

1. STRUCTURAL STEEL SHAPES AND PLATES SHALL CONFORM TO THE FOLLOWING:

ITEM	ASTM SPECIFICATION
WIDE FLANGE	A992, GRADE 50
TUBE STEEL (HSS)	A500, GRADE B
STEEL PIPE	A53, GRADE B
CHANNEL	A36
ANGLES, PLATES, MISC. STL.	A36

2. ALL BOLTS FOR STRUCTURAL STEEL MEMBERS SHALL CONFORM TO THE FOLLOWING:

ITEM	ASTM SPECIFICATION
ANCHOR BOLTS	F1554, GRADE 55
CONNECTION BOLTS	ASTM F3125, GRADE A325
EXPANSION ANCHORS	ESR-1917 & ESR-3785 (HLTI KB-T2) ESR-3037 & ER-240 (SIMPSON STRONG-BOLT 2)
ADHESIVE ANCHORS	ASTM F1554, GRADE 55

3. STEEL BEAMS SHALL BE ERECTED WITH NATURAL CAMBER UP.

4. WHERE CAMBER OF STEEL BEAMS IS SPECIFIED ON THE PLANS, CAMBER OF STEEL BEAMS MUST COMPLY WITHIN LIMITS SET BY AISC. SUPPORT CAMBERED STEEL MEMBERS IN A MANNER THAT WILL NOT RESULT IN ANY LOSS OF CAMBER DURING SHIPPING AND HANDLING. THE TESTING LAB SHALL VERIFY THE SPECIFIED BEAM CAMBERS AT THE JOBSITE WITH THE BEAMS LYING FLAT ON THE GROUND (WEIR PARALLEL TO THE GROUND). BEAMS WHOSE CAMBER FALLS OUTSIDE THE TOLERANCE SHALL BE RETURNED TO THE SHOP FOR CORRECTION.

5. STEEL MEMBERS SHALL NOT BE SPICED EXCEPT WHERE INDICATED ON THE DRAWINGS UNLESS APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.

6. BOLTS FOR STEEL BEAM AND COLUMN CONNECTIONS SHALL BE 1/2" DIAMETER ASTM F 3125, GRADE A325-N HIGH-STRENGTH WITH HARDENED WASHERS. UNLESS NOTED OTHERWISE, ALL BOLTED CONNECTIONS ARE BEARING TYPE. ALL BOLTS SHALL BE TIGHTENED SNUG TIGHT, UNLESS NOTED OTHERWISE.

7. THE DESIGN OF THE STRUCTURAL STEEL CONNECTIONS, WHICH ARE NOT FULLY DETAILED IN THESE DRAWINGS, IS THE RESPONSIBILITY OF THE CONTRACTOR AND THE STEEL FABRICATOR. THE DESIGN OF THE CONNECTIONS SHALL BE PERFORMED BY A LICENSED PROFESSIONAL ENGINEER WORKING FOR THE FABRICATOR IN ACCORDANCE WITH THE LATEST ADOPTED EDITION OF THE AISC 303 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.

8. REFER TO THE SIMPLE SHEAR STEEL BEAM CONNECTION DETAIL FOR ADDITIONAL CONNECTION REQUIREMENTS AND INFORMATION.

9. UNLESS NOTED OTHERWISE, ALL MOMENT CONNECTIONS SHALL BE MADE WITH FULL PENETRATION WELDS AND SHALL BE DESIGNED TO DEVELOP THE FULL CAPACITY OF THE MEMBER.

10. PROVIDE DOUBLE NUTS AND DOUBLE WASHERS FOR STEEL COLUMN ANCHOR BOLTS TO ALLOW FOR ADJUSTMENT IN BASE PLATE ELEVATION. PROVIDE HIGH STRENGTH, NON SHRINK, NON METALLIC GROUT BELOW BASE PLATES.

11. ALL STRUCTURAL STEEL EXCEPT EMBEDDED ITEMS, SHALL BE PAINTED WITH ONE COAT OF RUST INHIBITIVE PAINT. DO NOT PAINT TOP FLANGE OF COMPOSITE STEEL BEAMS OR SURFACE OF COLUMNS TO WHICH SHEAR STUD CONNECTIONS WILL BE WELDED.

12. ALL STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED OR COATING. COOLING TOWER GRILLAGE SHALL BE GALVANIZED WITH G-150 COATING. ANY DAMAGE TO THE GALVANIC MATERIAL DURING WELDING SHALL BE TOUCHED UP WITH GALVANIZING REPAIR PAINT. HIGH ZINC-DUST-CONTENT PAINT FOR REGALVANIZING WELDS AND REPAIR PAINTING GALVANIZED STEEL WITH DRY FILM CONSTRUCTING NOT LESS THAN 90 PERCENT ZINC DUST BY WEIGHT, AND COMPLYING WITH DOD-P-21035A OR SSPC-Paint 20.

13. ARCHITECTUALLY EXPOSED STRUCTURAL STEEL (AESS): WHERE NOTED ON PLANS OR BY THE ARCHITECT SHALL CONFORM TO AISC 303, SECTION 10.

14. LEDGER ANGLE FOR FACIA BRAC SUPPORT SHALL BE INSTALLED SUCH THAT THE SWEEP OR CAMBER DOES NOT EXCEED 1/8 INCH DEVIATION FROM STRAIGHT LINE.

1. THE MASONRY DESIGN HAS BEEN BASED ON A PRISM STRENGTH, $f_m = 2,000$ psi (TYP. U.N.O.) AND $f_m = 1,750$ psi (TYP. AT INTERIOR PARTITIONS).
2. HOLLOW CONCRETE MASONRY UNITS SHALL MEET ASTM C90, NORMAL WEIGHT, MINIMUM COMPRESSIVE STRENGTH OF 2,000 psi ON THE NET AREA OF THE BLOCK.
3. CMU MORTAR SHALL MEET ASTM C270. PROVIDE TYPE I MORTAR FOR INTERIOR PARTITIONS ONLY; TYPE S FOR ABOVE-GRADE EXTERIOR WALLS AND TYPE M FOR BELOW GRADE WALLS.
4. CMU GROUT SHALL MEET ASTM C476 AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH THAT MEETS OR EXCEEDS f_m AT 28 DAYS (2,000 psi MINIMUM). AGGREGATE SIZE SHALL NOT EXCEED 1/2 INCH.
5. HORIZONTAL JOINT REINFORCEMENT SHALL CONFORM TO ASTM A651.
6. REINFORCE CMU WALLS AND PROVIDE FOUNDATION DOWELS PER DETAILS. LAP VERTICAL BARS PER MASONRY LAP SCHEDULE UNLESS NOTED OTHERWISE. LAP HORIZONTAL JOINT REINFORCEMENT MIN. 12 INCHES AND PROVIDE CORNER AND TEE PIECES.
7. PLACE REINFORCEMENT PRIOR TO GROUTING, AND SECURE VERTICAL REINFORCEMENT WITH POSITIONERS.
8. GROUT VERTICAL CELLS, BOND BEAMS AND LINTELS AS DETAIL.
9. LOCATE VERTICAL CONTROL JOINTS IN CMU WALLS AT MAX. 20' ON CENTER (COORDINATE WITH THE ARCHITECTURAL DRAWINGS FOR LOCATIONS). DISCONTINUE HORIZONTAL JOINT REINFORCEMENT AND BOND BEAM REINFORCEMENT AT CONTROL JOINTS. DO NOT DISCONTINUE LINTEL REINFORCING.

1. ALL CONCRETE REINFORCEMENT SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A615, GRADE 60.
2. REINFORCING STEEL THAT REQUIRES WELDING SHALL CONFORM TO ASTM A706, WITH GRADES AS SHOWN ABOVE.
3. ALL REINFORCING BARS SHALL BE LAPPED 40 BAR DIAMETERS (2" MINIMUM) AT SPICES UNLESS NOTED OTHERWISE ON PLANS OR DETAILS. DEVIATIONS IN SPICE LOCATIONS AND LENGTHS ARE NOT ALLOWED UNLESS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
4. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. ALL LAPS IN WELDED WIRE FABRIC SHALL BE ONE MESH PLUS TWO INCHES.
5. CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH ACI 315 AND ACI 318.
6. LAP SPICES OF CONTINUOUS BEAM REINFORCEMENT SHALL BE MADE AT MIDSPAN FOR TOP BARS AND AT SUPPORTS FOR BOTTOM BARS.
7. PROVIDE CORNER BARS AT INTERSECTIONS OF WALLS, GRADE BEAMS AND STRIP FOOTINGS IN ACCORDANCE WITH THE TYPICAL DETAILS. BARS THAT HOOK AT DISCONTINUOUS ENDS SHALL HAVE THE HOOK PLACED HORIZONTALLY AT EXTERIOR CORNERS IN LINE OF CORNER BARS.
8. MINIMUM CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE AS NOTED BELOW. REFER TO ACI 318 SECTION 20.6 FOR CONDITIONS NOT NOTED:

CONDITION	PROTECTION
FOOTINGS	3 IN.
SLAB-ON-GROUND	MID-SLAB
GRADE BEAMS & WALLS	2 IN. FORMED SIDES 3 IN. BACKFILLED SIDES

1. WELDED CONSTRUCTION SHALL CONFORM TO THE FOLLOWING AMERICAN WELDING SOCIETY STANDARDS:

ITEM	SPECIFICATION
STRUCTURAL STEEL	AWS D1.1
METAL DECK	AWS D1.3
REINFORCING STEEL	AWS D1.4
2. ELECTRODES FOR FIELD AND SHOP WELDS SHALL BE E70XX, U.N.O.
3. ELECTRODES FOR WELDING OF REINFORCING STEEL SHALL BE E80XX.
4. ELECTRODES FOR WELDING OF METAL DECK SHALL CONFORM TO AWS D1.3.
5. WHEN WELDS ARE NOT CALLED OUT ON THE DRAWINGS, THEY ARE MINIMUM SIZE FILLET WELDS PER AWS D1.1. FILLET WELDS NOT SPECIFIED AS TO LENGTH ARE CONTINUOUS.
6. ALL GROOVE WELDS ARE FULL PENETRATION UNLESS NOTED OTHERWISE.

1. SUBMIT ELEVATOR SHOP DRAWINGS WITH SHAFT AND SUPPORT REQUIREMENTS PRIOR TO FABRICATING AND CONSTRUCTING ANY PORTION OF THE SHAFT AND SUPPORTING STRUCTURE. THE SHAFT AND SUPPORTING STRUCTURE WAS DESIGNED BASED ON INFORMATION PROVIDED DURING THE DESIGN PHASE AND MUST BE VERIFIED TO HAVE ADEQUATE CAPACITY AND THE APPROPRIATE CONFIGURATION TO ACCOMMODATE THE SELECTED ELEVATOR MANUFACTURER'S EQUIPMENT.
2. ALL ELEVATOR PIT DIMENSIONS AND ROUGH FLOOR OPENINGS AND CEIL HEIGHTS FOR ELEVATOR MUST BE COORDINATED WITH FINAL SHOP DRAWINGS. ANY CHANGES TO DIMENSIONS SHOWN ON PLANS MUST BE CLEARLY INDICATED ON THE STRUCTURAL SHOP DRAWINGS FOR APPROVAL.
3. THE ELEVATOR REACTIONS SHOWN ON THE PLANS MUST BE CONFIRMED WITH THE FINAL ELEVATOR AND THE SUPPORTING SILL SHOP DRAWINGS. ANY CHANGES TO THE REACTIONS SHOWN MUST BE CLEARLY INDICATED ON THE STRUCTURAL SHOP DRAWINGS FOR APPROVAL.
4. ALL REQUIRED ELEVATOR EMBEDS MUST BE COORDINATED WITH THE ELEVATOR MANUFACTURER PRIOR TO CONSTRUCTION.
5. REFER TO TYPICAL DETAILS FOR ELEVATOR SILL ANGLE CONDITIONS.

1. ALL CONCRETE SHALL BE LABORATORY DESIGNED AND CONTROLLED AND SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF 'BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE' (ACI 318). SAND AND COARSE AGGREGATES SHALL MEET THE REQUIREMENTS OF ASTM C-33 OR C-330. PORTLAND CEMENT SHALL BE TYPE I OR II AND SHALL MEET THE REQUIREMENTS OF ASTM C-150. IN ADDITION, CONCRETE SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS:

USAGE	28-DAY STRENGTH (PSI)	SUMP (IN.)	MAX. AGG. SIZE (IN.)
FOOTINGS	4,000	4-6	1 1/2"
GRADE BEAMS	4,000	3-5	1"
SLAB-ON-GROUND	3,000	3-5	3/4"

2. CONCRETE PROPORTIONS SHALL BE ESTABLISHED FROM CONCRETE COMPRESSIVE STRENGTH TEST RECORDS FROM THE FACILITY THAT WILL SUPPLY CONCRETE FOR THIS PROJECT. TEST RECORDS SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF ACI 318 AND THE SPECIFICATIONS AND SHALL BE REFLECTIVE OF THE PERIOD OF THE YEAR DURING WHICH THE CONCRETE IS TO BE PLACED. CHANGING MIXTURE PROPORTION, PROPERTIES, MATERIALS, MIXES, COMBINATIONS, ADDITIONS OR ANYTHING WHICH IS A CHANGE IN THE APPROVED MIX DESIGN REQUIRES A NEW MIX DESIGN SUBMITTAL.

3. SLUMP AT THE POINT OF PLACEMENT SHALL NOT EXCEED AMOUNT SPECIFIED. DO NOT PLACE CONCRETE WITH SLUMP AND TEMPERATURE OUTSIDE THE LIMITS PROVIDED ON THE APPROVED MIX DESIGNS. USE OF CHLORIDES IN ANY ADMIXTURE IS NOT PERMITTED. SLUMP RANGE DOES NOT APPLY TO SUPERPLASTICIZED CONCRETE.

4. FLY ASH SHALL NOT BE USED AS A CONSTITUENT IN ARCHITECTURALLY EXPOSED CONCRETE. FLY ASH IS ALLOWED IN ALL OTHER NON-ARCHITECTURALLY EXPOSED CONCRETE. FLY ASH SHALL MEET ASTM C618 TYPE II'S REQUIREMENTS. THE WEIGHT OF THE FLY ASH SHALL BE ADDED TO THE WEIGHT OF THE CEMENT IN THE CALCULATION OF WATER CEMENT RATIO. THE CONTRACTOR SHALL CONFIRM IN THE MIX DESIGN SUBMITTAL THAT THE USE OF FLY ASH WILL NOT INTERFERE WITH THE PERFORMANCE OF OTHER PRODUCTS AND MATERIALS THAT WILL BE IN CONTACT WITH THE CONCRETE.

5. AIR ENTRAINMENT IN NORMAL WEIGHT CONCRETE IS REQUIRED ONLY IN CONCRETE PERMANENTLY EXPOSED TO WEATHER CONDITIONS. AIR ENTRAINMENT IS REQUIRED FOR ALL EXPOSURE CONDITIONS FOR WEATHER CONTACT CONCRETE. PERCENT AIR ENTRAINMENT SHALL BE 4.5% FOR USUMMS 1.5% IN NOT AIR-ENTRAIN INTERIOR FLOOR SLABS THAT RECEIVE HAND TROWEL FINISH OR POLISHED CONCRETE.

6. NON CHLORIDE ACCELERATING ADMIXTURE MAY BE USED IN CONCRETE SLABS PLACED AT AMBIENT TEMPERATURES BELOW 50 DEGREES FAHRENHEIT AT CONTRACTOR'S OPTION.

7. THE COMPRESSIVE STRENGTH OF GROUT BELOW BASE PLATES SHALL BE AT LEAST 7500 PSI.

8. CONTRACTOR MUST SUBMIT THE MIX DESIGN SUBMITTALS TO THE PROJECTS TESTING LABORATORY PRIOR TO SUBMITTING TO THE ARCHITECT AND/OR STRUCTURAL ENGINEER.

1. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI STANDARD "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318).
2. MAINTAIN SCHEDULED STRUCTURAL DEPTH OF MEMBERS AS A MINIMUM WHILE FOLLOWING SLOPES AND GRADES SHOWN ON PLANS.
3. ALL CONCRETE THAT WILL BE COLORED OR STAINED MUST BE WET CURED.
4. ALL CONSTRUCTION JOINTS IN BEAMS, JOISTS, SLABS, AND WALLS SHALL BE PROVIDED WITH SKEW KEYS AS SHOWN IN DETAILS. SUBMIT LOCATIONS OF ALL CONSTRUCTION JOINTS TO ENGINEER FOR REVIEW.
5. HOT WEATHER CONCRETING SHALL CONFORM TO ACI 308 AND COLD WEATHER CONCRETING SHALL CONFORM TO ACI 308.
6. FOUNDATION WALLS (INCLUDING LOADING DOCKS AND ELEVATOR FITS) SHALL HAVE TEMPORARY BRACING BRACED/SHORING, IF NEEDED AGAINST THEM. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL WALL IS PERMANENTLY BRACED BY DIAPHRAGMS (SLAB ON GROUND OR STRUCTURED SLAB) THAT HAVE REACHED AT LEAST 70% OF DESIGN STRENGTH, UNLESS NOTED OTHERWISE.
7. GRADE BEAM DIMENSIONS AND/OR LOCATIONS MAY NOT BE ALTERED WITHOUT APPROVAL OF THE ENGINEER OF RECORD.
8. FOUNDATION PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE ARCHITECT/ENGINEER.
9. PROVIDE WATERSTOPS AT ALL BELOW GRADE CONSTRUCTION JOINTS IN CONCRETE ELEMENTS WHERE THERE IS EARTH SUBGRADE AT THE JOINT ON ONE SIDE OF THE CONCRETE MEMBER AND INTERIOR SPACE ON THE OTHER, REGARDLESS OF WHETHER THE WATERSTOP IS SPECIFICALLY INDICATED IN THE DETAIL. PROVIDE WATERSTOPS AT OTHER LOCATIONS INDICATED IN THE DOCUMENTS PRIOR TO CONSTRUCTION.
10. NO CONDUIT, SLEEVE, OR PIPE IS PERMITTED TO BE EMBEDDED IN THE CONCRETE WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER OR UNLESS SPECIFICALLY DETAILED OTHERWISE. ALL CONDUITS, PIPES, AND SLEEVES EMBEDDED IN CONCRETE SHALL COMPLY WITH THE SPACING AND SIZE REQUIREMENTS OF ACI 318.
11. REFER TO THE GEOTECHNICAL REPORT FOR RISK OF GROUNDWATER PRESENCE AND ITS POTENTIAL IMPACT ON NEW CONSTRUCTION. CONTRACTOR MUST CARRY AN APPROPRIATE ALLOWANCE IN THE BIDS FOR THE MEASURES SUCH AS SITE DEWATERING AND PIER CASINGS, ETC.

1. ALL ORGANIC AND DELETERIOUS MATERIAL, AS WELL AS ANY EXISTING PAVING, FOUNDATIONS, OR EXISTING FILL SHALL BE REMOVED FROM THE BUILDING PAD AREA PRIOR TO PLACEMENT OF FILL.
2. ALL FILL SHALL BE EXIST. ON-SITE MATERIALS OR SELECT FILL HAVING A PRIORITY BETWEEN 4-15 AND <35.
3. FILL SHALL BE PLACED IN LIFTS NOT EXCEEDING 4 INCHES AND BE COMPACTED TO BETWEEN 98-100% OF STANDARD PROCTOR (ASTM D6998) AT A MOISTURE CONTENT OF -2% TO -5% OF OPTIMUM.
4. TEST LABORATORY SHALL VERIFY THAT SOILS BELOW NEW FOOTINGS MEET THE MINIMUM ALLOWABLE BEARING PRESSURE SHOWN. IF NOT, CONTACT THE EIR FOR REVIEW.
5. VAPOR BARRIER SHALL BE PLACED AND LAPPED PER MANUFACTURER'S RECOMMENDATIONS.

1. FOUNDATION DESIGN AND SUBSURFACE INFORMATION IS BASED ON THE RECOMMENDATIONS GIVEN IN THE FOLLOWING GEOTECHNICAL INVESTIGATION REPORT:

GEOTECHNICAL CONSULTANT: TERRACON CONSULTANTS, INC.
REPORT NUMBER: E4H1500
REPORT DATE: NOVEMBER 12, 2021
SYSTEM TYPE: SHALLOW AND DEEP FOUNDATIONS
MOISTURE RETARDER: 15 MIL POLYETHYLENE BARRIER
2. THE CONTRACTOR MUST READ THE GEOTECHNICAL INVESTIGATION REPORT AND BE THOROUGHLY FAMILIAR WITH SITE AND SUBGRADE INFORMATION GIVEN THEREIN. ALL SUBGRADE PREPARATION AND FOUNDATION CONSTRUCTION SHALL BE PERFORMED IN STRICT COMPLIANCE WITH THE STRUCTURAL REQUIREMENTS AND THE GEOTECHNICAL REPORT, AND SHALL BE OBSERVED, TESTED, AND APPROVED BY A QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO PROCEEDING WITH FOUNDATION CONSTRUCTION.

1. ALL ORGANIC AND DELETERIOUS MATERIAL, AS WELL AS ANY EXISTING PAVING, FOUNDATIONS, OR EXISTING FILL SHALL BE REMOVED FROM THE BUILDING PAD AREA PRIOR TO PLACEMENT OF FILL.
2. SOIL BELOW THE BUILDING PAD SHALL BE MOISTURE CONDITIONED AS NOTED BELOW:

DEPTH OF MOISTURE CONDITIONING:	1 FOOT BELOW FFE
EXTEND CONDITIONING BEYOND BUILDING BY:	10 feet
EXTEND CONDITIONING BEYOND BUILDING FLATWORK BY:	10 feet
3. ALL FILL SHALL BE PLACED IN LOOSE LIFTS AND COMPACTED AT ELEVATED MOISTURE CONTENT AS DETAILED IN THE ABOVE REFERENCED GEOTECHNICAL REPORT. CARE SHALL BE EXERCISED IN PLACEMENT AND COMPACTION OF FILL IN ALL LEAVE-OUTS. VAPOR BARRIER SHALL BE PLACED AND LAPPED PER MANUFACTURER'S RECOMMENDATIONS.
4. THE FINAL BUILDING PAD SHALL BE APPROVED BY THE OWNER'S GEOTECHNICAL REPRESENTATIVE PRIOR TO POURING CONCRETE.

SHOP DRAWINGS SHALL BE PREPARED FOR ALL STRUCTURAL COMPONENTS. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST CONSTRUCTION DOCUMENTS.

- CONTRACTOR SHALL ALLOW TWO WEEKS FOR THE ENGINEER'S REVIEW OF EACH SUBMITTAL. SUBMITTALS WHICH DO NOT REFLECT THE CONTRACTOR'S APPROVAL, SIGNATURE AND DATE, OR DO NOT APPEAR TO HAVE BEEN REVIEWED BY THE CONTRACTOR WILL BE RETURNED WITHOUT REVIEW. THE RESPONSIBLE CONTRACTOR SHALL BE RESPONSIBLE FOR DELAYS CAUSED BY REJECTION OF IN-ADEQUATE OR INCORRECT SHOP DRAWINGS.
- OMISSION FROM THE SHOP DRAWINGS OF ANY REQUIREMENTS AND/OR CORRECTIONS/COMMENTS ON THE SHOP DRAWINGS DURING REVIEW SHALL NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE REQUIREMENTS OF THE CONSTRUCTION DOCUMENTS EVEN IF THE SHOP DRAWINGS HAVE BEEN REVIEWED AND RETURNED FOR APPROVAL. COMPLIANCE WITH THE STRUCTURAL CONSTRUCTION CONTRACT DOCUMENTS ONLY. APPROVAL ASSUMES NO RESPONSIBILITY FOR DIMENSIONS, QUANTITIES AND MATERIALS THAT PERFORM AS FABRICATED. APPROVAL OF THE SHOP DRAWINGS DOES NOT RELIEVE THE CONTRACTOR OF APPROVAL OF A SPECIFIC ITEM SHALL NOT INCLUDE APPROVAL OF AN ASSEMBLY OF WHICH THE ITEM IS A COMPONENT. CALCULATION REVIEW AND COMMENTS DO NOT INFLUENCE A DETAILED CHECK OF THE CALCULATIONS.
- ALL ITEMS DEVIATING FROM THE STRUCTURAL DRAWINGS OR FROM PREVIOUSLY SUBMITTED SHOP DRAWINGS SHALL BE CIRCLED.
- USE OF REPRODUCTIONS, OR ELECTRONIC FILES OF THE STRUCTURAL DRAWINGS FOR THE PREPARATION OF SHOP DRAWINGS IS NOT PERCEPTIBLE WITHOUT PRIOR WRITTEN AUTHORIZATION OF THE ENGINEER OR RECORDOR. IF SUCH AUTHORIZATION IS OBTAINED, DO NOT SUBMIT SHOP DRAWINGS WITH THE CONTRACT DOCUMENTS TITLE BLOCK AND/OR THE SEAL OF THE REGISTERED ENGINEER OR RECORDOR. ALTERATION OF A SEALED DOCUMENT WITHOUT PROPER NOTIFICATION OF THE RESPONSIBLE ENGINEER IS AN OFFENSE OF THE ENGINEERING PRACTICING ACT.

1. THE DRAWINGS AND SPECIFICATIONS DO NOT INDICATE THE MEANS AND METHODS OF CONSTRUCTION UNLESS NOTED OTHERWISE. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING ALL MEANS AND METHODS MEET THE REQUIREMENTS OF THE LATEST OSHA REGULATIONS.
2. ALL DETAILS AND SECTIONS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE, EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.
3. RESPONSES TO QUESTIONS AND RFI'S, COMMENTS MADE DURING THE REVIEW OF SUBMITTALS, AND DIRECTIVES PROVIDED IN ANY FORM BY THE ENGINEER TO THE CONTRACTOR DURING THE CONSTRUCTION PERIOD ARE INTENDED TO BE CLARIFICATIONS OF THE CONTRACT DOCUMENTS AND ARE NOT INTENDED TO REPRESENT A CHANGE IN COST OF THE PROJECT TO THE OWNER UNLESS A CHANGE ORDER DETAILING IS PROVIDED WITH DETAILED PRICING INFORMATION TO THE ARCHITECT BEFORE PURCHASING, ORDERING, FABRICATING, OR INSTALLING ANY COMPONENT RELATED TO SUCH CLARIFICATIONS AND CORRECTIONS.

THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES IF APPLICABLE, INCLUDING LAGGING, SHORING, AND PROTECTIVE MEASURES FOR EXISTING STRUCTURES, UTILITIES, STREETS AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.

5. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ASSEMBLE AND COORDINATE THE REQUIREMENTS OF ALL COMPONENTS OF THE CONTRACT DOCUMENTS AND VERIFY EXISTING CONDITIONS OF THE SITE. REF. ARCHITECTURAL AND ALL OTHER TRADES DOCUMENTS FOR SIZE AND LOCATION OF PIPES, VENTS, CHASES, DUCTS, OPENINGS, DEPRESSIONS, RECESSES, SLOPES, BLOCKOUTS, CURBS, EMBLEMENTS AND FINISHES NOT SHOWN ON THESE STRUCTURES. ALL SUCH CONDITIONS ARE TO BE CHECKED AND VERIFIED WITH THE ARCHITECTURAL DRAWINGS. THE GENERAL CONTRACTOR SHALL REPORT ANY DISCREPANCY BETWEEN THESE DOCUMENTS AND THOSE OF THE ARCHITECT AND ALL PROJECT CONSULTANTS OR ANY ERROR, OMISSION OR DIFFICULTY AFFECTING HIS WORK TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW.

