

ADDENDUM NUMBER 01

Project	LIPSEY'S	Date	03.16.2026
Project Location	HIGHLAND ROAD, ST. GEORGE, LA	Architect's Project Number	24.154
Owner/Client	LIPSEY'S		
To	ARDEL CONSTRUCTORS	Attention	NICK ZAEFF
<p>This Addendum will become part of the Contract Documents. The Contractor shall promptly inform subcontractors and all others performing or supplying any of the Work of all relevant contents of this Addendum. In case of conflicting provisions with previous addenda or communications, provisions in this Addendum supersede only those conflicting issues. It is the responsibility of the Contractor to notify all subcontractors from whom it accepts bids of all changes in the drawings and specifications covering this project. Receipt shall be acknowledged by inserting the addendum number and its date in the bid form.</p>			
Distribution	ARDEL, OWNER, CONSULTANTS		
Prepared by	Lori Prochaska	Date	03.16.2026
Instructions / Description / References / Dates			

GENERAL:

PRIOR APPROVALS:

SPECIFICATIONS:

DRAWINGS:

Delete the previously issued sheets and replace them with the attached:

- S0.01 GENERAL NOTES
- S0.02 GENERAL NOTES AND WIND DIAGRAMS
- S2.01 PARTIAL FOUNDATION PLAN – AREA A
- S2.02 PARTIAL FOUNDATION PLAN – AREA B
- S2.03 PARTIAL FOUNDATION PLAN – AREA C
- S2.04 PARTIAL FOUNDATION PLAN – AREA D
- S2.05 FOUNDATION PLAN – OFFICE
- S2.10 MEZZANINE FLOOR FRAMING PLANS
- S2.11 SECOND FLOOR FRAMING PLAN – OFFICE
- S2.12 THIRD FLOOR FRAMING PLAN – OFFICE
- S2.20 ROOF FRAMING PLAN – OFFICE
- S2.21 PARTIAL ROOF FRAMING PLAN – AREA A
- S2.22 PARTIAL ROOF FRAMING PLAN – AREA B
- S2.23 PARTIAL ROOF FRAMING PLAN – AREA C
- S3.01 FOUNDATION DETAILS
- S3.02 FOUNDATION DETAILS
- S3.03 FOUNDATION DETAILS
- S3.04 FOUNDATION DETAILS
- S4.01 FRAMING DETAILS

S4.02 FRAMING DETAILS
S4.03 FRAMING DETAILS
S4.11 FRAMING DETAILS
S4.13 FRAMING DETAILS
S5.02 PANEL DETAILS
S5.11 PANEL ELEVATIONS
S5.12 PANEL ELEVATIONS
S5.13 PANEL ELEVATIONS
S5.14 PANEL ELEVATIONS
S5.15 PANEL ELEVATIONS
S5.16 PANEL ELEVATIONS
S6.01 BRACE ELEVATIONS & DETAILS

GENERAL NOTES

I. CODES AND SPECIFICATIONS

- A. GENERAL BUILDING CODE
1. International Building Code 2021 with Louisiana Amendments
2. ASCE 7 - Minimum Design Loads for Buildings and Other Structures
B. CONCRETE CODES
1. ACI 318 - Building Code Requirements for Structural Concrete
2. ACI 301 - Specifications for Structural Concrete
3. ACI 315 - Details and Detailing of Concrete Reinforcement
4. AWS D14 - Structural Welding Code: Reinforcing Steel
5. CRSI - Manual of Standard Practice
C. STRUCTURAL STEEL CODES
1. AISC 360 - Specifications for Structural Steel Buildings
2. AISC 303 - Code of Standard Practice for Steel Buildings and Bridges
3. AWS D11 - Structural Welding Code: Steel
4. AWS D13 - Structural Welding Code: Stainless Steel
5. Research Council on Structural Connections - Specification for Structural Joints using High-Strength Bolts

- D. MASONRY CODES
1. TMS 402 - Building Code Requirements for Masonry Structures
2. TMS 602 - Specification for Masonry Structures
E. COLD FORMED STEEL (LIGHT GAGE STEEL) CODE
1. AISI S100 - North American Specification for the Design of Cold Formed Steel Structural Members

- F. CONFLICTS IN STRUCTURAL REQUIREMENTS
1. Where conflict exists between the various publications as specified herein, the strictest requirements of the various publications shall govern unless noted otherwise. Where conflict exists among the various parts of the Structural Contract Documents, (Structural Drawings, General Notes, Specifications) the strictest requirements shall govern.
All Codes and Specifications listed above shall include all amendments and addenda for the editions referenced in the International Building Code version listed in GENERAL BUILDING CODES, Paragraph 1.A., as per Section 102.4. Otherwise, the latest edition of the Code or Specification shall be used.

- G. TYPICAL DETAILS
A. Details labeled "Typical Details" on the Drawings shall apply to all situations unless otherwise noted. Details shall apply whether or not they are keyed in at each location. Questions regarding applicability of typical details shall be determined by the Engineer.

- H. DESIGN CRITERIA
A. GENERAL
1. Dead loads: Dead load materials assumed in the design are shown on the Architectural and Structural Drawings. Any changes in construction materials from those shown on the Architectural or Structural Drawings shall be reported by the General Contractor to the Structural Engineer for verification of load carrying capacity of the structure.
2. Partitions: An allowance of 15 psf has been made for partitions as a uniformly distributed live load, wherever partitions are used and locations subject to change excluding public restrooms, corridors, restrooms, and places of public assembly.
3. Mechanical Rooms: Loadings for mechanical rooms are based on the minimum live loads as specified in the requirements of ACI 318 and ACI 301. Equipment including housekeeping pads as shown on the Mechanical Drawings are larger, in which case, the actual loads are used. The General Contractor shall submit weights to the Structural Engineer prior to producing a concrete mix design. Submittal. See note 4 for Type II, Cement requirements.
4. Type II, Blended Cement shall conform to ASTM C595. Concrete mix designs utilize Type II, Cement, they shall have a different mix ratio than the Type II mix designs and the data for mix verification (historical data or trial batches) shall be from mixes utilizing Type II, Cement.
5. Fly ash conforming to ASTM C318, Type C or F may be used. The maximum amount of fly ash shall be 25% of the total cementitious material by weight.
6. Aggregates shall conform to ASTM C33 for normal-weight concrete and ASTM C530 for lightweight concrete.
7. Water used in mixing concrete shall conform to ASTM C94.

- I. LIVE LOADS
CATEGORY UNIFORM LOAD (PSF) CONCENTRATED LOAD (#)
Roof N/A 250
Guard Rails & Hand Rails 50 pif or 200 lbs. applied at top rail in any direction, intermediate rails = 25 pif.
Office Floors 50 2000
Corridors above First Floor 80 2000
Offices 80 2000
Assembly Areas, 100
Lobbies 100 2000
Auditorium-Movable Seating 100 0
Auditorium - Stage Area 150 0
Computer Rooms 150 2000
Corridors, First Floor 100 0
Mechanical Rooms, typical 150 Actual Equip. Wt.
Restrooms 60 2000
Roof - For Roof Gardens 100 0
Stairways, Exit Walkways 100 3000
Storage - Heavy 250 0
Storage - Light 125 0
Live loads have been reduced on any member based on the Code cited in CODES AND SPECIFICATIONS, Paragraph 1. A.

- J. WIND LOADS
1. Wind pressure based on the requirements of Code cited in CODES AND SPECIFICATIONS, Paragraph 1.A.
2. V = 127 mph, Exposure: C.
3. Risk Category II

- K. ELEVATOR LOADS
1. Machine Beam, Car Buffer, Counterweight Buffer, and Guide Rail Loads. Assumed elevator loads to be submitted to the Engineer for approval on the Drawings, including machine beam reactions, car buffer reactions, counterweight buffer reactions, and horizontal and vertical guide rail loads. The General Contractor shall submit to the Structural Engineer final elevator shop drawings showing all loads to the structure prior to the installation of the elevators for verification of load carrying capacity.
2. Machine Room Live Load. Elevator machine rooms are designed for a uniform (unreduced) live load of 100 psf.

- L. SNOW LOADS
1. Ground snow load, pg = 0 psf

- M. RAIN LOADS
1. 15-min Precipitation Intensity = 8.64 in./hr
2. 60-min Precipitation Intensity = 4.51 in./hr

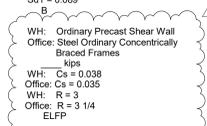
- N. SEISMIC LOADS (per ASCE 7-10)
1. Site classification D
2. Seismic importance factor I
3. Occupancy category II
4. Mapped spectral response accelerations Sa = 1.037
S1 = 0.056
S2 = 0.114
S3 = 0.089
5. Seismic design category B
6. Basic Seismic Force Resisting system

- O. DESIGN BEAR SHAP
1. Seismic response coefficient (Cs)
2. Response modification factor (R)

- P. ANALYSIS PROCEDURE (ELFP or MRSAs)

- Q. FOUNDATION - GENERAL

- R. GEOTECHNICAL REPORT
Foundation design is based on the following geotechnical report: Proposed "Geotechnical Engineering Report, Old Perkins Road Site, Geo-Prairieville, LA" Prepared by: Terracore Report No. EH2551235 Dated: 11/06/2025
Additional comments therein that relate to the work shown on these drawings shall be followed.
B. FOUNDATION APPROVAL AND INSPECTION BY AUTHORIZED INSPECTOR
1. Unless authorized otherwise by the Owner or Architect, the General Contractor shall notify the Geotechnical Engineer or other authorized inspector for review of foundation bearing surface, inspection of foundation installation, and foundation installation methods (including construction drawings, methods where required) and shall not place concrete prior to inspector's approval.
C. FOUNDATION REINFORCING STEEL INSPECTION
1. The Contractor shall notify the special inspector that the work is ready for inspection at least 24 hours before inspection.
2. The Contractor shall not pour any foundation concrete without inspection and approval by the special inspector, including but not limited to mix design verification, rebar placement, rebar welding, and concrete placement. Refer to the Special Inspection section of these General Notes for more information.
D. GRADE BEAMS
1. Grade Beam Side Forms
a. Form exposed faces of grade beams.
2. Grade beam bottom steel shall be chaired at 5 foot maximum centers using beam bolsters providing 3 inch bottom cover to reinforcing steel. Beam bolsters used shall be spliced for support on soil.
3. Bottom bars of grade beam shall be spliced (where required) at footings only. Top bars shall be spliced (where required) at the midspan of the grade beams. Lap length shall be 36 times bar diameter unless specified otherwise. Provide corner bars to match top and bottom steel.



- S. SLAB-ON-GRADE CONSTRUCTION
1. Slab on grade construction shall follow the recommendations of Guide for Concrete Floor and Slab Construction ACI 302.1R and ACI 302.2R.
a. Floor Finishes / Floor Levelness (FFFL) = 25/20 O.C.I.
b. Floor Finishes / Floor Levelness (FFFL) = 45/35 Warehouse.
2. Subgrade Preparation shall conform to the soils report noted above. If report does not specify the amount of select fill, provide fill as required above to reduce PFR to 1 inch.
3. Cranes for erecting tilt wall panels are not allowed on the floor slab.
F. VAPOR RETARDER
1. Vapor retarder shall: have no more than 0.01 perms when tested in accordance with ASTM E-98; meet or exceed the requirements of ASTM E-1745, Class A, (U.N.O.) in accordance with ACI 302.2R. Acceptable products include:
a. Stego Vapour, 15 mils, by Stego Industries LLC
b. Husky Yellow Guard 5 mils, by Husky Industries LLC
c. Approved equal conforming to performance requirements
3. All seams shall be overlapped a minimum of 6 inches and taped with polyethylene tape. All penetrations, knock-outs, and openings shall be sealed using a combination of the vapor retarder and polyethylene tape. Tape shall be watered by vapor retarder vendor.
3. Review of the installed product by a representative of the supplier is recommended.

- T. DRILLED PIER FOUNDATION
A. DESIGN SOIL PRESSURES
1. Total allowable pier capacities were calculated using the ultimate capacities per the geotechnical report (shown below) and an ASD factor of safety of 2.0.

Table with 4 columns: DRILLED SHAFT DIAMETER (INCHES), ULTIMATE COMPRESSION CAPACITY (KIP(S)), ULTIMATE TENSION CAPACITY (KIP(S)), and a diagram of a drilled pier cross-section.

- 2. Static load test to be performed per the geotechnical recommendations.
B. FIELD INSPECTION OF BEARING STRATUM
1. The bearing stratum of each drilled pier shall be inspected and approved by the Geotechnical Engineer or other authorized inspector prior to pouring of concrete.
C. BEARING ELEVATION
1. The bottom elevation of piers is shown on the drawings for bid purposes. The actual required bearing elevation may vary as required to provide proper capacity as determined by the Geotechnical Engineer. Footings shall be poured immediately adjacent to the pier.
D. PIER CAPS AND WIDENED GRADE BEAMS
1. Refer to Drawings for requirements at pier caps and widened grade beams.

VI. CONCRETE

- A. SPECIFICATIONS
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. Concrete shall conform to the requirements of ACI 318 and ACI 301.
3. Type III Portland Cement conforming to ASTM C150 shall be used, unless noted otherwise. The use of alternative cement types (such as Type II, Cement) shall be submitted to the Engineer-of-Record for approval prior to producing a concrete mix design. Submittal. See note 4 for Type II, Cement requirements.
4. Type II, Blended Cement shall conform to ASTM C595. Concrete mix designs utilize Type II, Cement, they shall have a different mix ratio than the Type II mix designs and the data for mix verification (historical data or trial batches) shall be from mixes utilizing Type II, Cement.
5. Fly ash conforming to ASTM C318, Type C or F may be used. The maximum amount of fly ash shall be 25% of the total cementitious material by weight.
6. Aggregates shall conform to ASTM C33 for normal-weight concrete and ASTM C530 for lightweight concrete.
7. Water used in mixing concrete shall conform to ASTM C94.

