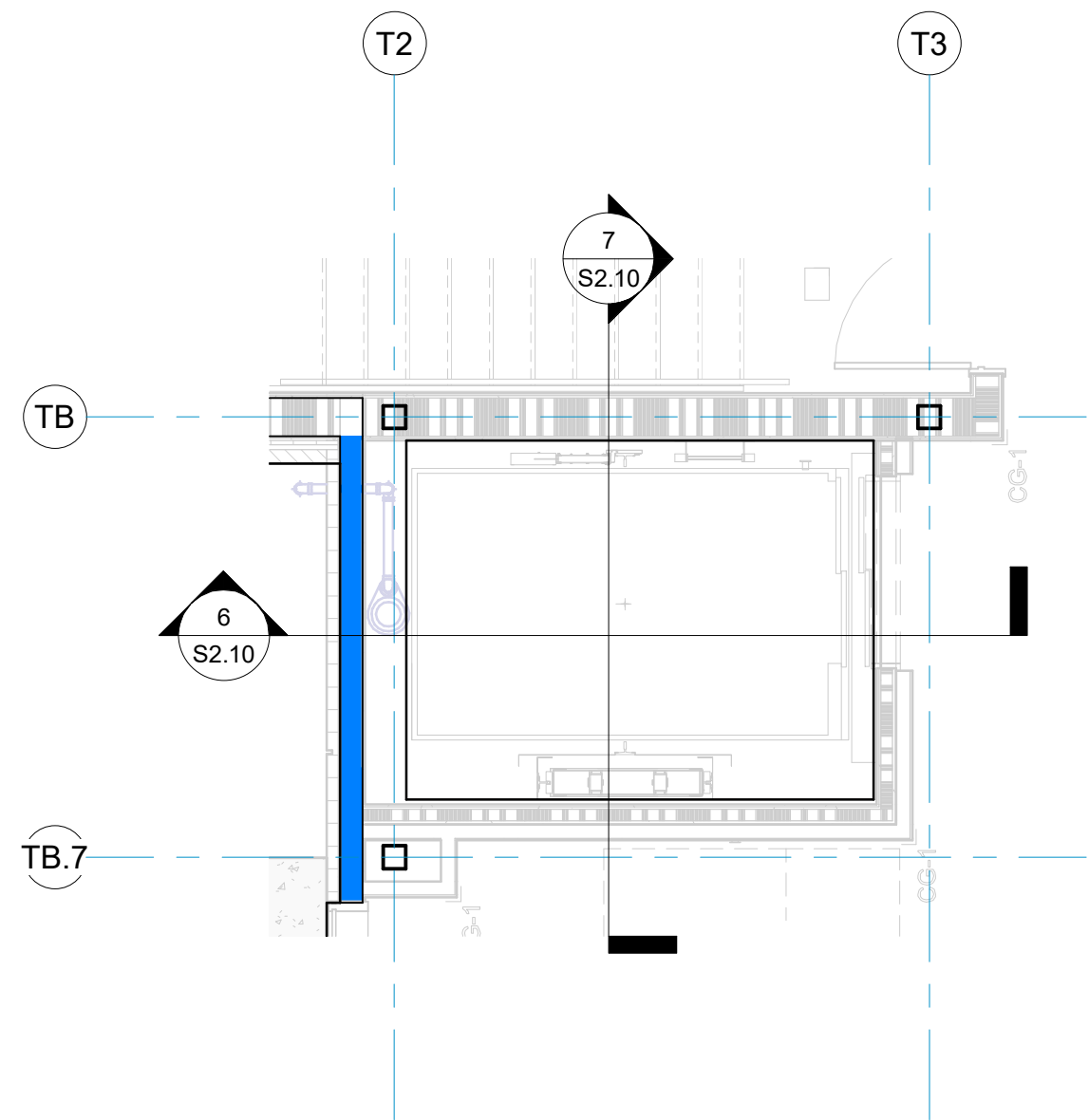
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S2.10 1/4" = 1'-0"



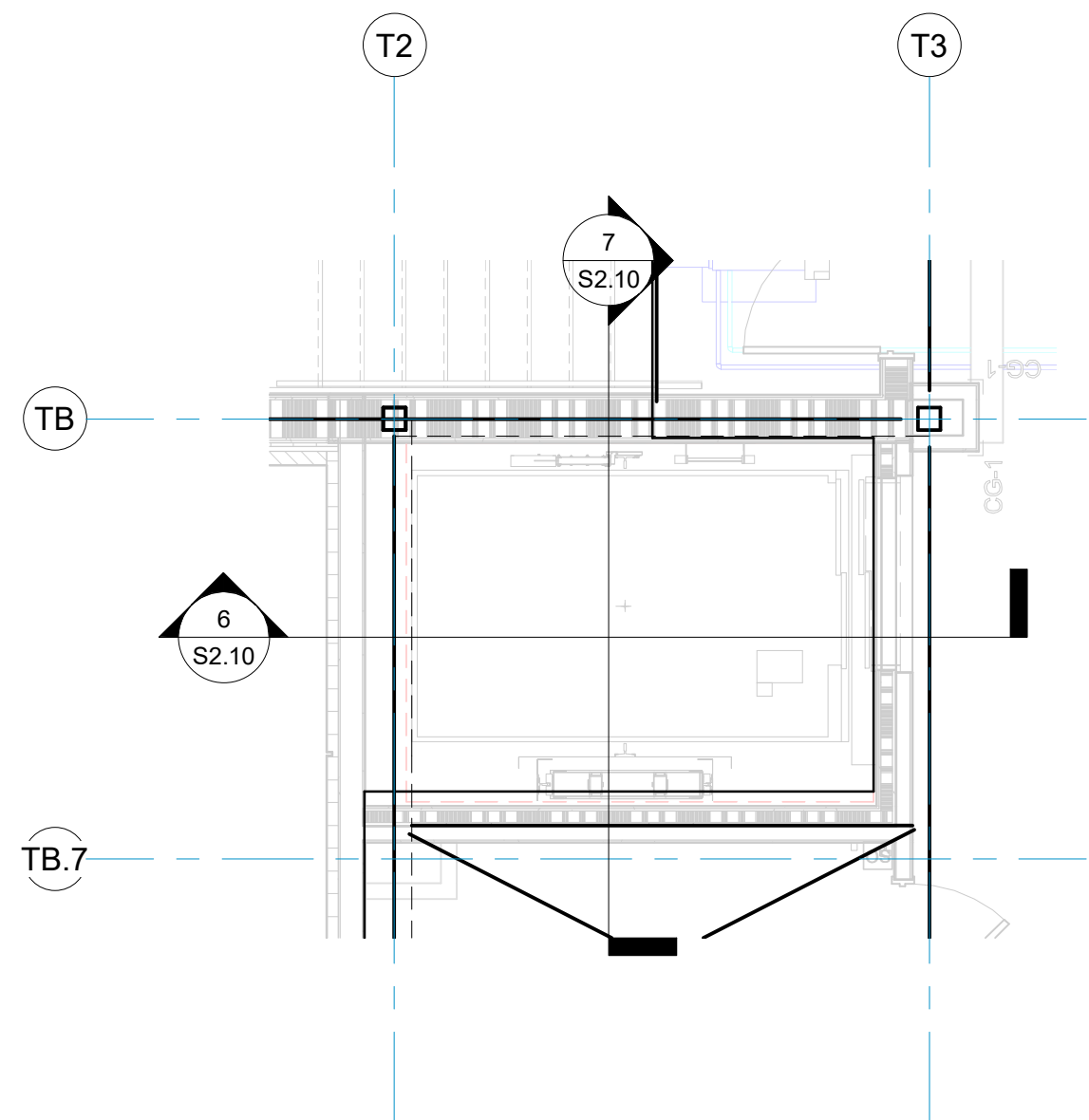
3 ELEVATOR 1 - LEVEL 2 PLAN
S2.10 1/4" = 1'-0"



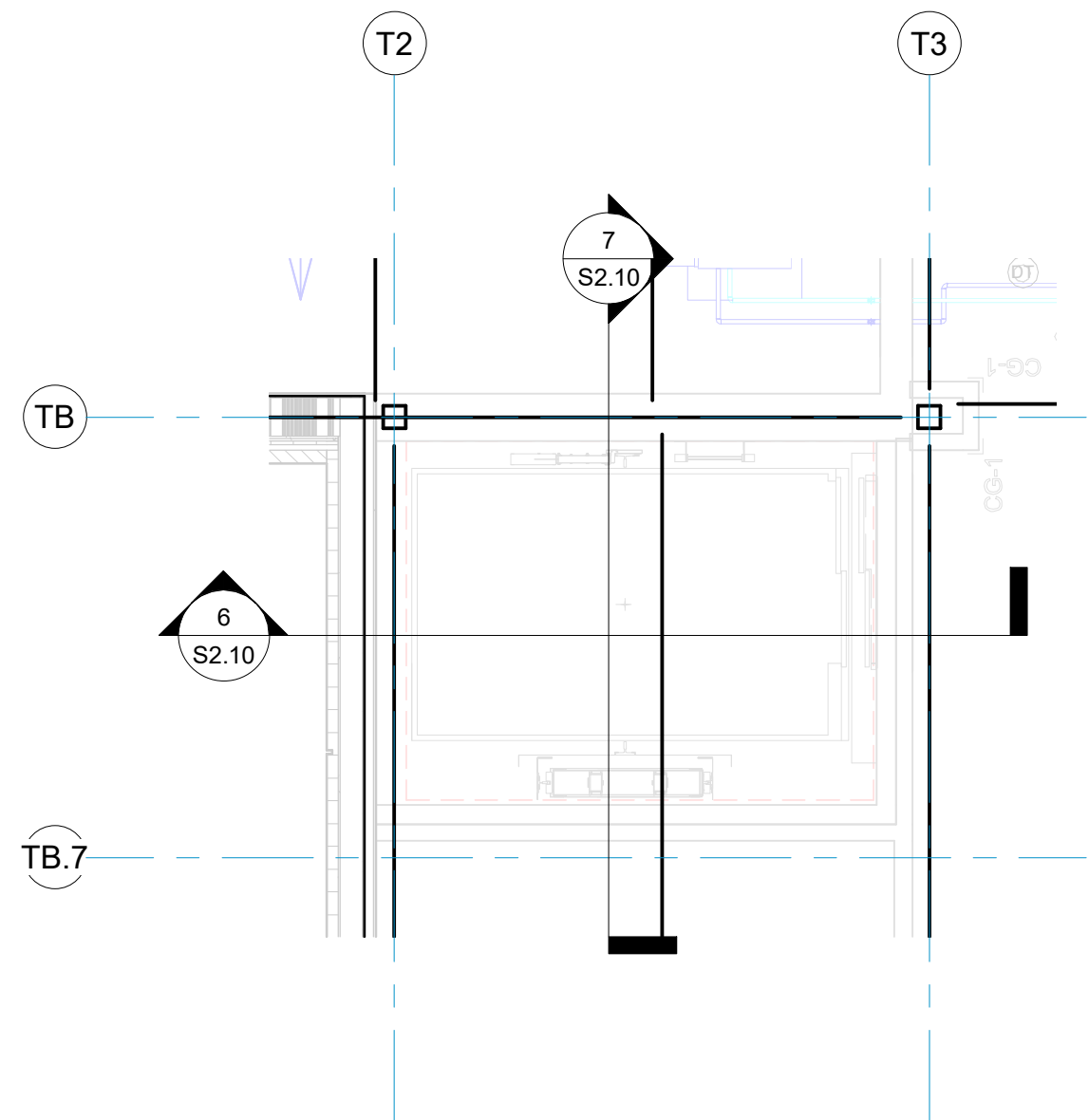
5 ELEVATOR 1 - LEVEL 4 PLAN
S2.10 1/4" = 1'-0"



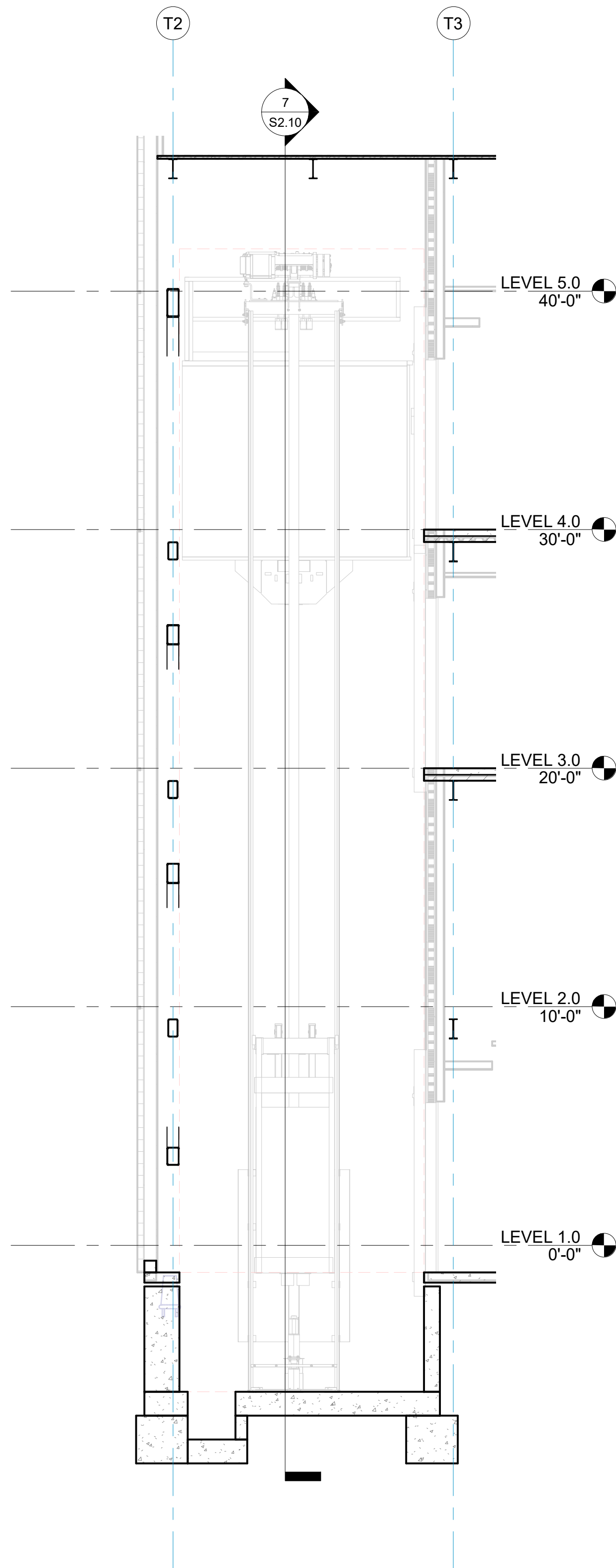
2 ELEVATOR 1 - LEVEL 1 PLAN
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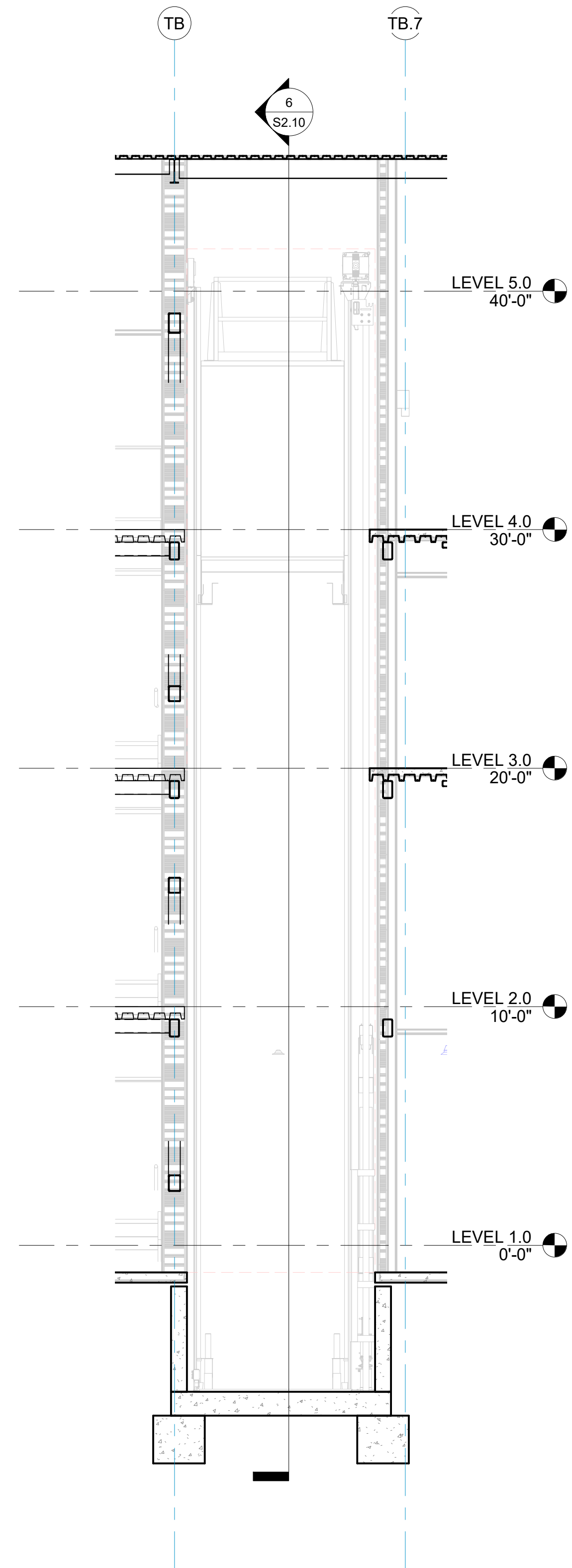
4 ELEVATOR 1 - LEVEL 3 PLAN
S2.10 1/4" = 1'-0"



8 ELEVATOR 1 - ROOF PLAN
S2.10 1/4" = 1'-0"



6 ELEVATOR 1 SECTION
S2.10 1/4" = 1'-0"



7 ELEVATOR 1 SECTION
S2.10 1/4" = 1'-0"

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STATE OF LOUISIANA
Professional Engineer
JOHN B. BROWN
No. 12345
Exp. 12/31/2025

DATE

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TYLER STREET PARKING GARAGE
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AT
ST. TAMMANY PARISH HOSPITAL

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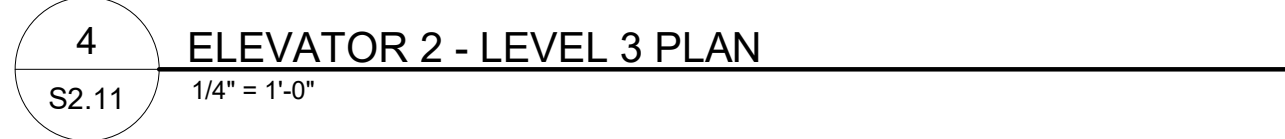
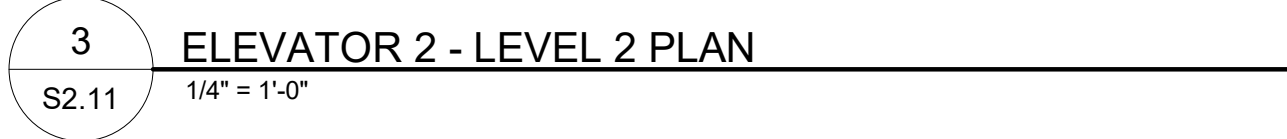
ELEVATOR 1 PLANS & SECTIONS

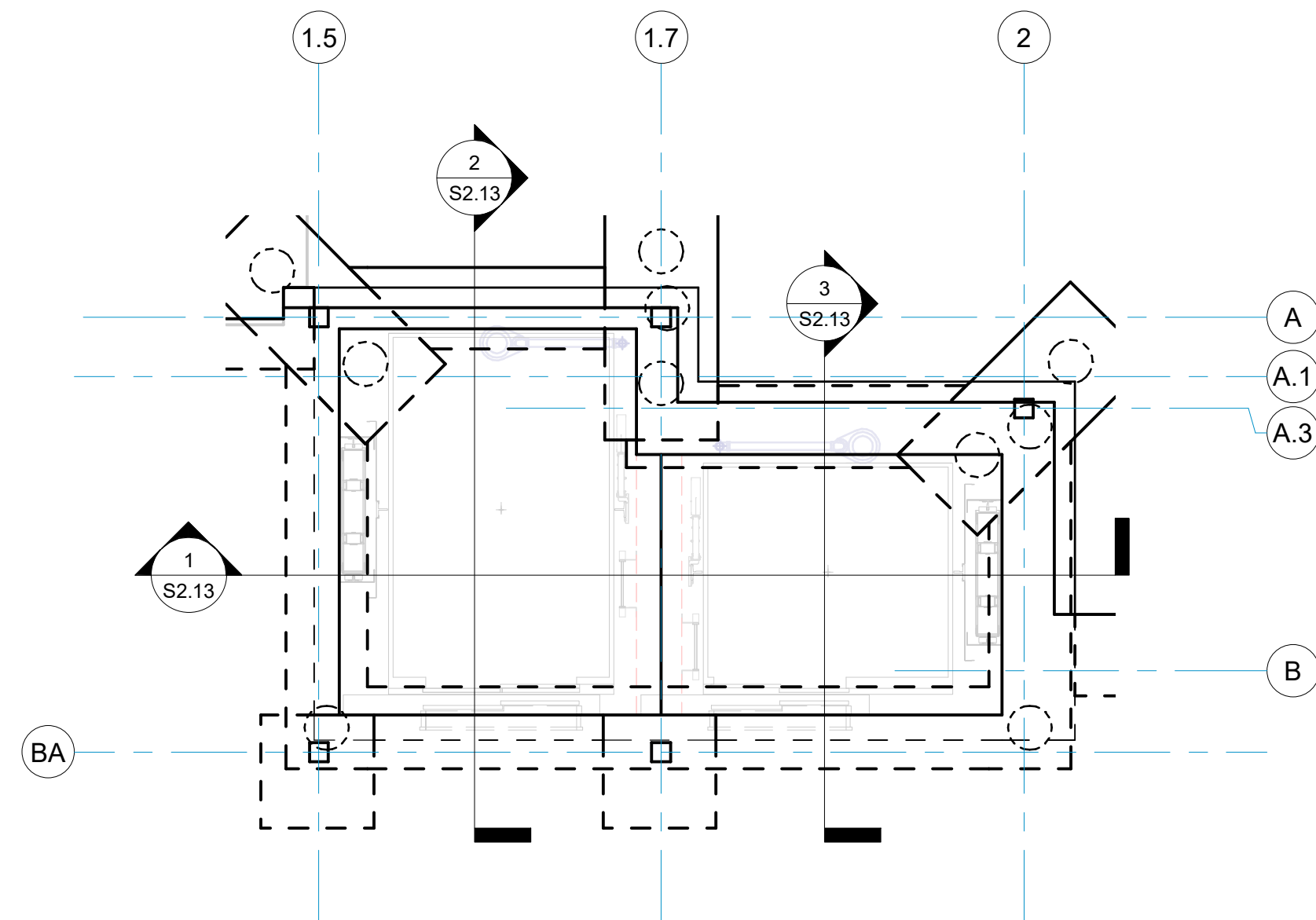
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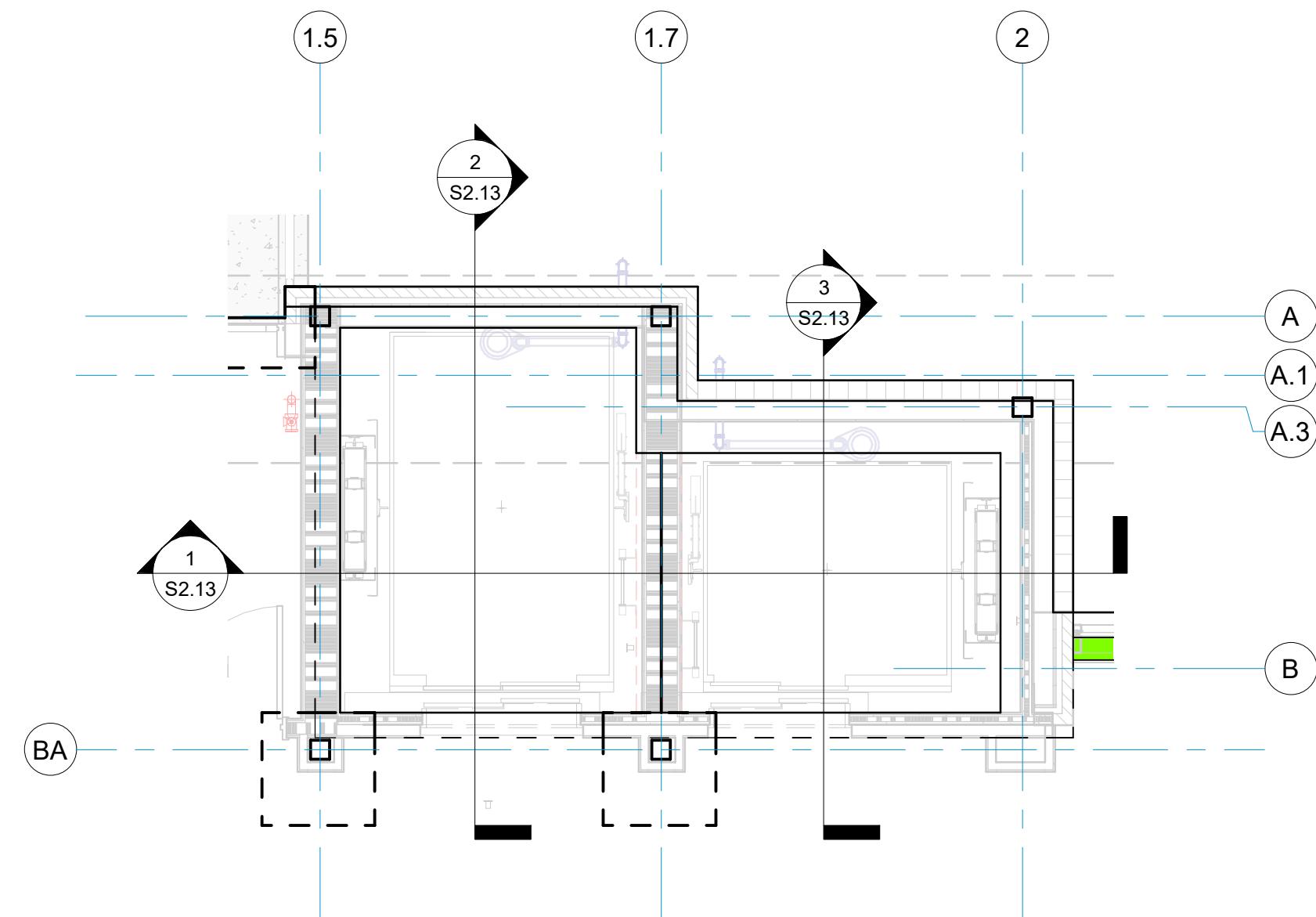
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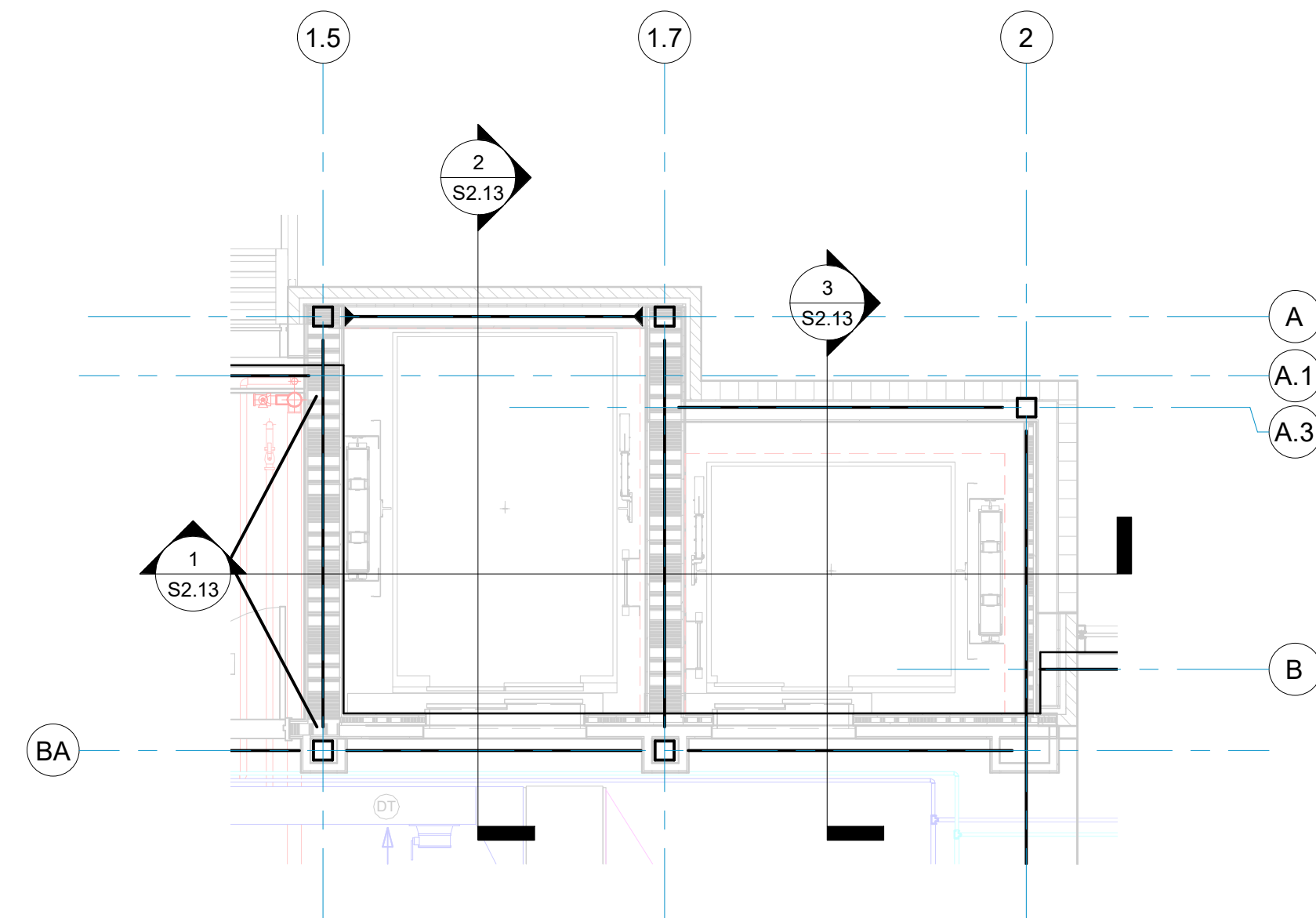




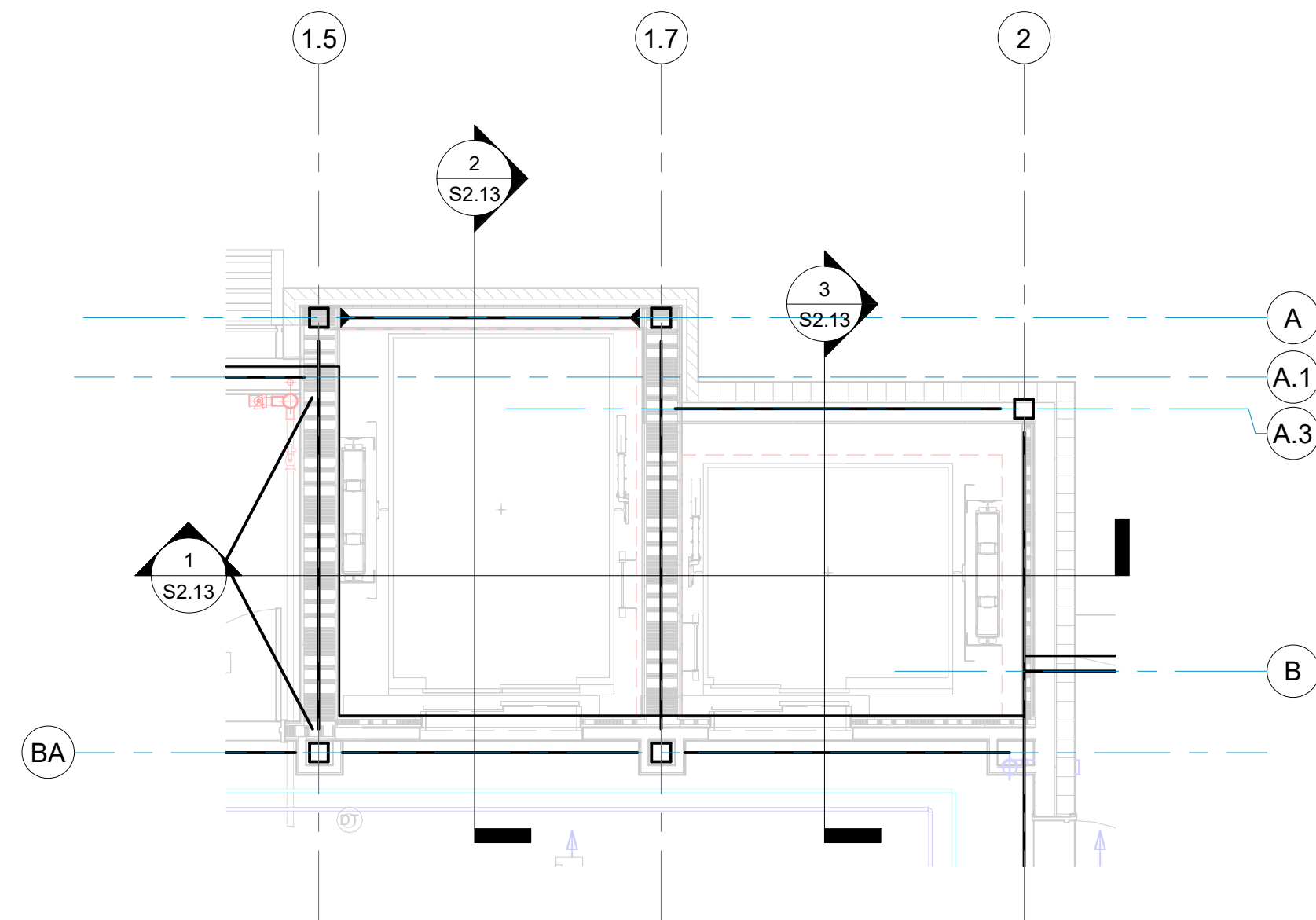
1 ELEVATOR 3&4 - PIT PLAN
S2.12 1/4" = 1'-0"



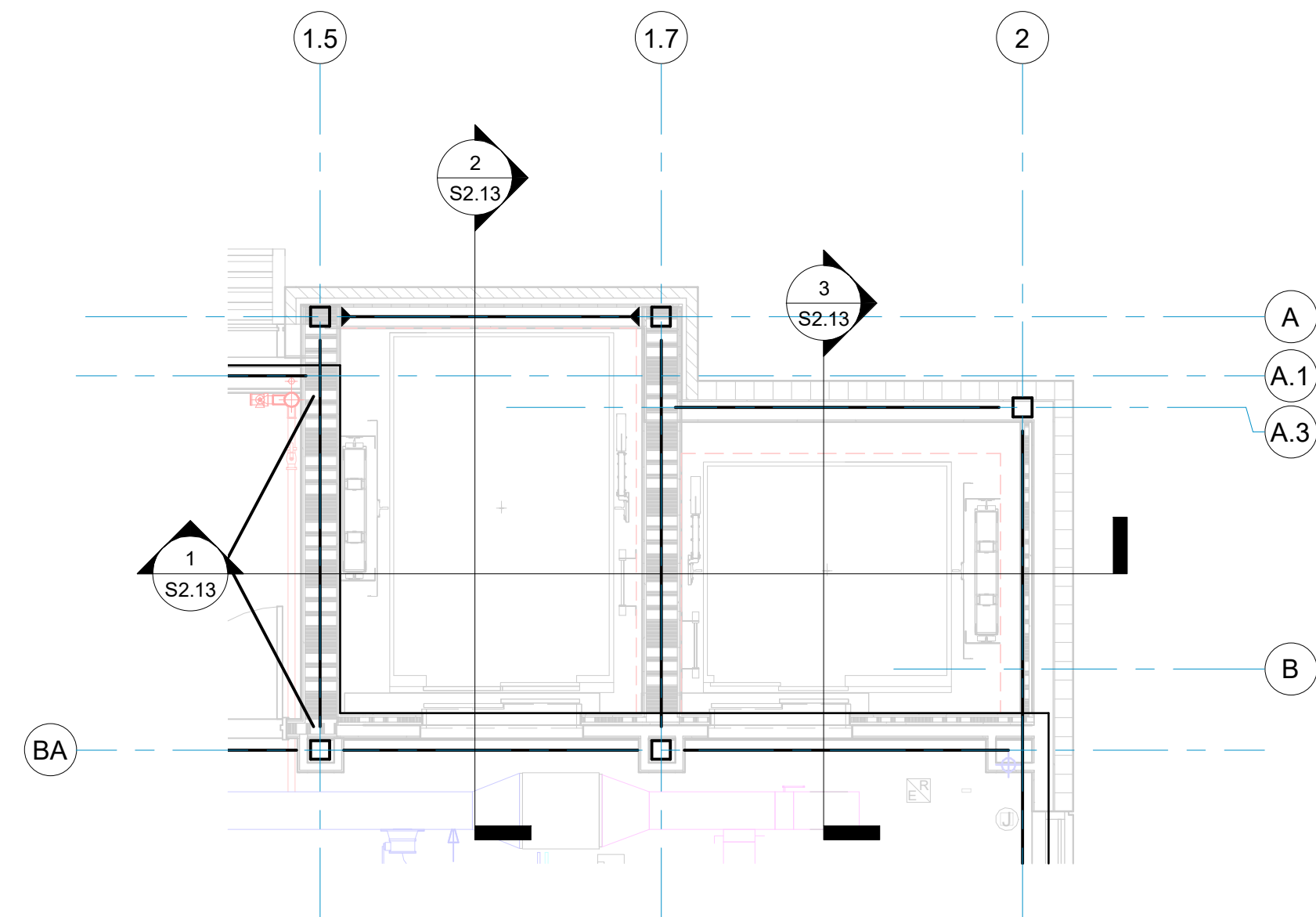
2 ELEVATOR 3&4 - LEVEL 1 PLAN
S2.12 1/4" = 1'-0"



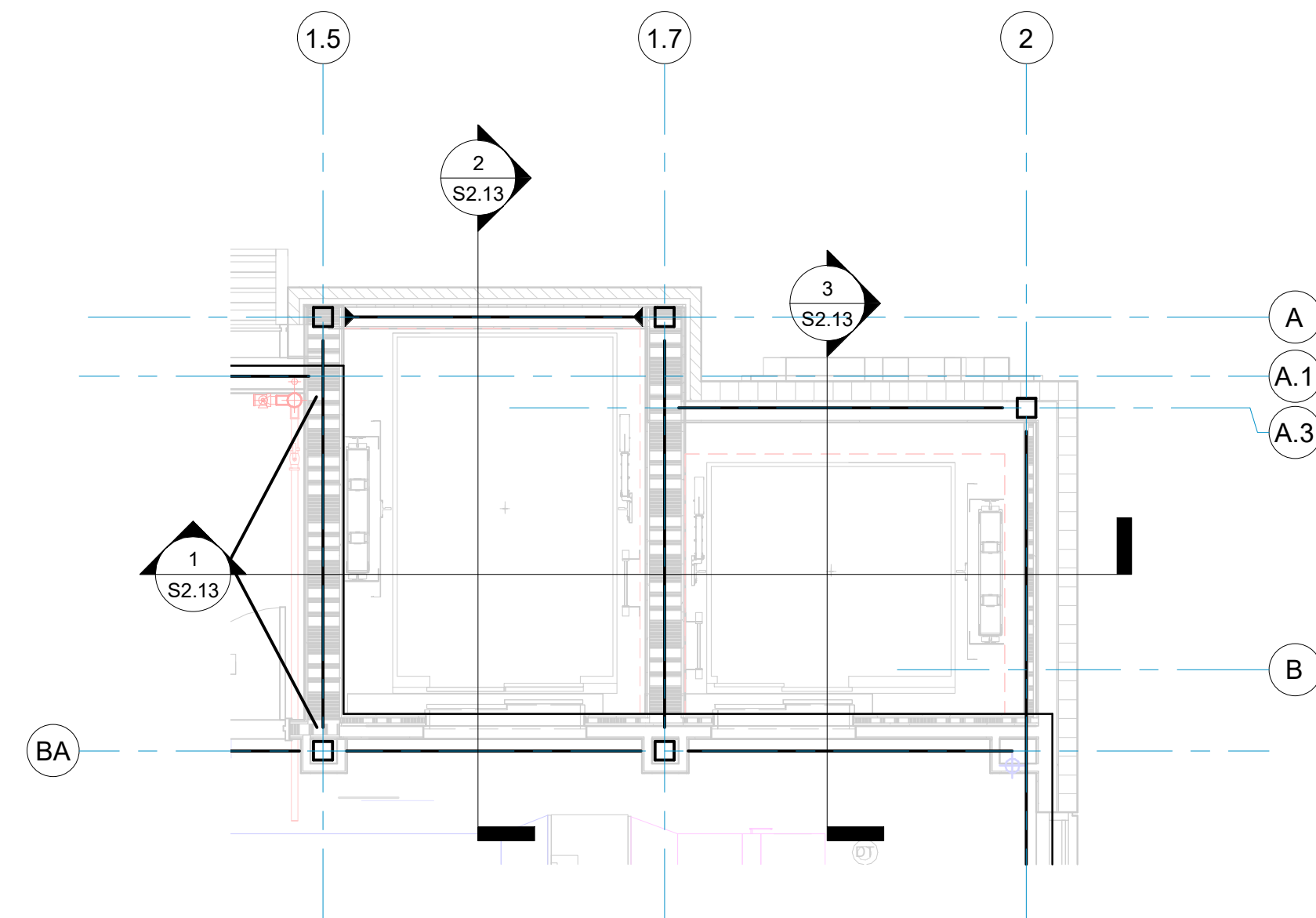
3 ELEVATOR 3&4 - LEVEL 2 PLAN
S2.12 1/4" = 1'-0"



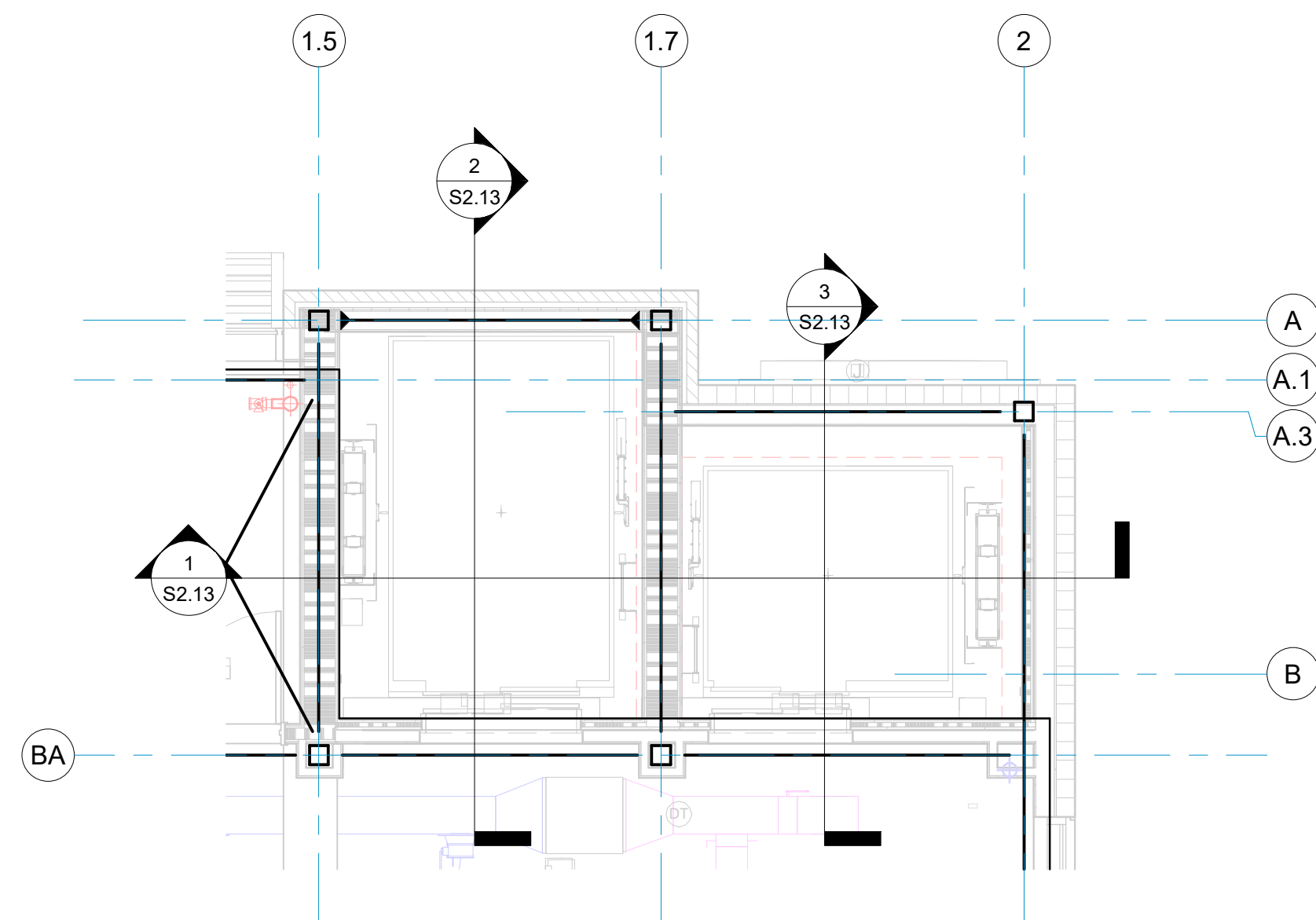
4 ELEVATOR 3&4 - LEVEL 3 PLAN
S2.12 1/4" = 1'-0"



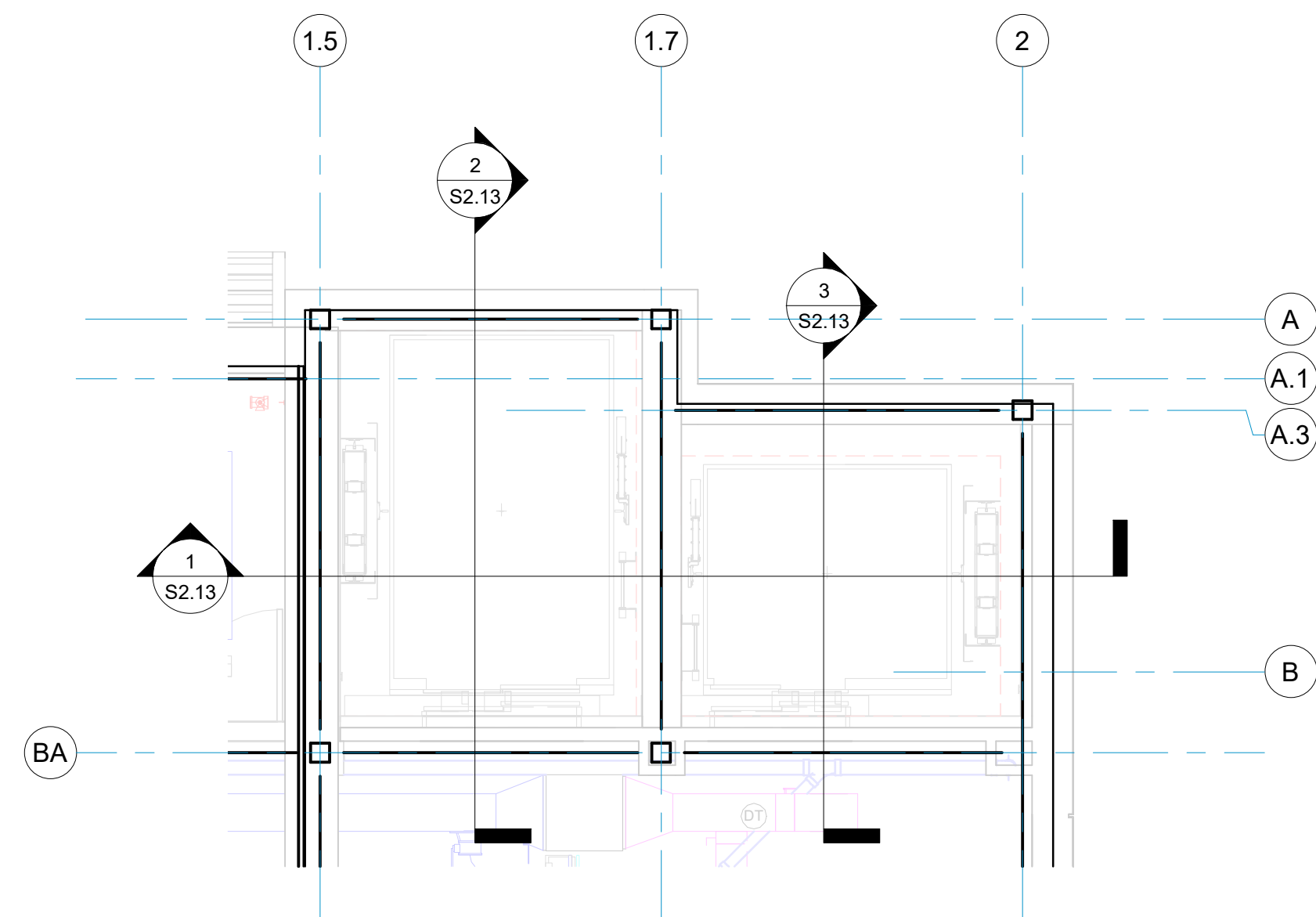
5 ELEVATOR 3&4 - LEVEL 4 PLAN
S2.12 1/4" = 1'-0"



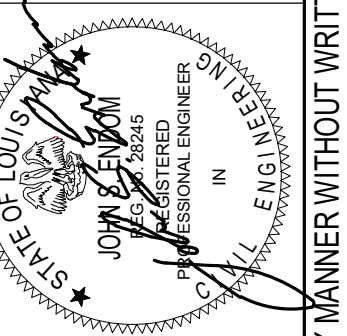
6 ELEVATOR 3&4 - LEVEL 5 PLAN
S2.12 1/4" = 1'-0"



7 ELEVATOR 3&4 - LEVEL 6 PLAN
S2.12 1/4" = 1'-0"



8 ELEVATOR 3&4 - ROOF PLAN
S2.12 1/4" = 1'-0"



DATE	NO.	ISSUE

TYLER STREET PARKING GARAGE
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1028 SOUTH TYLER STREET, COVINGTON, LA 70433
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DOCUMENTS

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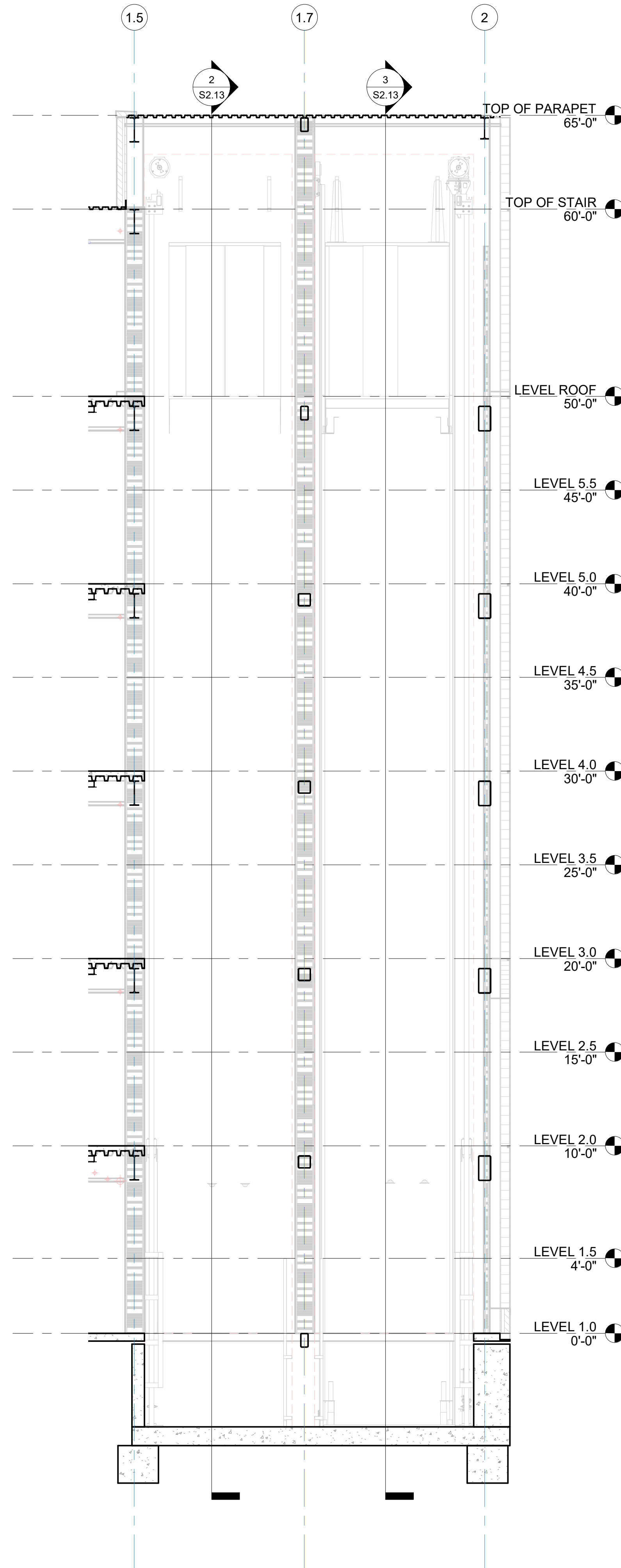
ELEVATOR 3&4 PLANS

Job No. 222058.01

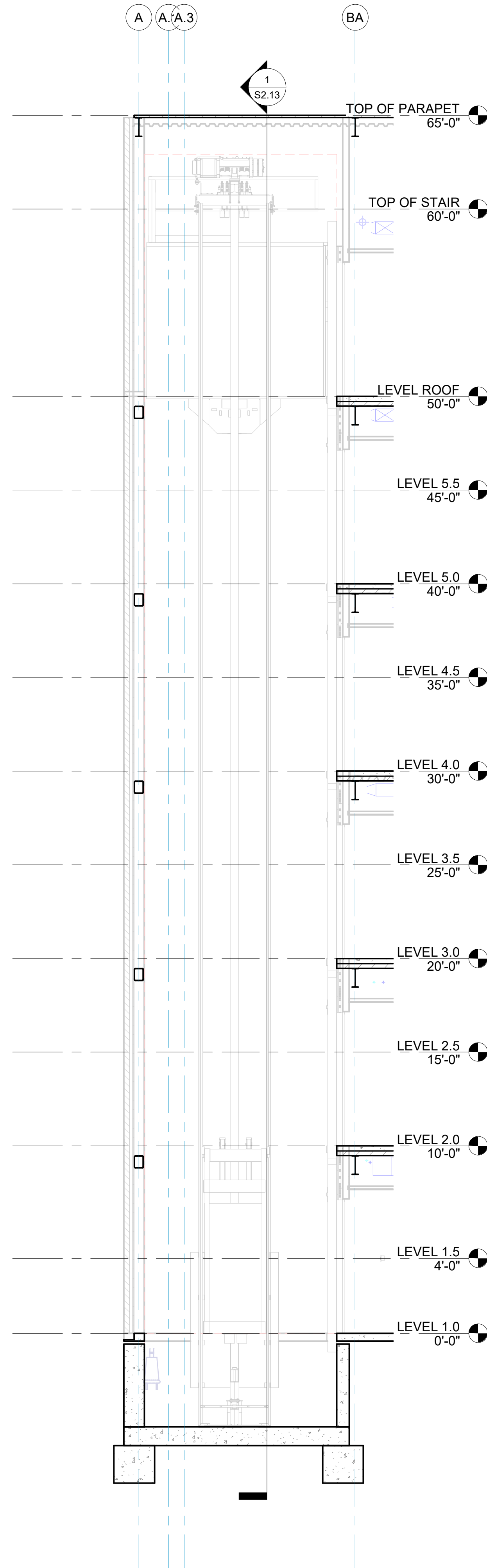
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S2.12
Sheet No.

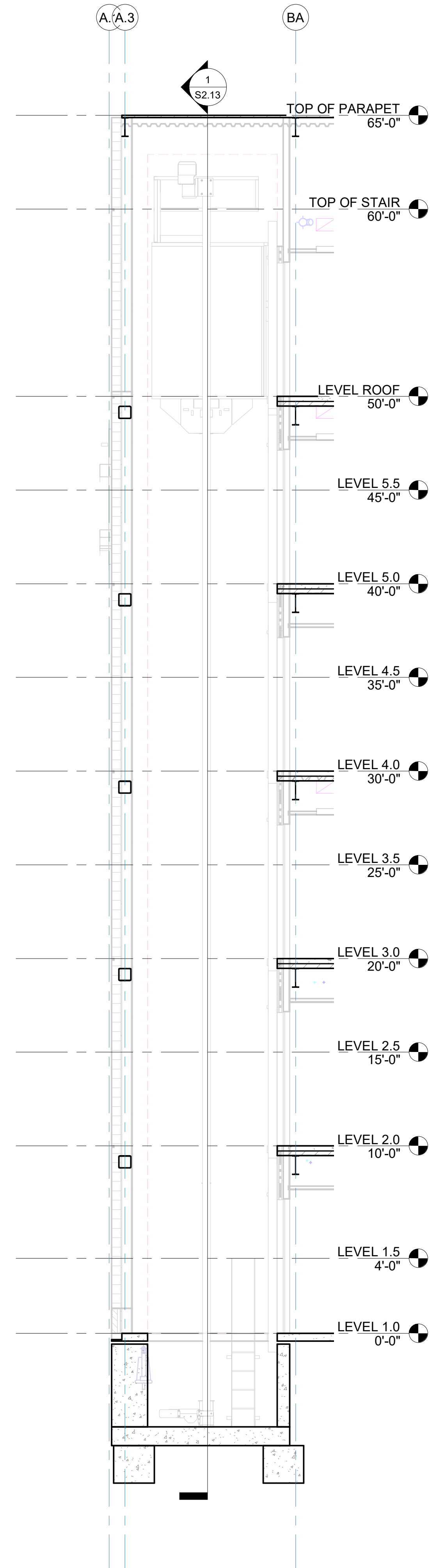
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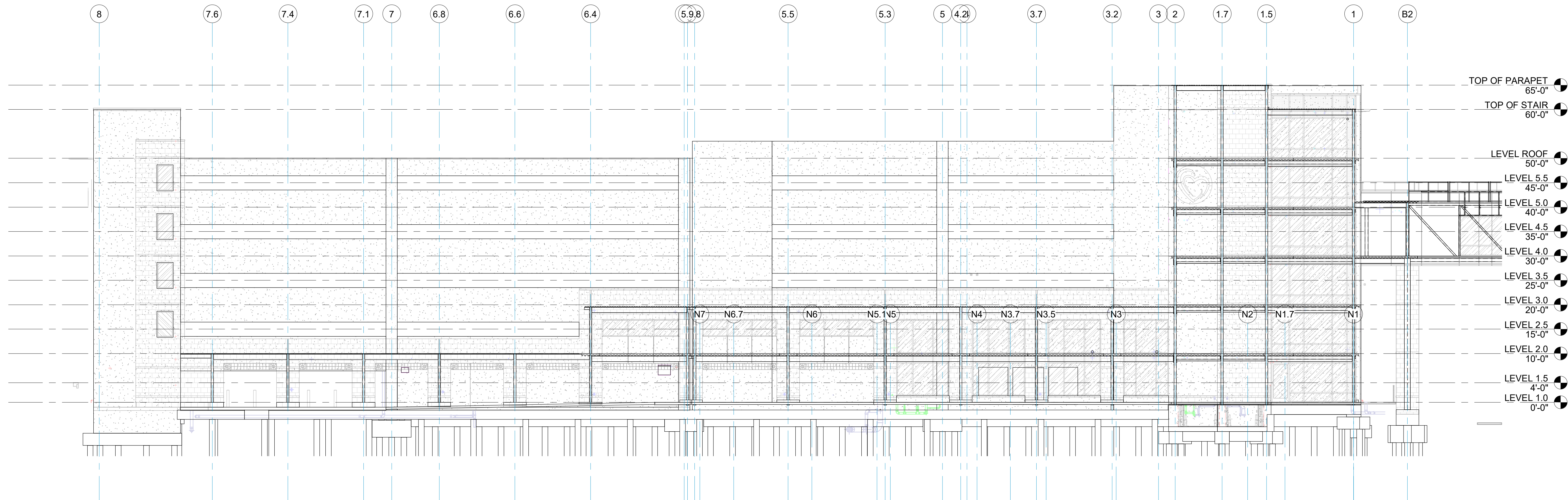
1 ELEVATOR 3&4 SECTION
S2.13 1/4" = 1'-0"



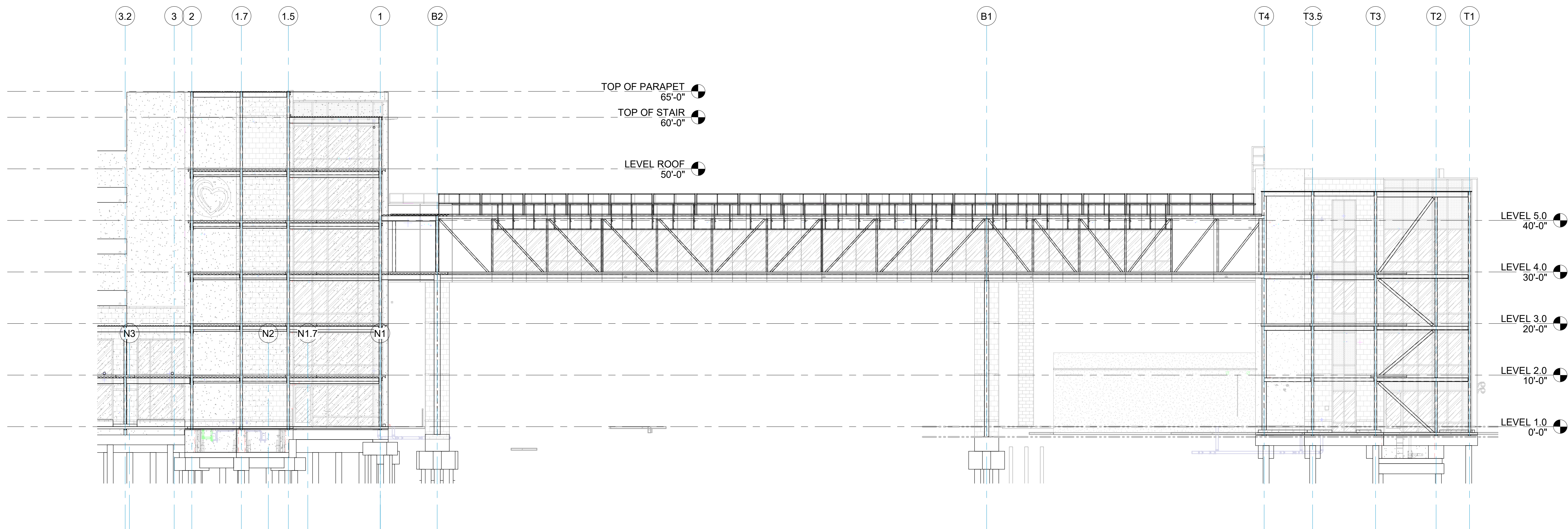
2 ELEVATOR 3 SECTION
S2.13 1/4" = 1'-0"



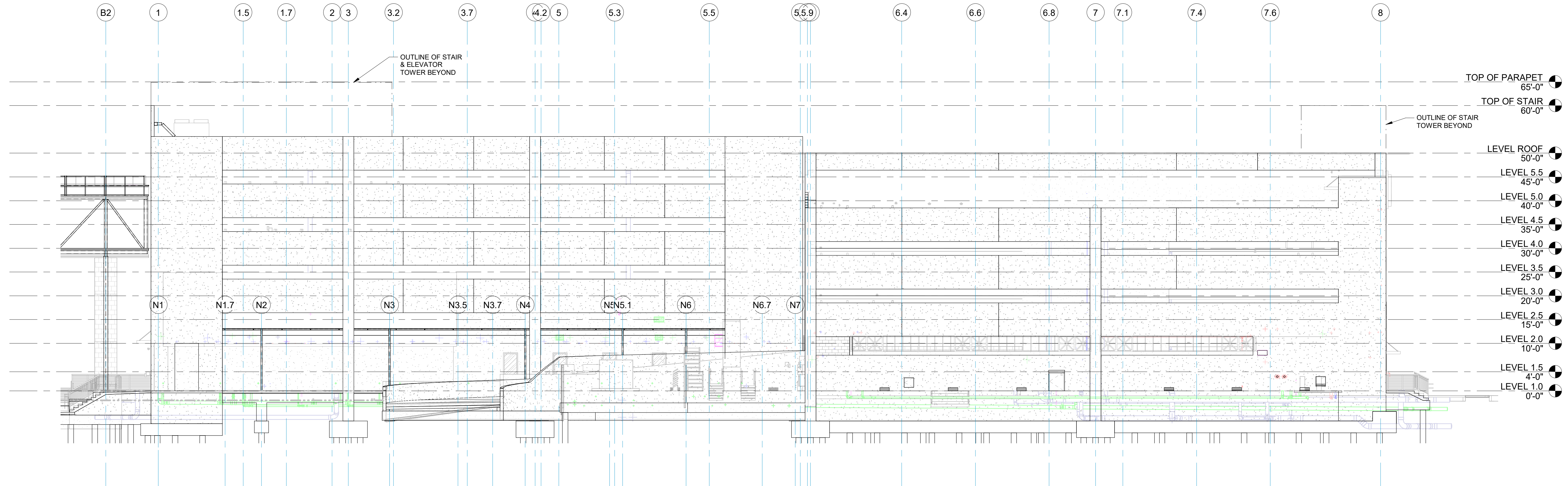
3 ELEVATOR 4 SECTION
S2.13 1/4" = 1'-0"



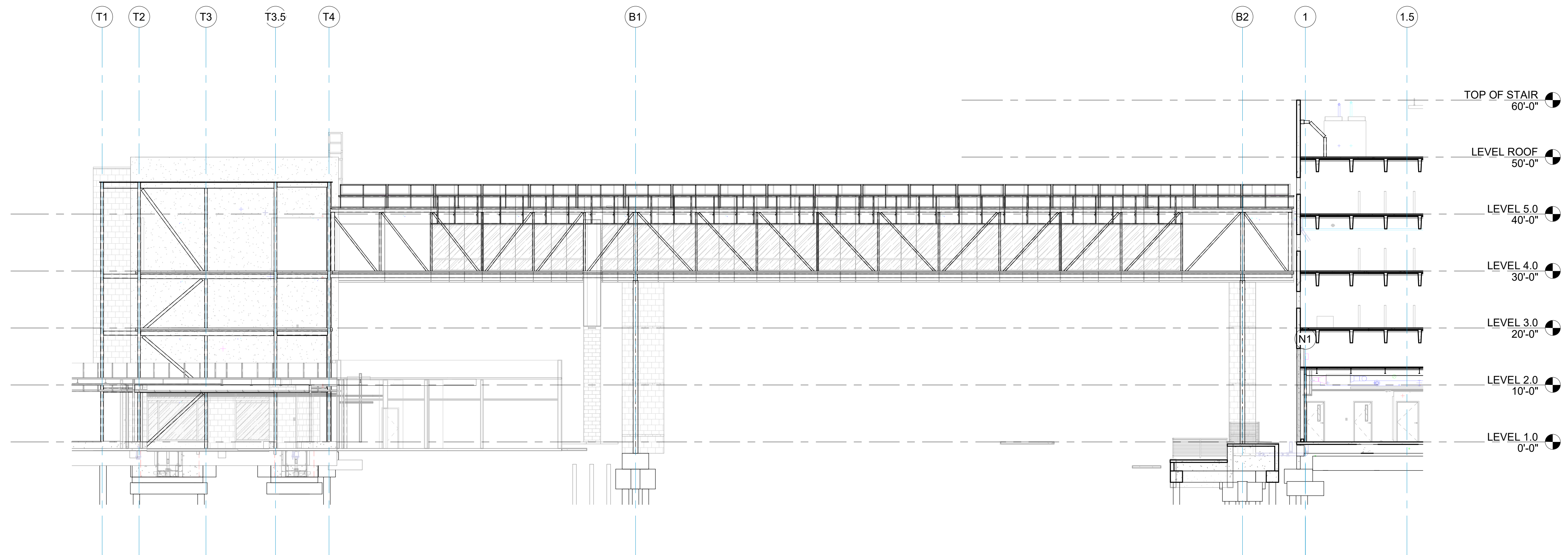
1
S3.10
ELEVATION - TYLER STREET
3/32" = 1'-0"



2
S3.10
ELEVATION - TYLER STREET Part 2
3/32" = 1'-0"



1
S3.11
ELEVATION - HARRISON STREET
3/32" = 1'-0"



2
S3.11
ELEVATION - HARRISON STREET Part 2
3/32" = 1'-0"

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NO. 10000

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619 WEST 12TH AVENUE, COVINGTON, LA 70433

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NORTHLAKE SURGICAL ASSOCIATES

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EXTERIOR ELEVATIONS

Job No.