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7. SPECIAL INSPECTIONS SHALL BE DONE BY A QUALIFIED, INDEPENDENT INSPECTION COMPANY. JOBSITE VISITS BY THE ENGINEER SHOULD BE CONSIDERED A PERIODIC CHECK TO INFORM THE OWNER OF DEFECTS IN THE WORK OF THE CONTRACTOR AND DO NOT CONSTITUTE, OR SUBSTITUTE, INSPECTIONS UNLESS SPECIFICALLY CONTRACTED FOR.
8. STRUCTURAL MEMBERS AND FOUNDATIONS SUPPORTING MECHANICAL EQUIPMENT HAVE BEEN DESIGNED FOR THE WEIGHT OF THE UNITS SHOWN IN THE STRUCTURAL DRAWINGS. ANY CHANGES IN SIZE, WEIGHT, OR COMPOSITION OF UNITS SHALL BE REPORTED TO THE ARCHITECT PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS OR RELATED MATERIALS.
9. NOTICE IN WRITING OF ANY PROPOSED SUBSTITUTIONS OR ANY PROPOSED DEVIATIONS TO THE STRUCTURE AS REQUIRED BY THESE DOCUMENTS SHALL BE SUBMITTED WITH A CURRENT ICC REPORT.
10. DIMENSIONS SHOWN IN STRUCTURAL PLANS, SECTIONS, AND DETAILS TAKE PRECEDENT OVER SCALE. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS NOT INDICATED IN THE STRUCTURAL DRAWINGS.
11. THESE PLANS MUST BE SUBMITTED TO THE GEOTECHNICAL ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION.
12. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN ALL CONTRACT DOCUMENTS AND LATEST ADDENDUM AND TO SUBMIT SUCH DOCUMENTS TO ALL SUBCONTRACTORS AND MATERIAL SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS, FABRICATION OF ANY STRUCTURAL MEMBERS, AND ERECTION IN THE FIELD.
13. THE CONTRACTOR IS RESPONSIBLE FOR ARRANGING PRECONSTRUCTION MEETINGS FOR THE FOUNDATION AND SUPERSTRUCTURE ELEMENTS WITH A MINIMUM OF TWO WEEKS OF NOTICE PRIOR TO START OF THE RELEVANT WORK. ATTENDANCE SHALL INCLUDE THE CONTRACTORS, APPROPRIATE SUBCONTRACTORS, FABRICATORS, INSPECTORS, ARCHITECT/ENGINEERS. ON THE MEETING AGENDA SHALL BE REVIEW OF WORK SCOPE, PROJECT SCHEDULE OF THE ELEMENT IN QUESTION, CONTACT INFORMATION OF RESPONSIBLE PARTIES, INSPECTION POINTS, REVIEW OF MATERIALS AND ANY SPECIAL REQUIREMENTS, CLARIFICATIONS, TESTING AND ACCEPTANCE, AND ANY OTHER TOPICS DEEMED APPROPRIATE BY THE CONTRACTOR OR THE ARCHITECT.
14. THE ROOF STRUCTURE AND ITS SUPPORTING ELEMENTS HAVE BEEN DESIGNED WITH THE ASSUMPTION THAT SUFFICIENT DRAINAGE HAS BEEN PROVIDED TO PREVENT ANY PONDING OF WATER. IT IS THE RESPONSIBILITY OF THE BUILDING OWNER TO MAINTAIN THE ROOF DRAINAGE SYSTEM SUCH THAT IT

1. FOOTING DESIGNS ARE BASED UPON THE INFORMATION LISTED BELOW. REFER TO PLANS AND DETAILS FOR FOOTING DIMENSIONS AND REINFORCEMENT.
ALLOWING BEARING PRESSURE: 3000 psi
FOOTING BEARING DEPTH: 18" MINIMUM. REF: FOUNDATION PLANS
2. FOOTING CONSTRUCTION AND CONCRETING SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT. FULL FOOTING EXCAVATIONS WITH CONCRETE AS SOON AS PRACTICAL, BUT NO LONGER THAN 12 HOURS AFTER THE START OF EXCAVATION. IF THE BEARING SOILS ARE SOFTENED BY SURFACE WATER ABSORPTION OR EXPOSURE TO THE ENVIRONMENT, CONTACT THE GEOTECHNICAL ENGINEER REGARDING RECOMMENDATIONS FOR RE-WORKING BEARING MATERIAL, BENEATH FOOTINGS.
3. ALL FOOTINGS SHALL BE INSPECTED BY A REPRESENTATIVE OF A QUALIFIED GEOTECHNICAL LABORATORY OR OTHER PARTY TO INSURE THAT THE PROPER BEARING MATERIAL IS PRESENT BENEATH THE FOOTINGS AND HAD BEEN PREPARED IN ACCORDANCE WITH THE RECOMMENDATIONS GIVEN IN THE GEOTECHNICAL REPORT.
4. TEMPLATE SHALL BE UTILIZED TO SET DOWELS AND ANCHOR BOLTS IN FOOTINGS. DETAILS OF THE TEMPLATES SHALL BE PROVIDED IN THE FOOTING SHOP DRAWINGS.
5. THE EARTHWORK CONTRACT SHALL CONTAIN PROVISIONS FOR PENETRATION OR REMOVAL OF HARD ROCK AND OBSTRUCTIONS.
6. UTILITIES SHALL NOT BE LOCATED BELOW FOOTINGS AND SLEEVES ARE NOT PERMITTED THROUGH FOOTINGS.
7. FOOTING THICKNESS INDICATED IN THESE DRAWINGS ARE THE MINIMUM THICKNESSES REQUIRED FOR STRUCTURAL PERFORMANCE. IF ANY COLUMN ANCHOR BOLTS EXTEND WITHIN 3" OF BOTTOM OF FOOTING, THE CONTRACTOR SHALL LOWER THE BOTTOM OF FOOTING AS REQUIRED TO MAINTAIN A 3" OF CONCRETE BELOW THE ANCHOR BOLTS.
8. THE CONTRACTOR SHALL VERIFY THAT NO UTILITIES ARE LOCATED WITHIN FOOTINGS UNLESS SPECIFICALLY DETAILED IN THESE DRAWINGS.

1. THE GENERAL CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS AT THE SITE AND REPORT ANY DISCREPANCIES FROM ASSUMED CONDITIONS SHOWN ON THE DRAWINGS TO THE ARCHITECT AND ENGINEER PRIOR TO THE CONSTRUCTION AND ERECTION OF ANY MEMBER. DIFFERENCES SHALL BE CLOUDED IN THE SHOP DRAWINGS.
2. ANY MODIFICATIONS TO THE CONSTRUCTION DOCUMENTS THAT ARE NECESSARY AS A RESULT OF FIELD VERIFICATIONS PERFORMED BY THE CONTRACTOR SHALL BE IMMEDIATELY SUBMITTED TO THE ARCHITECT FOR APPROVAL. UPON APPROVAL, THE CONTRACTOR SHALL INCORPORATE THESE MODIFICATIONS INTO THE SHOP DRAWINGS PRIOR TO SUBMITTING THEM TO THE ENGINEER.
3. THE CONTRACTOR MUST BUILD APPROPRIATE CONTINGENCIES INTO THE PROJECT'S BUDGET AND SCHEDULE TO ACCOUNT FOR MODIFICATIONS TO STRUCTURAL FRAMING THAT MAY BE NECESSARY TO ACCOMMODATE EXISTING CONDITIONS THAT ARE DISCOVERED TO BE DIFFERENT FROM THOSE SHOWN ON THESE PLANS.

SYMBOL	DESCRIPTION
	GRIDLINE IDENTIFIER
	EXISTING GRIDLINE IDENTIFIER
	DETAIL MARKER
	ELEVATION MARKER
	FINISH FLOOR ELEVATION MARKER
	SPOT ELEVATION MARKER
	WORK POINT MARKER
	ELEVATION CHANGE IN SLAB OR DECK
	DECK OR SLAB SPAN DIRECTION
	MASONRY WALL
	WINDOW IN MASONRY WALL
	DOOR IN MASONRY WALL
	CONCRETE COLUMN
	STEEL COLUMN
	STANDARD PAN WIDTH SKIP PAN WIDTH
	STEEL BEAM
	STEEL BEAM MOMENT CONNX
	STEEL BEAM SPLICE
	VERTICAL STEEL BRACE - REF. ELEVATION

1.	STRUCTURAL DESIGN STANDARDS:	
	2021 INTERNATIONAL BUILDING CODE	
	LOCAL AMENDMENTS TO BUILDING CODE	
	ASCE 7-16	
2.	STRUCTURAL DESIGN LOADS:	
	RISK CATEGORY	II
	LIVE LOADS:	
	PUBLIC AREAS	100 psf
	ROOF	20 psf
	BALCONIES (PRIVATE)	60 psf
	STAIRS & CORRIDORS	100 psf
	DWELLING UNIT	40 psf
	DEAD LOADS:	
	GYPCRETE FILL	9 psf/in
	LIGHT-WEIGHT CONCRETE FILL	10 psf/in
	FULLWOOD FLOOR DECKING	3 psf/in
	CEILING/MECHANICAL	5 psf/in
	TRUSSES (AVERAGE)	3 psf
	PARTITIONS	5 psf
	MISCELLANEOUS	2 psf
	(NOTE: ALL DEAD LOADS APPROXIMATE. FINAL LOADS MUST BE COORDINATED W/ EOR.)	
	WIND LOAD:	
	WIND SPEED, $V_{@}$	126 MPH
	IMPORTANCE FACTOR, I_w	1.0
	EXPOSURE CATEGORY	C
	MWFRS LATERAL LOAD	41 psf
	NET C&C ROOF UPLIFT, EDGE	66 psf (EDGE STRIP "x" = 10 ft.)
	NET C&C ROOF UPLIFT, FIELD	42 psf
	C&C WALL LOAD, EDGE	33 psf (EDGE STRIP "x" = 10 ft.)
	C&C WALL LOAD, FIELD	26 psf
	(ALL C&C WIND LOADS SHOWN ARE SERVICE LOADS)	
	SEISMIC LOAD:	
	SITE CLASS	D
	SEISMIC DESIGN CATEGORY	B
	IMPORTANCE FACTOR, I_e	1.0
	S_1	0.088
	S_{vs}	0.056
	S_{vs}	0.094
	S_{vs}	0.09
	BASE SHEAR	kips
	SEISMIC FORCE-RESISTING SYSTEM	LIQUID FRAME (WOOD) WALL SHEATHED WITH WOOD
		STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE (R=6.5)
	SEISMIC COEFFICIENT, C_s	0.027

A.B.	ANCHOR BOLT	LBS	POUNDS
ADD.	ADDITIONAL	LDG	LANDING
ADJ.	ADJACENT	LLH	LONG LEGS HORIZONTAL
ADH.	ADHESIVE	LLV	LONG LEG VERTICAL
AFP.	ABOVE FINISH FLOOR	LT	LIGHT
ALT.	ALTERNATE	LVL	LEVEL
ARCH.	ARCHTIGHT	LWT	LIGHT WEIGHT
B.L.	BRCK LEDGE	MAT.	MATERIAL
B.LDG	BUILDING	MAX.	MAXIMUM
BM	BEAM	MECH.	MECHANICAL
B.O.D.	BOTTOM OF DECK	MEZZ.	MEZZANINE
BOTT.	BOTTOM	MFR.	MANUFACTURER
BRG.	BEARING	MID.	MIDDLE
BTWN	BETWEEN	MIN.	MINIMUM
CANT.	CANTILEVER	MISC.	MISCELLANEOUS
CP	CHG IN PLACE	MTL	MATCH LINE
C.J.	CONSTRUCTION JOINT		
C.L.	CENTERLINE	(N)	NEW
CLM	CLEAR	NIC	NOT IN CONTRACT
CMU	CONCRETE MASONRY UNIT	NO	NUMBER
C.O.	C.O.D.	NS	NEAR SIDE
COL.(S)	COLUMN(S)	NSG	NON-SHRINK GROUT
COMP.	COMPOSITE	NTS	NOT TO SCALE
CONC.	CONCRETE	NW	NORMAL WEIGHT
CONN.	CONNECTION		
CONST.	CONSTRUCTION	O.C.	ON CENTER
CONT.	CONTINUOUS	O.D.	OUTSIDE DIAMETER
CONTR.	CONTRACTOR	O.F.	OUTSIDE FACE
CTR	CENTER	O.P.H.	OPPOSITE HAND
ALT.	ALTERNATE	OPNG	OPENING

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ISSUE FOR PRICING 12/15/23

PROJECT DESCRIPTION

RESIDENCES @ HIGHLAND

BATON ROUGE, LA

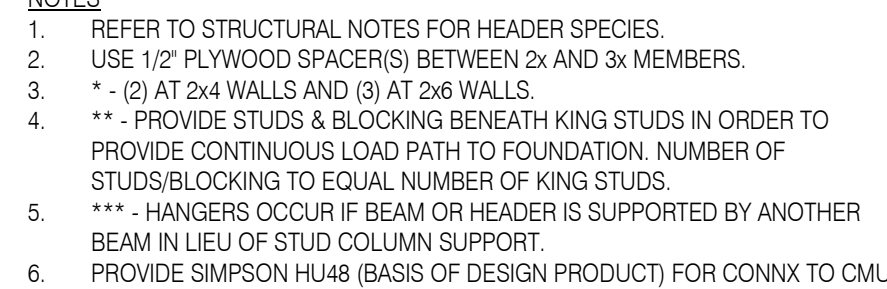
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DRAWING DESCRIPTION

19.000.00	\$1.10
Project Number	Sheet
Scale	1/8" = 1'-

	CONNECTION, LOCATION	FASTENING ^a
1.	JOIST TO SILL OR GIRDER, TOENAIL	3-8d
2.	BRIDGING TO JOIST, TOENAIL EACH END	3-8d
3.	1" x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
4.	WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d
5.	2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d
6.	SOLE PLATE TO JOIST OR BRIDGING, TYPICAL FACE NAIL	16d @ 16" O.C.
	SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL, BRACED WALL PANELS	3-16d PER 16"
7.	TOP PLATE TO STUD, END NAIL	2-16d
8.	STUD TO SOLE PLATE	4-8d TOENAIL OR 2-16d END NAIL
9.	DOUBLE STUDS, FACE NAIL	16d @ 24" O.C.
10.	DOUBLE TOP PLATES, TYPICAL FACE NAIL	16d @ 16" O.C.
	DOUBLE TOP PLATES, LAP SPICE	3-8d
11.	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL	1-16d
12.	RIM JOIST TO TOP PLATE, TOENAIL	8d @ 6" O.C.
13.	TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d
14.	CONTINUOUS HEADER, TWO PIECES, ALONG EDGE	16" O.C.
15.	CEILING JOISTS TO PLATE, TOENAIL	3-8d
16.	CONTINUOUS HEADER TO STUD, TOENAIL	4-8d
17.	CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d
18.	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d
19.	RAFTER TO PLATE, TOENAIL	3-8d
20.	1" x 2" DIAGONAL BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
21.	1" x 6" SHEATHINGS TO EACH BEARING WALL, FACE NAIL	2-8d
22.	WIDER THAN 1" x 6" SHEATHING TO EACH BEARING, FACE NAIL	3-8d
23.	BUILT-UP CORNER STUDS	2d" O.C.
24.	BUILT-UP GIRDER AND BEAMS, FACE NAIL	2d @ 32" O.C. TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES AT EACH SPICE
25.	2" PLANKS, AT EACH BEARING	16d
26.	COLLAR TIE TO RAFTER, FACE NAIL	3-10d
27.	JACK RAFTER TO HIP	3-10d TOENAIL OR 2-16d FACE NAIL
28.	ROOF RAFTER TO 2x RIDGE BEAM	2-16d TOENAIL OR 2-16d FACE NAIL
29.	JOIST TO BAND JOIST, FACE NAIL	3-16d
30.	LEDGER STRIP, FACE NAIL	3-16d
31.	WOOD STRUCTURAL PANELS AND PARTICLE BOARD, ^b SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING); 1/2" AND LESS 19/32 TO 3/4" 7/8 TO 1" 1-1/8 TO 1-1/4"	6d; 8d ^c OR 8d ^d 10d ^e OR 8d ^d
	SINGLE FLOOR (COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING); 3/4" AND LESS 7/8 TO 1" 1-1/8 TO 1-1/4"	6d 10d ^e OR 8d ^d
32.	PANEL SIDING (TO FRAMING) 1/2" 5/8"	6d ^f 8d ^f
33.	FIBERBOARD SHEATHING, ^g 1/2" 25/32"	NO. 11 GA. ROOFING NAIL ^h OR 8d ^f NO. 11 GA. ROOFING NAIL ^h OR 8d ^f
34.	INTERIOR PANELING 1/4" 3/4"	4d 6d



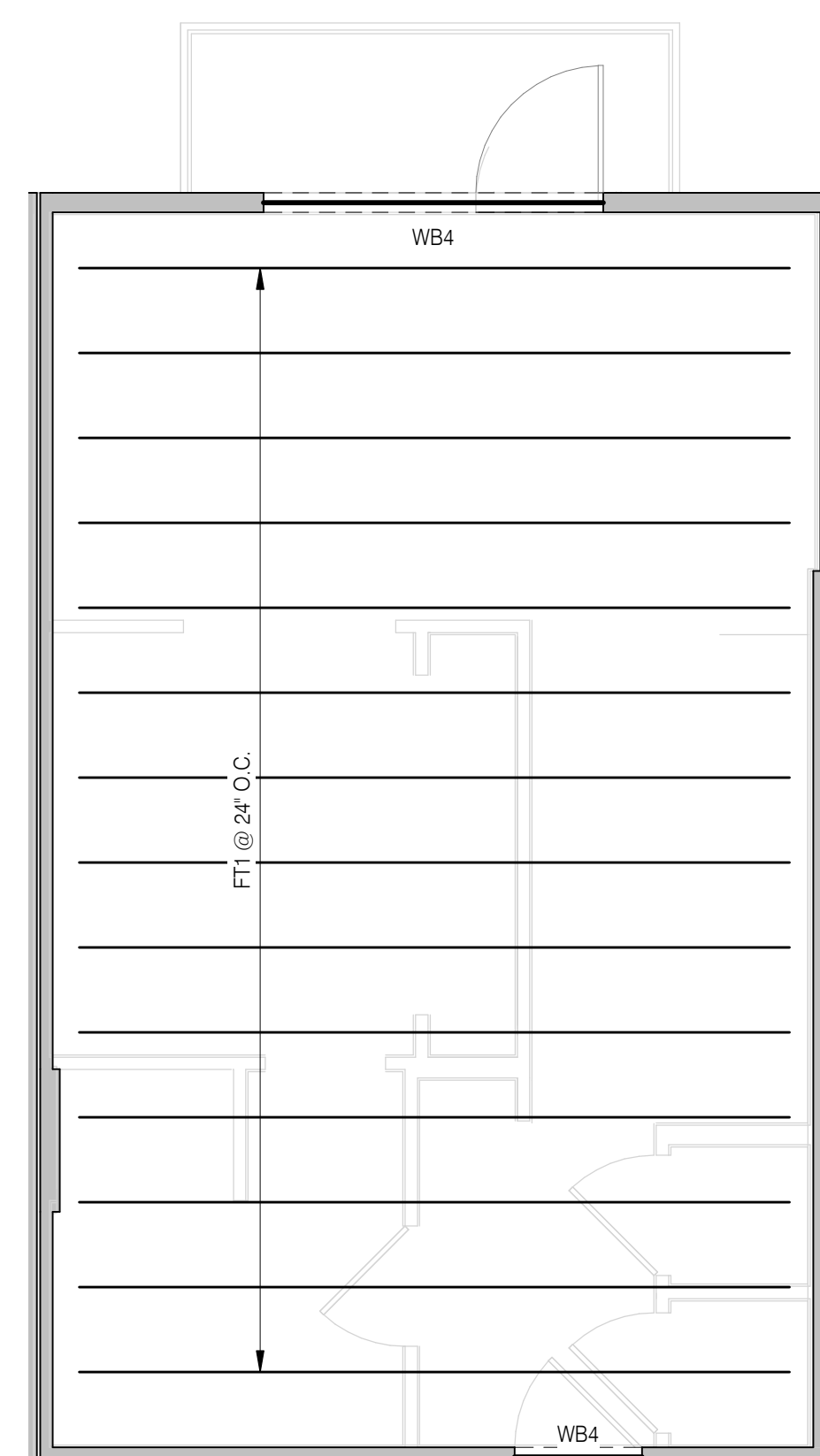
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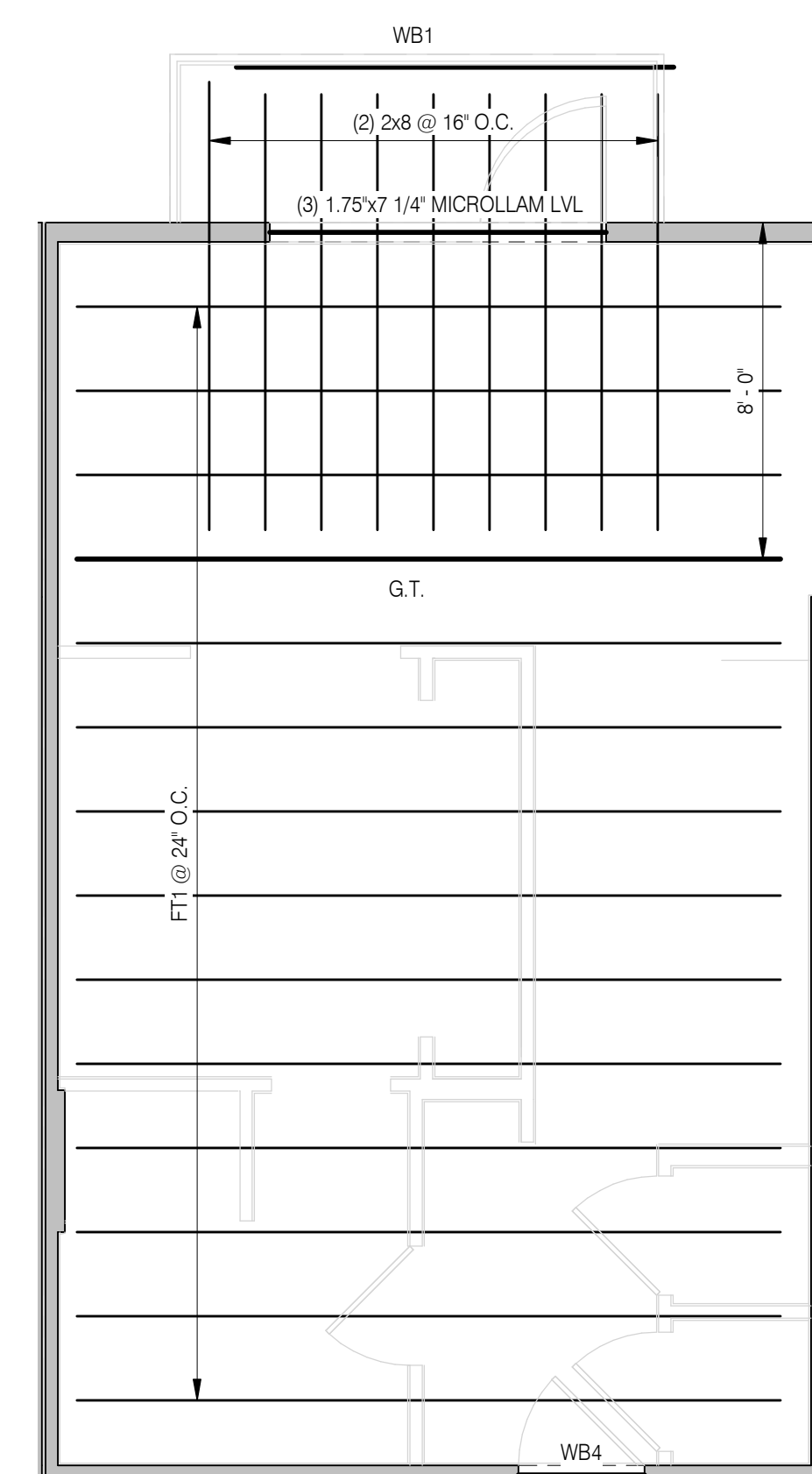
PROJECT DESCRIPTION

RESIDENCES @ HIGHLAND

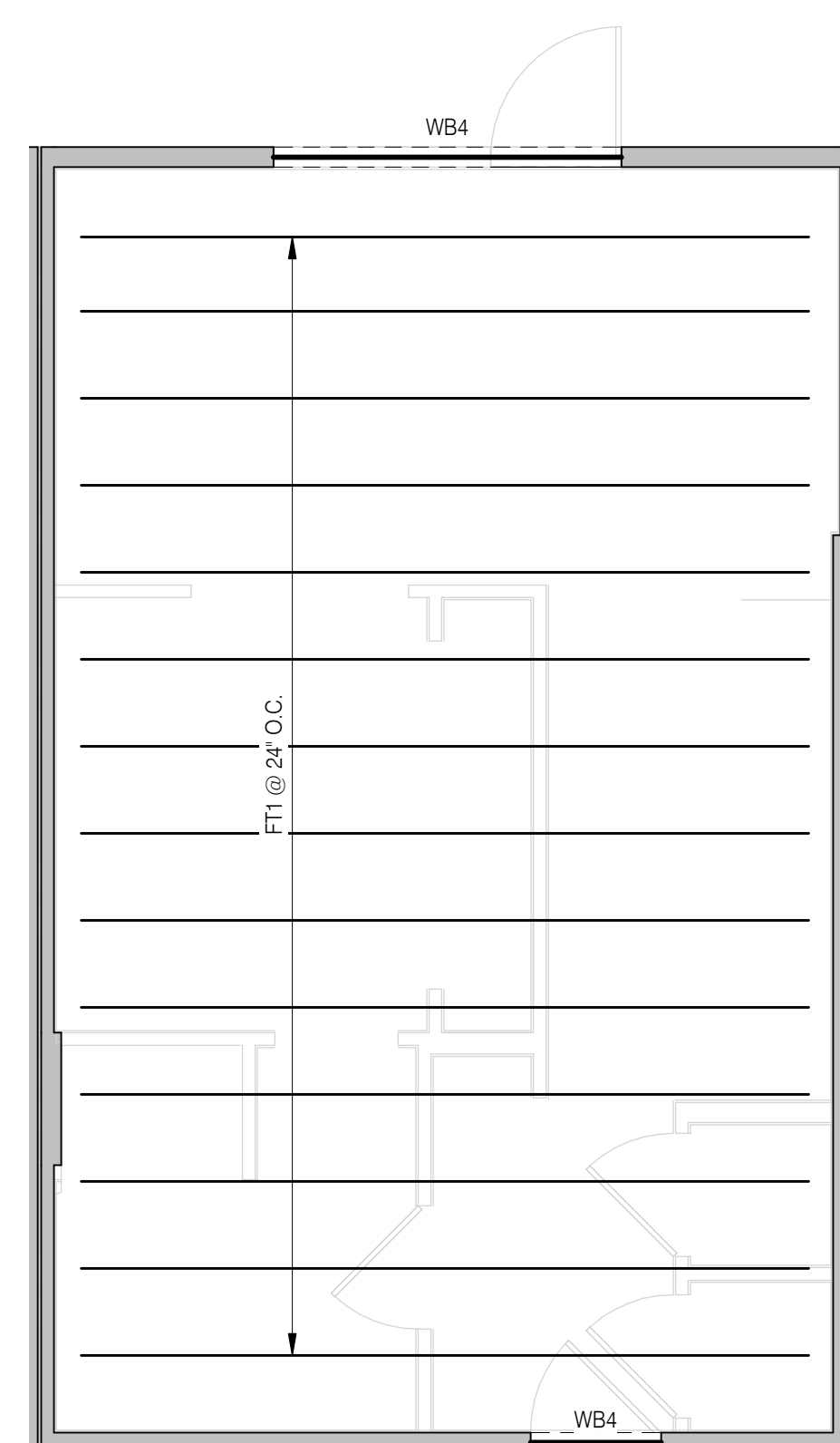
BATON ROUGE, LA



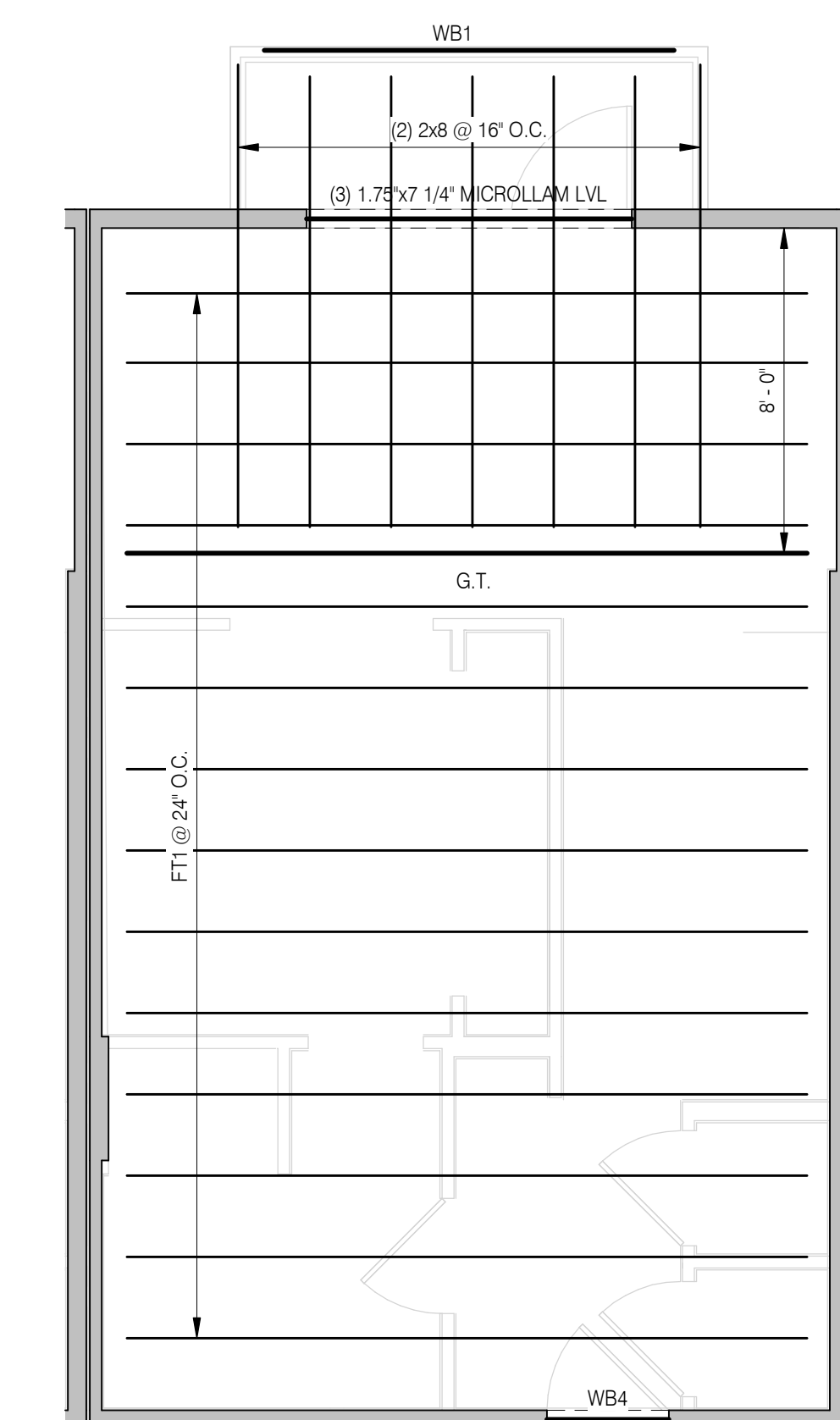
4	Unit A0-ALT-GF	$1/4'' = 1'-0''$
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2 Unit A0-GF	1/4" = 1'-0"
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3 Unit A0-ALT	$1/4" = 1'-0"$
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1 Unit A0	$1/4'' = 1'-0''$
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DRAWING DESCRIPTION