Table with 5 columns: USAGE, 28 DAY COMP. CONC, TYPE, NOMINAL MAX. AGGREGATE SIZE, and STRENGTH (PSI).

- 2. Concrete for slab-on-grade shall have a maximum water-cement ratio of 0.50.
3. Structures used shall be compatible with the Engineer's testing laboratory approval prior to placement of concrete in the field.
4. Any adjustments in approved mix designs including changes in admixtures must be submitted in writing to the Engineer and Owner's testing laboratory for approval prior to use in the field.
5. Concrete designed to be pumped shall be noted so on the mix designs and shall have mix proportions compatible with the pumping process.
6. Mix designs shall be proportioned based upon trial batching or experience as required by ACI 318.
D. EPOXY ADHESIVES
1. Typical epoxy adhesive for drilling and setting reinforcing into previously-cast, or existing, concrete shall be Hilti HV 200 system, or approved equivalent. Reference manufacturer specifications for installation procedures.

VII. REINFORCING STEEL

- A. SPECIFICATION
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. Reinforcement shall comply with the requirements of ACI 318 and ACI 318.
3. All steel reinforcement shall conform to ASTM A615, Grade 60.
4. All welded reinforcement shall conform to ASTM A1064.
5. All welded smooth wire fabric shall conform to ASTM A1064 (Yield strength = 65,000 psi).
6. All welded deformed wire fabric shall conform to ASTM A1064 (Yield strength = 70,000 psi).
7. All welded wire fabric shall be furnished in flat sheets only.
8. Welding shall comply with the requirements of AWS D14.
B. DETAILING AND BAR SUPPORTS
1. Detailing of and bar supports for reinforcing steel shall be in accordance with the ACI Standard Details and Detailing of Concrete Reinforcement as reported by ACI Committee 318. All continuous reinforcing steel shall be lapped using a Class B lap splice, unless specified otherwise.
C. MANUAL OF CONCRETE PRACTICE
1. Unless noted otherwise, methods of estimating, detailing, fabricating, placing and contracting for reinforcing materials shall follow the Manual of Standard Practice as published by the Concrete Reinforcing Steel Institute.
D. PLACEMENT OF WELDED WIRE FABRIC
1. Welded wire fabric shall be continuous across the entire concrete surface and not be interrupted by beams or girders and properly lapped just one cross wire spacing plus 2 inches.

- E. REINFORCING STEEL COVERAGE
Reinforcing steel coverage should conform to the requirements specified below. The reinforcing steel detailer shall adjust reinforcing steel cage sizes at intersecting structural members as required to allow clearance for intersecting reinforcing bar layers maintaining minimum specified cover. Cover in structural members not specified below shall conform to the requirements of ACI 318 Section 7.7 unless specified otherwise on the drawings.
1. Foundation Members
a. Grade Beams 1 1/2" top, 3" bottom, 2" sides (3" sides if cast against soil)
b. Drilled Piers 3" sides
c. Interior Slab-on-Grade 1 1/2" top cover for one layer of steel (u.o.n. on contraction joint typical detail) 1 1/2" top cover (u.o.n. on contraction joint) 3" bottom cover for two layers of steel.
d. Exterior Slab-on-Grade 2" top cover for one layer of steel. 2" top cover, 3" bottom cover for two layers of steel.
e. Basement Walls 2" dirt face, 3/4" interior face
f. Foundation Retaining Walls 2" both faces
g. Sump Walls, Pit Walls 2" both faces
h. Shallow Spread and Strip Footings 3" bottom and sides, 2" top
i. Industrial Slab-on-Grade 2" top cover for one layer of steel
j. Tilt-Up Panels 1" outside face from back of reveals, 3/4" interior face, 1" edges

VIII. POST-INSTALLED ANCHORS

- A. GENERAL
1. Except where indicated on the drawings, post-installed anchors shall consist of the following anchor types as provided by Hilti, Inc. or approved equal.
B. ANCHORING TO CONCRETE
1. Adhesive Anchors
a. Adhesive anchors for cracked and uncracked concrete use:
i. Hilti HIT-HY 200 V3 Safe Set System with the Hilti HIT-Z drill bit per ICC
ii. Hilti HIT-HY 200 V3 Safe Set System with Hilti hollow drill bit and VC 150/300 with Hilti HAS threaded rod per ICC ESR-4868 or approved equal
iii. Hilti HIT-RE 500 V3 Safe Set System with Hilti hollow drill bit and VC 150/300 with Hilti HAS threaded rod per ICC ESR-3814 or approved equal
iv. Hilti HIT-RE 500 V3 Safe Set System with Hilti HIT-RT roughening tool with Hilti HAS threaded rod per ICC ESR-3814 for diamond core holes or approved equal
b. Basis of design includes the following design parameters:
i. Cracked concrete
ii. Water-saturated concrete
iii. Base material temperature of 23 to 104 degrees Fahrenheit
iv. Allowable with hammer-drill, hollow drill bit system, and core drilling methods
2. Mechanical Anchors
a. Medium duty mechanical anchors for cracked and uncracked concrete use:
i. Hilti Kwik Bolt-TZ2 expansion anchors Safe Set System with hollow drill bit and VC 150/300 and SI-AT-A22 Adaptive Torque Module per ICC ESR-4266 or approved equal
ii. Hilti Kwik Bolt 3 expansion anchors safe set system with hollow drill bit and VC 150/300 and SI-AT-A22 with adaptive torque (uncracked concrete only) per ICC ESR-3027 or approved equal
iii. Hilti Kwik HUS EZ and Kwik HUS EZ-IE screw anchors Safe Set System with hollow drill bit and VC 150/300 per ICC ESR-3027 or approved equal
b. Heavy duty mechanical anchors for cracked and uncracked concrete use:
i. Hilti HDS underhead anchors per ICC ESR-1546 or approved equal
ii. Hilti HSL-3 expansion anchors per ICC ESR-1545 or approved equal
c. Mechanical anchors specified as carbon steel may only be used in dry hole conditions. It is the responsibility of the contractor to provide dry hole conditions for installation, or they shall contact the engineer of record for a stainless steel anchor alternative.

- C. GENERAL REQUIREMENTS
1. Anchor capacity used in design shall be based on the technical data published by Hilti or such other method as approved by the structural engineer of record. Substitution requests for alternate products must be approved in writing by the structural engineer of record prior to use. Contractor shall provide calculations that have been sealed by another licensed engineer demonstrating that the substituted product is capable of meeting the performance of the specified product. Substitutions will be evaluated by their having an ICC ESR showing compliance with the relevant building code for seismic uses, load resistance, installation category, and availability of comprehensive installation instructions. Adhesive anchor evaluation will also consider creep, in-service temperature, installation temperature, moisture condition of concrete, and drilling methods.
2. Install anchors per the Manufacturer Printed Installation Instructions (MPII), as included in the anchor packaging.
3. Adhesive anchors in upwardly-inclined orientation and/or embedment depths greater than 10 inches must be installed using the Hilti Profi Piston Plug System.
4. The contractor shall arrange an anchor manufacturer's representative to provide onsite installation training for all of the anchoring products specified. The structural engineer of record must receive documented confirmation that all personnel who install anchors are trained prior to the commencement of anchor installation.
5. Anchor capacity is dependent upon spacing between adjacent anchors and proximity of anchors to edge of concrete. Install anchors in accordance with spacing and edge clearances indicated on the drawings.
6. Existing reinforcing bars in the concrete structure may conflict with specific anchor locations. Unless noted on the drawings that the bars can be cut, the contractor shall review the existing structural drawings and shall undertake to locate the position of the reinforcing bars at the locations of the concrete anchors by ferrocass, GPR, x-ray, or other means.
7. All hole conditions are assumed to be dry or water-saturated, unless noted otherwise. The contractor shall be responsible for removing water in order to avoid water-filled hole or submerged hole conditions before installing anchors.

IX. POST-INSTALLED REBAR

- A. GENERAL
1. Except where indicated on the drawings, post-installed rebar shall use the following products as provided by Hilti, Inc. or approved equal.
B. REBAR DOWELING INTO CONCRETE
1. Adhesive Anchors
a. Adhesive anchors for cracked and uncracked concrete use:
i. Hilti HIT-HY 200 V3 Safe Set System with the Hilti hollow drill bit and VC 150/300 with continuously deformed rebar per ICC ESR-4868 or approved equal
ii. Hilti HIT-RE 500 V3 Safe Set System with Hilti hollow drill bit and VC 150/300 with continuously deformed rebar per ICC ESR-3814 or approved equal
iii. Hilti HIT-RE 500 V3 Safe Set System with Hilti HIT-RT roughening tool with continuously deformed rebar per ICC ESR-3814 in diamond core holes or approved equal
b. Basis of design includes the following design parameters:
i. Cracked concrete
ii. Water-saturated concrete
iii. Base material temperature of 23 to 104 degrees Fahrenheit
iv. Allowable with hammer-drill, hollow drill bit system, and core drilling methods
v. Current ICC-ES report with approval for development of bar using ACI provisions for embedment depths greater than 20 bar diameters
C. GENERAL REQUIREMENTS
1. Anchor capacity used in design shall be based on the technical data published by Hilti or such other method as approved by the structural engineer of record. Substitution requests for alternate products must be approved in writing by the structural engineer of record prior to use. Contractor shall provide calculations that have been sealed by another licensed engineer demonstrating that the substituted product is capable of meeting the performance of the specified product. Substitutions will be evaluated by their having an ICC ESR showing compliance with the relevant building code for seismic uses, load resistance, installation category, and availability of comprehensive installation instructions. Adhesive anchor evaluation will also consider creep, in-service temperature, installation temperature, moisture condition of concrete, and drilling methods.
2. Install anchors per the Manufacturer Printed Installation Instructions (MPII), as included in the anchor packaging.
3. Adhesive anchors in upwardly-inclined orientation and/or embedment depths greater than 10 inches must be installed using the Hilti Profi Piston Plug System.
4. The contractor shall arrange an anchor manufacturer's representative to provide onsite installation training for all of the anchoring products specified. The structural engineer of record must receive documented confirmation that all personnel who install anchors are trained prior to the commencement of anchor installation.
5. Anchor capacity is dependent upon spacing between adjacent anchors and proximity of anchors to edge of concrete. Install anchors in accordance with spacing and edge clearances indicated on the drawings.
6. Existing reinforcing bars in the concrete structure may conflict with specific anchor locations. Unless noted on the drawings that the bars can be cut, the contractor shall review the existing structural drawings and shall undertake to locate the position of the reinforcing bars at the locations of the concrete anchors by ferrocass, GPR, x-ray, or other means.
7. All hole conditions are assumed to be dry or water-saturated, unless noted otherwise. The contractor shall be responsible for removing water in order to avoid water-filled hole or submerged hole conditions before installing anchors.

X. CONCRETE FORMWORK

- A. RESPONSIBILITY
1. The design, construction, and safety of all formwork shall be the responsibility of the General Contractor. All forms, shores, backshores, falsework, bracing, and other temporary supports shall be engineered to support all loads imposed including the wet weight of concrete, construction equipment, live loads, lateral loads due to wind and wet concrete impalement. The Contractor shall also be responsible for determining when temporary supports, shores, backshores, and other bracing may be safely removed.
B. SUBMITTALS
1. The General Contractor shall submit for Owner's record only, formwork shop drawings. Formwork shop drawings shall include all items described in Paragraph A, including calculations. Formwork shop drawings shall be sealed by a registered Engineer in the state that the project is located.

XI. NON-SHRINK GROUT FOR BASE PLATES AND BEARING PLATES

- A. SPECIFICATIONS
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. Non-shrink grout shall conform to Corps of Engineers Specification for Non-Shrink Grout, CE-CRD-C621.
3. Twenty-eight day compressive strength shall be 5,000 psi, as determined by grout cube tests.
4. Minimum thickness of grout under all base plates and bearing plates shall be 1 inch.
B. TYPE
1. Grout for base plates and bearing plates shall be a non-metallic, shrinkage resistant, premixed, non-corrosive, non-staining product containing Portland cement, silica sands, shrinkage compensating agents, and fluidity improving compounds.
C. PLACEMENT
1. Grout shall be placed in a fluid, flowable state under base plates that have a form built around them for grout confinement.
2. Grout shall be cured according to manufacturer's recommendations.

XII. STRUCTURAL STEEL

- A. MATERIAL
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. Structural steel shall comply with the requirements of AISC 360 and AISC 303.
3. All hot rolled steel plates, shapes and bars shall be new steel conforming to ASTM Specification A6.
4. All Wide Flange W-Shapes shall conform to ASTM A992, Grade 50.
5. All Wide Flange M- and S-Shapes shall conform to ASTM A36.
6. All Wide Flange HP-Shapes shall conform to ASTM 572, Grade 50.
7. All Hollow Structural Shapes (HSS) shall conform to ASTM A500, Grade C.
8. All Pipe Sections shall conform to ASTM A53, Grade B.
9. All Channels, Angles and Plates shall conform to ASTM A36.
10. All girders shall conform to ASTM A588 Grade 50.
11. All nuts shall conform to ASTM A563.
12. All washers shall conform to ASTM F436.
13. Hilti HAS threaded rod per ICC ESR-3814 for diamond core holes or approved equal
14. All deformed bar anchors shall conform to ASTM A1064, Type C (Yield Strength = 70 ksi).