222058.01

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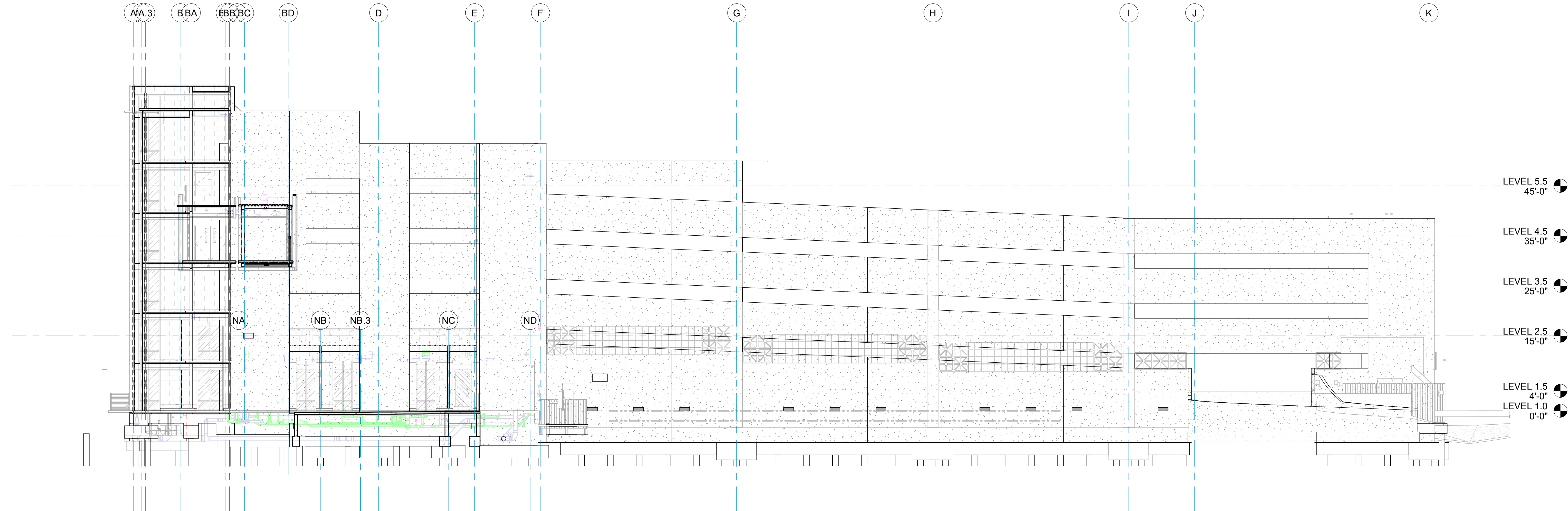
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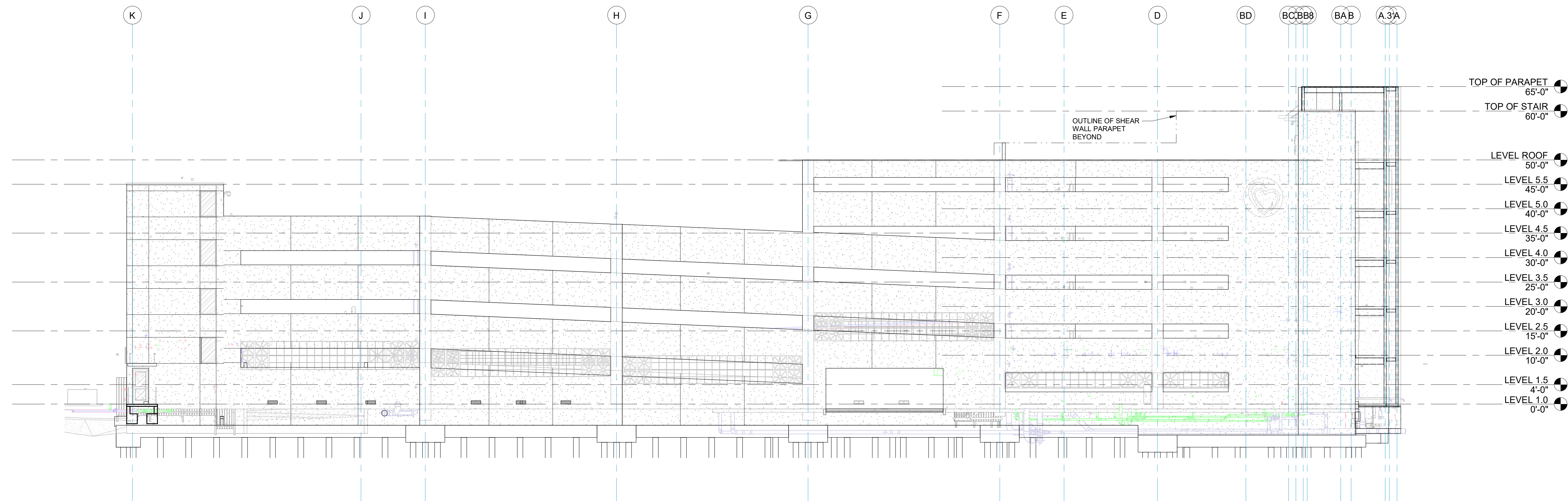
S3.11

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1
S3.12
ELEVATION - 11TH AVENUE
3/32" = 1'-0"



2
S3.12
ELEVATION - 12TH AVENUE
3/32" = 1'-0"

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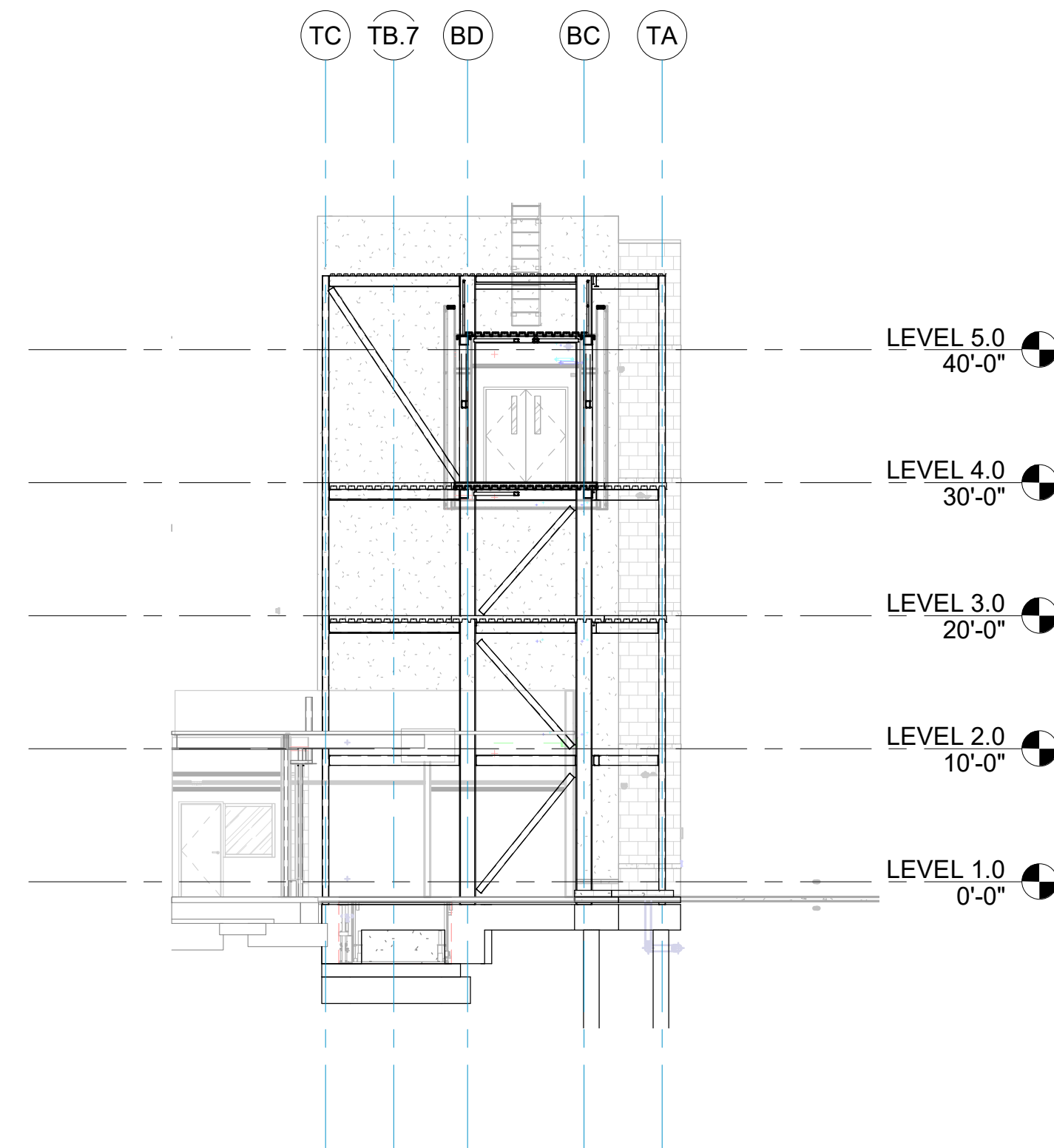
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EXTERIOR ELEVATIONS

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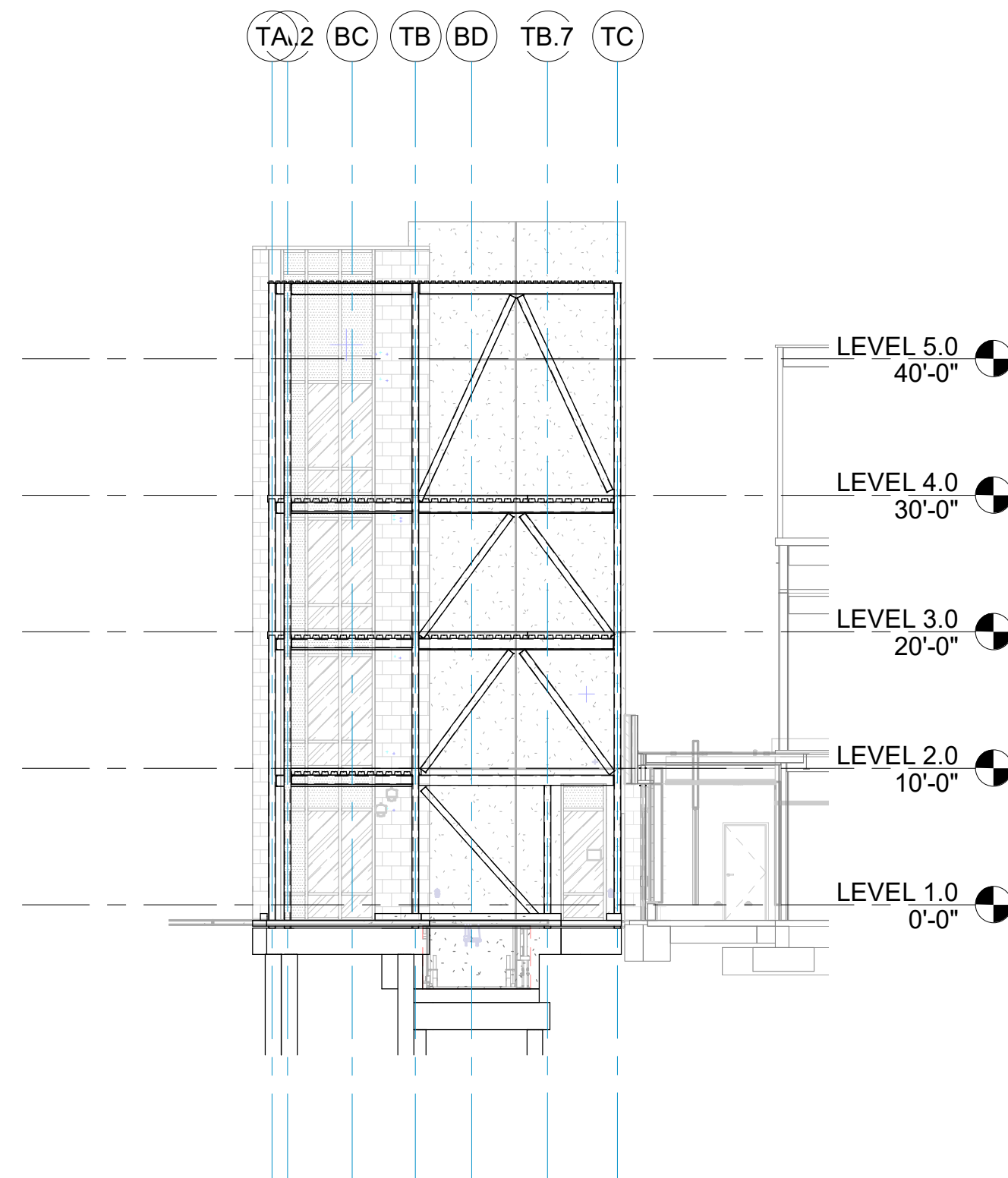
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1
S3.13

ELEVATION - HOSPITAL TOWER EAST

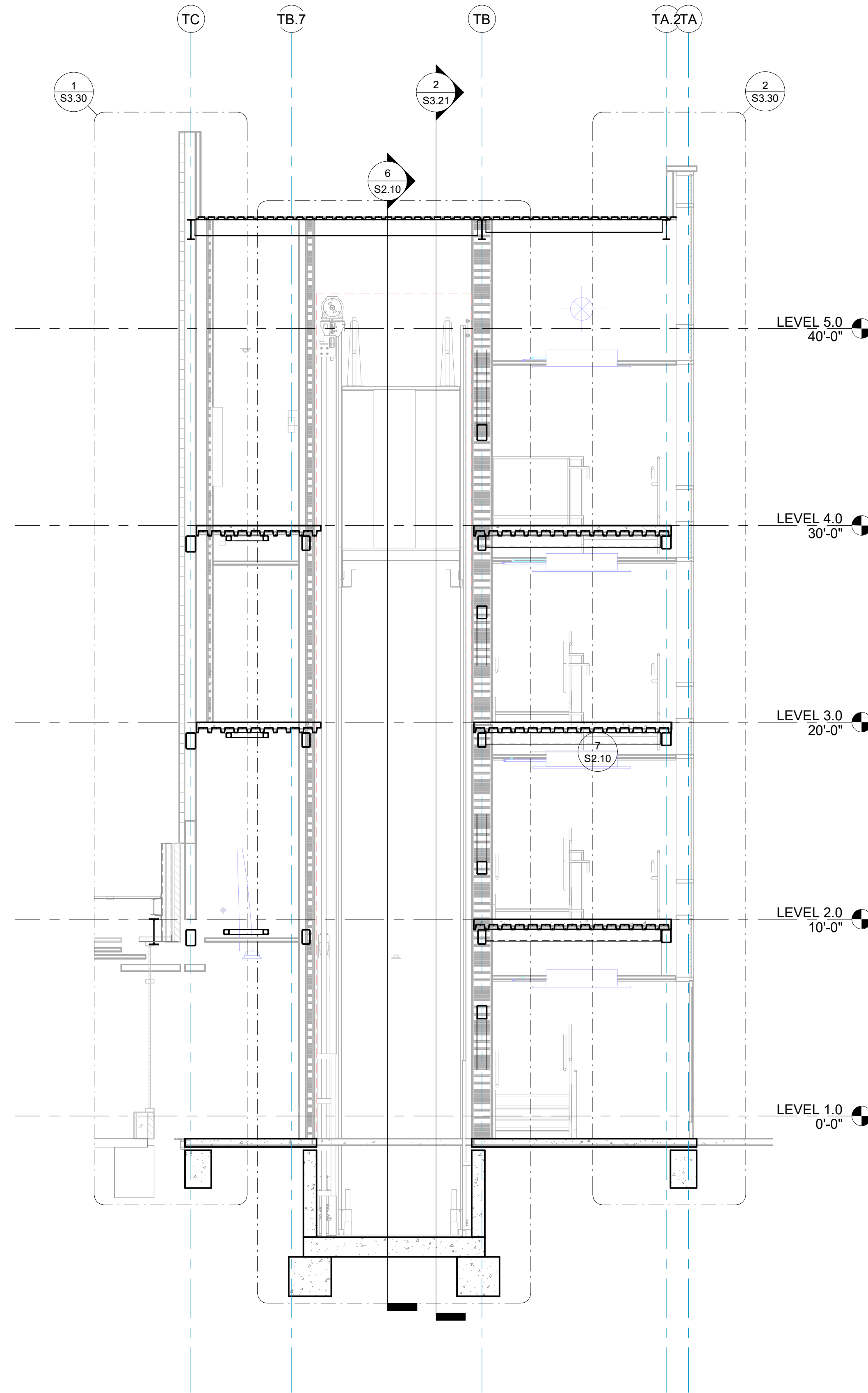
3/32" = 1'-0"



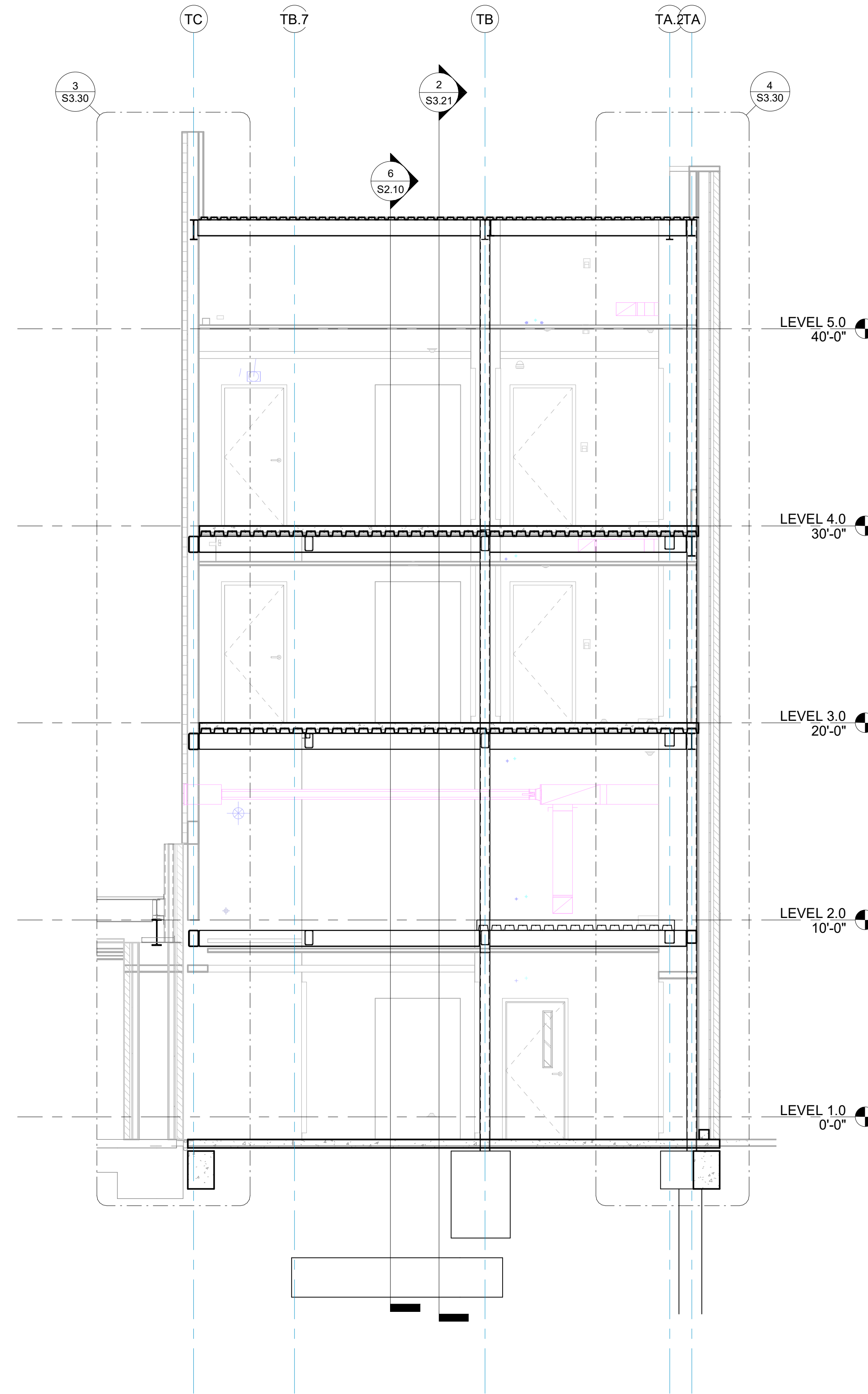
2
S3.13

ELEVATION - HOSPITAL TOWER WEST

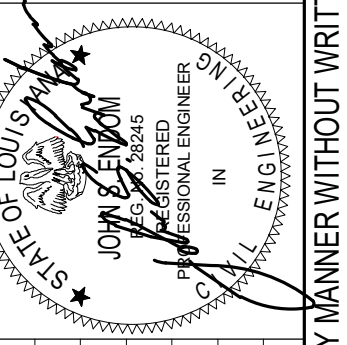
3/32" = 1'-0"



1 BUILDING SECTION
1/4" = 1'-0"



2 BUILDING SECTION
1/4" = 1'-0"



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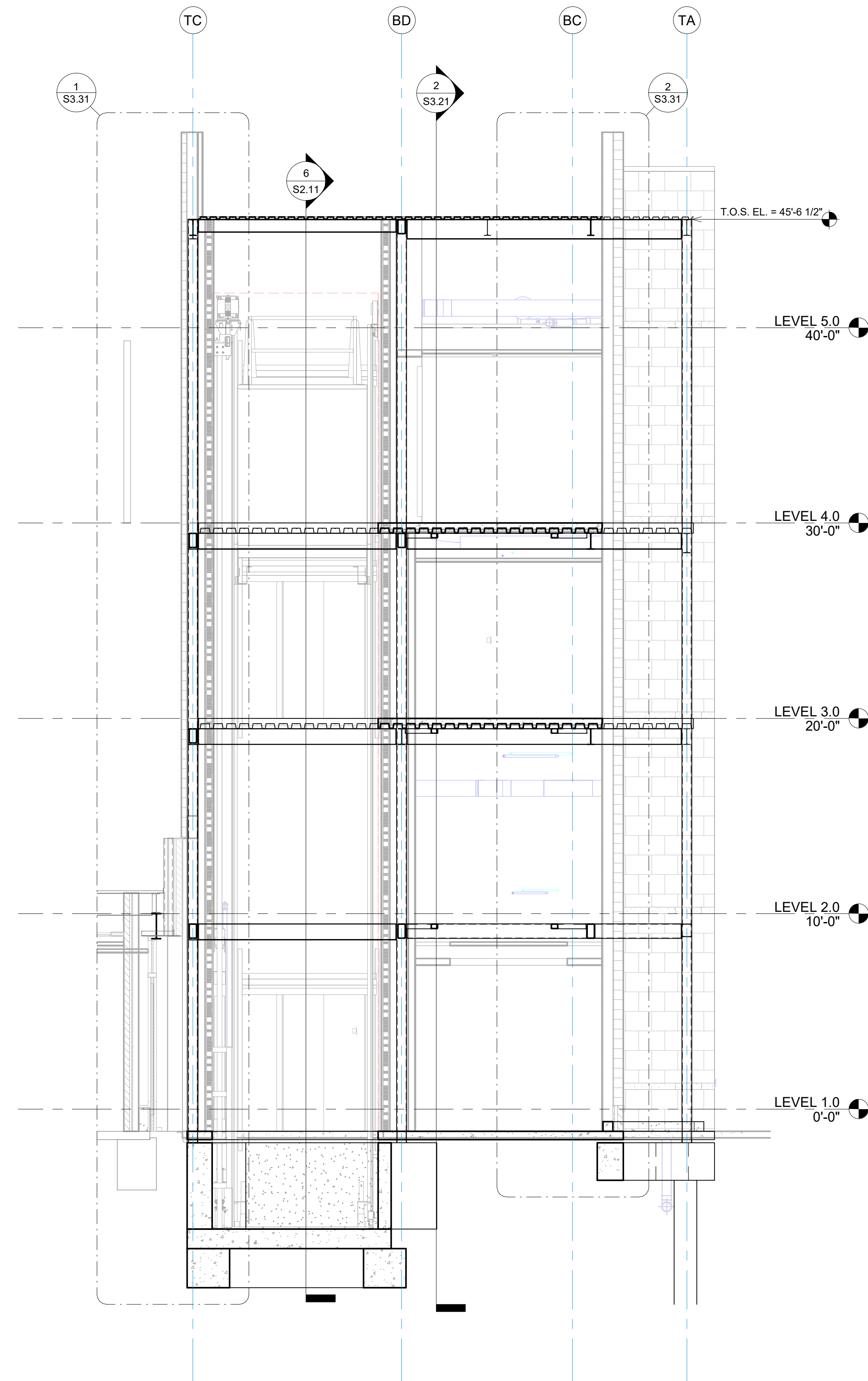
BUILDING SECTIONS

Job No. 222058.01

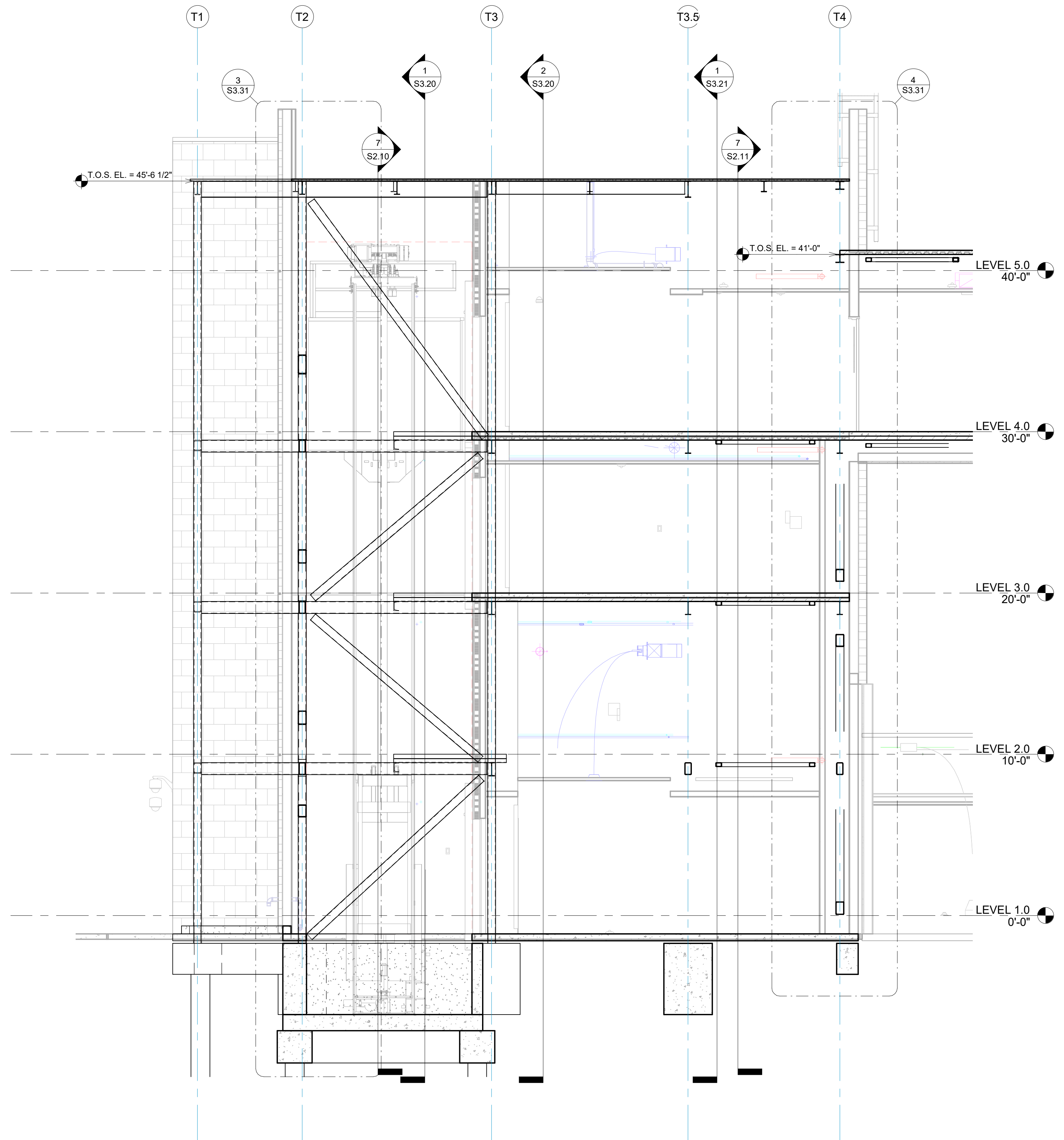
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1 BUILDING SECTION
1/4" = 1'-0"



2 BUILDING SECTION
1/4" = 1'-0"



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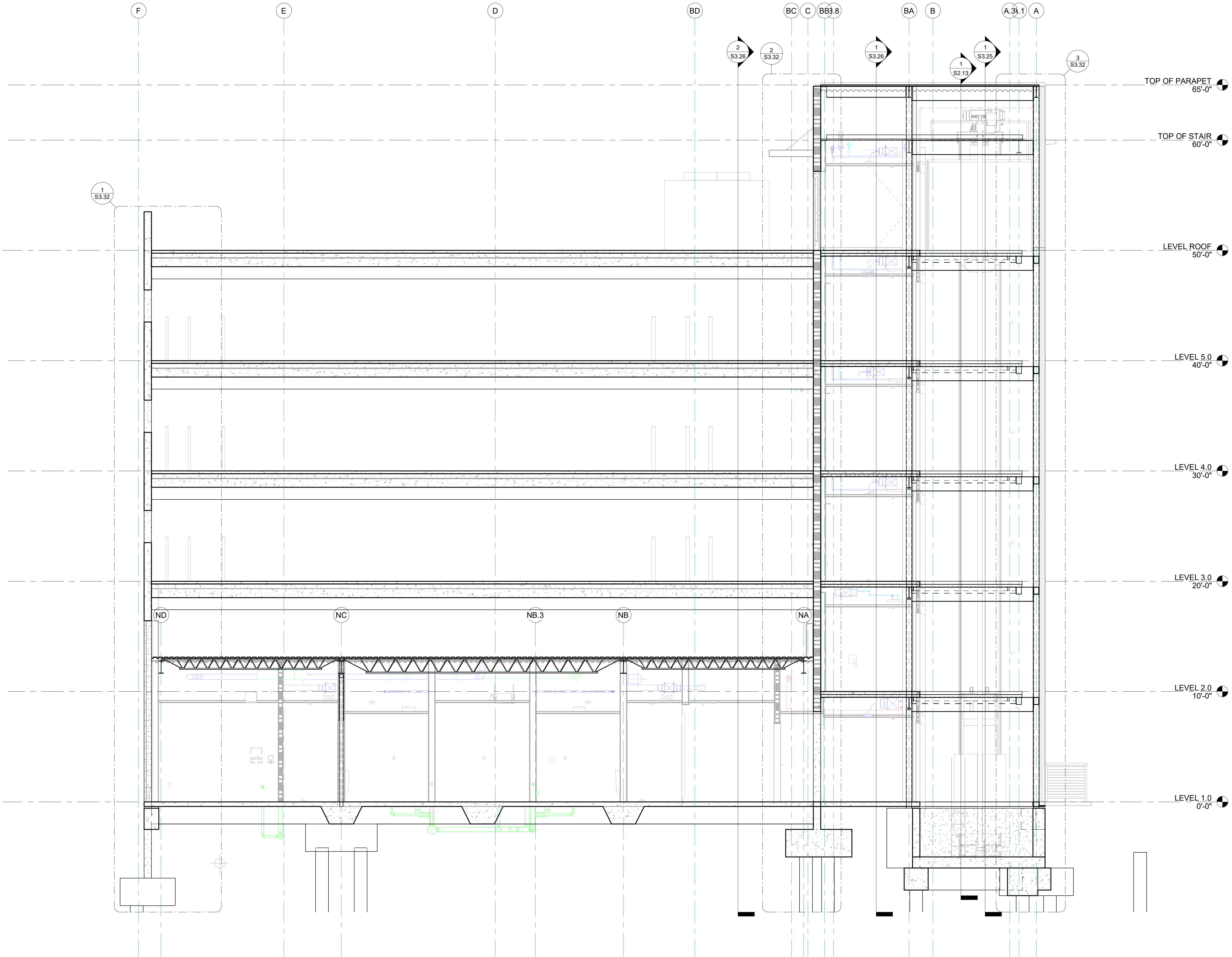
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1
S3.22
BUILDING SECTION
1/4" = 1'-0"

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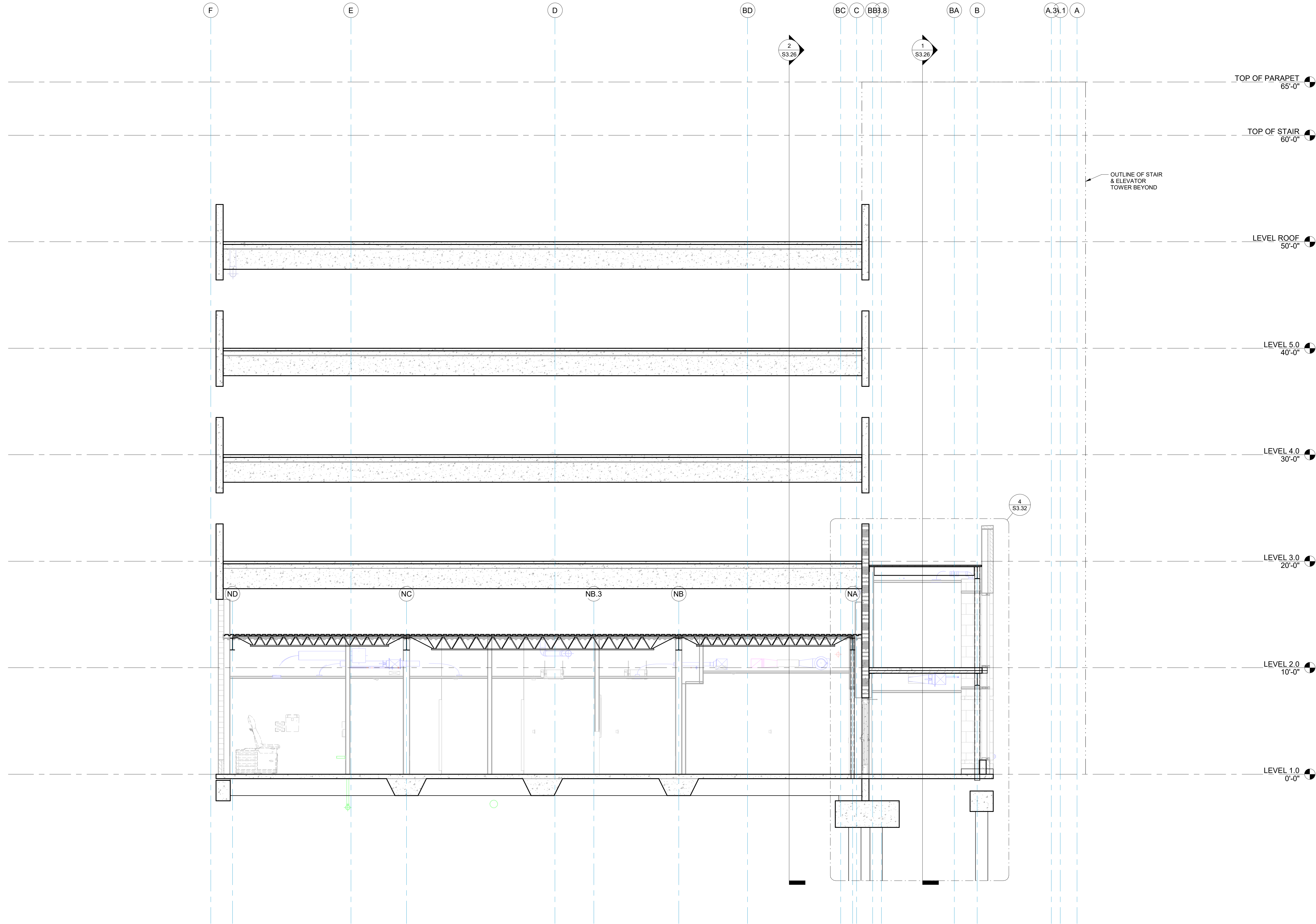
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1 BUILDING SECTION
S3.23 1/4" = 1'-0"

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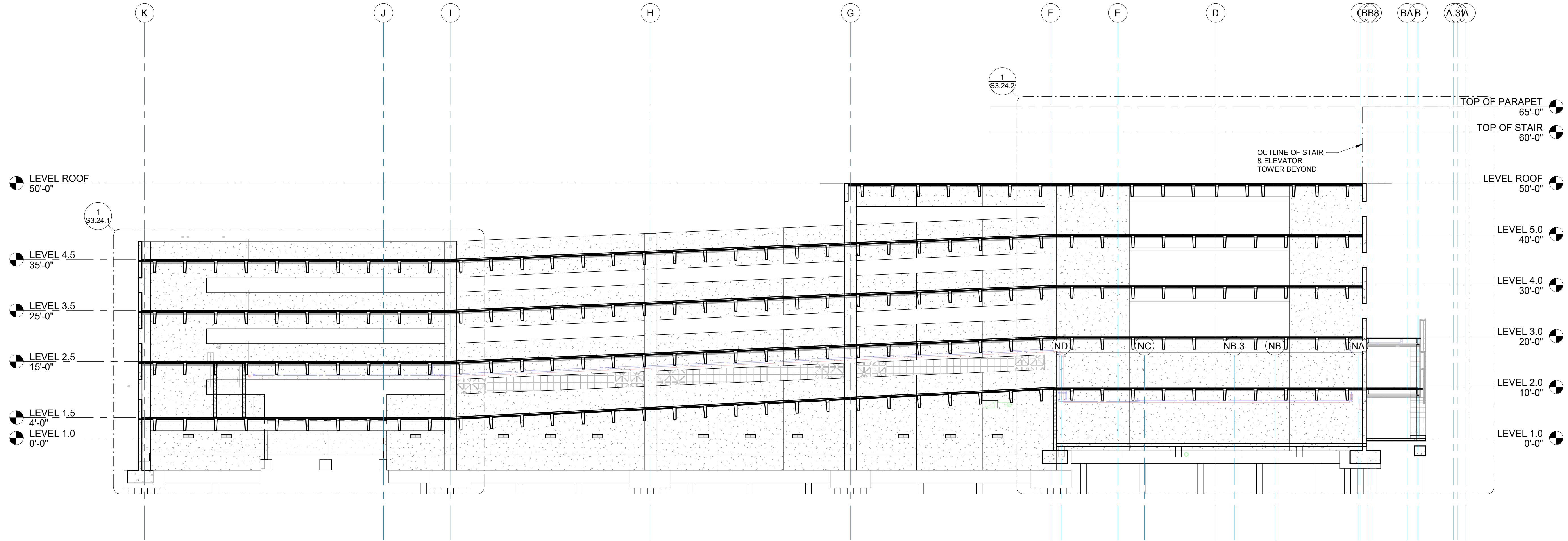
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Job No. 222058.01

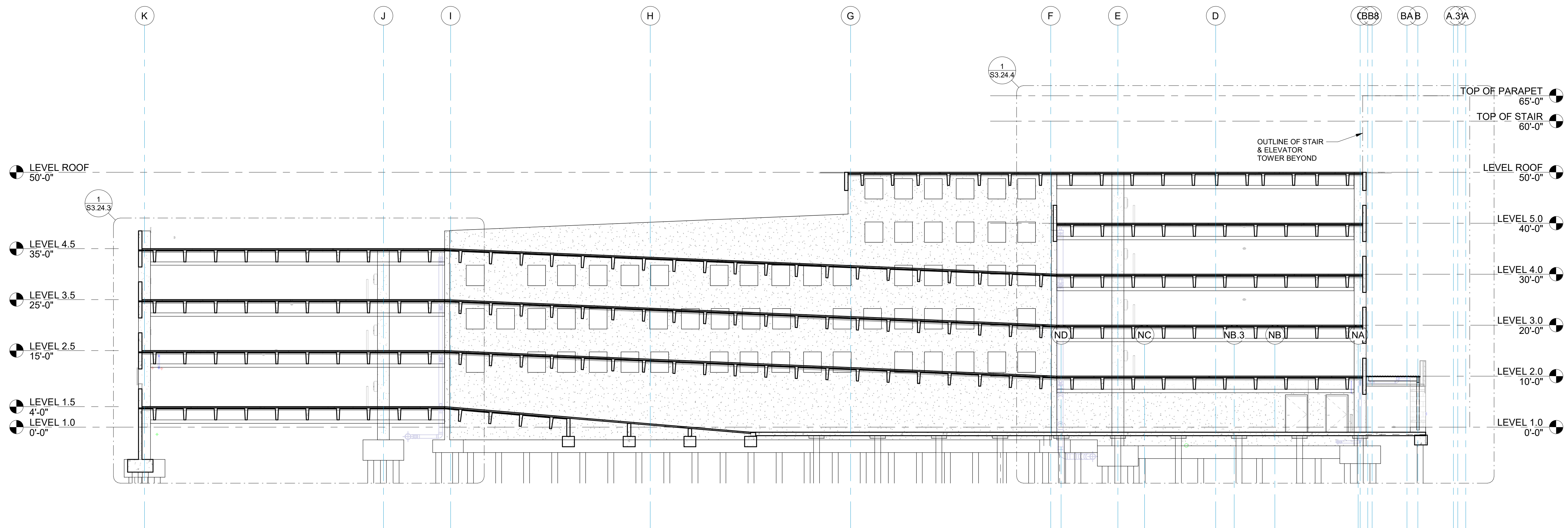
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1 BUILDING SECTION
S3.24 3/32" = 1'-0"



2 BUILDING SECTION
S3.24 3/32" = 1'-0"

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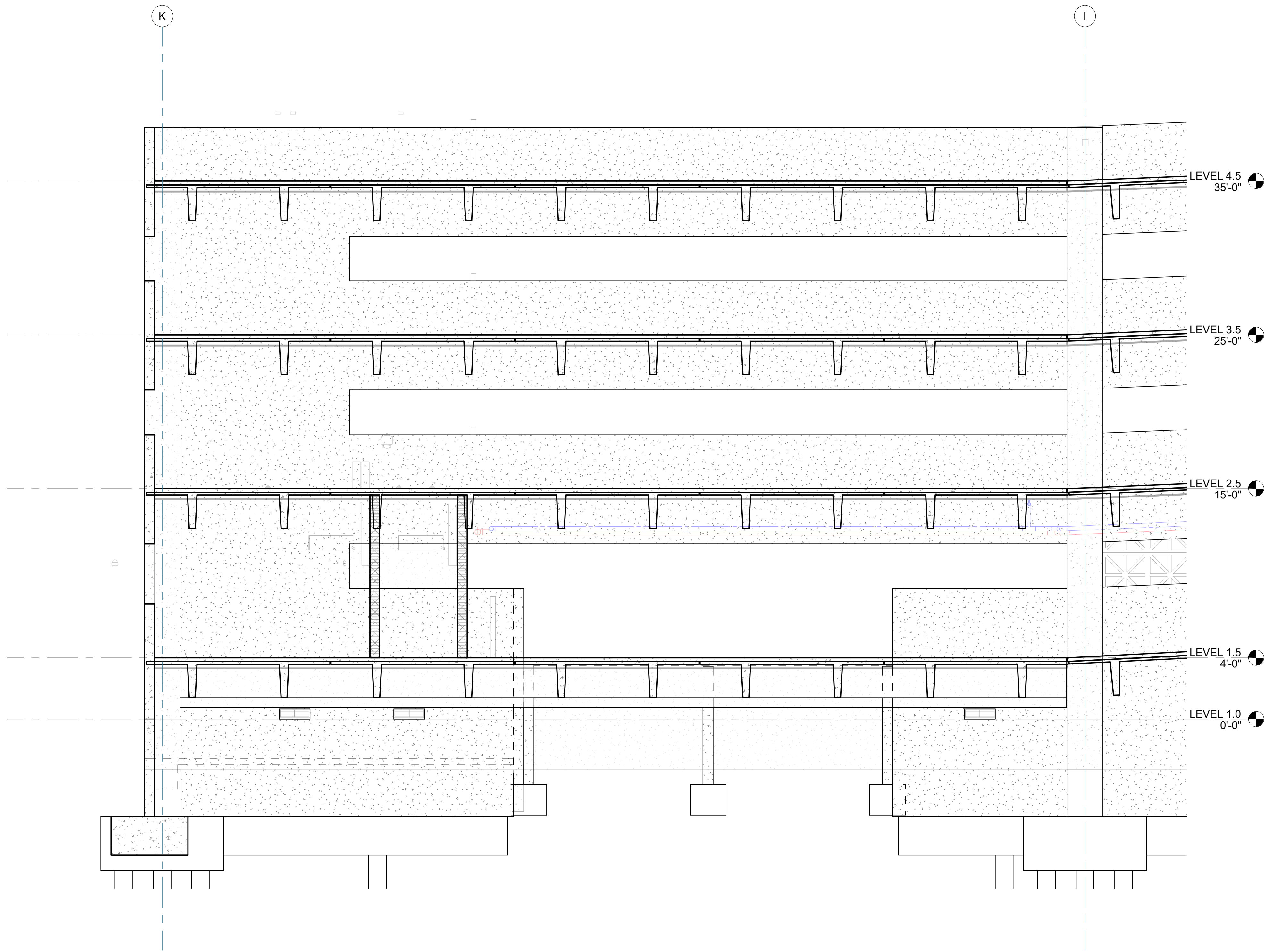
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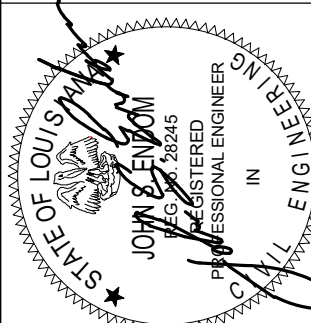
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1 BUILDING SECTION
S3.24.1 1/4" = 1'-0"



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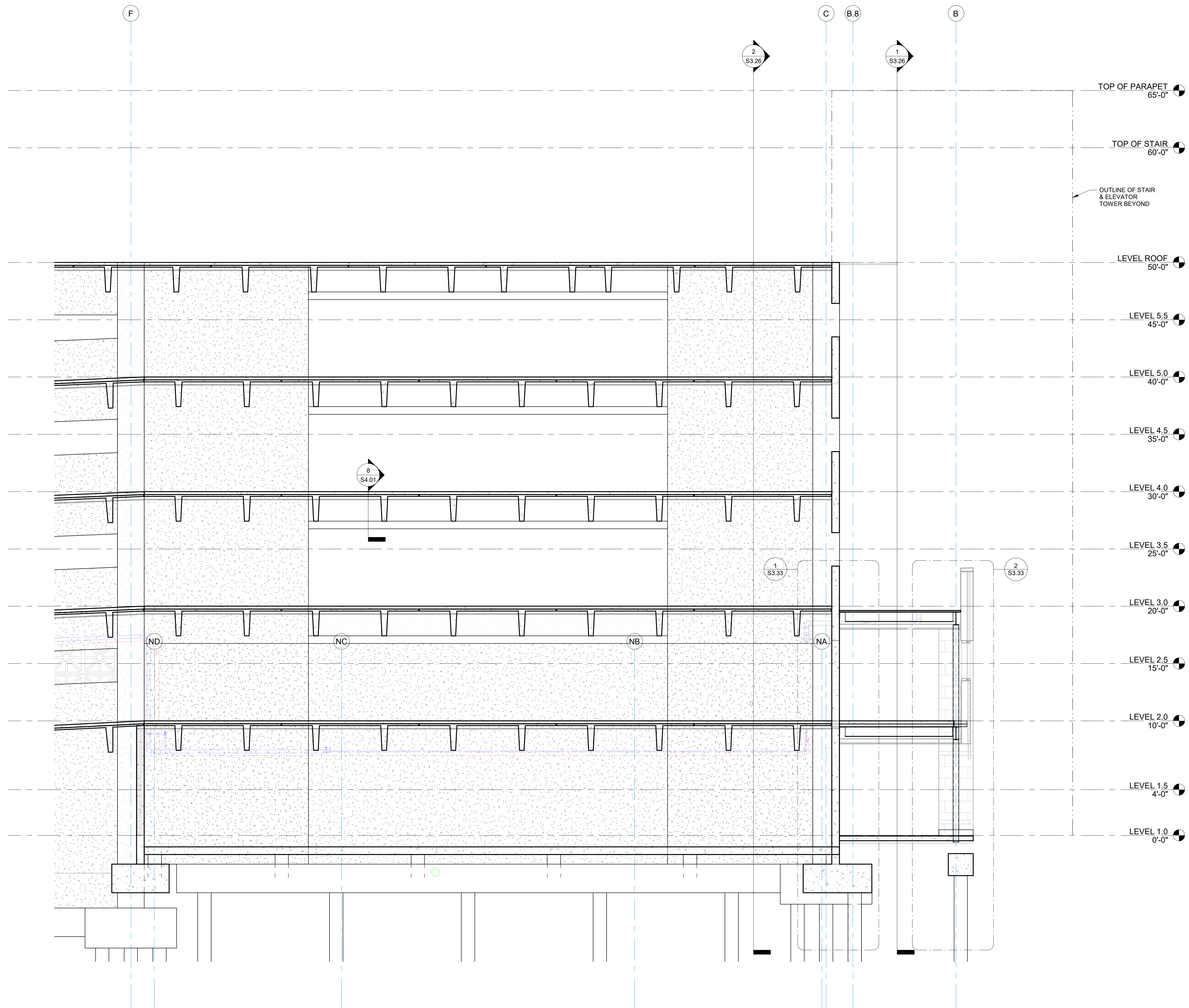
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1 BUILDING SECTION
S3.24.2 1/4" = 1'-0"

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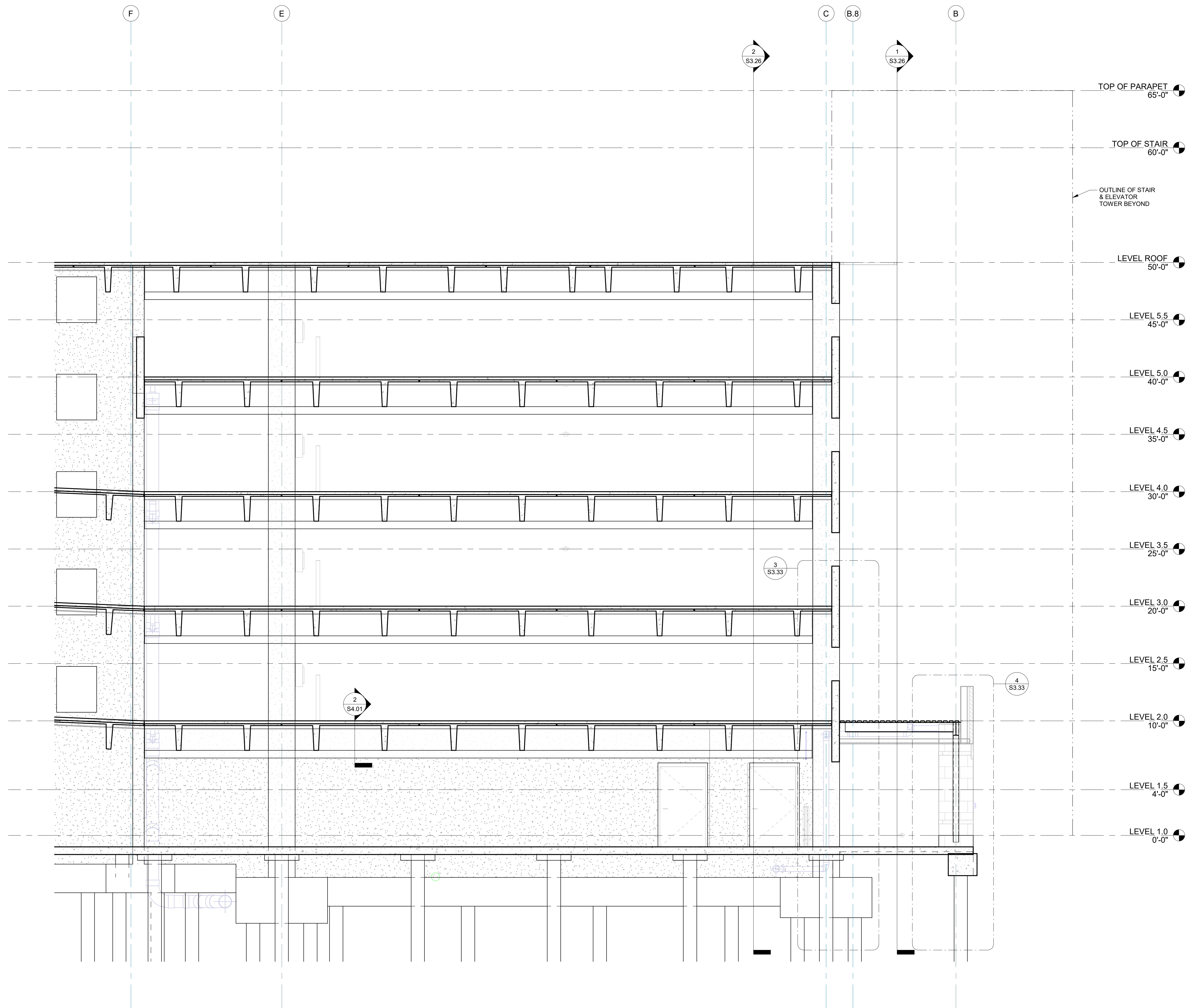
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1
S3.24.4
BUILDING SECTION
1/4" = 1'-0"

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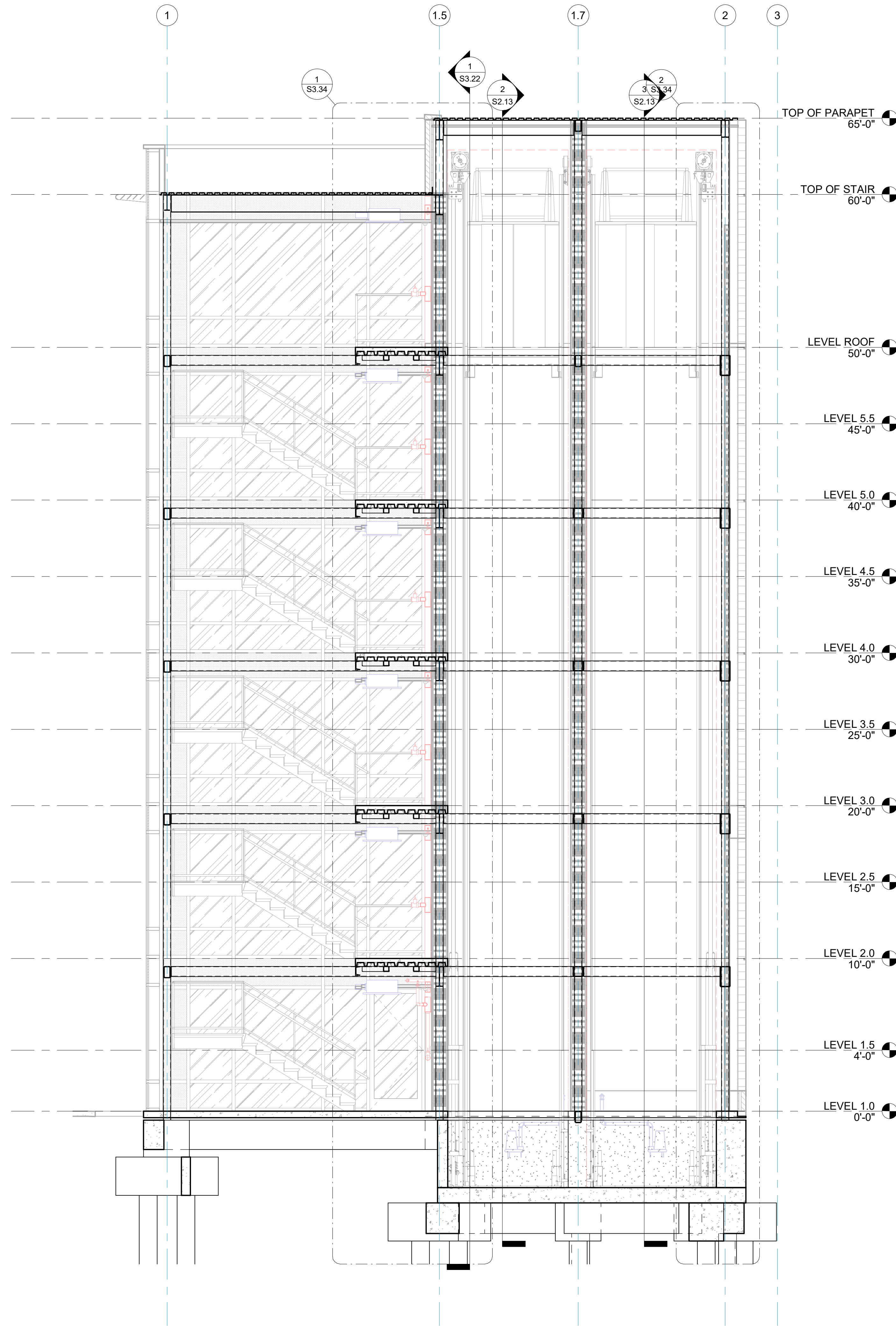
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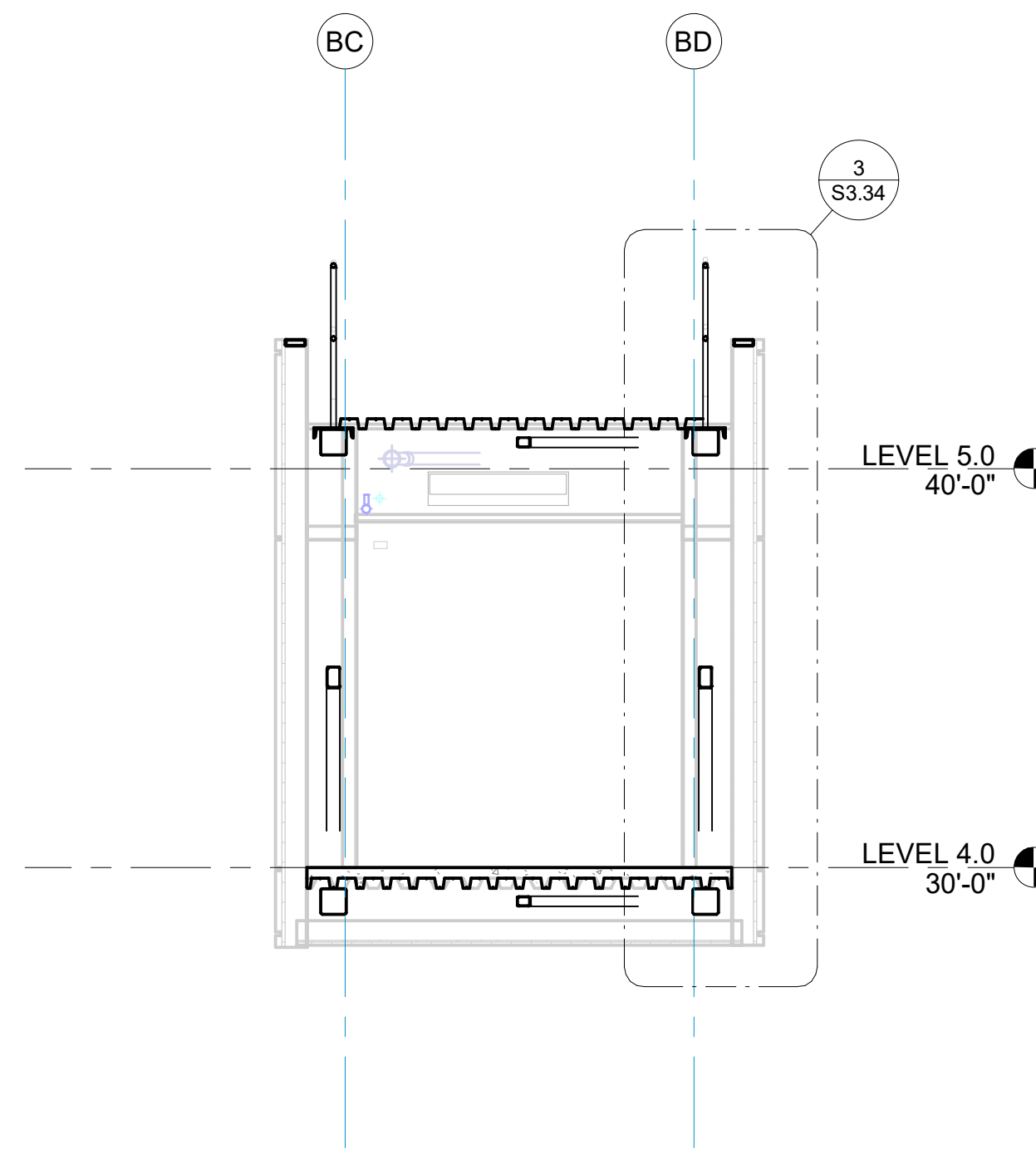
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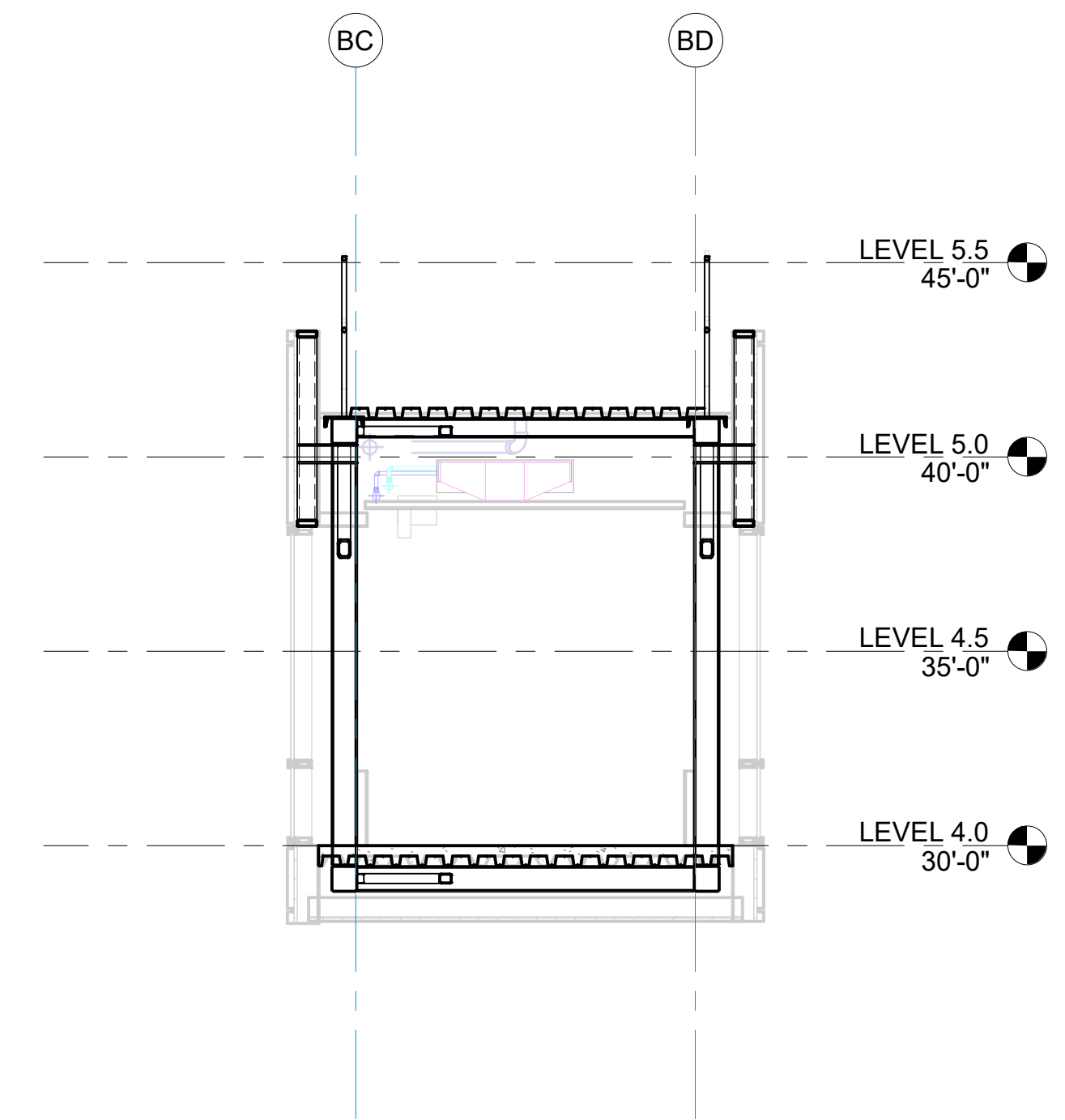
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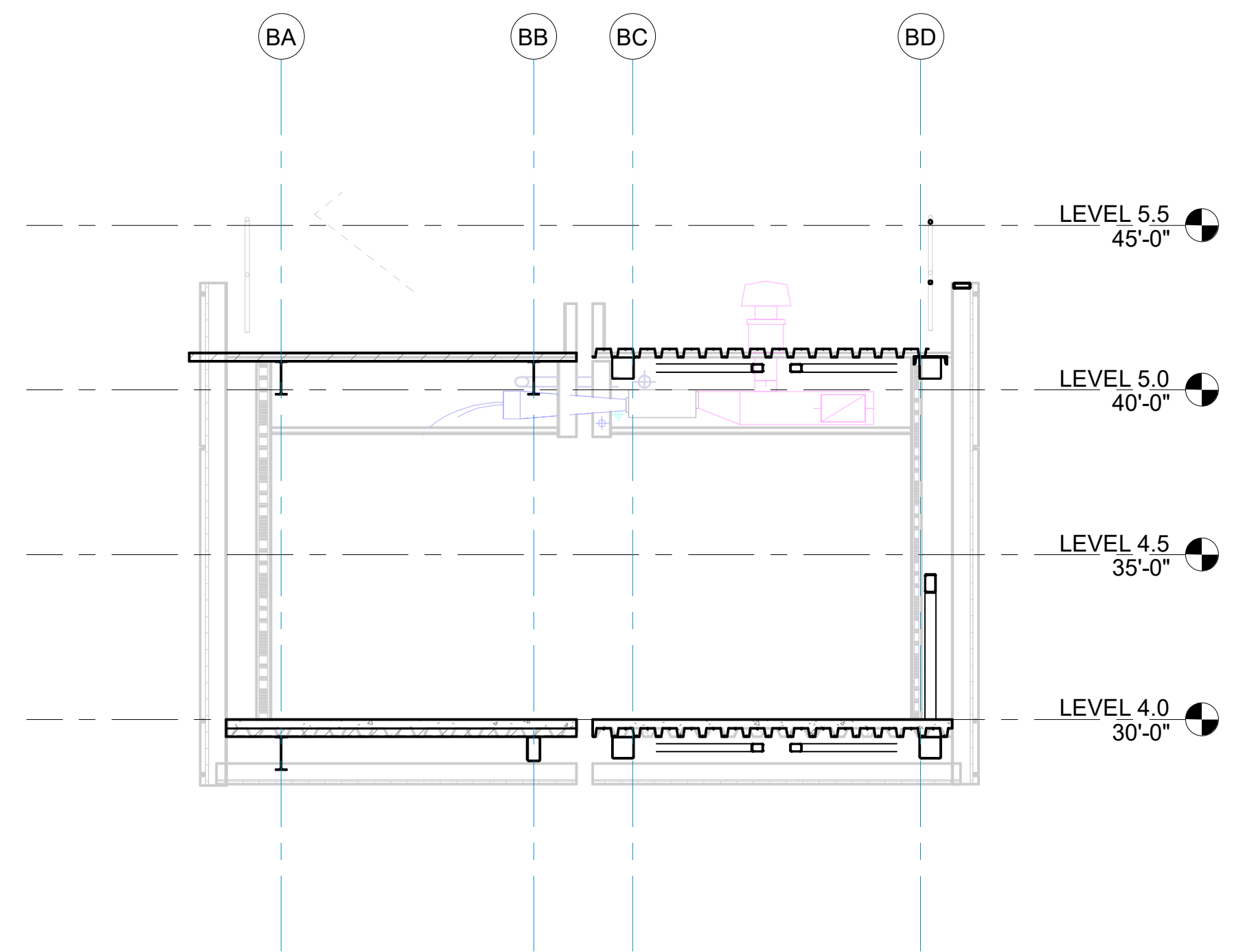
1
S3.25
BUILDING SECTION
1/4" = 1'-0"



2
S3.25
BUILDING SECTION
1/4" = 1'-0"



3
S3.25
BUILDING SECTION
1/4" = 1'-0"



4
S3.25
BUILDING SECTION
1/4" = 1'-0"

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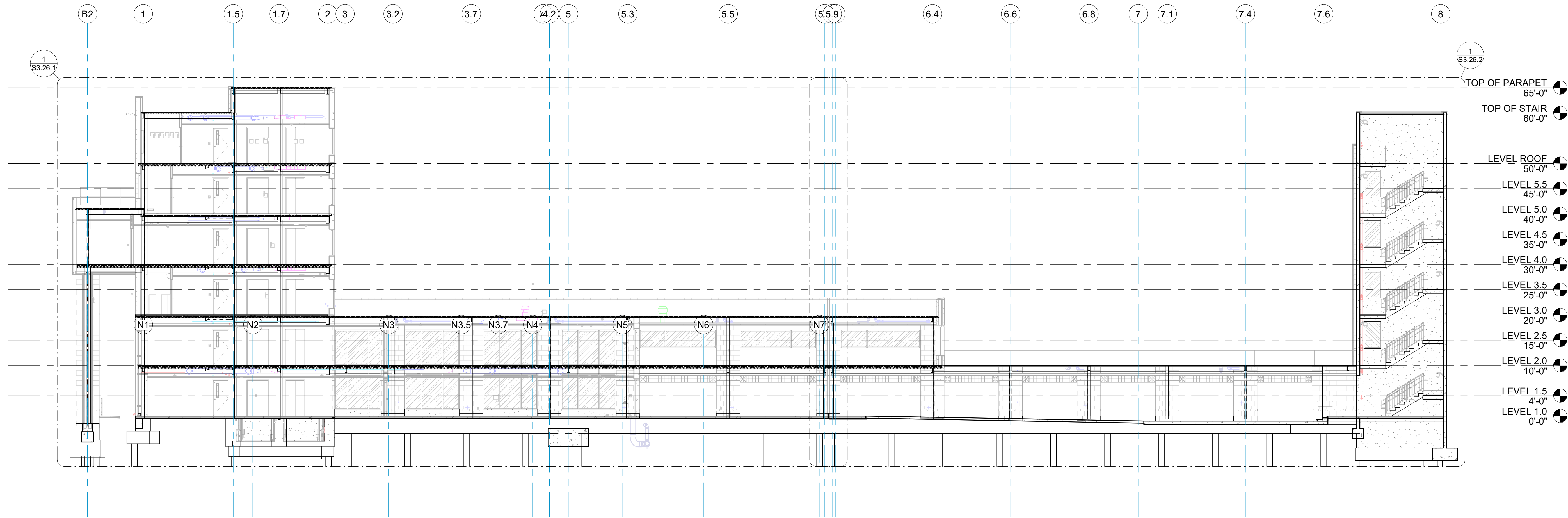
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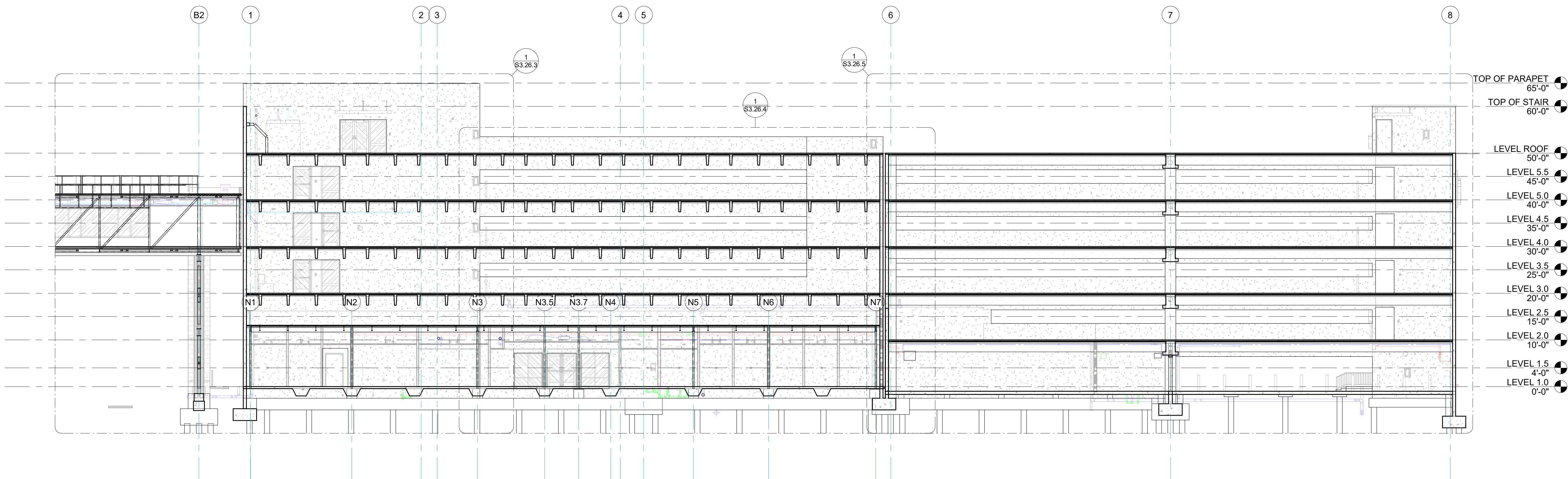
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S3.26 3/32" = 1'-0"



2 BUILDING SECTION
S3.26 3/32" = 1'-0"

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1028 SOUTH TYLER STREET, COVINGTON, LA 70433
ST. TAMMANY PARISH HOSPITAL

CONSTRUCTION DOCUMENTS

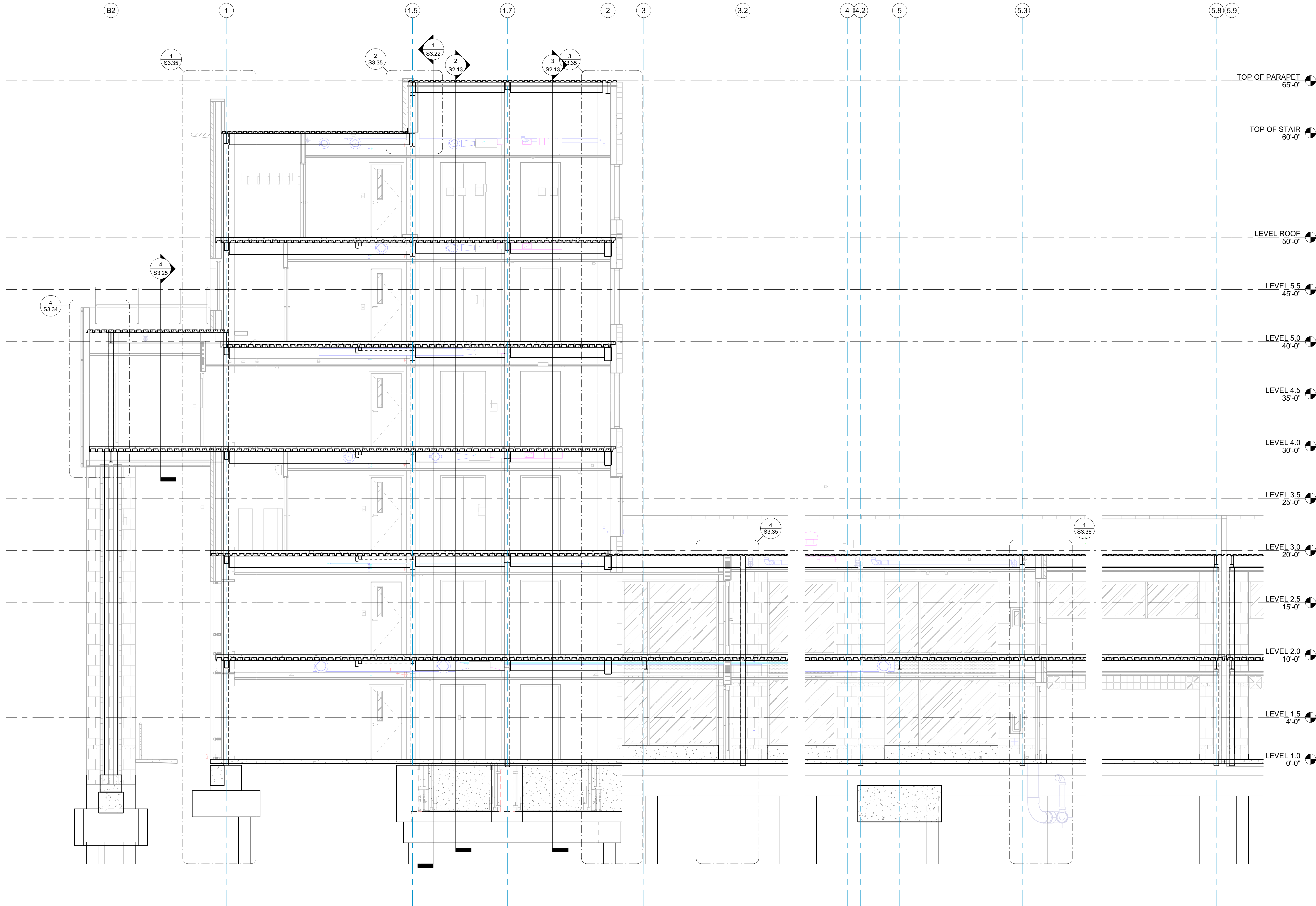
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BUILDING SECTIONS

Job No. 222058.01
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S3.26
Sheet No.