UNIT FRAMING
PLANS

19,000.00	\$1.20
Project Number	Sheet
Scale	1/4" = 1'-0"

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PROJECT DESCRIPTION

RESIDENCES @ HIGHLAND

BAÏON ROUGE, LA

DOCUMENT SUBMISSION RECORD

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DRAWING DESCRIPTION

UNIT FRAMING
PLANS

19.000.00
Project Number

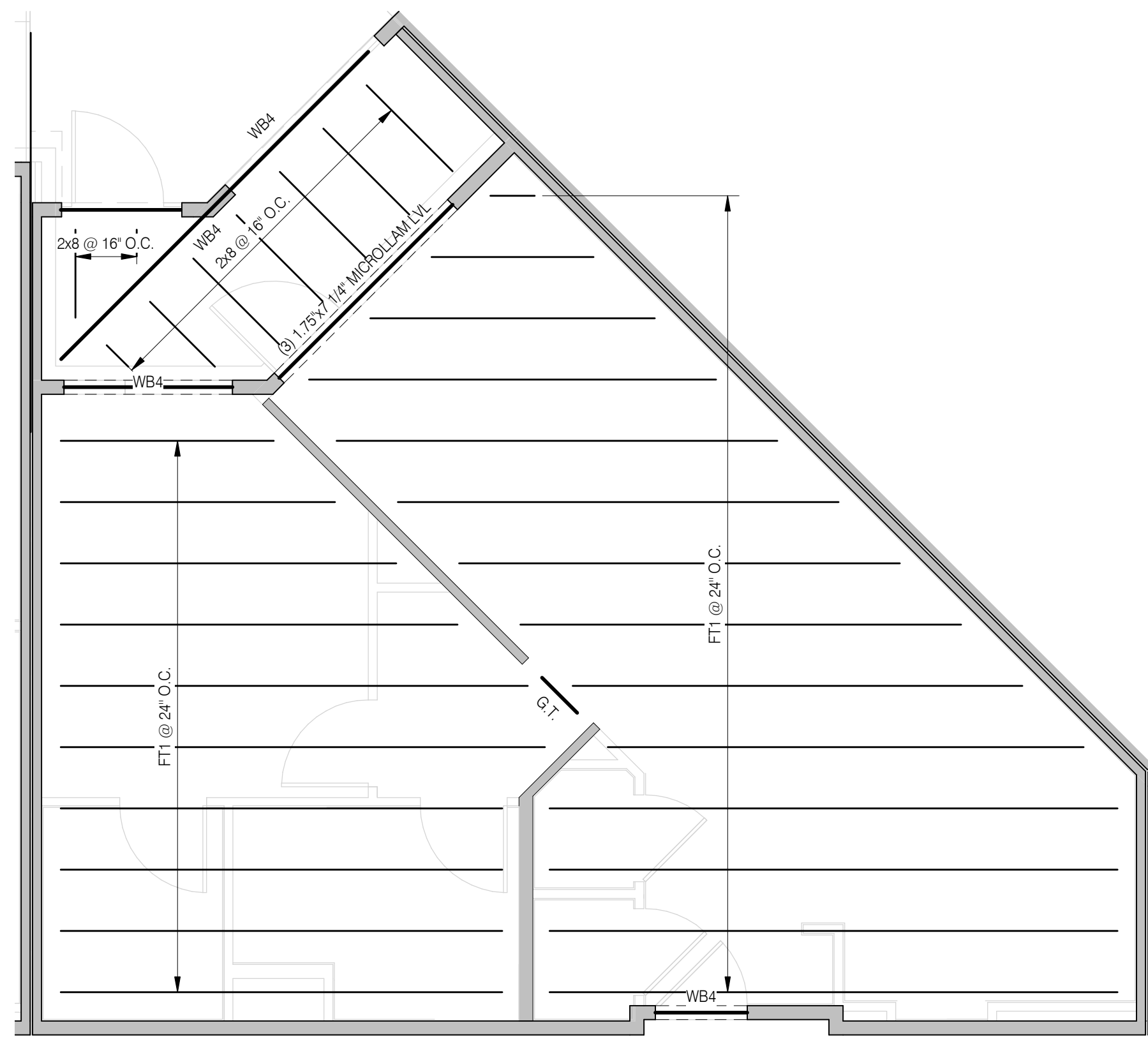
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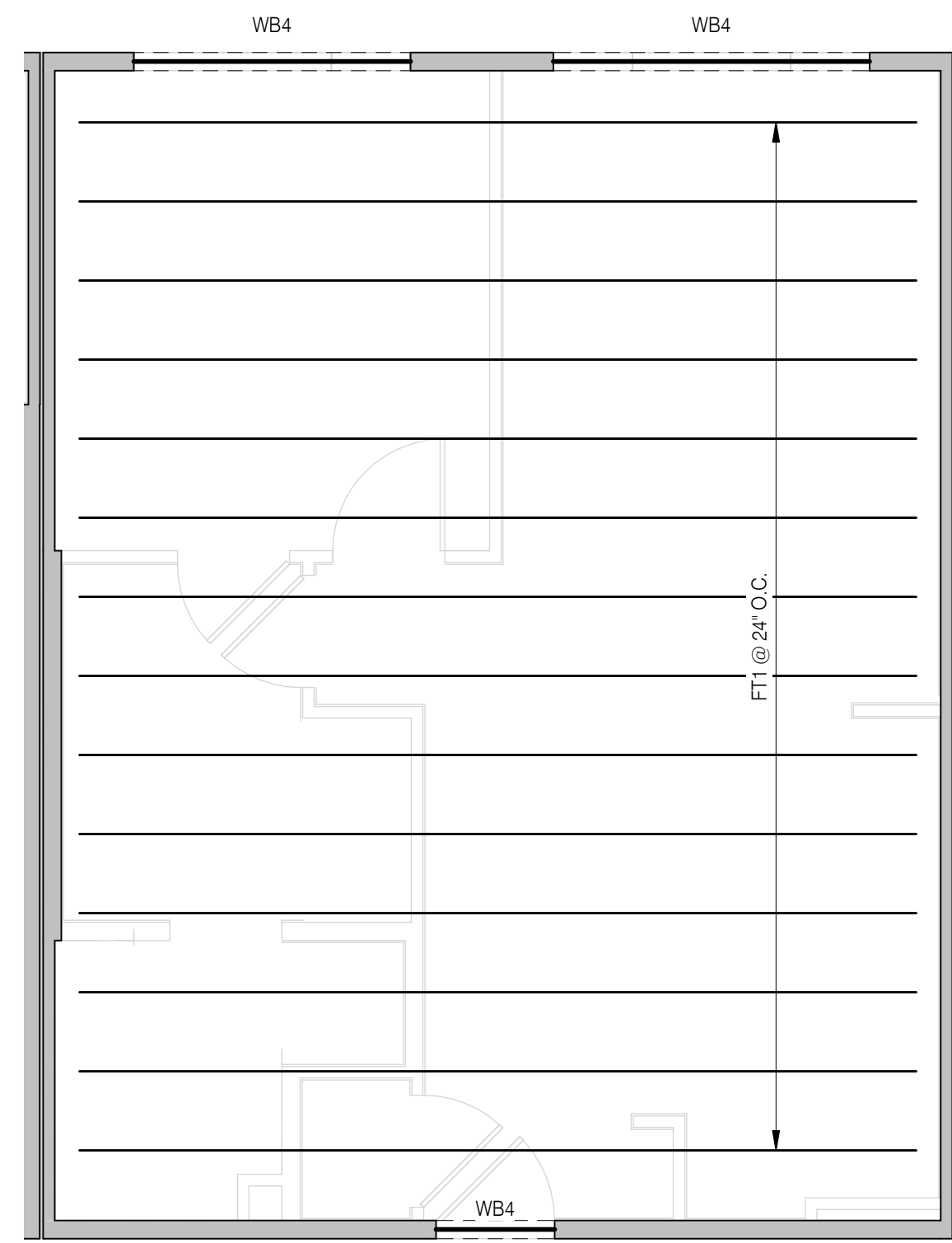
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$$1/4'' = 1'-0''$$

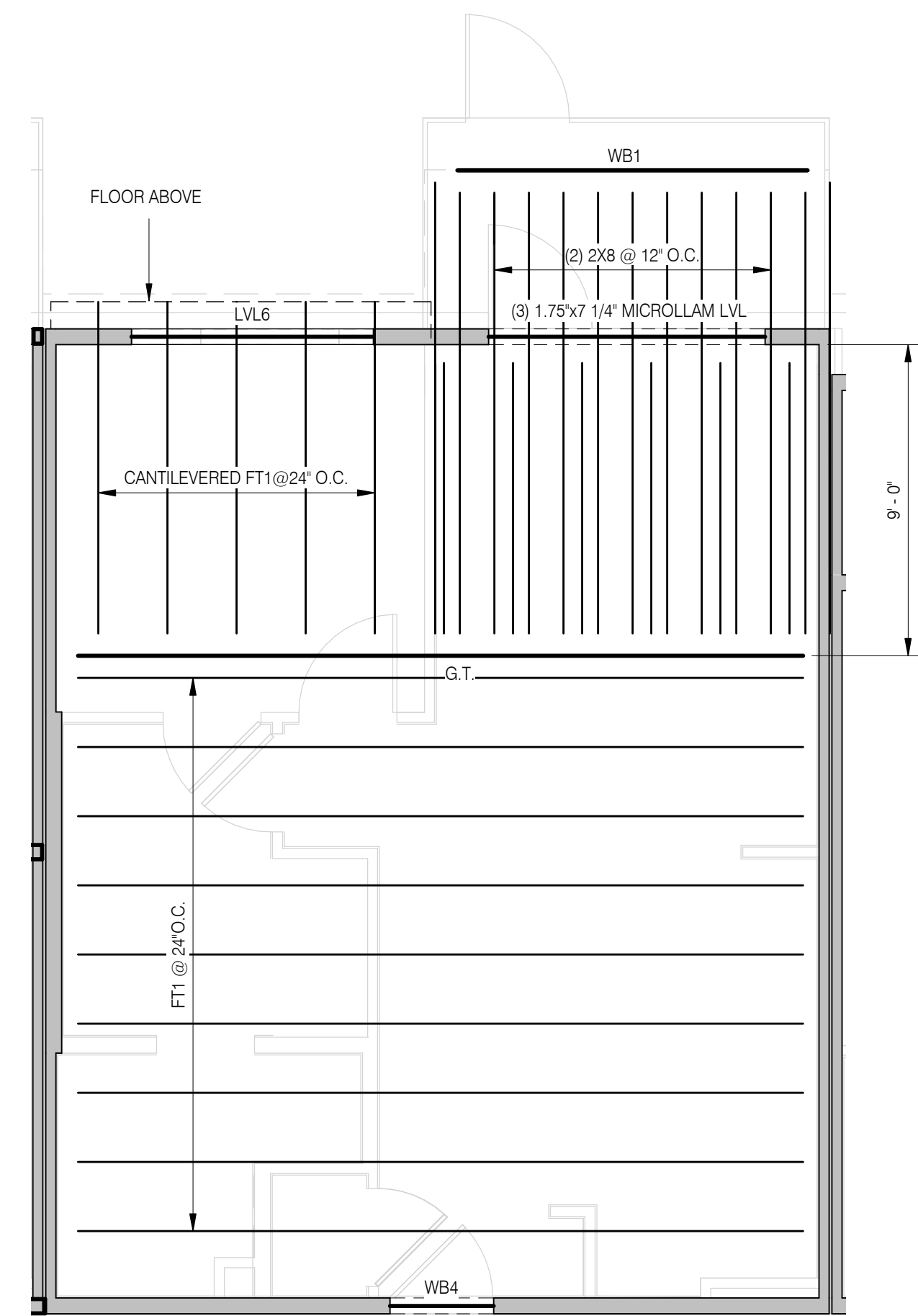
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6	Unit A3-GF	$1/4" = 1'-0"$
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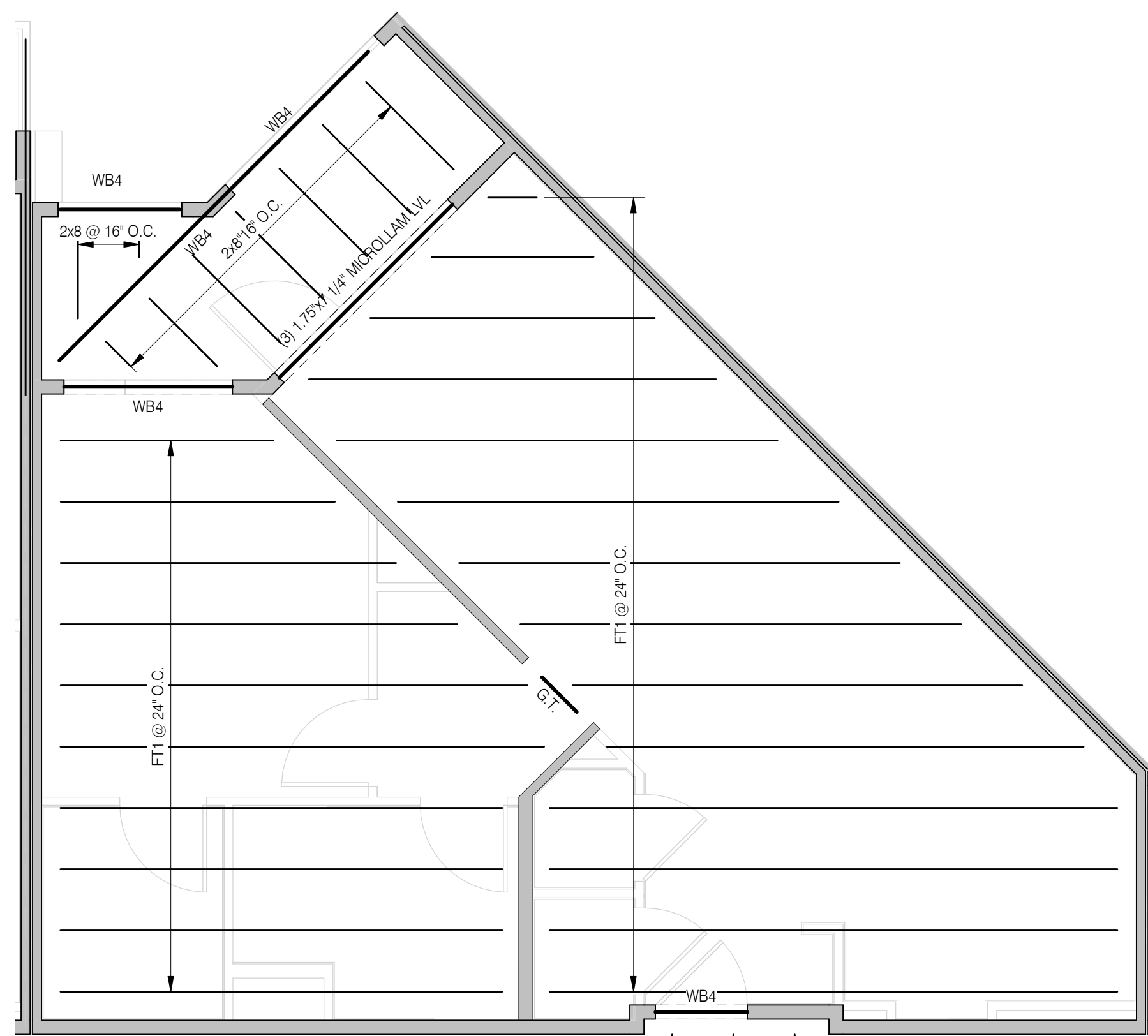


4 Unit A2	$1/4'' = 1'-0''$
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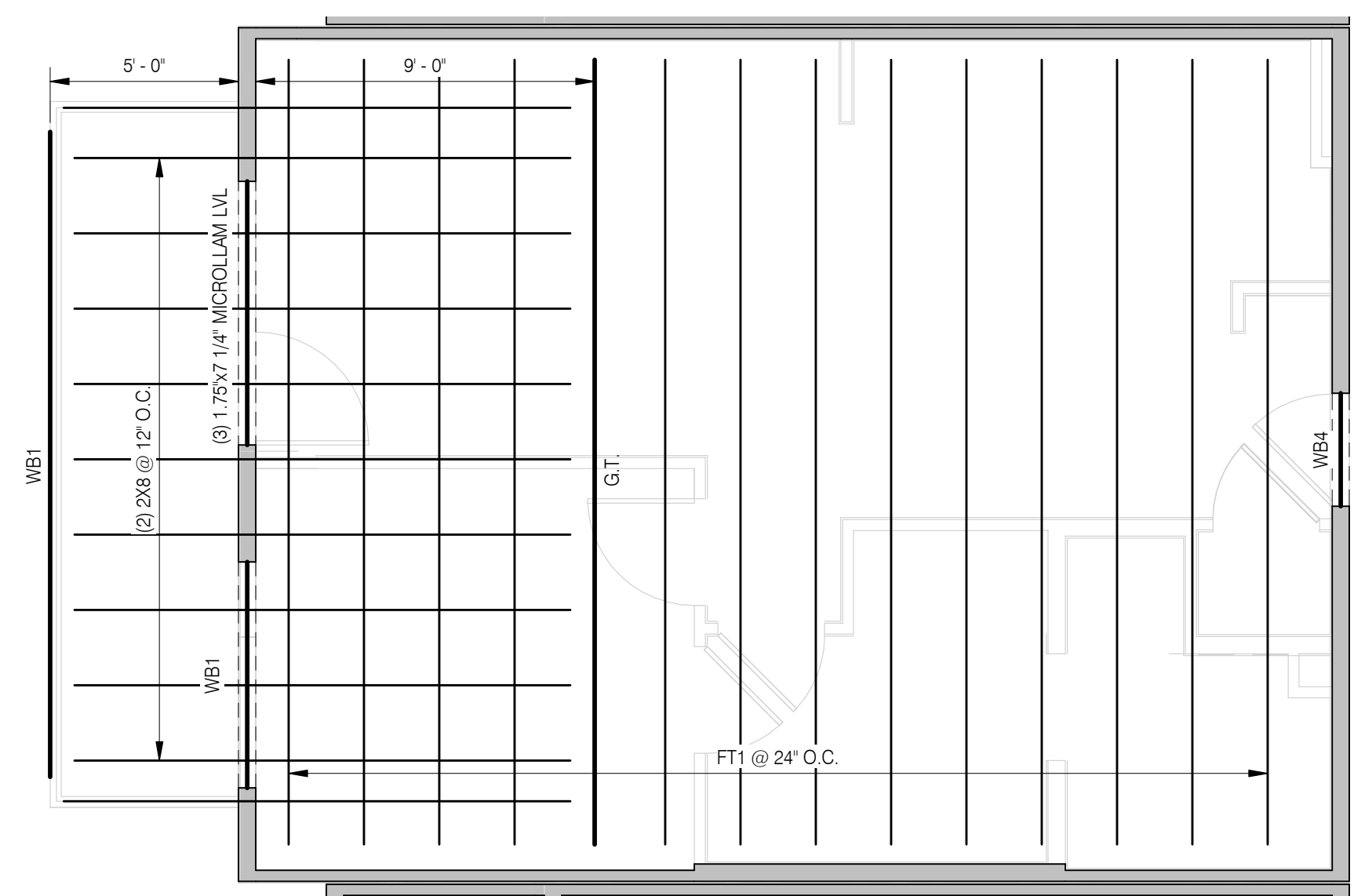


2 Unit A1-GF	$1/4'' = 1'-0''$
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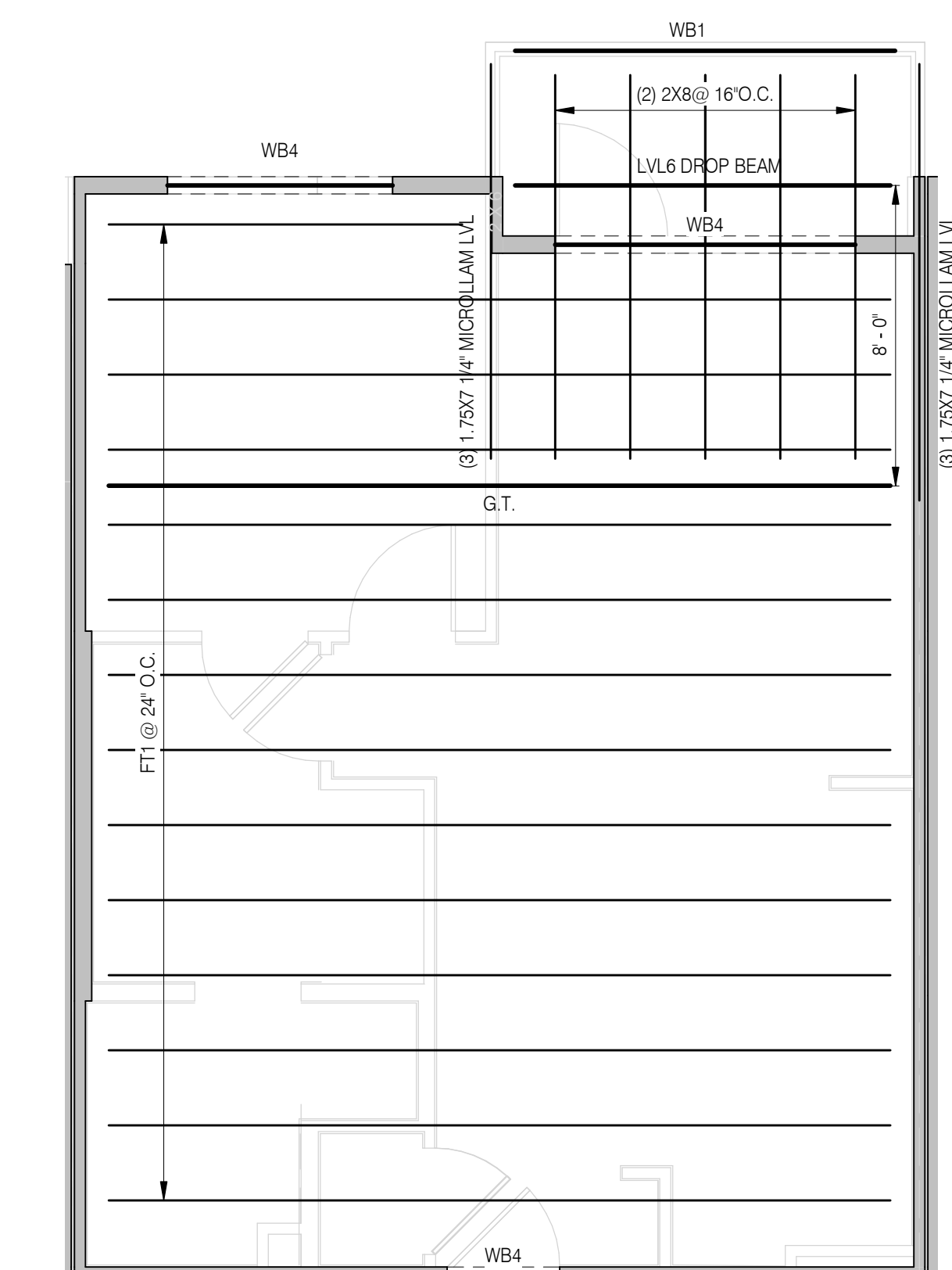
NOTE:
**DENOTES BEAM IS WITHIN FLOOR SYSTEM