- B. CONNECTIONS
1. Typical connection details are indicated on the Drawings.
2. The design of all steel connections shall be performed under the direct supervision of a registered professional engineer in the state where the project is located. Engineering calculations shall be submitted, if requested.
3. It is the intention of the plans and specifications that shop connections be welded or bolted and that field connections be bolted, unless detailed otherwise on the Drawings.
4. All typical beam splice connections shall be standard double angle or single angle framed beam connections. Shear tab connections may be used at locations where double angle connections are not possible. Seated beam connections shall not be used unless indicated in the Drawings. Provide full depth shear tab if beam frames on only one side of a girder.
5. The beam connections shall be designed for the reactions listed below. Any connection capacities that do not meet or exceed the reactions listed below shall be clearly indicated on the beam piece marks and submitted for review as part of the shop drawing review process.
C. GENERAL REQUIREMENTS
1. Non-Connections shall be designed to support a reaction R (unless specified otherwise) equal to one-half the total uniform load capacity from the table of Uniform Load Constants in the AISC Manual.
2. Composite beams: Support a reaction R (unless specified otherwise) equal to a multiplier, defined below, times one-half the total uniform load capacity from the table of Max Total Uniform Load.

Table with 4 columns: Beam Size, Multiplier, Beam Size, Multiplier. Lists various beam sizes and their corresponding multipliers.

- 3. Add to the reaction listed above, any loads or reactions of members supported by the beam within three feet of beam end and the vertical components of forces in brace members framing into the beams.
6. Bracing connections shall develop full tensile forces at each end of the bracing member unless bracing forces are specified on the Drawings.
7. All zinc coating for steel shapes and plates shall average not less than 2.3 oz. with no individual thickness less than 2.0 oz.
8. Galvanize all nuts, bolts, and washers used in the connection of galvanneal steel.
9. Protect all field welded connections with "Z.R.C. Cold Galvanizing Compound" as manufactured by Z.R.C. Product Company or approved equal.
D. STAIRS, HANDRAILS, AND ACCESS LADDERS
1. Stairs, handrails, guardrails and access ladders (including attachment to structure) shall be designed by the fabricator for the loads indicated in IBC Chapter 16, OSHA, and other requirements noted in the architectural drawings. For access ladders not supported by concrete walls, all landings and steel columns shall be included in the fabricator's scope of work. Shop drawings shall be sealed by a Professional Engineer, representing the fabricator, in the state where the project is located.

E. ALUMINUM CANOPIES

- 1. Steel elements supporting aluminum canopies (designed by others) have been designed based on preliminary assumptions of canopy reactions. The canopy structure, and connection of canopy to the steel, shall be designed by the canopy supplier's engineer. GC to provide structural engineer with final canopy reactions per the fabricating schedule, to confirm final structural design is sufficient for canopy support.

XIII. STRUCTURAL BOLTS AND THREADED FASTENERS

- A. SPECIFICATION
1. All bolts shall conform to ASTM F3125, Grade A325, Type 1, unless noted otherwise in the drawings.
2. All nuts shall conform to ASTM F436.
3. All washers shall conform to ASTM F436.
B. DESIGN
1. Minimum bolt diameter shall be 3/4 inch.
2. Connection Type: Unless noted otherwise on the Drawings or in these General Notes, all connections shall be bearing type connections using standard holes (hole diameter nominally 1/16 inch in excess of nominal bolt diameter with threads included in the shear planes).
C. INSTALLATION
1. Fastener Tension: High strength bearing bolts shall be tightened using an impact wrench to a snug tight condition. The snug tight condition is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench.
2. All bolts shall be new and shall not be reused.

XIV. WELDING OF STRUCTURAL STEEL

- A. WELDER CERTIFICATION
1. All shop and field welders shall be certified according to AWS procedures for the welding process and welding position used.
B. MINIMUM SIZE AND STRENGTH
1. Fillet Welds: Minimum size of fillet welds shall be as specified in the AISC Manual.
2. Partial Penetration Groove Welds: The minimum effective throat thickness of partial penetration groove welds shall be as specified in the AISC Manual.
3. Minimum Strength of Welded Connections: Unless noted otherwise on the drawings, all shop and field welds shall develop the full tensile strength of the member or elements joined.
a. All connections designated in the documents as moment connections shall be designed to develop the full flexural capacity of the member that has the lesser flexural capacity of the members at the connection, unless noted otherwise in the Drawings. In cases where multiple beams are moment connected to a single column, each beam-to-column interface must be designed for the full flexural capacity of the member that has the lesser flexural capacity, unless noted otherwise in the Drawings.
4. Corners of all miscellaneous steel shall consist of 1/4" fillet welds all-around (minimum) if no other connection information is provided on the structural drawings.
5. All slotted corners, and anywhere a gap may exist between base metal and connecting material, weld size shall be increased to account for gap width (per AWS recommendations).

C. FILLER METAL REQUIREMENTS

- 1. Strength: Weld shall be as specified in the AISC Manual.
2. Electrodes: Electrodes for various welding processes shall be as specified below.
a. SMAW: E70XX low hydrogen
b. SAW: F7X-EXXX

D. WELDING

- 1. All welding shall comply with the requirements of AWS.
2. All full penetration welds shall be tested to verify compliance, unless noted otherwise.
3. All fillet welds shall be visually inspected, unless noted otherwise.

XV. OPEN WEB STEEL JOISTS

- A. SPECIFICATIONS
1. Open web steel joists and joist girders shall be designed, manufactured and erected per the manufacturer's specifications (SJI) Specifications.
2. Joists and joist girders shall be cambered for dead load. Provide standard SJI camber unless specified otherwise on the Drawings.
3. Provide chord extensions where shown on the Drawings.
4. Provide flat bearing for all joists and joist girders. Provide minimum end bearing of 2 1/2 inches for K-series joists and 4 inches for LH and DLH joists. If sufficient bearing does not exist, stagger the end of each joist and center bearing over center of support.
5. Provide horizontal and diagonal bracing as required by SJI Standards and Specifications.
6. Provide standard SJI bearing depth for all joists and joist girders. When K-series joists bear on the same beam or joist girder as LH- or DLH-series joists, seat depth shall be increased to match the LH- or DLH-series joist.
7. Top chord of joists shall be double angles or tees.
8. Provide minimum end anchorage to steel and masonry, as required by SJI Standards and Specifications.
9. Joists and joist girders shall be shop painted per Architectural specifications.
10. End connections for joists and joist girders with bottom chord extensions shall only be completed after all full joist load is applied.

- B. DESIGN
1. The design of all open web joists shall conform to the codes and specifications cited above.
2. All joists shall carry the design loads as specified in the SJI load tables as the minimum requirements. Additionally, joists shall be designed to carry any other load types and patterns as indicated on the structural drawings. Examine the structural drawings for any other loads required to be carried by any joist.
3. End joist shall be designed for a net service level uplift pressure = 30 PSF, unless noted otherwise in the wind pressure diagrams.
4. Increase size as required to conform with Factory Mutual and U.L. requirements.
5. Design shall include slope greater than 1:4 inch per foot, the joist seat shall be designed for a 3,000 lb longitudinal point load parallel to the joist applied at the joist bearing elevation (unless noted otherwise in roof framing details).
C. SUBMITTAL
1. Joist manufacturer shall submit the following for Engineer's review and approval:
a. Produce certification letter that joists and joist girders comply with all documents noted in these general notes.
b. Shop drawings showing joist layout, bridging and other accessories.
c. Design calculations for H/VAC units suspended from or supported on the joists. Design shall be signed and sealed by a registered Engineer. Calculations are for Owner's record only.
D. OSHA REQUIREMENTS
1. Comply with all OSHA requirements. Provide additional steel, bridging, etc. as required. Provide vertical plates shop welded to structural steel columns and/or beams and bottom chord extensions at joists at columns, as required by OSHA.

XVI. STEEL ROOF DECK (WITH RIGID INSULATION BOARD)

- A. SPECIFICATIONS
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. The design, fabrication, and erection of all roof deck shall conform to the Steel Deck Institute (SDI) Standards.
3. Galvanized steel deck shall conform to ASTM A653 Structural Quality, with a minimum yield strength of 80 ksi. Steel deck shall be galvanized with a protective zinc coating conforming to ASTM A653 G90 class.
B. ROOF DECK SCHEDULE
1. All roof deck shall be 22 gauge, unless noted otherwise on plan, as manufactured by Vulcraft, or roof deck with equivalent section properties.
2. Roof deck shall be Toris 5.5A (20 gauge) as manufactured by Epic Metals, or roof deck with equivalent section properties.
C. ATTACHMENT
1. Roof deck shall be attached to structural supports as follows unless noted otherwise on plan:
For use with 1.5B deck supported by joists and WF beams
Attachment Pattern Hilti XASN 24
Sideclip fastener Hilti SLC
Sidelap spacing 12" o.c.
Perimeter spacing 6" o.c.
For use with Toris 5.5A deck supported by joists and WF beams
Attachment Type 3/4" puddle
Attachment Pattern 24"
Sideclip fastener #10 TEK screws
Sidelap spacing 36" o.c.
Perimeter spacing 36" o.c.
a. Welds shall penetrate all layers of deck material at end laps and side joints and shall have good fusion to the supporting members.
b. Welding supports shall be used when framing steel deck less than 0.028 inches thick.
2. Deck supported on cold-formed light gage framing (C-Joists or trusses) shall be attached to each support member using #12 TEK steel screws at all ribs where sides lap and each rib in between.
3. Provide a minimum end bearing of 2 inches over supports.
4. End laps of sheets shall be a minimum of two inches and shall occur over supports.
5. Provide roof deck construction which is listed and conforms to the UL Fire Resistance Directory and Building Materials Directory. Deck attachment shall be increased above the minimum specified herein as required to achieve a Class 90 uplift rating.

D. DECK SPANS

- 1. Deck spans shall not exceed the maximum center to center spans as required by SDI criteria. Where possible, steel deck shall extend over three or more supports. Two span deck shall be used only where deck layout does not permit the use of three spans. Single span deck is not permitted.

E. ACCESSORIES

- 1. Use the following accessories as required to provide a finished surface for the application of insulation and roofing:
a. Ridge, hip and valley plates (14 inch)
b. Cant strips (20 gauge)
c. Sump pans (14 gauge)
d. Inside or outside closure channels (20 gauge)
e. Butt strips at change of deck directions (20 gauge)
f. Filter sheets (20 gauge)
g. Rubber closures

F. ROOF OPENINGS

- 1. Roof openings less than 6 inch square or diameter require no reinforcement. Openings 6 inch to 10 inch shall be reinforced with a 20 gauge galvanized plate welded to the deck at each corner and 6 inch maximum centers with a 5/8 inch diameter puddle weld or steel sheet stress.
2. Unless indicated otherwise on the drawings, openings over 10 inch wide or diameter shall be reinforced with an angle L 2 1/2 x 2 1/2 x 1/4 framing each side of the opening and spanning between supports for spans 4 feet or less and L 3 x 3 x 1/4 for spans greater than 4 feet but less than 6 feet unless specified otherwise on drawings.
3. Larger openings shall be referred to the Engineer for framing.

G. SUBMITTALS

- 1. The steel roof deck manufacturer shall submit the following:
a. Product specifications and installation instructions for each type of deck.
b. A certification of product compliance with SDI Standards and these General Notes.
c. Shop drawings showing complete deck layout, attachment details, accessories, and all other details for Engineer review and approval prior to the start of erection.
H. PAINTING
1. Bottom of galvanized roof deck shall be shop painted white per Architects requirements after fabrication.



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KEY PLAN:

REVISION SCHEDULE

Table with 3 columns: NO., DATE, DESCRIPTION. Lists revision dates and descriptions.

LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO.: 24.154

GENERAL NOTES

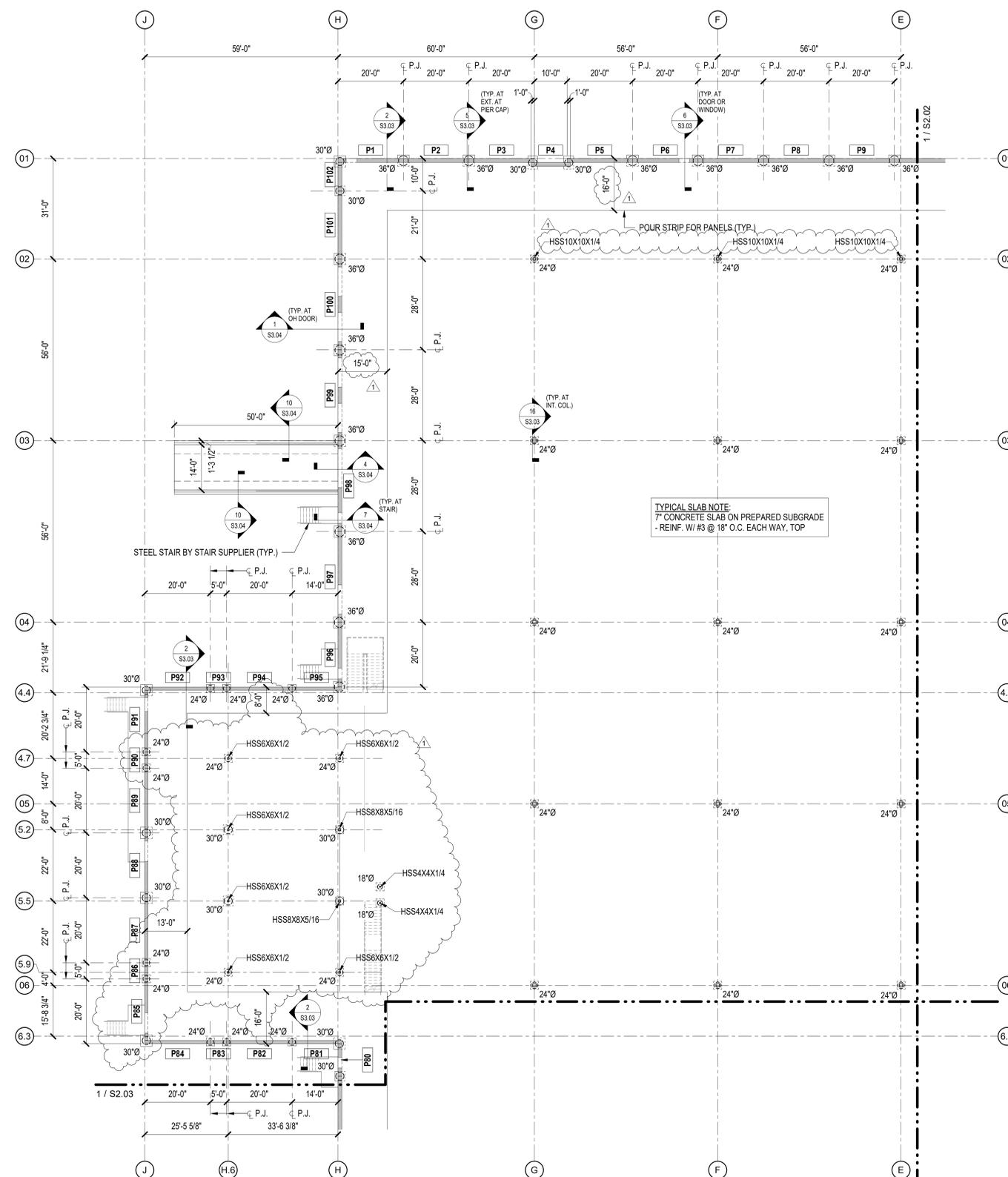
S0.01

GENERAL NOTES

1. ALL STRAIGHT SHAFTS UNDER TILT-UP PANELS SHALL BE CENTERED UNDER PANELS AND CENTERED ON PANEL JOINTS, U.N.O.
2. STRAIGHT SHAFTS NOTED THUS: 24"Ø INDICATE THE A 24" DIAMETER STRAIGHT SHAFT.
3. STRAIGHT SHAFTS UNDER GRADE BEAMS (WITHOUT COLUMNS) SHALL BE CENTERED UNDER GRADE BEAMS, U.N.O.
4. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
5. ALL COLUMNS ARE HSS12X12X1/4 TYP., U.N.O.
6. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.
7. SEE DETAIL 8/S3.02 FOR HSS BASE PLATES.

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KEY PLAN:



1 PARTIAL FOUNDATION PLAN
1/16" = 1'-0"



REVISION SCHEDULE

NO.	DATE	DESCRIPTION
12/15/25	50% DD	
01/13/26	100% DD	
02/27/26	ISSUE FOR BID	
1 03/16/26	ADDENDUM 1	

HIGHLAND RD

PROJECT ADDRESS

RMA PROJECT NO: 251172

PARTIAL FOUNDATION PLAN - AREA A

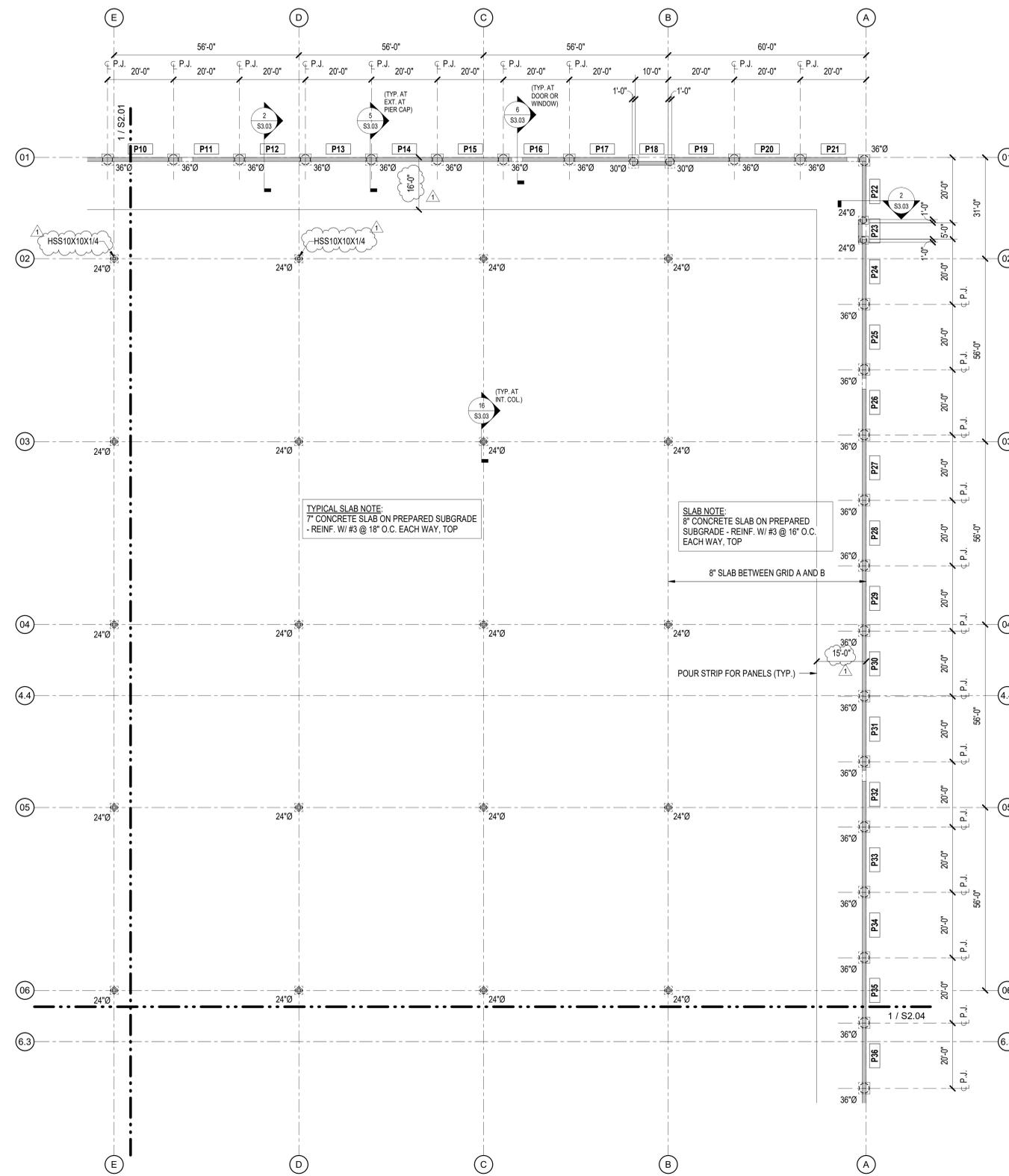
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5. ALL COLUMNS ARE HSS12X12X1/4 TYP., U.N.O.
6. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.
7. SEE DETAIL 8/S3.02 FOR HSS BASE PLATES.



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KEY PLAN:



1 PARTIAL FOUNDATION PLAN
1/16" = 1'-0"

HIGHLAND RD

PROJECT ADDRESS

RMA PROJECT NO: 251172

PARTIAL
FOUNDATION PLAN
- AREA B

S2.02

GENERAL NOTES

1. ALL STRAIGHT SHAFTS UNDER TILT-UP PANELS SHALL BE CENTERED UNDER PANELS AND CENTERED ON PANEL JOINTS. U.N.O.
2. STRAIGHT SHAFTS NOTED THUS: 24"Ø INDICATE THE A 24" DIAMETER STRAIGHT SHAFT.
3. STRAIGHT SHAFTS UNDER GRADE BEAMS (WITHOUT COLUMNS) SHALL BE CENTERED UNDER GRADE BEAMS, U.N.O.
4. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
5. ALL COLUMNS ARE HSS12X12X1/4 TYP., U.N.O.
6. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.
7. SEE DETAIL 8/S3.02 FOR HSS BASE PLATES.

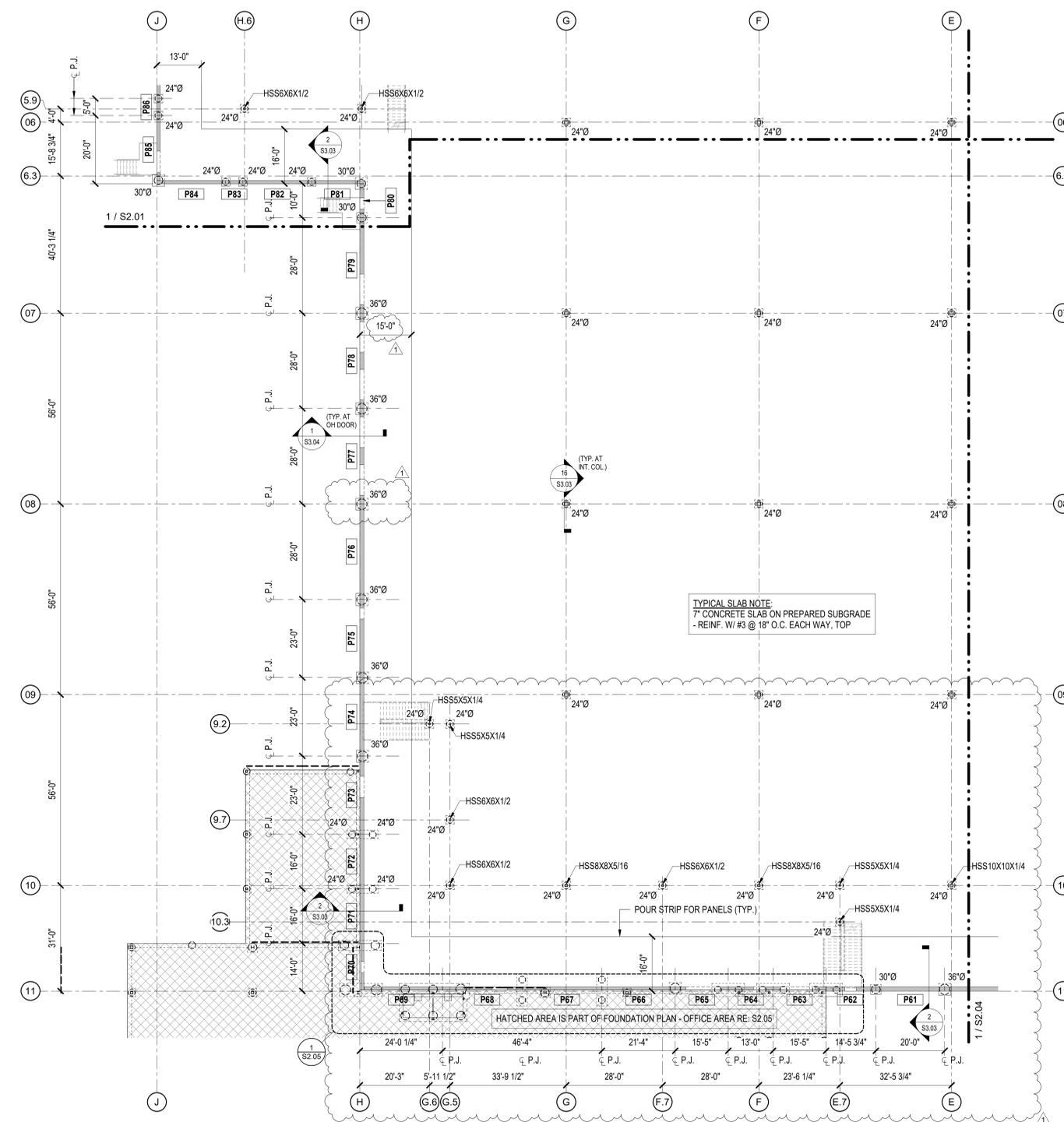
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1	03/16/26	ADDENDUM 1

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RMA PROJECT NO: 251172

PARTIAL
FOUNDATION PLAN
- AREA C

S2.03

1 PARTIAL FOUNDATION PLAN
1/16" = 1'-0"

GENERAL NOTES

1. ALL STRAIGHT SHAFTS UNDER TILT-UP PANELS SHALL BE CENTERED UNDER PANELS AND CENTERED ON PANEL JOINTS, U.N.O.
2. STRAIGHT SHAFTS NOTED THUS: 24"Ø INDICATE THE A 24" DIAMETER STRAIGHT SHAFT.
3. STRAIGHT SHAFTS UNDER GRADE BEAMS (WITHOUT COLUMNS) SHALL BE CENTERED UNDER GRADE BEAMS, U.N.O.
4. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
5. ALL COLUMNS ARE HSS12X12X1/4 TYP., U.N.O.
6. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.
7. SEE DETAIL 8/S3.02 FOR HSS BASE PLATES.