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1 BUILDING SECTION
S3.26.1 1/4" = 1'-0"

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NO. 10000
EXPIRATION DATE 12/31/2025

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619 WEST 12TH AVENUE, COVINGTON, LA 70433
AND
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NORTHLAKE SURGICAL ASSOCIATES
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5.8 5.9

1
S3.24

6.4

6.6

6.8

7

7.4

2
S3.24

7.6

8

TOP OF PARAPET
65'-0"

TOP OF STAIR
60'-0"

LEVEL ROOF
50'-0"

LEVEL 5.5
45'-0"

LEVEL 5.0
40'-0"

LEVEL 4.5
35'-0"

LEVEL 4.0
30'-0"

LEVEL 3.5
25'-0"

LEVEL 3.0
20'-0"

LEVEL 2.5
15'-0"

LEVEL 2.0
10'-0"

LEVEL 1.5
4'-0"

LEVEL 1.0
0'-0"

4
S3.36

2
S3.36

3
S3.36

1

BUILDING SECTION

S3.26.2
1/4" = 1'-0"

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CONSTRUCTION
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BUILDING SECTIONS

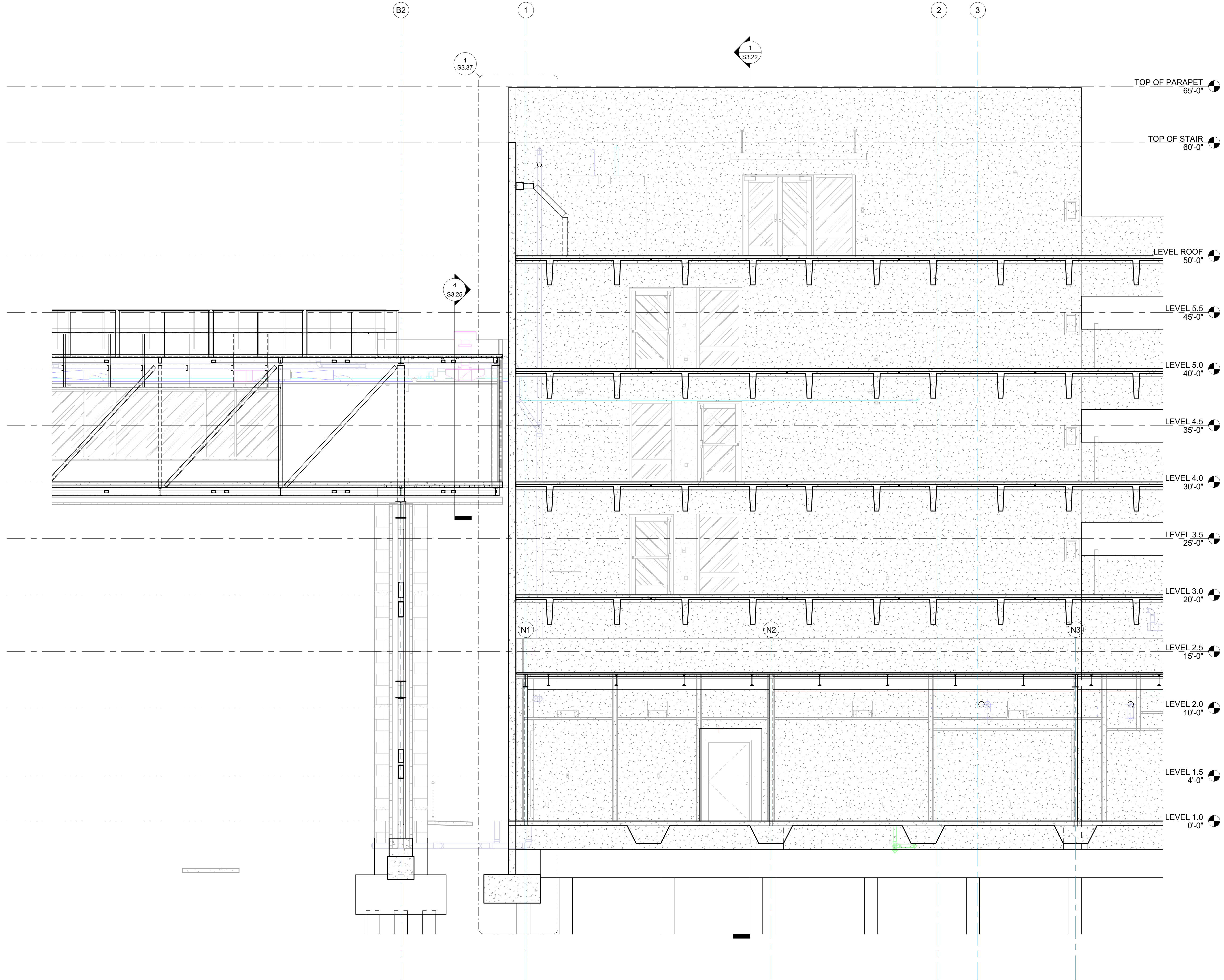
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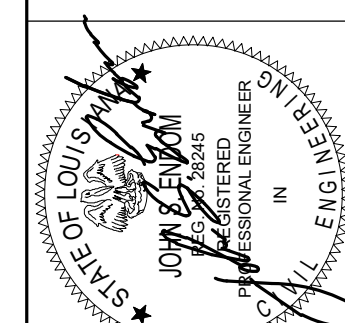


1
S3.26.3
BUILDING SECTION
1/4" = 1'-0"

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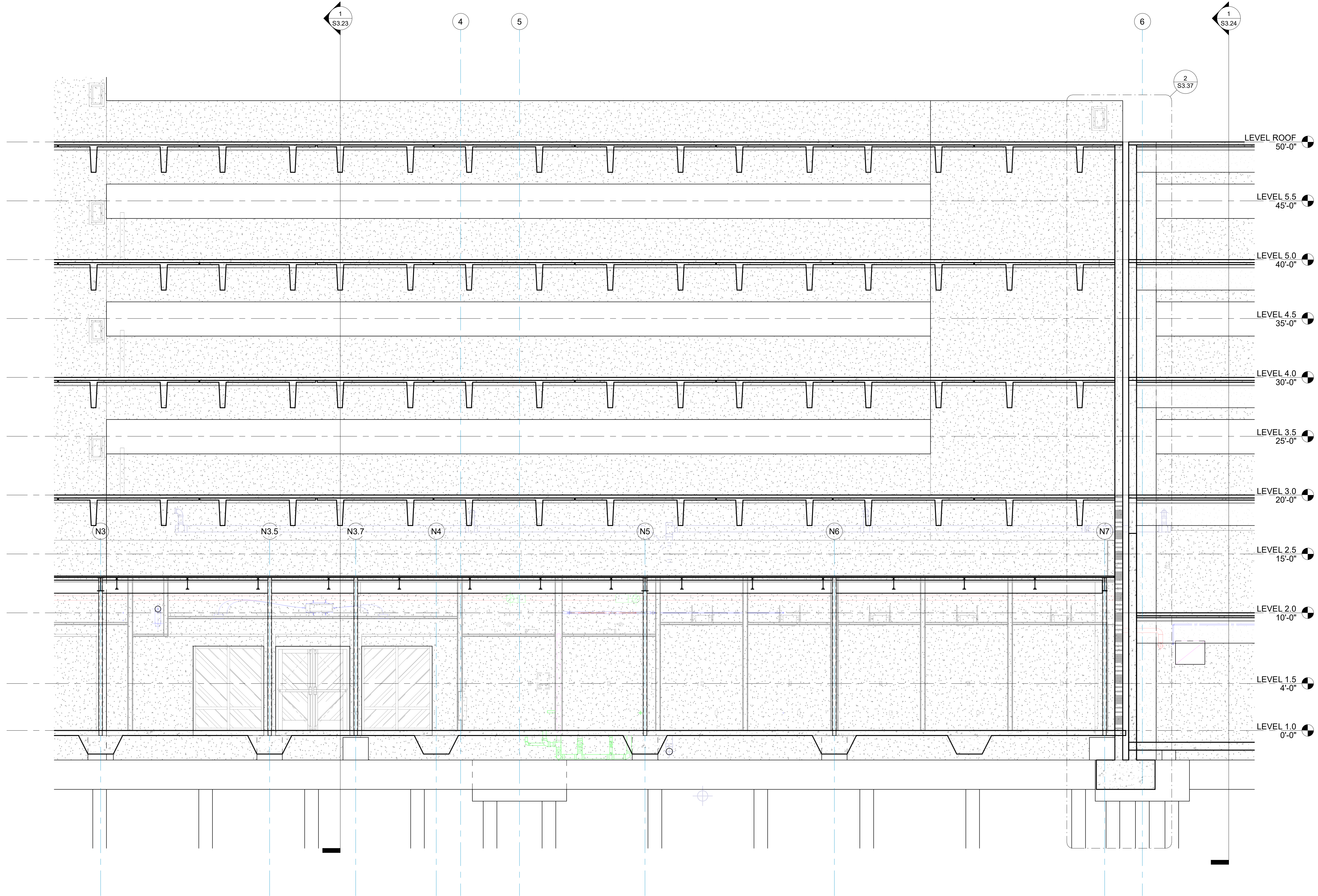
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BUILDING SECTIONS

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1
S3.26.4

BUILDING SECTION
1/4" = 1'-0"

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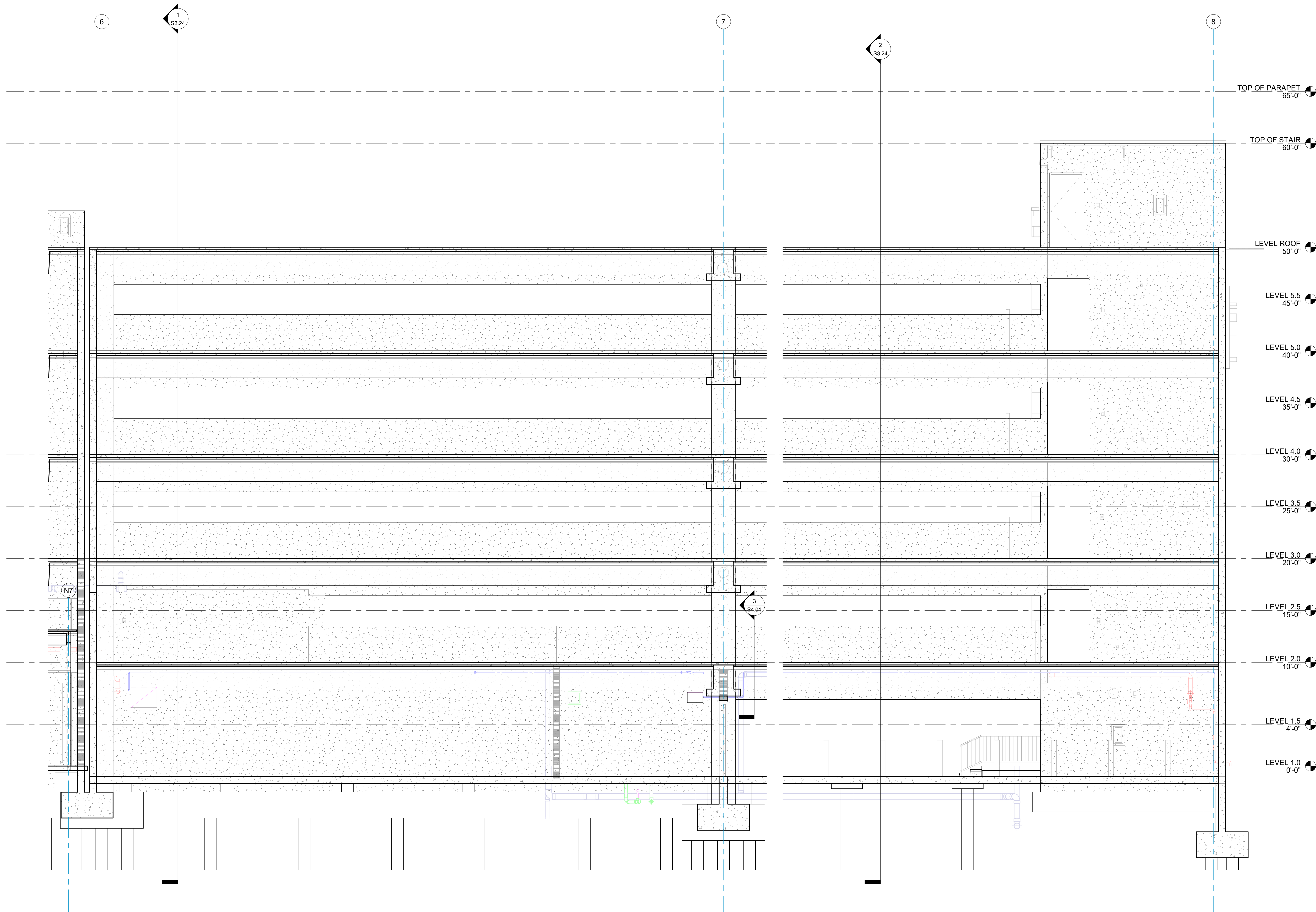
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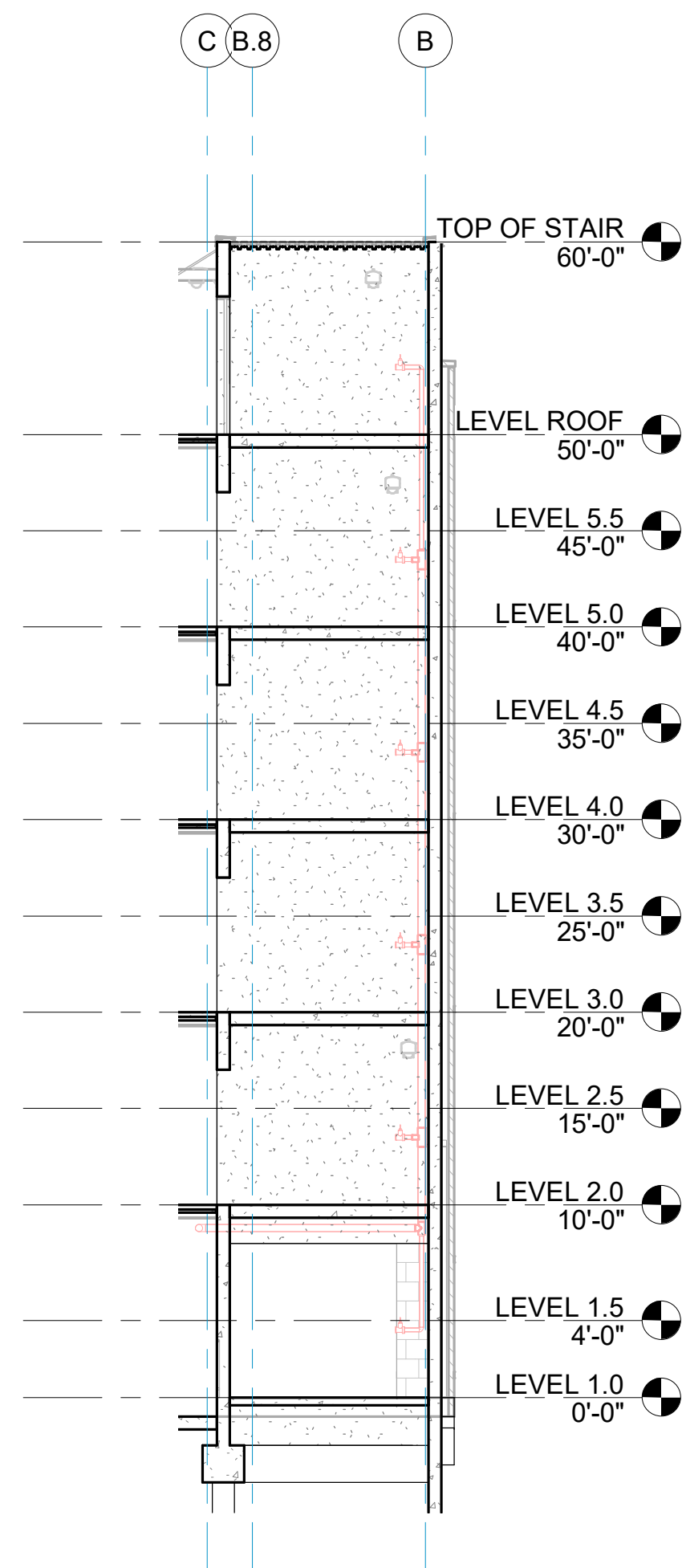
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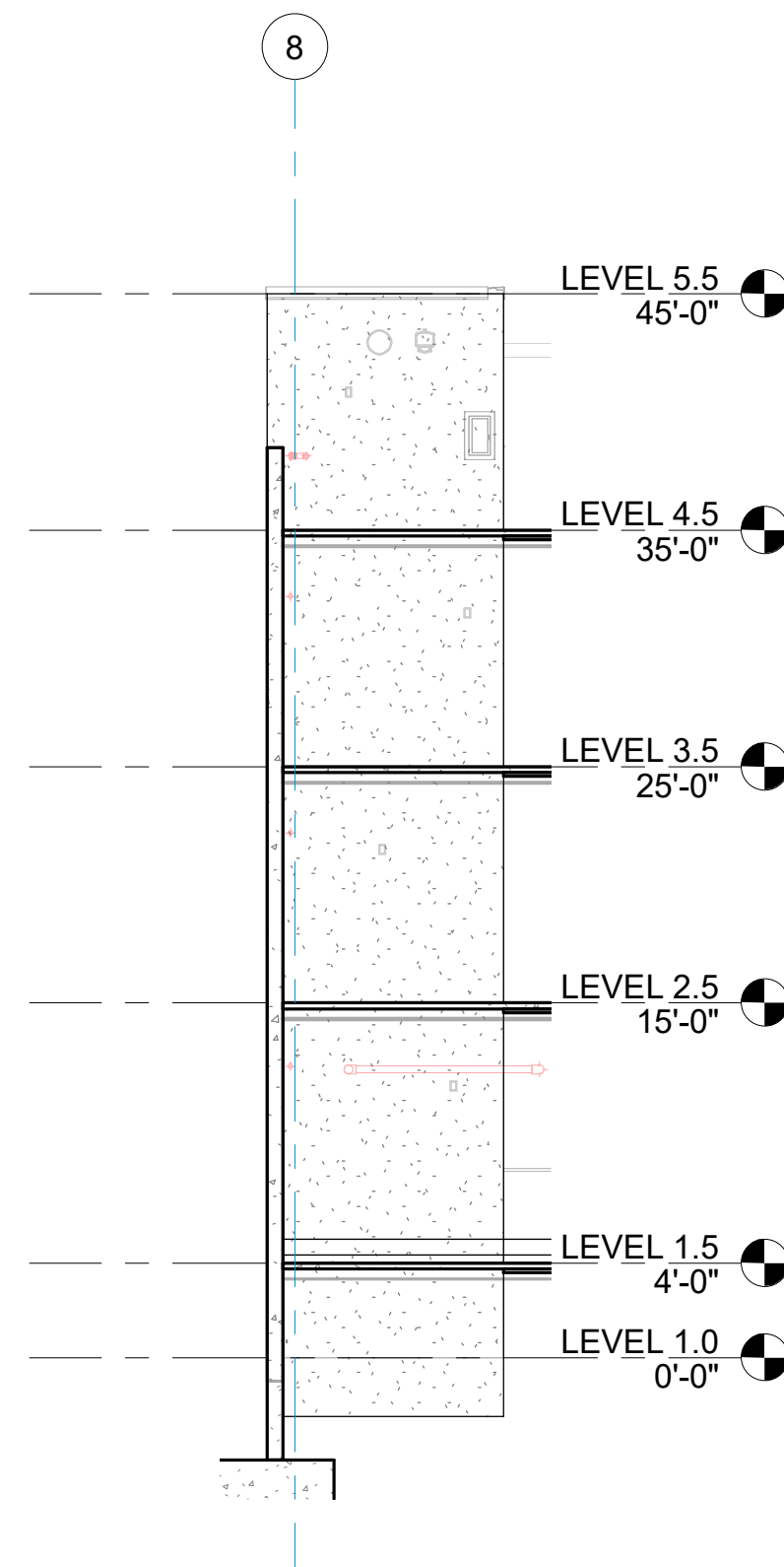
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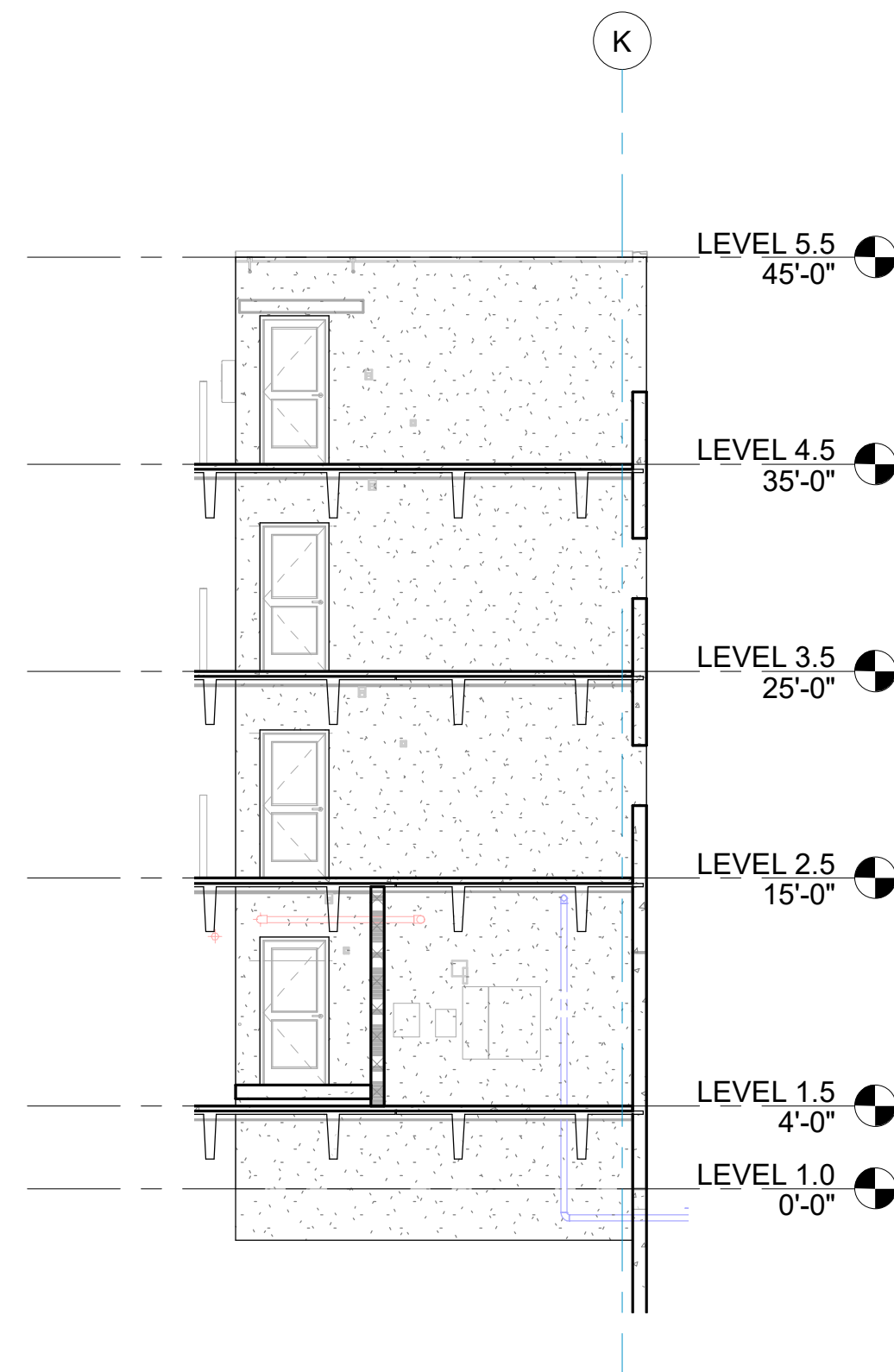
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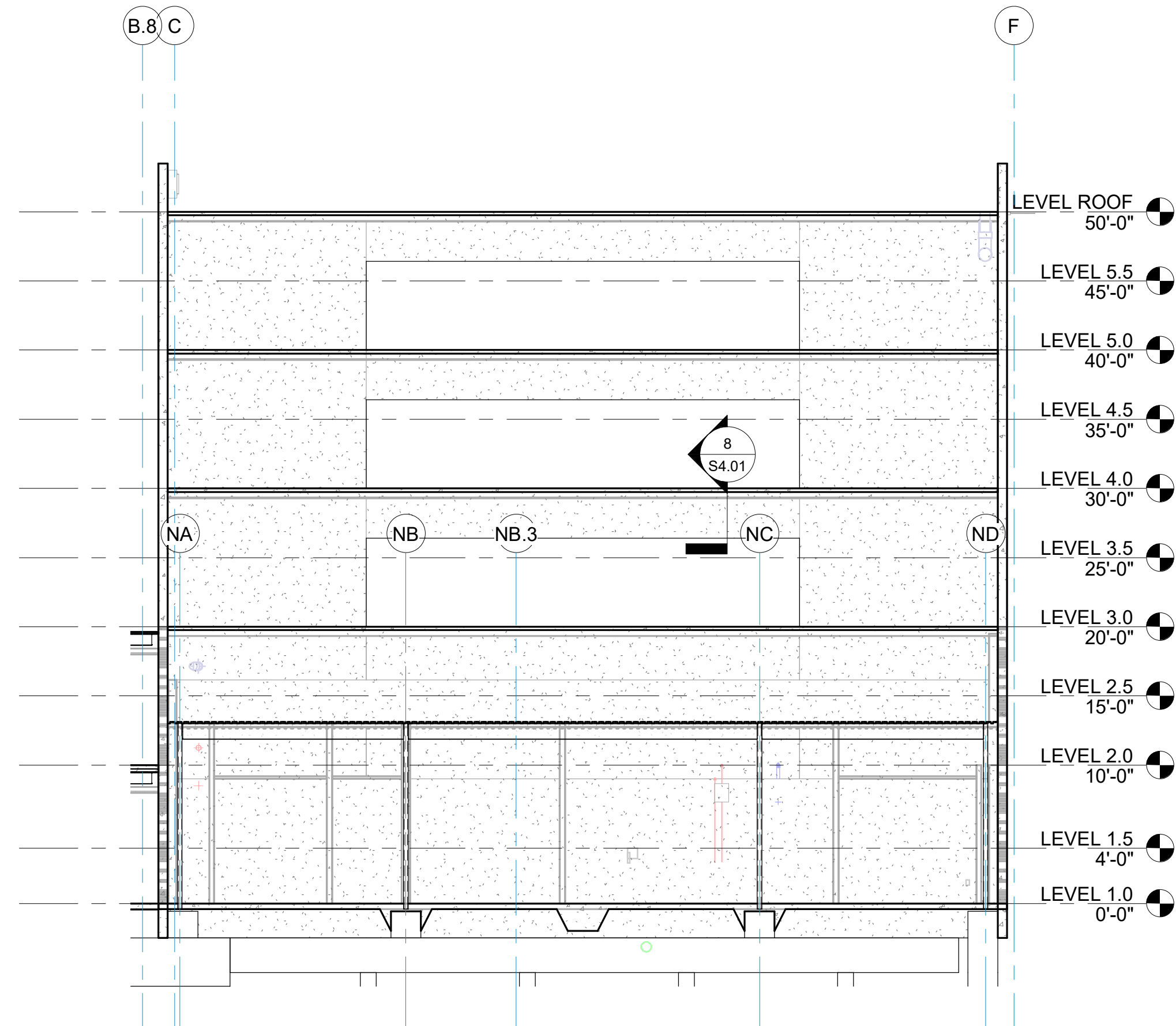
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S3.27
1/8" = 1'-0"



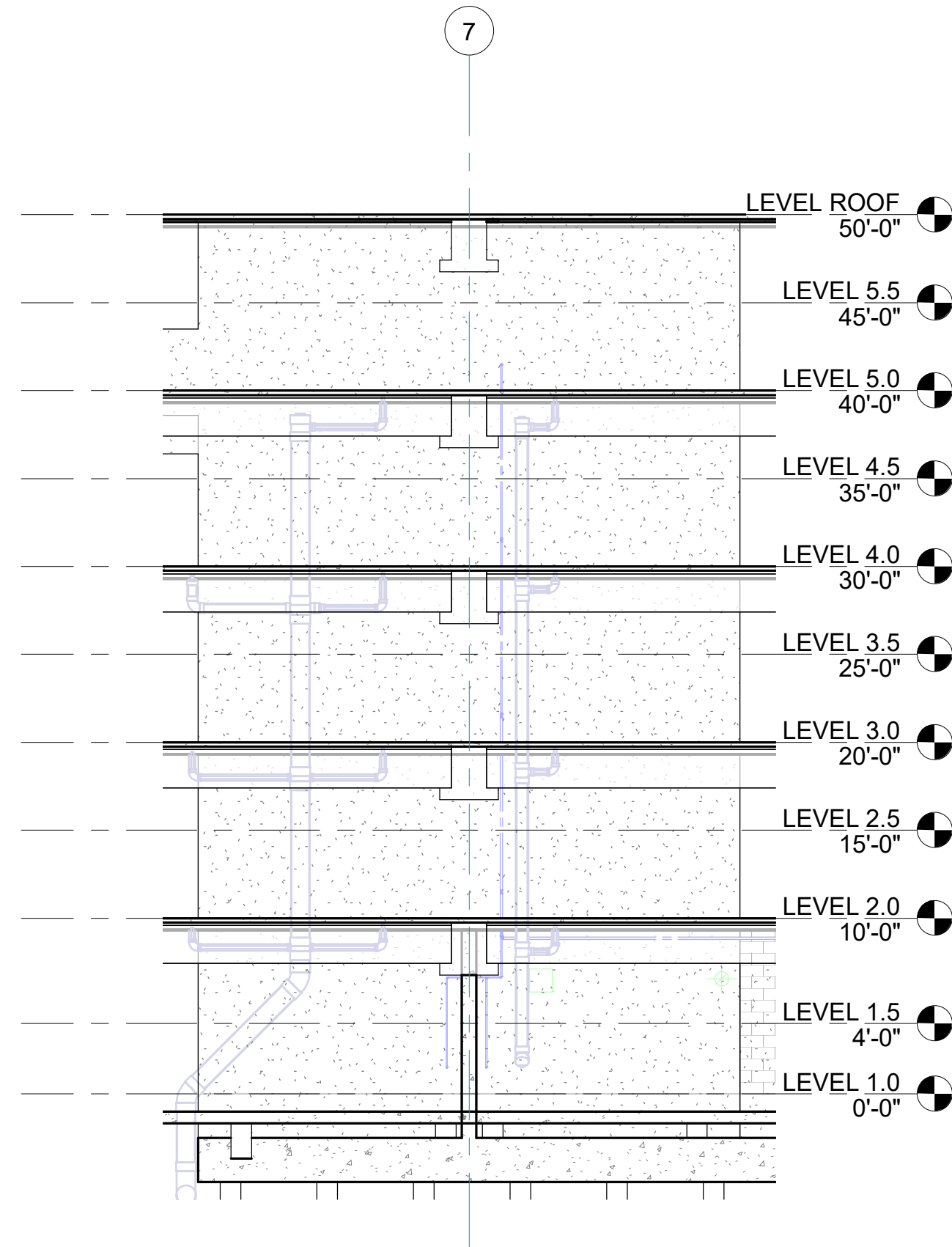
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S3.27
1/8" = 1'-0"



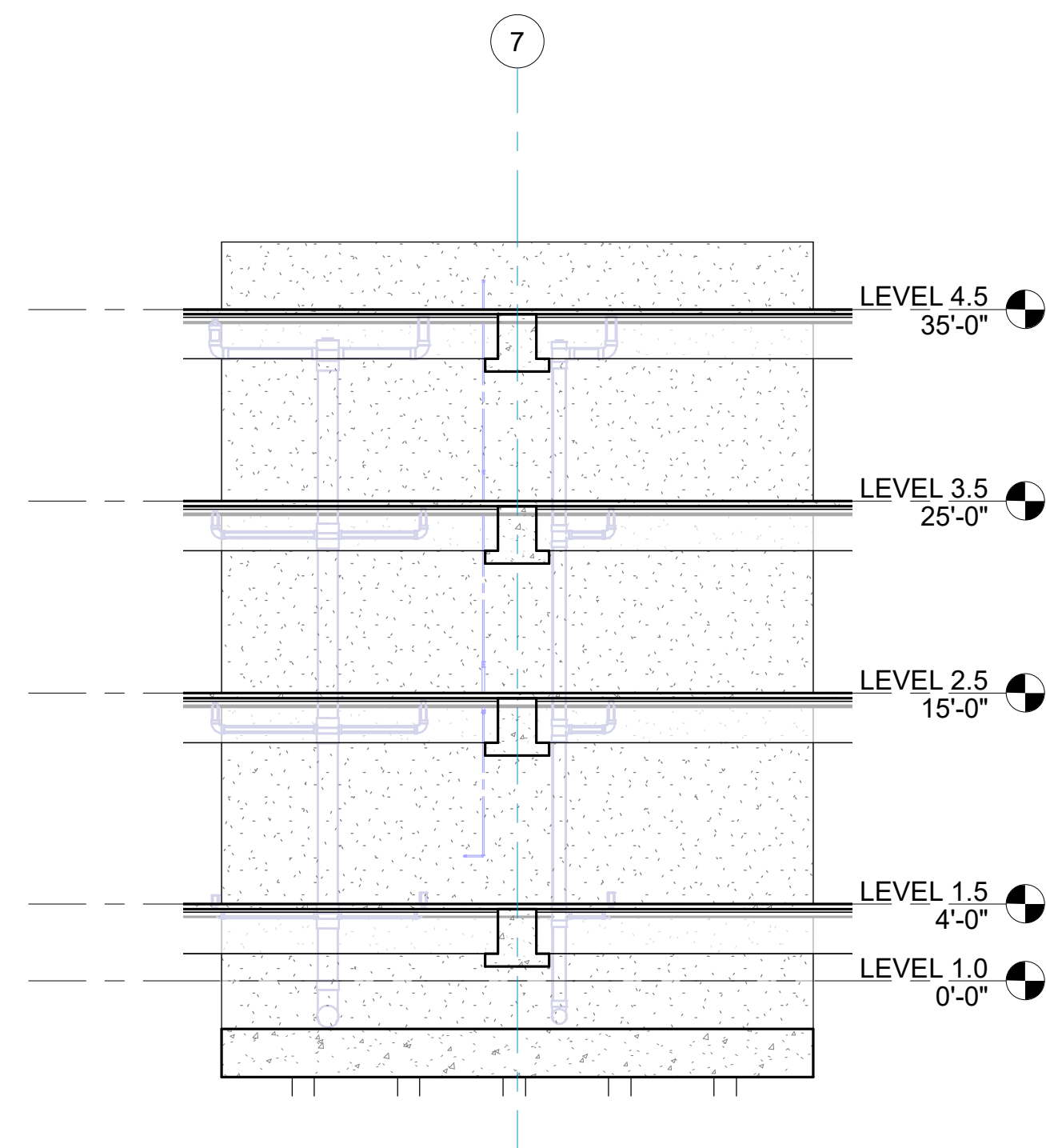
3
S3.27
1/8" = 1'-0"



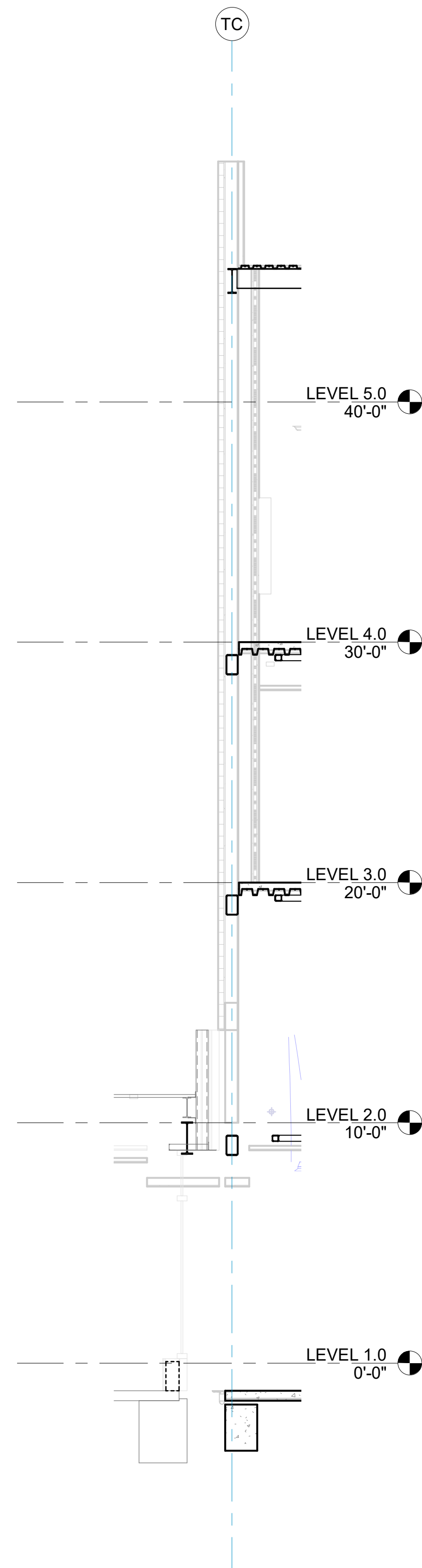
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1/8" = 1'-0"



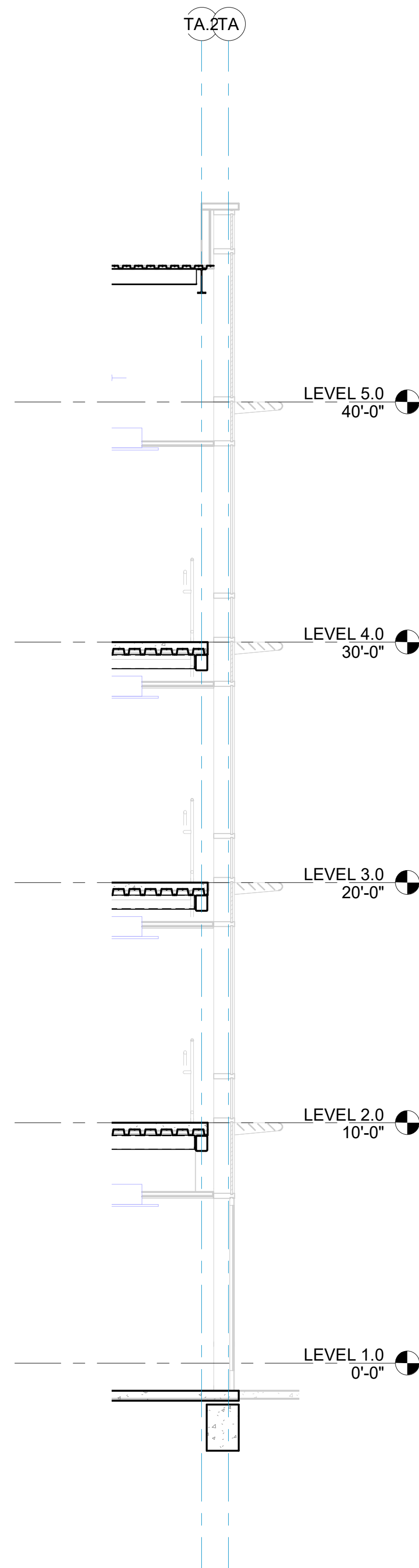
5
S3.27
1/8" = 1'-0"



6
S3.27
1/8" = 1'-0"



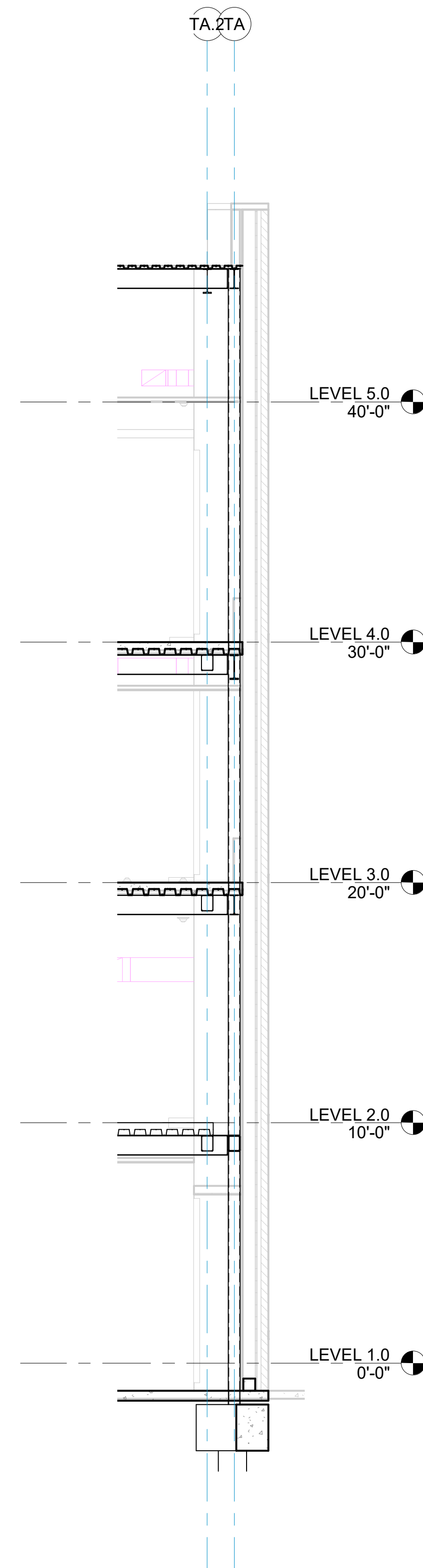
1 WALL SECTION
S3.30 1/4" = 1'-0"



2 WALL SECTION
S3.30 1/4" = 1'-0"



3 WALL SECTION
S3.30 1/4" = 1'-0"



4 WALL SECTION
S3.30 1/4" = 1'-0"

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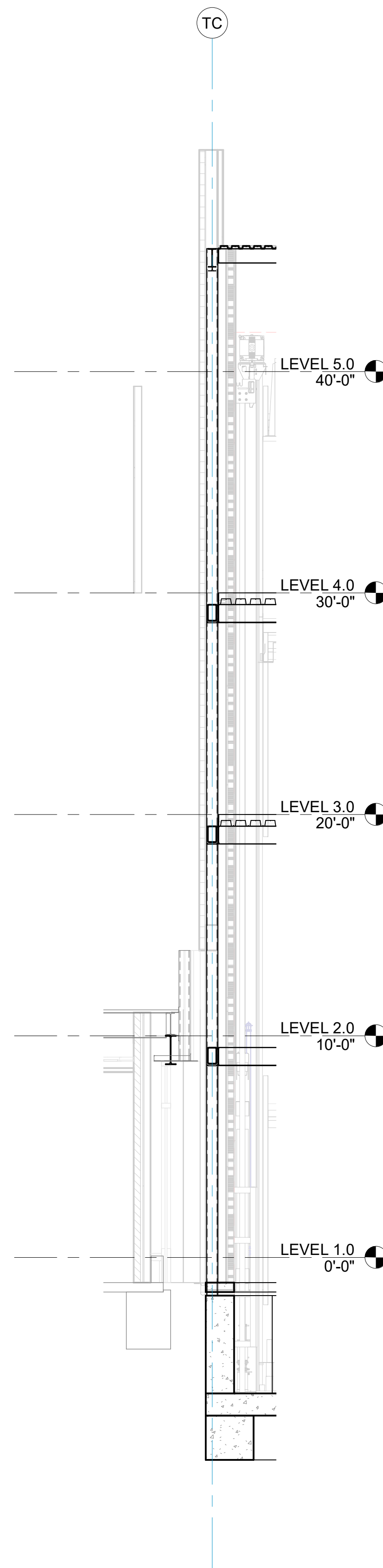
WALL SECTIONS

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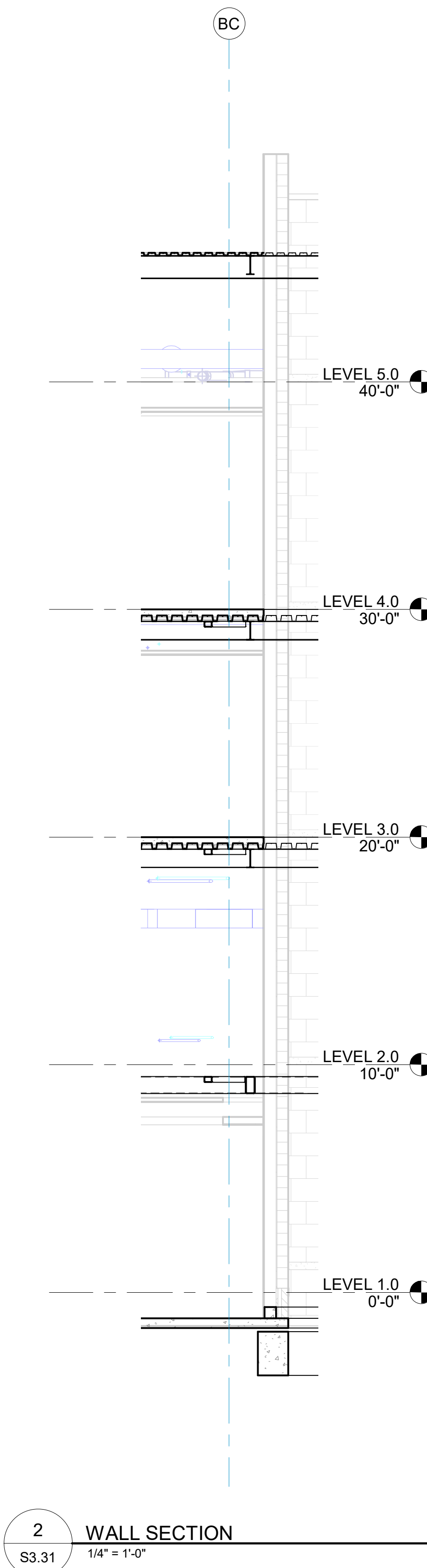
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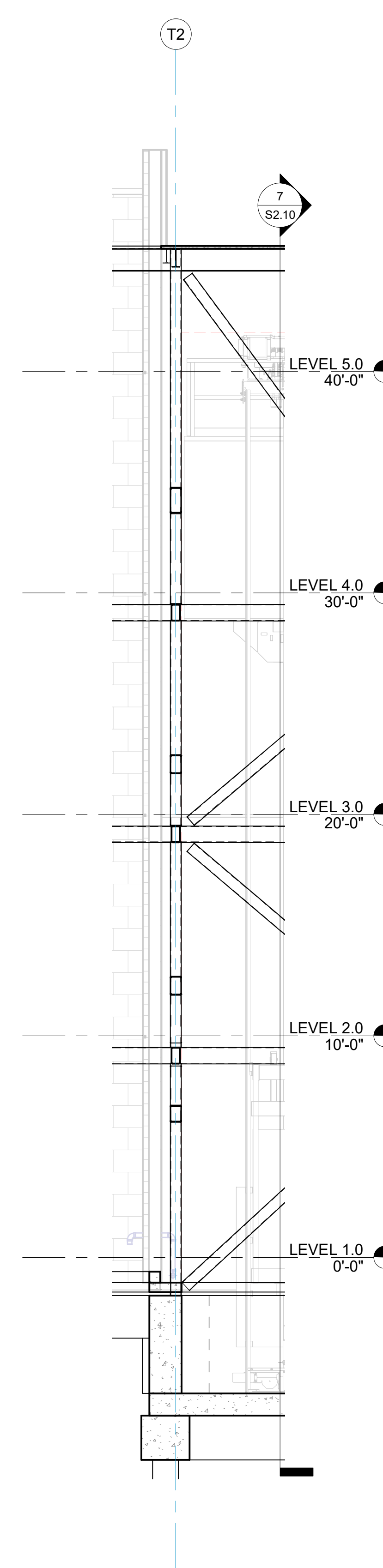
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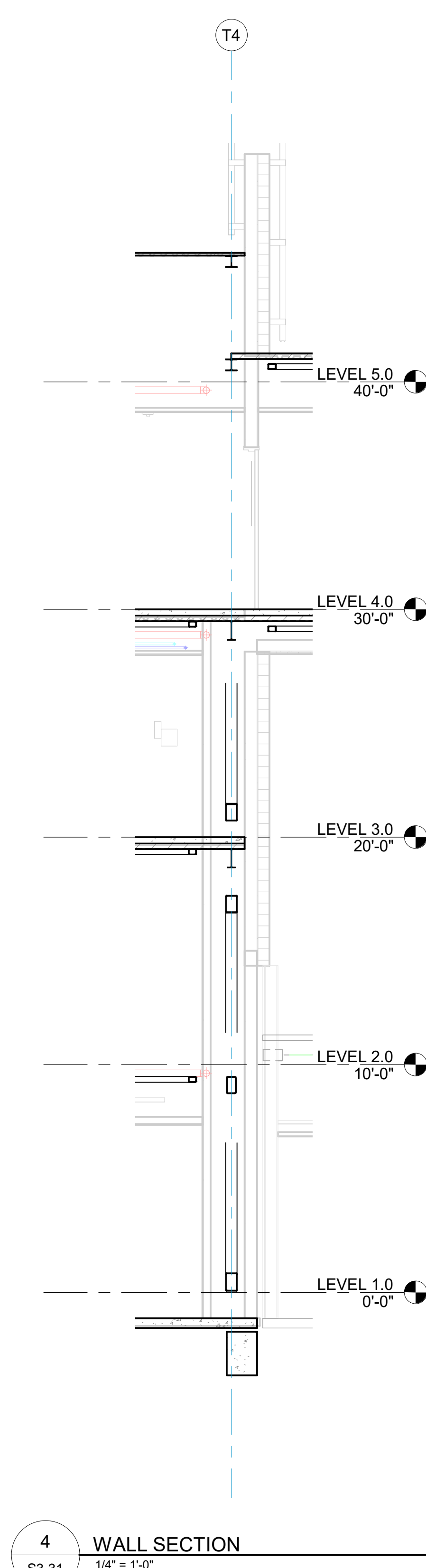
1 WALL SECTION
S3.31 1/4" = 1'-0"



2 WALL SECTION
S3.31 1/4" = 1'-0"



3 WALL SECTION
S3.31 1/4" = 1'-0"



4 WALL SECTION
S3.31 1/4" = 1'-0"

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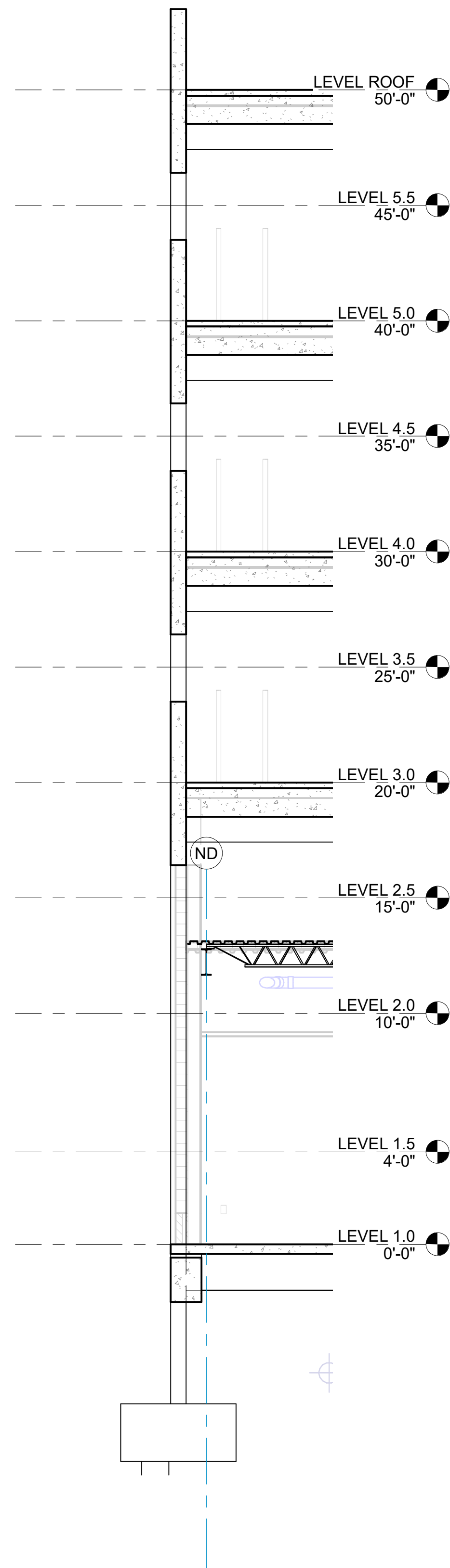
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WALL SECTIONS

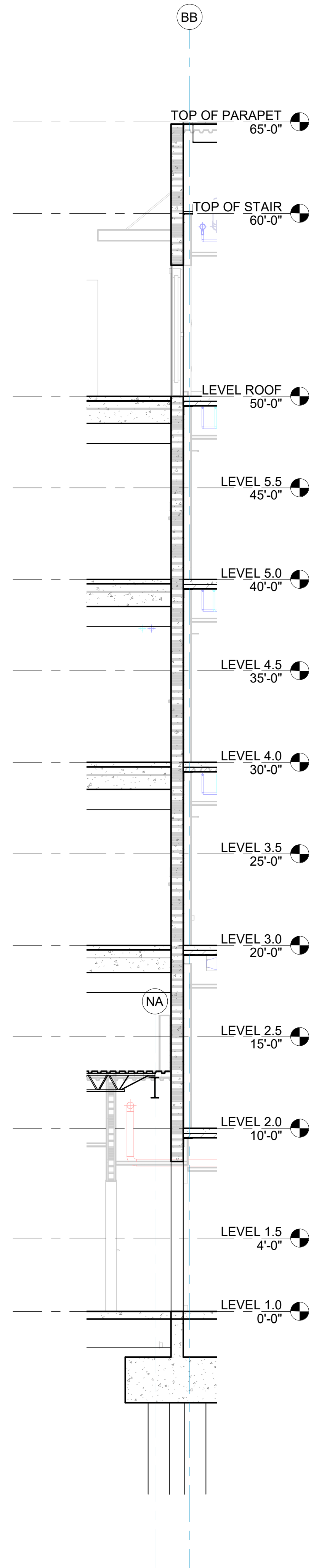
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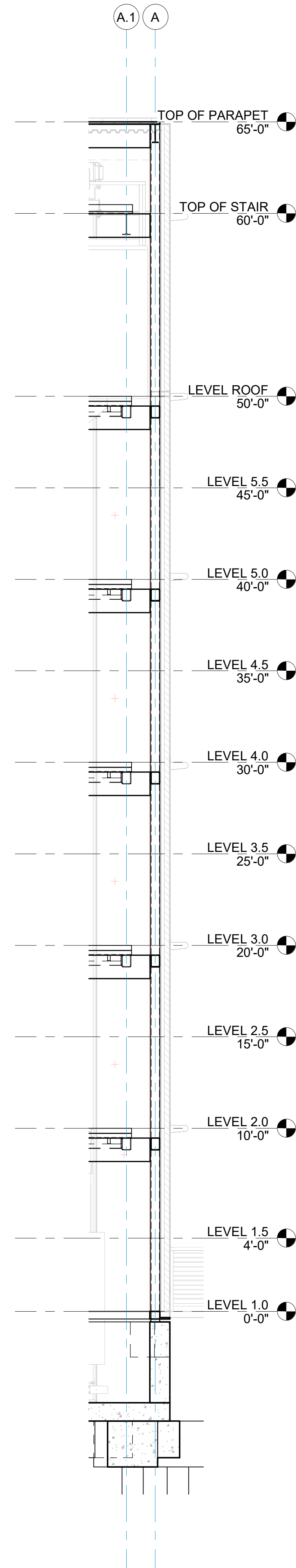
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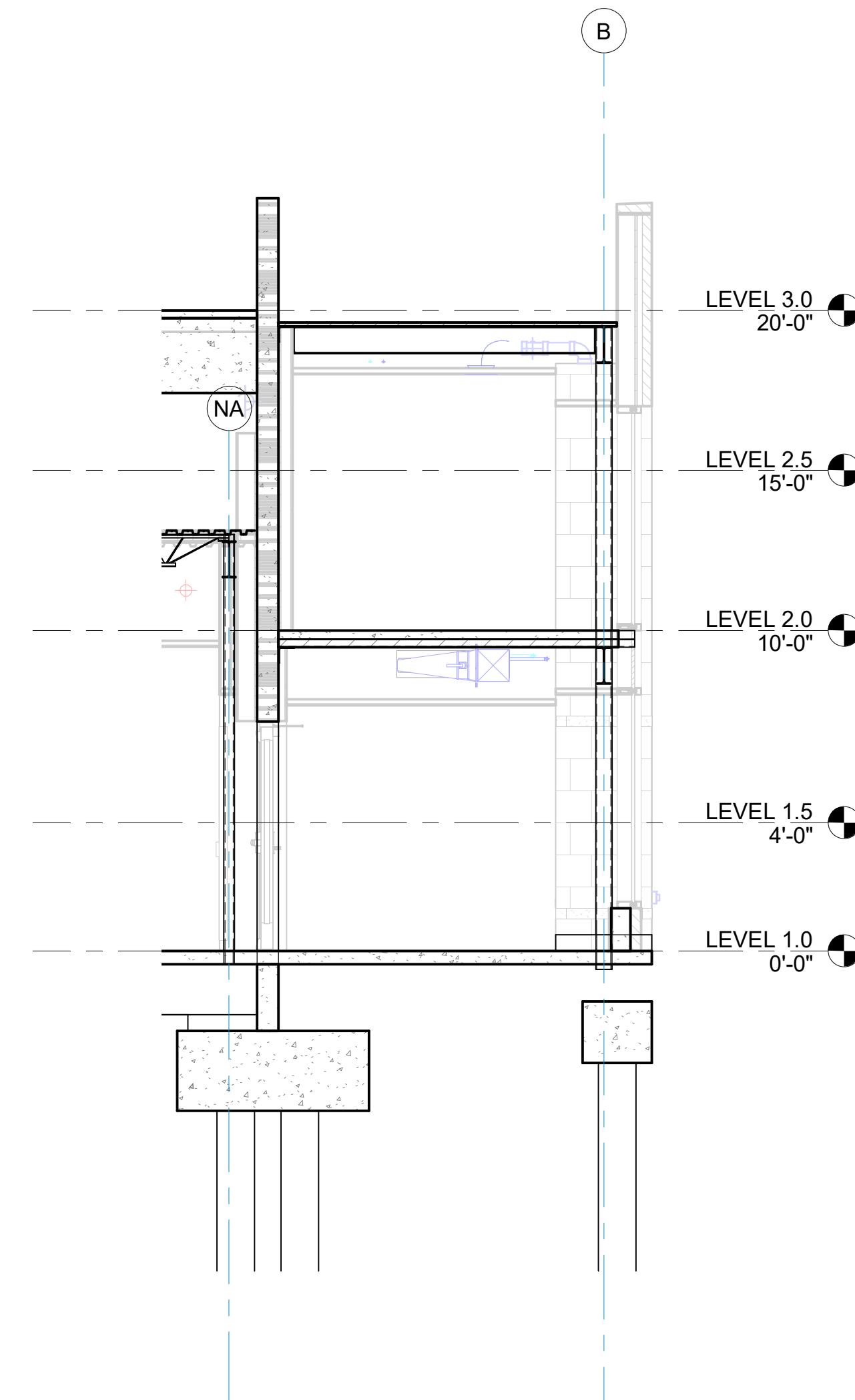
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S3.32 1/4" = 1'-0"