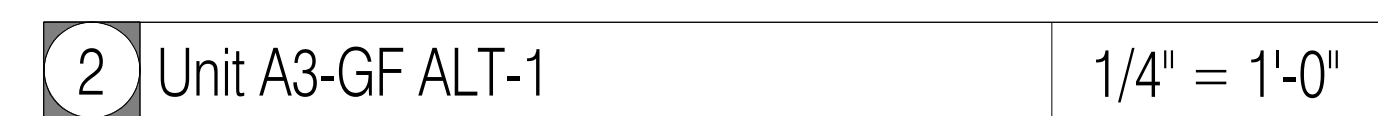
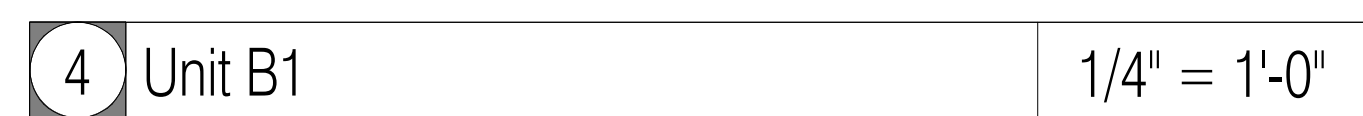
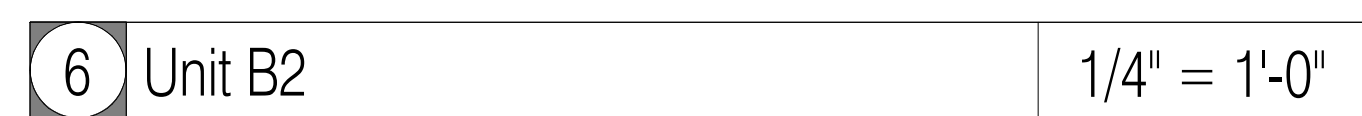
5	Unit A3	$1/4" = 1'-0"$
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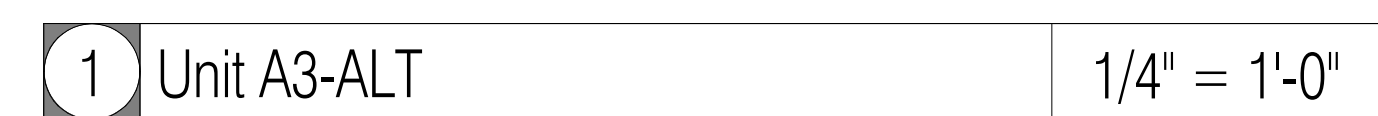
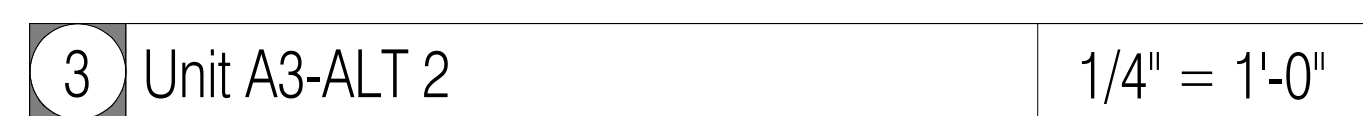
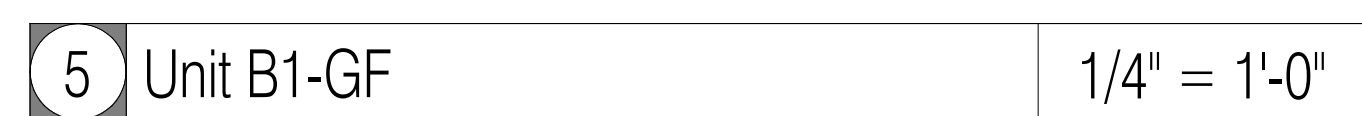
3	Unit A1-ALT	$1/4" = 1'-0"$
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1 Unit A1	$1/4'' = 1'-0''$
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NOTE:
**DENOTES BEAM IS WITHIN FLOOR SYSTEM





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PROJECT DESCRIPTION

RESIDENCES @ HIGHLAND

BATON ROUGE, LA

DOCUMENT SUBMISSION RECORD

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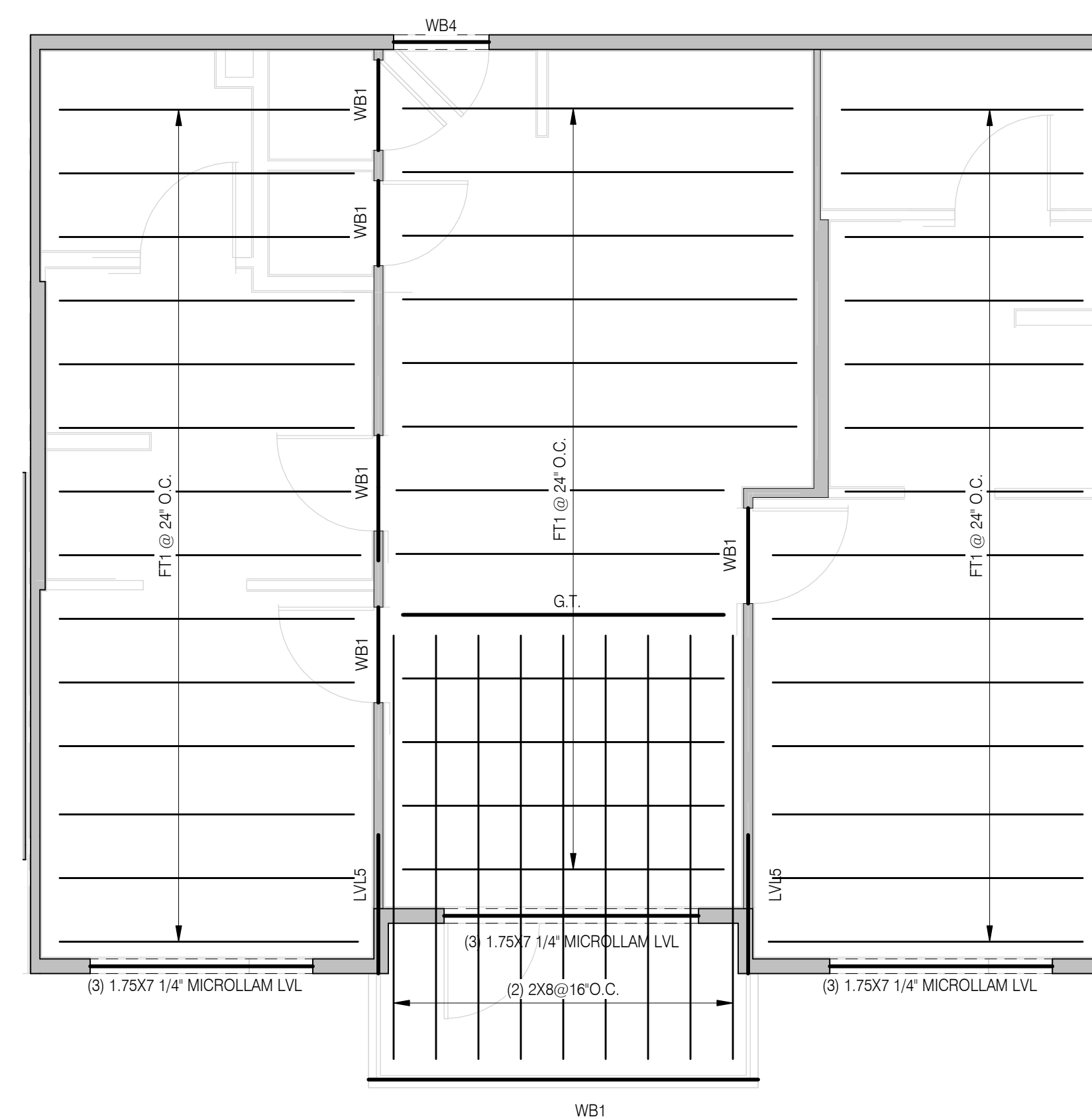
DRAWING DESCRIPTION

UNIT FRAMING
PLANS

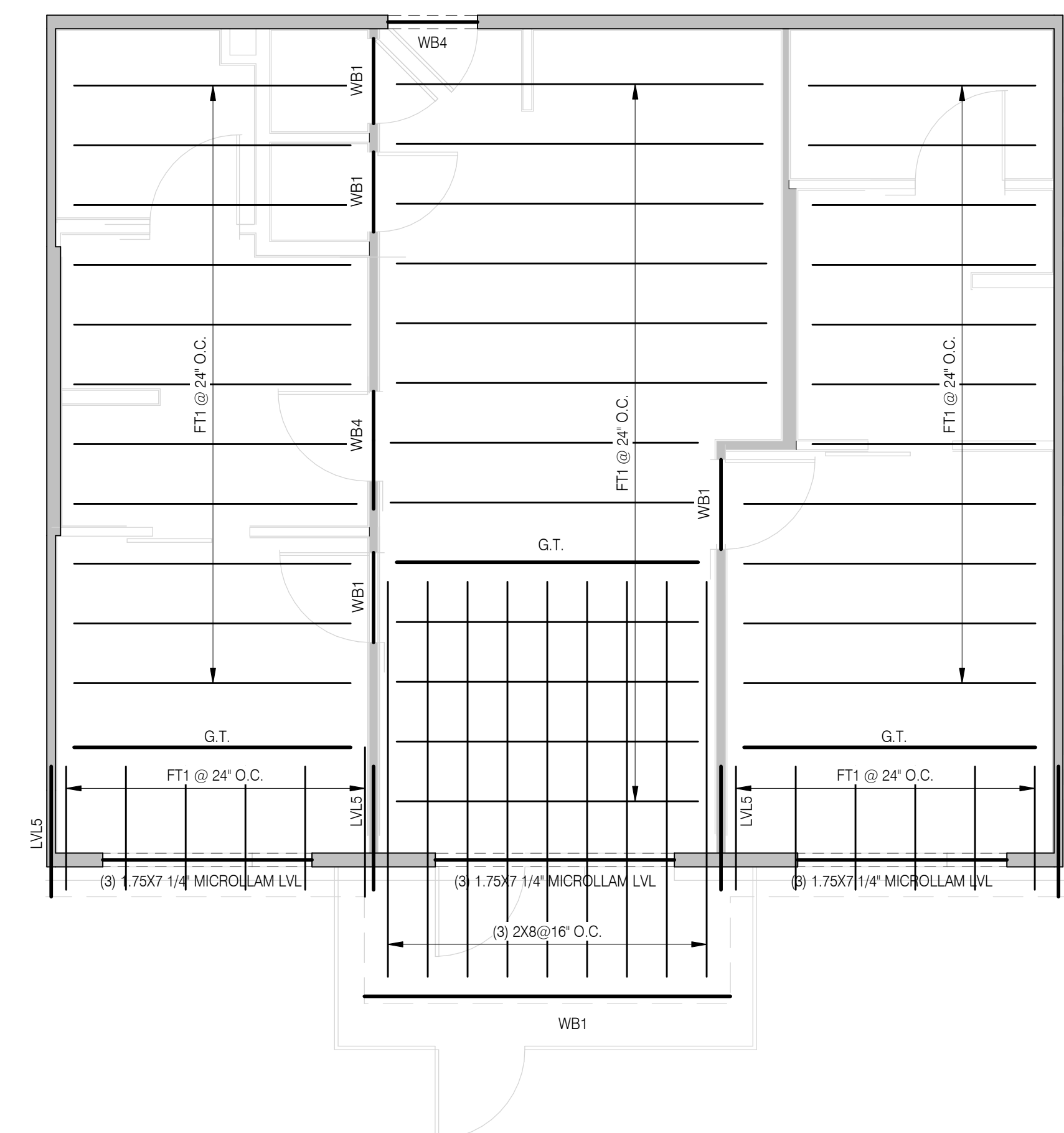
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Project Number

S1.25
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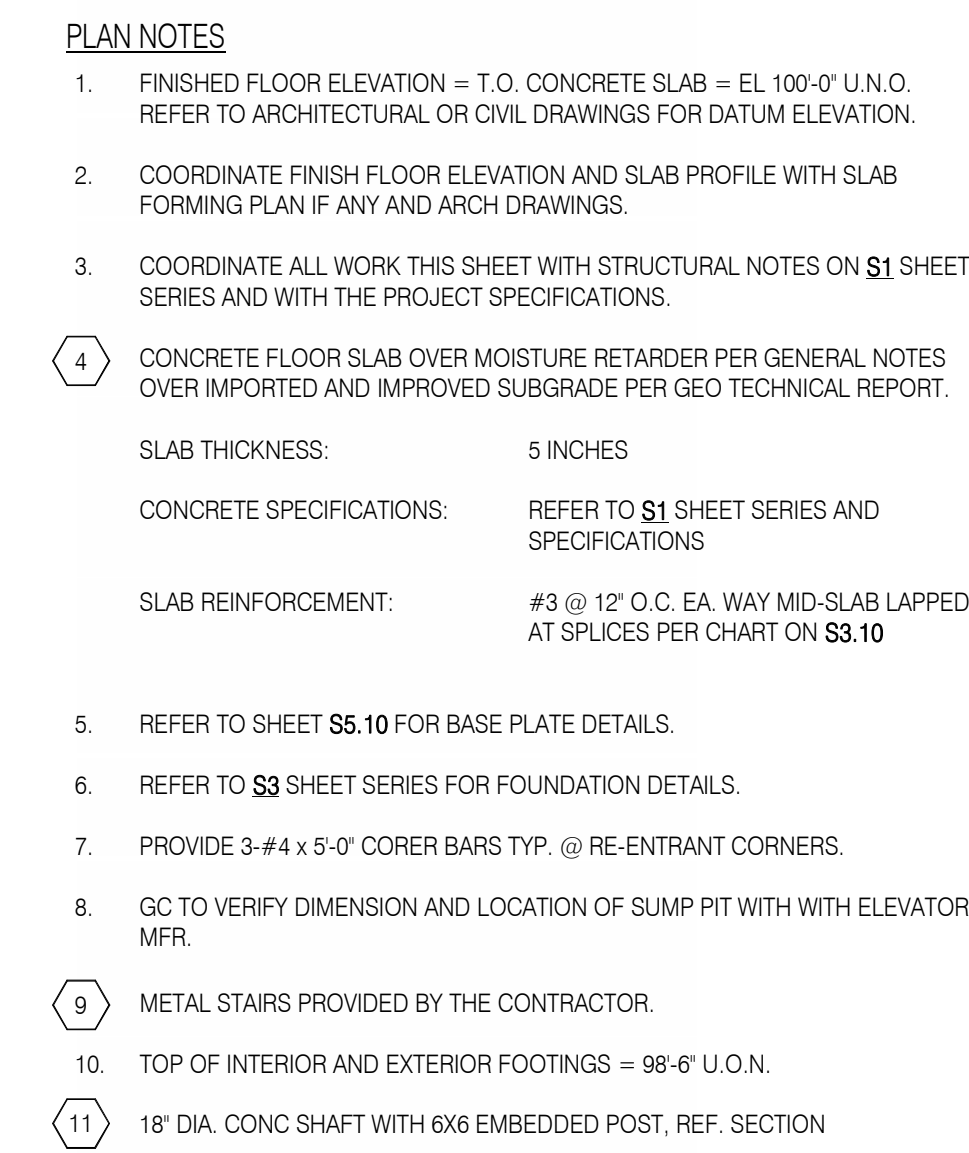
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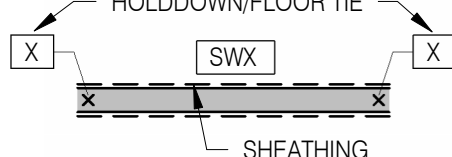
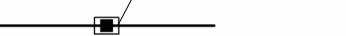


2	Unit B2-ADA	$1/4'' = 1'-0''$
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1	Unit B2-ADA_GF	1/4" = 1'-0"
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SYMBOL LEGEND	
	LOAD-BEARING WALL BELOW (REF. 6S1.12 FOR LOAD-BEARING WALL SCHEDULE)
	SHEAR WALL BELOW (REF. 4S1.12 FOR SHEAR WALL SCHEDULE)
	HOLDDOWN / FLOOR TIE APPROX. LOCATION PER SHEAR WALL SCHEDULE (REF. 2S1.12 FOR HOLDDOWN/FLOOR TIE SCHEDULE)
	
	BEAM (REF. 1S1.12 FOR BEAM SCHEDULE)
	POINT LOAD FROM ABOVE. SUPPORTED BY THE TRUSS. TRUSS MFR TO INCLUDE THIS LOADING IN TRUSS DESIGN.
	LINE LOAD FROM ABOVE. SUPPORTED THE TRUSS. PROVIDE TRUSS BLOCKING DIRECTLY BENEATH LOAD-BEARING WALL. TRUSS MANUFACTURER TO INCLUDE THIS LOADING IN TRUSS DESIGN.

STRIP FOOTING SCHEDULE					
MARK	WIDTH	THICKNESS	REINFORCING		NOT
			LATERAL	LONGITUDINAL	
WF1	18"	12"	#5 @ 12" O.C.	#4 @ 12" O.C.	
WF2	24"	12"	#5 @ 12" O.C.	#4 @ 12" O.C.	
WF3	30"	12"	#5 @ 12" O.C.	#4 @ 12" O.C.	

SPREAD FOOTING SCHEDULE					
MARK.	AxB	THICKNESS	REINFORCING		NOT
			TOP BARS	BOTT. BARS	
SF1	36"x36"	16"	4-#4 @ 12" O.C.	4-#4 @ 12" O.C.	
SF2	54"x54"	16"	4-#5 @ 12" O.C.	4-#5 @ 12" O.C.	
SF3	60"x60"	16"	5-#5 @ 12" O.C.	5-#5 @ 12" O.C.	
SF4	66"x66"	16"	5-#6 @ 12" O.C.	5-#6 @ 12" O.C.	
SF5	66"x66"	16"	6-#6 @ 12" O.C.	6-#6 @ 12" O.C.	

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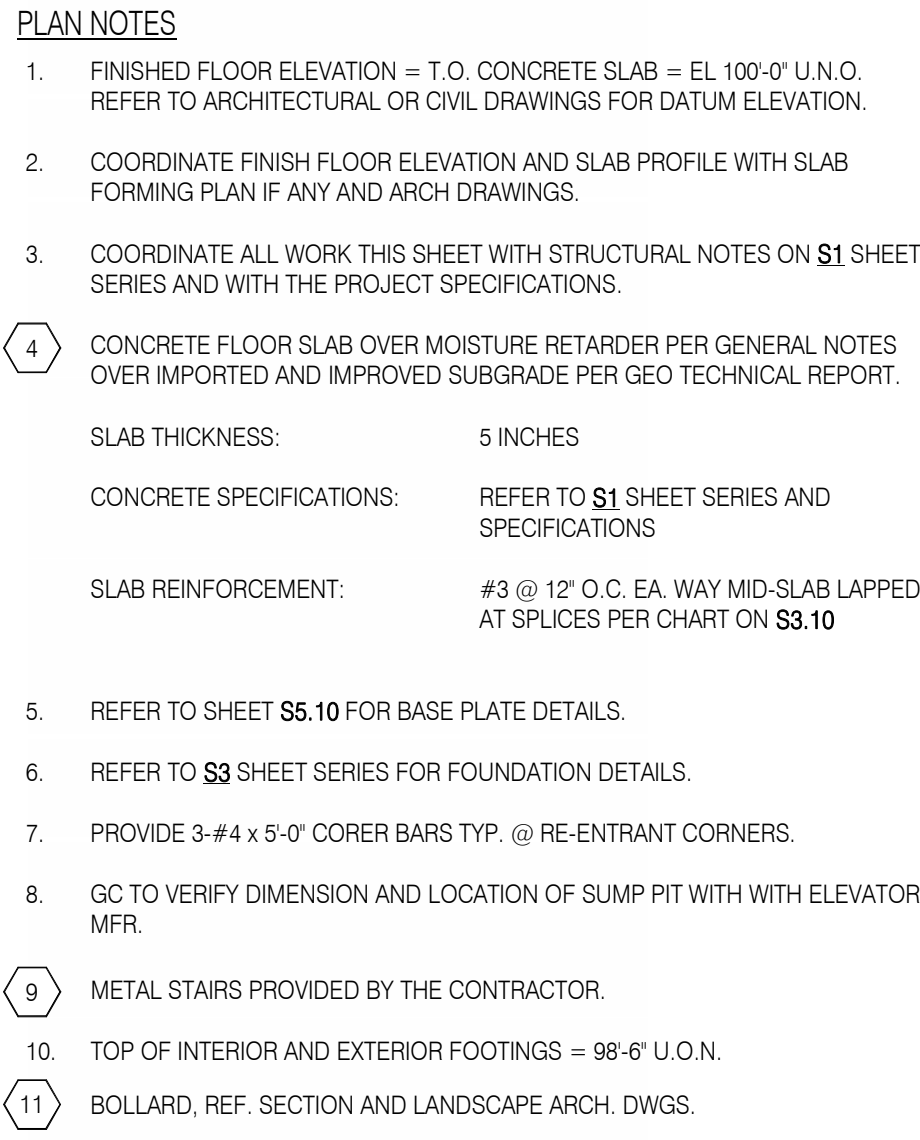
RESIDENCES @ HIGHLAND

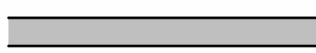
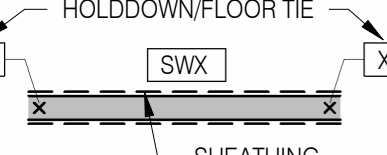
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DRAWING DESCRIPTION	
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FOUNDATION
PLAN - SECTOR
1

19.000.00	S2.1
Project Number	Sheet
Scale	$1/8" = 1'$



SYMBOL LEGEND	
	LOAD-BEARING WALL BELOW (REF. §5.1.12 FOR LOAD-BEARING WALL SCHEDULE)
	SHEARWALL BELOW (REF. 4.5.1.12 FOR SHEARWALL SCHEDULE)
	HOLDDOWN / FLOOR TIE APPROX. LOCATION PER SHEARWALL SCHEDULE (REF. 2.5.1.12 FOR HOLDDOWN/FLOOR TIE SCHEDULE)
	HOLDDOWN/FLOOR TIE
	BEAM (REF. 1.5.1.12 FOR BEAM SCHEDULE)
	POINT LOAD FROM ABOVE, SUPPORTED BY THE TRUSS. TRUSS MFR TO INCLUDE THIS LOADING IN TRUSS DESIGN.
	LINE LOAD FROM ABOVE, SUPPORTED THE TRUSS. PROVIDE TRUSS BLOCKING DIRECTLY BENEATH LOAD-BEARING WALL. TRUSS MANUFACTURER TO INCLUDE THIS LOADING IN TRUSS DESIGN.

STRIP FOOTING SCHEDULE					
MARK	WIDTH	THICKNESS	REINFORCING		NOT
			LATERAL	LONGITUDINAL	
WF1	18"	12"	#5 @ 12" O.C.	#4 @ 12" O.C.	
WF2	24"	12"	#5 @ 12" O.C.	#4 @ 12" O.C.	
WF3	30"	12"	#5 @ 12" O.C.	#4 @ 12" O.C.	

SPREAD FOOTING SCHEDULE					
MARK.	AuB	THICKNESS	REINFORCING		NOT
			TOP BARS	BOTT. BARS	
SF1	36"x36"	16"	4-#4 @ 12" O.C.	4-#4 @ 12" O.C.	
SF2	54"x54"	16"	4-#5 @ 12" O.C.	4-#5 @ 12" O.C.	
SF3	60"x60"	16"	5-#5 @ 12" O.C.	5-#5 @ 12" O.C.	
SF4	66"x66"	16"	5-#6 @ 12" O.C.	5-#6 @ 12" O.C.	
SF5	66"x66"	16"	6-#6 @ 12" O.C.	6-#6 @ 12" O.C.	

GRADE BEAM SCHEDULE					
MARK	WIDTH	DEPTH	REINFORCING		NOTES
			TOP BARS	BOTT. BARS	
GB1	18'	24'	(3) #7	(3) #7	

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PROJECT DESCRIPTION

RESIDENCES @ HIGHLAND

BATON ROUGE, LA

DOCUMENT SUBMISSION RECORD		
No.	Description	Date

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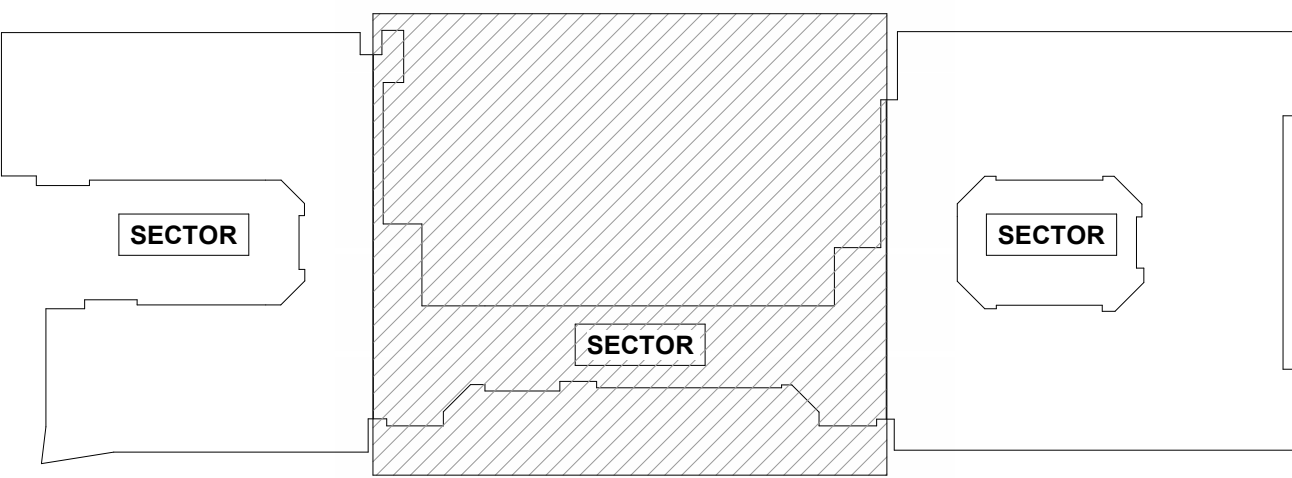
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FOUNDATION
PLAN - SECTOR
2

19.000.00
Project Number

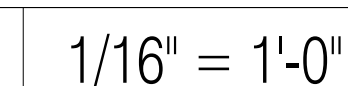
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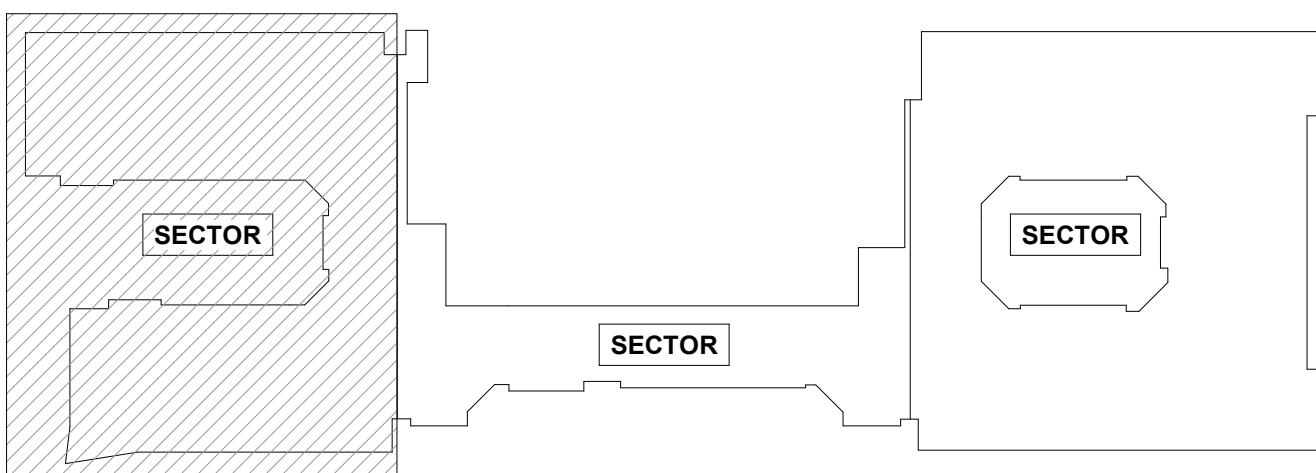
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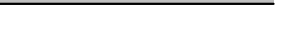
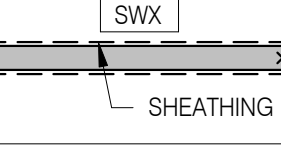
1	FOUNDATION PLAN - AREA B	1/8" = 1'-0"
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1. REFER TO SHEET §51 SERIES FOR STRUCTURAL INFORMATION, WOOD FRAMING NOTES AND WOOD SCHEDULES AND TYPICAL DETAILS.
2. THE BACKGROUND SHOWN ON THE PLAN IS FOR THE FLOOR BELOW.
3. FOR UNIT FRAMING AND/OR FOR UNIQUE CONDITIONS REFER TO OTHER FRAMING PLANS LISTED ON THE OVERALL FRAMING PLAN.
4. ALL EXTERIOR WALLS ARE CONSIDERED LOAD-BEARING WALLS REGARDLESS OF HATCHING.
5. REFER TO §51.12 FOR NON STRUCTURAL HEADER SCHEDULE.
6. DRAG TRUSSES AND SHEARWALL BLOCKING MUST BE PROVIDED BY TRUSS MANUFACTURER ABOVE SHEAR WALLS. REFER TO TRUSS BLOCK DETAIL 295.11 AND TYP. SHEARWALL FLOOR THE DETAIL 755.10.
7. PROVIDE 3/4" FLOOR DECK PER WOOD FRAMING NOTES ON SHEET §51.11 ON TOP OF PRE-ENGINEERED TRUSSES PROVIDED AS PER TRUSS SCHEDULE ON §51.12.
8. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONAL CONTROL.
9. COORDINATE LEVEL ELEVATIONS WITH ARCHITECTURAL DRAWINGS. REFER TO PLAN FOR TOP OF SLAB AND DECK ELEVATIONS.
10. TOP OF STEEL BEAM ELEVATIONS = 3" BELOW BOTTOM OF DECK, U.O.N.