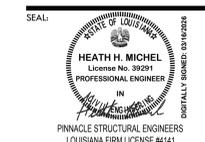
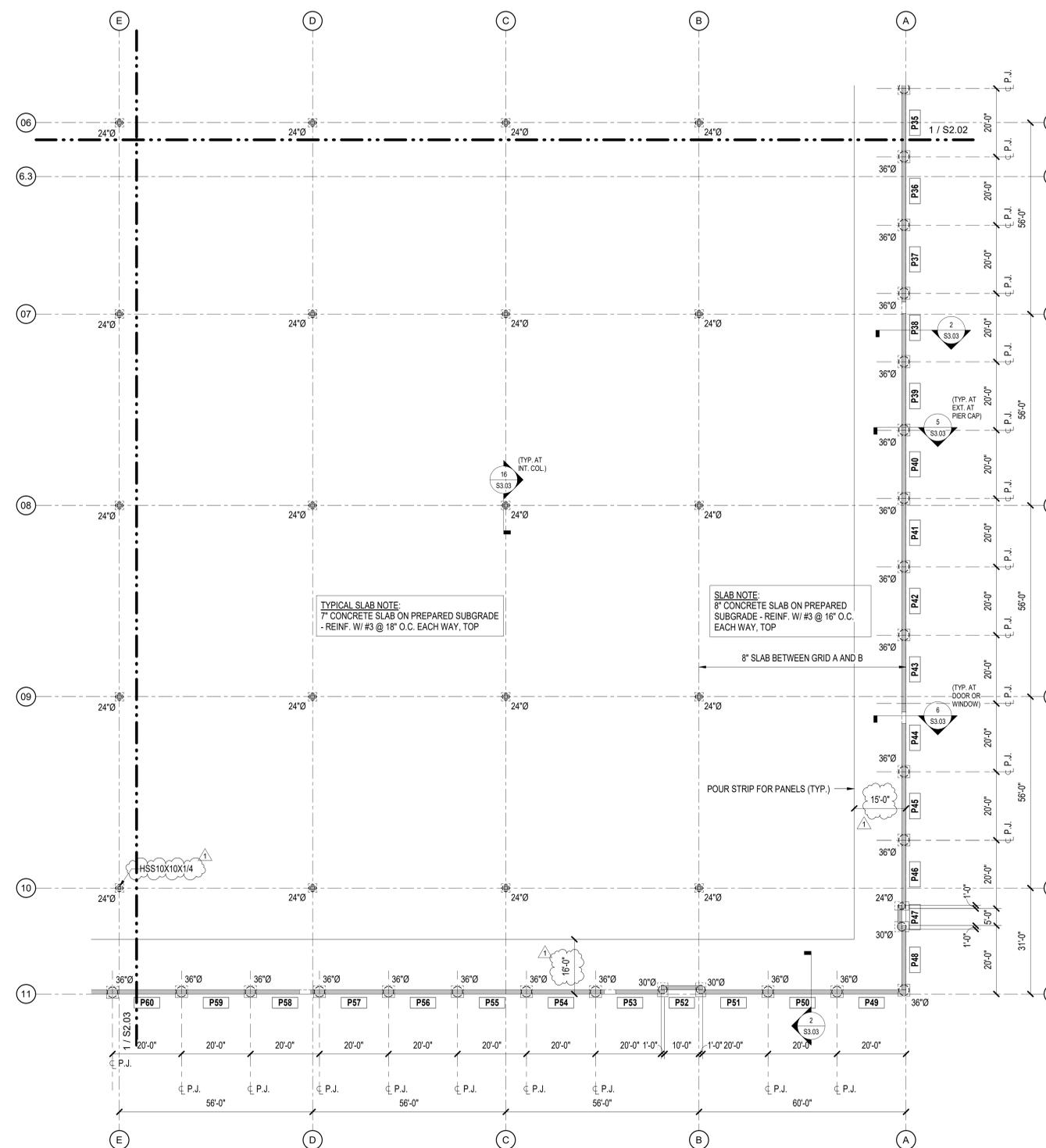
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1	03/16/26	ADDENDUM 1

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PROJECT ADDRESS

RMA PROJECT NO: 251172

PARTIAL FOUNDATION PLAN - AREA D

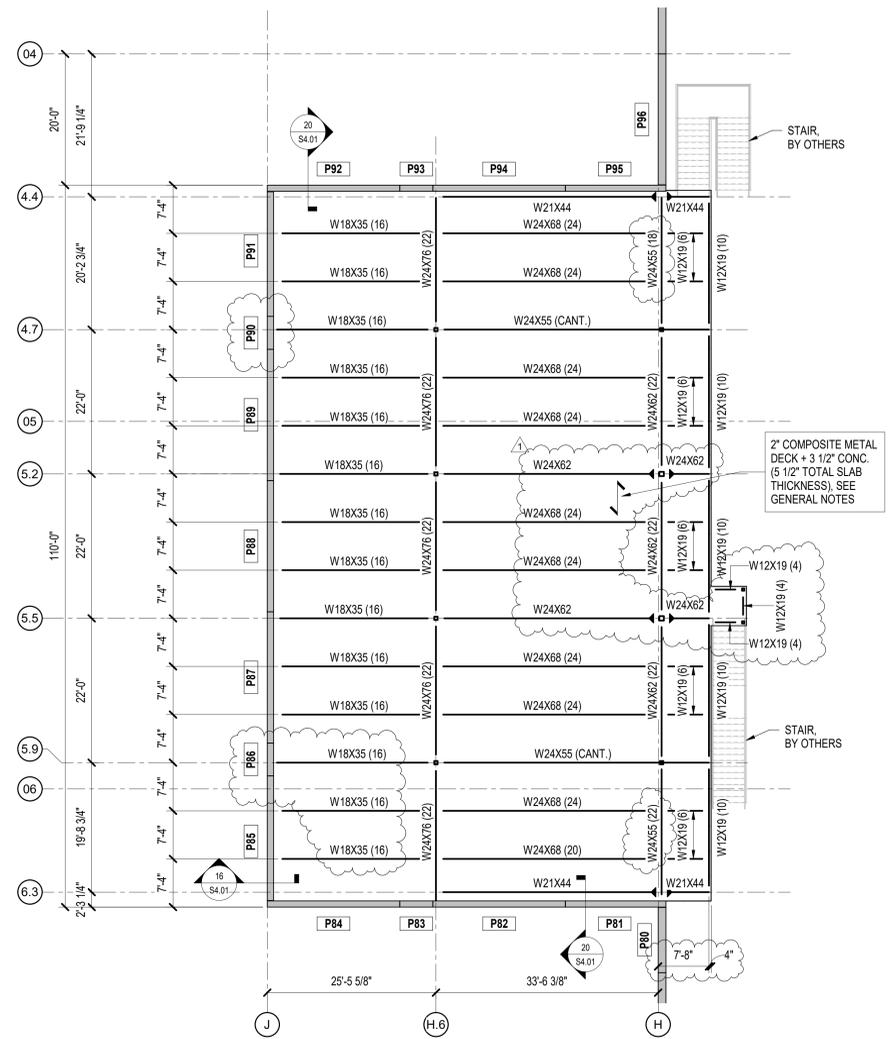
S2.04

1 PARTIAL FOUNDATION PLAN
1/16" = 1'-0"

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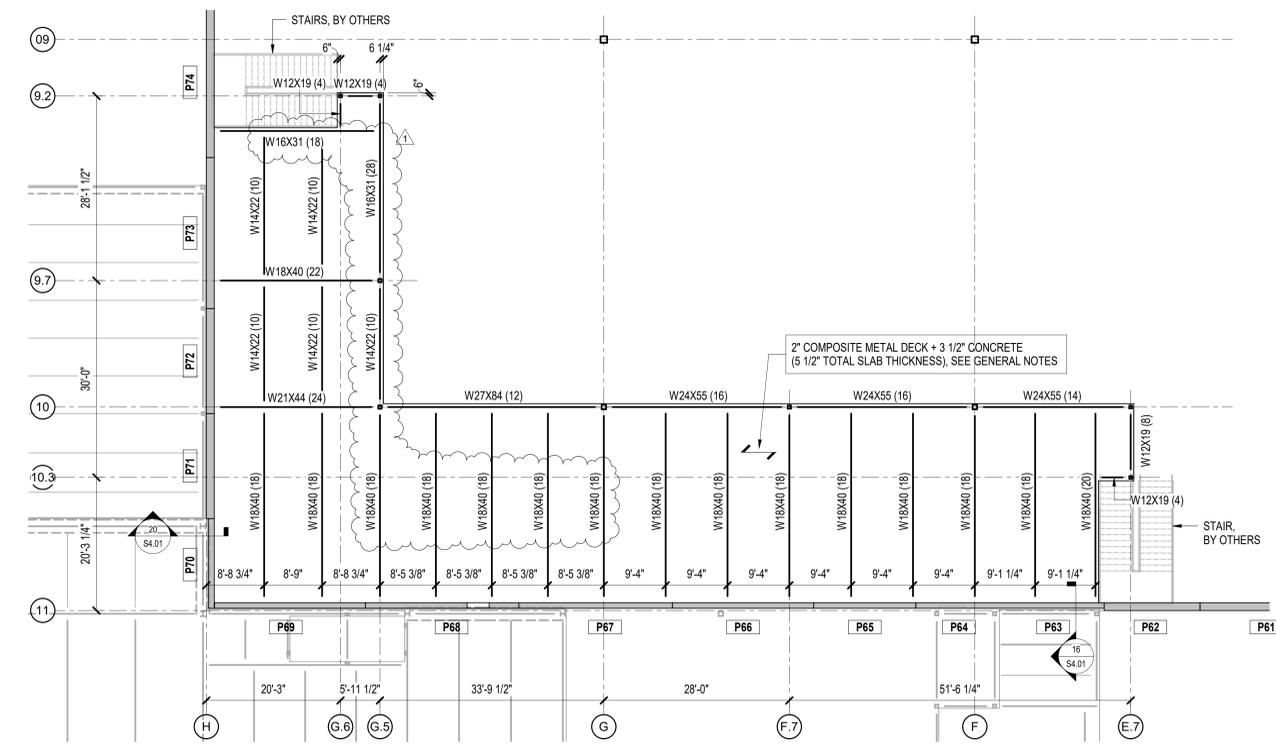
KEY PLAN:

1. FINISHED FLOOR ELEVATION = 14'-0" ABOVE FINISHED FIRST FLOOR.
2. LOCATIONS SHOWN THUS: W16X31 (12) C = 3/4" INDICATES A W16X31 WITH THE NUMBER OF SHEAR CONNECTORS AS 12 AND UPWARD CAMBER AT MID SPAN OF BEAM AS 3/4".
3. LOCATIONS NOTED THUS: [PX] INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
4. SEE SHEET S2.01 FOR COLUMN SIZES.



2 FRAMING PLAN - WEST MEZZANINE
3/32" = 1'-0"

1. FINISHED FLOOR ELEVATION = 18'-0" ABOVE FINISHED FIRST FLOOR.
2. LOCATIONS SHOWN THUS: W16X31 (12) C = 3/4" INDICATES A W16X31 WITH THE NUMBER OF SHEAR CONNECTORS AS 12 AND UPWARD CAMBER AT MID SPAN OF BEAM AS 3/4".
3. LOCATIONS NOTED THUS: [PX] INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
4. SEE SHEET S2.03 FOR COLUMN SIZES.



1 SOUTH MEZZANINE
3/32" = 1'-0"



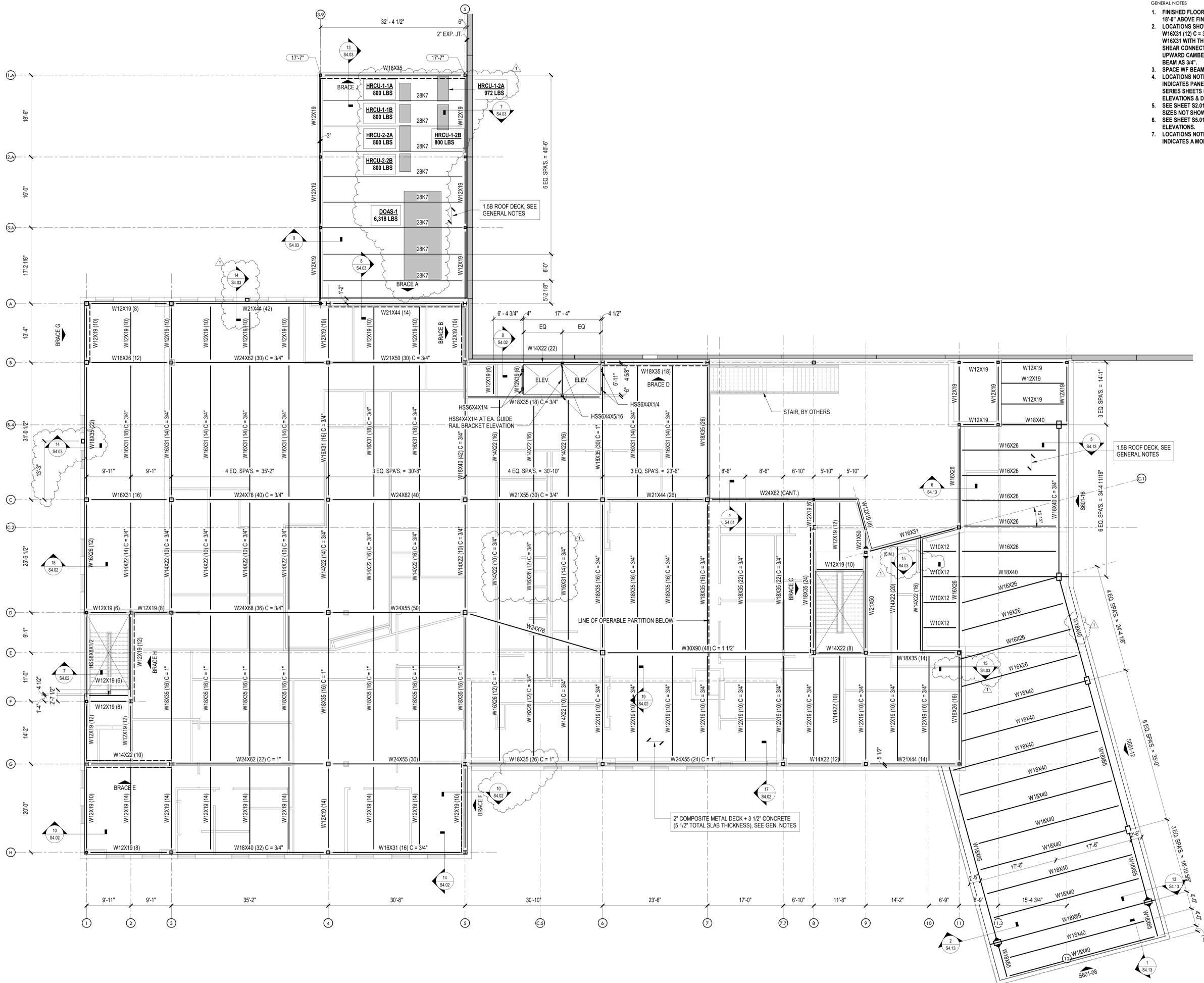
NO.	DATE	DESCRIPTION
01	13/26	100% DD
02	27/26	ISSUE FOR BID
1	03/16/26	ADDENDUM 1

HIGHLAND RD

PROJECT ADDRESS

RMA PROJECT NO: 251172

MEZZANINE FOUR
FRAMING PLANS



- GENERAL NOTES
1. FINISHED FLOOR ELEVATIONS = 18'-0" ABOVE FINISH FIRST FLOOR.
 2. LOCATIONS SHOWN THUS: W16X31 (12) C = 3/4" INDICATES A W16X31 WITH THE NUMBER OF SHEAR CONNECTORS AS 12 AND UPWARD CAMBER AT MID-SPAN OF BEAM AS 3/4".
 3. SPACE WF BEAMS EQUALLY, U.N.O.
 4. INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
 5. SEE SHEET S2.01 FOR ALL COLUMNS SIZES NOT SHOWN.
 6. SEE SHEET S5.01 FOR BRACE ELEVATIONS.
 7. LOCATIONS NOTED THUS: INDICATES A MOMENT CONNECTION.