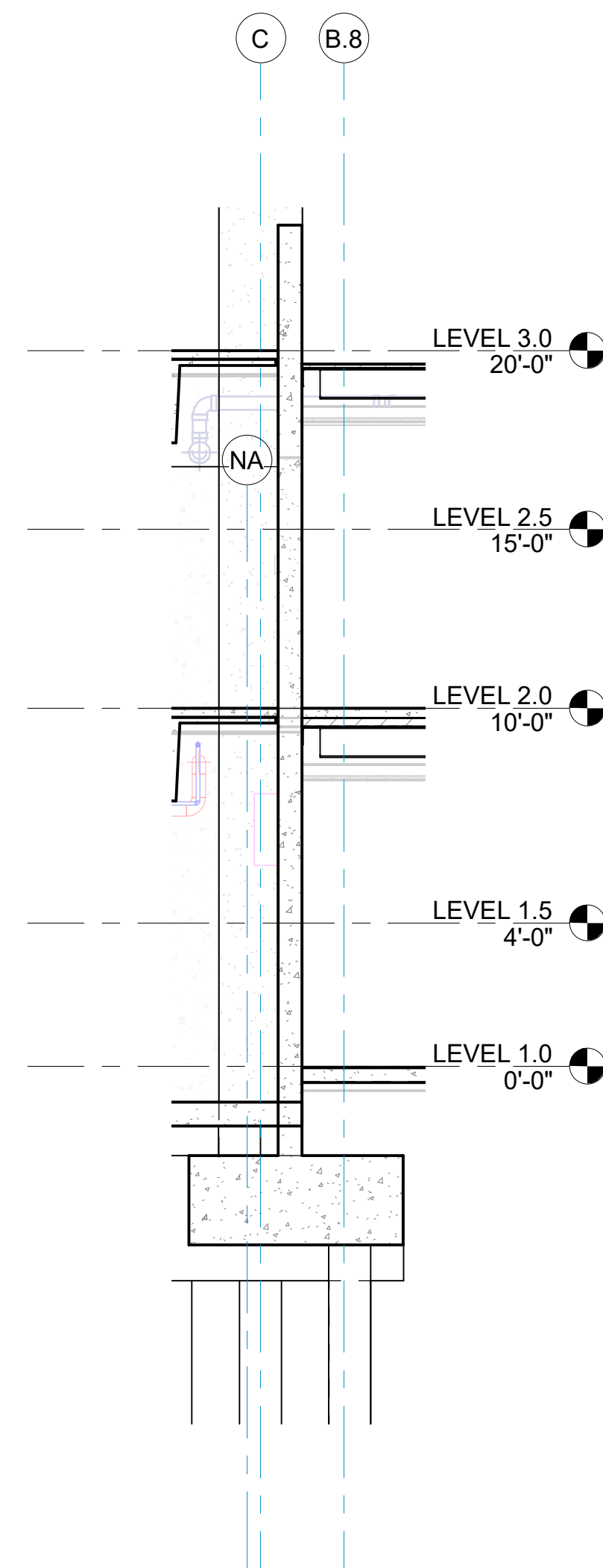
2 WALL SECTION
S3.32 1/4" = 1'-0"



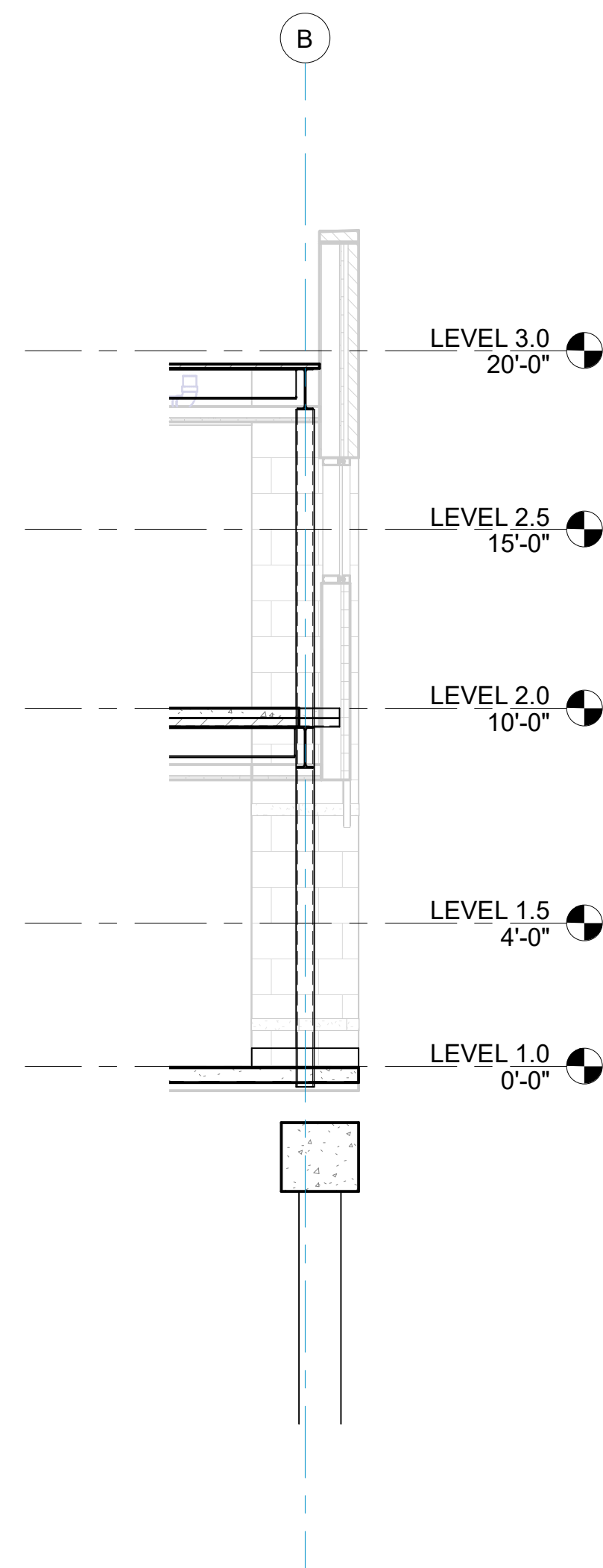
3 WALL SECTION
S3.32 1/4" = 1'-0"



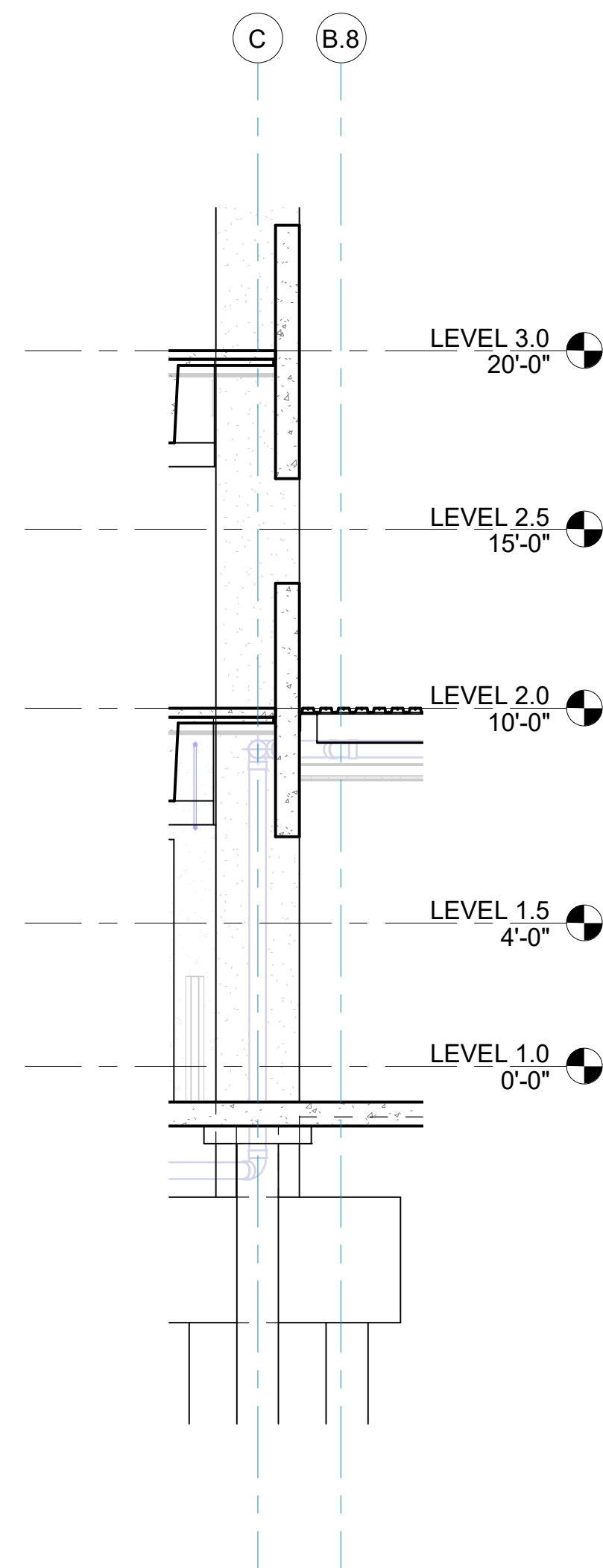
4 WALL SECTION
S3.32 1/4" = 1'-0"



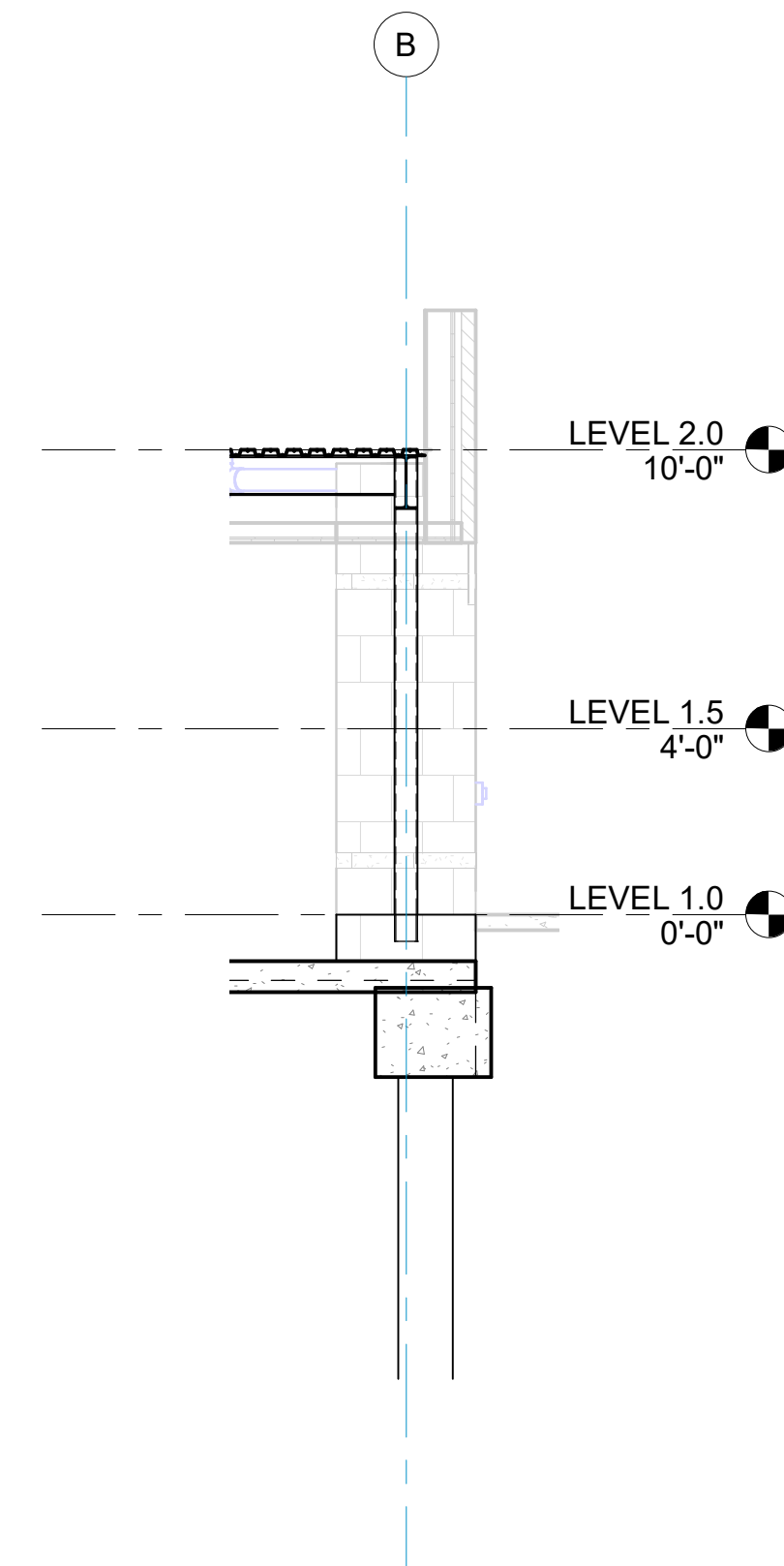
1 WALL SECTION
S3.33 1/4" = 1'-0"



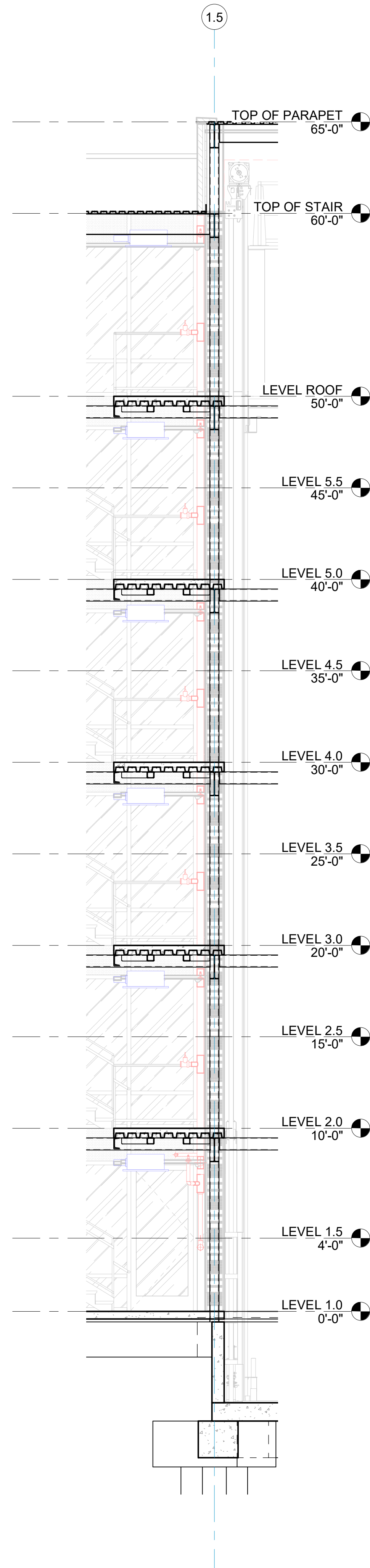
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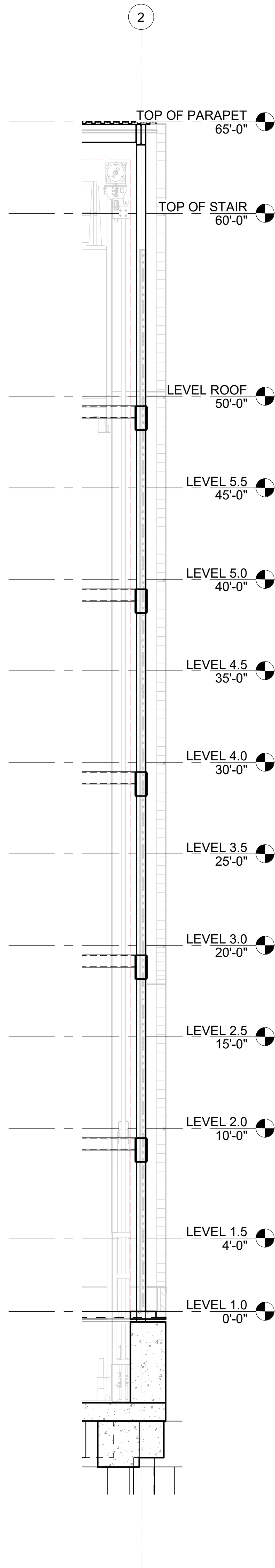
3 WALL SECTION
S3.33 1/4" = 1'-0"



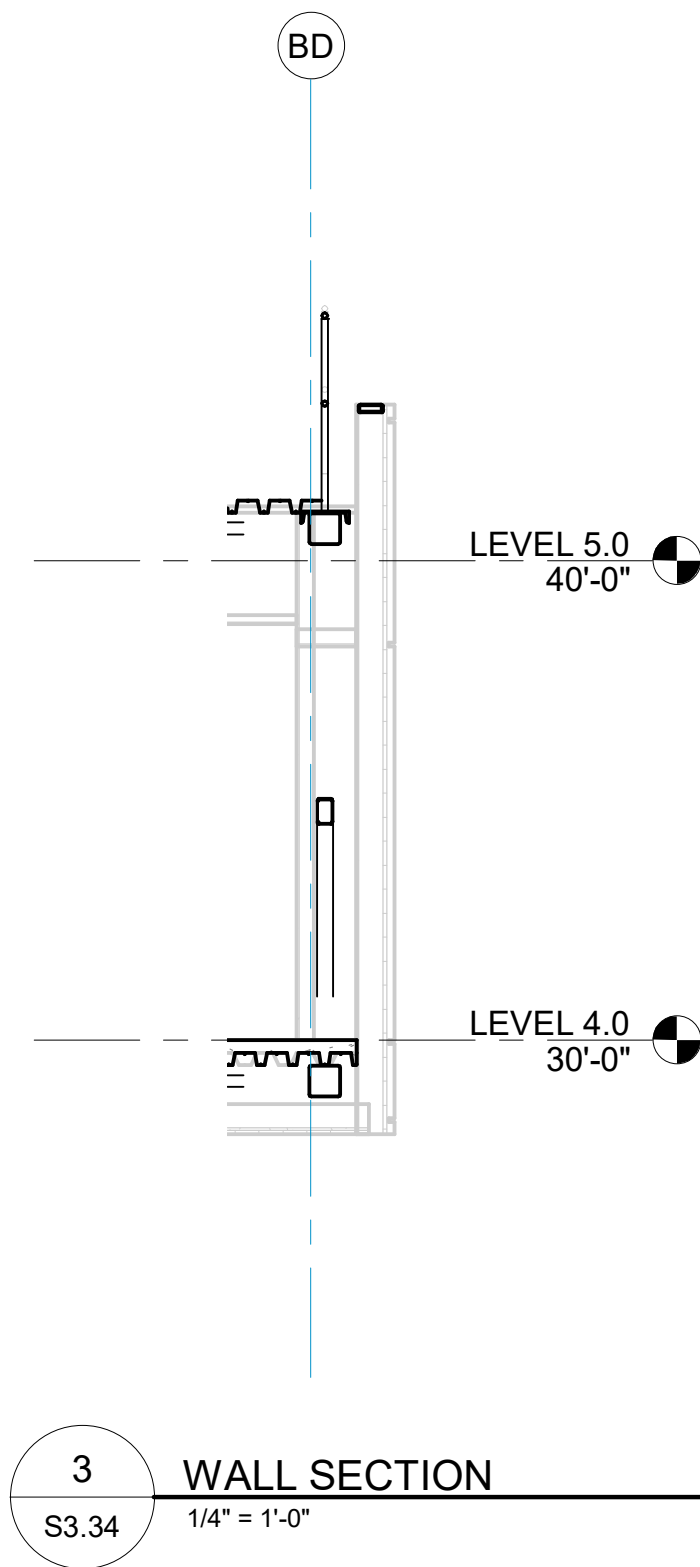
4 WALL SECTION
S3.33 1/4" = 1'-0"



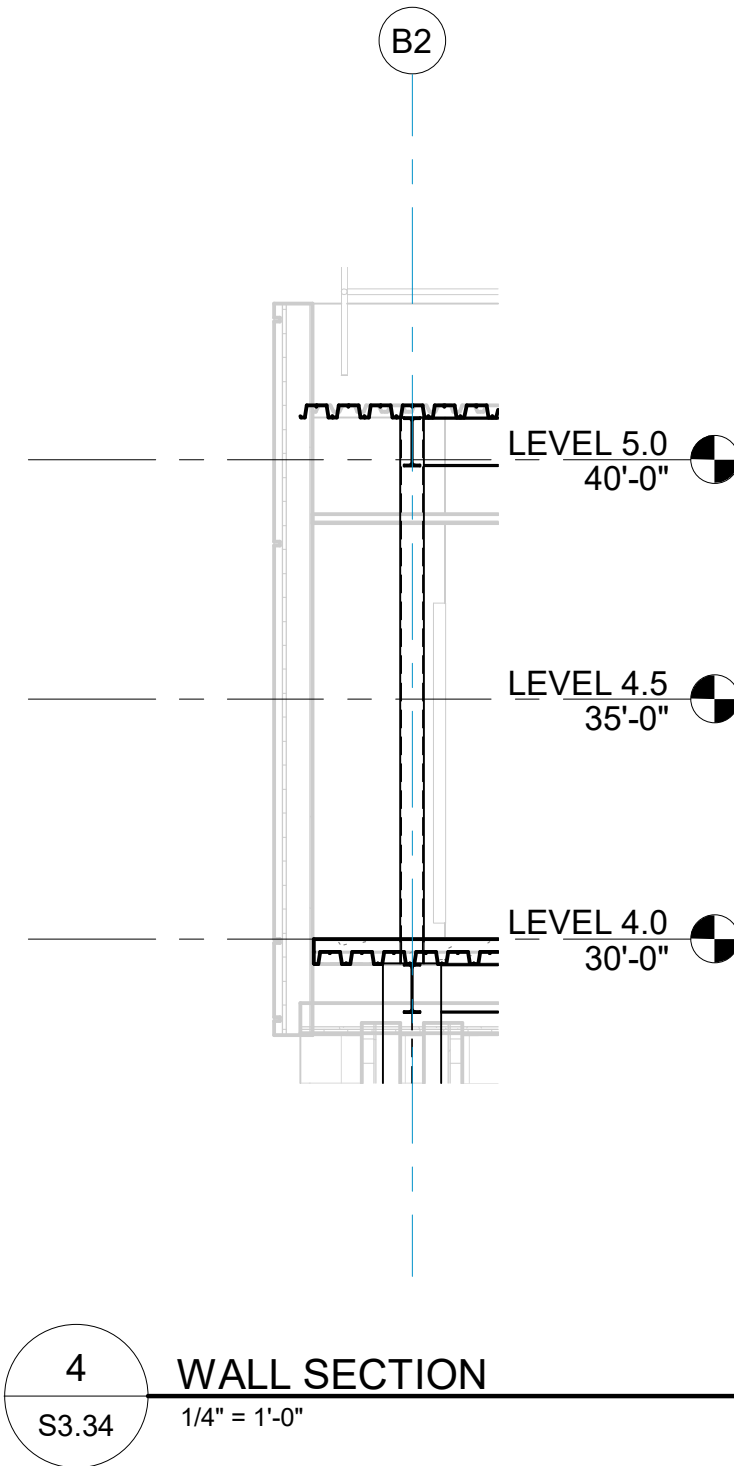
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S3.34 1/4" = 1'-0"



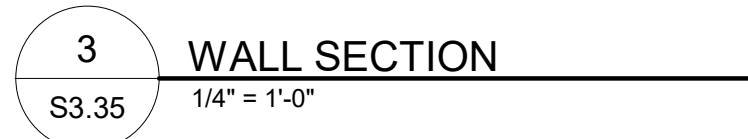
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S3.34 1/4" = 1'-0"

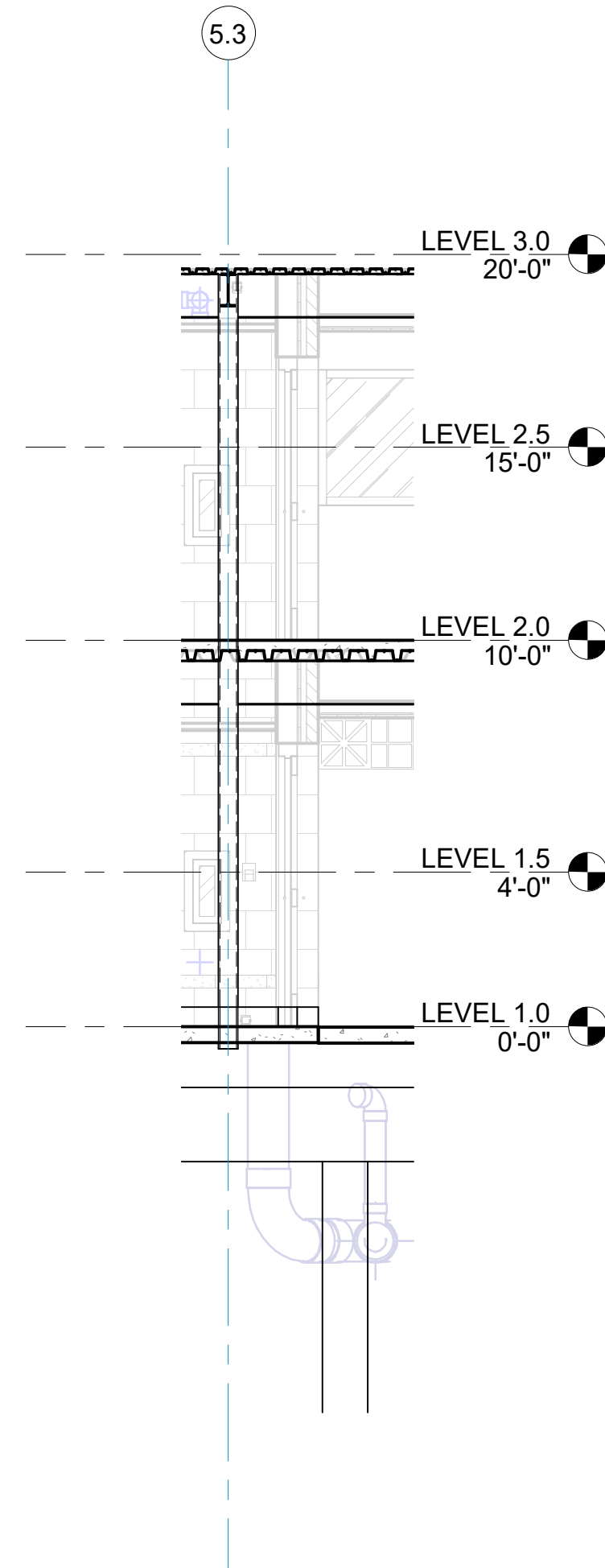


3 WALL SECTION
S3.34 1/4" = 1'-0"

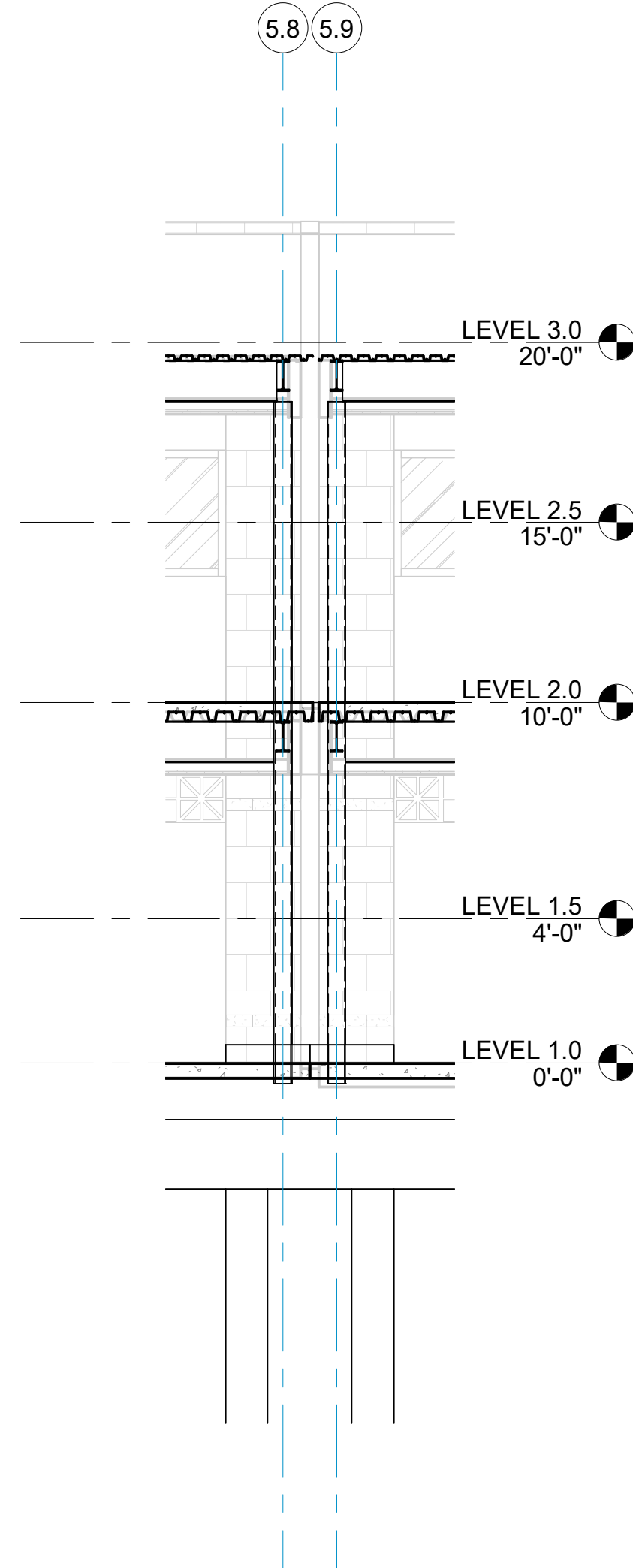


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S3.34 1/4" = 1'-0"

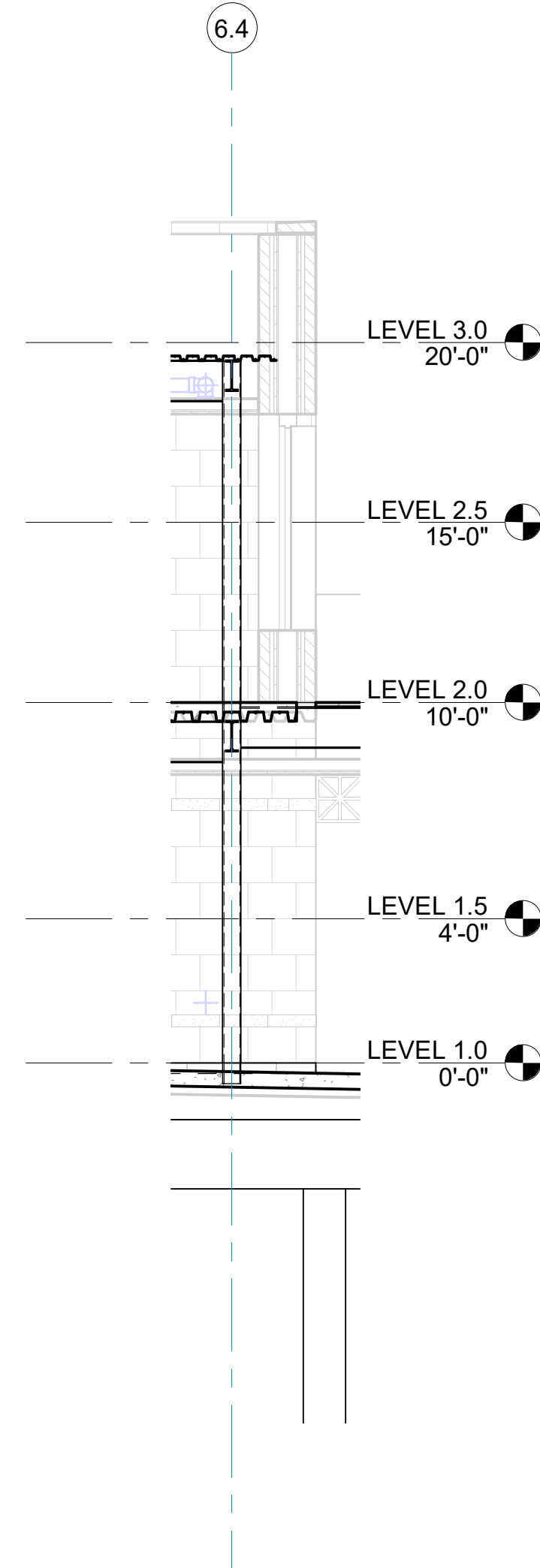




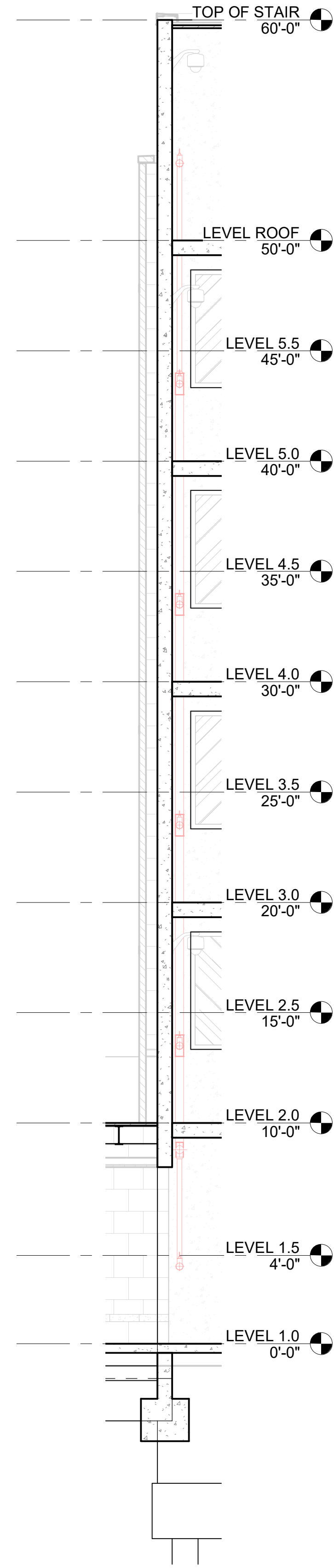
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S3.36 1/4" = 1'-0"



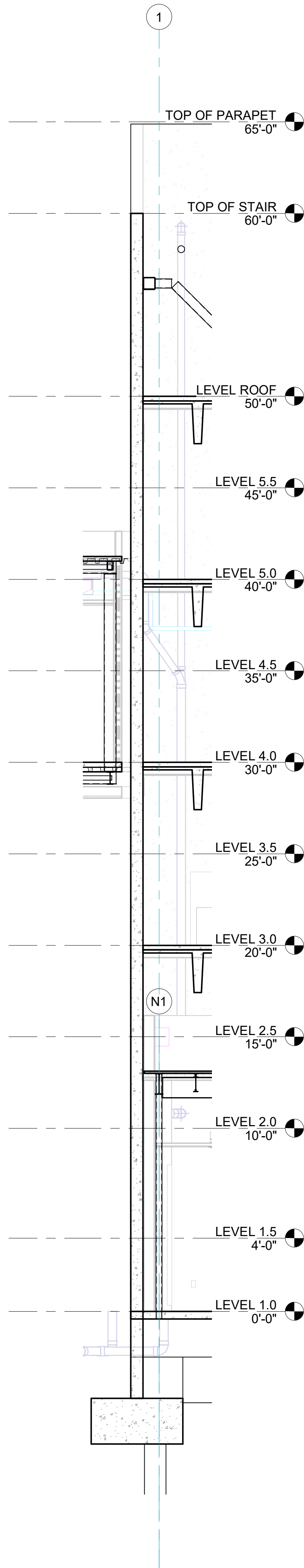
2 WALL SECTION
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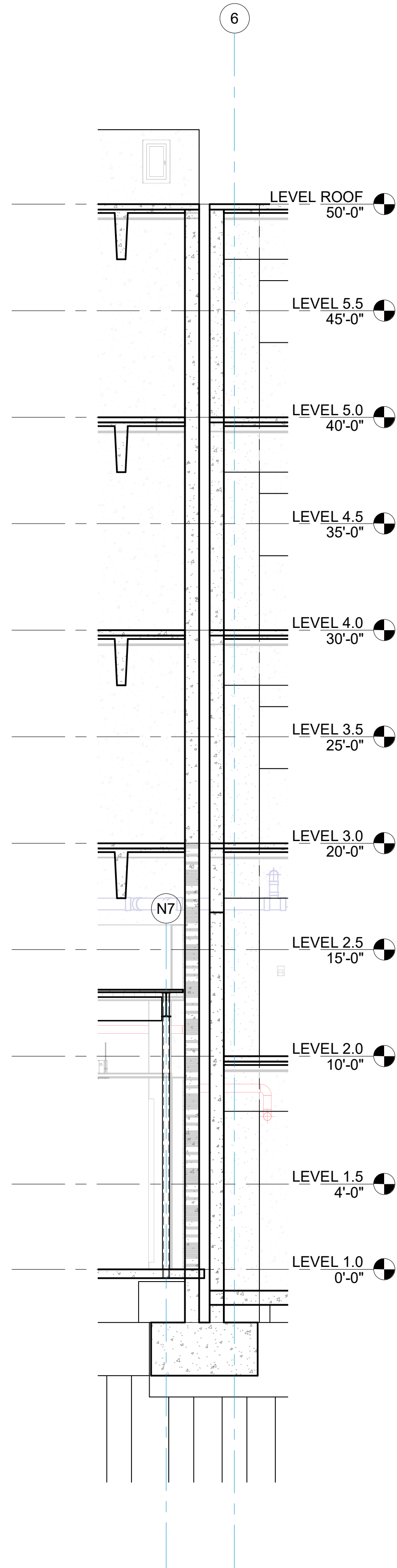
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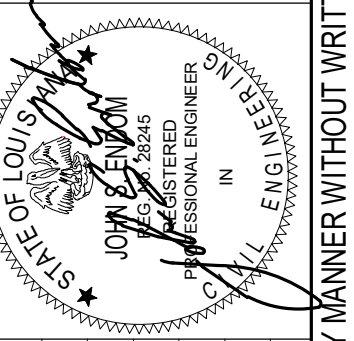
4 WALL SECTION
S3.36 1/4" = 1'-0"



1 WALL SECTION
S3.37 1/4" = 1'-0"



2 WALL SECTION
S3.37 1/4" = 1'-0"



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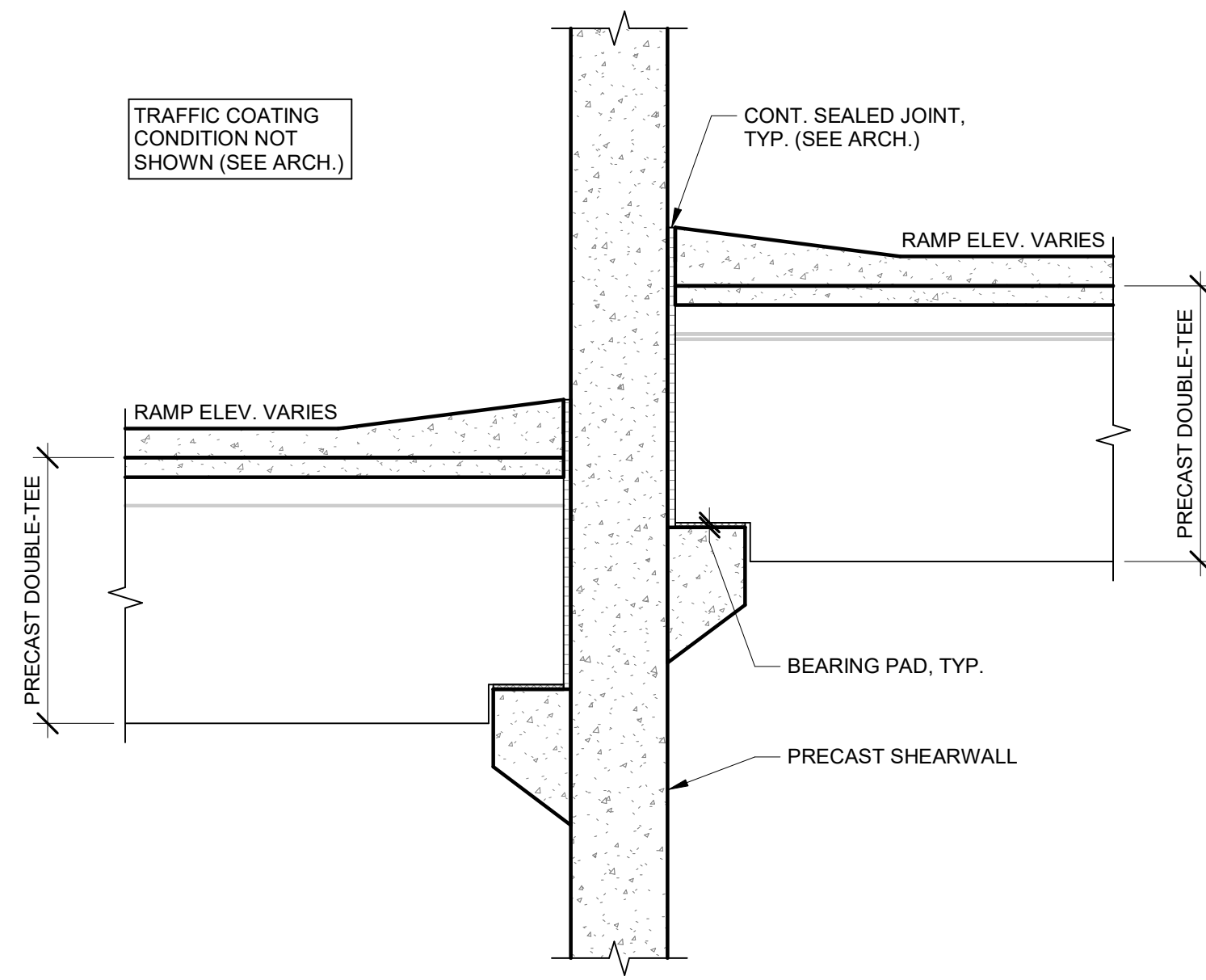
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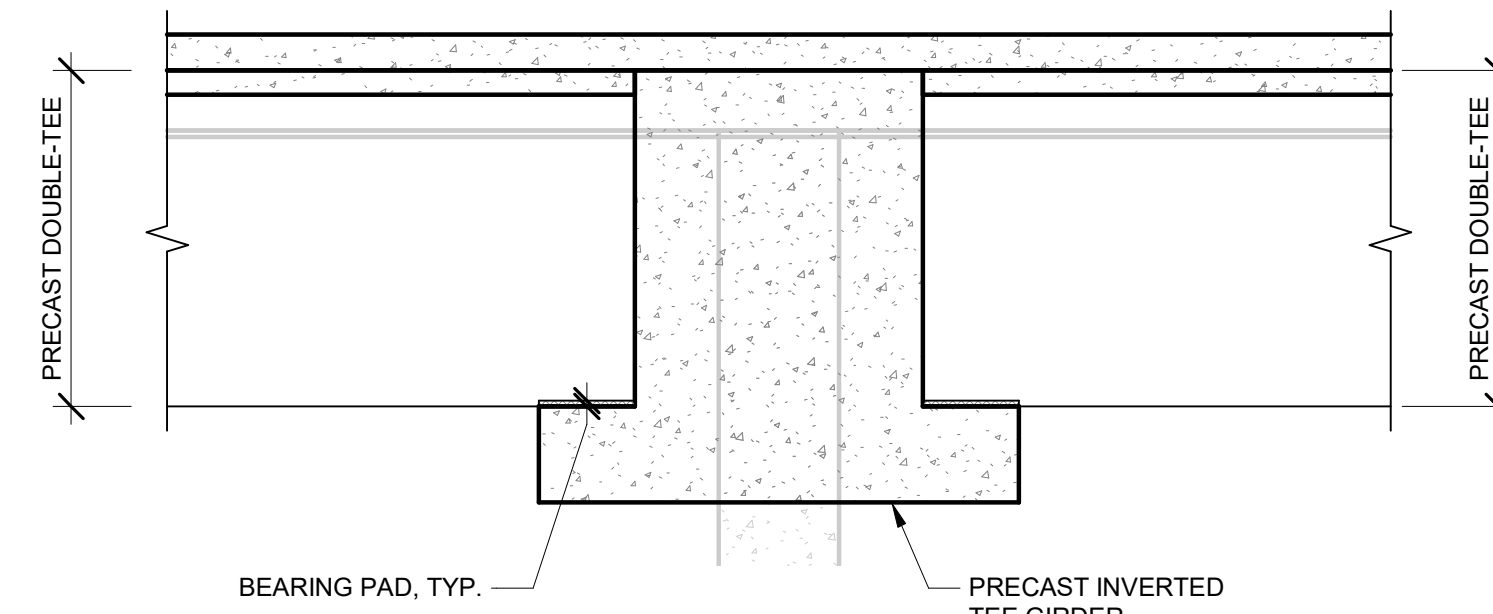
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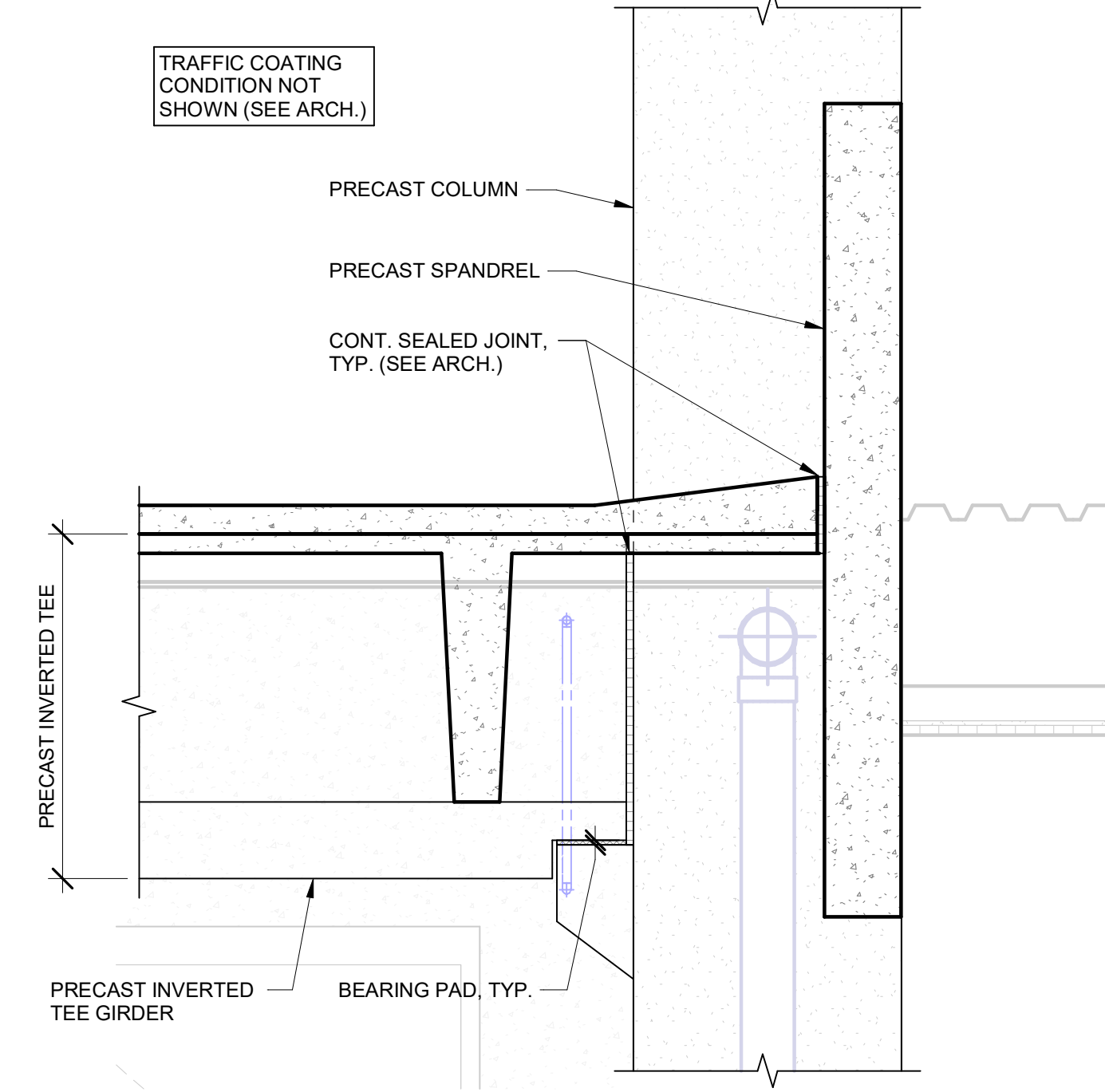




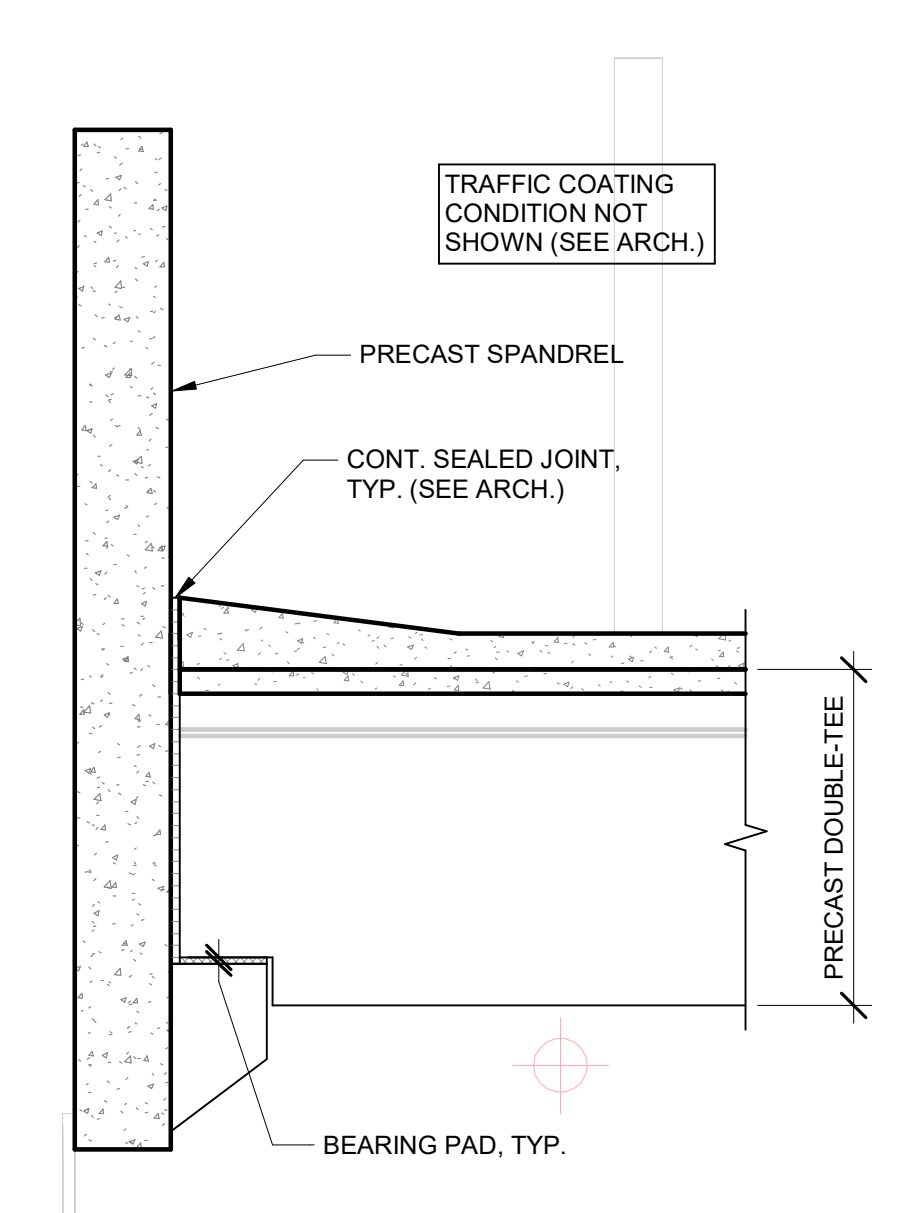
1 TYPICAL SECTION AT LONGITUDINAL SHEAR WALL
S4.01 3/4" = 1'-0"



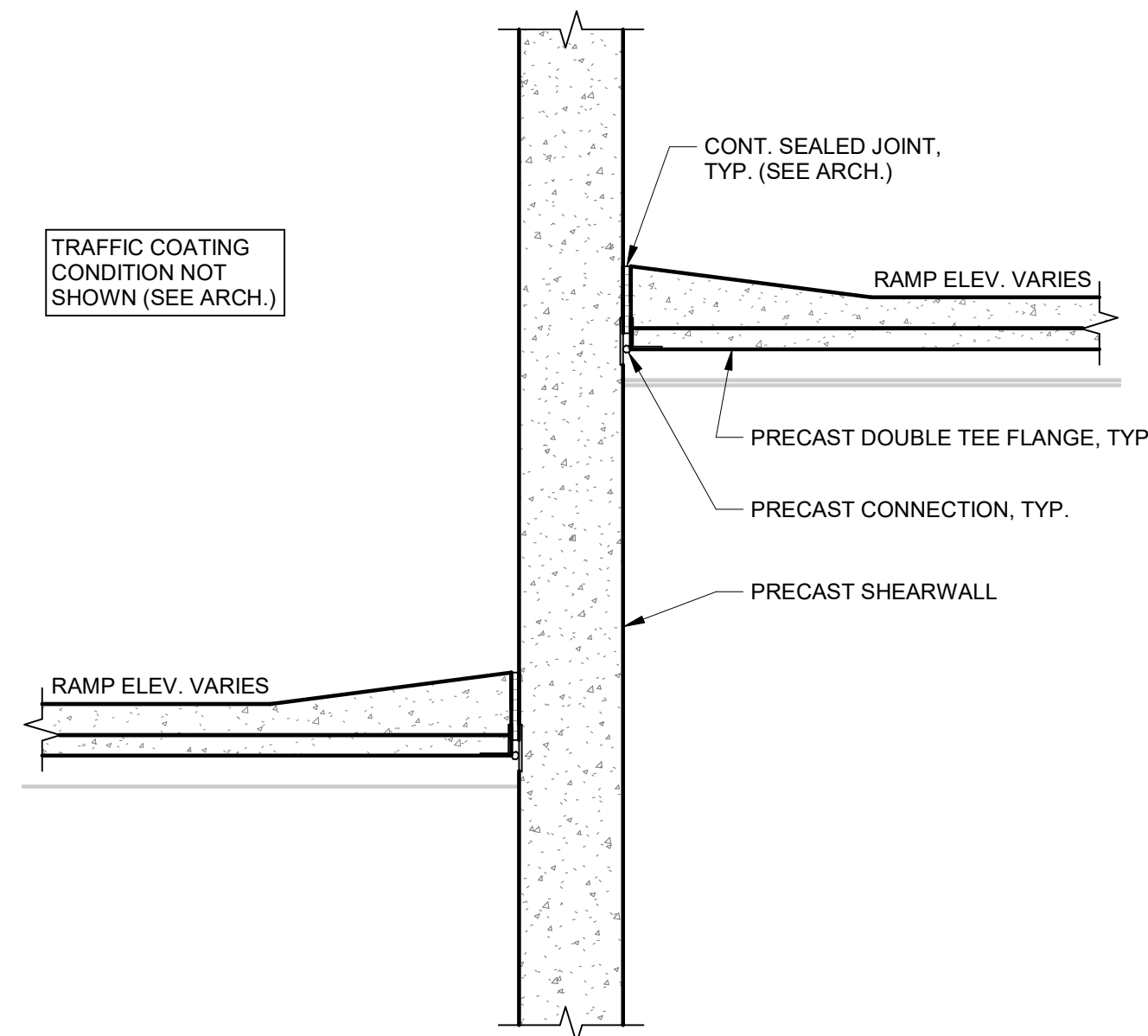
2 TYPICAL SECTION AT INVERTED TEE GIRDER
S4.01 3/4" = 1'-0"



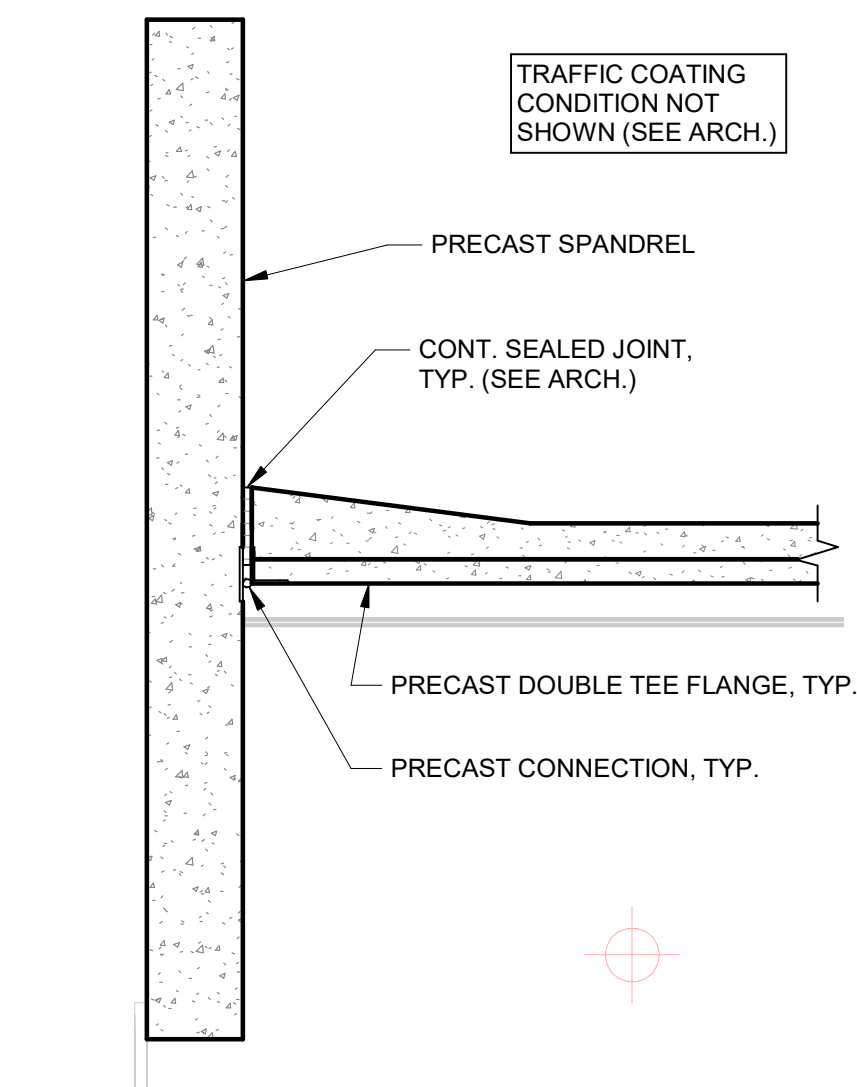
3 TYPICAL SECTION AT INVERTED TEE GIRDER
S4.01 3/4" = 1'-0"



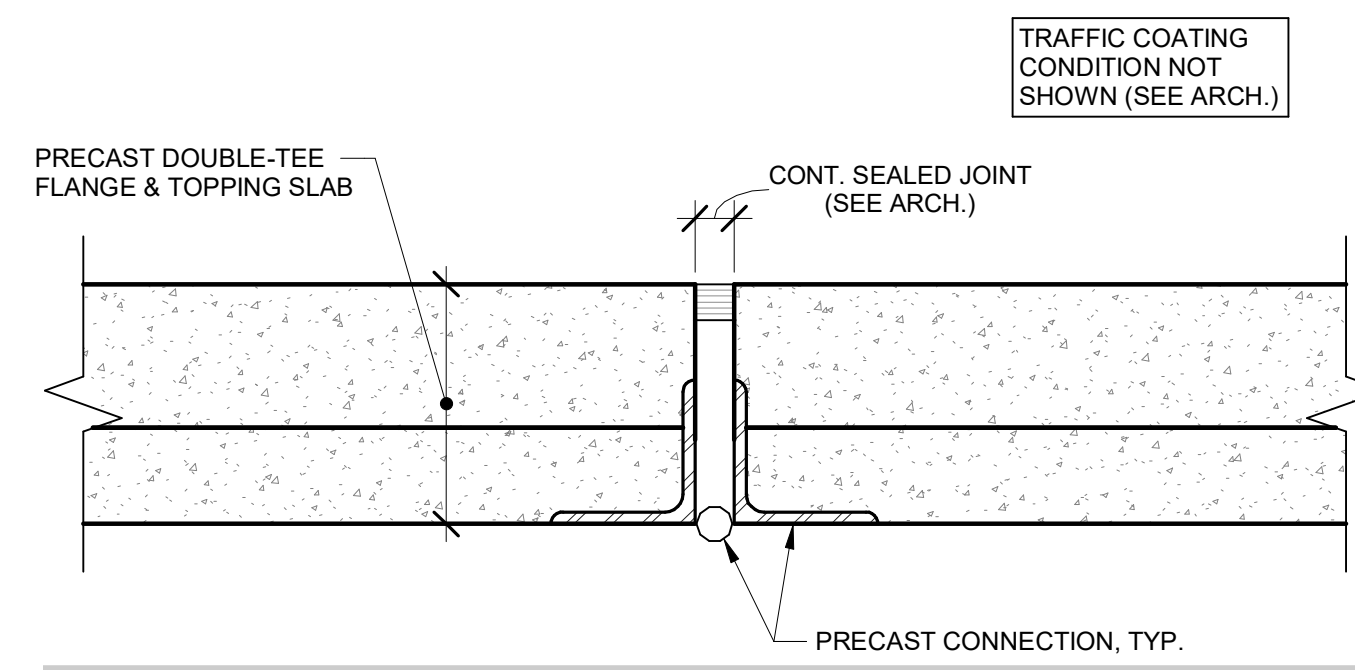
4 TYPICAL SECTION OF DOUBLE TEE
SUPPORT AT SPANDREL
S4.01 3/4" = 1'-0"



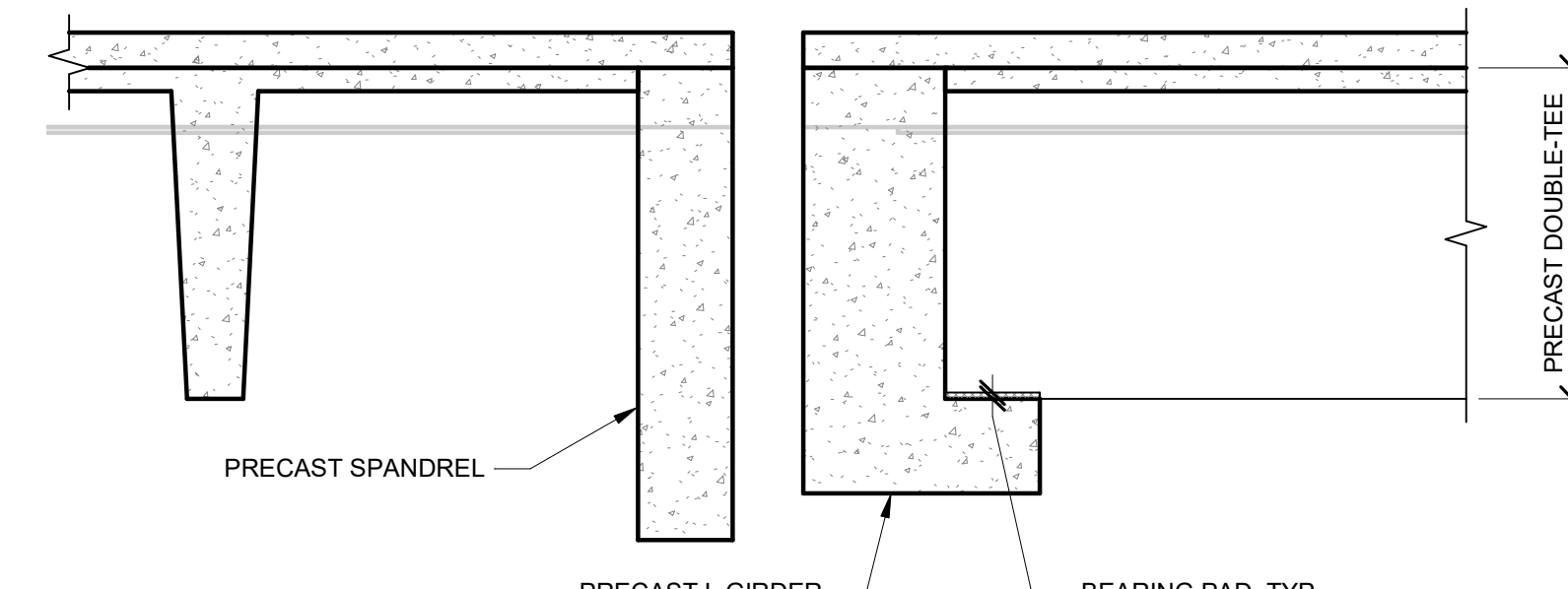
5 TYPICAL SECTION OF DOUBLE TEE FLANGE AT SHEARWALL
S4.01 3/4" = 1'-0"



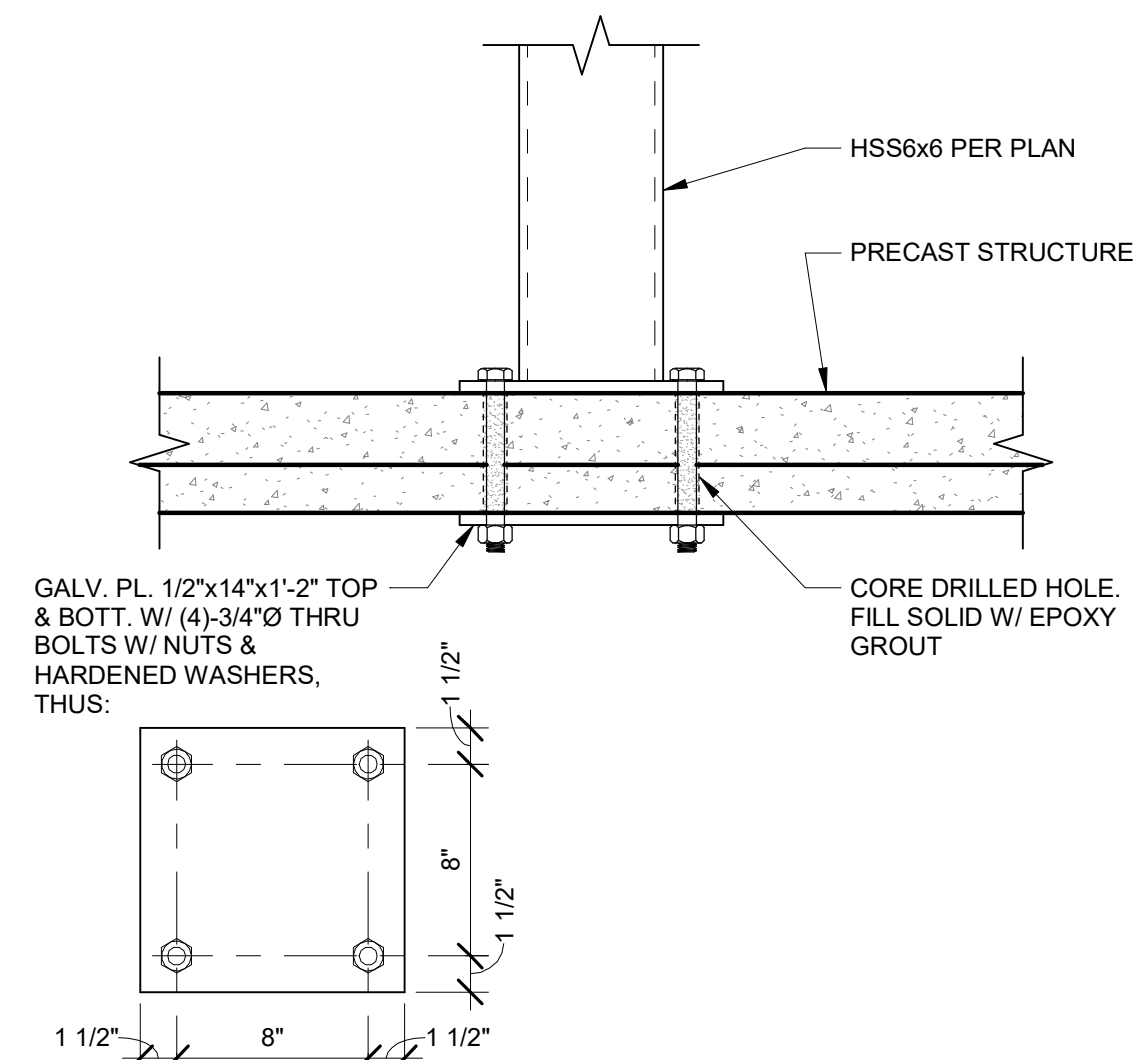
6 TYPICAL SECTION OF DOUBLE TEE
FLANGE AT SPANDREL
S4.01 3/4" = 1'-0"