	LOAD-BEARING WALL BELOW (REF. 8/S1.12 FOR LOAD-BEARING WALL SCHEDULE)
	SHEARWALL BELOW (REF. 4/S1.12 FOR SHEARWALL SCHEDULE)
×	HOLDDOWN / FLOOR TIE APPROX. LOCATION PER SHEARWALL SCHEDULE (REF. 2/S1.12 FOR HOLDDOWN/FLOOR TIE SCHEDULE)
	HOLDDOWN/FLOOR TIE
	BEAM (REF. 1/S1.12 FOR BEAM SCHEDULE)
	POINT LOAD FROM ABOVE, SUPPORTED BY THE TRUSS. TRUSS MFR TO INCLUDE THIS LOADING IN TRUSS DESIGN.
	LINE LOAD FROM ABOVE SUPPORTED BY THE TRUSS. PROVIDE TRUSS BEARING DIRECTLY BENEATH LOAD-BEARING WALL. TRUSS MANUFACTURER TO INCLUDE THIS LOADING IN TRUSS DESIGN.
	BEAM TO BEAM MOMENT CONNECTION. RE. TYP. DETAIL

Scott R. Armstrong, P.E.
Texas License No. 83205

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PERMIT, OR CONSTRUCTION.

ISSUE FOR PRICING 12/15/23

PROJECT DESCRIPTION

RESIDENCES @ HIGHLAND

BATON ROUGE, LA

DOCUMENT SUBMISSION RECORD

[illegible]

DRAWING DESCRIPTION

SECOND FLOOR FRAMING PLAN - SECTOR 1

19,000.00	\$2.21
Project Number	Sheet
Scale	1/8" = 1'-0"

1/4/2024 1:03:49 PM

1 SECOND FLOOR FRAMING PLAN - SECTOR 1

$$1/8'' = 1'-0''$$

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$$1/16'' = 1'-0''$$



19,000.00
Project Number

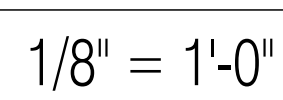
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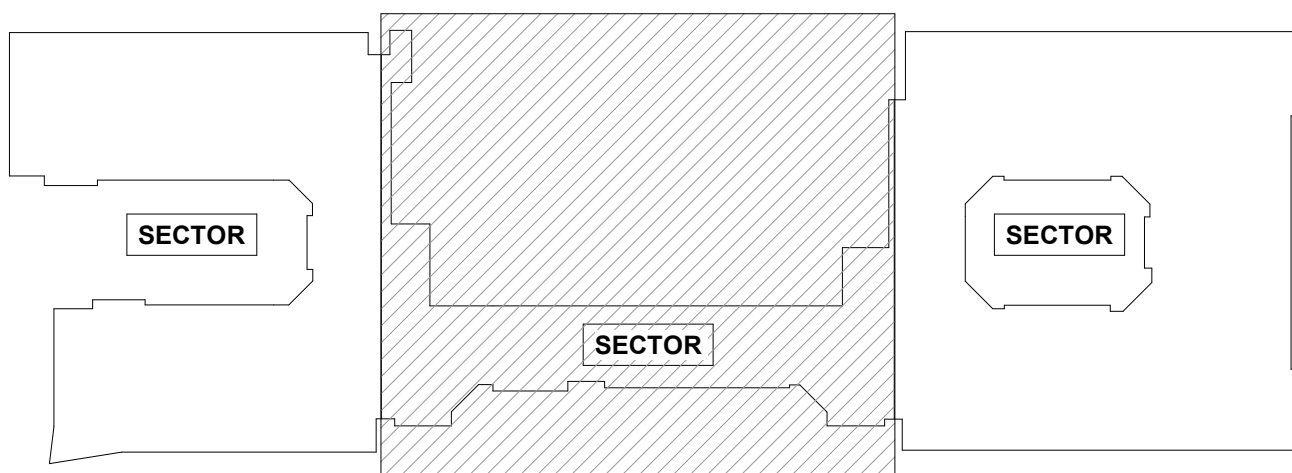
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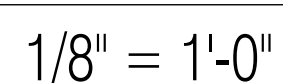

$$1/16'' = 1'-0'''$$



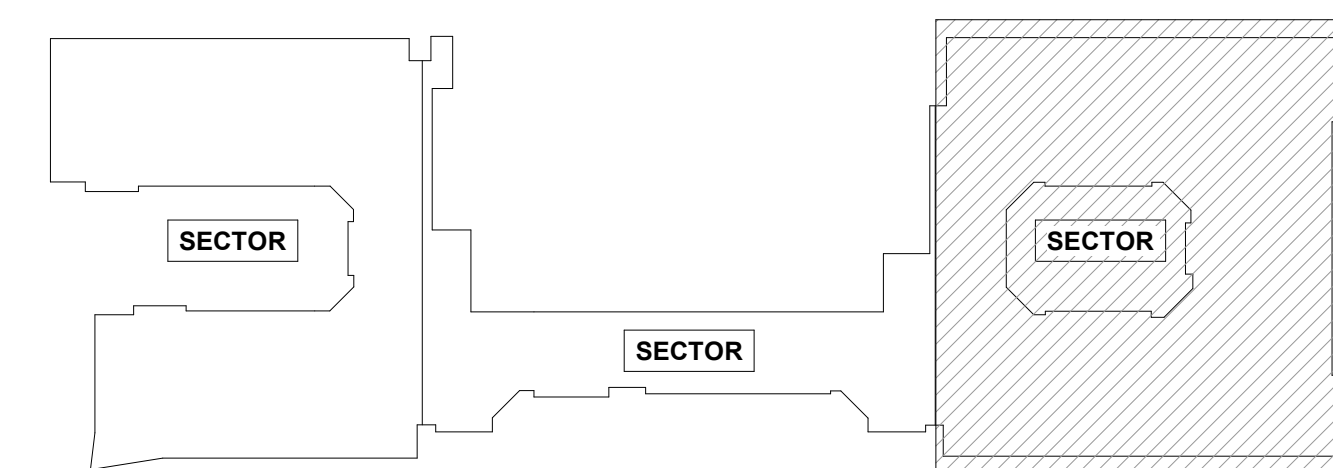


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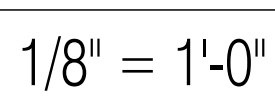
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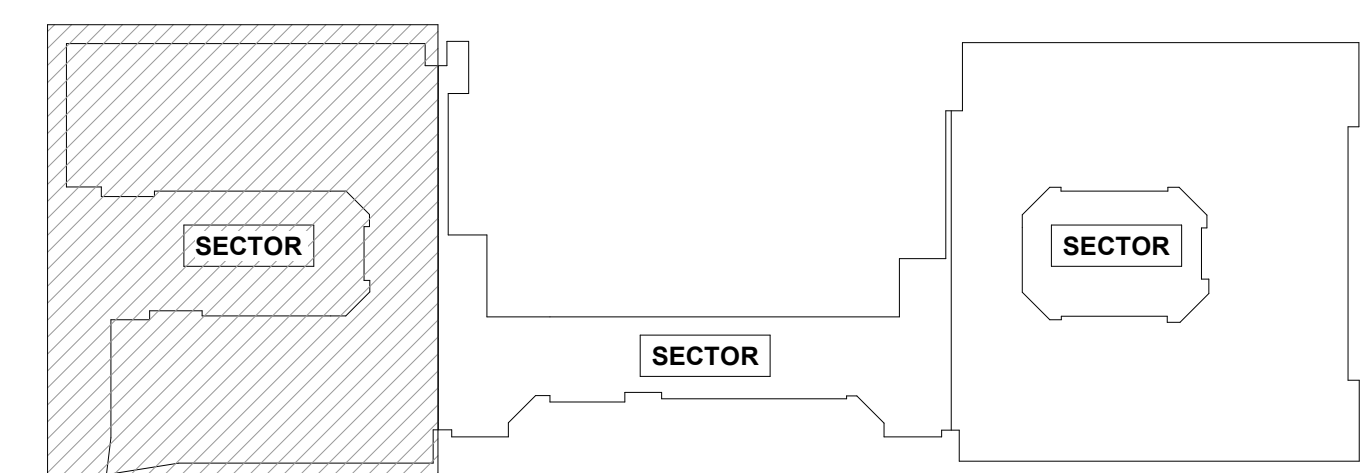
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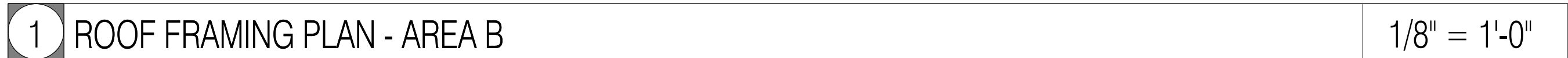




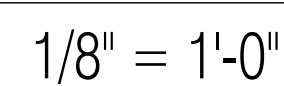


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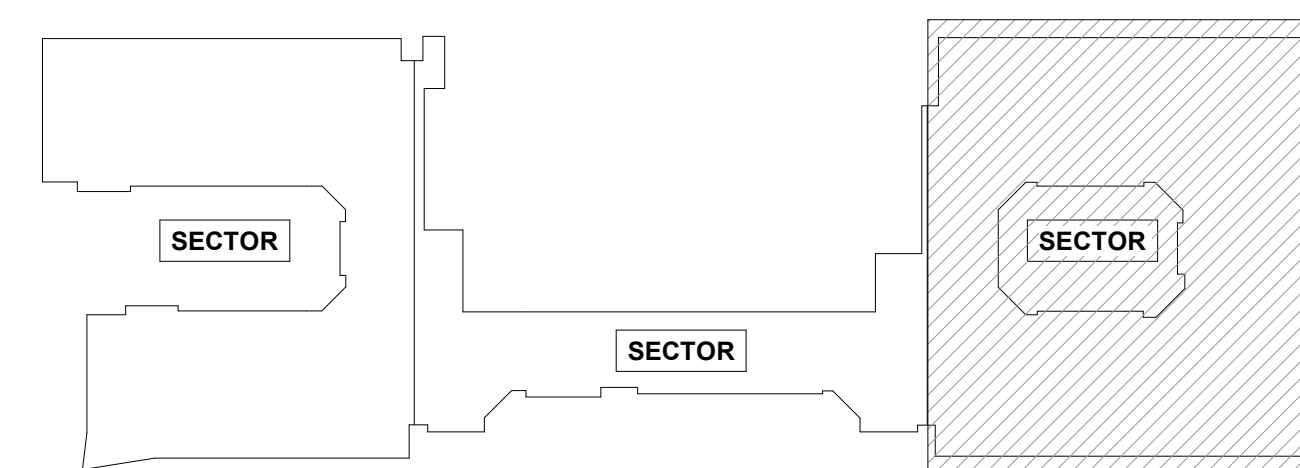




-
- A diagram showing a three-sector system. A central sector is shaded with diagonal lines and labeled "SECTOR". It is flanked by two unshaded sectors, each labeled "SECTOR".



- 1/4/2024 1:06:05 PM



G. TYPICAL SLAB DROP - BTWN 1'-0" AND 5'-0"

F. TYPICAL SLAB DROP - 1'-0" OR LESS

E. TYPICAL THICKENED SLAB

D. TYPICAL THICKENED SLAB EDGE

C. TYPICAL REINFORCEMENT AT SLAB PENETRATIONS

B. TYPICAL REINF. AT RE-ENTRANT CORNERS

A. TYPICAL REINFORCEMENT

2 TYP. SLAB-ON-GROUND DETAILS

11 PIPE THROUGH FDN STEMWALL

10 TYP. HORIZ. GR. BM PENETRATION

C. INTERSECTION REINFORCEMENT

B. CORNER REINFORCEMENT

A. TYP. FOOTING STEP DETAIL (LESS THAN 3'-0")

9 TYP. STRIP FOOTING REINF. DETAIL

C. GRADE BEAM INTERSECTION

B. GRADE BEAM CORNER

A. TYPICAL SECTION

TYP. GRADE BEAM REINF. DETAIL

NOTE: CONSTRUCTION JOINT SHALL BE LOCATED IN THE MIDDLE THIRD OF THE SPAN BETWEEN SUPPORTS. DO NOT LOCATE A CONSTRUCTION JOINT IN AN END SPAN

TYP. BEAM CONSTR. JOINT

DOWELS
@ 4 1/2" SLAB: #4 @ 2'-0"
@ 18" O.C. IN ADDITION TO TYP. SLAB REINFORCING

TYP. CONSTRUCTION JOINT DTL

B. TYPICAL CONTROL JOINT

A. TYPICAL CONSTRUCTION JOINT

SLAB ON GRADE JOINTS

REBAR DOWEL SCHEDULE

NOTES

1. ADHESIVE TO BE ONE OF THE FOLLOWING PRODUCTS:
 - HILTI HIT - HY 200 SAFE SET
 - SIMPSON SET - XP
2. ANCHOR RODS SHALL BE ONE OF FOLLOWING:
 - FOR HILTI SYSTEM: HILTI HAS RODS OR HIT-Z R
 - FOR SIMPSON SYSTEM: A36 THREADED RODS
3. FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR
4. EMBEDMENTS SHOWN ARE TYPICAL FOR EACH ANCHOR
5. SPECIAL INSPECTIONS ARE REQUIRED FOR ADHESIVE

DH. ANCHOR & DWL SCHEDULE

fc	SPICE CLASS	BAR SIZE									
		#3	#4	#5	#6	#7	#8	#9	#10	#11	
psi		fy	60ksi	60ksi	60ksi	60ksi	60ksi	60ksi	60ksi	60ksi	
0000	CLASS A 1.0 Ld	17	22	28	33	48	55	62	70	78	
	CLASS A 1.3 Ld	22	29	36	43	63	72	81	91	101	
0000	CLASS A 1.0 Ld	15	19	24	29	42	48	54	61	67	
	CLASS A 1.3 Ld	19	25	31	37	54	62	70	79	87	
0000	CLASS A 1.0 Ld	13	17	22	26	38	43	48	54	60	
	CLASS A 1.3 Ld	17	23	28	34	49	56	63	71	78	
0000	CLASS A 1.0 Ld	12	16	20	24	34	39	44	50	55	
	CLASS A 1.3 Ld	16	21	26	31	45	51	57	64	71	
0000	CLASS A 1.0 Ld	12	15	18	22	32	36	41	46	51	
	CLASS A 1.3 Ld	14	19	24	28	41	47	53	60	66	

TENSION DEVELOPMENT LENGTH = L_d LENGTHS LISTED IN TABLE ARE IN INCHES. USE THE ABOVE DEVELOPMENT LENGTH AND LAP SPACE TABLE FOR BEAMS, JOISTS, COLUMNS, WALLS, SLABS, ETC. WHEN THE CLEAR SPACE BETWEEN BARS IS GREATER THAN OR EQUAL TO TWO BAR DIAMETERS AND CLEAR COVER IS NOT LESS THAN ON BAR DIAMETER.

WHEN THE CLEAR SPACE BETWEEN BARS IS LESS THAN TWO BAR DIAMETERS, MULTIPLY DEVELOPMENT AND SPACE LENGTHS LISTED IN TABLE BY 1.50.

PROVIDE LAP SPACE LENGTH BASED ON THE LARGER BAR BEING LAPPED WHEN BARS OF DIFFERENT SIZES ARE LAP SPACED.

FOR TOP BARS, MULTIPLY THE DEVELOPMENT AND SPACE LENGTHS BY 1.3. TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE BAR.