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KEY PLAN:

SEAL

HEATH H. MICHEL
 License No. 39291
 PROFESSIONAL ENGINEER
 IN
 STRUCTURAL ENGINEERING
 PINNACLE STRUCTURAL ENGINEERS
 LOUISIANA FIRM LICENSE #4141

NO.	DATE	DESCRIPTION
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1 03/16/26	ADDENDUM 1	

LIPSEY'S NEW HEADQUARTERS
 HIGHLAND RD, ST. GEORGE, LA
 RMA PROJECT NO.: 24.154

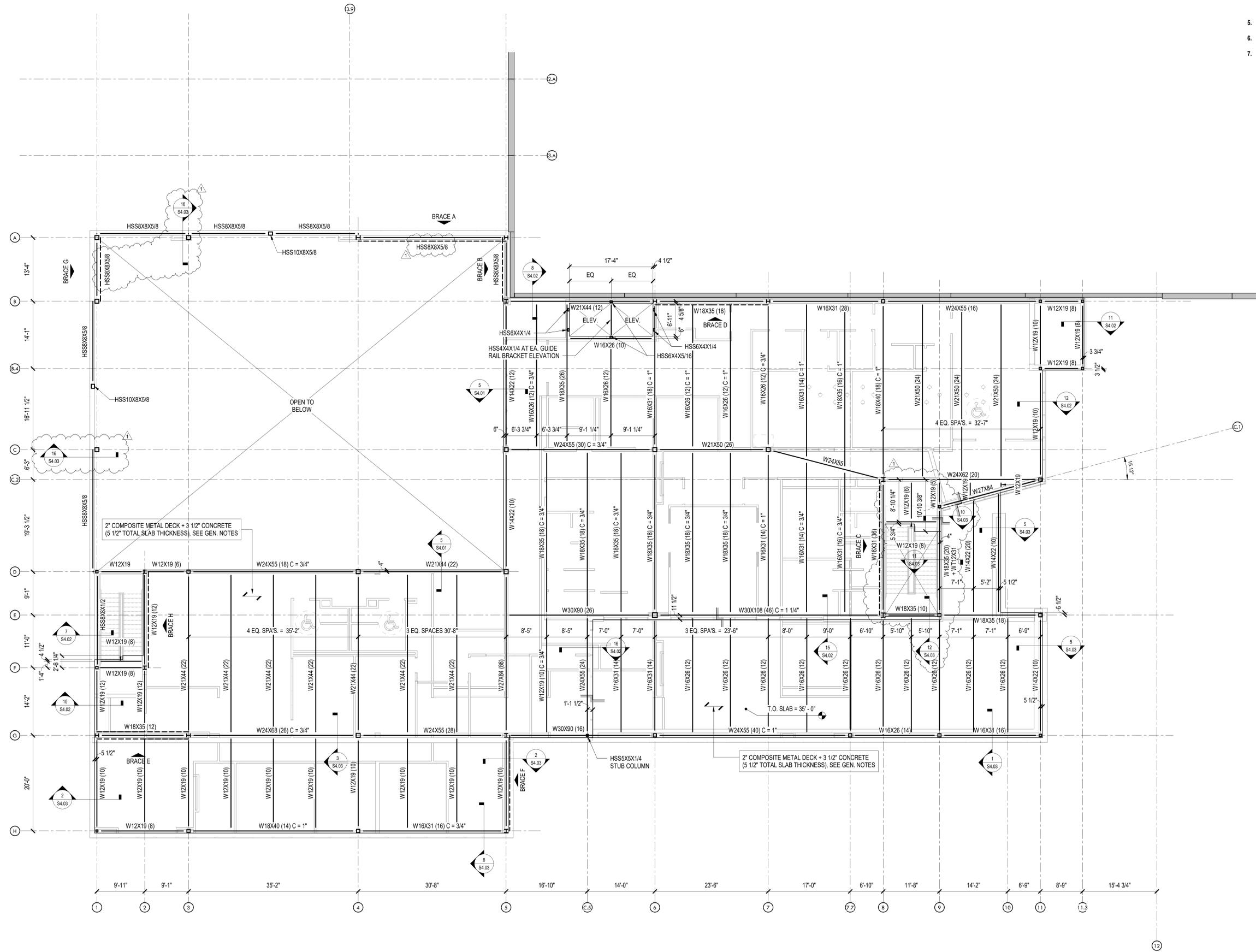
SECOND FLOOR FRAMING PLAN - OFFICE

1 SECOND FLOOR FRAMING PLAN - OFFICE
 1/8" = 1'-0"

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KEY PLAN:

- GENERAL NOTES
1. FINISHED FLOOR ELEVATIONS = 35'-0" ABOVE FINISH FIRST FLOOR. LOCATIONS SHOWN THUS: [Symbol]
 2. W16X31 (12) C = 3/4" INDICATES A W16X31 WITH THE NUMBER OF SHEAR CONNECTORS AS 12 AND UPWARD CAMBER AT MID-SPAN OF BEAM AS 3/4".
 3. SPACE WF BEAMS EQUALLY, U.N.O.
 4. LOCATIONS NOTED THUS: [Symbol] INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
 5. SEE SHEET S2.01 FOR ALL COLUMNS SIZES NOT SHOWN.
 6. SEE SHEET S5.01 FOR BRACE ELEVATIONS.
 7. LOCATIONS NOTED THUS: [Symbol] INDICATES A MOMENT CONNECTION.



1 THIRD FLOOR FRAMING PLAN
1/8" = 1'-0"



REVISION SCHEDULE

NO.	DATE	DESCRIPTION
12/15/25	50% DD	
01/14/26	100% DD	
02/27/26	ISSUE FOR BID	
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LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO.: 24.154

THIRD FLOOR FRAMING PLAN - OFFICE

GENERAL NOTES

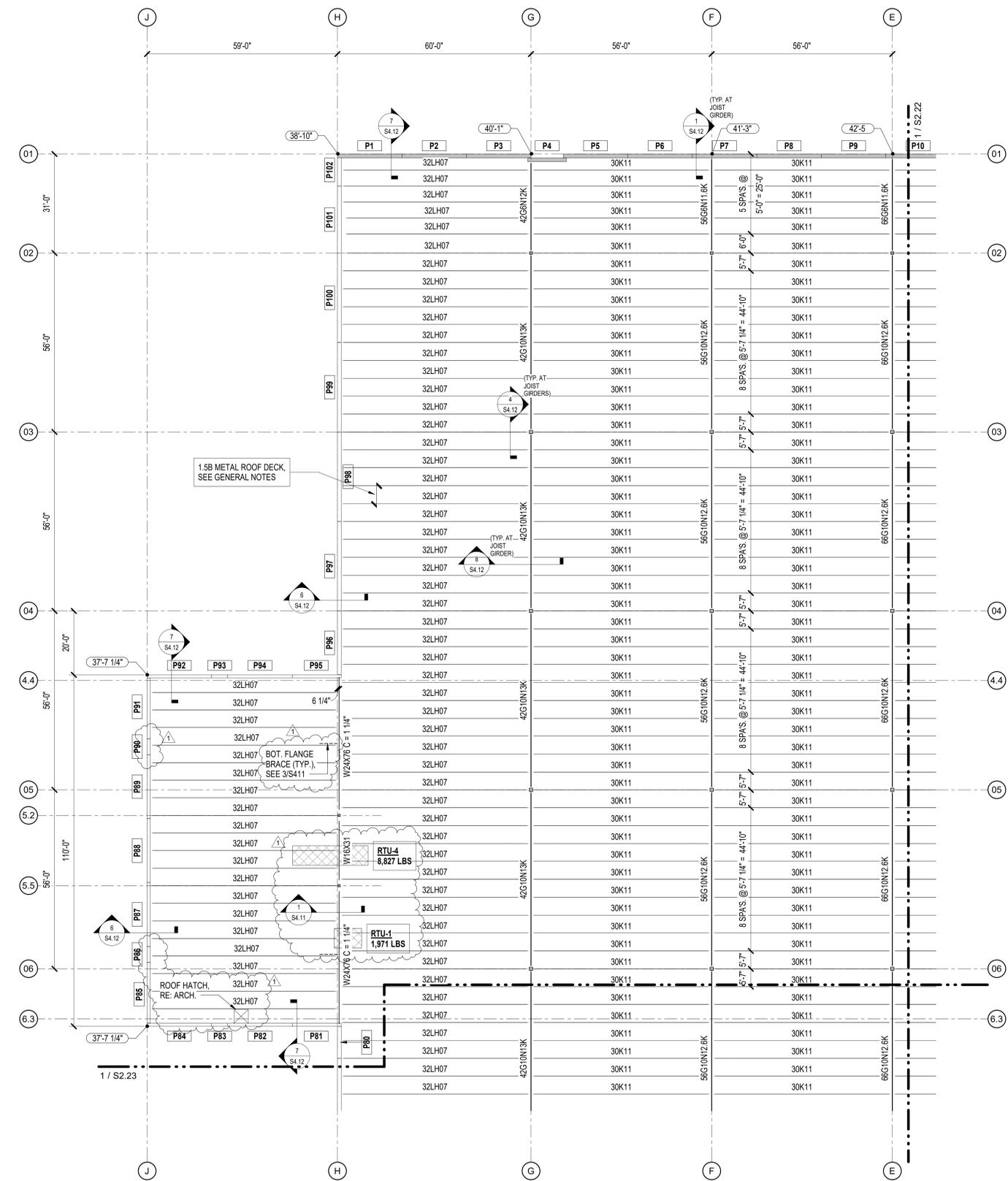
- LOCATIONS SHOWN THUS: **XX-XX** INDICATES TOP OF STEEL (T.O.S.) ELEVATION (TOP OF STEEL = TOP OF JOIST OR BOTTOM OF DECK).
- JOISTS SHALL BE DESIGNED FOR ADDITIONAL LOADS RESULTING FROM RTU'S AND OTHER MECHANICAL EQUIPMENT ON THE ROOF OR SUSPENDED FROM THE ROOF STRUCTURE. SEE ARCHT'L. AND MEP DRAWINGS FOR LOCATIONS.
- SPACE JOISTS EQUALLY (6'-0" O.C. MAX.)
- REFER TO ARCHT'L. DRAWINGS / MEP FOR LOCATION OF ROOF DRAINS, OVERFLOW DRAINS & TAPERED INSULATION.
- LOCATIONS NOTED THUS: **PX** INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
- SEE SHEET S2.01-S2.04 FOR COLUMN SIZES.
- JOISTS SHALL BE DESIGNED FOR ALL POINT AND LINE LOADS SHOWN ON THE ROOF PLANS WITH AN "L" = "DESIGNATION AS PART OF THE ROOF LIVE LOAD CASE."