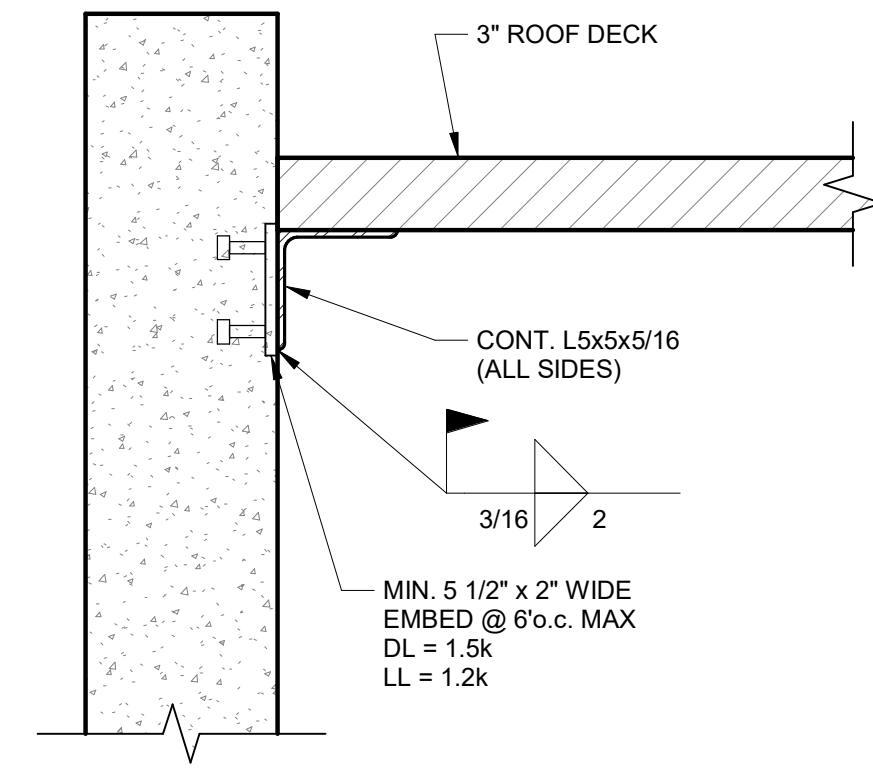
7 TYPICAL SECTION OF DOUBLE TEE FLANGE AT SPANDREL
S4.01 3" = 1'-0"



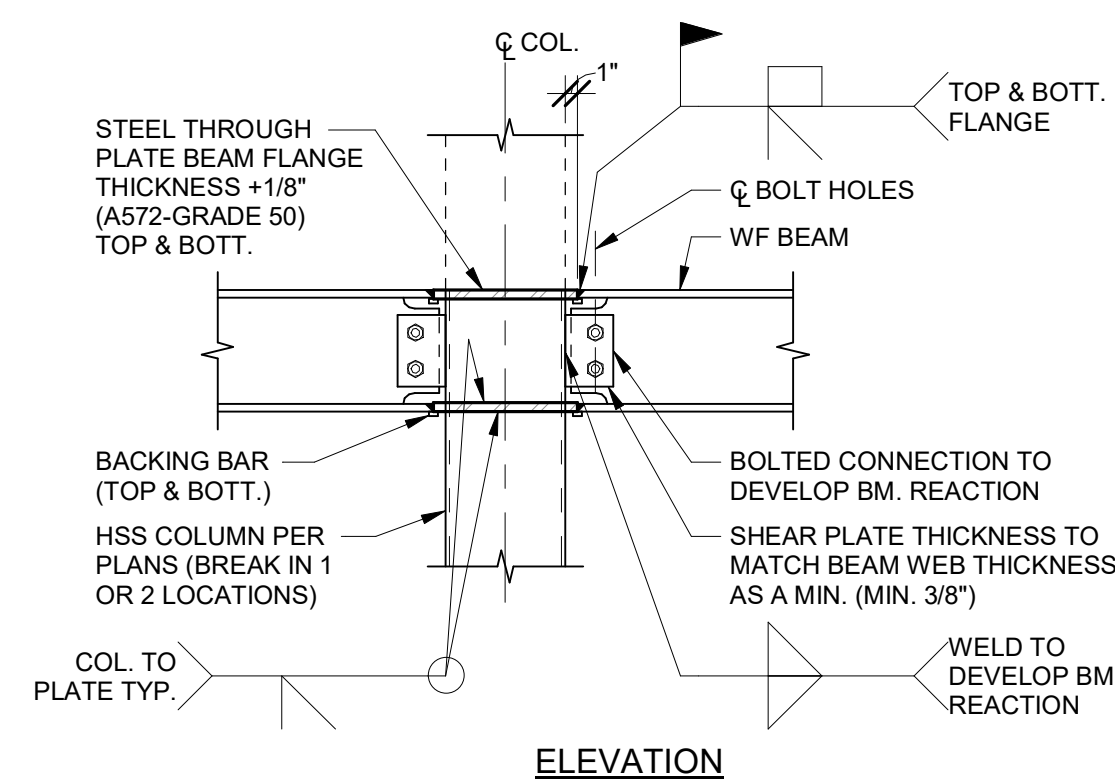
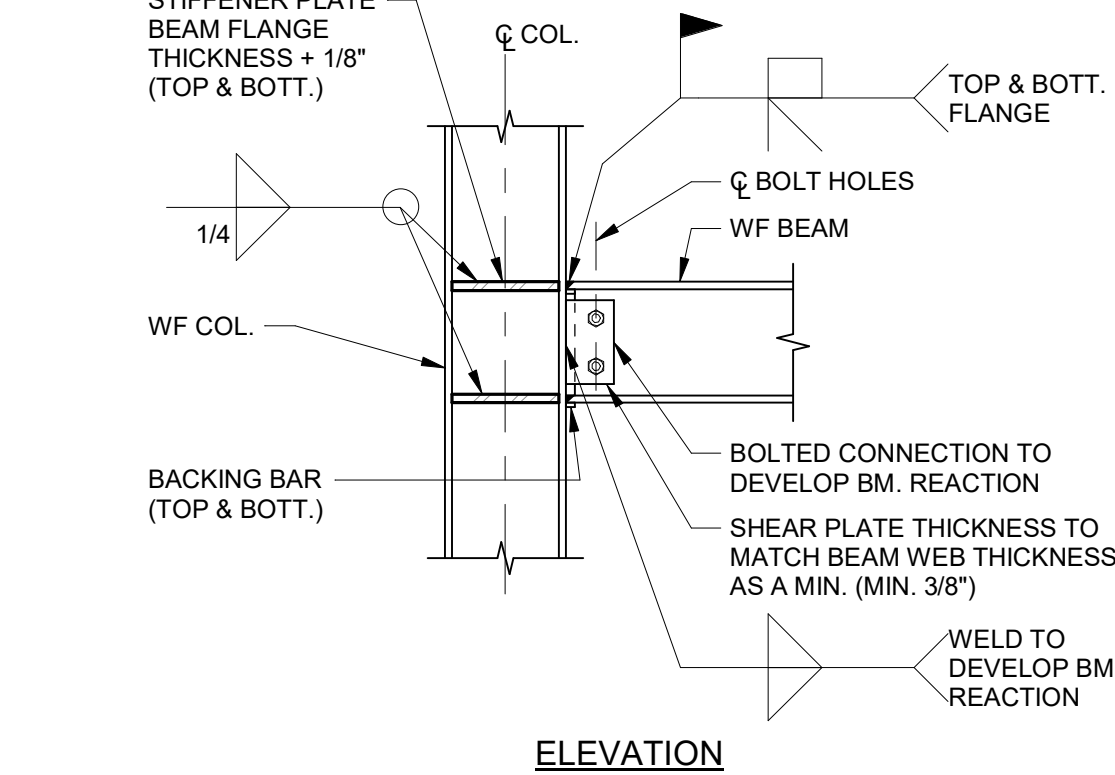
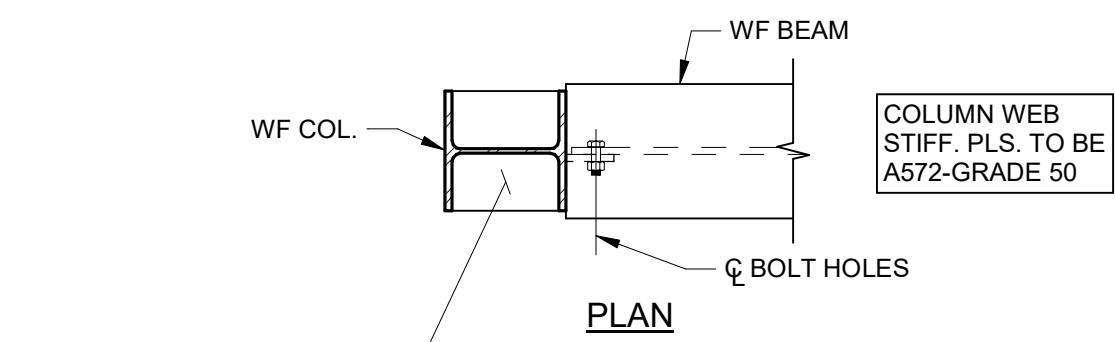
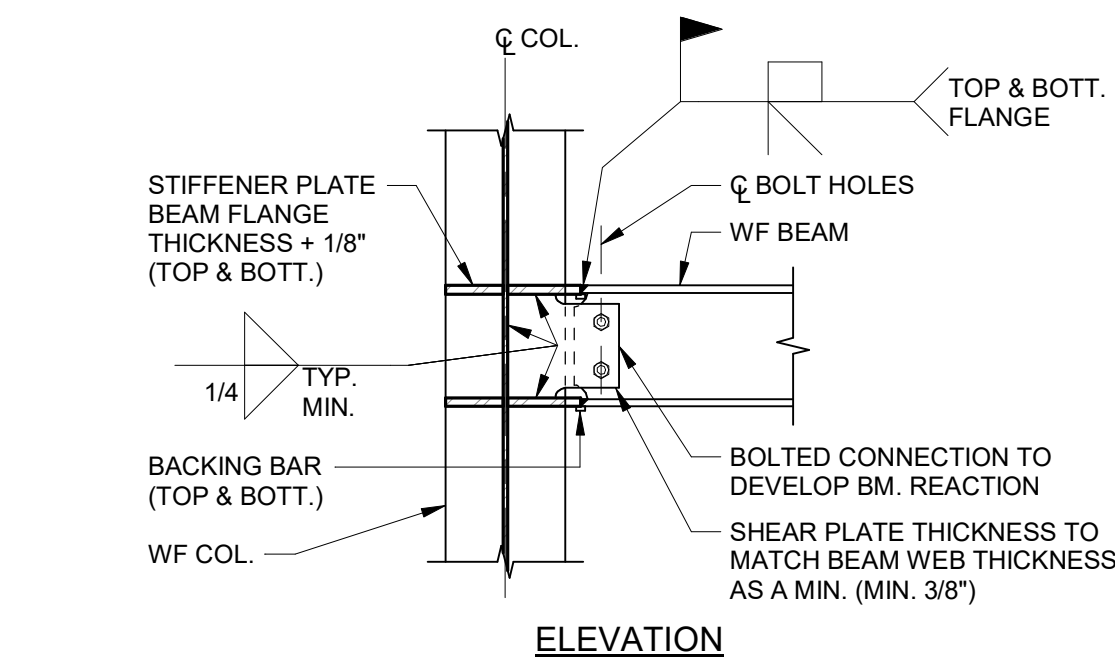
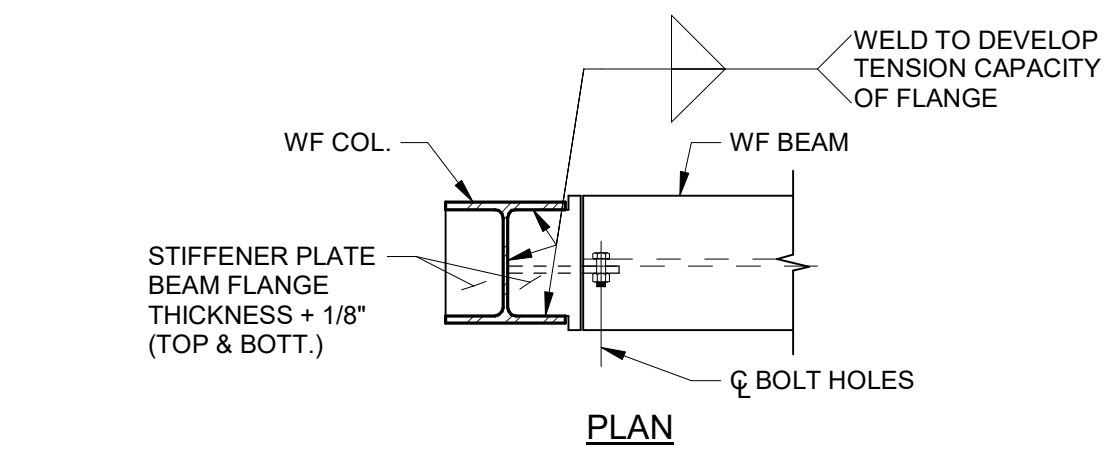
8 TYPICAL SECTION AT INVERTED TEE GIRDER
S4.01 3/4" = 1'-0"



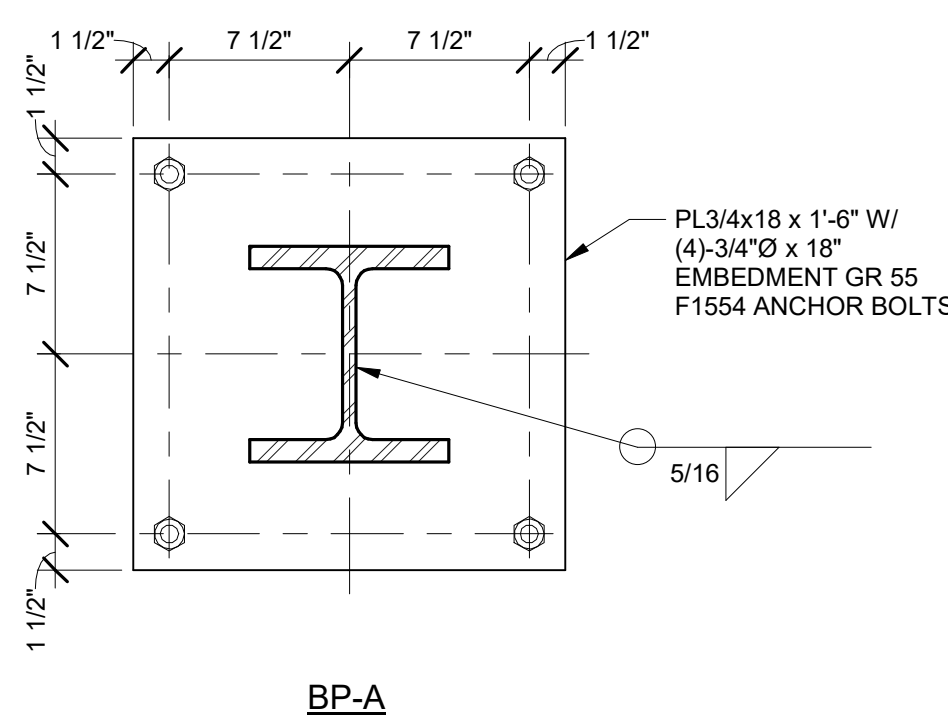
9 STEEL POST BASE PLATE
S4.01 1 1/2" = 1'-0"



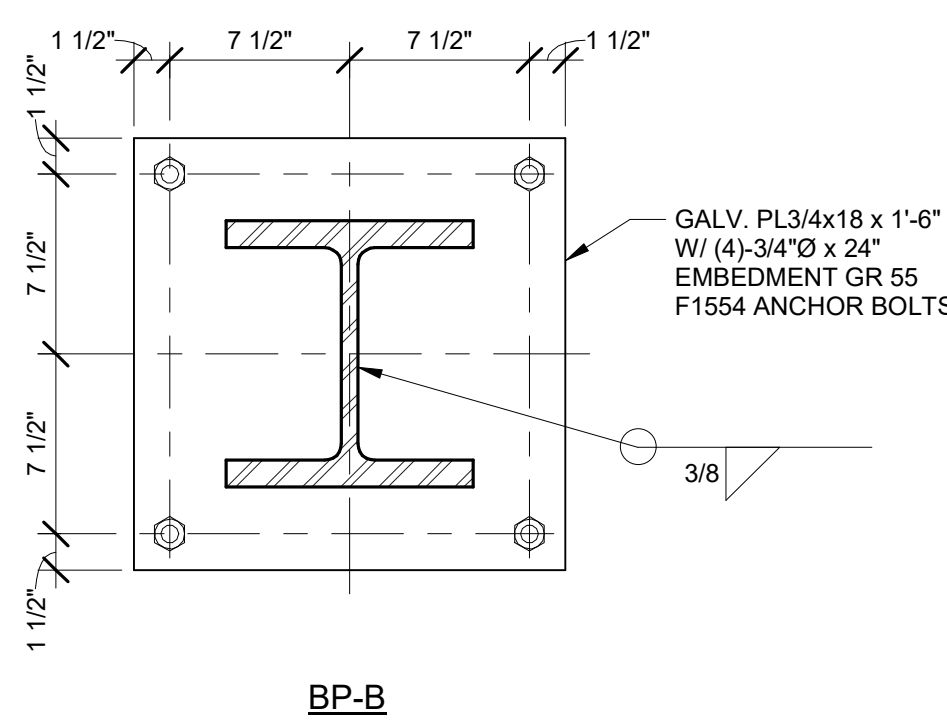
10 3" DECK SUPPORT
S4.01 1 1/2" = 1'-0"



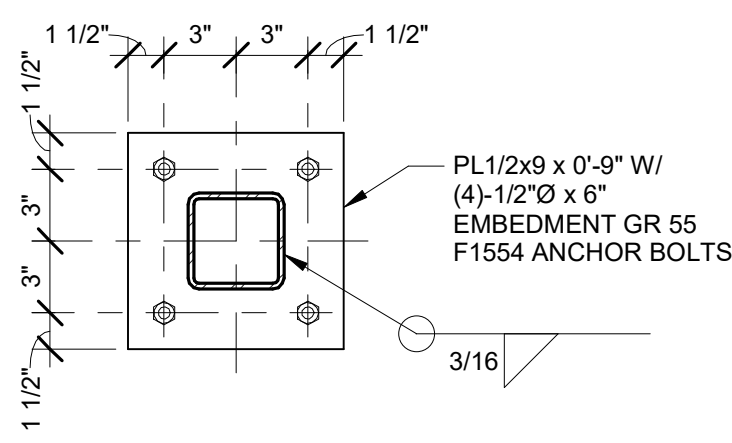
1 COLUMN TO BEAM MOMENT CONNECTION DETAIL
S5.00 3/4" = 1'-0"



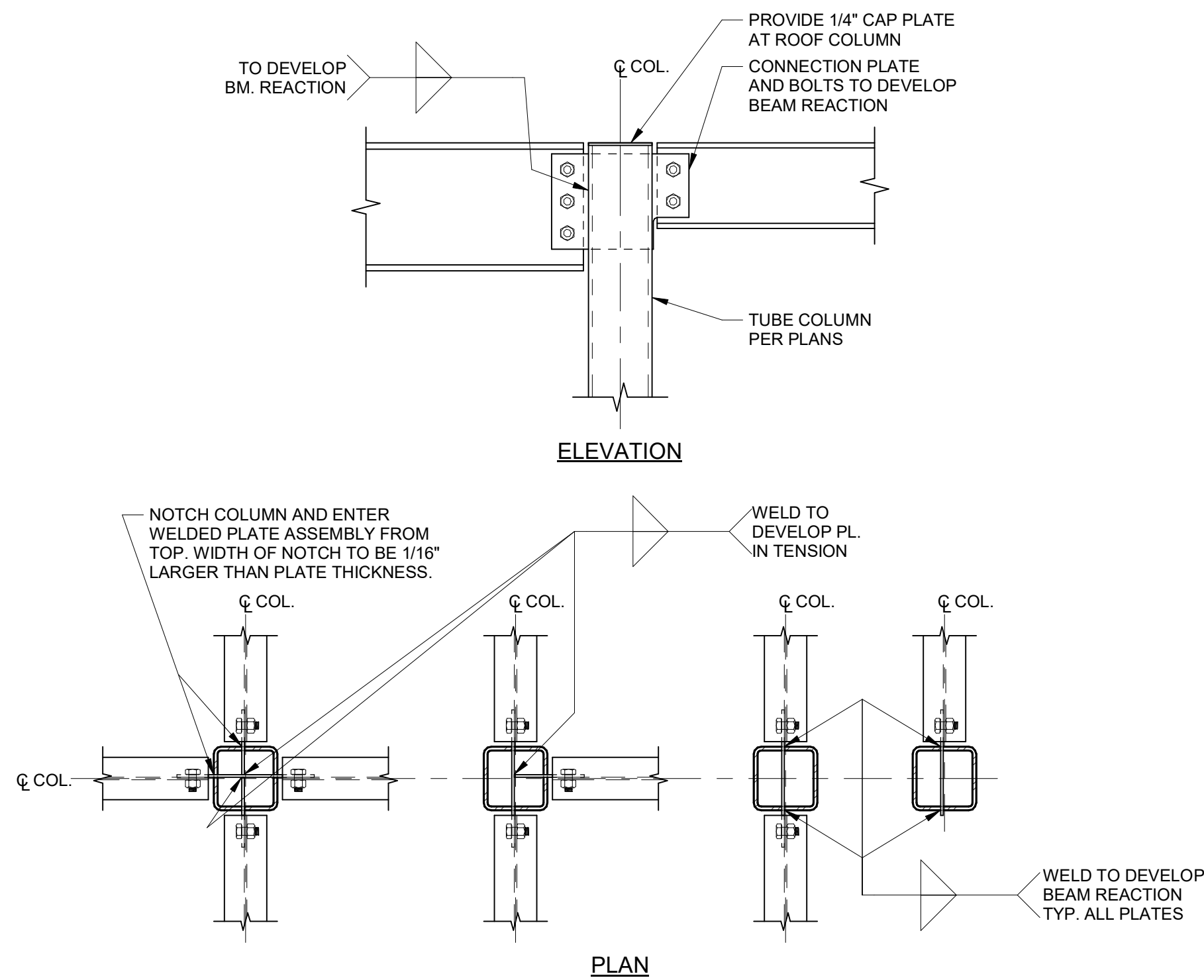
8 BASE PLATE DETAILS
S5.00 1 1/2" = 1'-0"



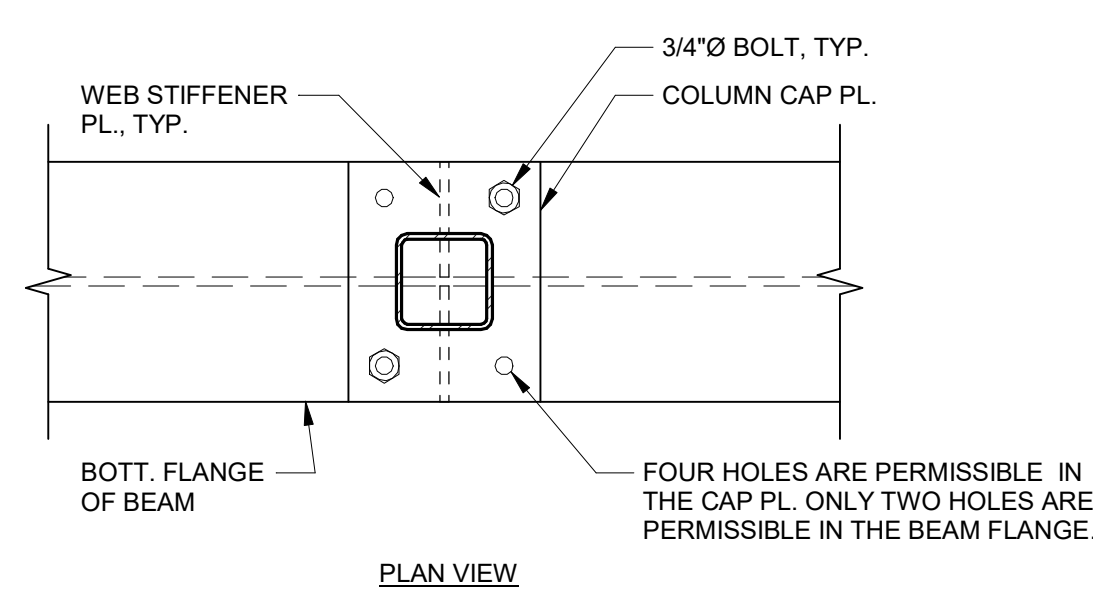
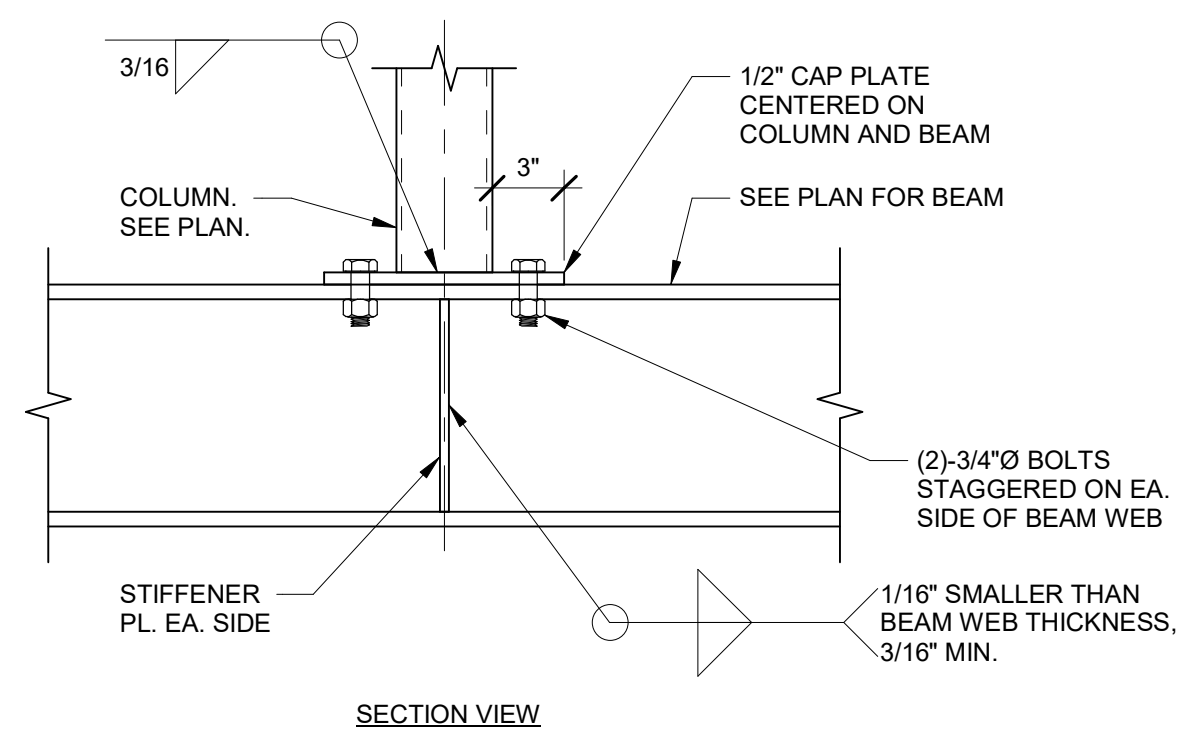
BP-B



BP-C

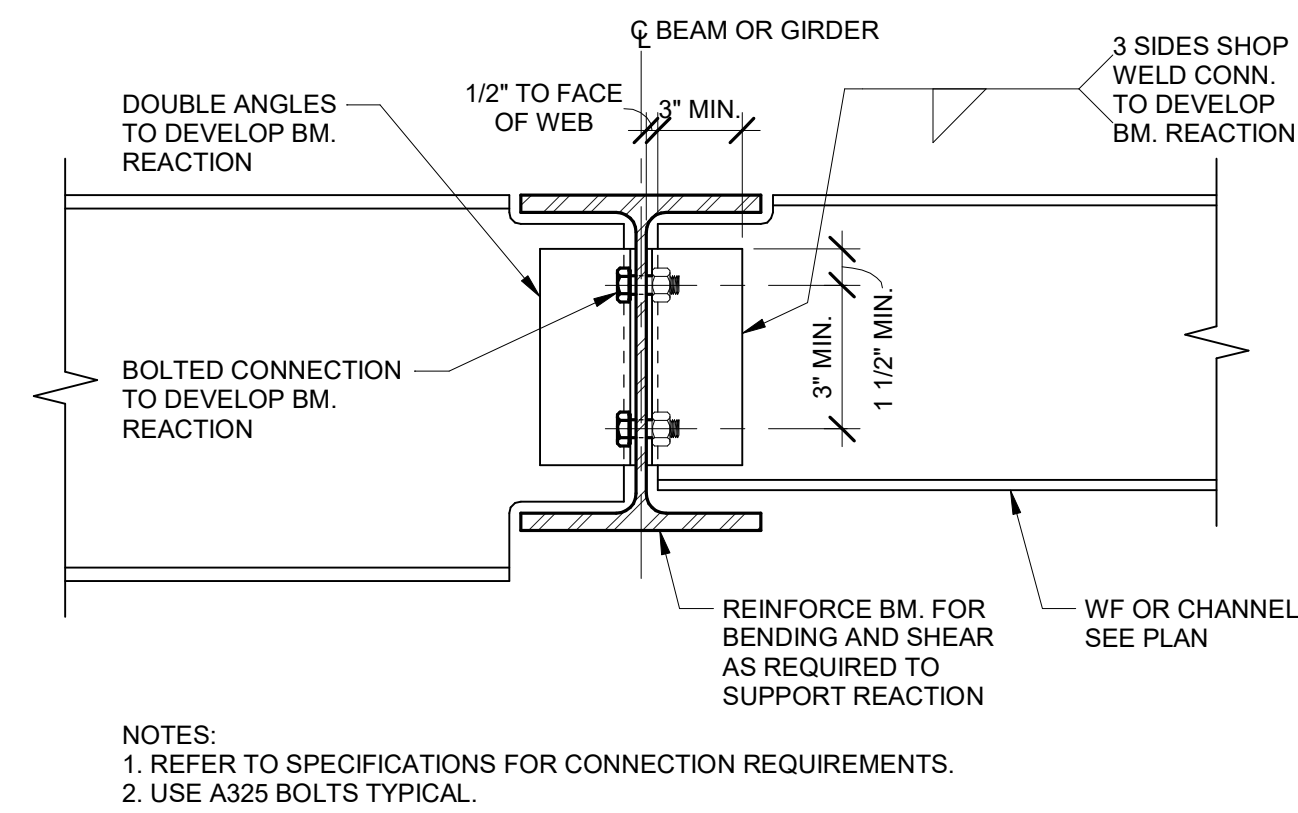


2 STEEL WIDE FLANGE BEAM CONNECTION TO TUBE COLUMNS
S5.00 1" = 1'-0"

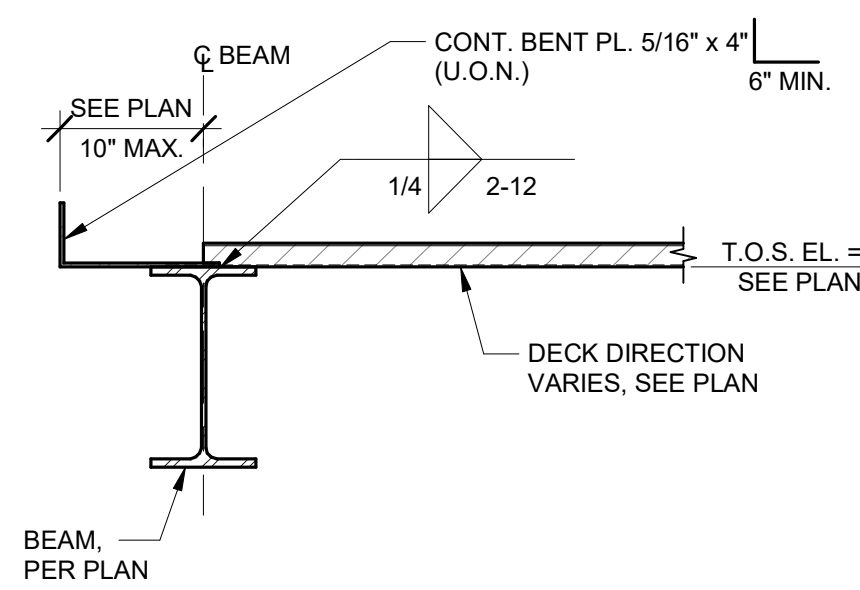


NOTES
1. STIFFENER PLATE SHALL BE EQUAL IN THICKNESS TO THE COLUMN WALL THICKNESS OR BEAM WEB THICKNESS, WHICHEVER IS GREATER.

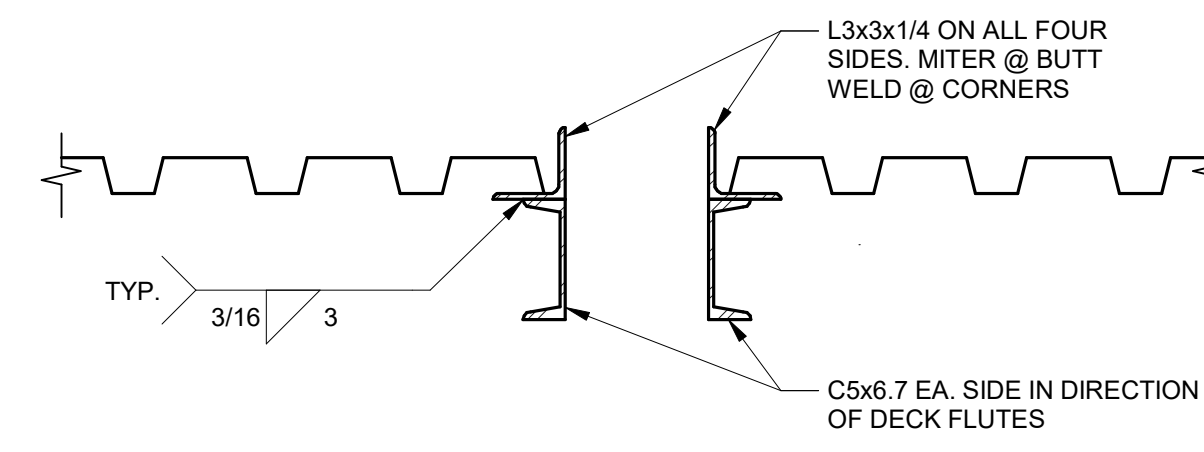
3 TYPICAL WF OR HSS POST CONNECTION DETAIL
S5.00 1 1/2" = 1'-0"



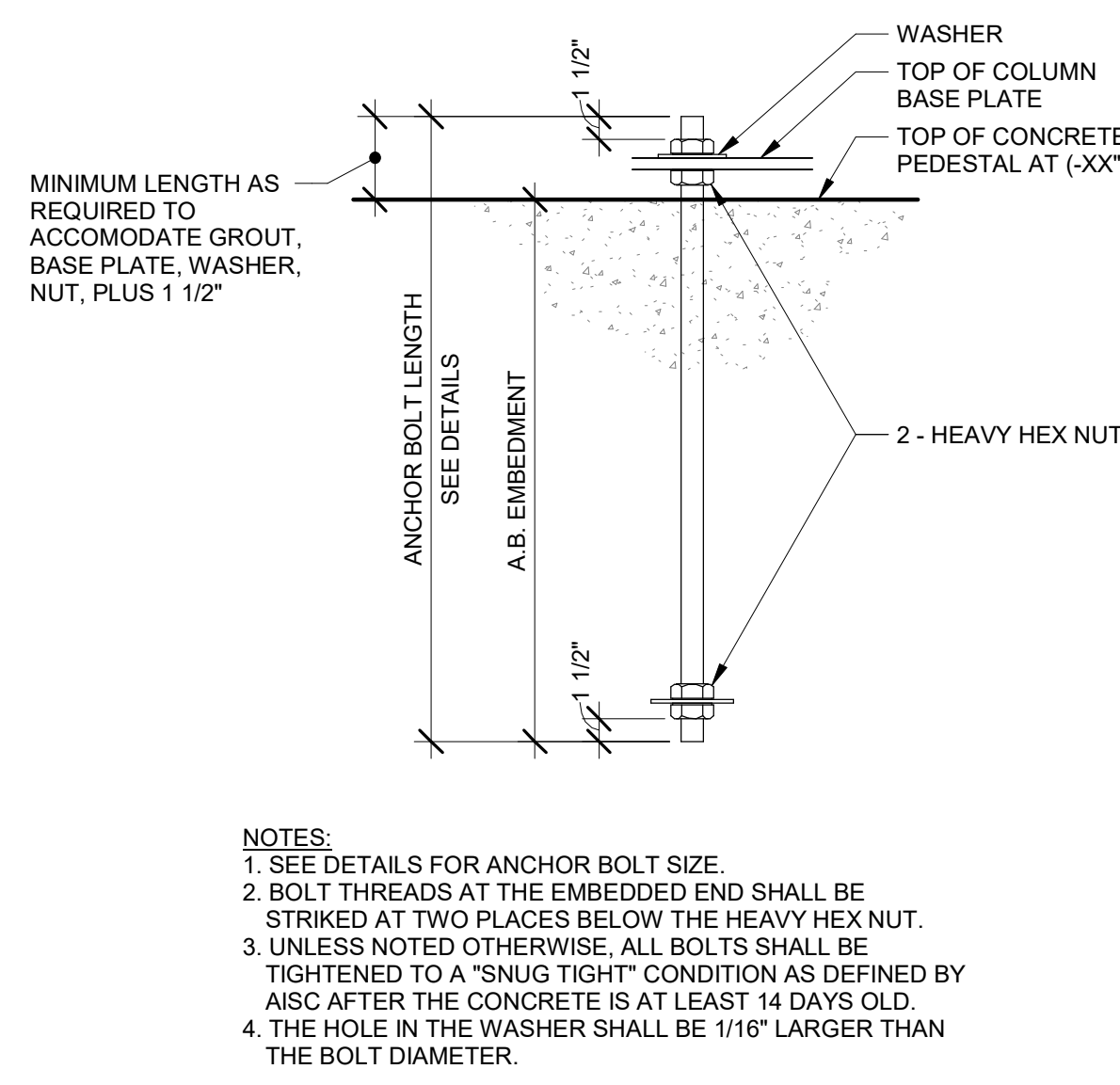
4 TYPICAL BEAM TO BEAM CONNECTION
S5.00 1 1/2" = 1'-0"



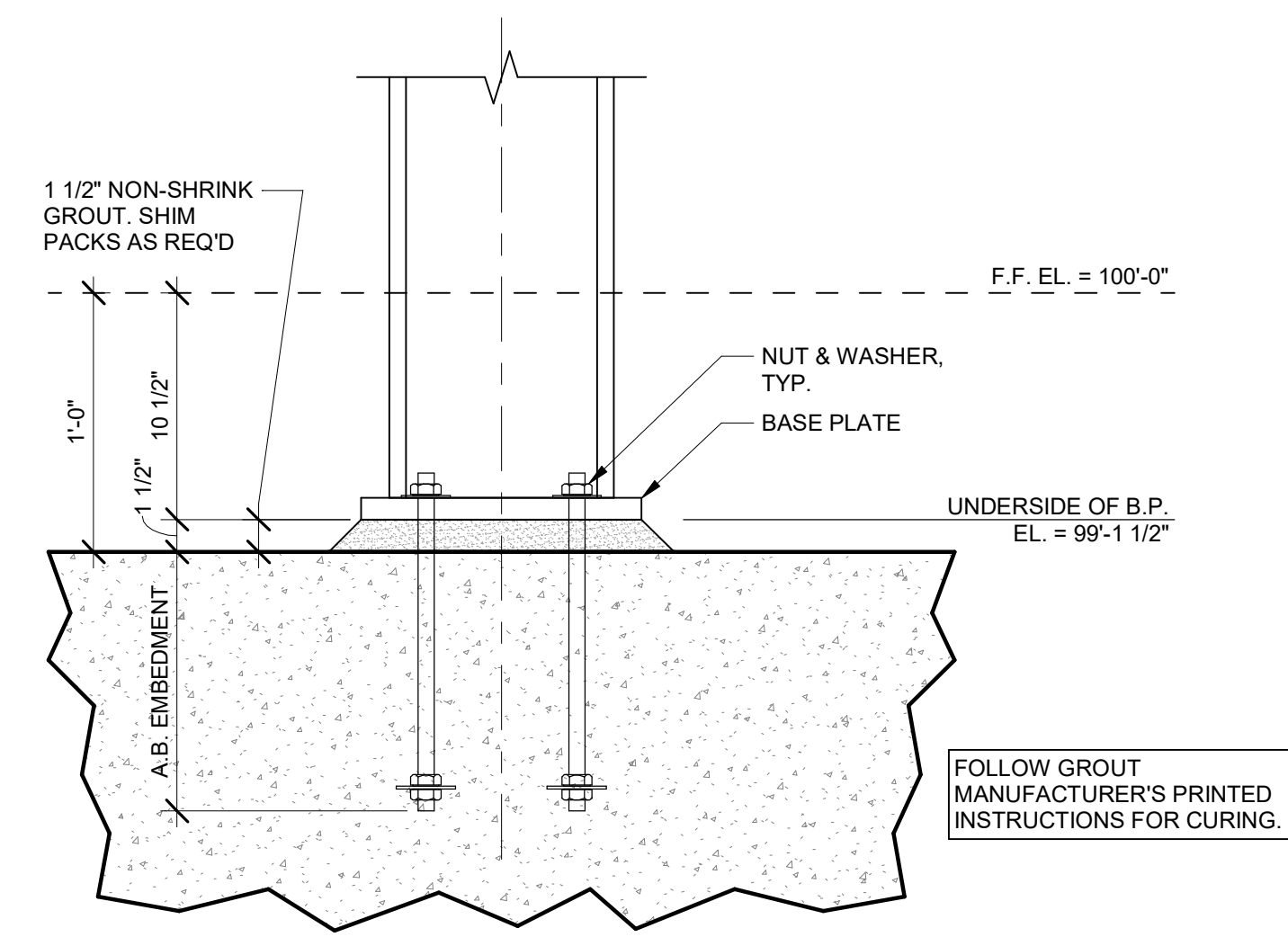
7 TYPICAL ROOF EDGE
S5.00 1" = 1'-0"



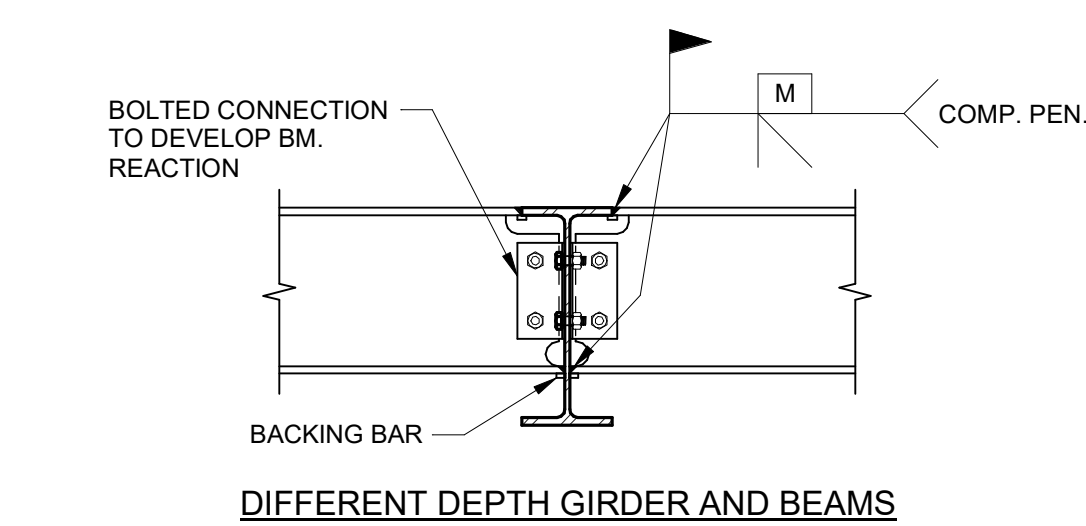
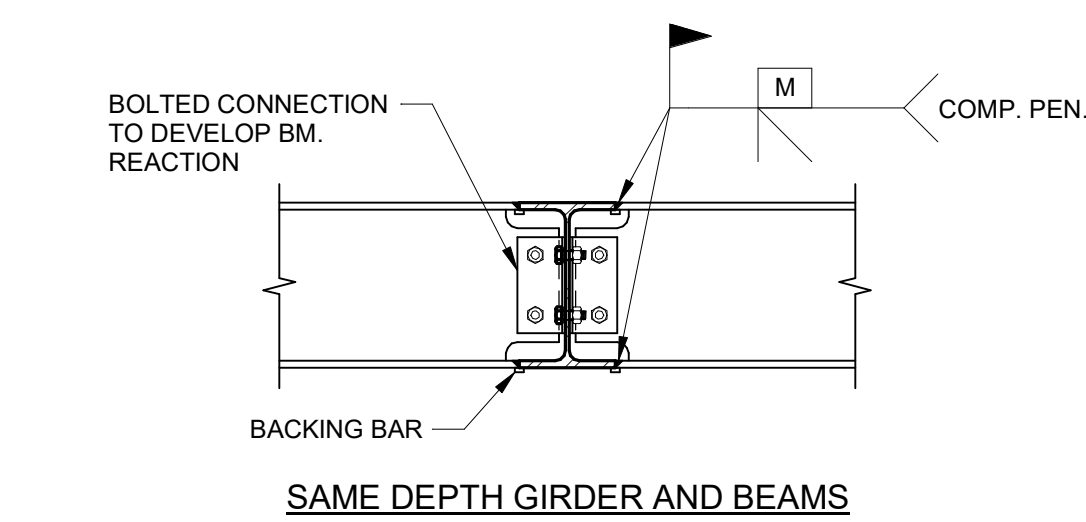
6 TYPICAL ROOF DRAIN SUPPORT
S5.00 1 1/2" = 1'-0"



9 ANCHOR BOLT DETAIL
S5.00 1" = 1'-0"



10 TYPICAL BASE PLATE DETAIL
S5.00 1 1/2" = 1'-0"



5 BEAM TO BEAM MOMENT CONNECTION DETAIL
S5.00 3/4" = 1'-0"

DATE	NO.	ISSUE

TYLER STREET PARKING GARAGE
619 WEST 12TH AVENUE, CONVINGTON, LA 70433
AND
ST. TAMMANY HEALTH SYSTEM
NORTH LAKE SURGICAL ASSOCIATES
1028 SOUTH TYLER STREET, CONVINGTON, LA 70433
AT
ST. TAMMANY PARISH HOSPITAL

CONSTRUCTION DOCUMENTS

Date
05 NOV 2025

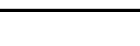
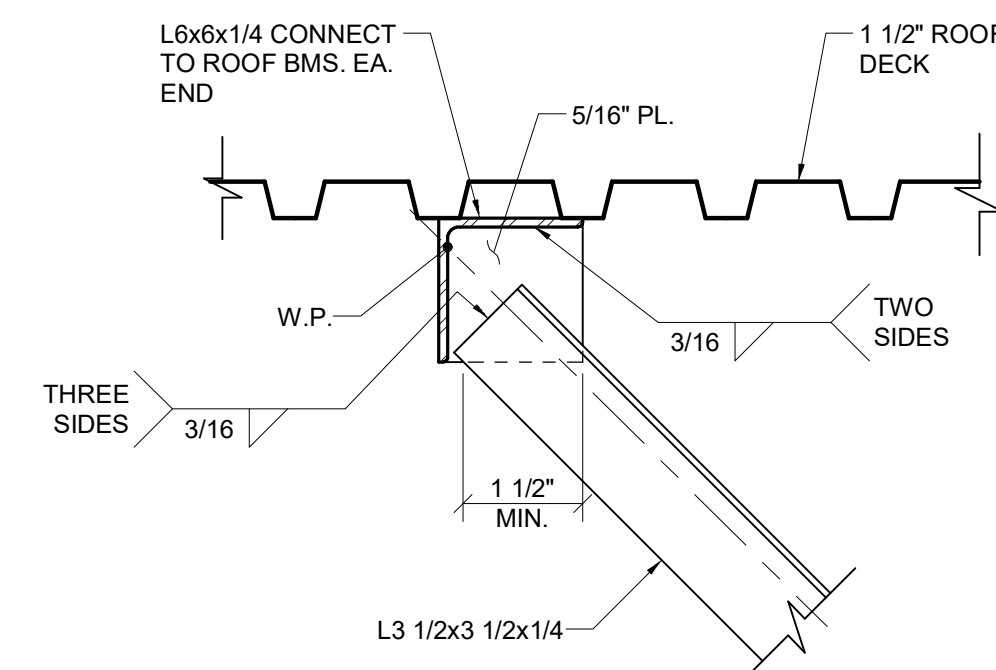
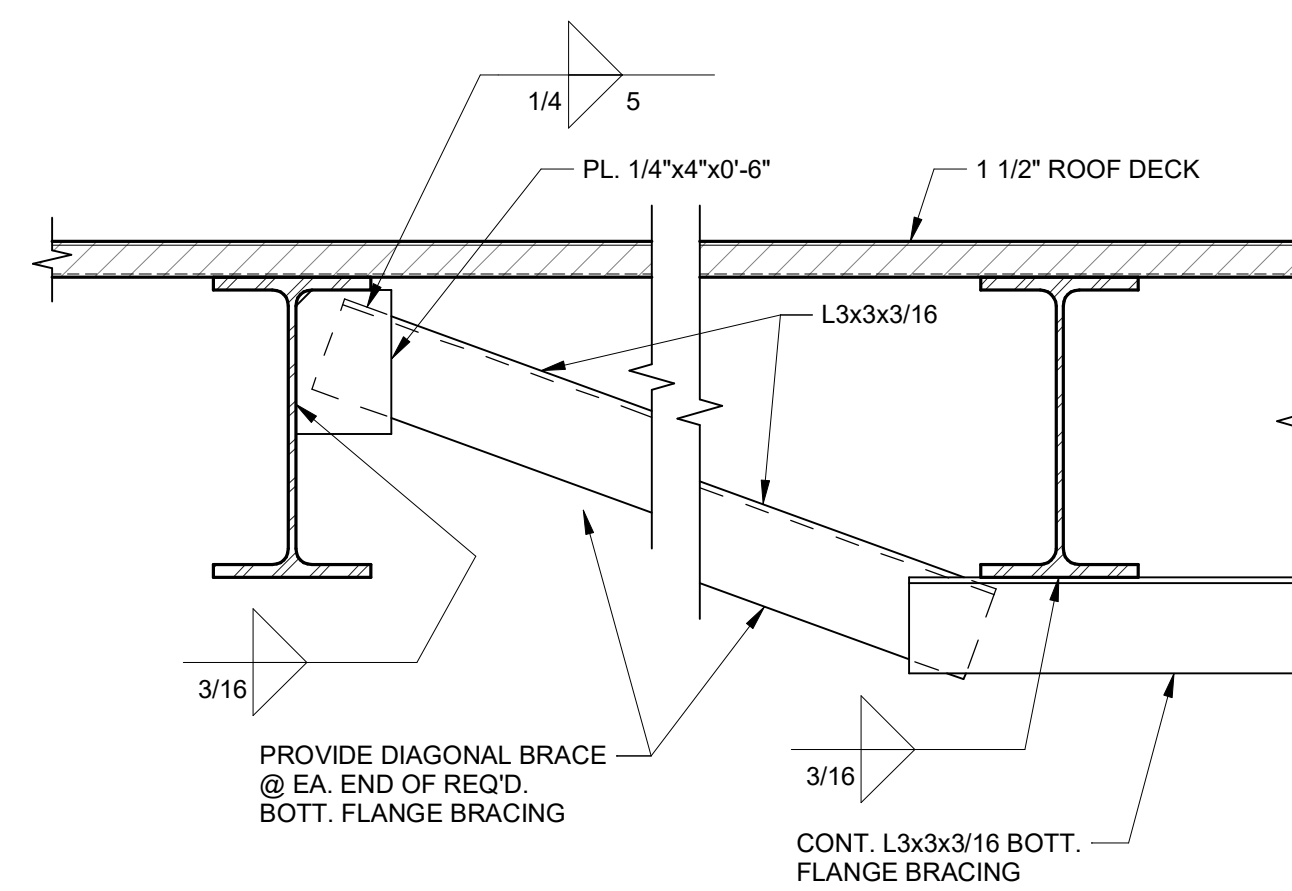
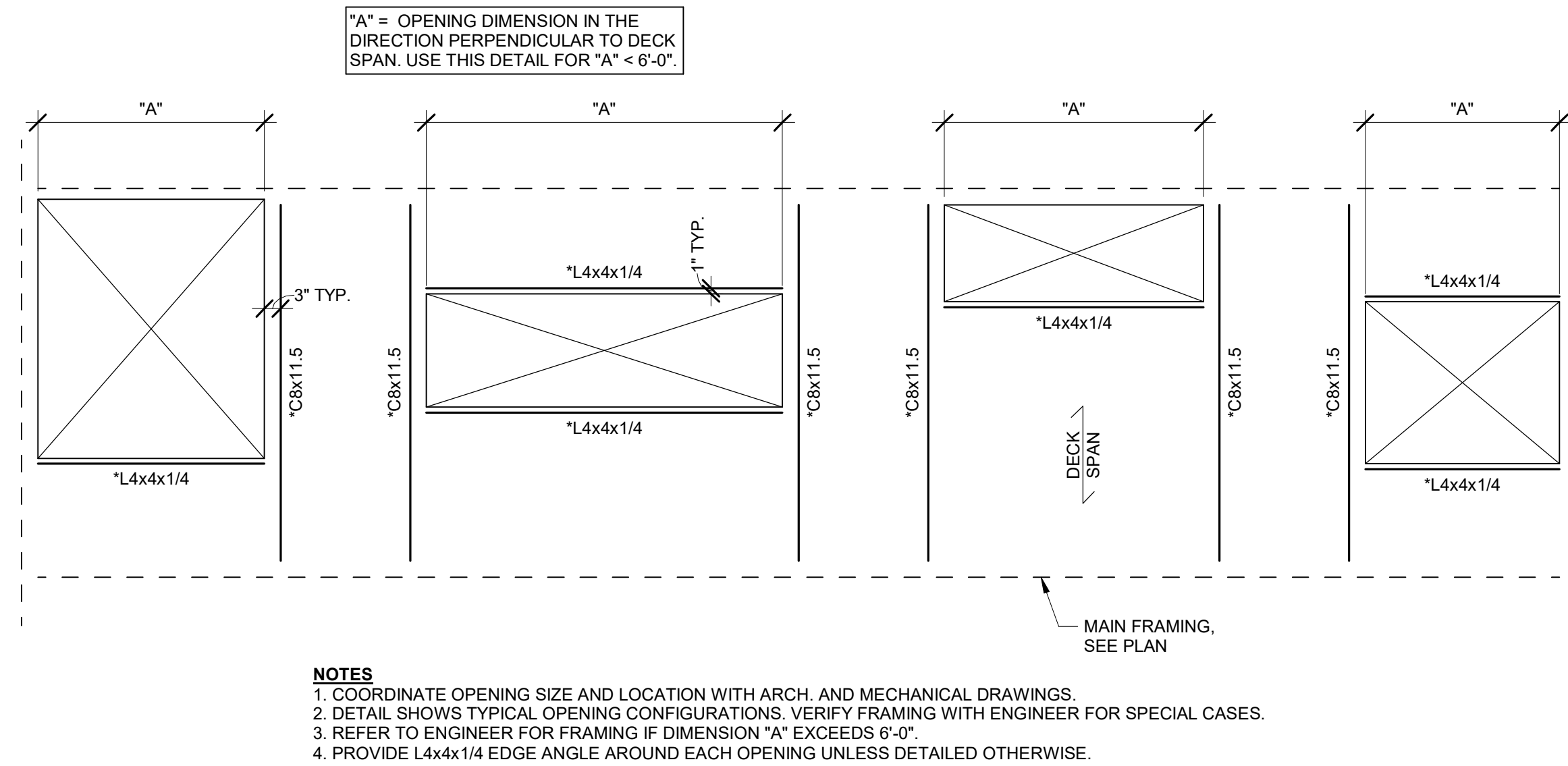
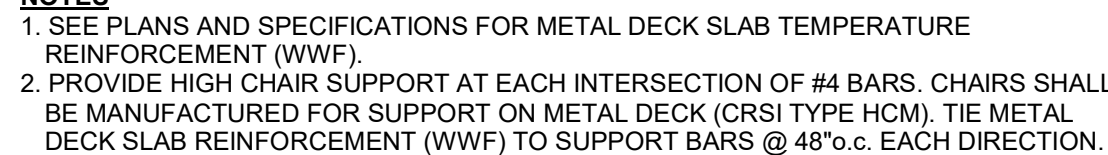
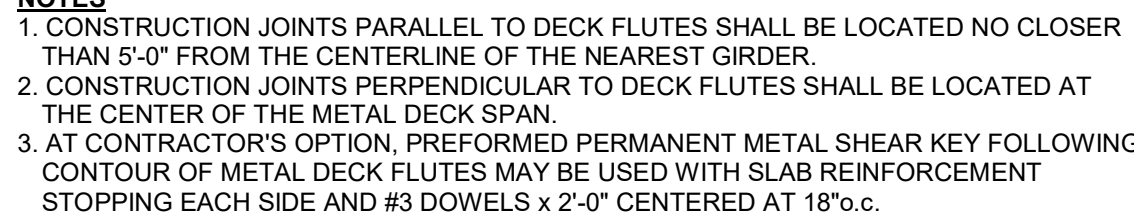
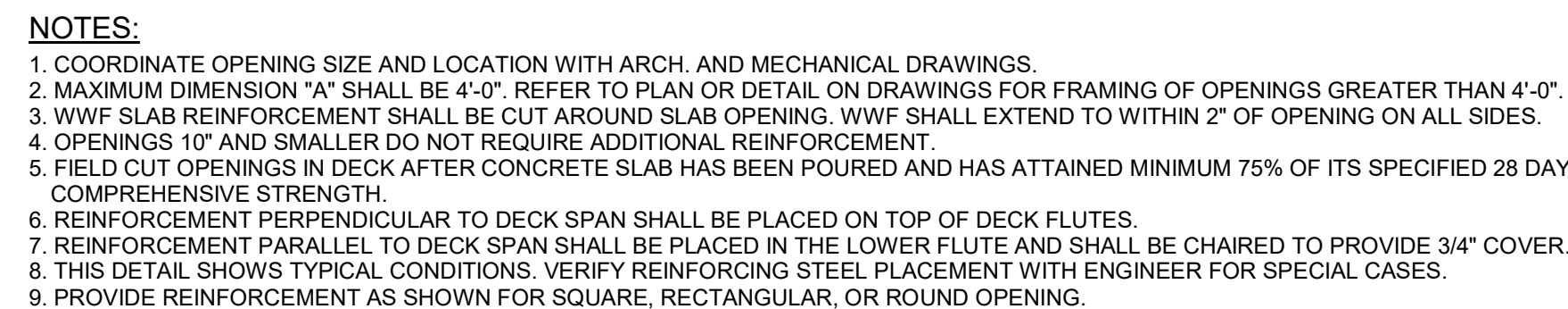
TYPICAL STEEL FRAMING DETAILS

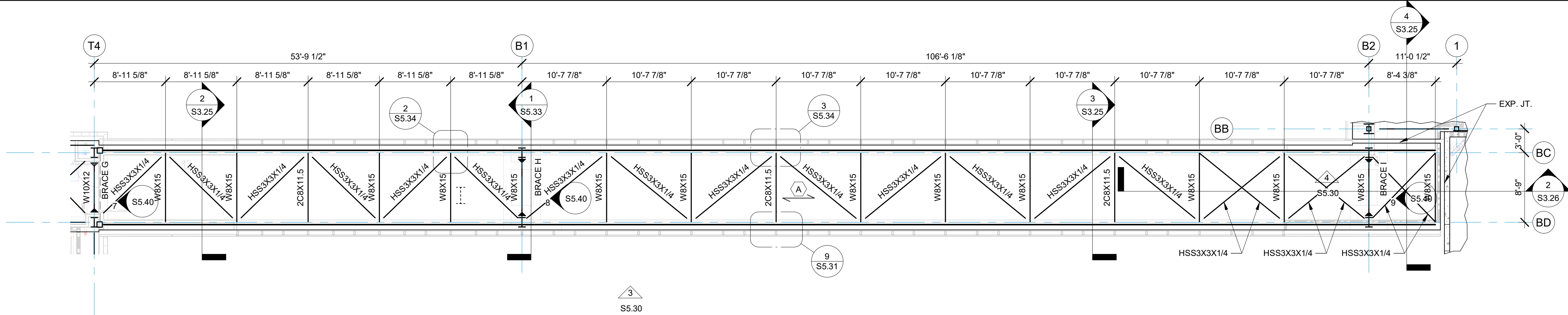
Job No. 222058.01

Drawn by
Author
Checked by
Checker

S5.00
Sheet No.

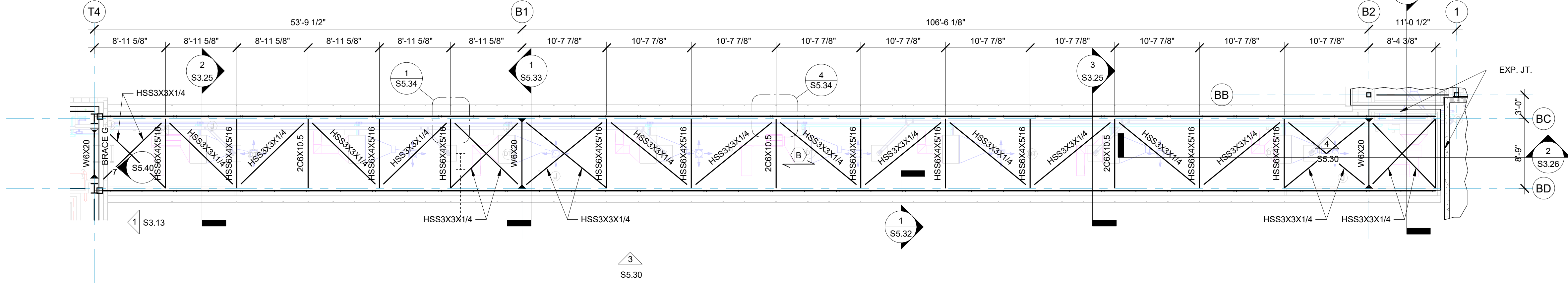
THESE DRAWINGS ARE THE SOLE PROPERTY OF FLWB ARCHITECTS, APC AND ARE ISSUED AS INSTRUMENTS OF SERVICE. THESE DRAWINGS SHALL NOT BE COPIED, REPRODUCED OR USED IN ANY MANNER WITHOUT WRITTEN CONSENT OF FLWB ARCHITECTS, APC.