EXTENSION LAP SPLICE SCHEDULE

FOOTING PLAN LEGEND

TYPICAL SPREAD FOOTING

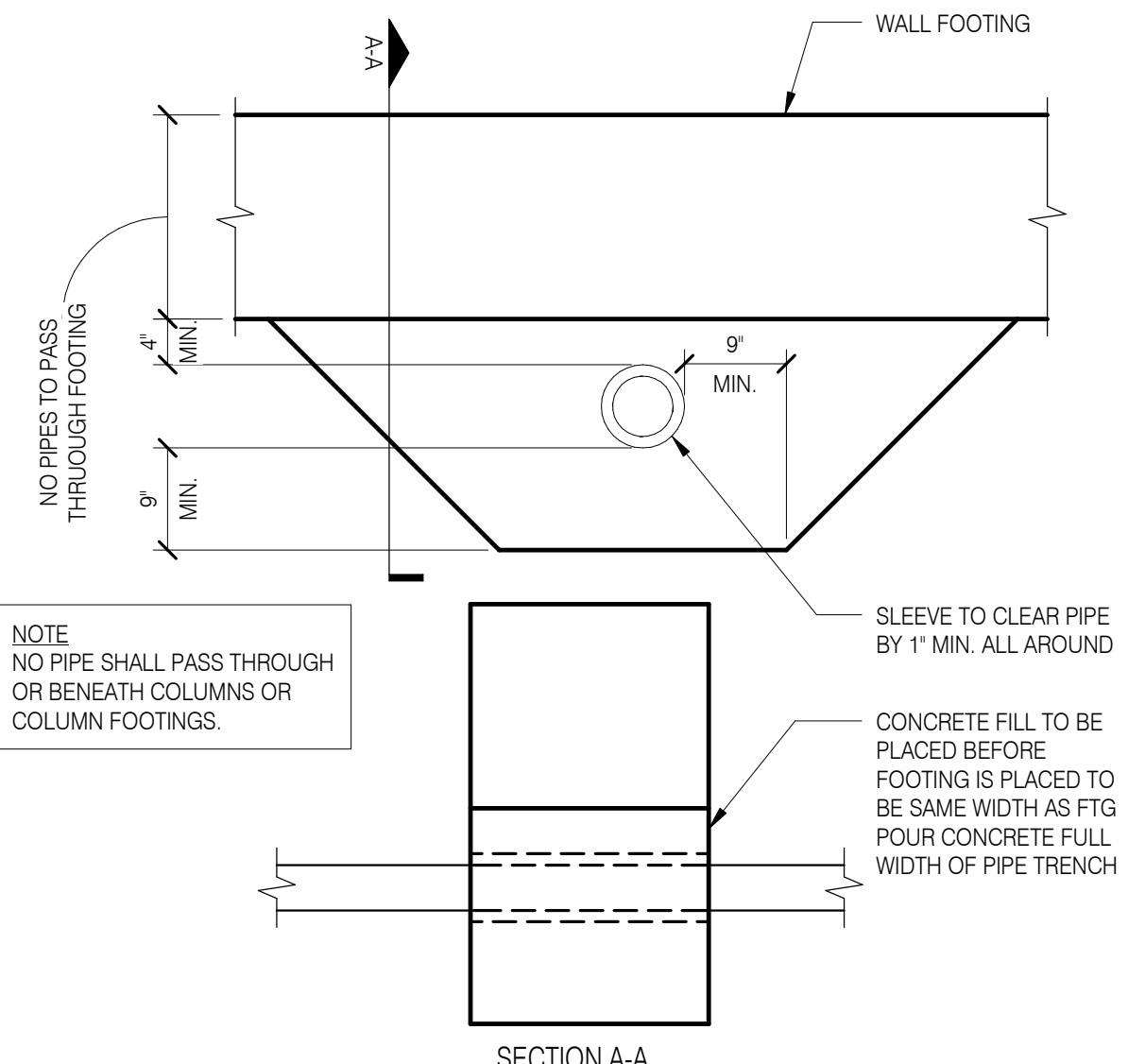
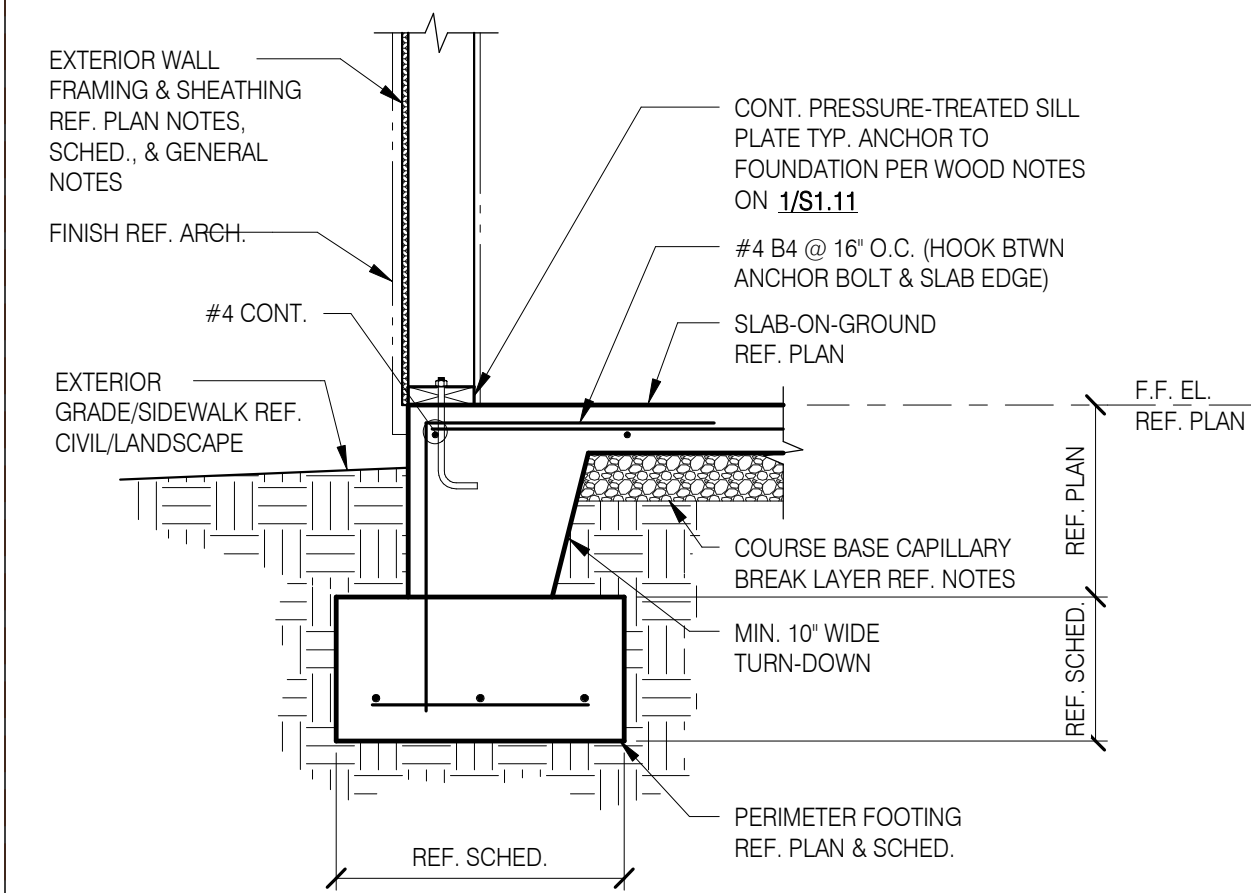
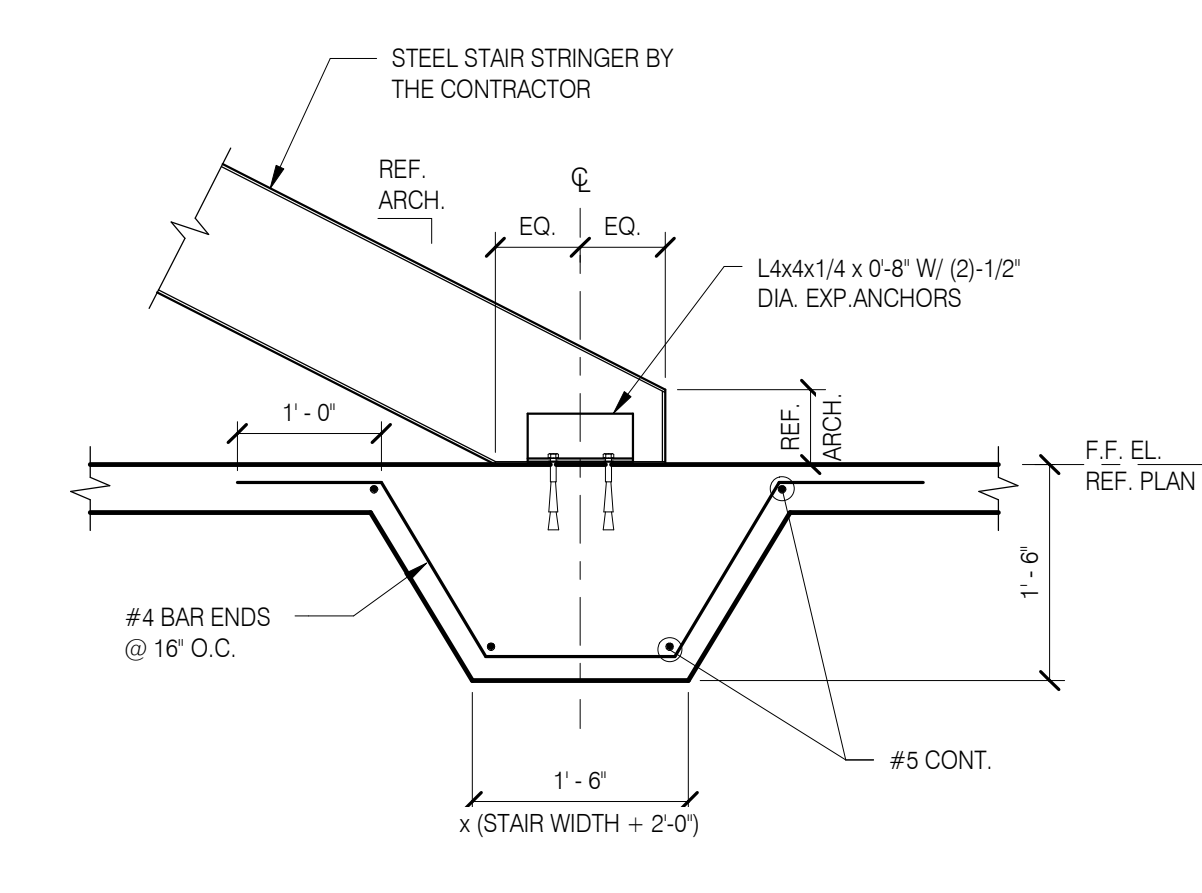
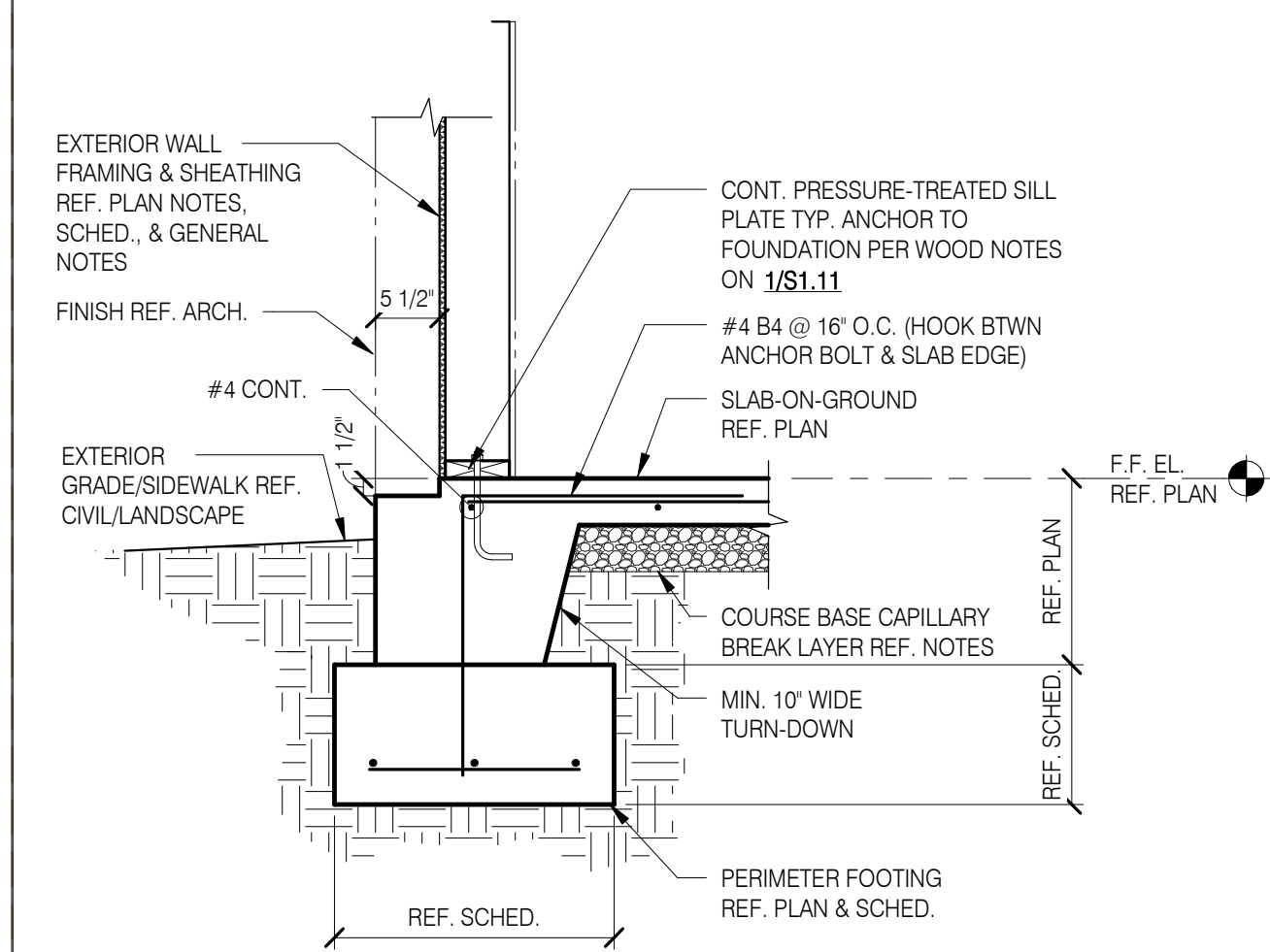
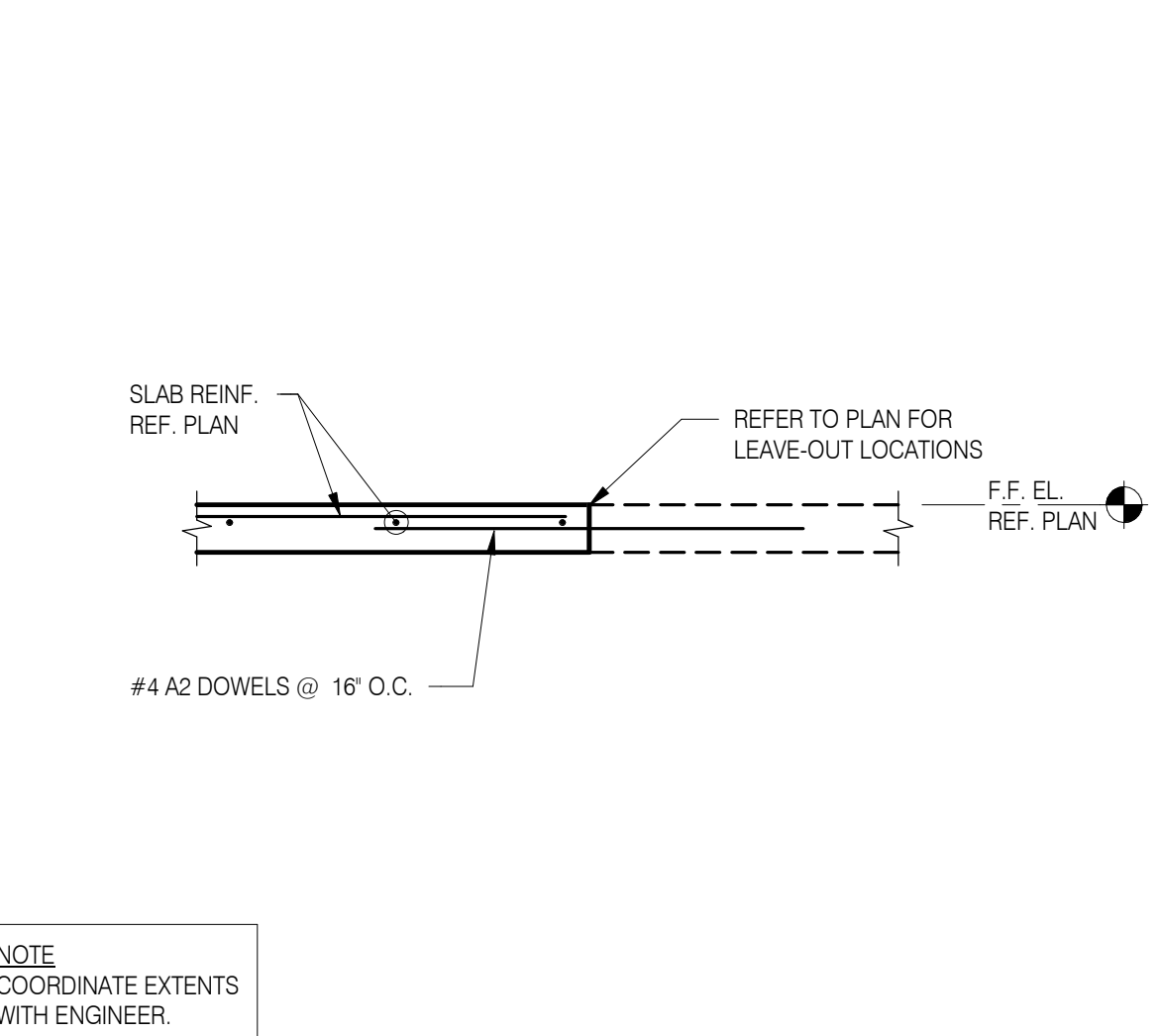
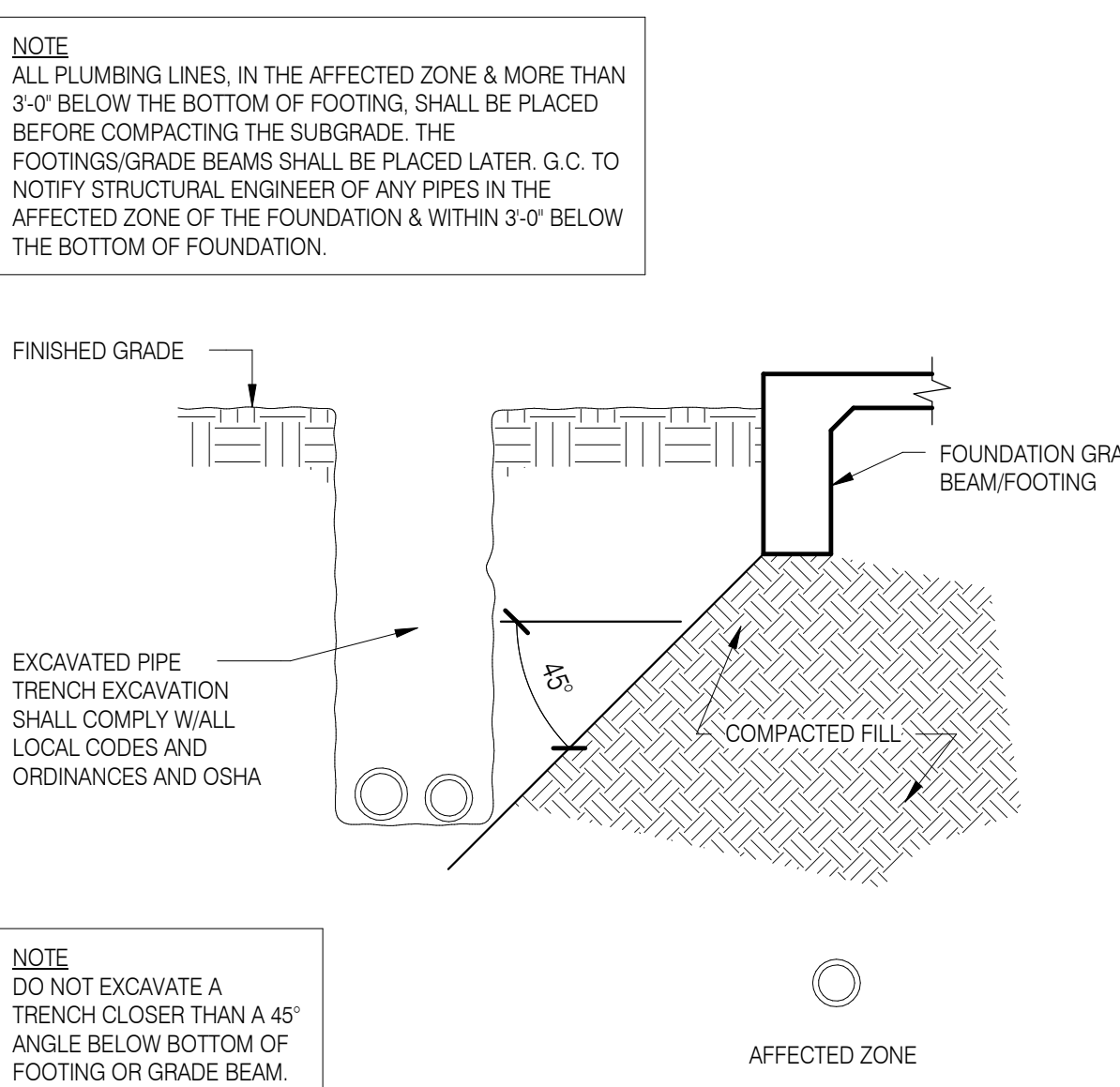
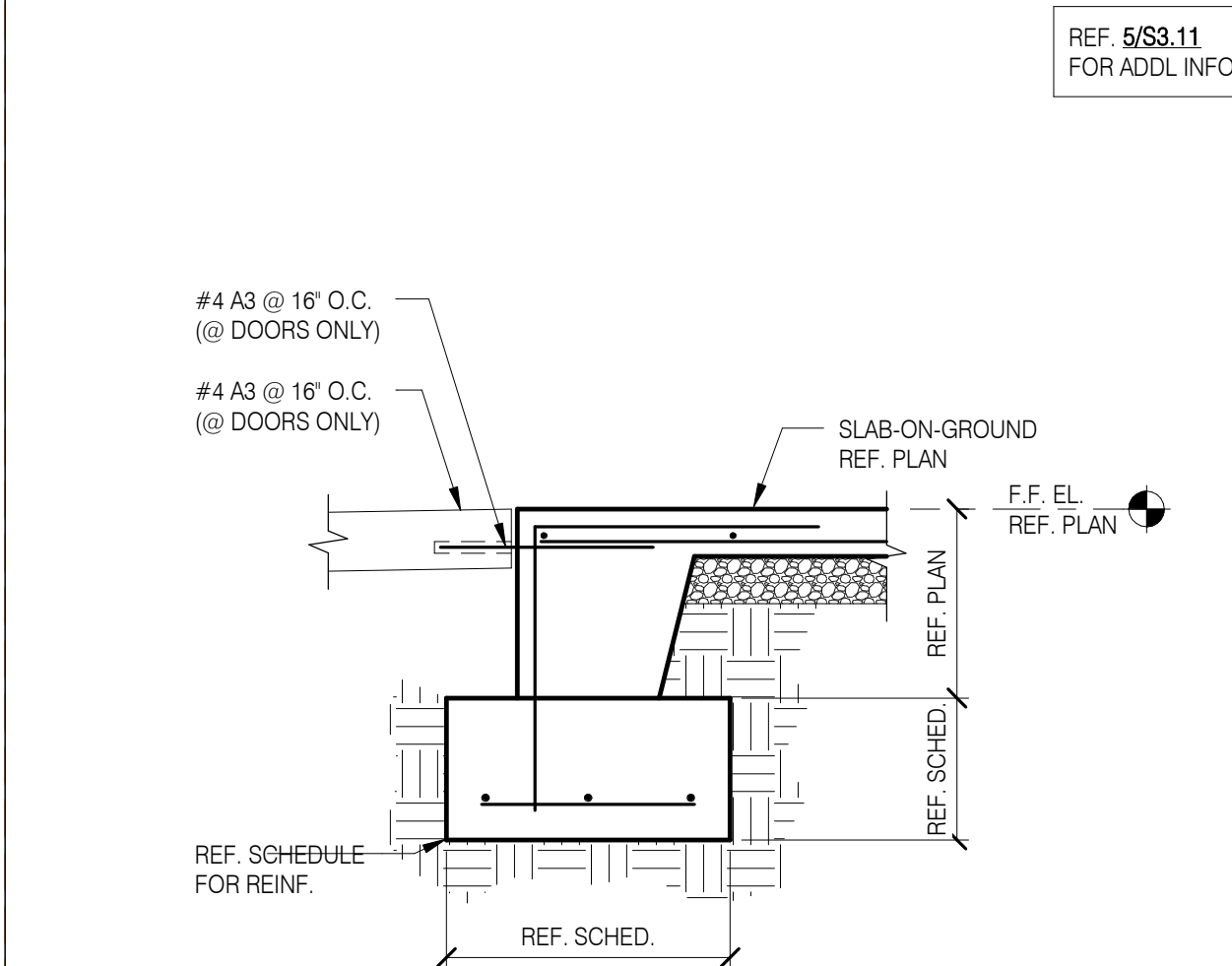
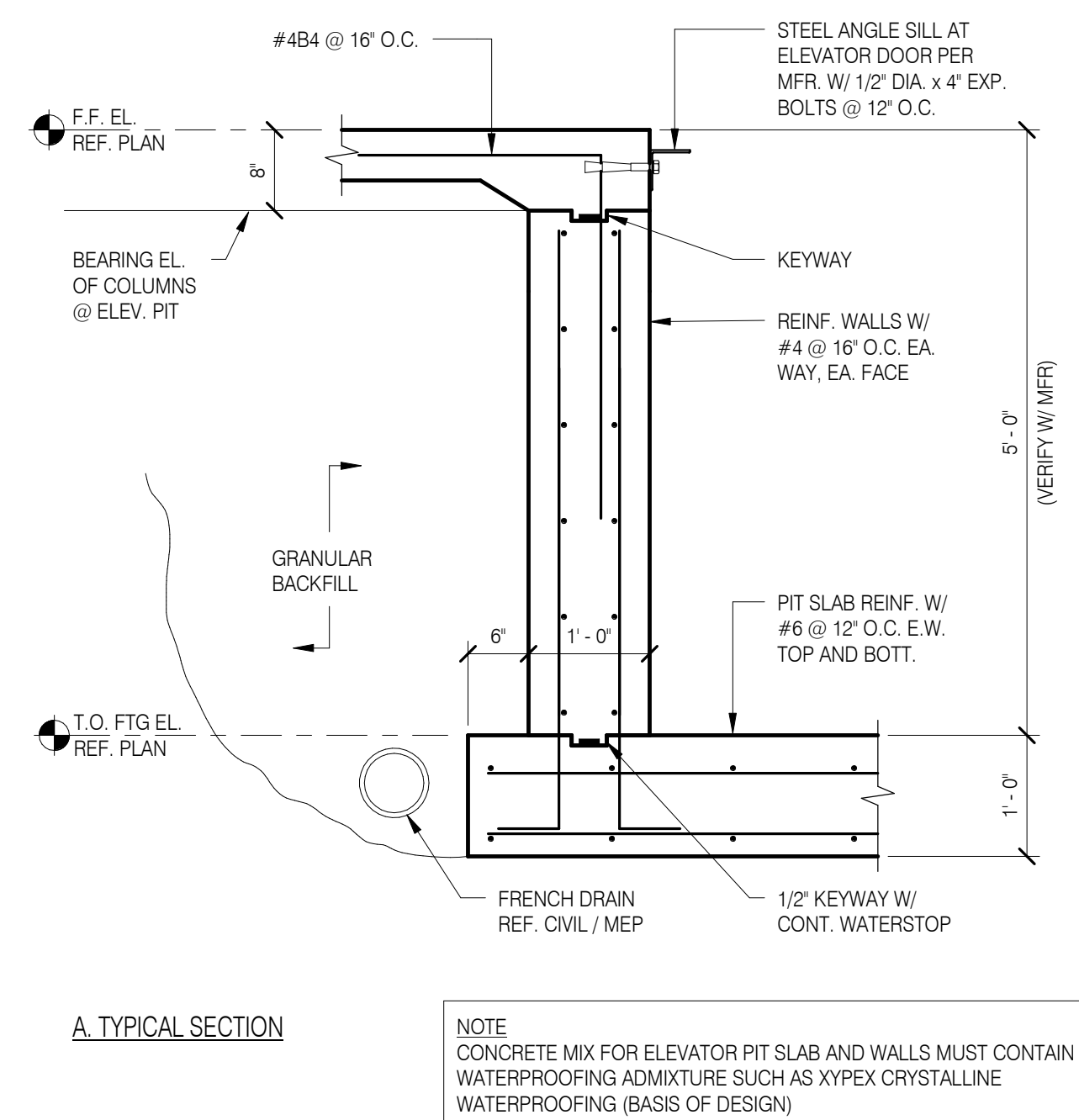
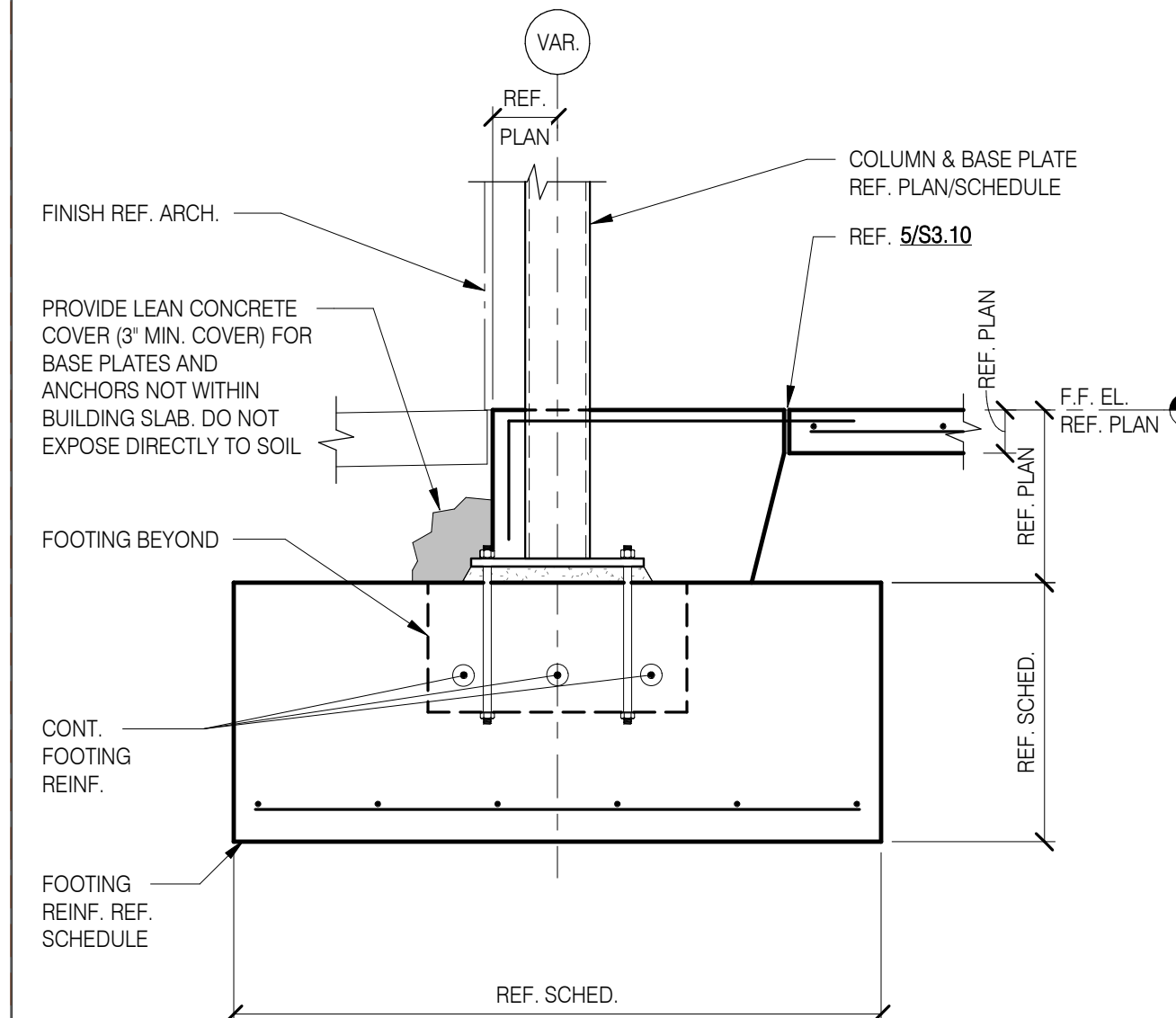