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KEY PLANS



1 ENLARGED ROOF FRAMING PLAN
 1/16" = 1'-0"

SEAL

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 PROJECT ADDRESS

RMA PROJECT NO: 251172

PARTIAL ROOF FRAMING PLAN - AREA A

GENERAL NOTES

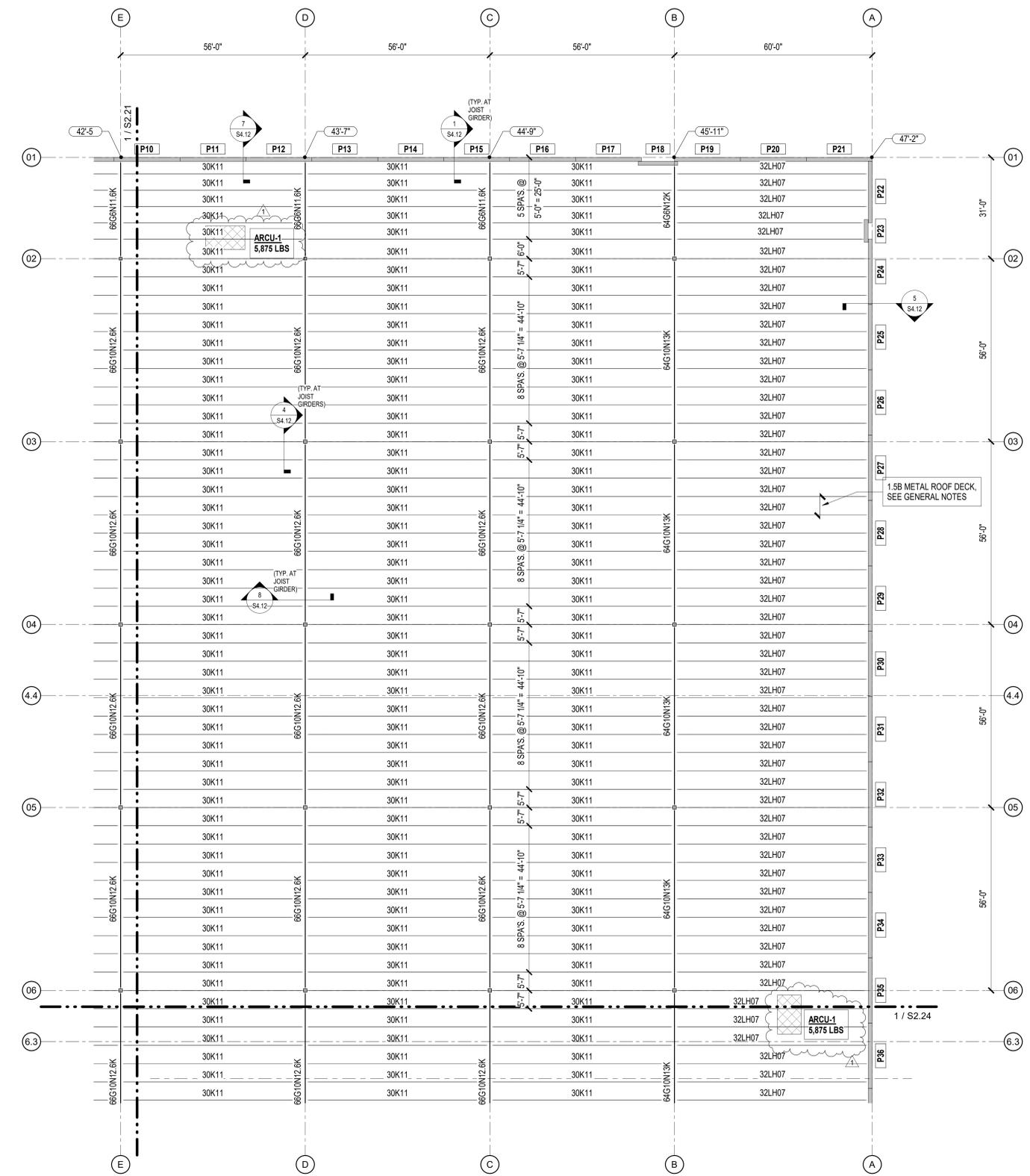
- LOCATIONS SHOWN THUS: (XX-XX) INDICATES TOP OF STEEL (T.O.S.) ELEVATION (TOP OF STEEL = TOP OF JOIST OR BOTTOM OF DECK).
- JOISTS SHALL BE DESIGNED FOR ADDITIONAL LOADS RESULTING FROM RTU'S AND OTHER MECHANICAL EQUIPMENT ON THE ROOF OR SUSPENDED FROM THE ROOF STRUCTURE. SEE ARCHT'L. AND MEP DRAWINGS FOR LOCATIONS.
- SPACE JOISTS EQUALLY (6'-0" O.C. MAX.)
- REFER TO ARCHT'L. DRAWINGS / MEP FOR LOCATION OF ROOF DRAINS, OVERFLOW DRAINS & TAPERED INSULATION.
- LOCATIONS NOTED THUS: [PX] INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
- SEE SHEET S2.01-S2.04 FOR COLUMN SIZES.
- JOISTS SHALL BE DESIGNED FOR ALL POINT AND LINE LOADS SHOWN ON THE ROOF PLANS WITH AN "L" = " DESIGNATION AS PART OF THE ROOF LIVE LOAD CASE.

BITTER MAHER ARCHITECTS
 2987 Government Street (Baton Rouge, LA 70806)
 p 225.383.4321
 BITTERMAHER.COM

Pinnacle STRUCTURAL ENGINEERS
 3120 Southwest Freeway, Suite 410
 Houston, TX 77098
 713.807.8811 voice
 25105

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KEY PLANS:



1 ENLARGED ROOF FRAMING PLAN
 1/16" = 1'-0"

SEAL

STATE OF LOUISIANA
HEATH H. MICHEL
 License No. 39281
 PROFESSIONAL ENGINEER
 IN
 CIVIL ENGINEERING
 PINNACLE STRUCTURAL ENGINEERS
 LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
12/15/25	50% DD	
01/13/26	100% DD	
02/27/26	ISSUE FOR BID	
1	03/16/26	ADDENDUM 1

HIGHLAND RD
 PROJECT ADDRESS

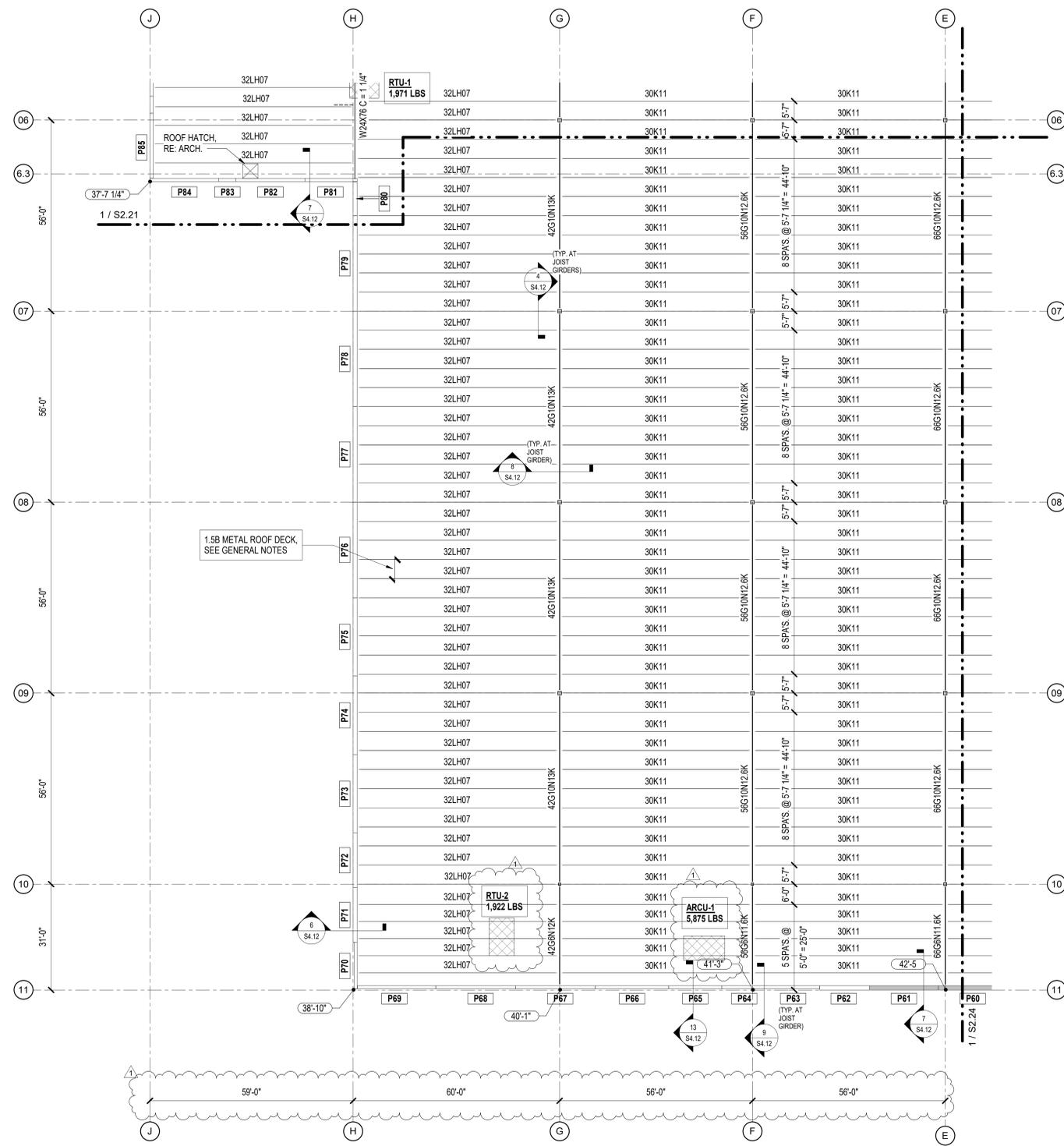
RMA PROJECT NO.: 251172

PARTIAL ROOF FRAMING PLAN - AREA B

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KEY PLANS:

- GENERAL NOTES
- LOCATIONS SHOWN THUS: **XX-XX** INDICATES TOP OF STEEL (T.O.S.) ELEVATION (TOP OF STEEL = TOP OF JOIST OR BOTTOM OF DECK).
 - JOISTS SHALL BE DESIGNED FOR ADDITIONAL LOADS RESULTING FROM RTU'S AND OTHER MECHANICAL EQUIPMENT ON THE ROOF OR SUSPENDED FROM THE ROOF STRUCTURE. SEE ARCHT'L. AND MEP DRAWINGS FOR LOCATIONS.
 - SPACE JOISTS EQUALLY (6'-0" O.C. MAX.)
 - REFER TO ARCHT'L. DRAWINGS / MEP FOR LOCATION OF ROOF DRAINS, OVERFLOW DRAINS & TAPERED INSULATION.
 - LOCATIONS NOTED THUS: **PX** INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
 - SEE SHEET S2.01-S2.04 FOR COLUMN SIZES.
 - JOISTS SHALL BE DESIGNED FOR ALL POINT AND LINE LOADS SHOWN ON THE ROOF PLANS WITH AN "L" = "DESIGNATION AS PART OF THE ROOF LIVE LOAD CASE."



1 ENLARGED ROOF FRAMING PLAN
1/16" = 1'-0"



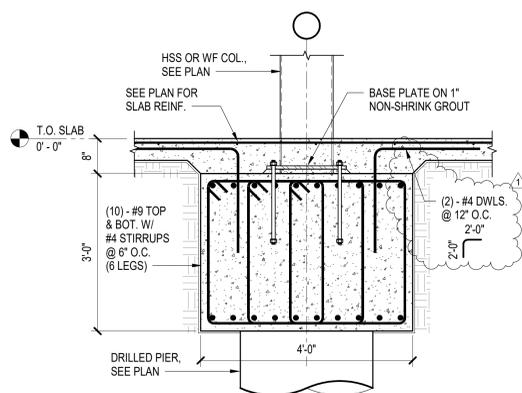
REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
12/15/25	50% DD	
01/13/26	100% DD	
02/27/26	ISSUE FOR BID	
1	03/16/26	ADDENDUM 1

HIGHLAND RD

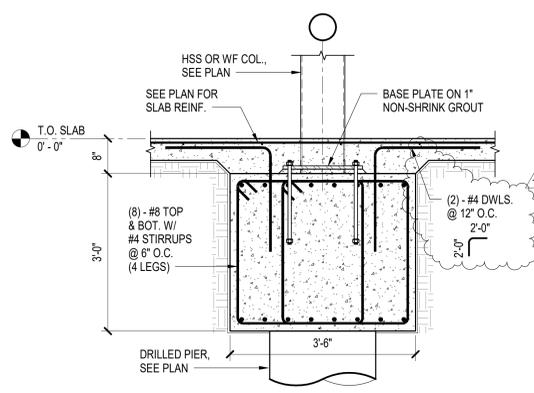
PROJECT ADDRESS

RMA PROJECT NO: 251172

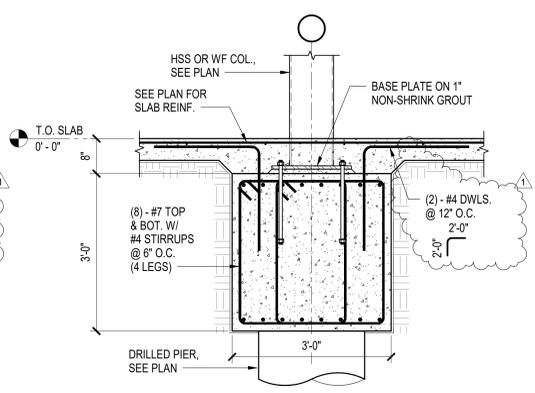
PARTIAL ROOF
FRAMING PLAN -
AREA C



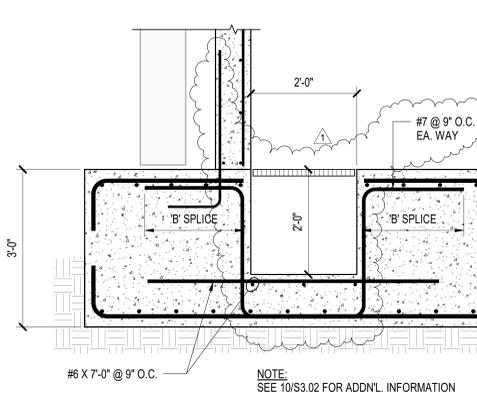
20 DETAIL AT INTERIOR GRADE BEAM AND COLUMN
3/4" = 1'-0"



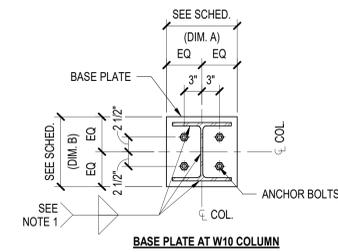
16 DETAIL AT INTERIOR GRADE BEAM AND COLUMN
3/4" = 1'-0"



12 DETAIL AT INTERIOR GRADE BEAM AND COLUMN
3/4" = 1'-0"



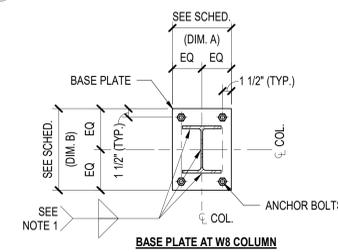
8 DETAIL AT ELEVATOR SUMP PIT
3/4" = 1'-0"



COLUMN	BASE PLATE A X B X THICKNESS	ANCHOR BOLTS
W 10 X 49	12" X 12" X 3/4"	(4) - 3/4"Ø
W 10 X 54	14" X 12" X 3/4"	(4) - 3/4"Ø
W 10 X 68	14" X 12" X 3/4"	(4) - 3/4"Ø

- NOTES:
- MINIMUM FILLET WELD ACCORDING TO A.I.S.C. (1/4" MIN.)
 - USE OVERSIZED HOLES FOR ANCHOR BOLTS ACCORDING TO A.I.S.C.
 - WASHERS SHALL BE INSTALLED OVER OVERSIZED HOLES
 - CONTRACTOR SHALL PROVIDE 1/8" THICK TEMPLATE FOR ANCHOR BOLT INSTALLATION.