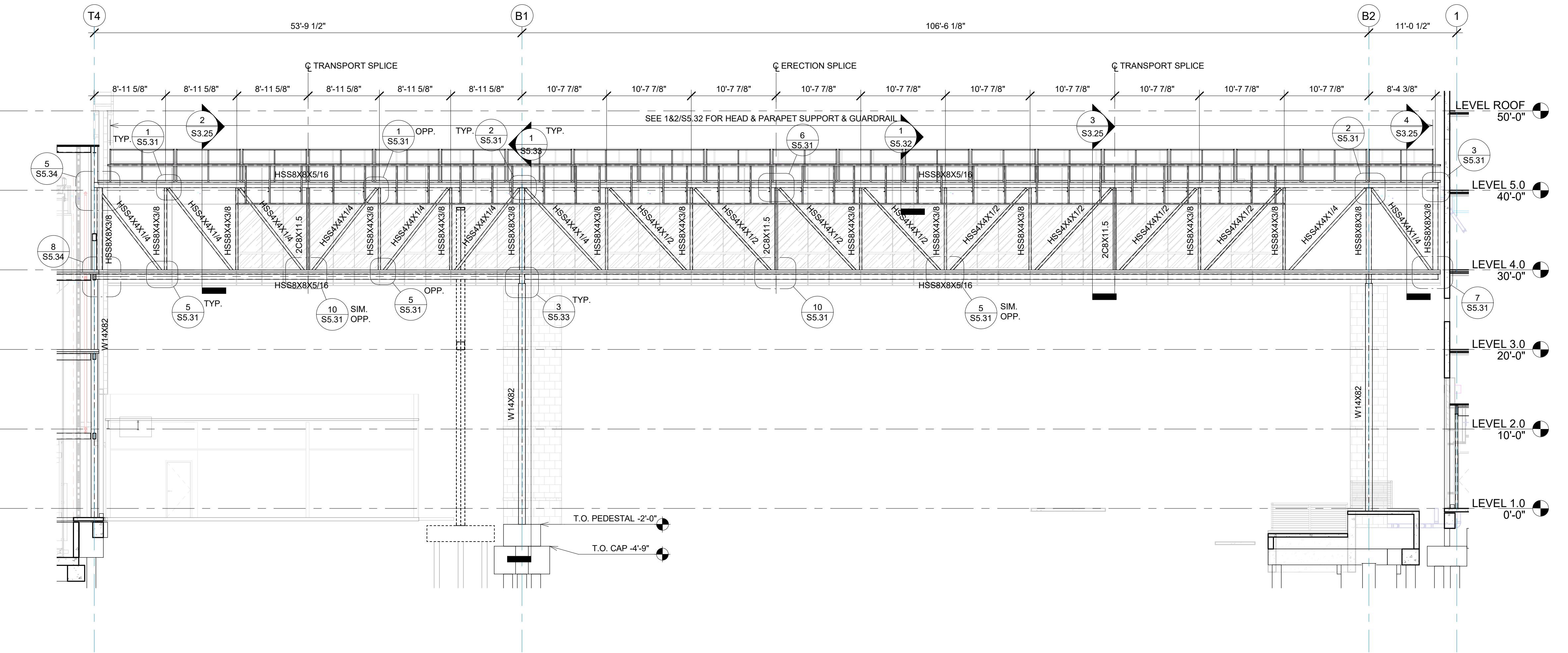


1 TRUSS FLOOR
S5.30 1/8" = 1'-0"

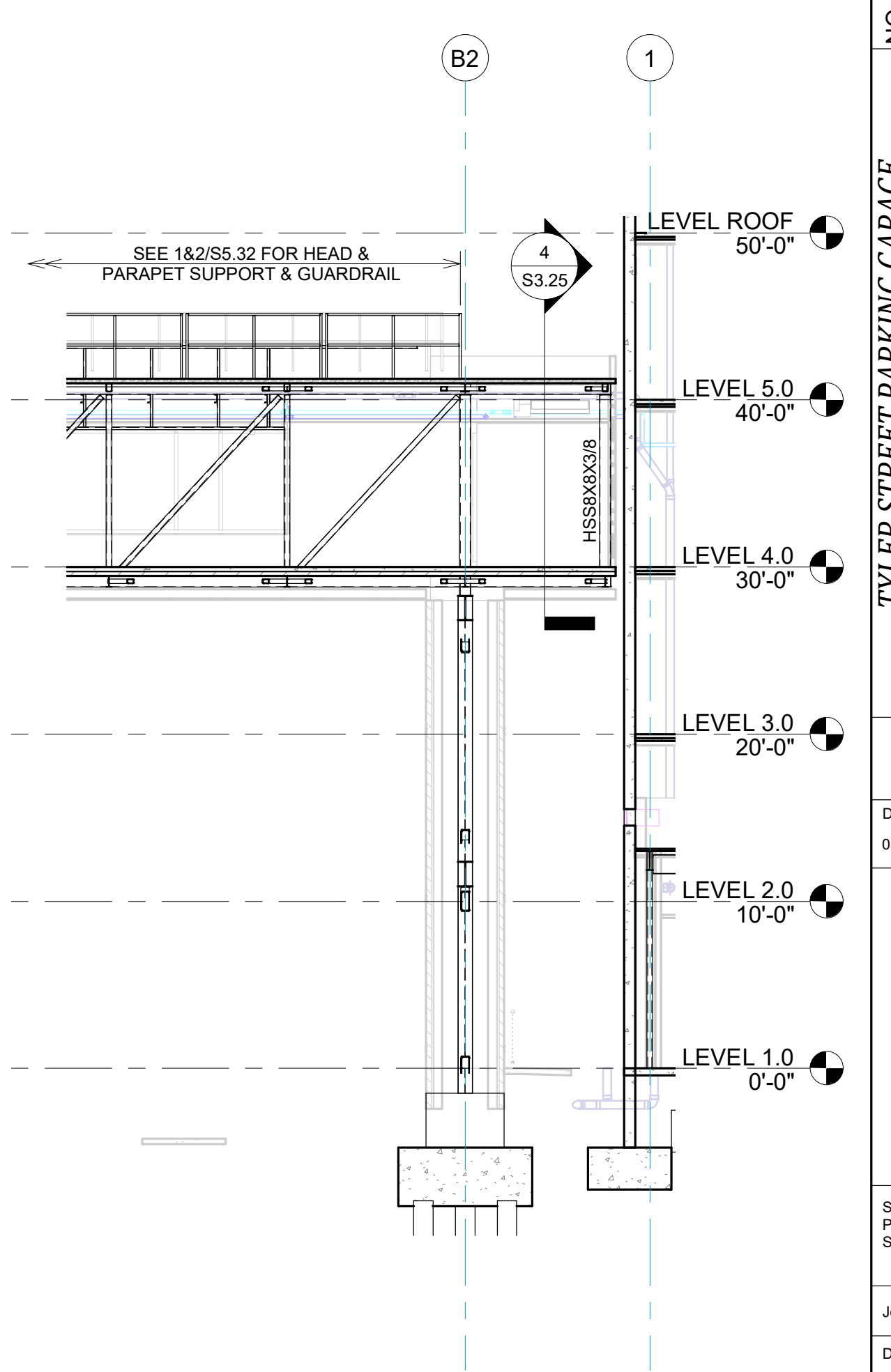
TRUSS SHALL BE FABRICATED
AND SHIPPED IN (4) PIECES,
CONNECTED ON SITE, AND
ERECTED IN (2) PIECES



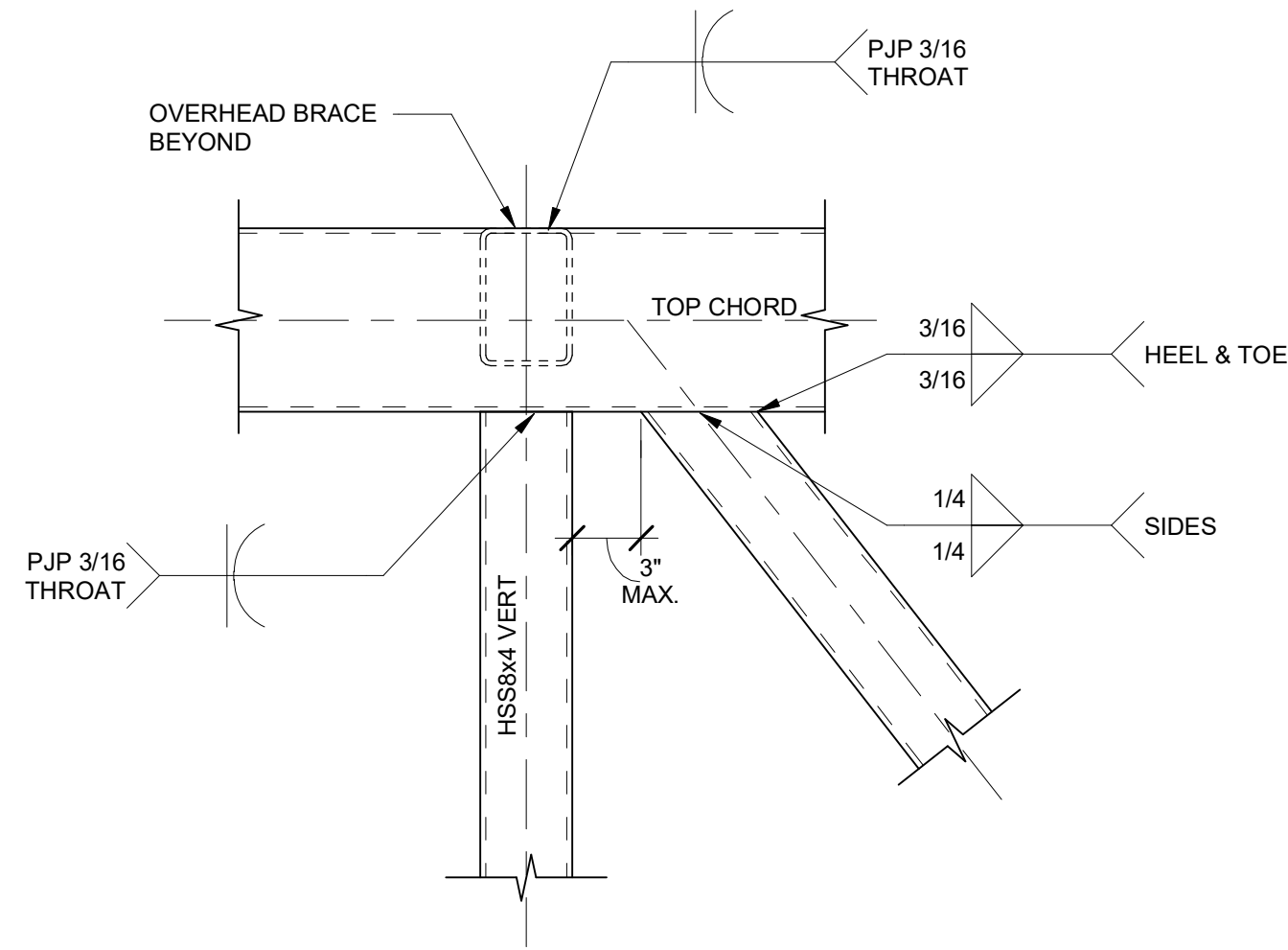
2 TRUSS ROOF
S5.30 1/8" = 1'-0"



3 SKYBRIDGE TRUSS ELEVATION
S5.30 1/8" = 1'-0"



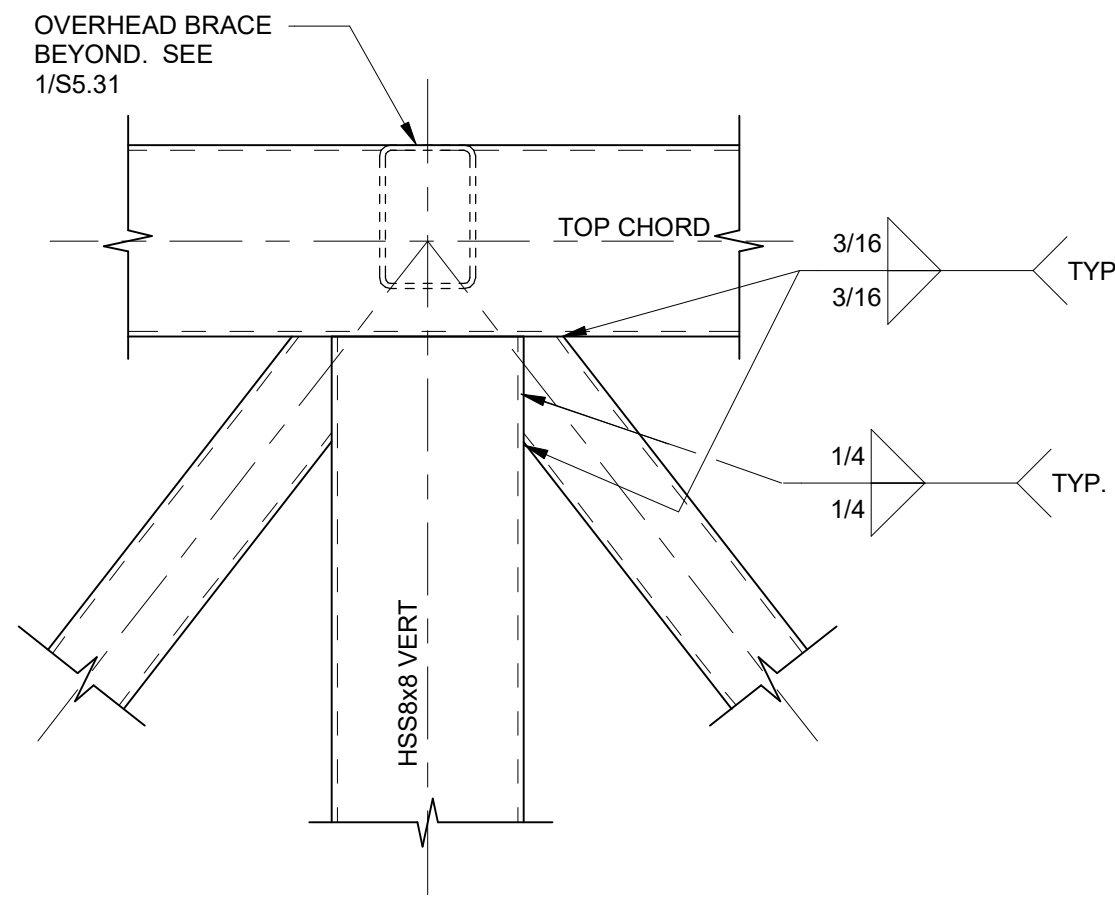
4 SKYBRIDGE TRUSS ELEVATION
S5.30 1/8" = 1'-0"



1
S5.31

DETAIL

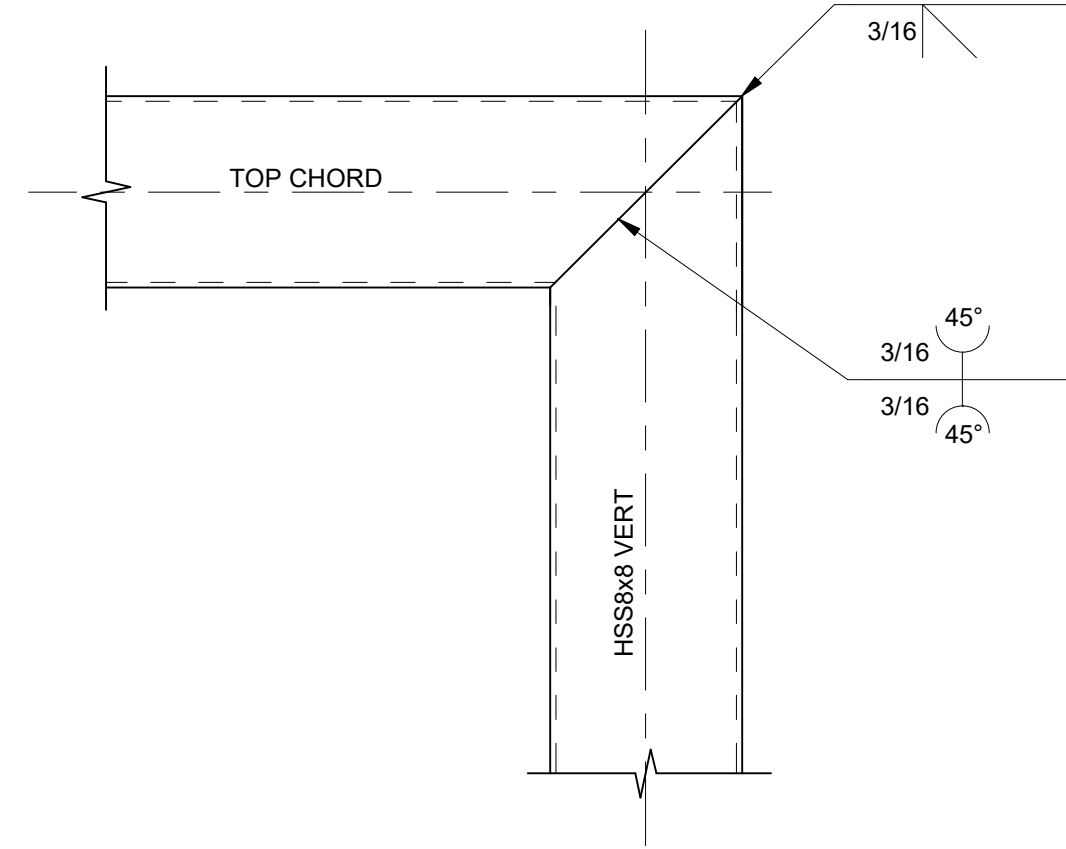
1 1/2" = 1'-0"



2
S5.31

DETAIL

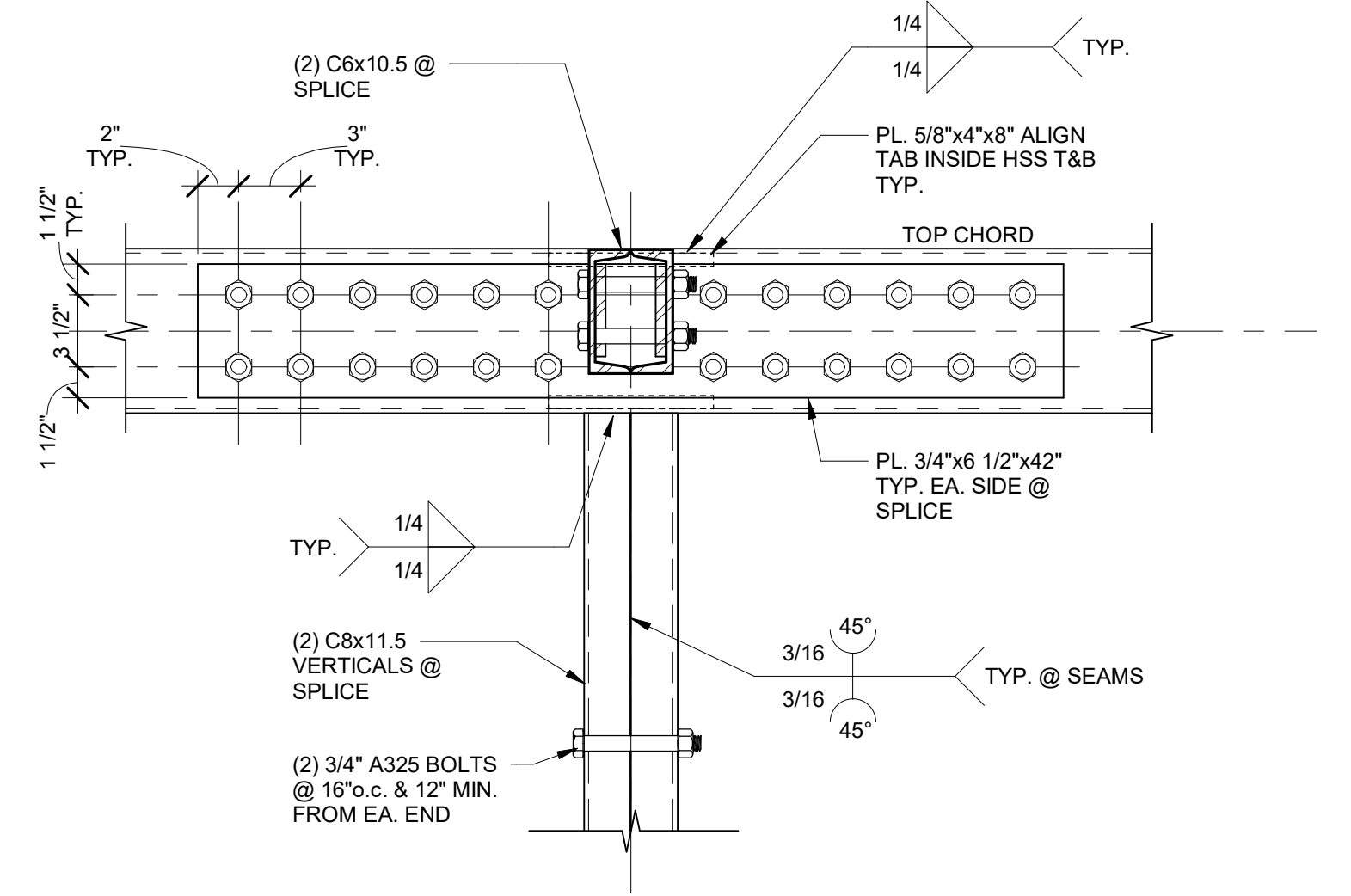
1 1/2" = 1'-0"



3
S5.31

DETAIL

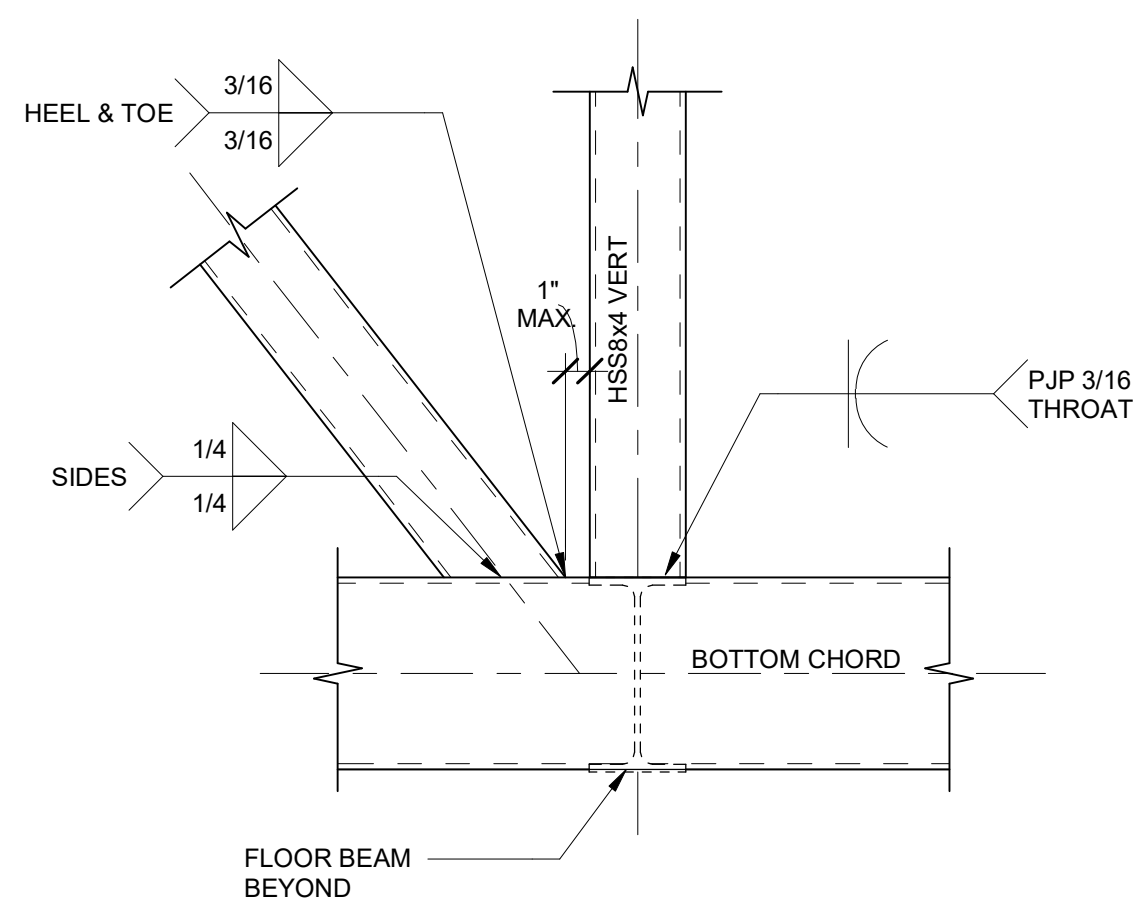
1 1/2" = 1'-0"



4
S5.31

ELEVATION - TOP CHORD @ CONSTRUCTION & TRANSPORT SPLICE

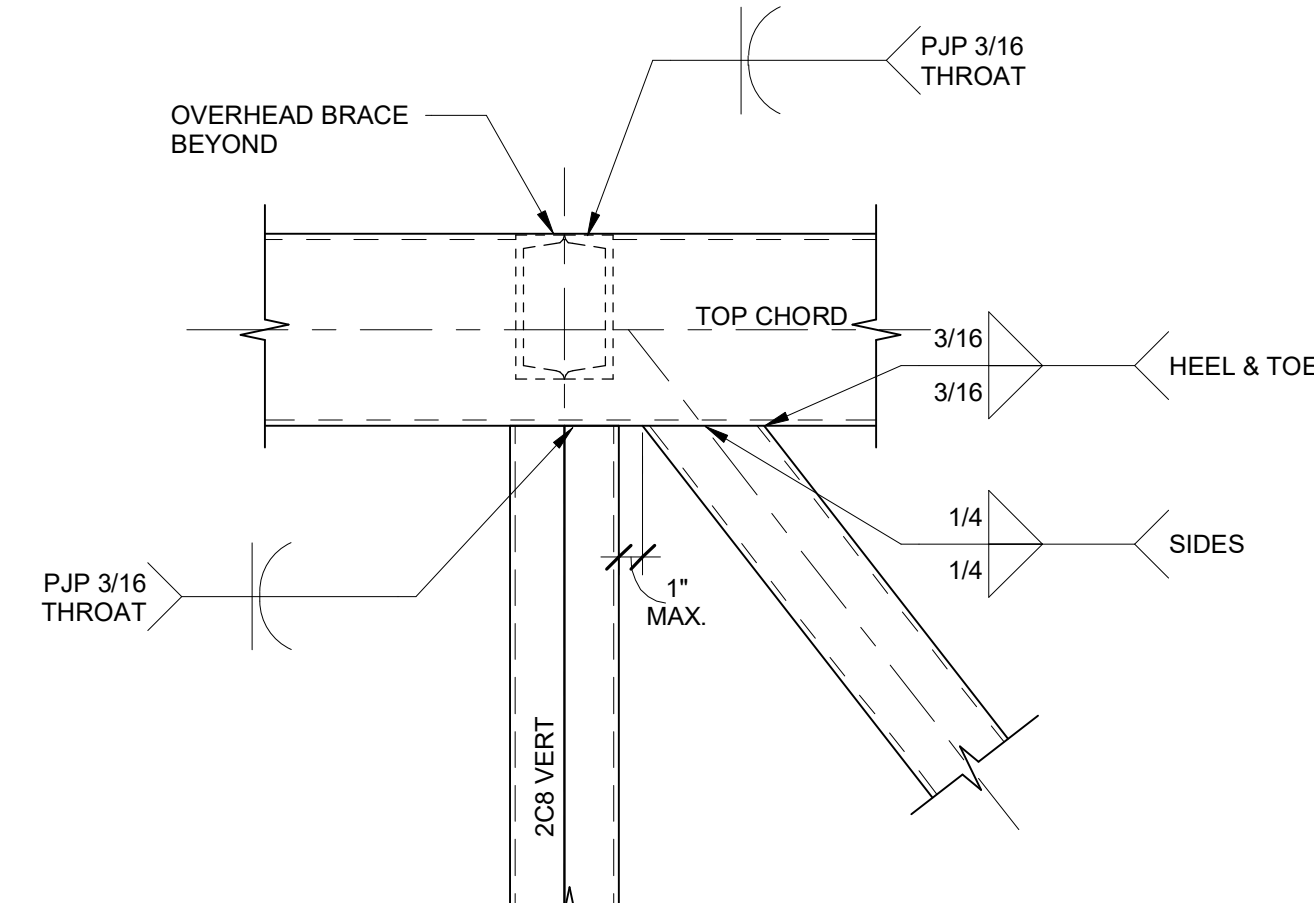
1 1/2" = 1'-0"



5
S5.31

DETAIL

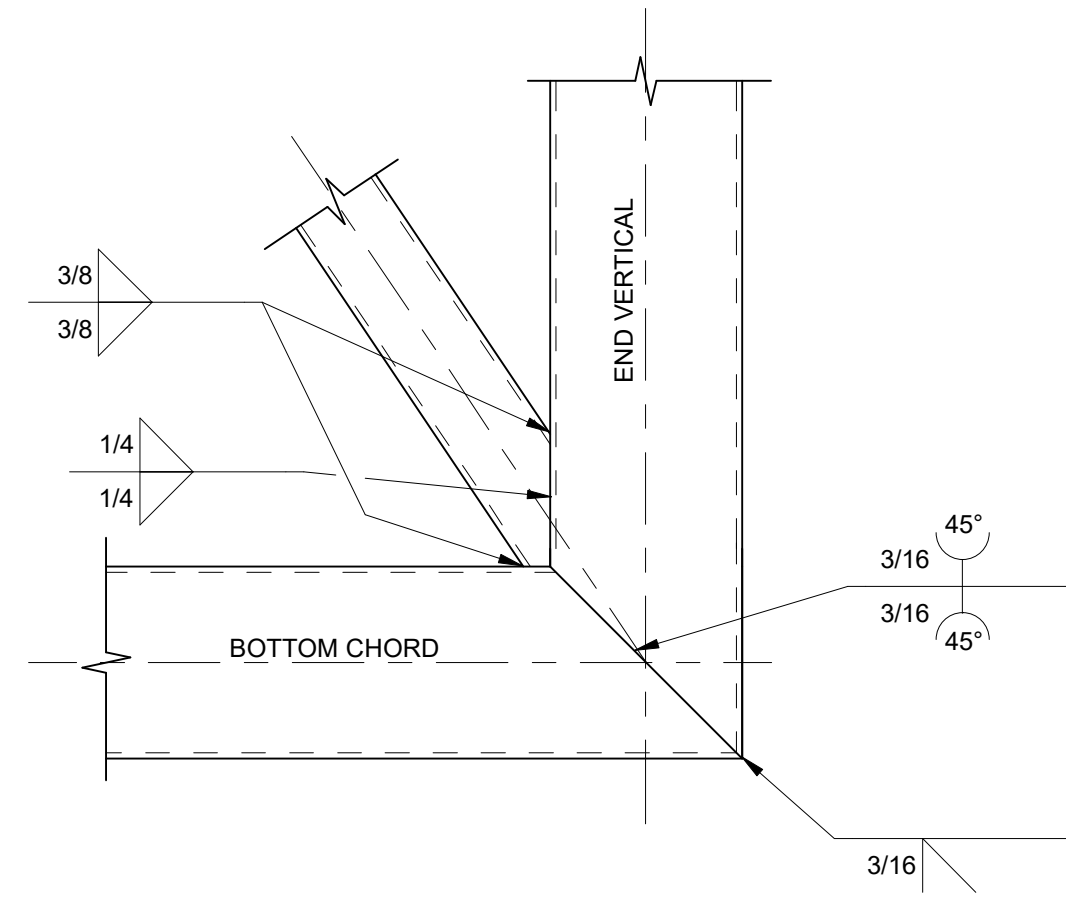
1 1/2" = 1'-0"



6
S5.31

DETAIL

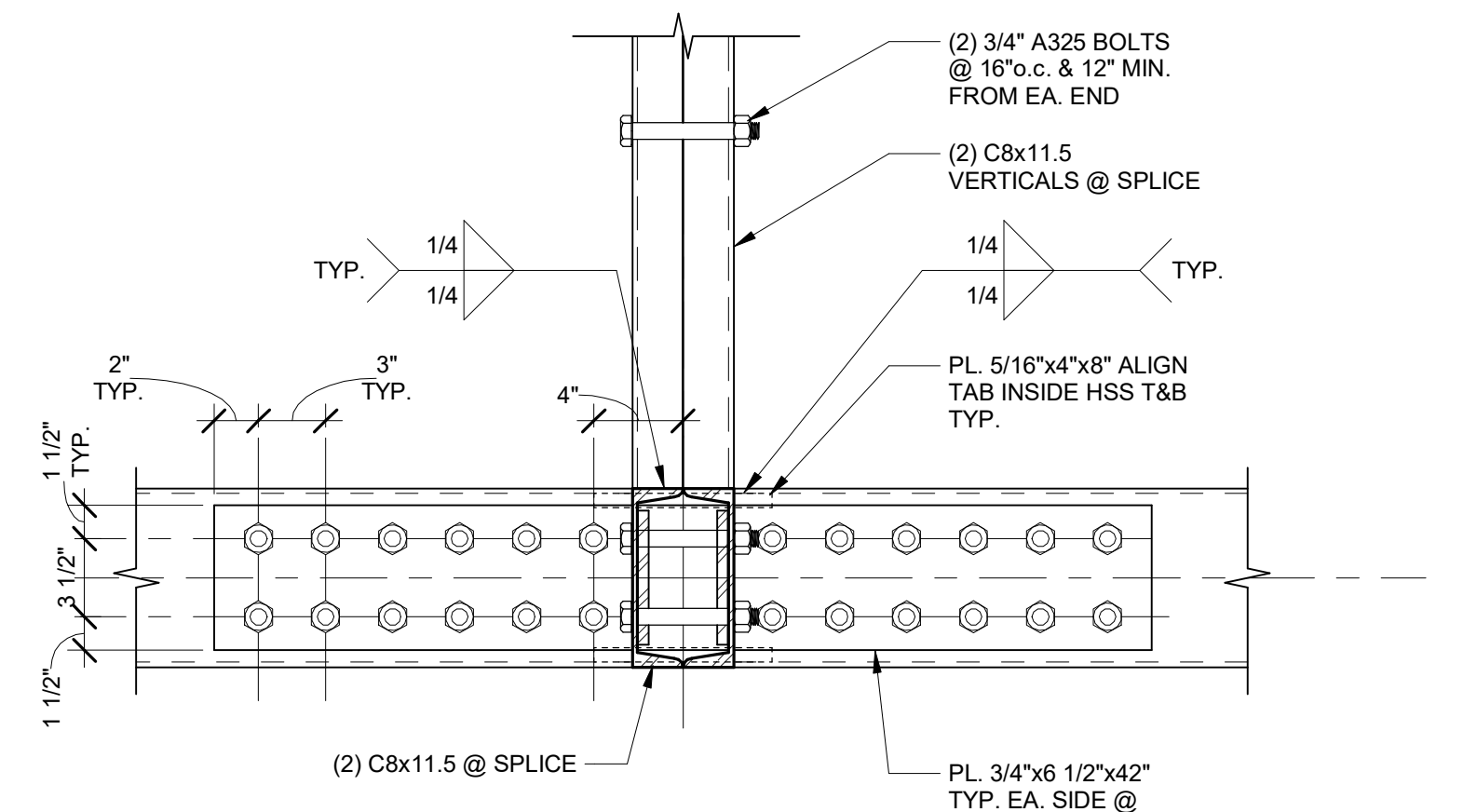
1 1/2" = 1'-0"



7
S5.31

DETAIL

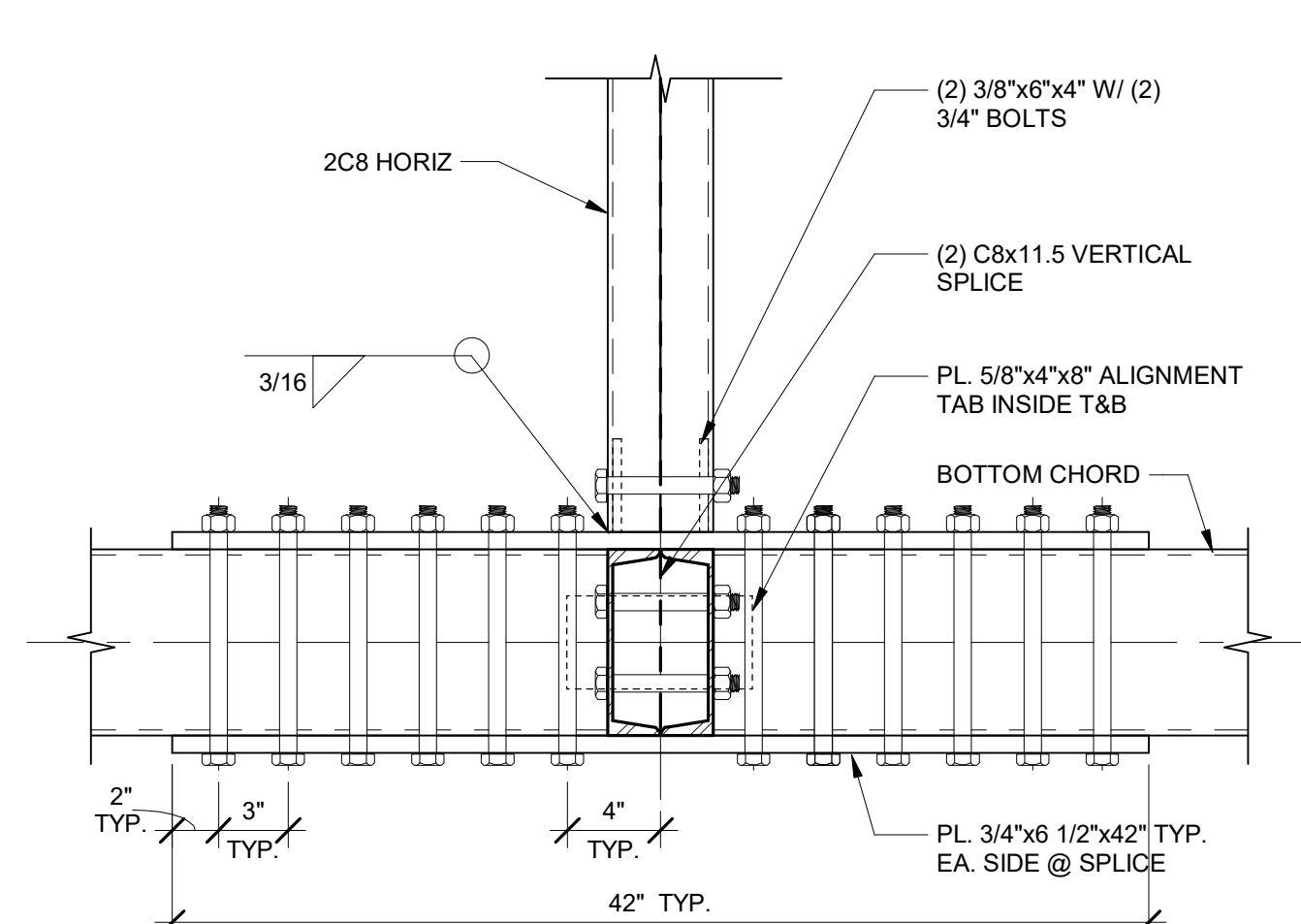
1 1/2" = 1'-0"



8
S5.31

ELEVATION - BOTTOM CHORD @ CONSTRUCTION & TRANSPORT SPLICE

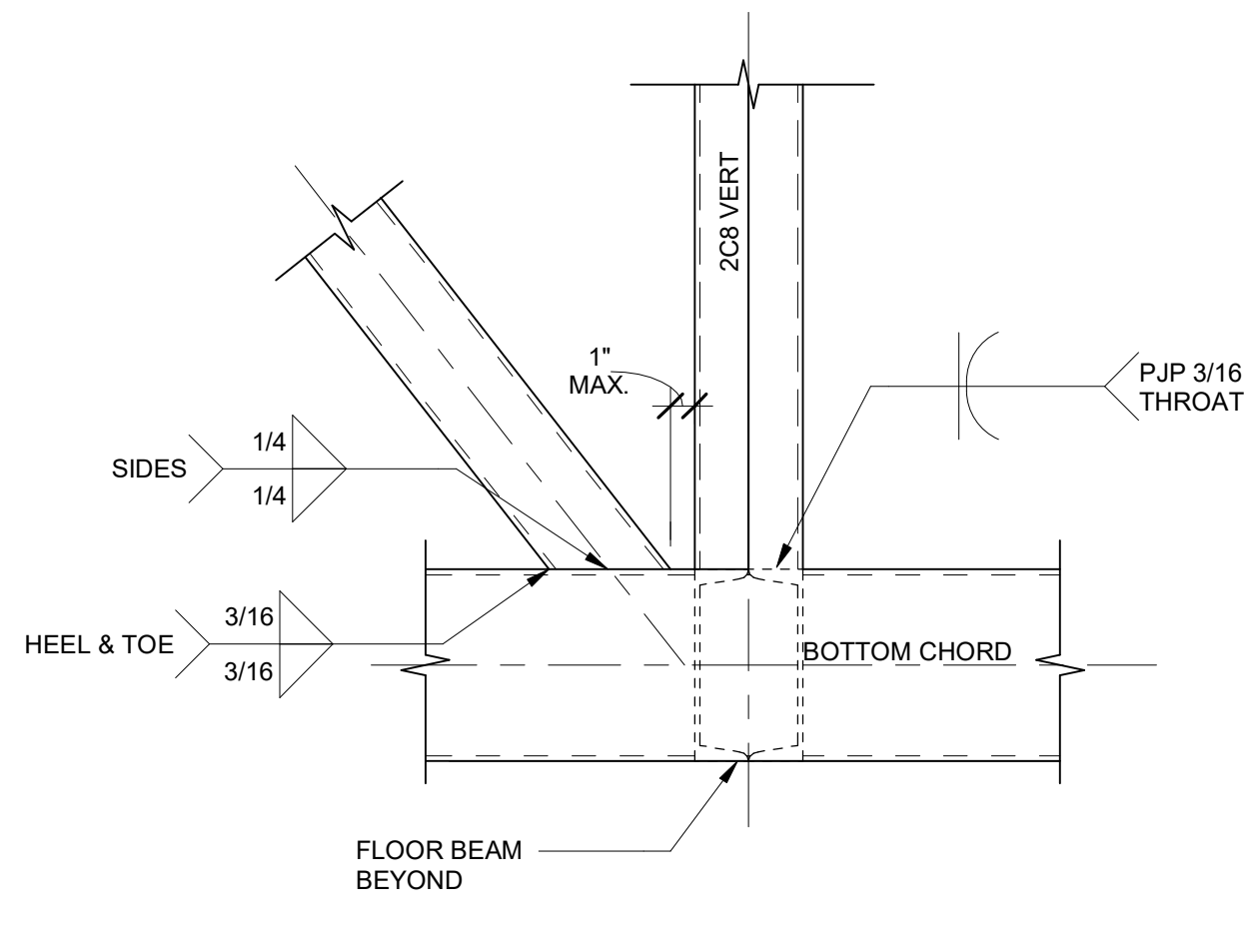
1 1/2" = 1'-0"



9
S5.31

PLAN VIEW - BOTTOM CHORD @ CONSTRUCTION & TRANSPORT SPLICE

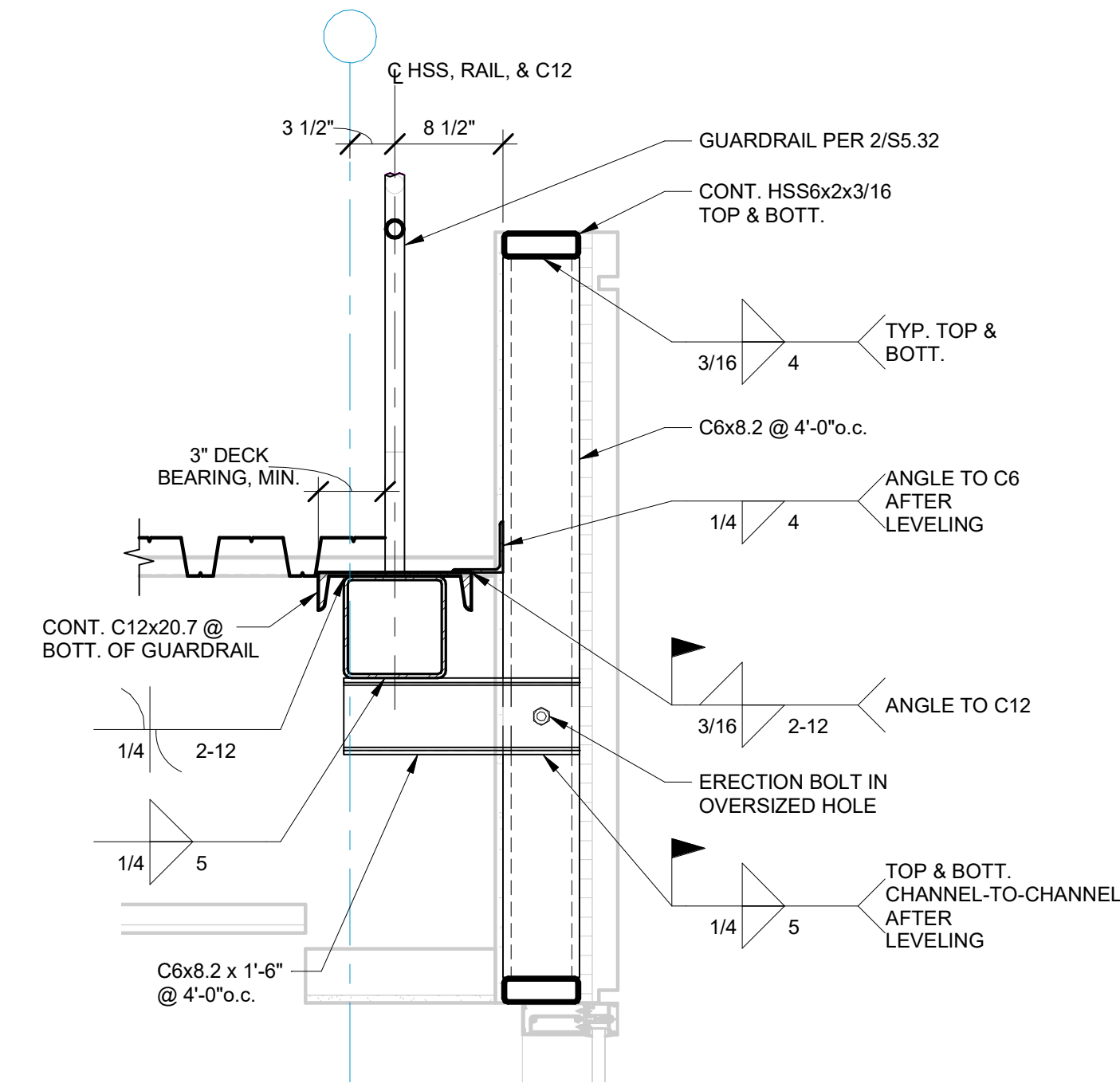
1 1/2" = 1'-0"



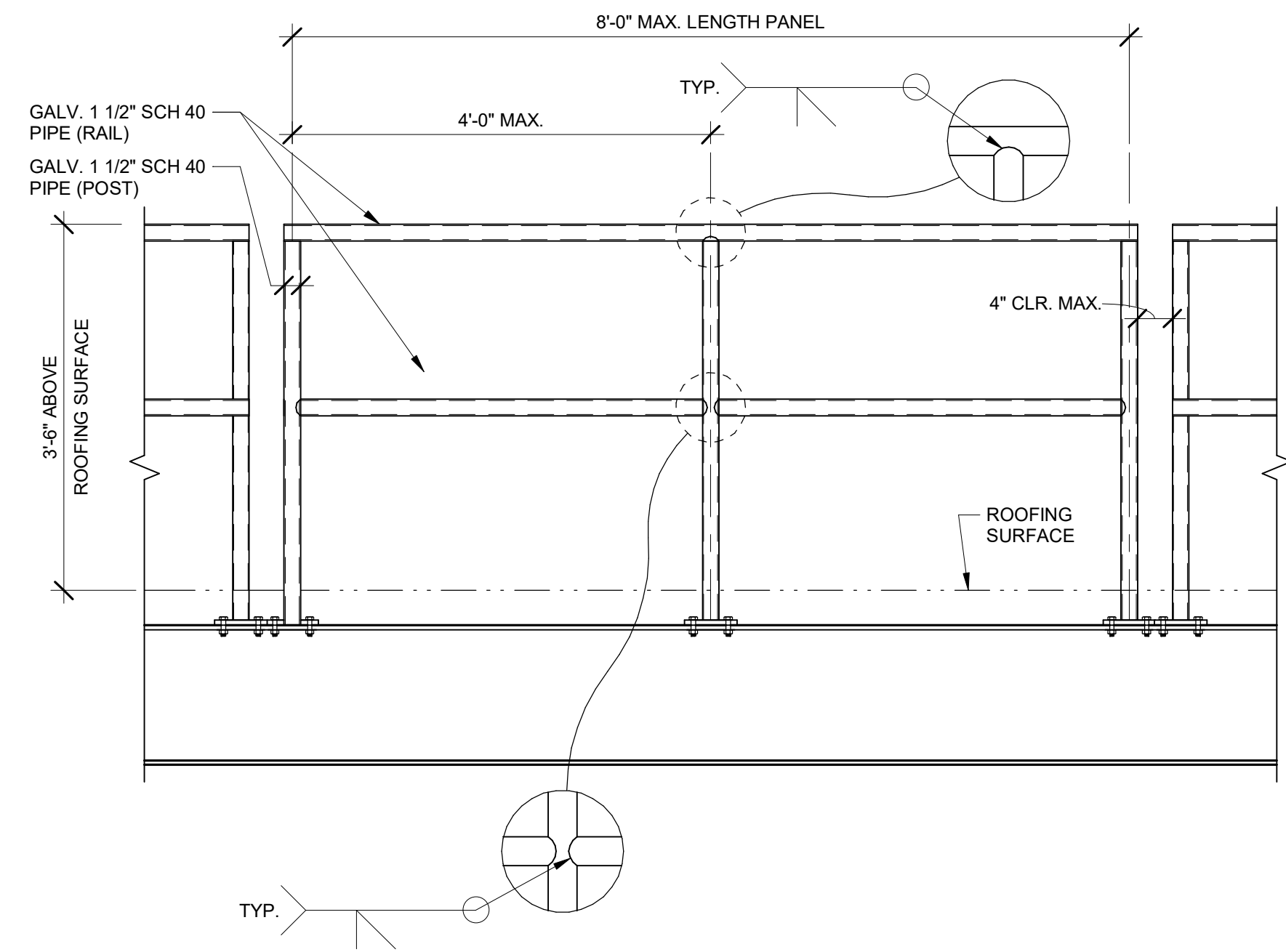
10
S5.31

DETAIL

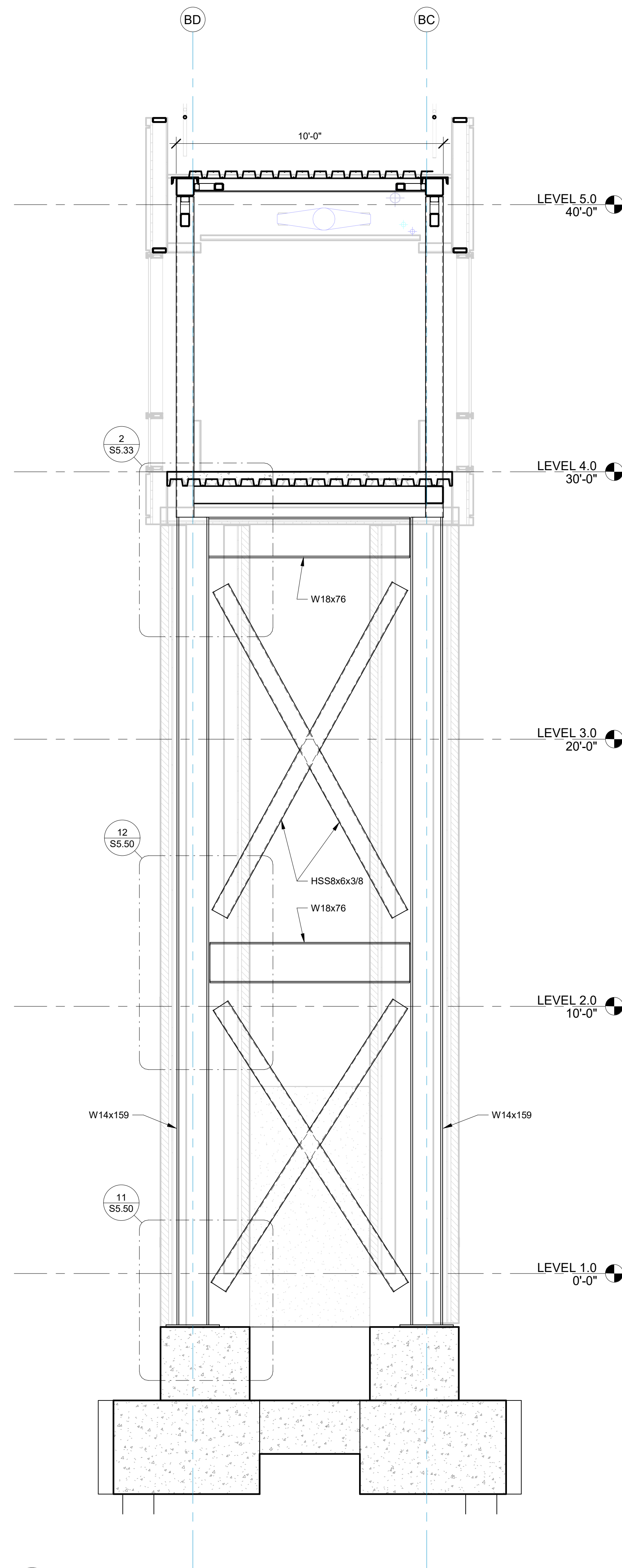
1 1/2" = 1'-0"



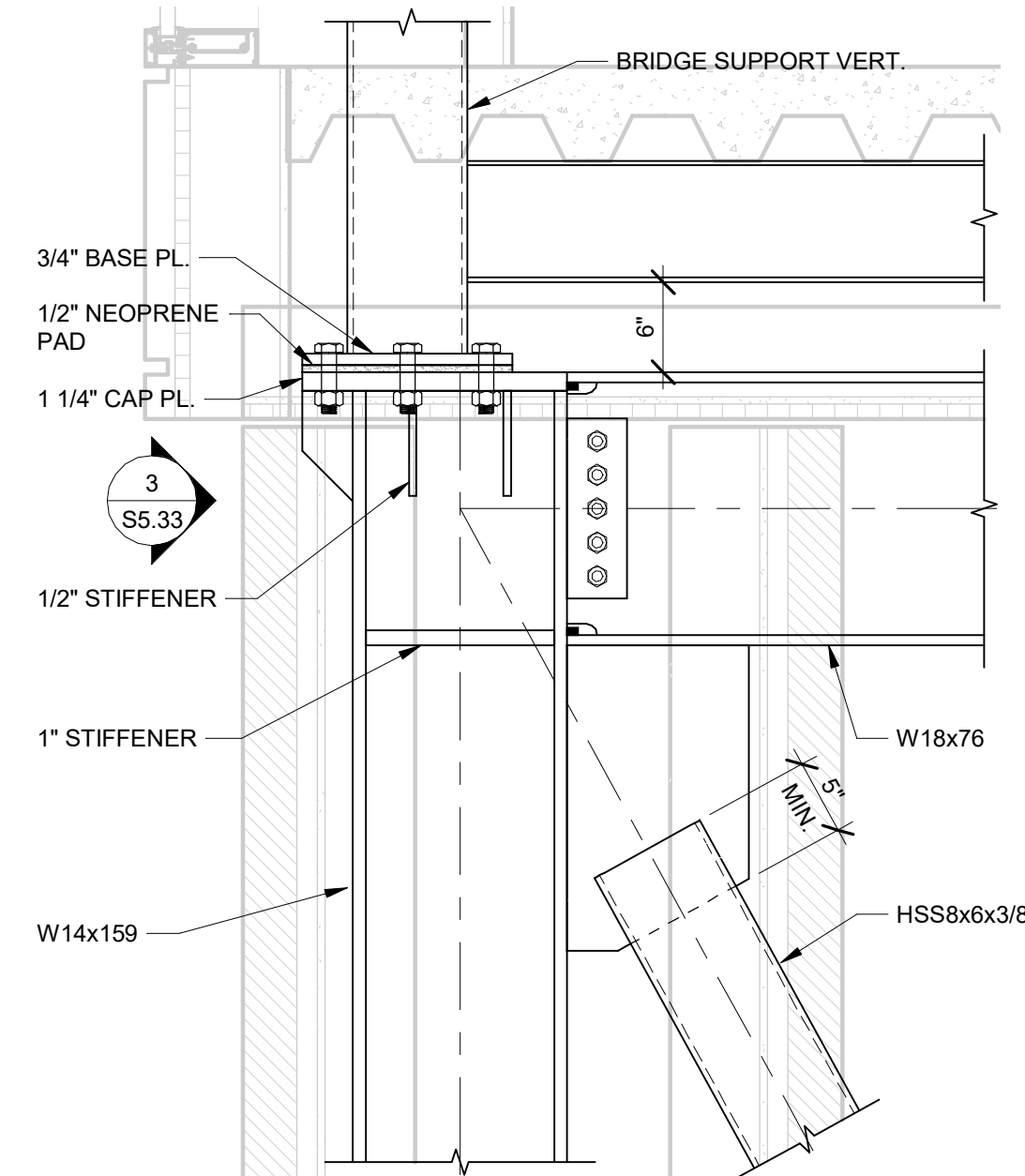
1 HEAD & PARAPET SUPPORT @ SKYBRIDGE
S5.32 1" = 1'-0"



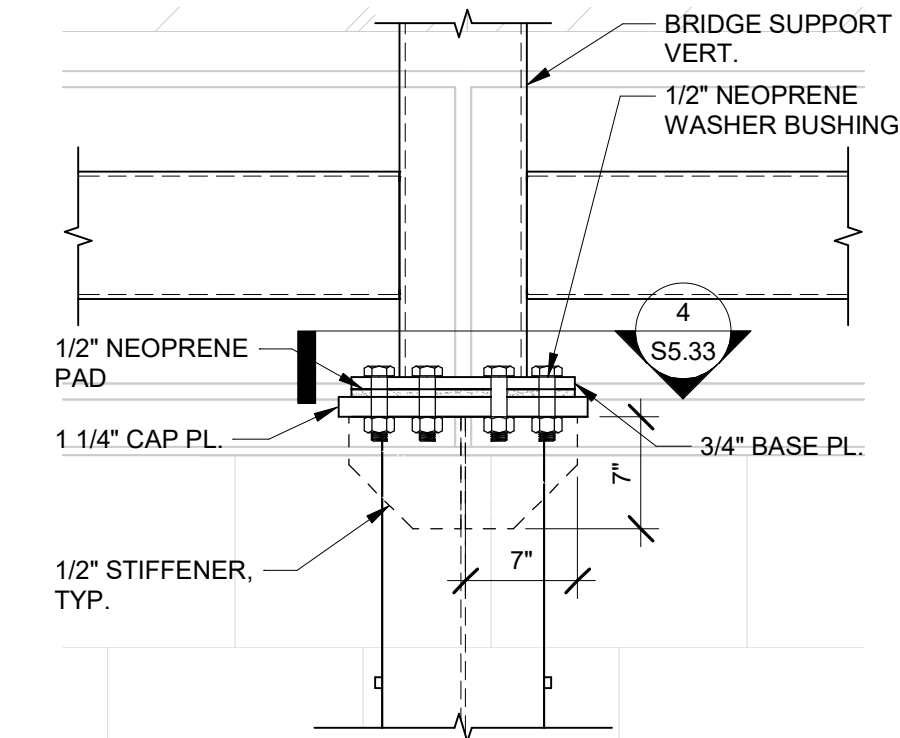
2 GUARDRAIL DETAIL
S5.32 3/4" = 1'-0"



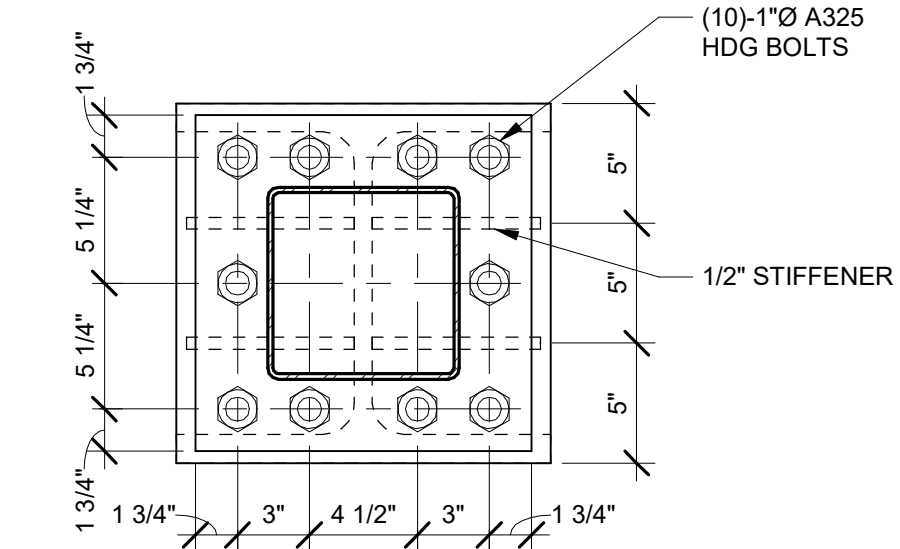
1 S5.33/1
3/8" = 1'-0"



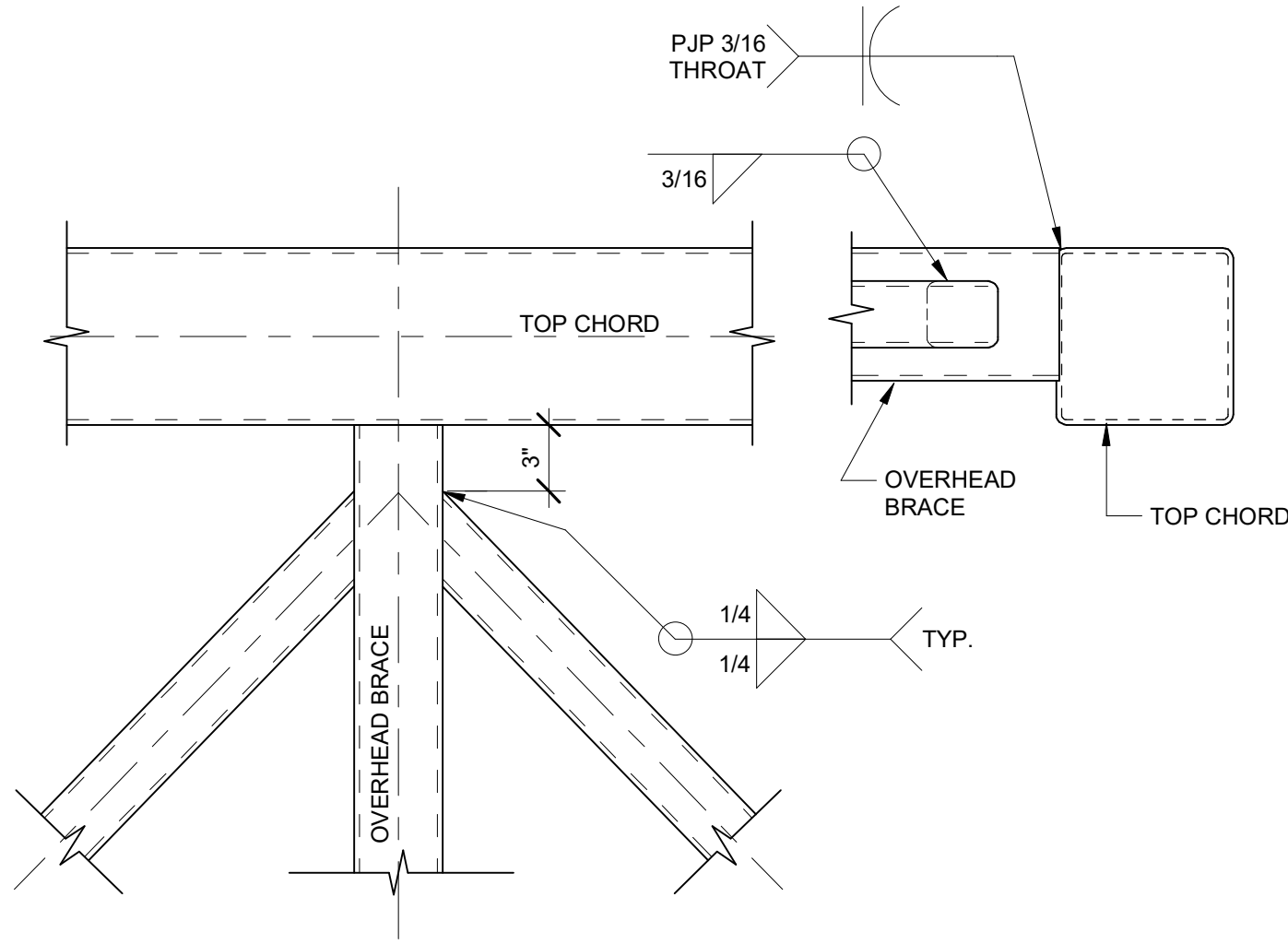
2 BRACE DETAIL
1" = 1'-0"



3 DETAIL
1" = 1'-0"

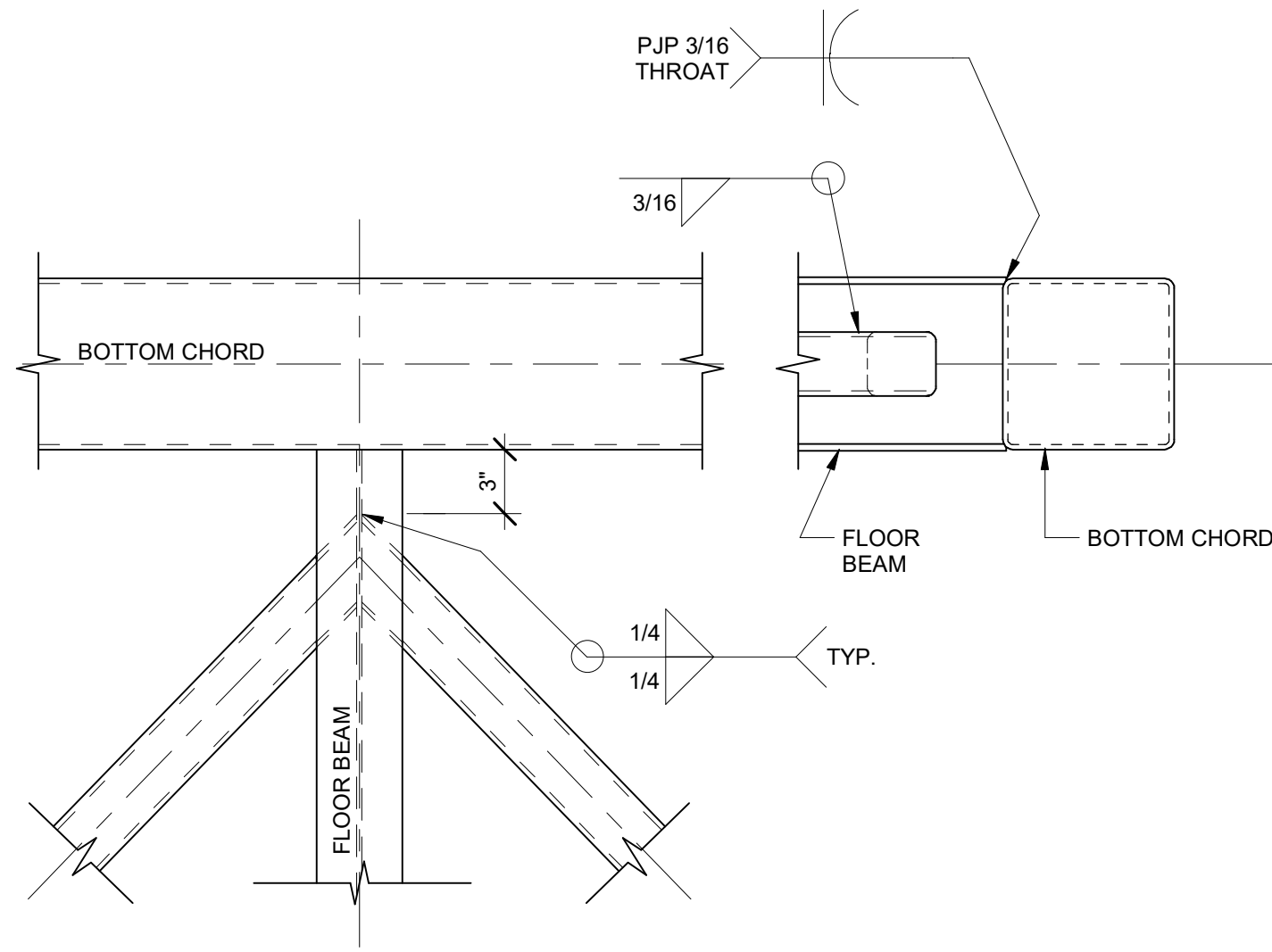


4 BRIDGE VERT SUPPORT BASE PLATE
1 1/2" = 1'-0"



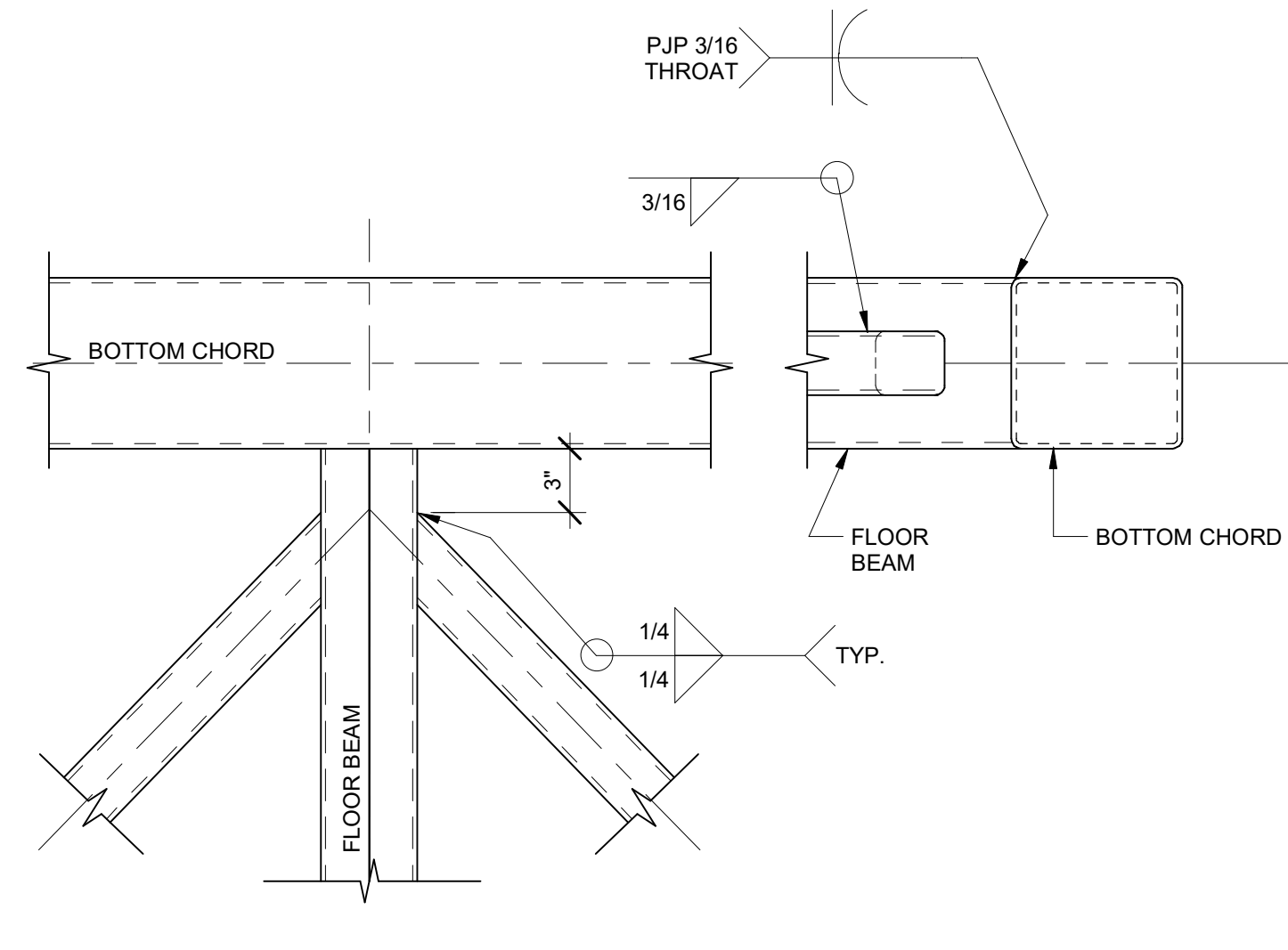
1
S5.34

DETAIL
1 1/2" = 1'-0"