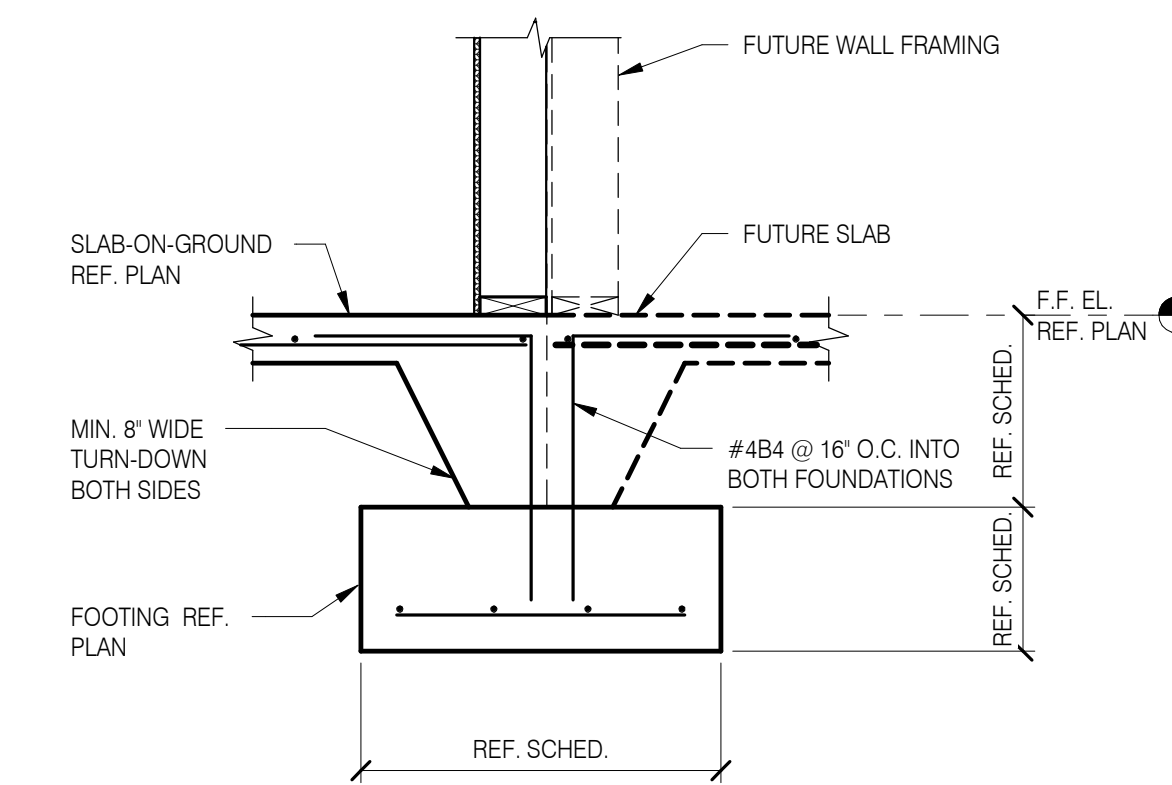
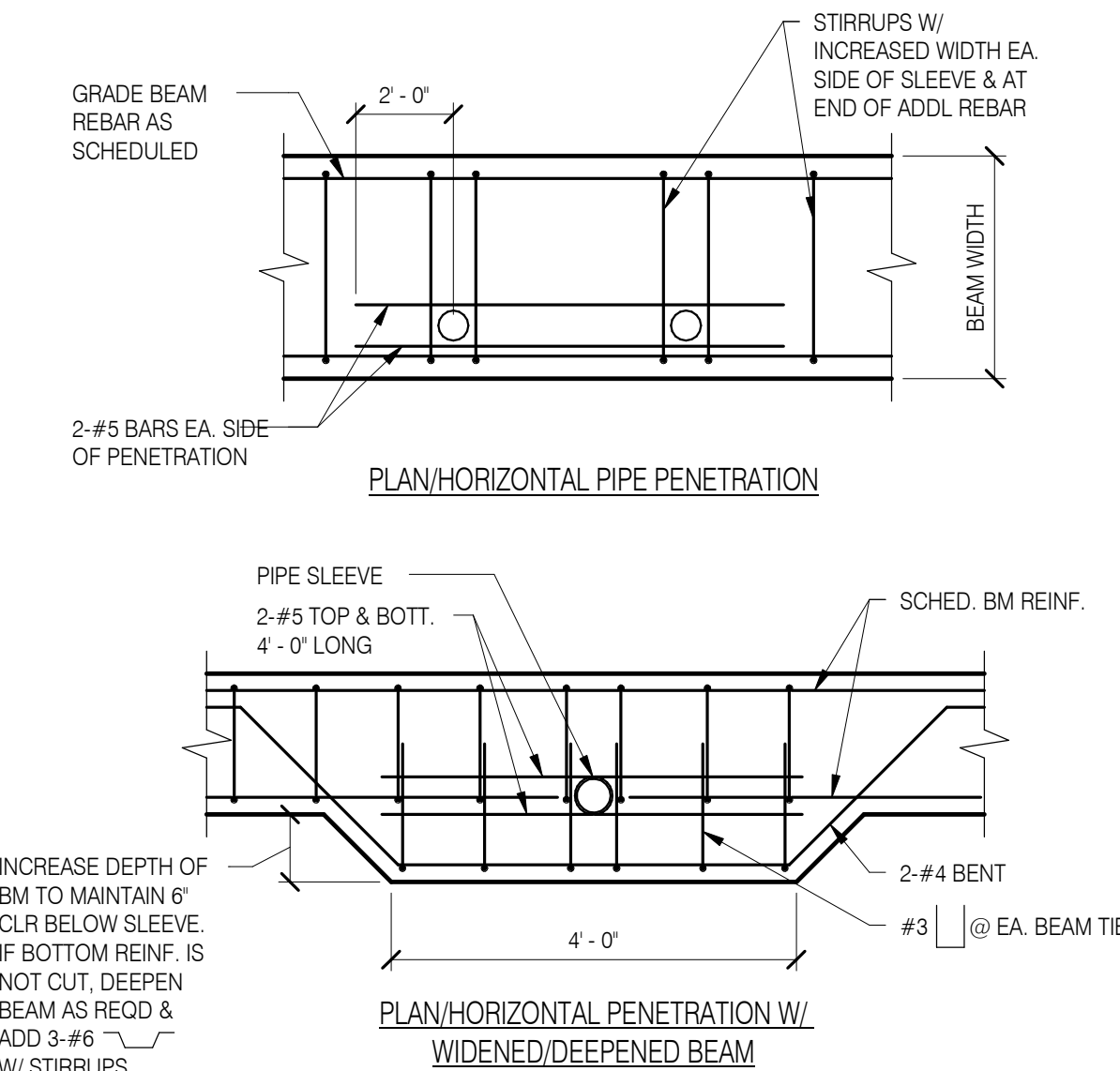
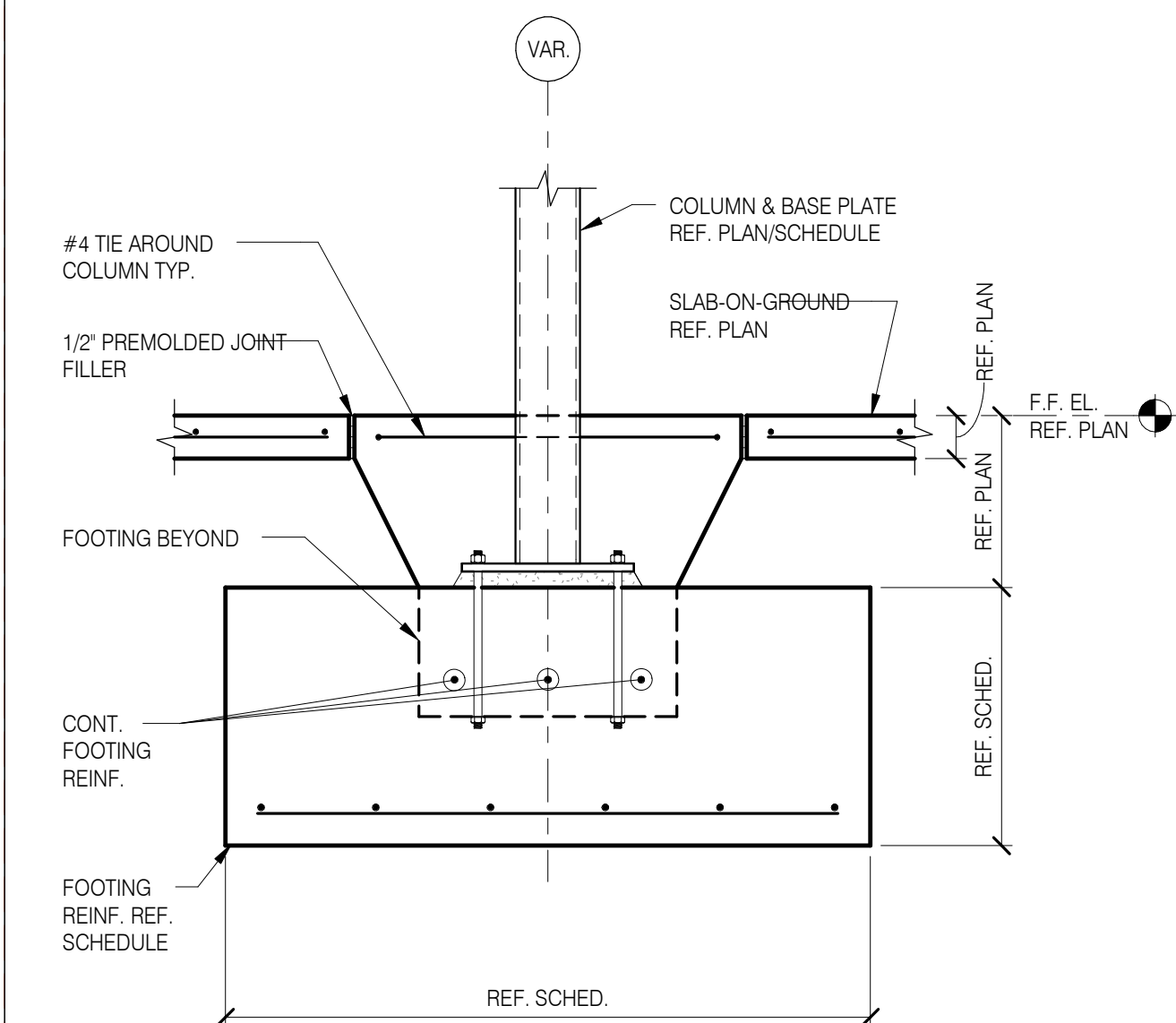
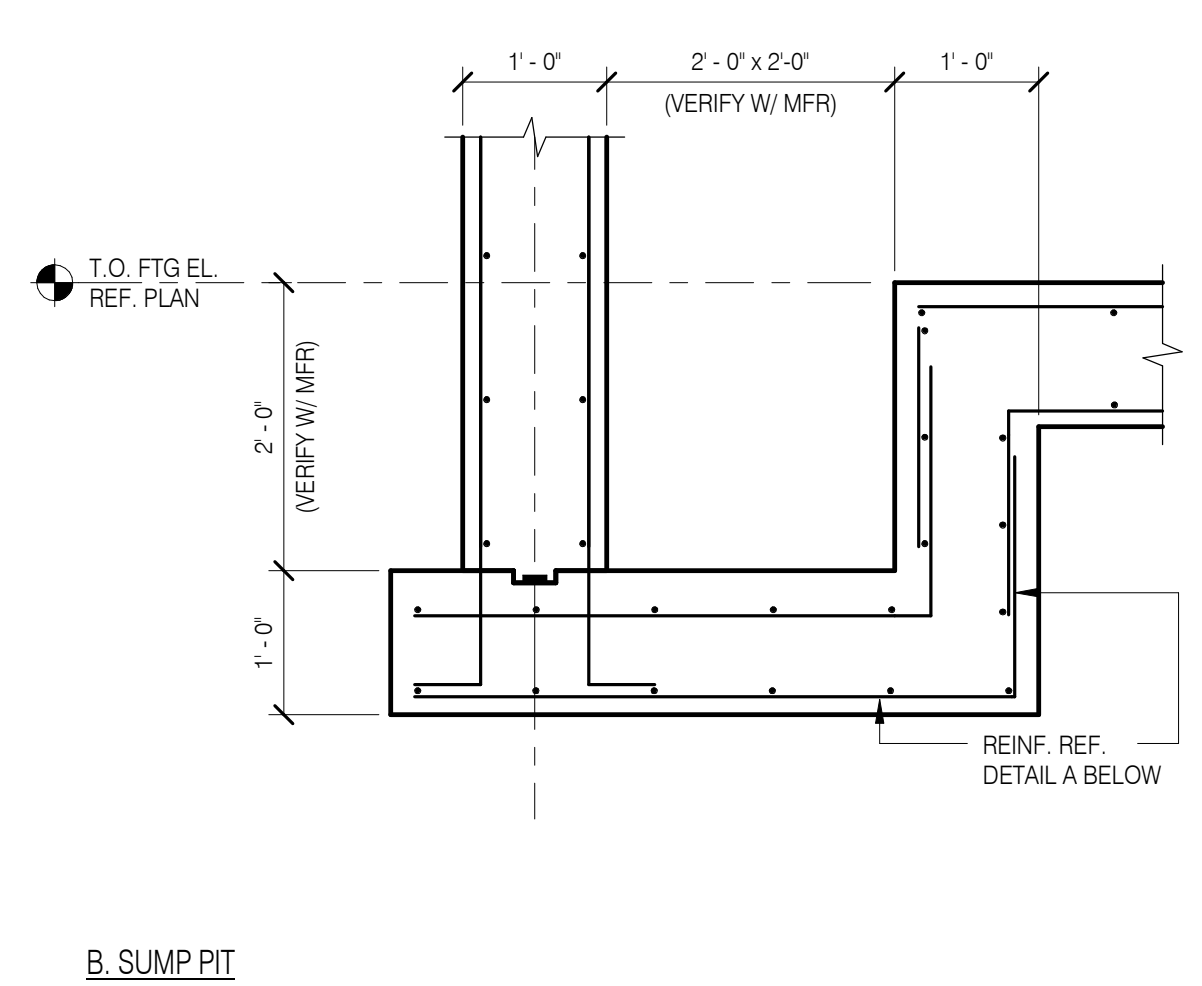
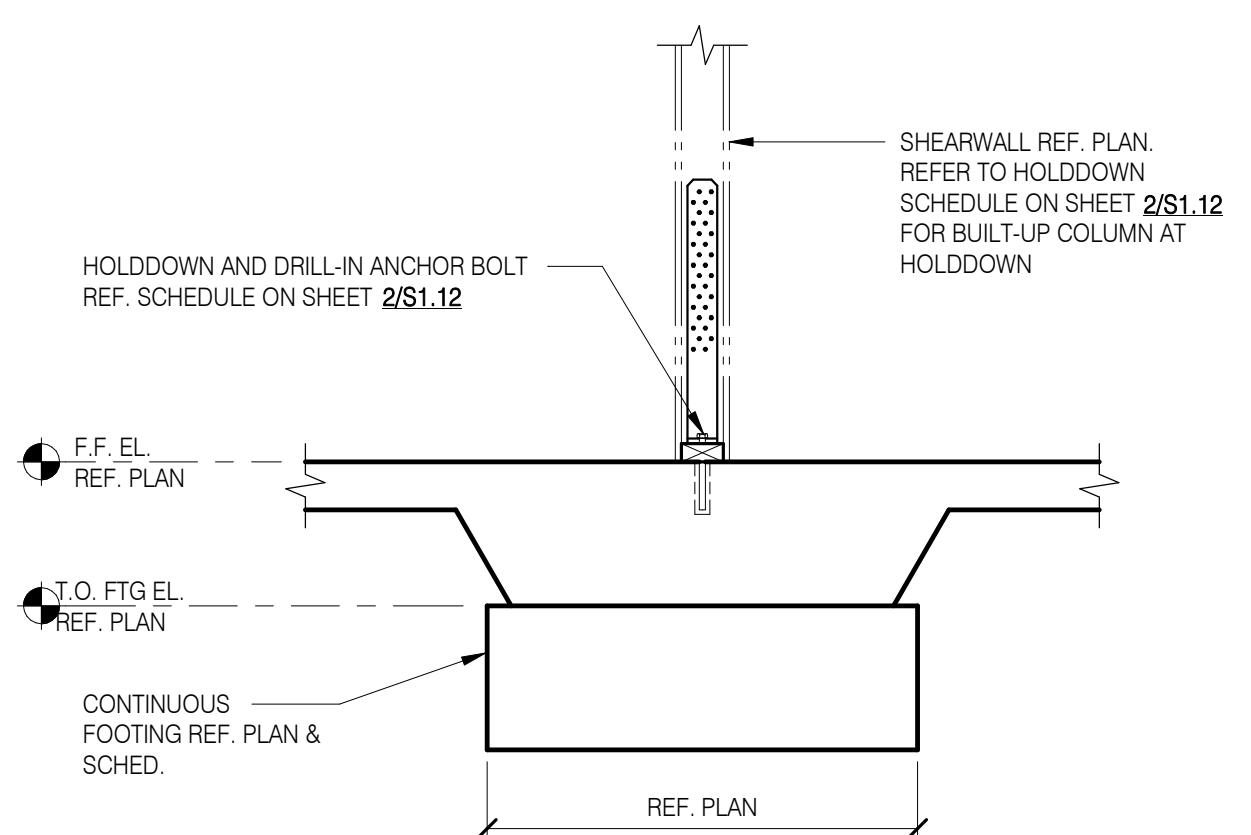
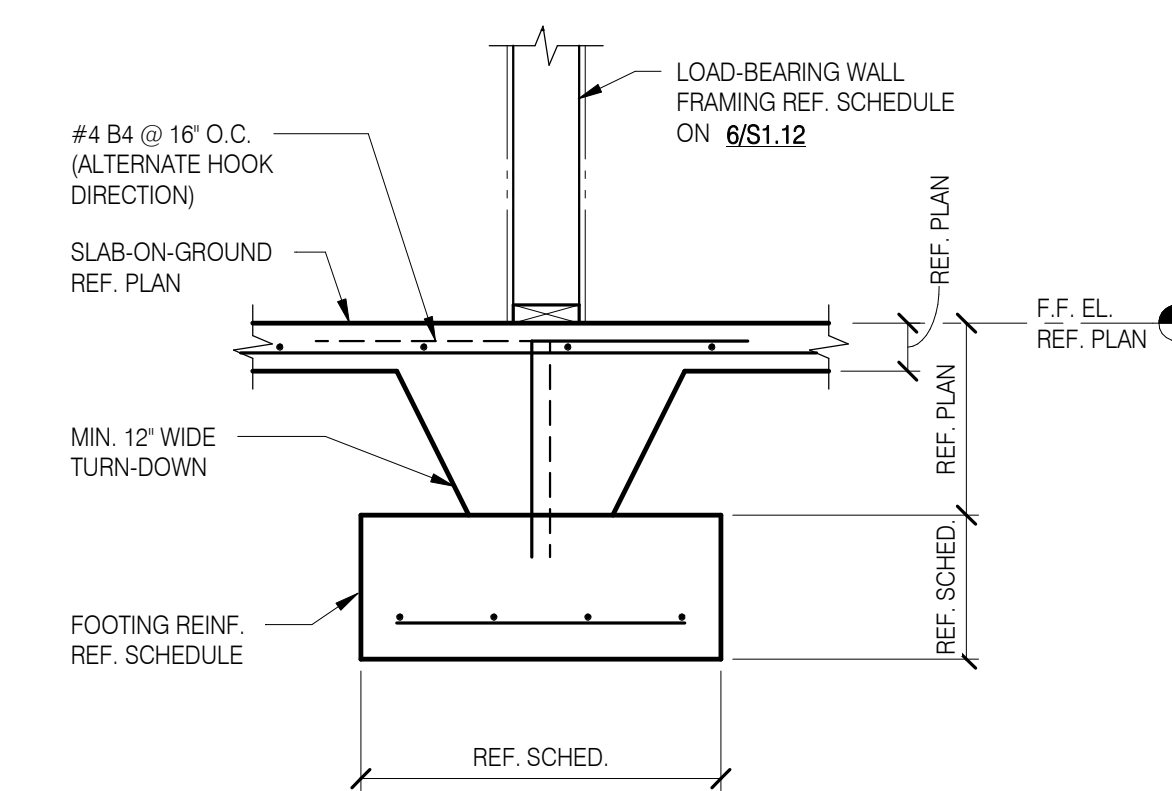
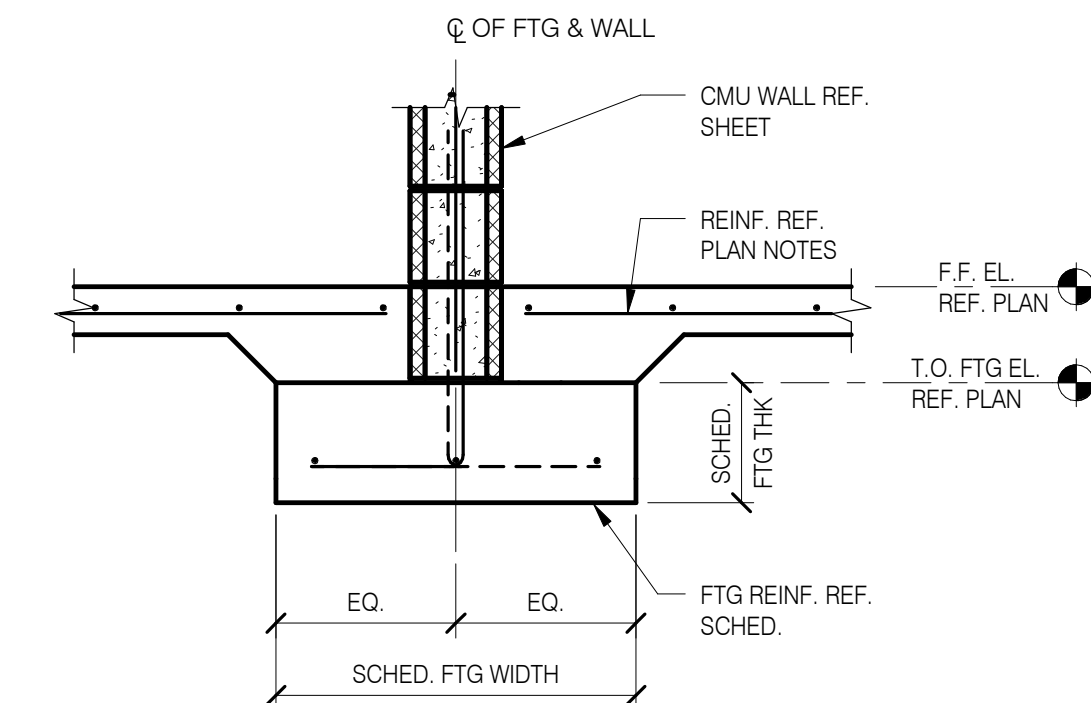
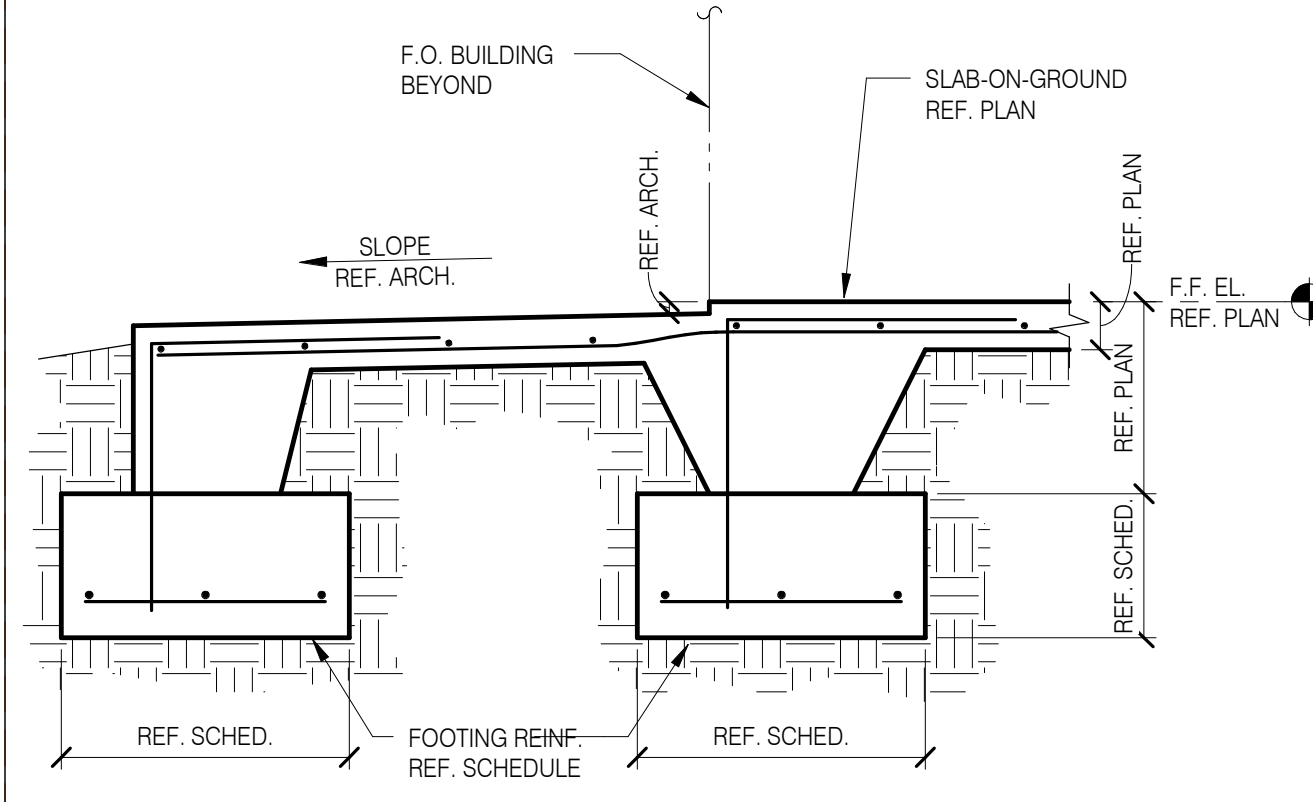
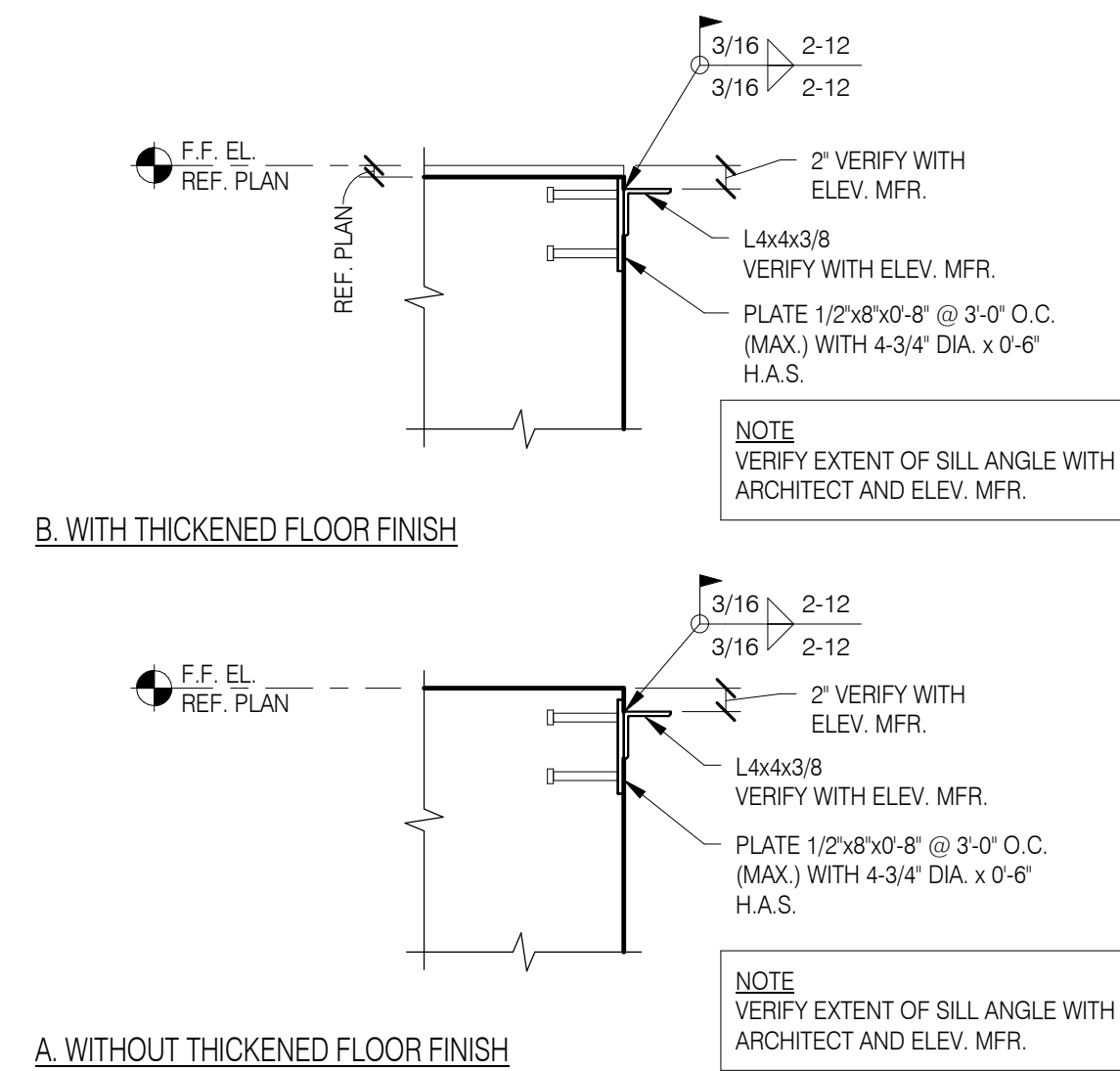
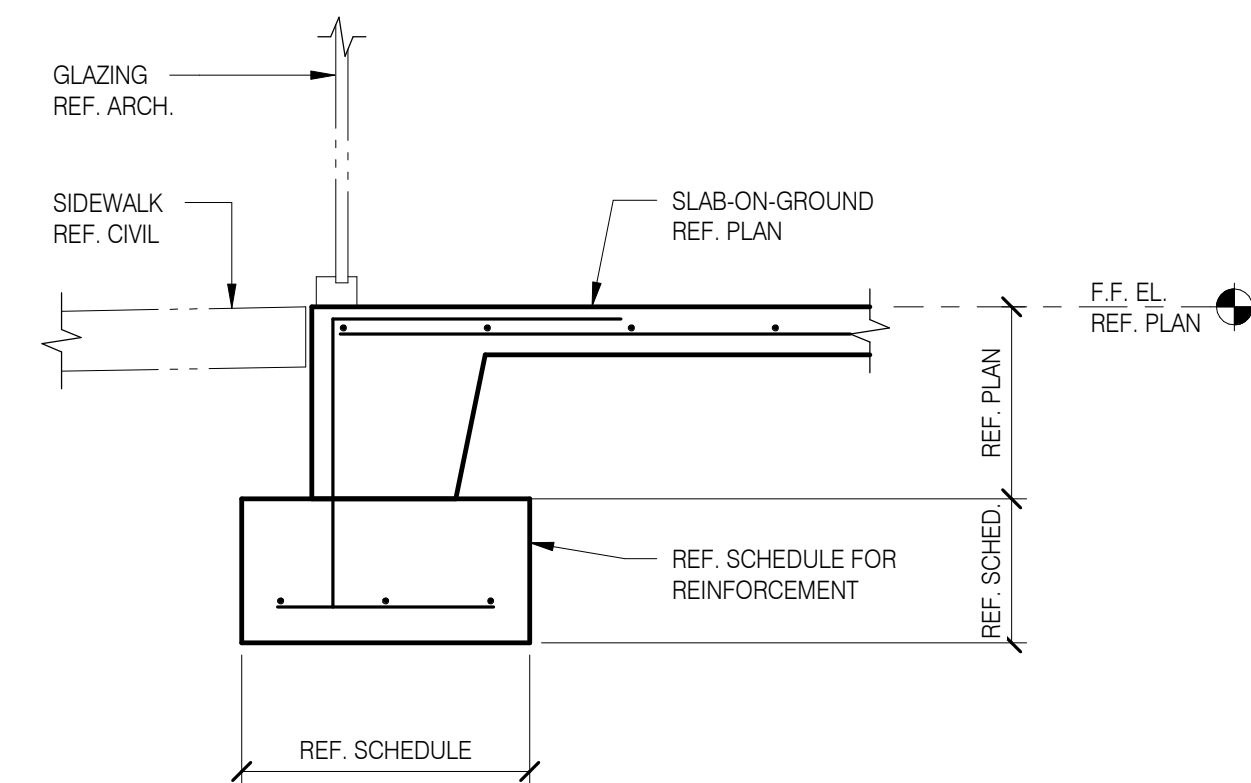
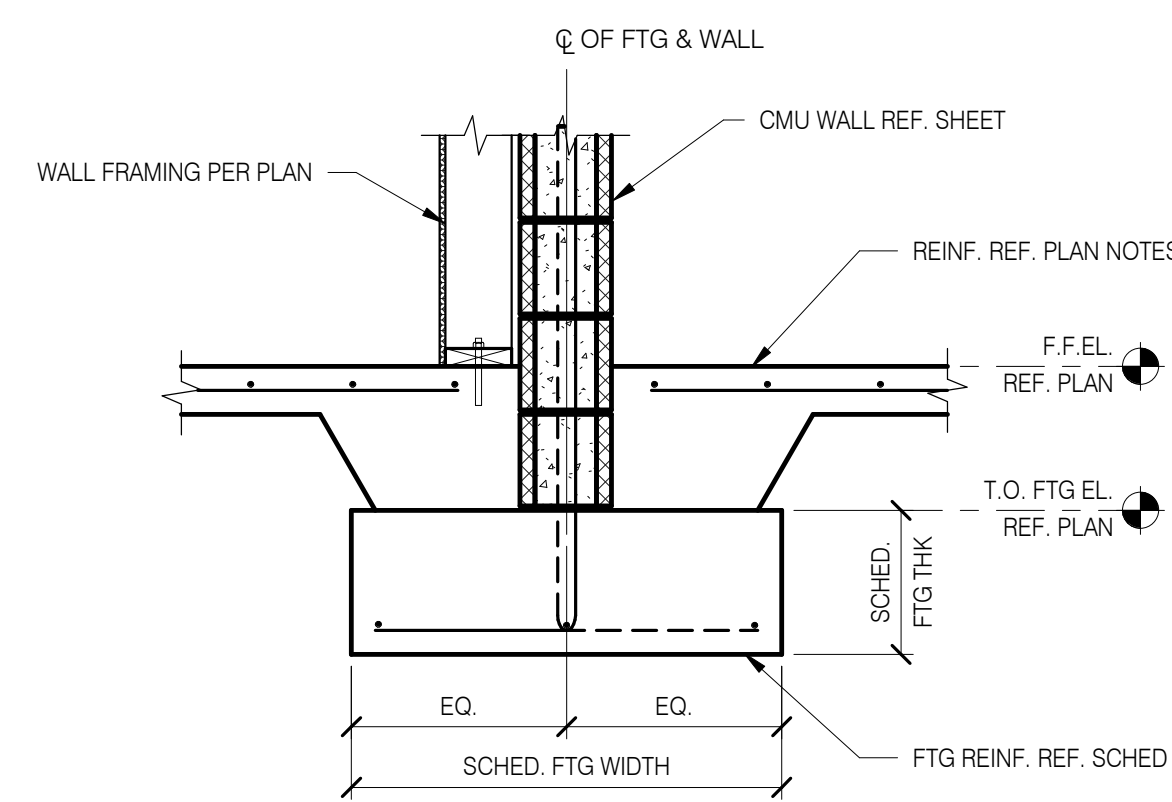
YP. FOOTING DETAILS