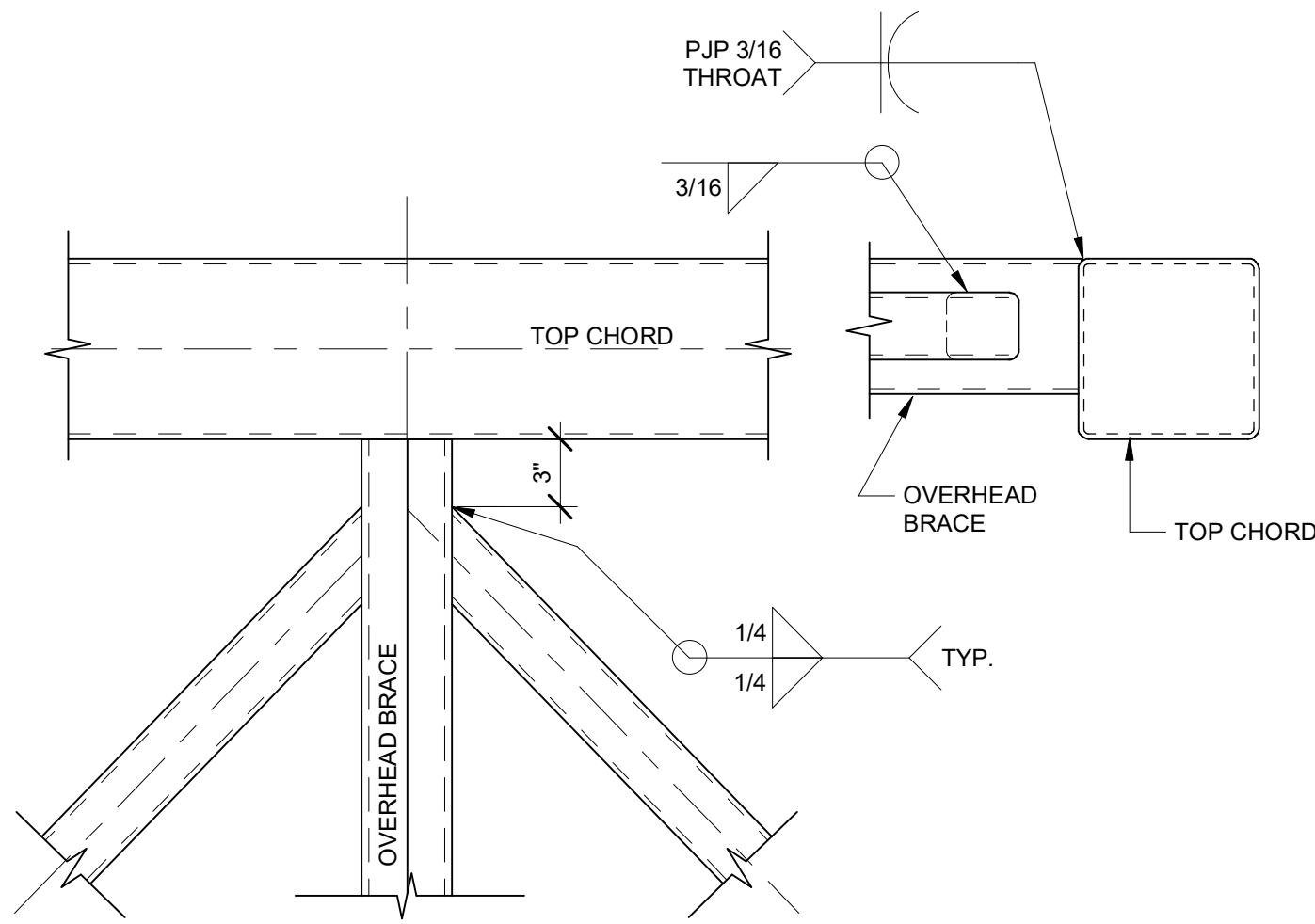
2
S5.34

DETAIL
1 1/2" = 1'-0"



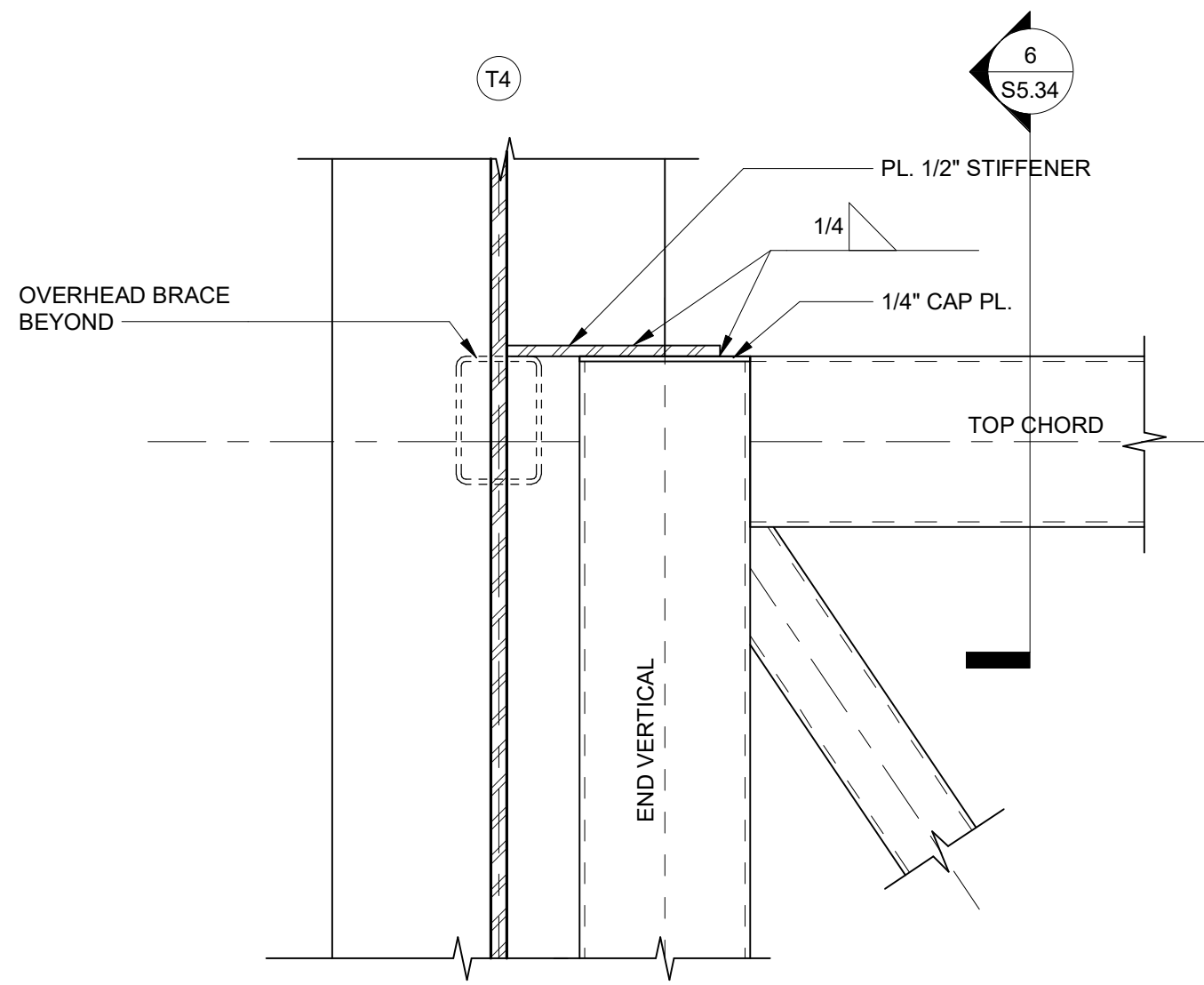
3
S5.34

DETAIL
1 1/2" = 1'-0"



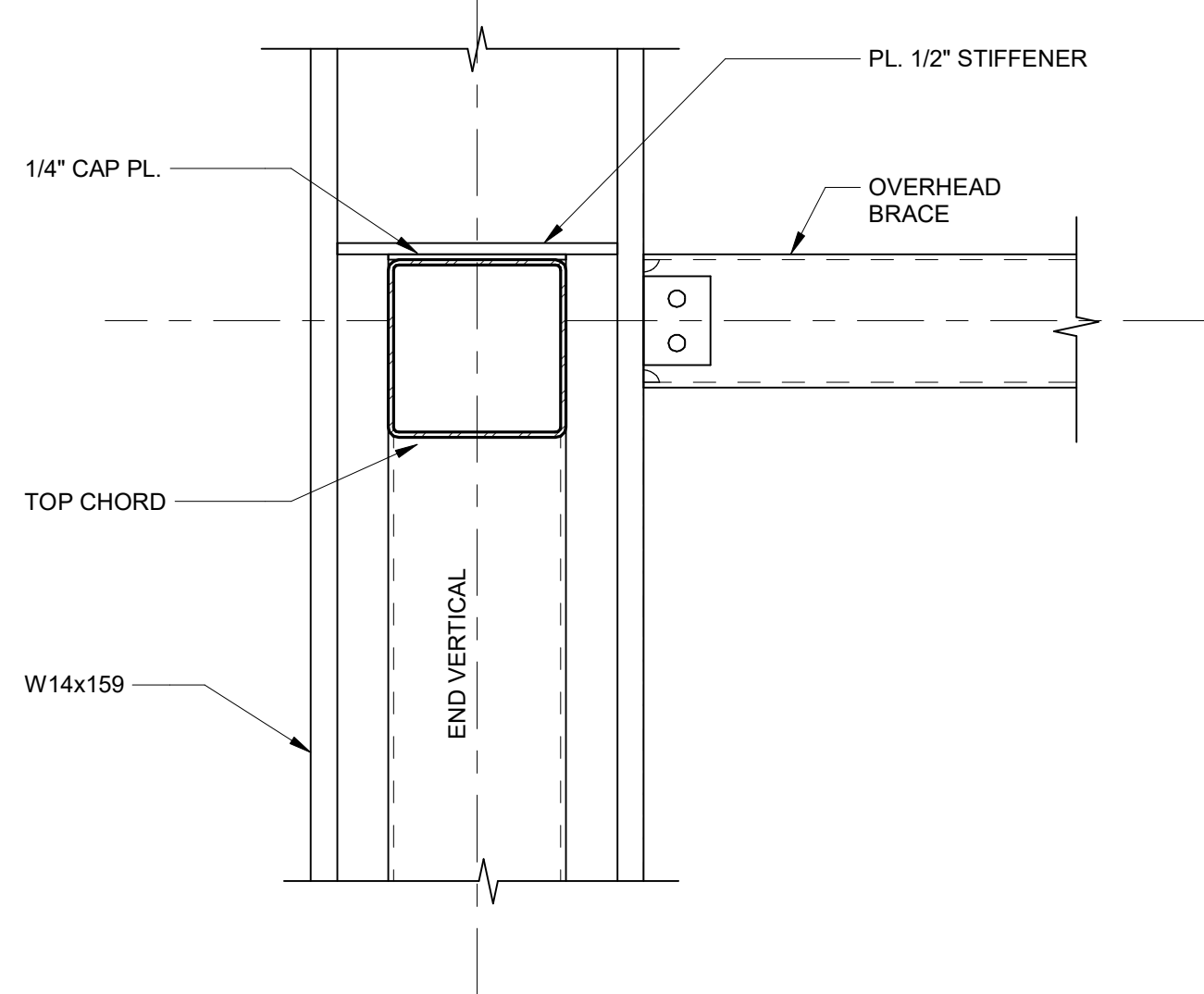
4
S5.34

DETAIL
1 1/2" = 1'-0"



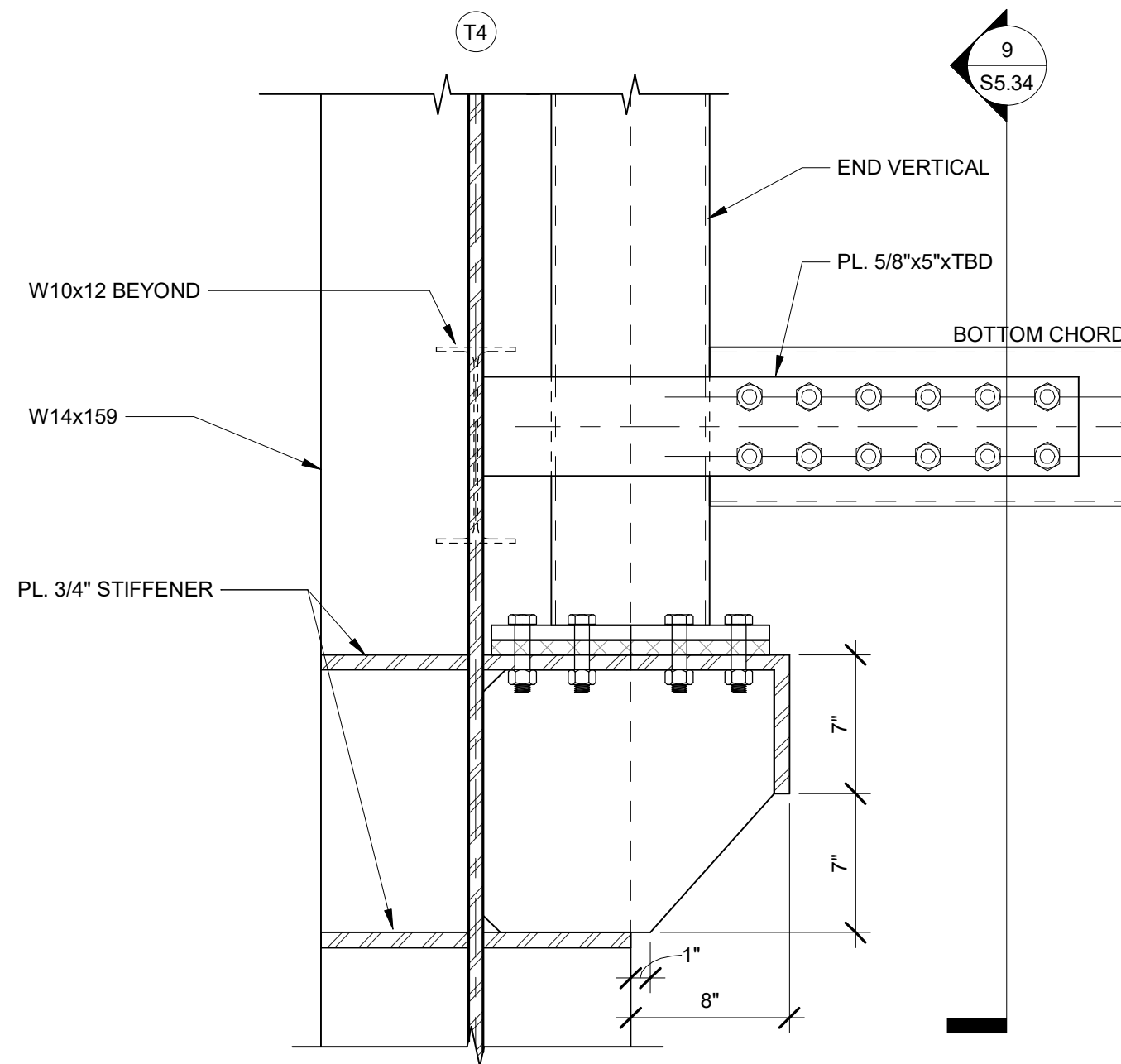
5
S5.34

DETAIL
1 1/2" = 1'-0"



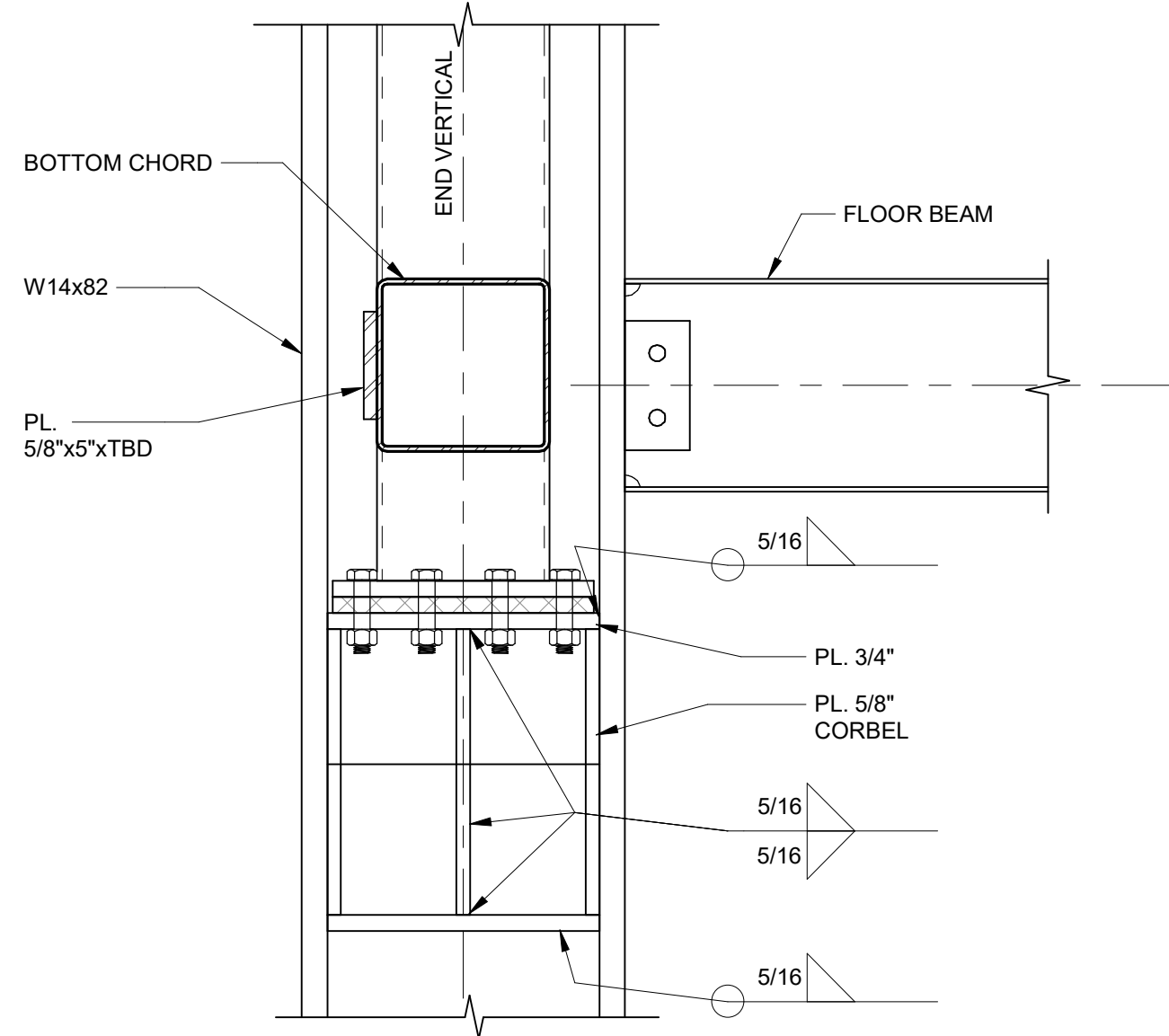
6
S5.34

DETAIL
1 1/2" = 1'-0"



8
S5.34

DETAIL
1 1/2" = 1'-0"

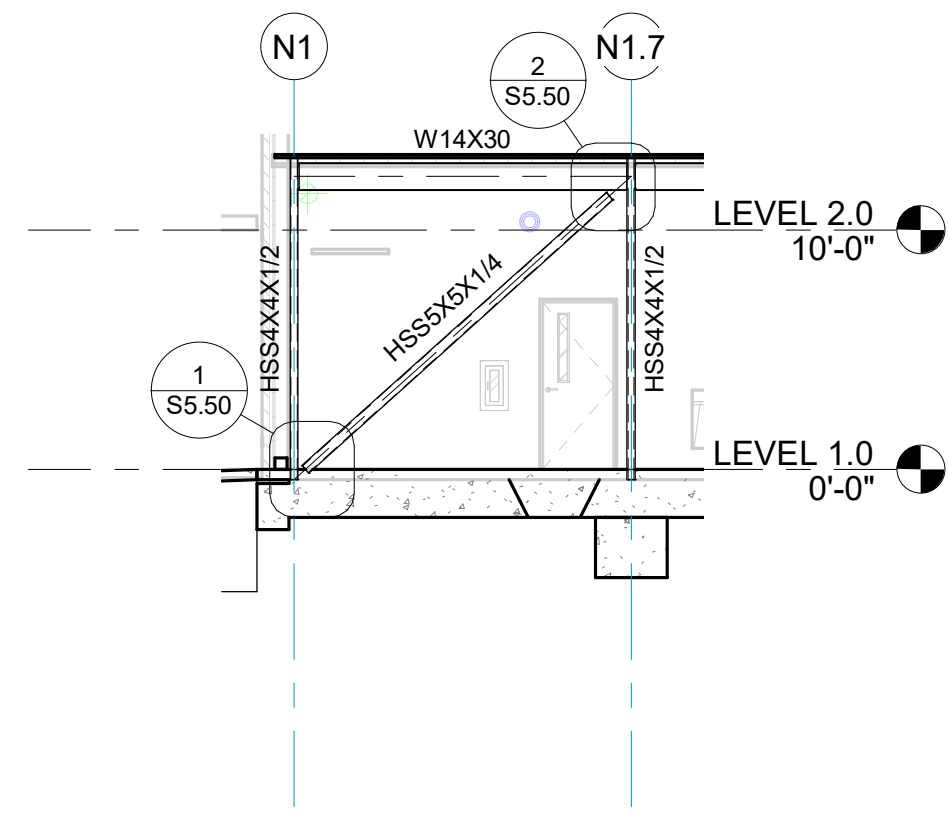


9
S5.34

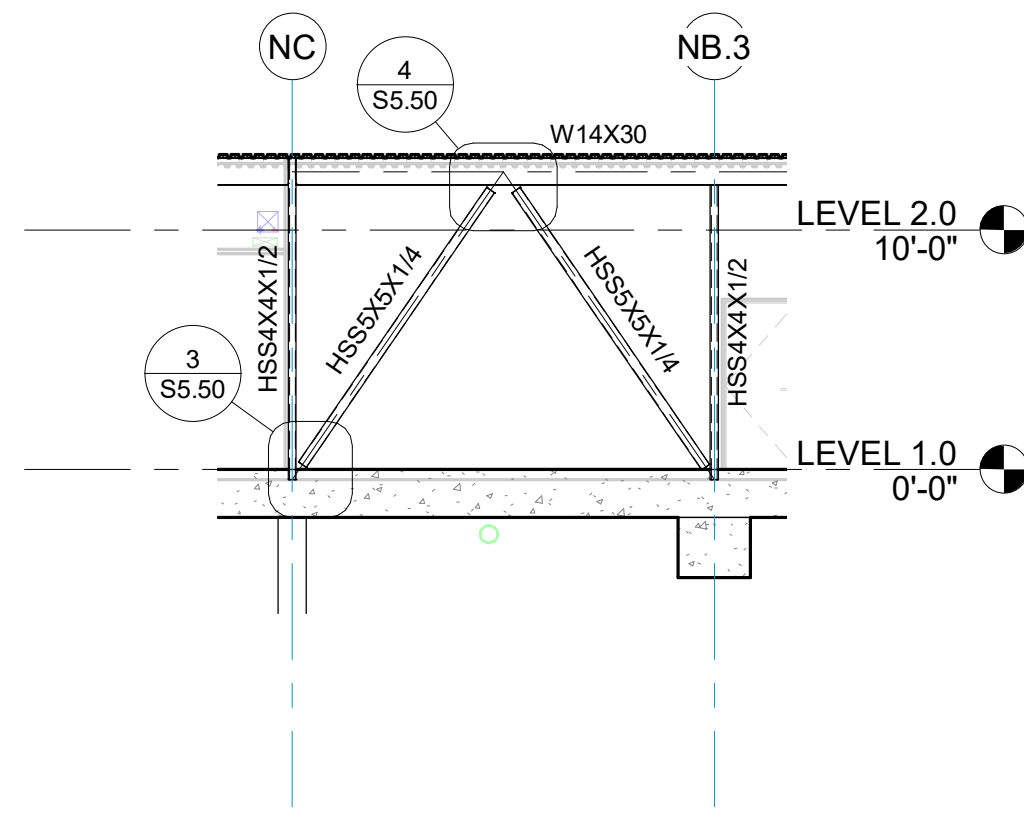
DETAIL
1 1/2" = 1'-0"

7
S5.34

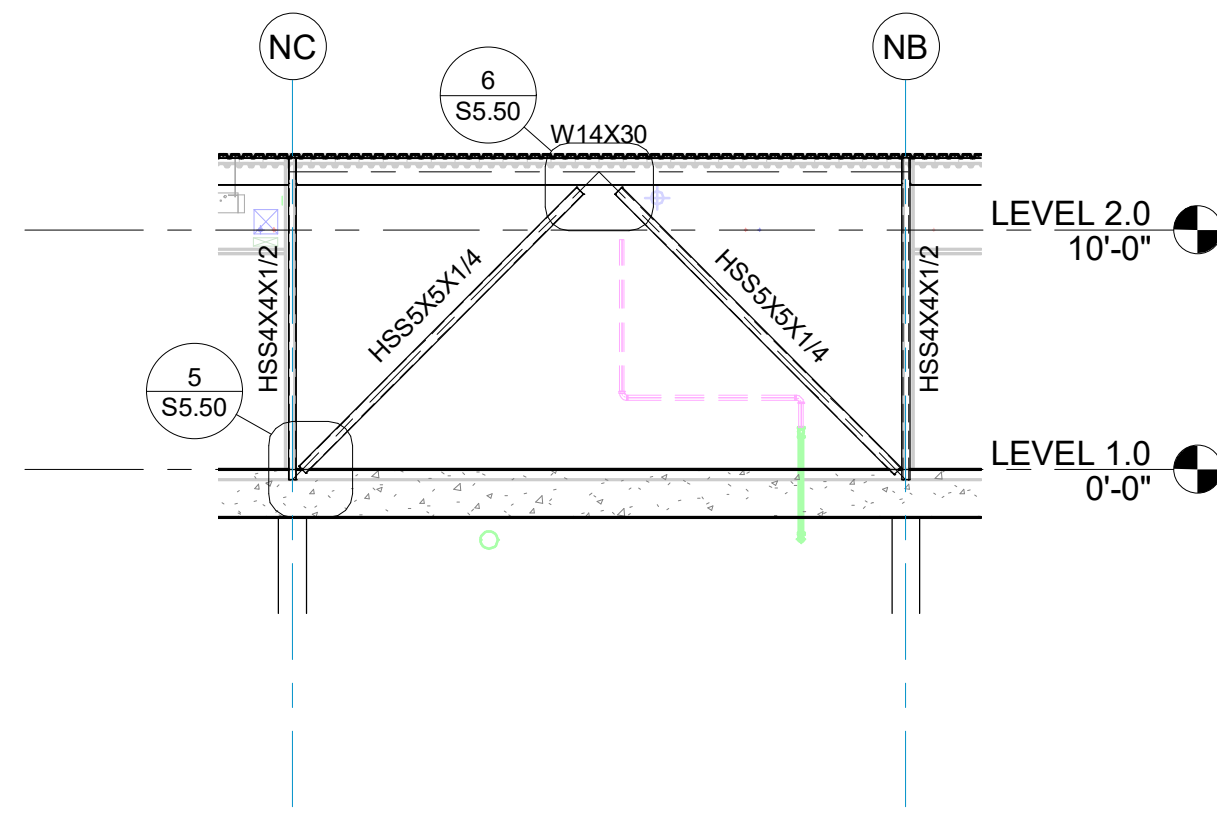
NOT USED



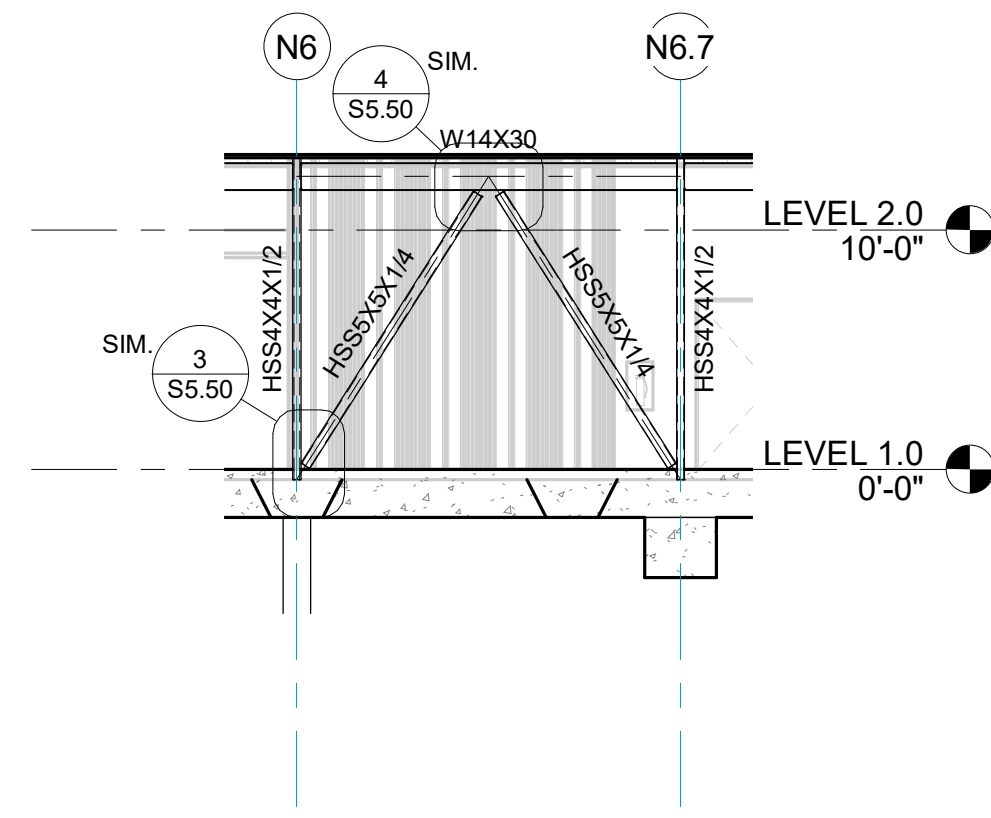
1 BRACE A
S5.40 1/8" = 1'-0"



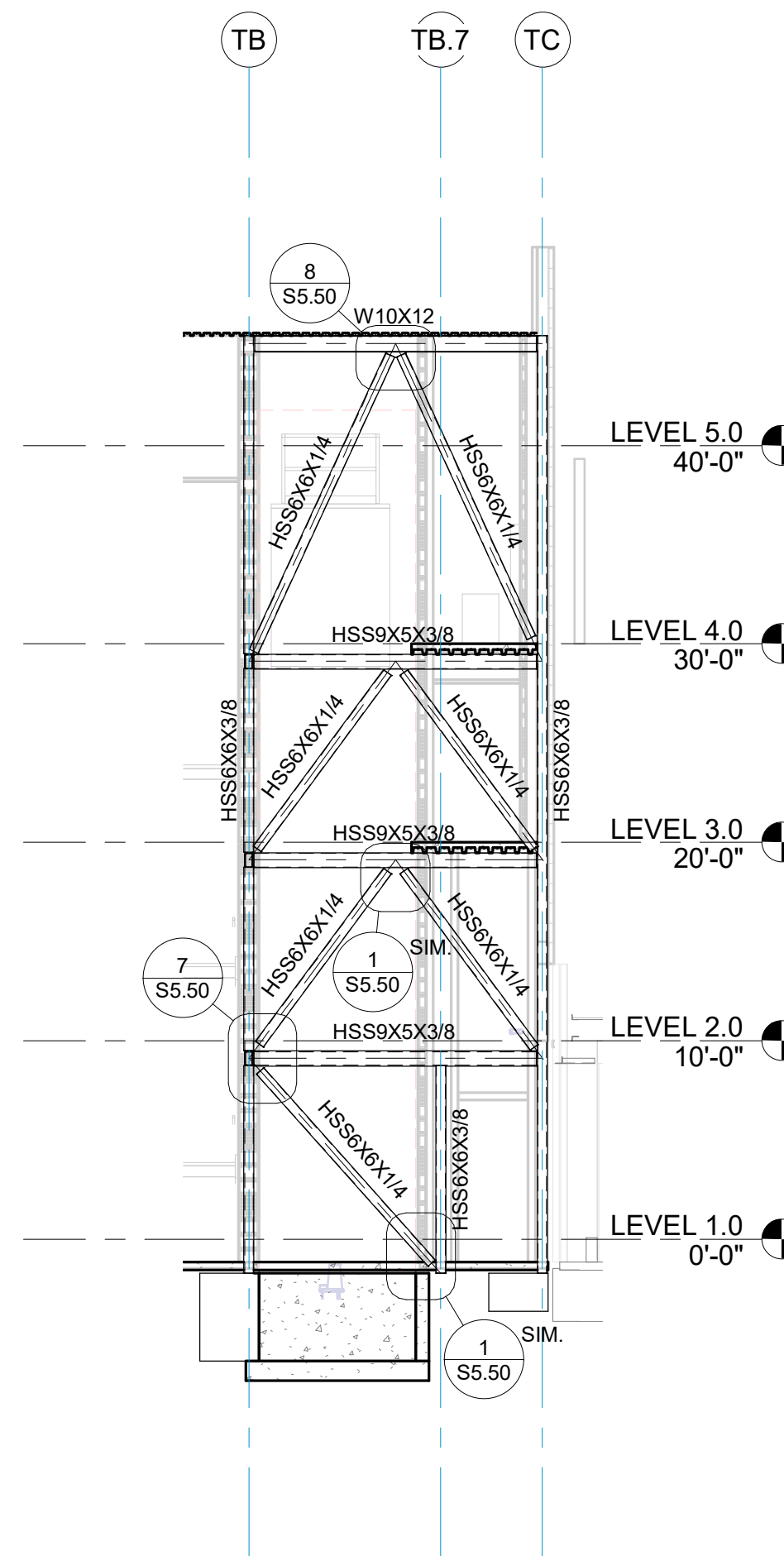
2 BRACE B
S5.40 1/8" = 1'-0"



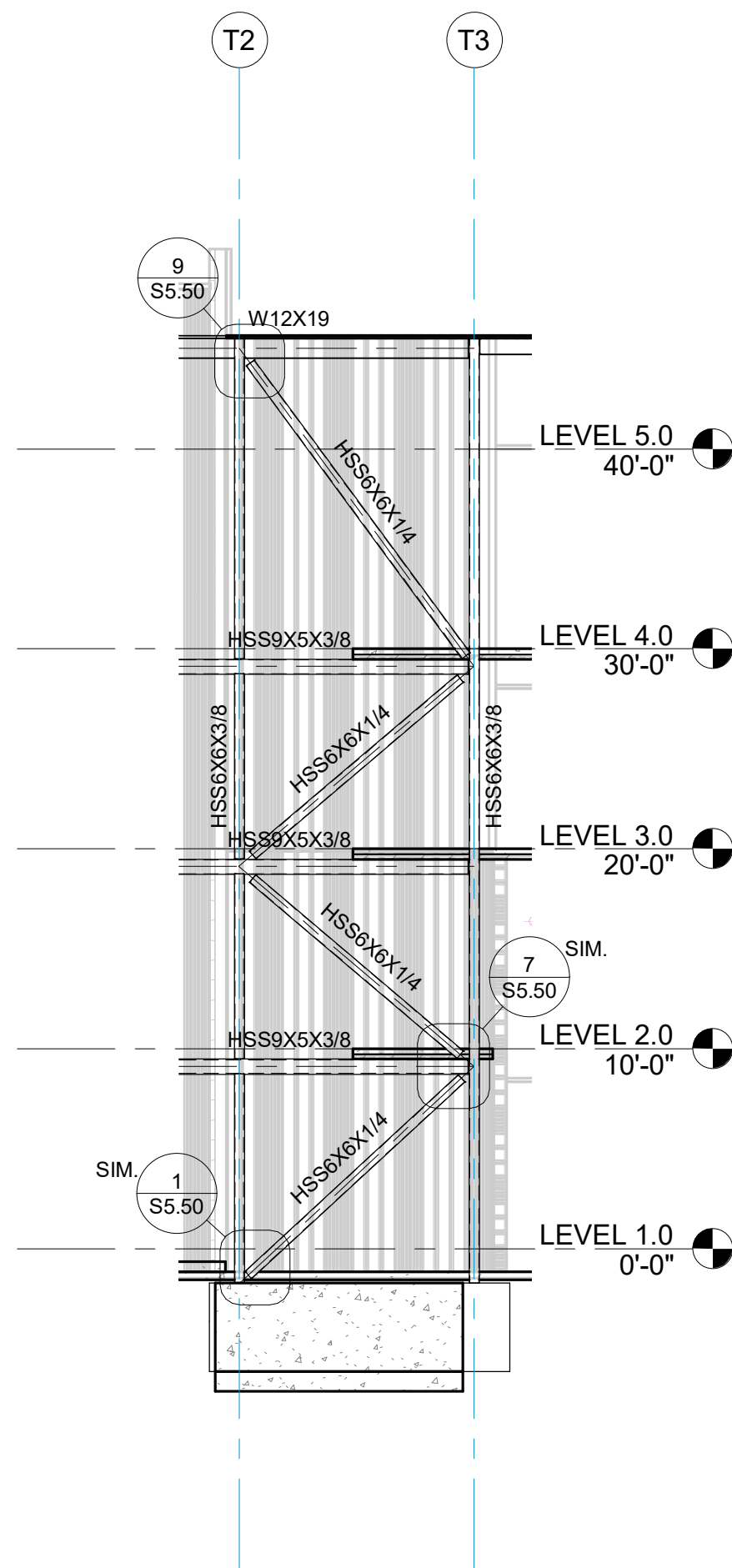
3 BRACE C
S5.40 1/8" = 1'-0"



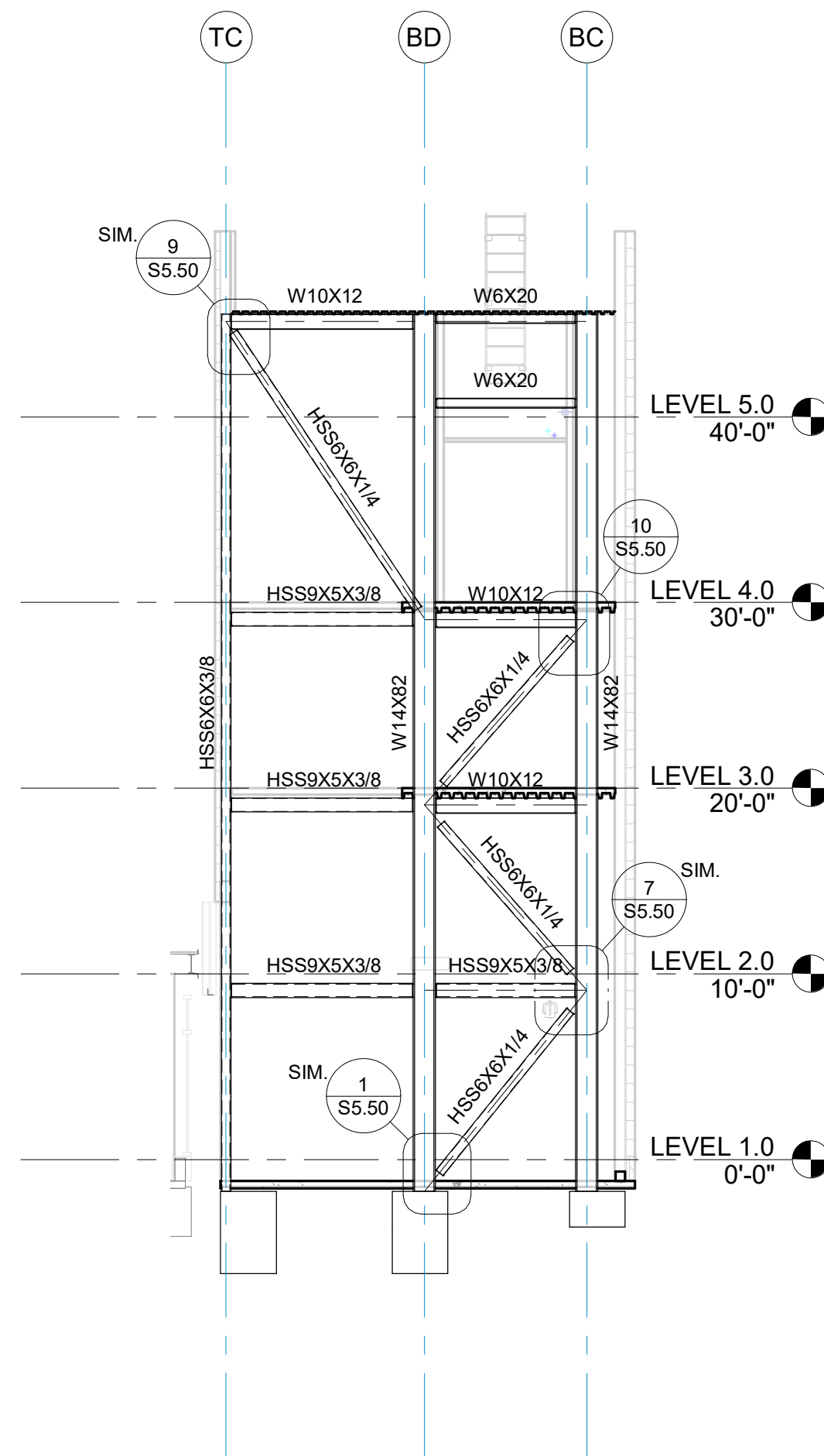
4 BRACE D
S5.40 1/8" = 1'-0"



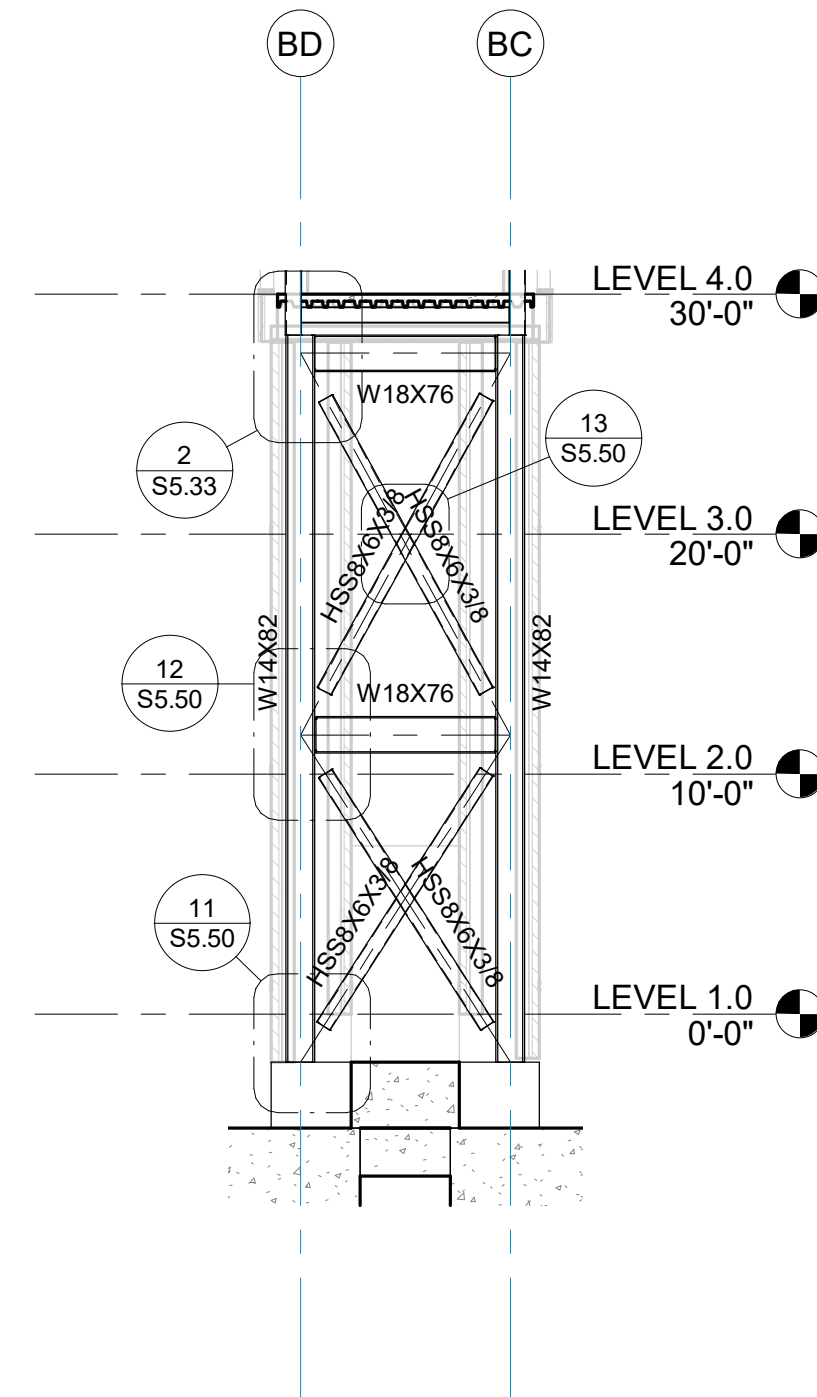
5 BRACE E
S5.40 1/8" = 1'-0"



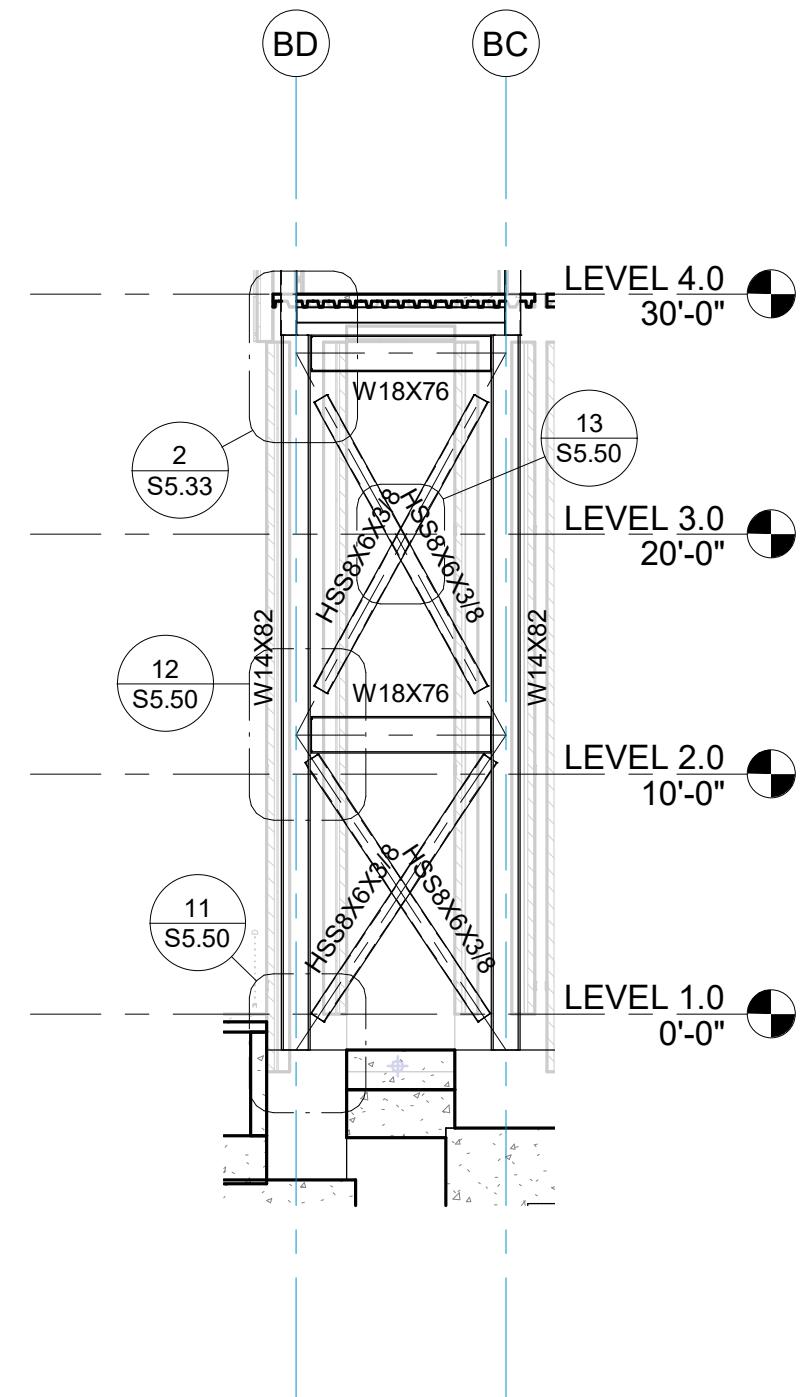
6 BRACE F
S5.40 1/8" = 1'-0"



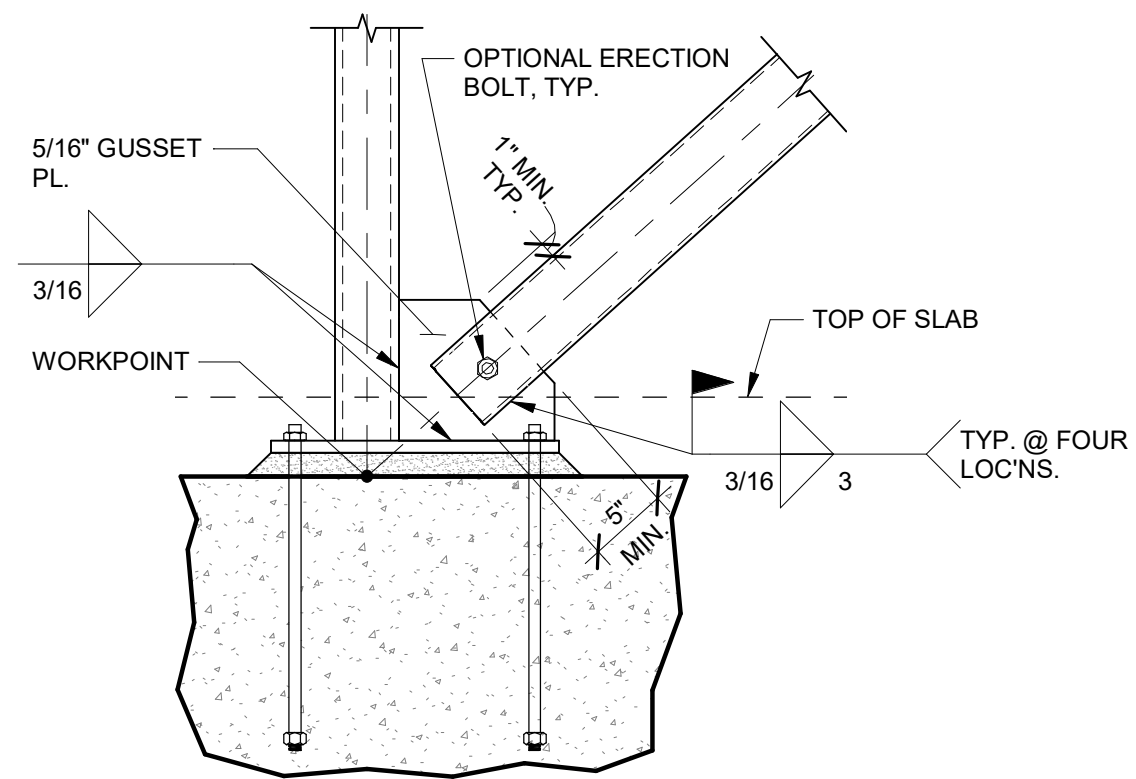
7 BRACE G
S5.40 1/8" = 1'-0"



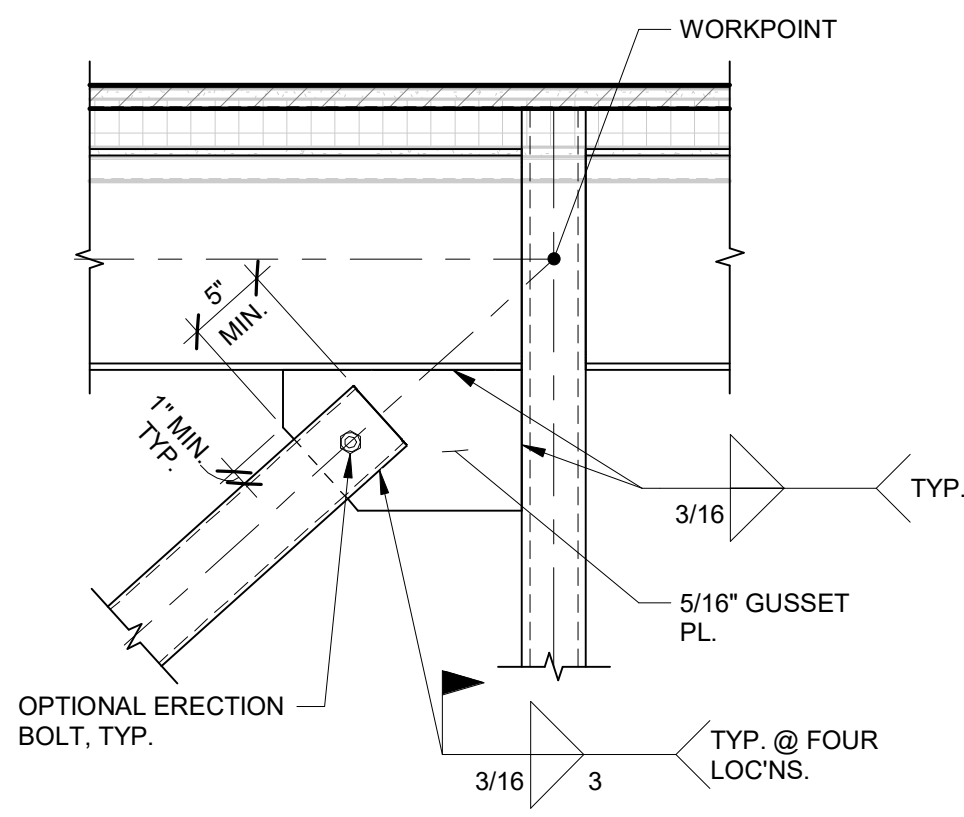
8 BRACE H
S5.40 1/8" = 1'-0"



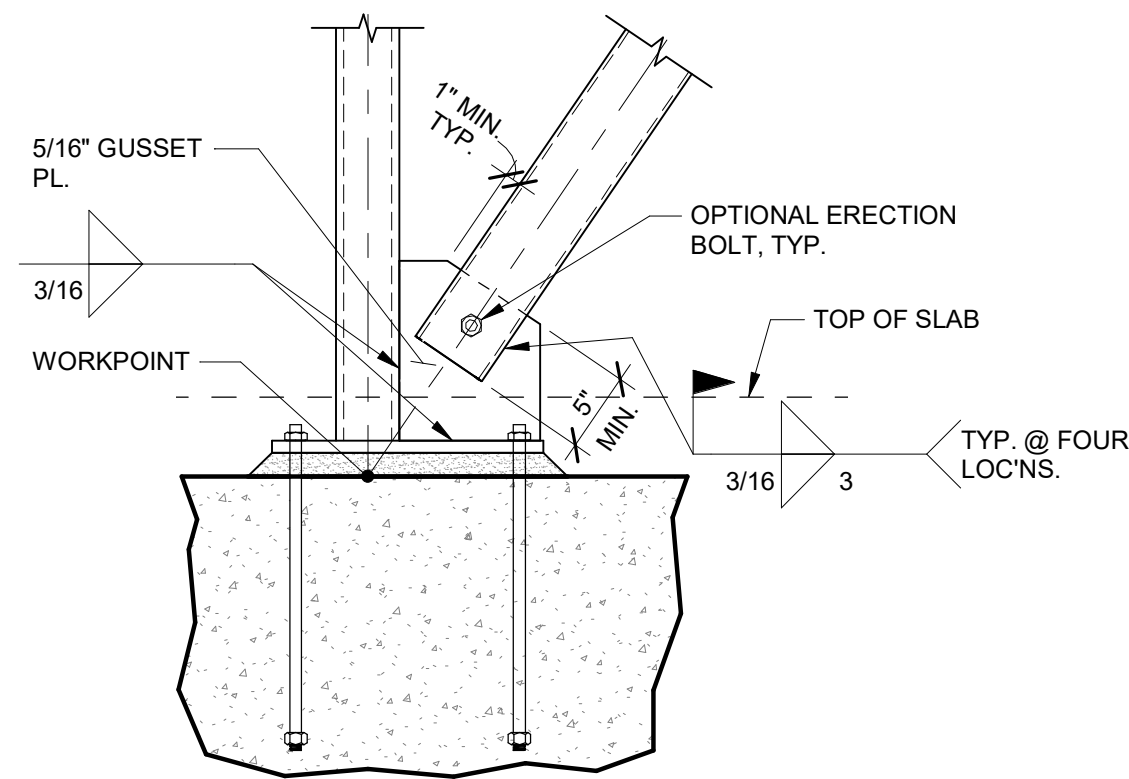
9 BRACE I
S5.40 1/8" = 1'-0"



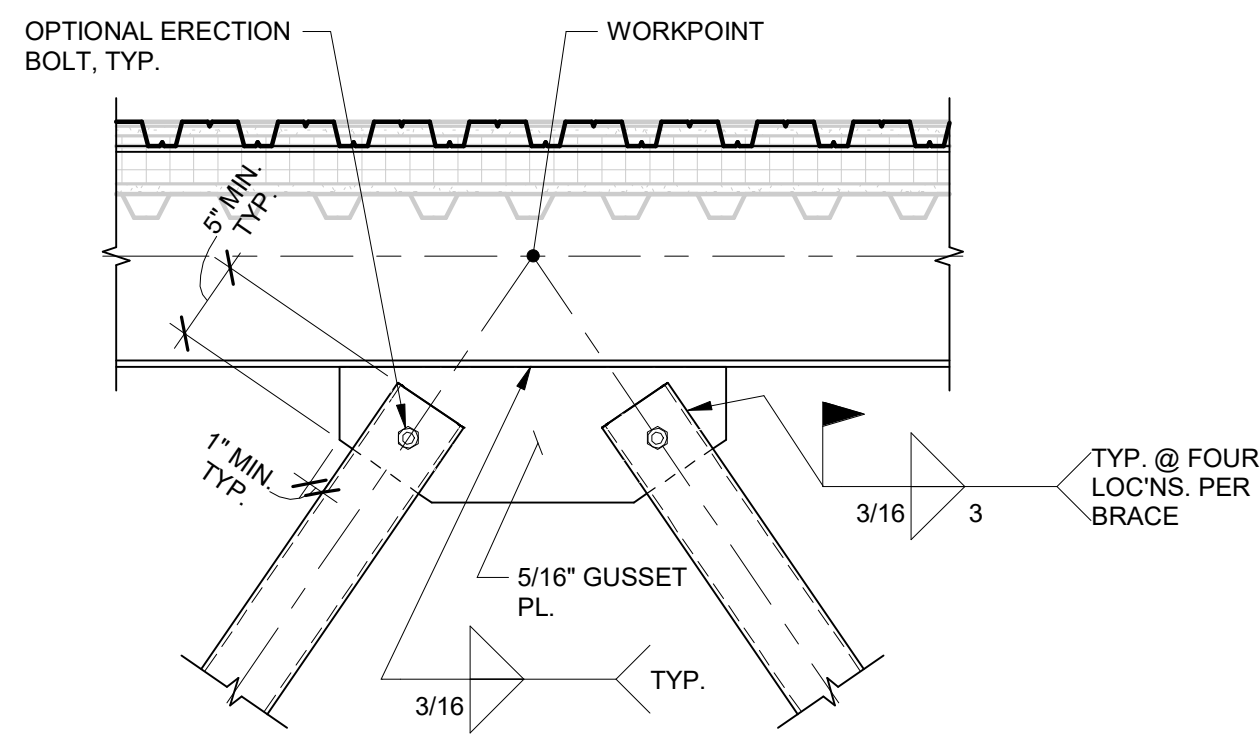
1 BRACE DETAIL
S5.50 1" = 1'-0"



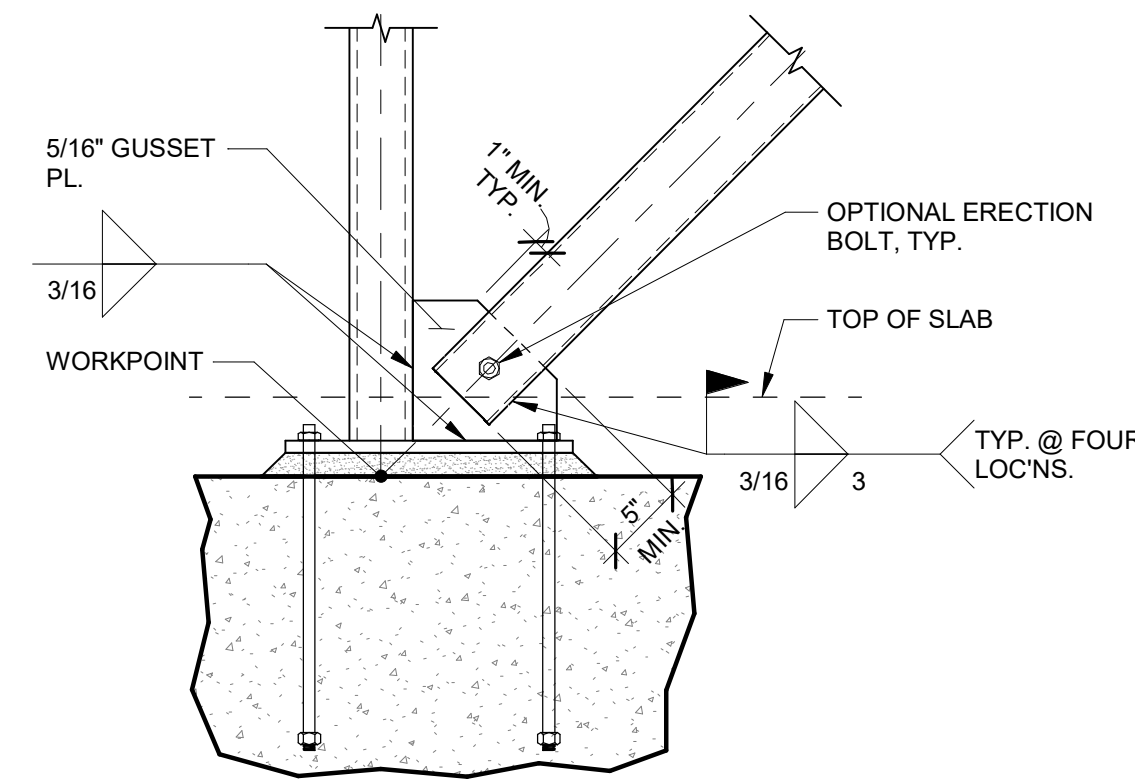
2 BRACE DETAIL
S5.50 1" = 1'-0"



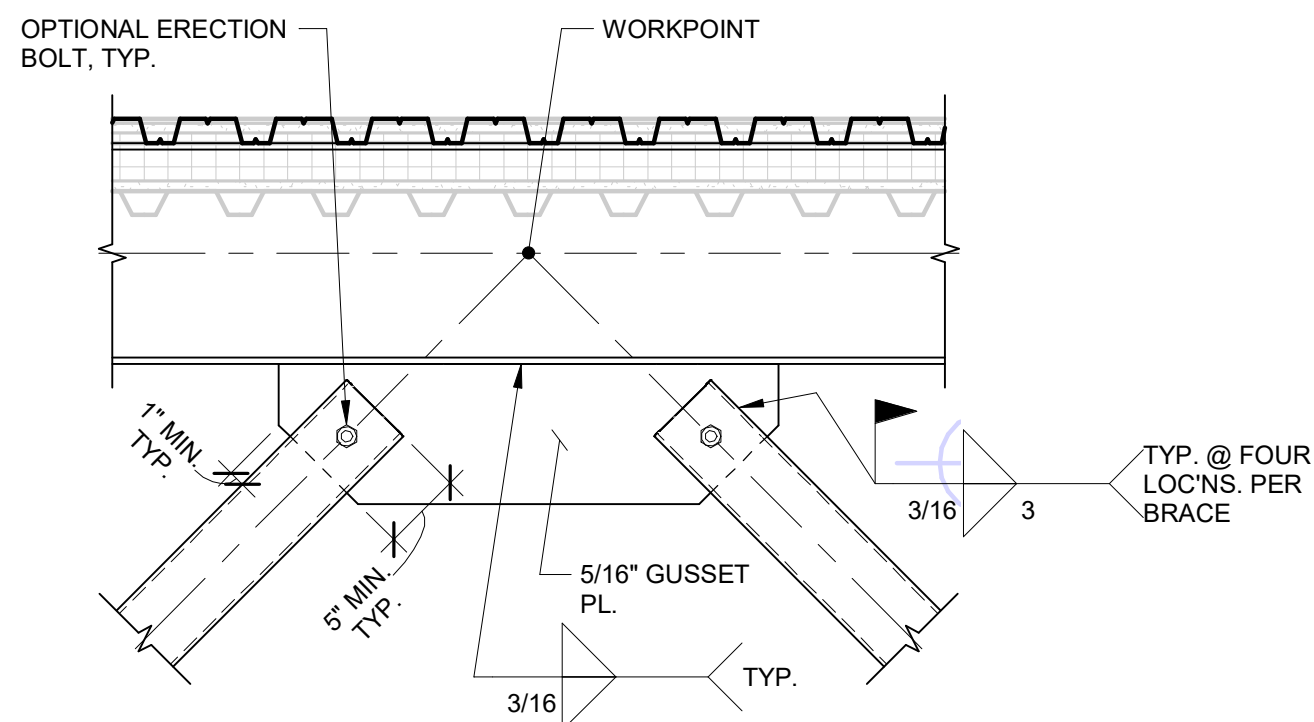
3 BRACE DETAIL
S5.50 1" = 1'-0"



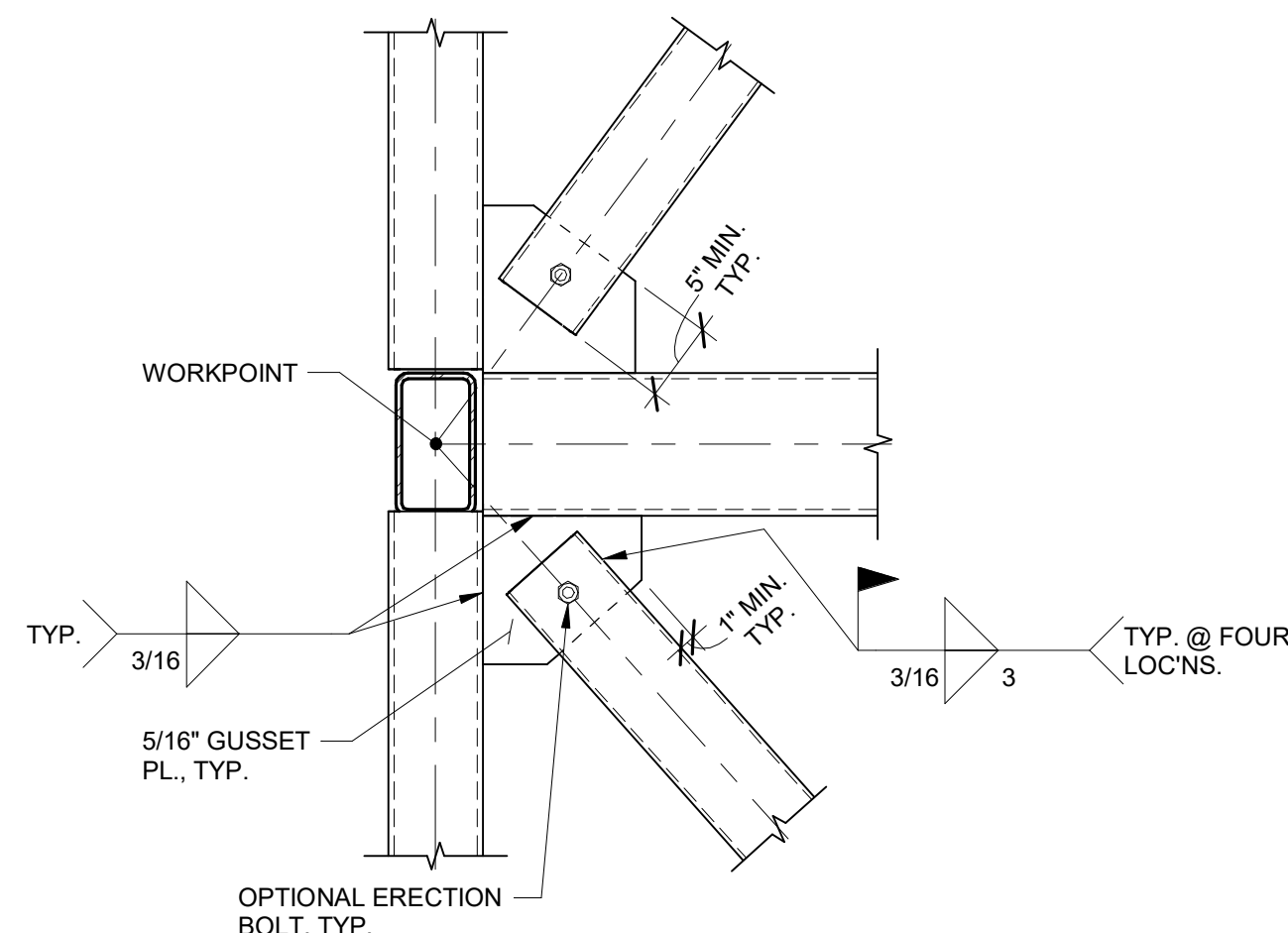
4 BRACE DETAIL
S5.50 1" = 1'-0"



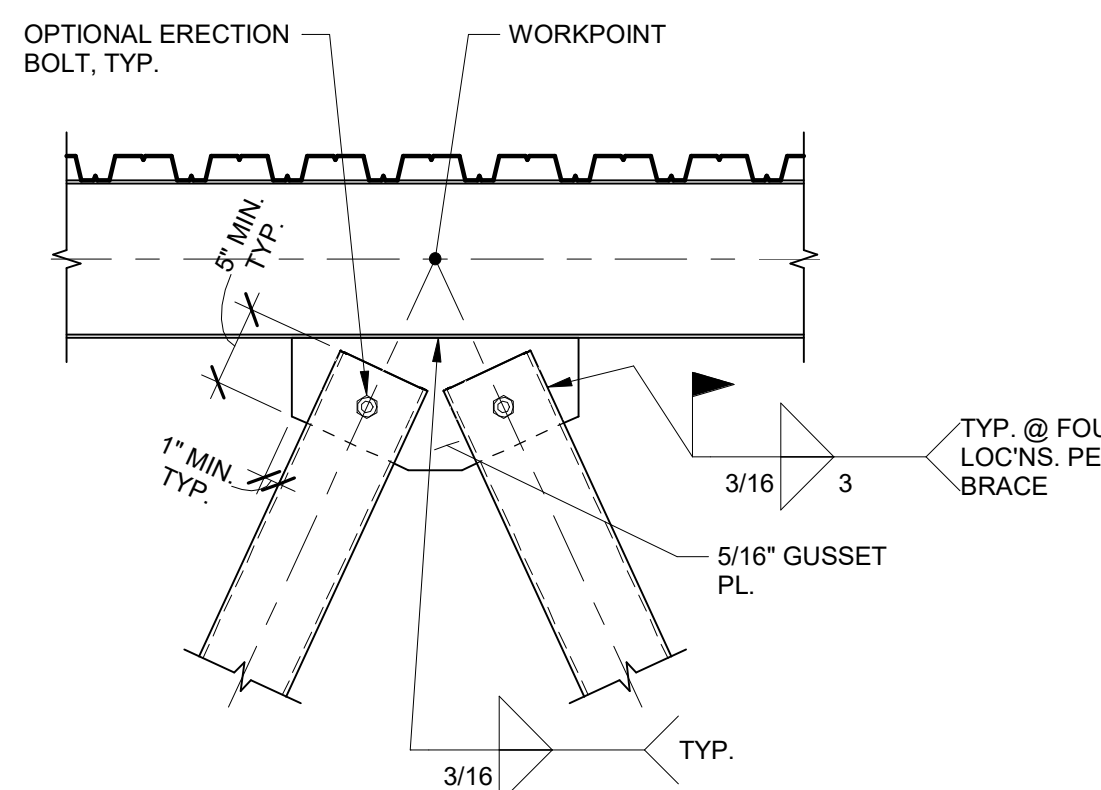
5 BRACE DETAIL
S5.50 1" = 1'-0"



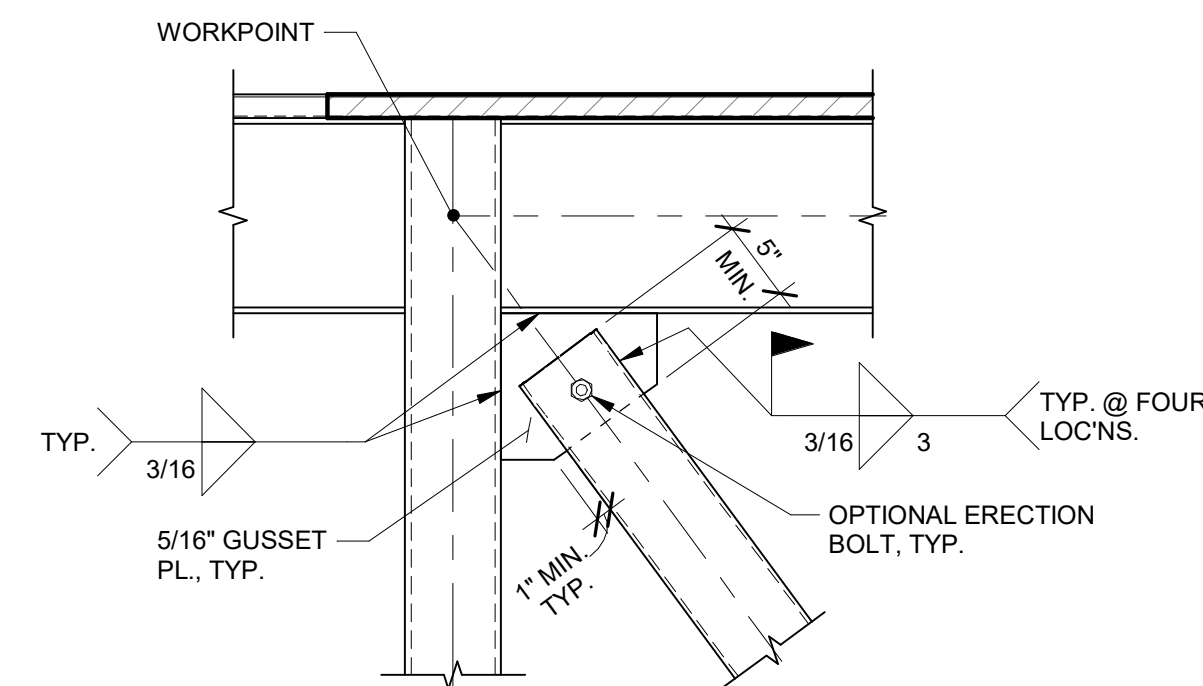
6 BRACE DETAIL
S5.50 1" = 1'-0"



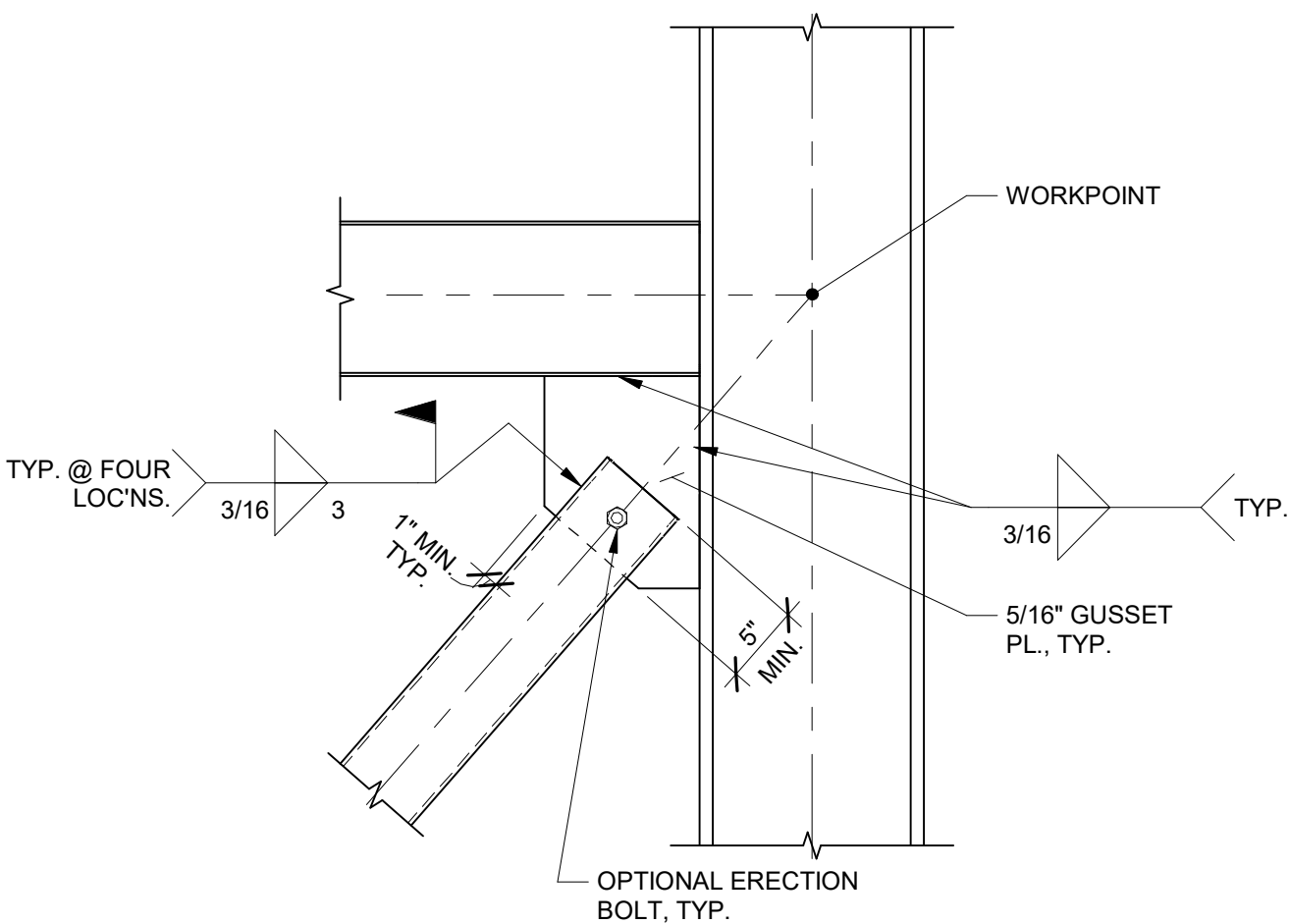
7 BRACE DETAIL
S5.50 1" = 1'-0"



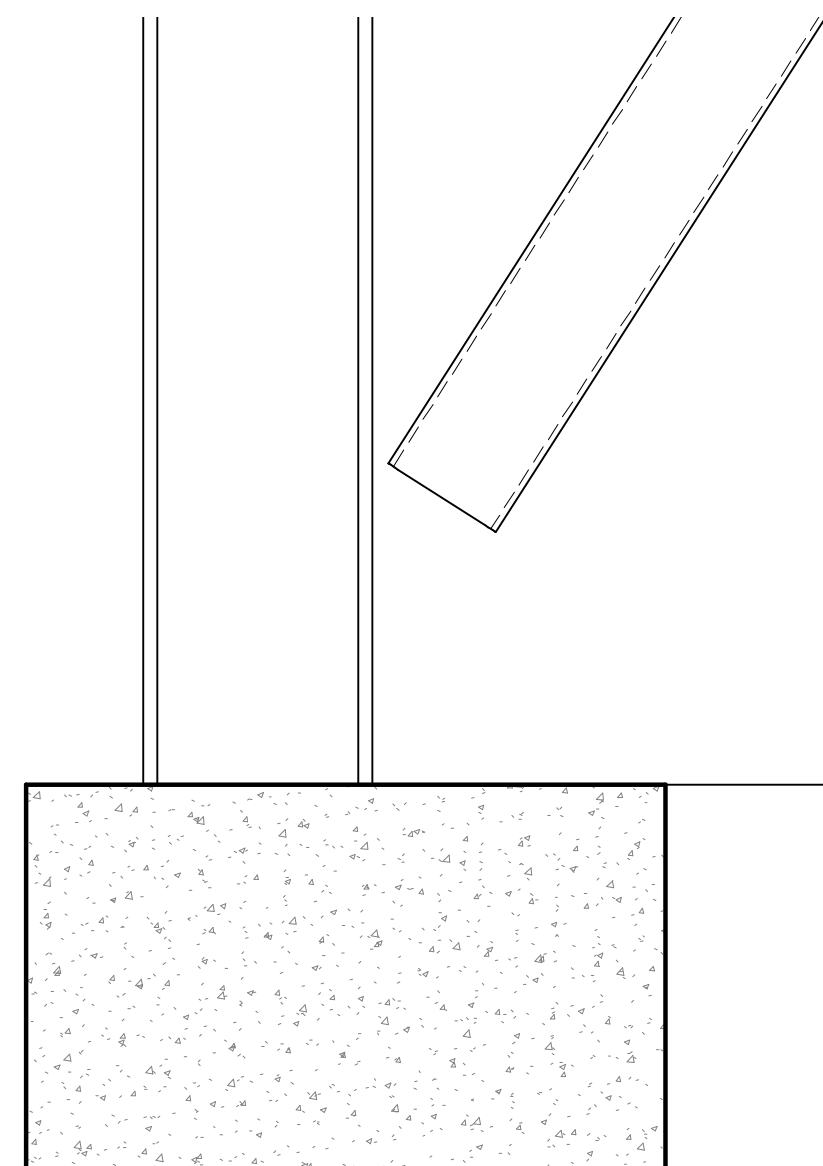
8 BRACE DETAIL
S5.50 1" = 1'-0"



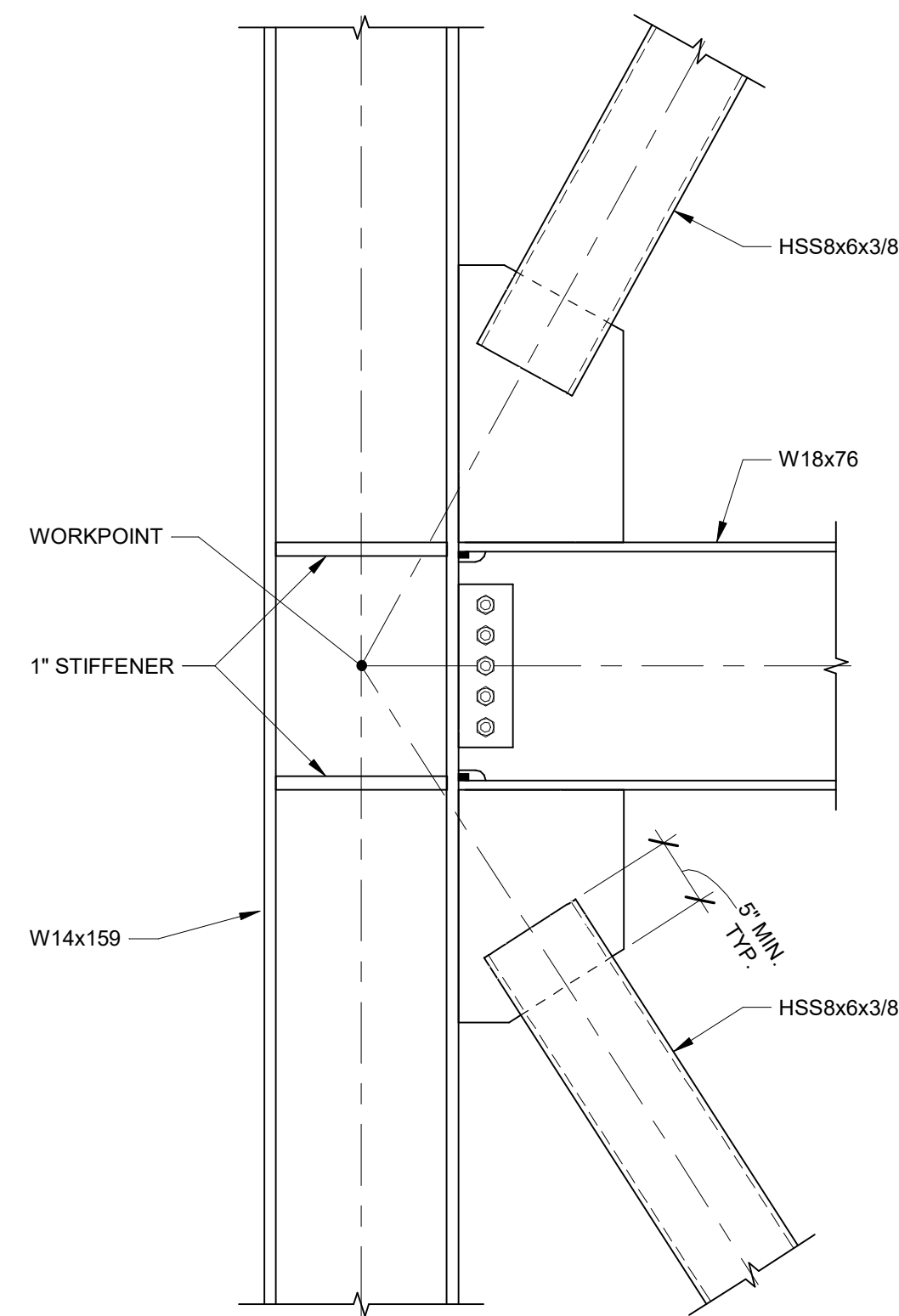
9 BRACE DETAIL
S5.50 1" = 1'-0"



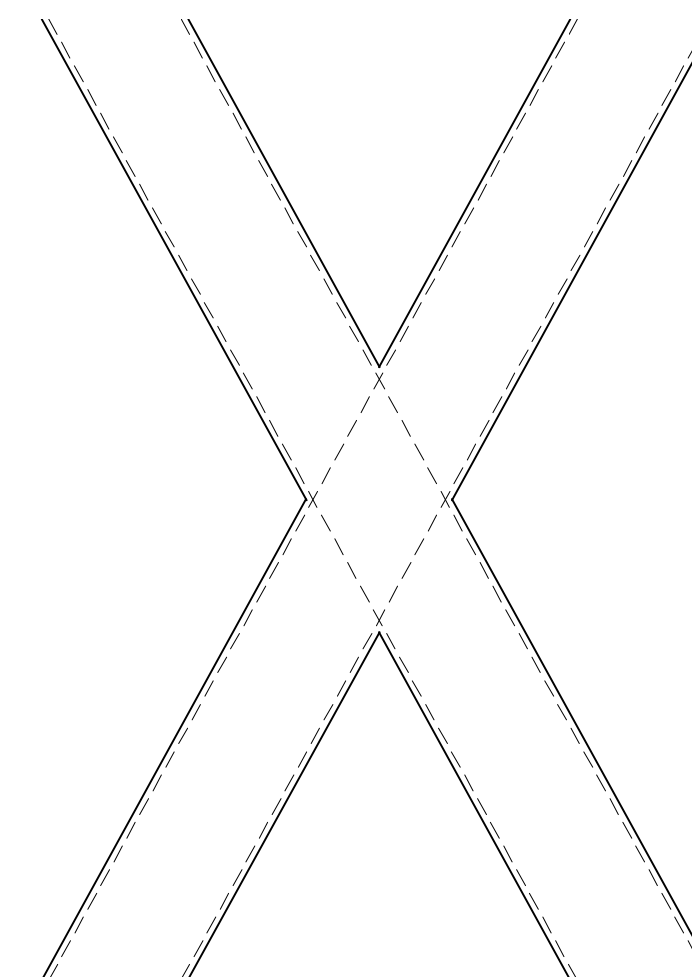
10 BRACE DETAIL
S5.50 1" = 1'-0"



11 BRACE DETAIL
S5.50 1" = 1'-0"



12 BRACE DETAIL
S5.50 1" = 1'-0"



13 BRACE DETAIL
S5.50 1" = 1'-0"

NO.	ISSUE	DATE

TYLER STREET PARKING GARAGE
619 WEST 12TH AVENUE, COVINGTON, LA 70433
AND
ST. TAMMANY HEALTH SYSTEM
NORTHLAKE SURGICAL ASSOCIATES
1028 SOUTH TYLER STREET, COVINGTON, LA 70433
AT
ST. TAMMANY PARISH HOSPITAL

CONSTRUCTION
DOCUMENTS

Date
05 NOV 2025

STEEL BRACED FRAME
DETAILS

Job No. 222058.01

Drawn by
Author

Checked by
Checker

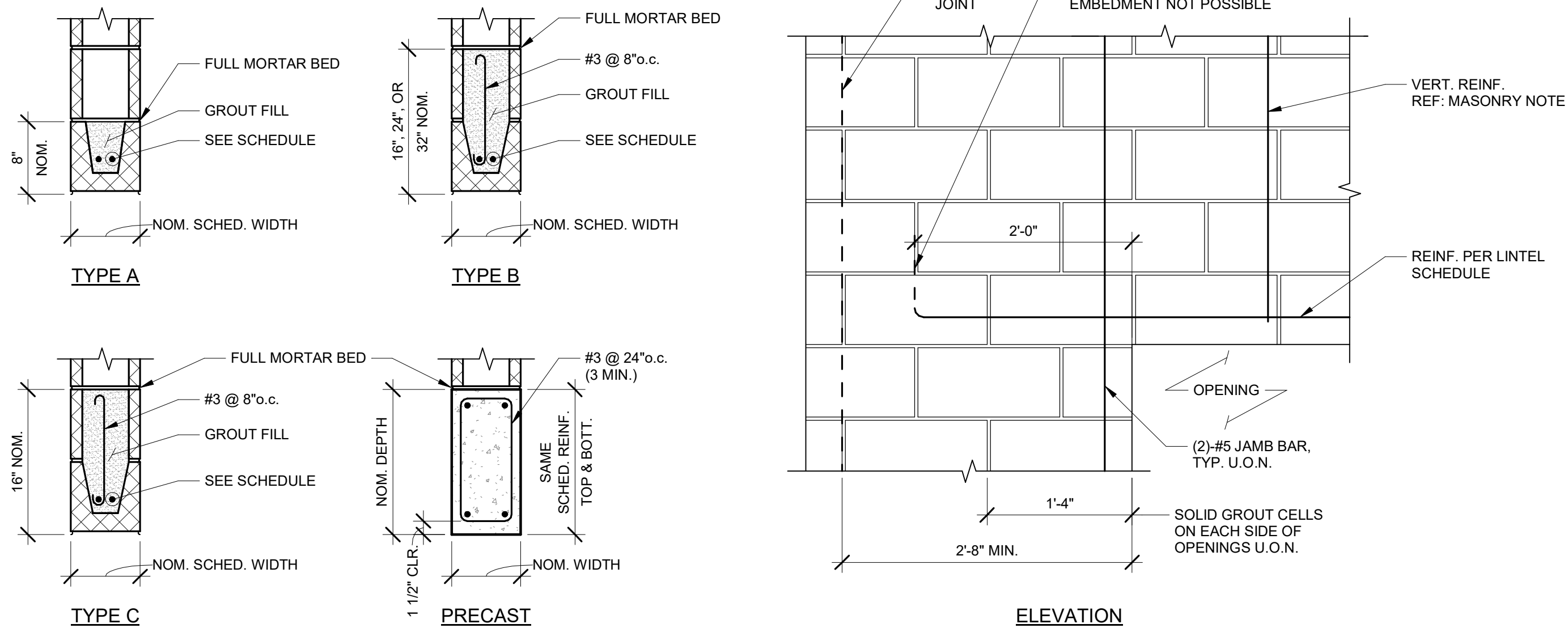
S5.50

Sheet No.

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CMU LINTEL SCHEDULE						
CLEAR SPAN	REINFORCING REQUIREMENTS IN NOMINAL LINTEL SECTION					
	6x8	6x16	8x8	8x16	8x24	8x32
3-4	(1)-#3	----	(2)-#4	----	----	----
4-8	(1)-#4 (4'-0")	----	(2)-#4 (4'-0")	(2)-#4	----	----
6-0	(2)-#4 (5'-0")	(1)-#4	N/A	(2)-#4	----	----
7-4	N/A	(1)-#5	N/A	(2)-#4	(2)-#4	----
8-8	N/A	(1)-#6	N/A	(2)-#6	(2)-#5	----
10-0	N/A	(1)-#7	N/A	(2)-#6	(2)-#5	----
11-4	N/A	N/A	N/A	(2)-#6 (5'-0")	(2)-#6	----
12-8	N/A	N/A	N/A	(2)-#6 (3'-0")	(2)-#6	----
14-0	N/A	N/A	N/A	----	(2)-#6 (8'-0")	(2)-#6 (11'-0")

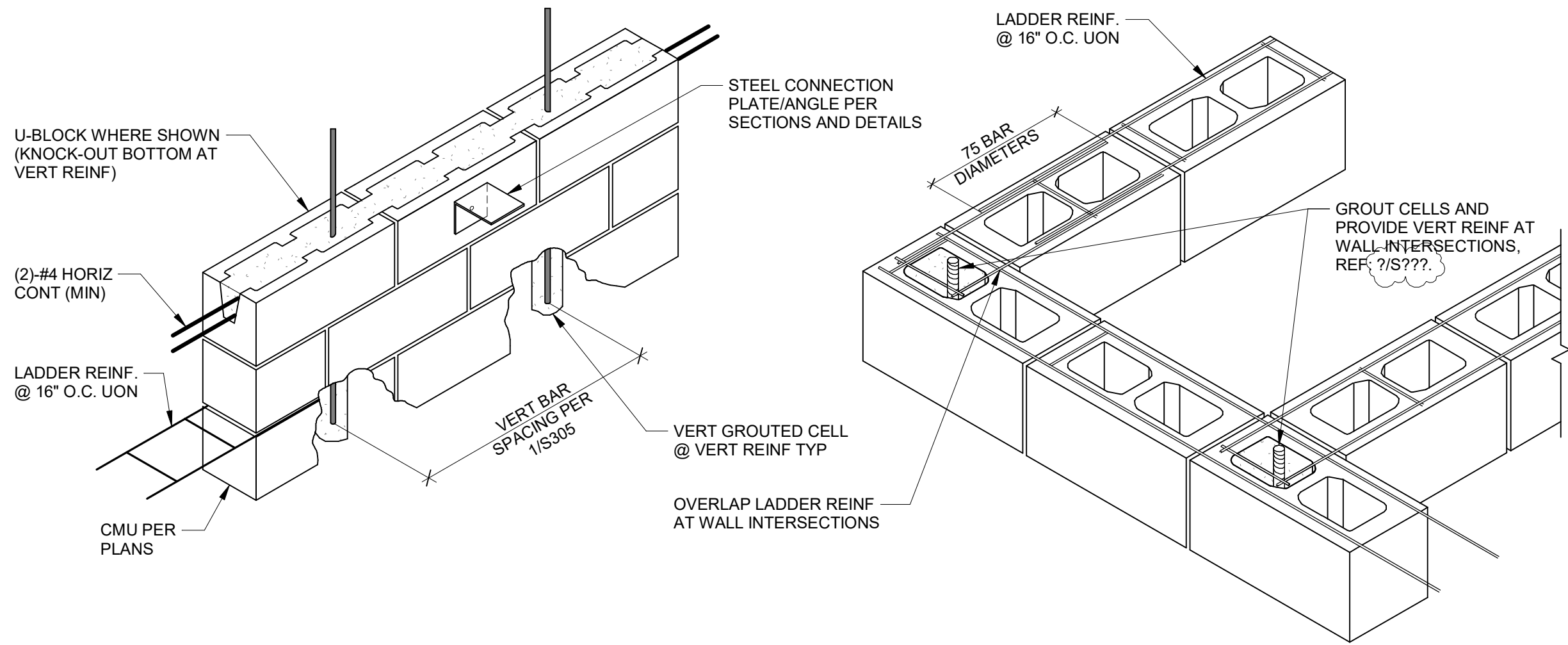
NOTES
1. SEE ARCHITECTURAL DWGS. FOR SIZE AND LOCATION OF OPENINGS.
2. PROVIDE 1'-4" OF BEARING.
3. MINIMUM MASONRY COMPRESSIVE STRENGTH OF GROUTED PRISM f_m = 1500 PSI 4. LINTELS SCHEDULED AS 16" HIGH MAY BE EITHER TYPE B OR TYPE C AS DETAILED BELOW.
4. PRECAST CONCRETE LINTELS AS SHOWN MAY BE USED WITH ARCHITECTURAL APPROVAL MINIMUM f_c = 3000 PSI.
5. 8x16 = NOMINAL WIDTH x MIN. DEPTH OF PRECAST LINTEL.
6. WHERE CRITICAL, MAXIMUM ALLOWABLE HT. OF CMU ABOVE LINTEL IS GIVEN IN PARENTHESIS AFTER THE REINF.



REINFORCED MASONRY NOTES:
1. ALL MASONRY WALLS TO HAVE CELLS REINFORCED AS SHOWN, U.N.O. FILL WITH 3000 PSI PEA GRAVEL MIX CONCRETE, NOT MORTAR, IN ACCORDANCE WITH TMS 602-16 AND TMS 402-16. PLACE CONCRETE IN 4'-0" LIFTS AT 9" TO 11" SLUMP (HIGH-RANGE WATER REDUCING ADMIXTURE, ASTM C494 TYPE F OR TYPE G). CONCRETE SHALL BE MIXED AT THE PLANT AND DELIVERED TO THE SITE BY A READY-MIX CONCRETE COMPANY. THE CONCRETE SHALL NOT BE MIXED ON SITE.
2. LAP ALL VERTICAL CMU REINFORCING A MINIMUM OF 30". ALL BARS SHALL BE TIED.
3. TESTING LABORATORY SHALL INSPECT EA. CELL PRIOR AND DURING CONCRETING, REF: SPECIAL INSPECTION SHEET.
4. AT MASONRY OPENINGS WITH MASONRY LINTEL ABOVE OPENINGS, PROVIDE DOWELS WITH 1'-6" LEG INTO LINTEL, MATCH VERT STEEL SIZE & REINF.
5. SUBMIT METHOD OF GROUTING PROCEDURE TO ENGINEER FOR REVIEW.
6. PRISM STRENGTH OF CMU (f_m) TO BE 1500 PSI GROSS OR 2500 PSI NET ON UNIT.
7. ALL MASONRY SHALL HAVE STANDARD HORIZONTAL LADDER REINFORCING AT 16" o.c. GALV.
8. MORTAR TO BE TYPE S - ASTM C 270.
9. EXTERIOR WALL VERTICAL REINFORCING TO BE #5 @ 16" O.C. W/ MATCHING FOUNDATION DOWELS UNLESS NOTED OTHERWISE. BARS TO BE CENTERED WITHIN CELLS. PROVIDE VERTICAL BARS AT CORNERS, WALL ENDS, WALL INTERSECTIONS, AND EACH SIDE OF OPENINGS (PER TYP DETAIL).
10. INTERIOR WALL VERTICAL REINFORCING TO BE #4 @ 16" O.C. MAX. W/ MATCHING FOUNDATION DOWELS UNLESS NOTED OTHERWISE. BARS TO BE CENTERED WITHIN CELLS. PROVIDE VERTICAL BARS AT CORNERS, WALL ENDS, WALL INTERSECTIONS, AND EACH SIDE OF OPENINGS (PER TYP DETAIL).
11. REFER TO MASONRY LINTEL SCHEDULE FOR LINTEL SIZE AND DETAILS FOR ALL LINTELS OVER MASONRY OPENINGS SHOWN ON PLAN.
12. REFER TO THE ARCHITECTURAL DRAWINGS FOR THE ROUGH MASONRY OPENING DIMENSIONS.
13. REFER TO ARCHITECTURAL DRAWINGS FOR BOTTOM OF MASONRY LINTEL ELEVATION DIMENSIONS.
14. MASONRY LINTELS ARE TO BEAR A MINIMUM OF 1'-4", UNLESS OTHERWISE NOTED, ON EITHER SIDE OF MASONRY OPENING. HORIZONTAL REINFORCEMENT TO PROJECT 2'-0" INTO JAMB.
15. MAXIMUM SPACING OF CMU CONTROL JOINTS NOT TO EXCEED 25'-0".
16. ALL CMU WALLS TO HAVE AN 8" DEEP BOND BEAM AT THE TOP OF THE WALL WITH A MINIMUM OF (2) #4 CONTINUOUS HORIZONTAL BARS, UNLESS NOTED OTHERWISE.
17. ALL CMU WALLS TO HAVE AN 8" DEEP BOND BEAM AT WINDOW SILLS WITH A MINIMUM OF (2) #4 HORIZONTAL BARS EXTENDING 24" PAST OPENING, UNLESS NOTED OTHERWISE.

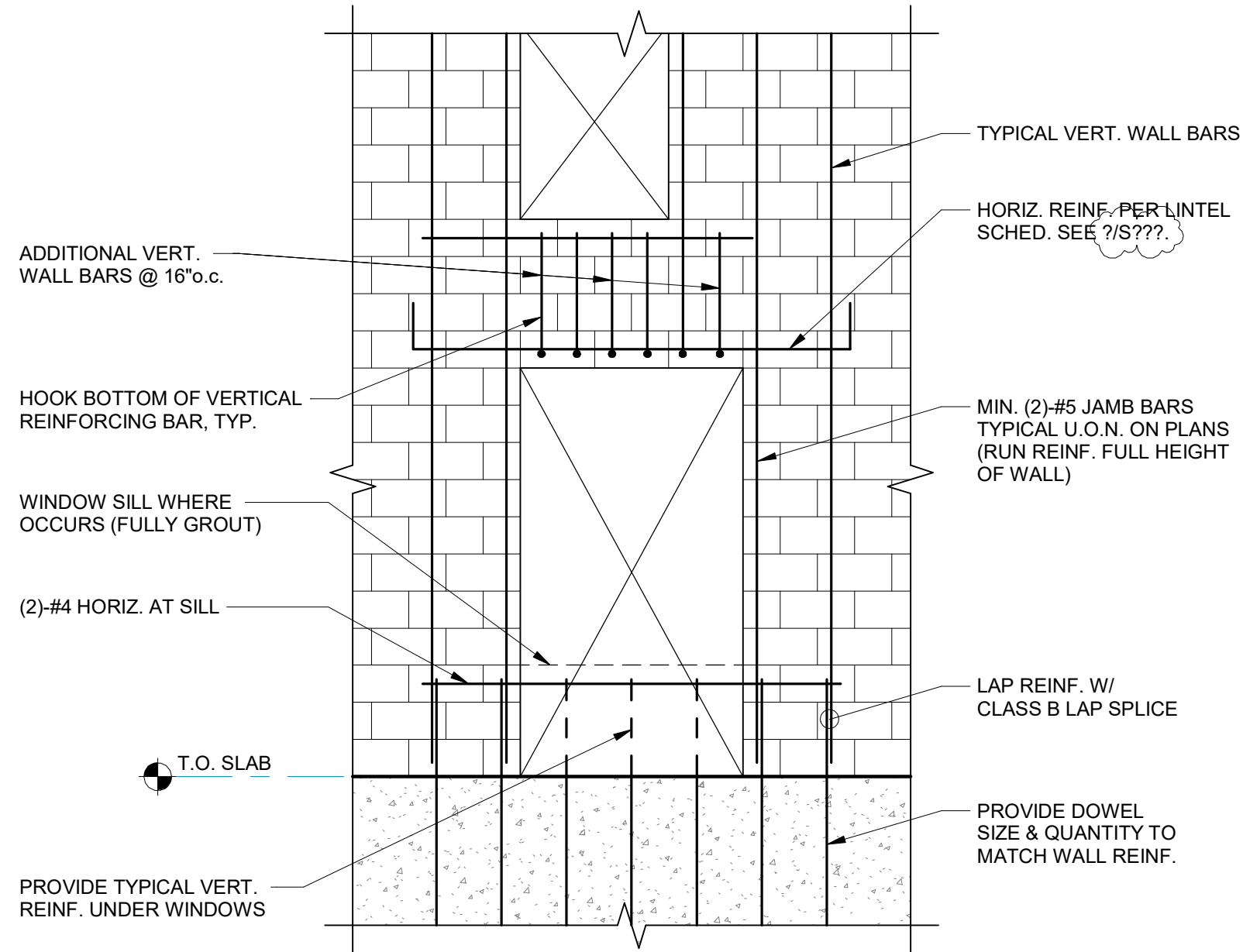
1 NON-LOAD BEARING CMU LINTEL

S6.00 1" = 1'-0"



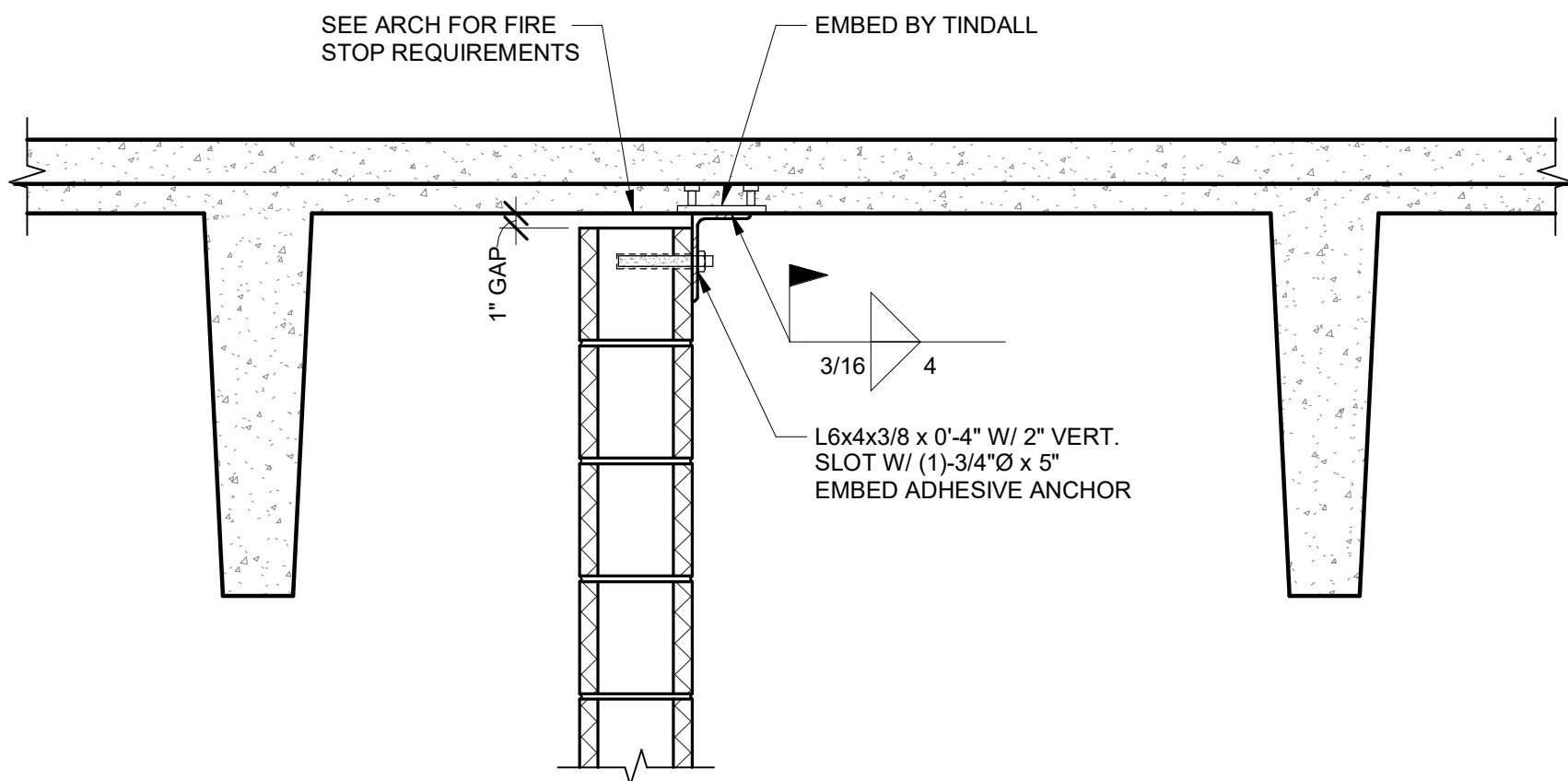
2 TYPICAL NON-LOAD BEARING CMU WALL

S6.00 1" = 1'-0"



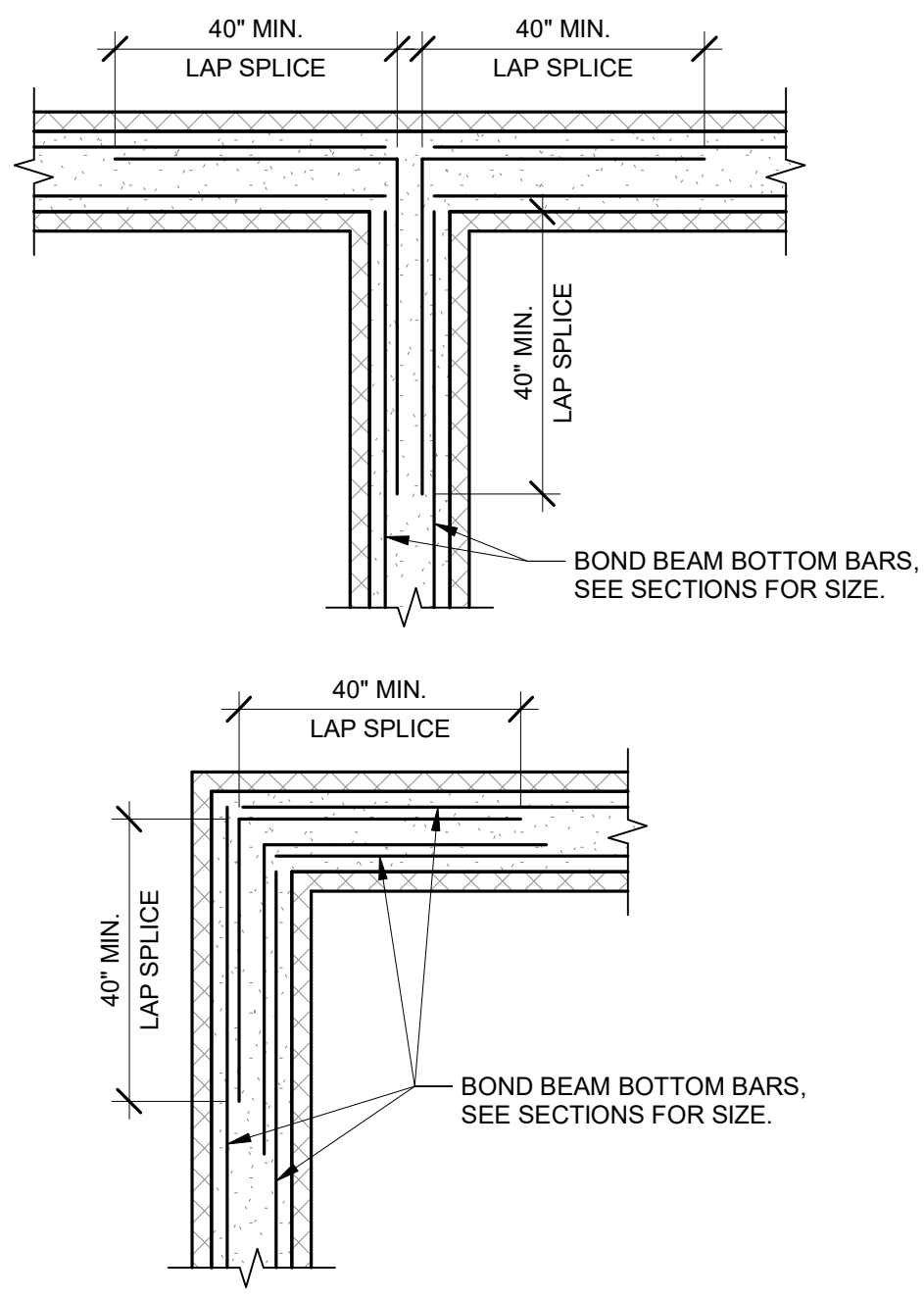
5 TYPICAL OPENING IN CMU WALL

S6.00 3/4" = 1'-0"



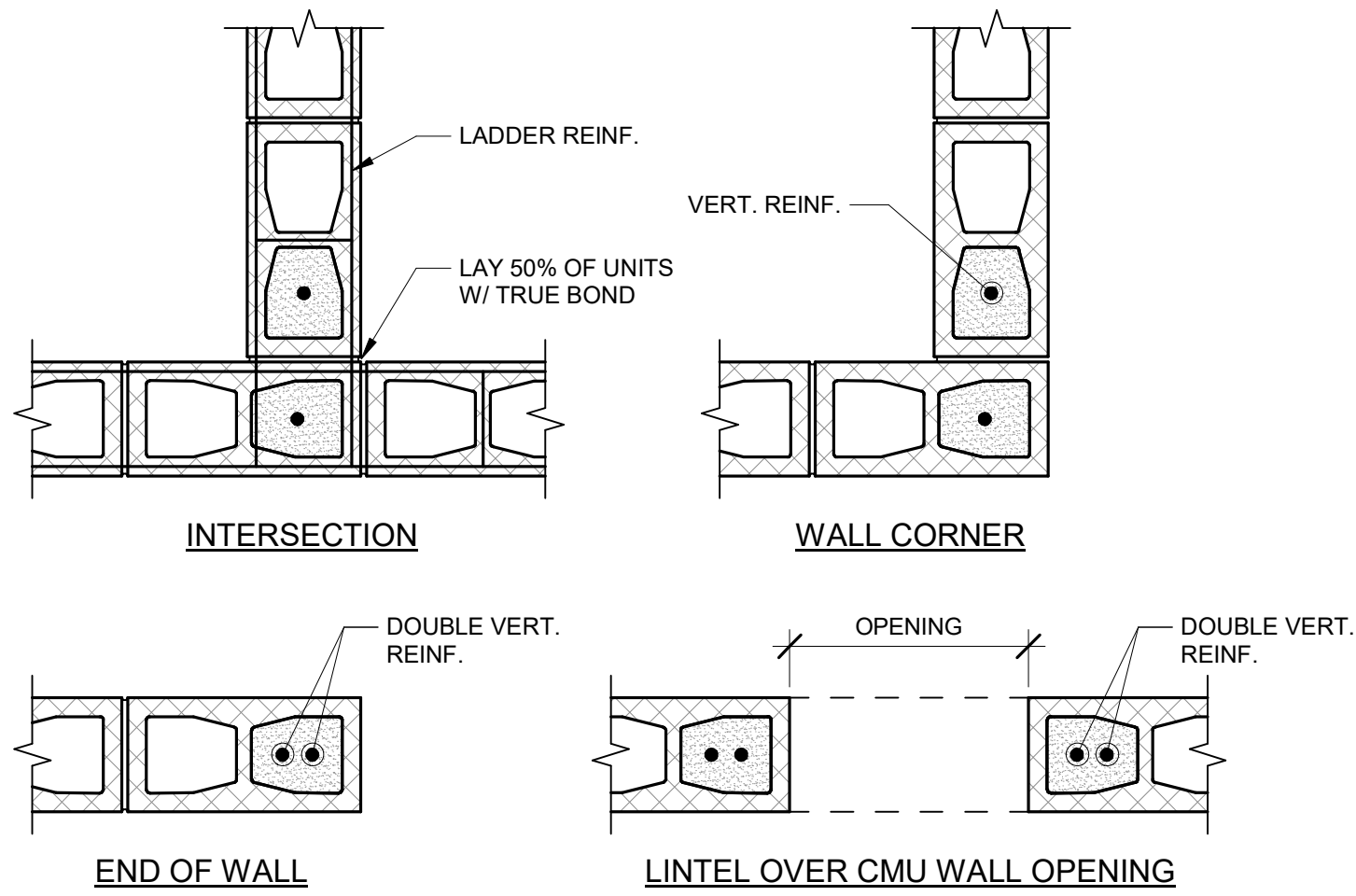
6 TOP OF WALL BRACING - WALL PARALLEL TO DTs

S6.00 1" = 1'-0"



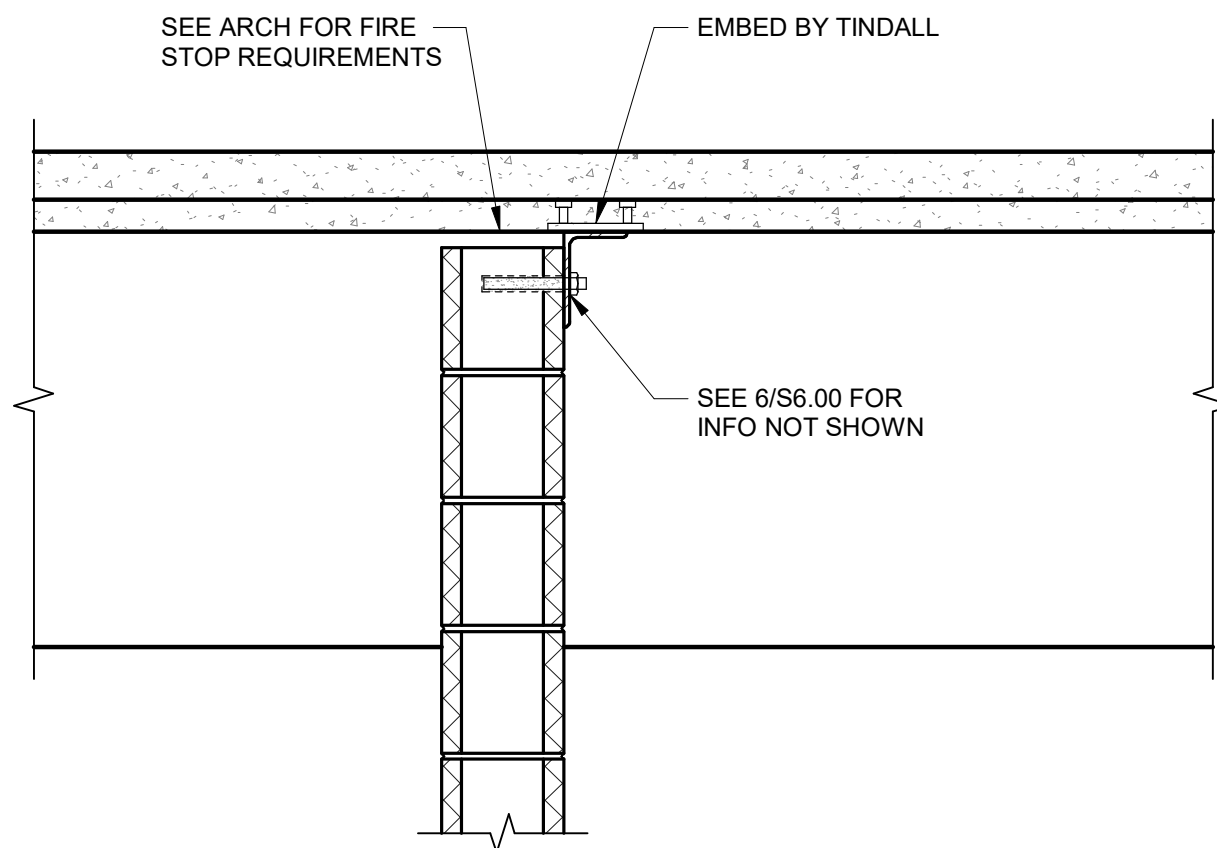
3 TYPICAL U-BLOCK INTERSECTION

S6.00 1" = 1'-0"



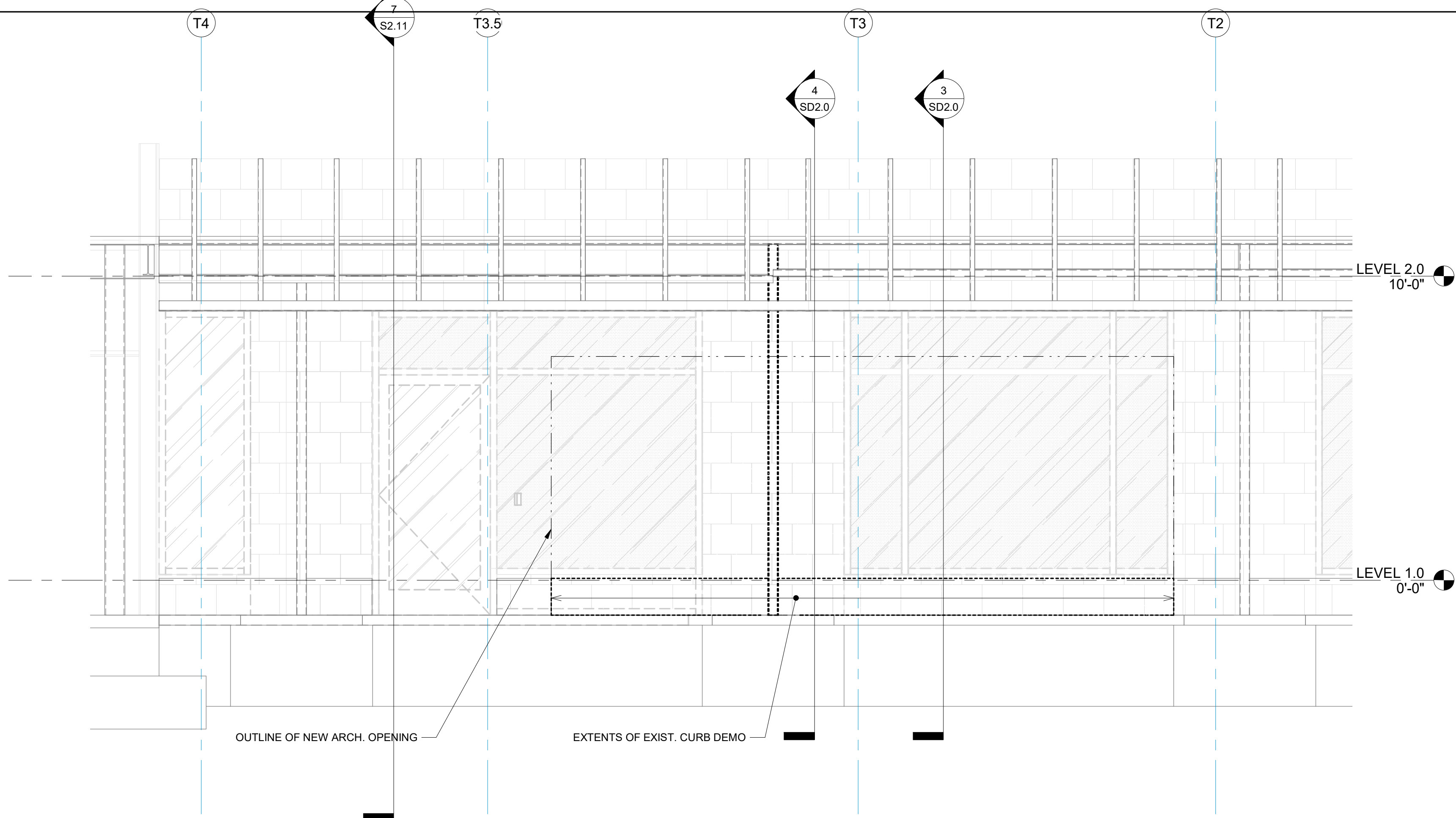
4 TYPICAL REINFORCED WALL INTERSECTION

S6.00 1" = 1'-0"

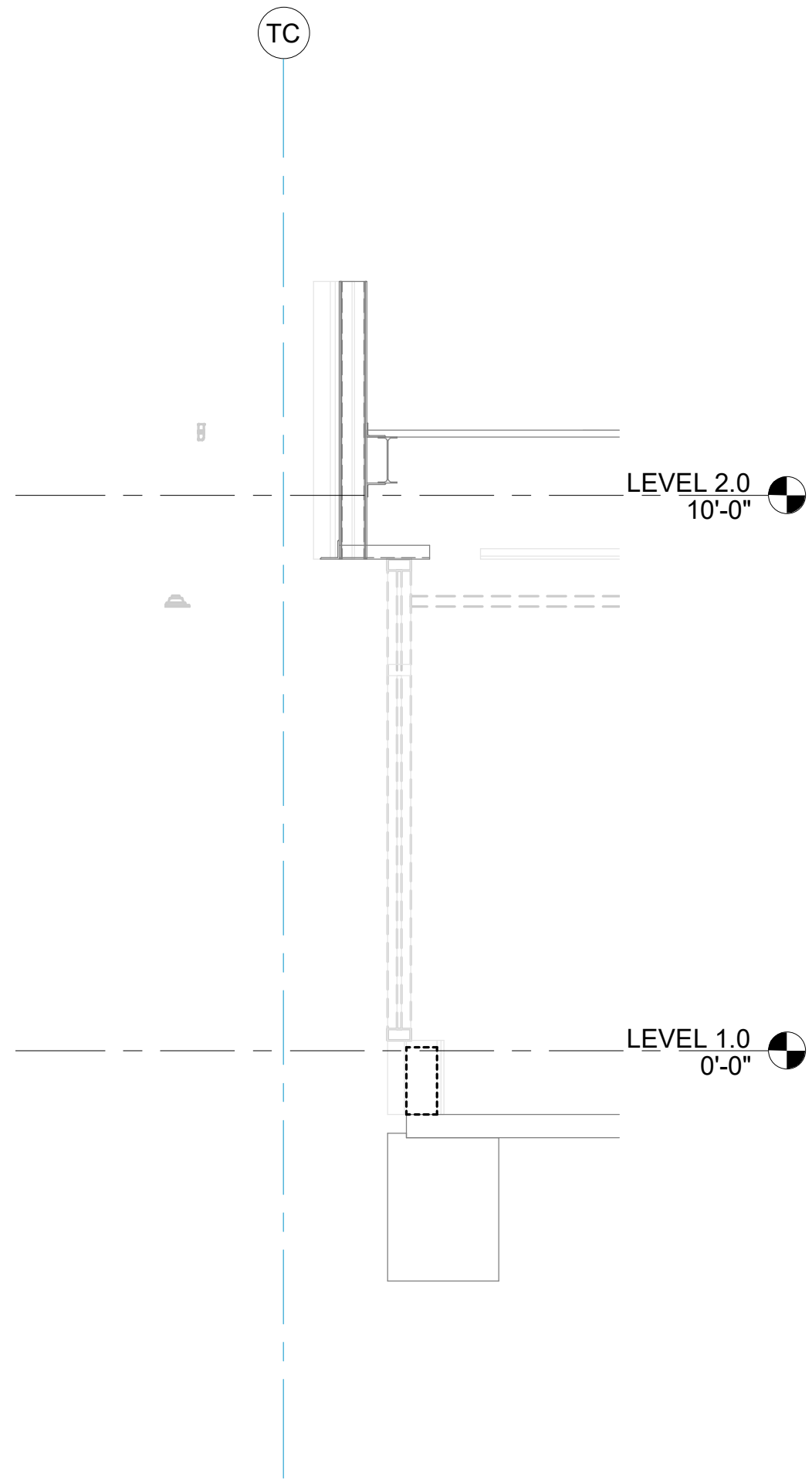


7 TOP OF WALL BRACING - WALL PARALLEL TO DTs

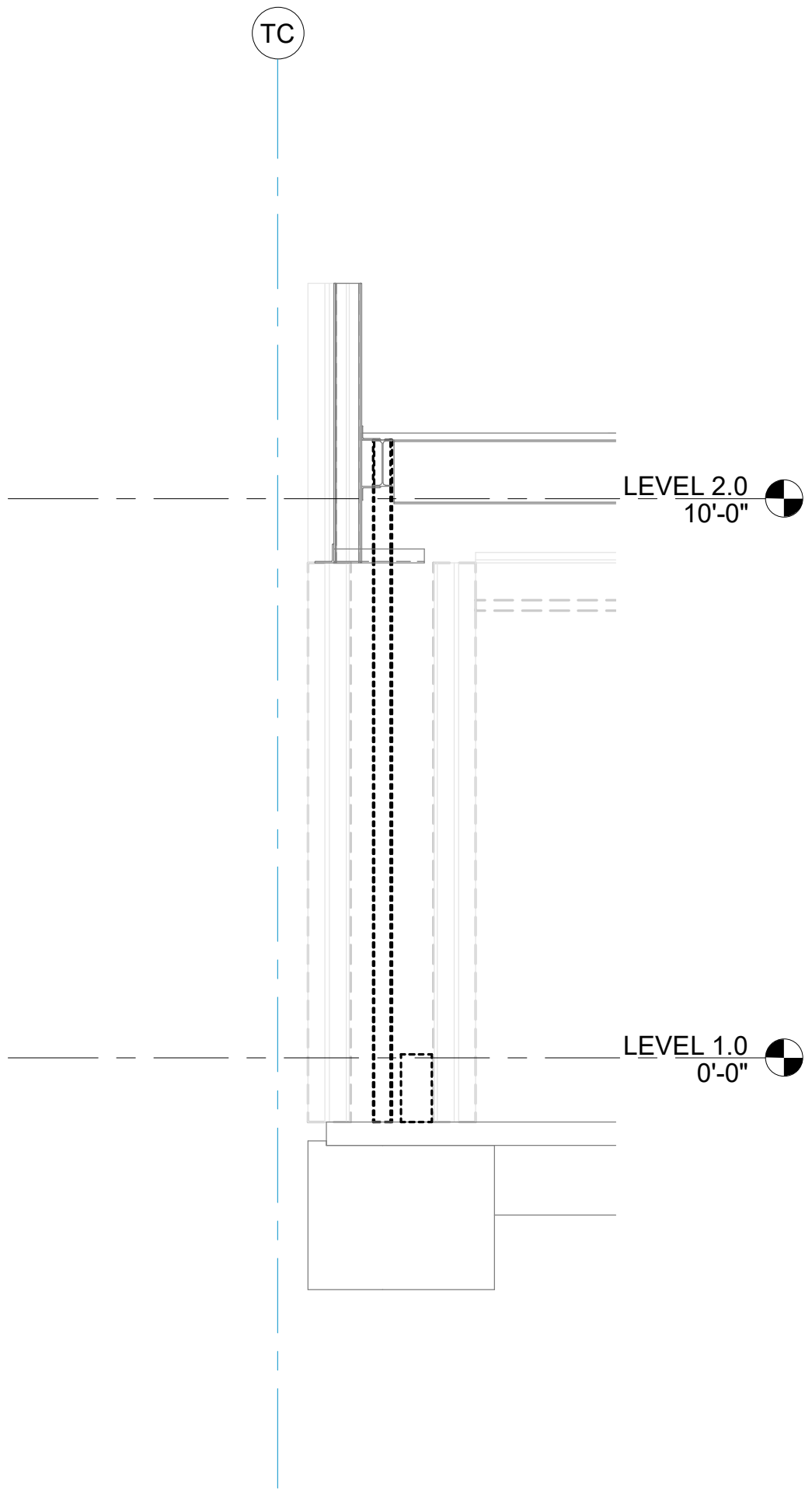
S6.00 1" = 1'-0"



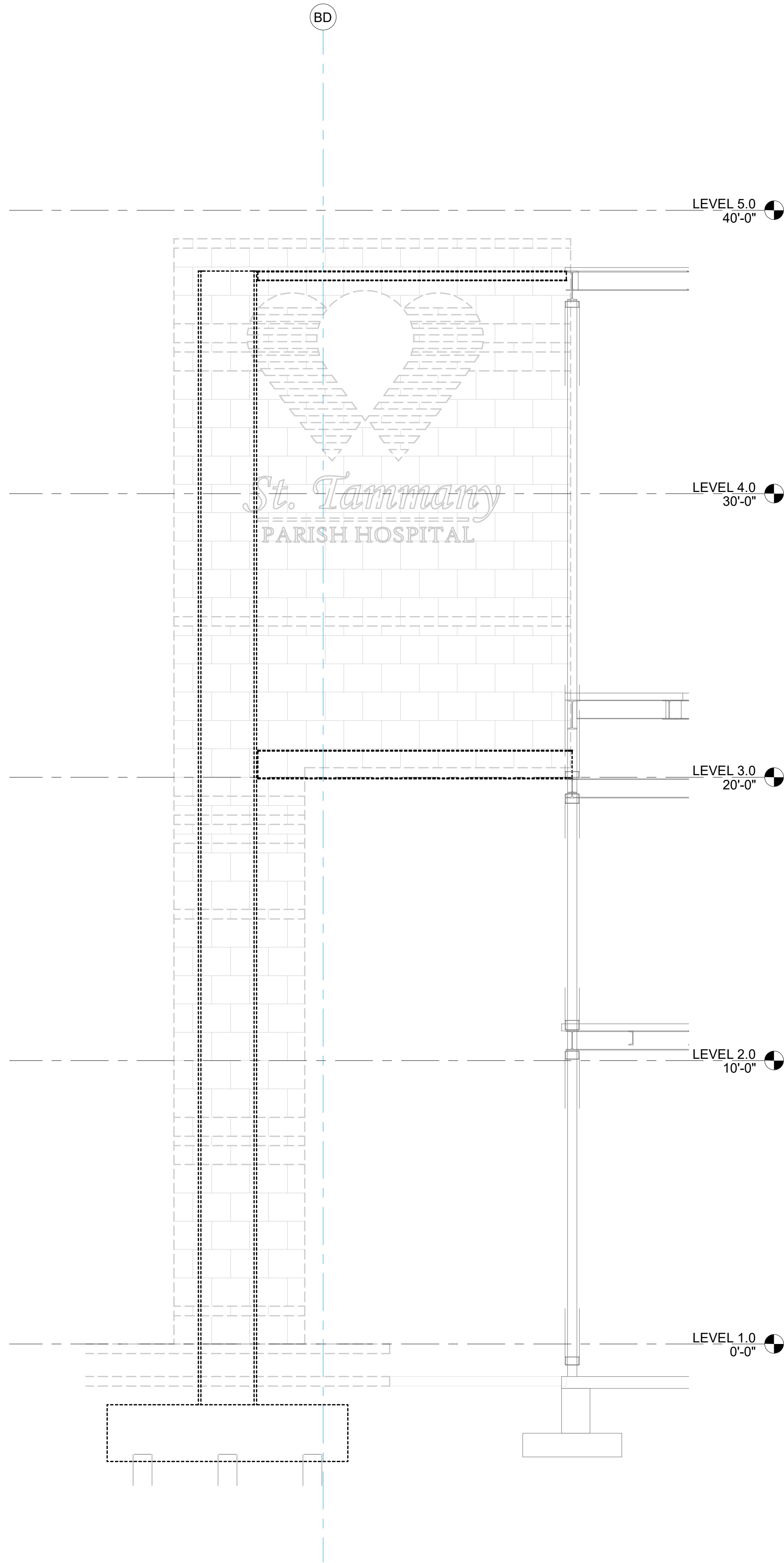
1 DEMO ELEVATION
SD2.0 3/8" = 1'-0"



3 DEMO SECTION @ EXIST. WINDOW
SD2.0 3/8" = 1'-0"



4 DEMO SECTION @ EXIST. MASONRY PILASTER
SD2.0 3/8" = 1'-0"



2 DEMO SECTION @ EXIST. STAIR
SD2.0 3/8" = 1'-0"

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Professional Engineer
JOHN B. BROWN
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Exp. 12/31/2025

DATE

NO. ISSUE

TYLER STREET PARKING GARAGE
619 WEST 12TH AVENUE, COVINGTON, LA 70433
AND
ST. TAMMANY HEALTH SYSTEM
NORTHLAKE SURGICAL ASSOCIATES
1028 SOUTH TYLER STREET, COVINGTON, LA 70433
AT
ST. TAMMANY PARISH HOSPITAL

CONSTRUCTION DOCUMENTS

Date
05 NOV 2025

DEMO ELEVATIONS & SECTIONS

Job No. 222058.01

Drawn by
Author

Checked by
Checker

SD2.0
Sheet No.

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