DOCUMENT SUBMISSION RECORD[illegible]

DRAWING DESCRIPTION

TYPICAL CONCRETE DETAILS

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Project Number	Sheet
Scale	NT



DOCUMENT SUBMISSION RECORD		
No.	Description	Date

[illegible]

NTS

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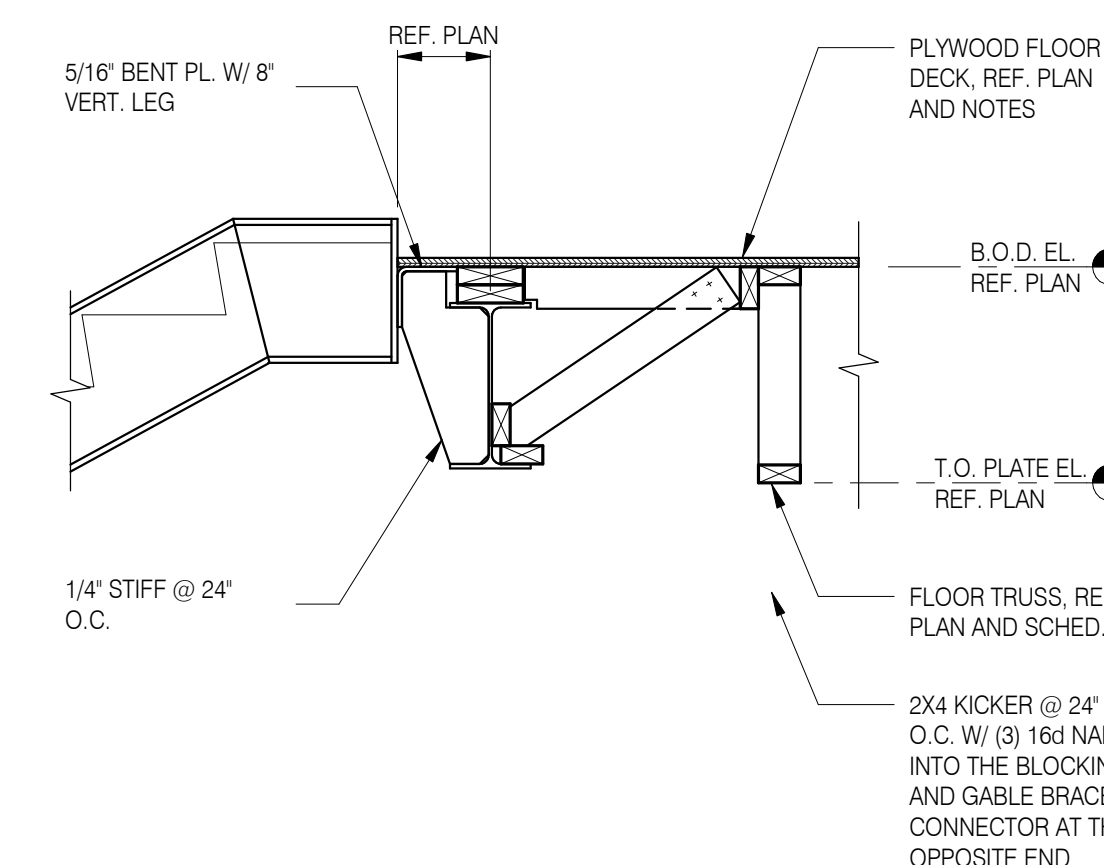
RESIDENCES @ HIGHLAND

BATON ROUGE, LA

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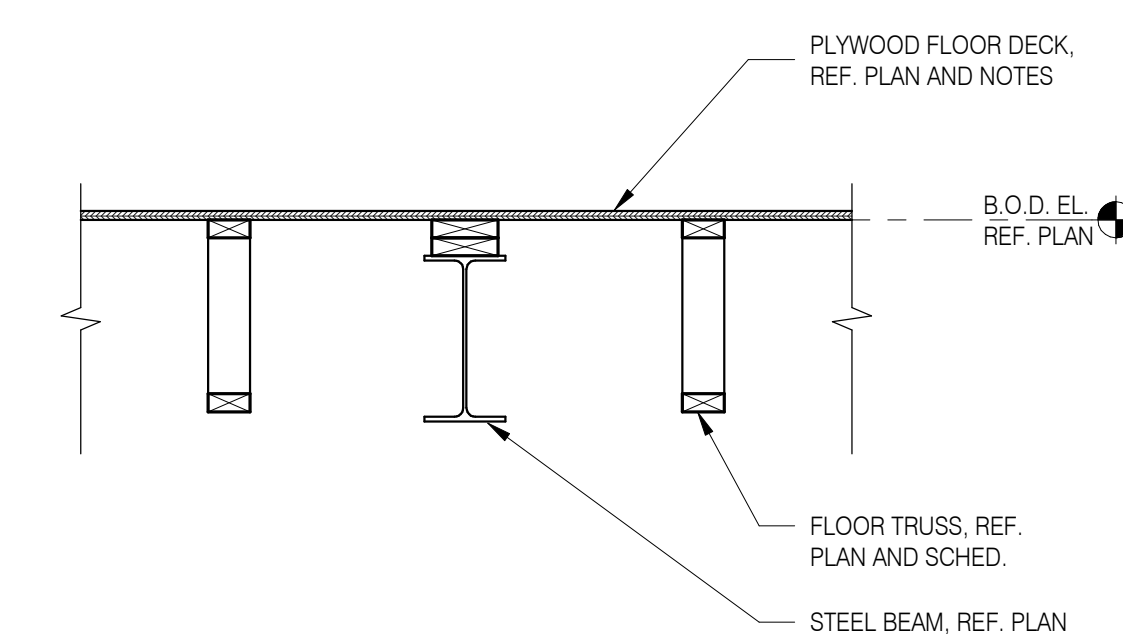
STEEL LATERAL BRACING DETAILS

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Project Number	Sheet
Scale	As indicated



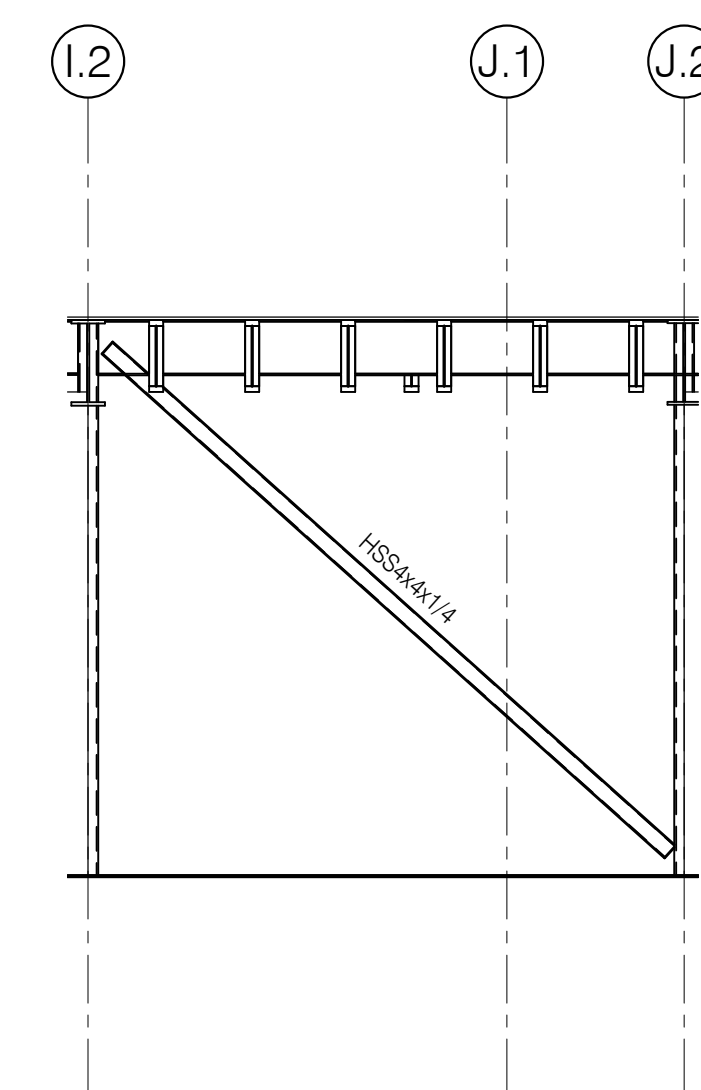
5 DETAIL AT TOP OF STAIR

NTS

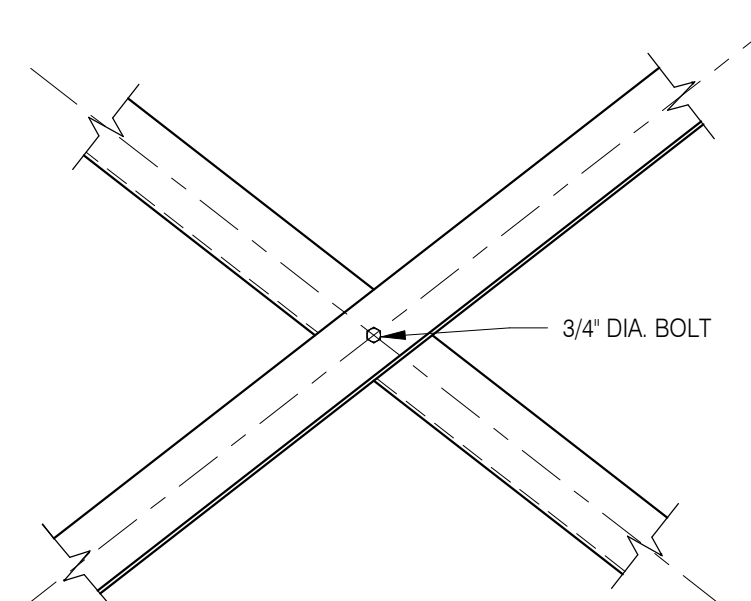
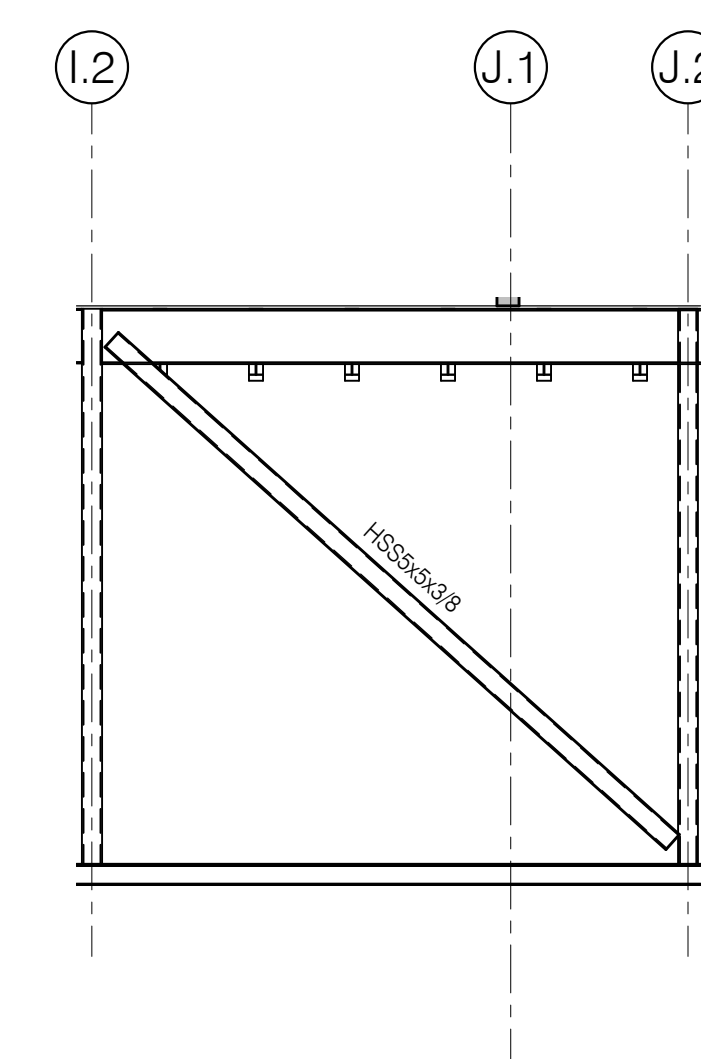


4 DETAIL AT STEEL BEAM

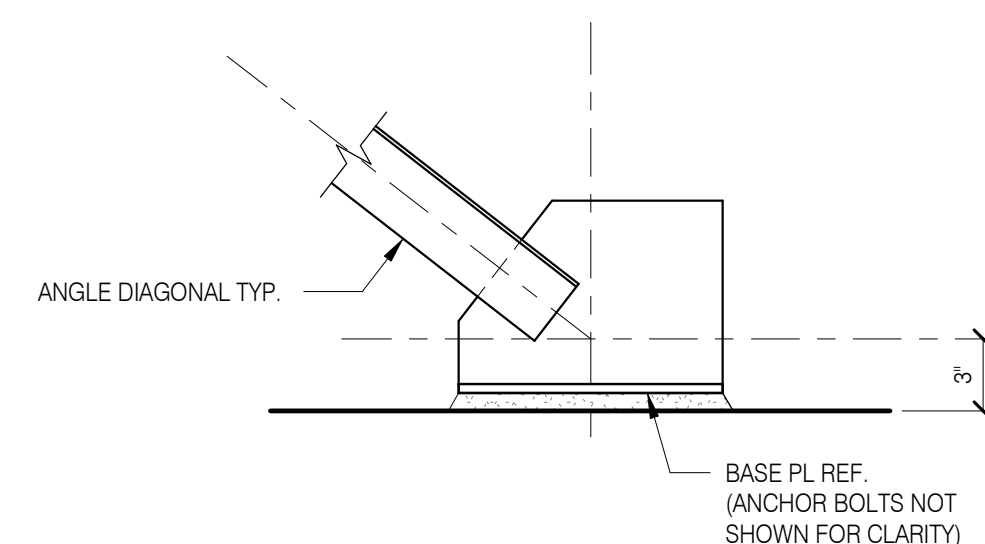
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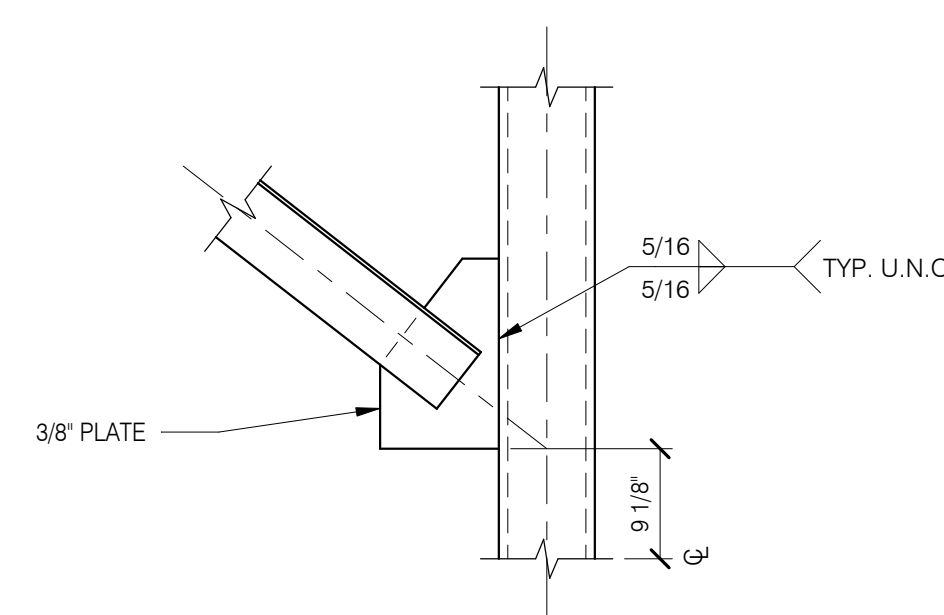
2 BRACE ELEVATION

$$1/4'' = 1'-0''$$


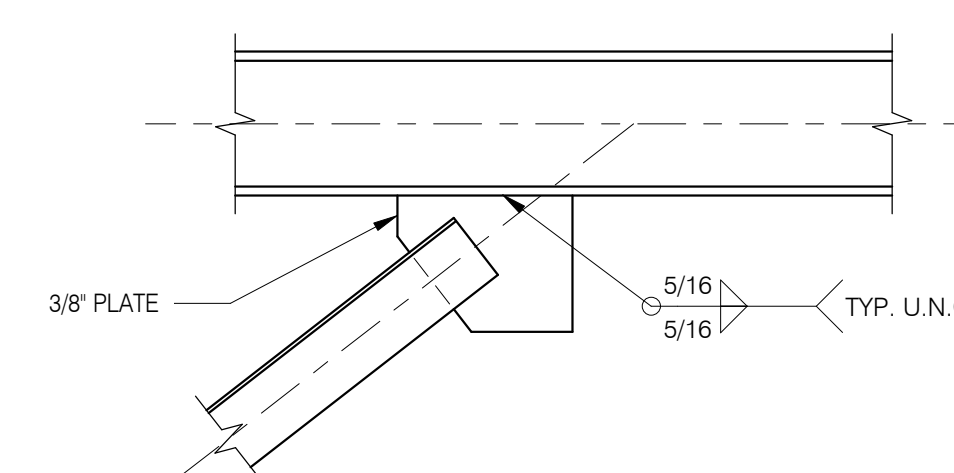
E. BRACE INTERSECTION



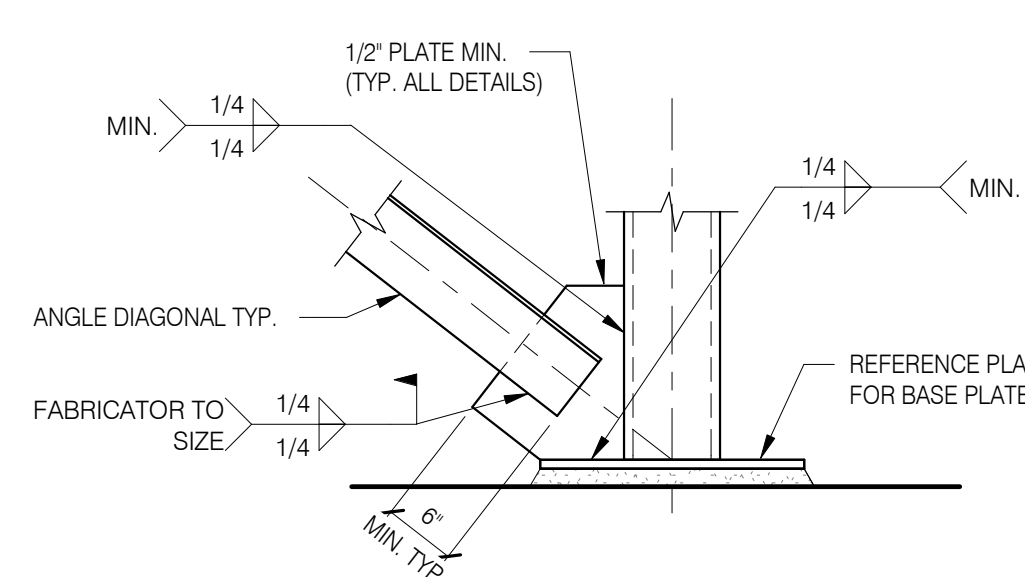
F. BRACE TO BASE PLATE W/O COLUMN



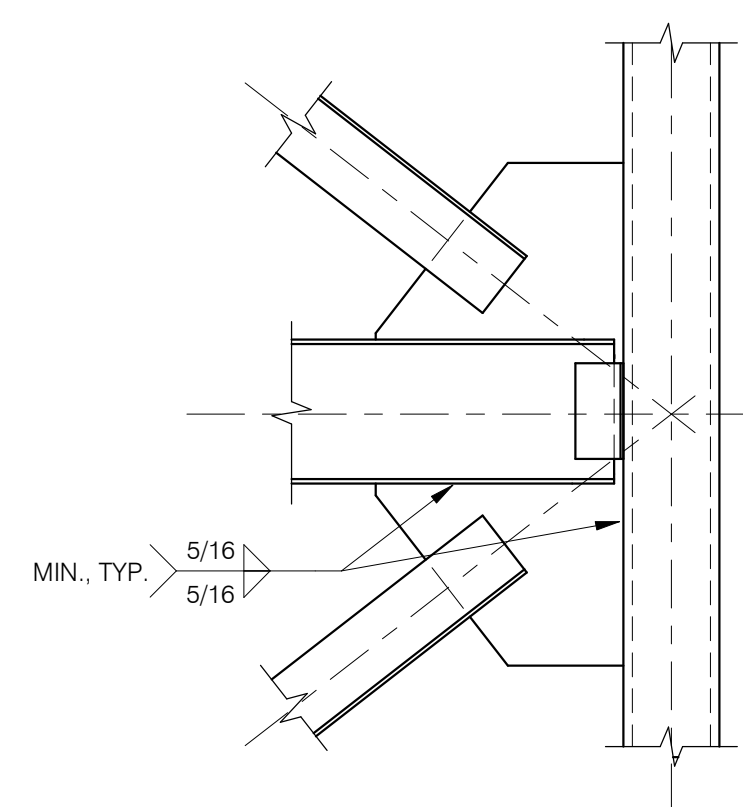
G. KNEE BRACE TO COLUMN FLANGE



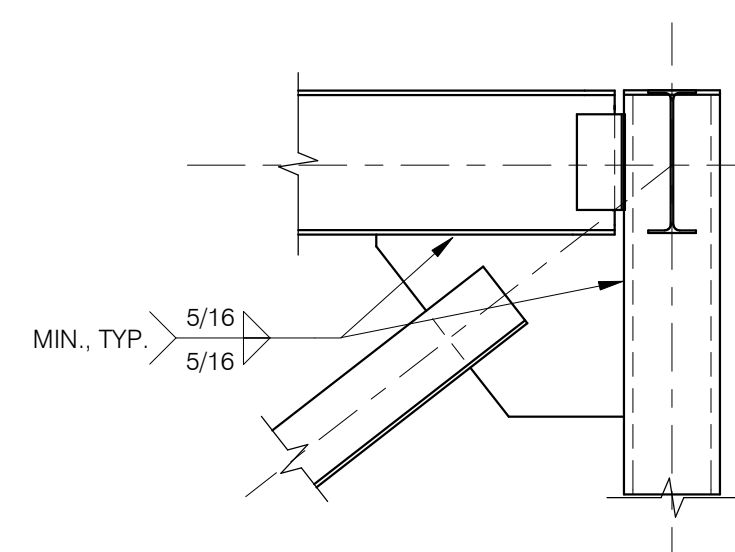
H. KNEE BRACE TO BEAM



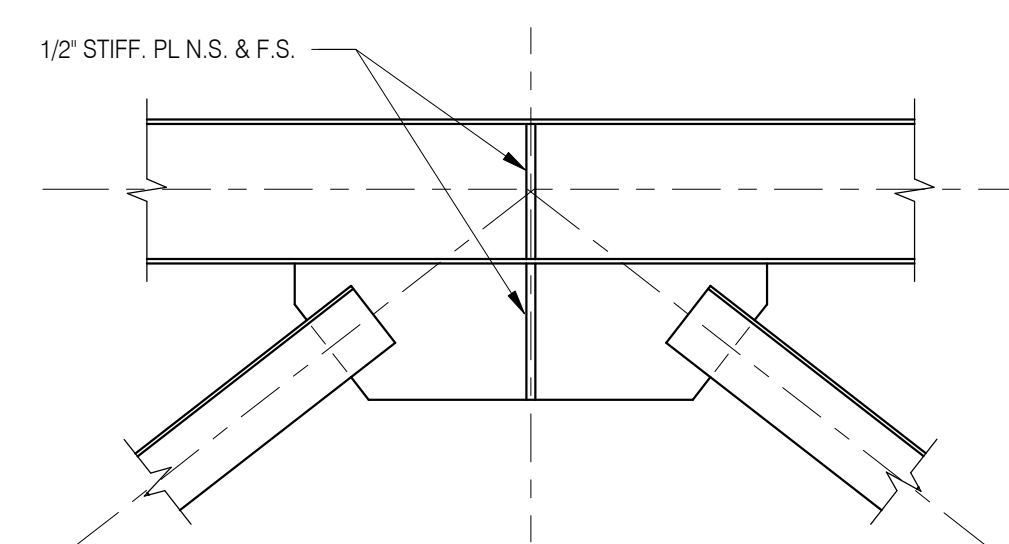
A. BRACE TO COLUMN AT BASE PLATE



B. BRACE TO COLUMN AT FLOOR OR ROOF



C. BRACE TO COL. AT ROOF



D. CHEVRON BRACE TO BEAM CONNECTION

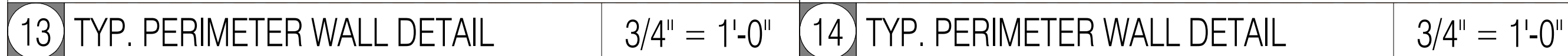
NOTE
FABRICATOR TO SIZE THE BRACE CONNECTIONS FOR
FORCES SHOWN ON BRACE FRAME ELEVATIONS.

3 BRACED FRAME DETAILS

$3/4'' = 1'-0''$

1 BRACE ELEVATION

$1/4'' = 1'-0''$



NTS

$$3/4'' = 1'-0''$$
$$3/4'' = 1'-0''$$
$$3/4'' = 1'-0''$$
$$3/4'' = 1'-0''$$

NTS

NTS

$$3/4'' = 1'-0''$$

$$3/4'' = 1'-0''$$

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$$3/4'' = 1'-0''$$
$$3/4'' = 1'-0''$$
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