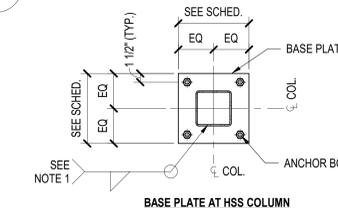
4 TYPICAL DETAIL
BASE PLATE AT WIDE FLANGE COLUMNS
1" = 1'-0"



COLUMN	BASE PLATE A X B X THICKNESS	ANCHOR BOLTS
W 8 X 35	16" X 10" X 3/4"	(4) - 3/4"Ø
W 8 X 40	16" X 10" X 3/4"	(4) - 3/4"Ø
W 8 X 48	16" X 10" X 3/4"	(4) - 3/4"Ø
W 8 X 58	16" X 12" X 3/4"	(4) - 3/4"Ø
W 8 X 67	16" X 12" X 3/4"	(4) - 3/4"Ø

- NOTES:
- MINIMUM FILLET WELD ACCORDING TO A.I.S.C. (1/4" MIN.)
 - USE OVERSIZED HOLES FOR ANCHOR BOLTS ACCORDING TO A.I.S.C.
 - WASHERS SHALL BE INSTALLED OVER OVERSIZED HOLES
 - CONTRACTOR SHALL PROVIDE 1/8" THICK TEMPLATE FOR ANCHOR BOLT INSTALLATION.

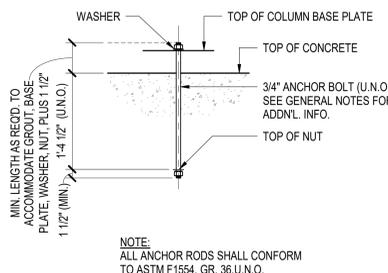
3 TYPICAL DETAIL
BASE PLATE AT WIDE FLANGE COLUMNS
1" = 1'-0"



COLUMN	BASE PLATE	ANCHOR BOLTS
HSS 4 X 4	12" X 12" X 3/4"	(4) - 3/4"Ø
HSS 5 X 5	12" X 12" X 3/4"	(4) - 3/4"Ø
HSS 6 X 6	12" X 12" X 3/4"	(4) - 3/4"Ø
HSS 8 X 8	14" X 14" X 3/4"	(4) - 3/4"Ø
HSS 10 X 10	16" X 16" X 3/4"	(4) - 3/4"Ø
HSS 12 X 12	18" X 18" X 3/4"	(4) - 3/4"Ø

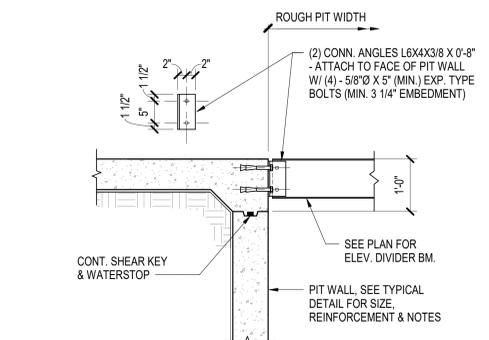
- NOTES:
- MINIMUM FILLET WELD ACCORDING TO A.I.S.C. (1/4" MIN.)
 - USE OVERSIZED HOLES FOR ANCHOR BOLTS ACCORDING TO A.I.S.C.
 - WASHERS SHALL BE INSTALLED OVER OVERSIZED HOLES
 - CONTRACTOR SHALL PROVIDE 1/8" THICK TEMPLATE FOR ANCHOR BOLT INSTALLATION.

2 TYPICAL DETAIL
BASE PLATE AT HSS COLUMNS
1" = 1'-0"



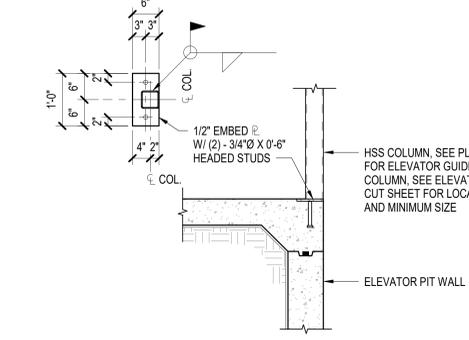
NOTE:
ALL ANCHOR RODS SHALL CONFORM TO ASTM F1554, GR. 36, U.N.O.

1 TYPICAL DETAIL - ANCHOR BOLTS
1" = 1'-0"



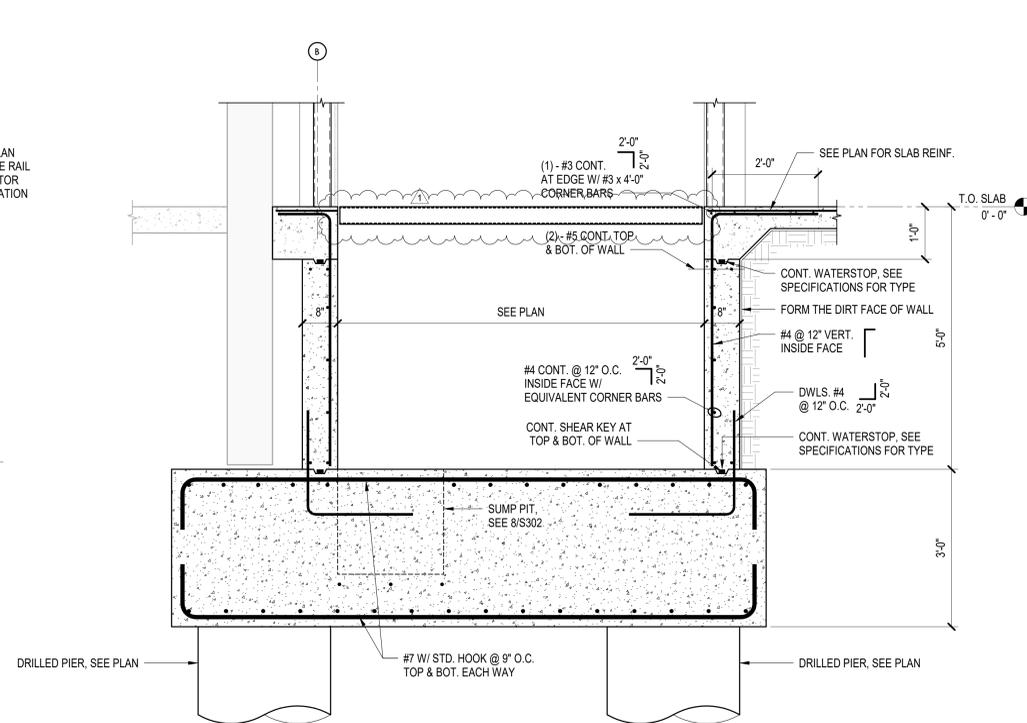
- NOTES:
- INSTALL EXPANSION BOLTS ACCORDING TO MANUFACTURER'S REQUIREMENTS.

19 TYPICAL DETAIL
ELEVATOR DIVIDER BEAM CONNECTION TO PIT WALL
3/4" = 1'-0"



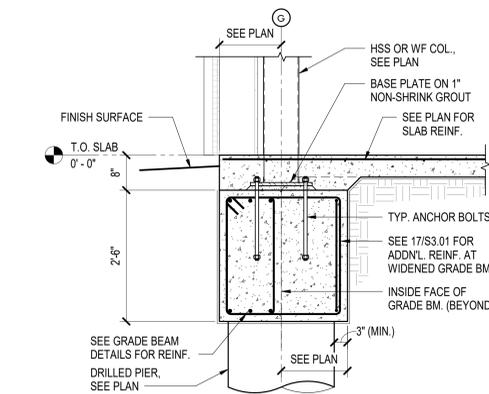
- NOTES:
- SEE TYP. DETAIL - ELEVATOR PIT FOR ADDNL. INFO. NOT SHOWN

15 DETAIL AT HSS COLUMN BEHIND ELEVATOR RAIL
3/4" = 1'-0"

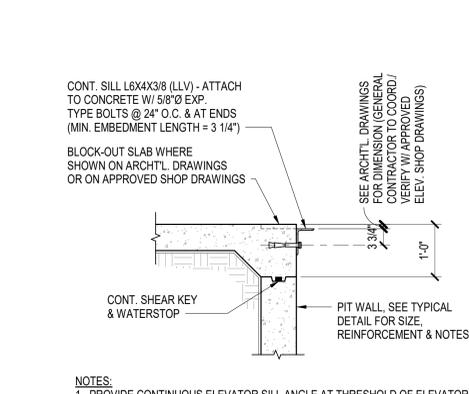


- NOTES:
- BRACE WALL AS REQUIRED UNTIL UPPER SLAB IS POURED, & HAS REACHED 2,500 PSI MIN.
 - SEE PLAN FOR SUMP PIT, WHERE REQUIRED. SLOPE PIT BOTTOM SLAB 1" TO SUMP PIT.
 - SEE OTHER DETAILS FOR ELEVATOR SILL DETAIL.
 - GENERAL CONTRACTOR TO COORDINATE ELEVATOR PIT SIZE, DEPTH, SILL DETAIL & BLOCK-OUT REQUIREMENTS WITH ELEVATOR MANUFACTURER'S APPROVED SHOP DRAWINGS.
 - WHERE ELEVATOR PIT SLAB IS BLOCKED OUT TO ALLOW FOR PLACEMENT OF HYDRAULIC ELEVATOR JACK, FILL BLOCK-OUT WITH CONCRETE AFTER JACK IS SET.

10 DETAIL AT ELEVATOR PIT
3/4" = 1'-0"

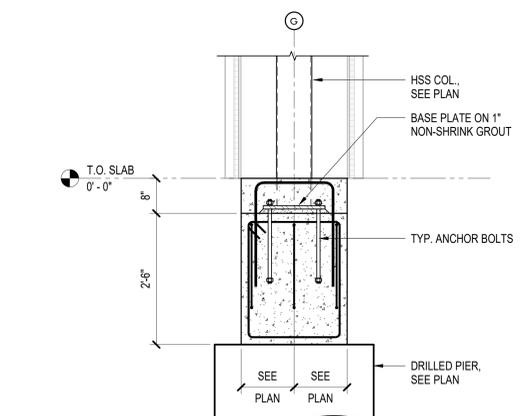


18 DETAIL AT EXTERIOR GRADE BEAM AT HSS COLUMN
3/4" = 1'-0"

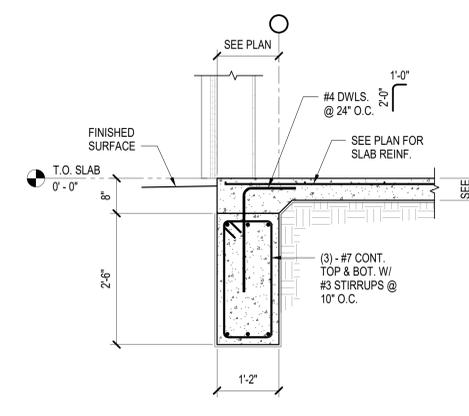


- NOTES:
- PROVIDE CONTINUOUS ELEVATOR SILL ANGLE AT THRESHOLD OF ELEVATOR PIT. SEE ARCHTL. DRAWINGS FOR LOCATION.
 - INSTALL EXPANSION BOLTS ACCORDING TO MANUFACTURER'S REQUIREMENTS.

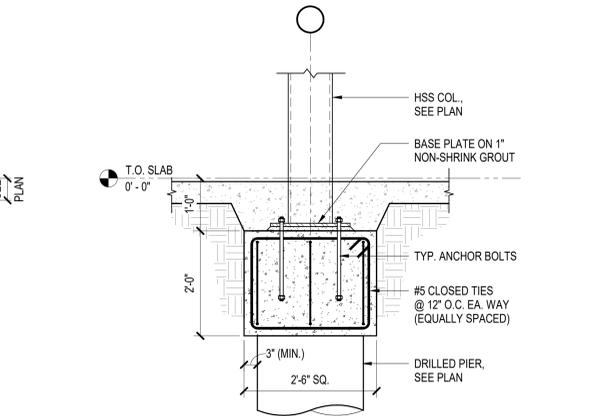
14 TYPICAL DETAIL
ELEVATOR PIT SILL ANGLE CONNECTION
3/4" = 1'-0"



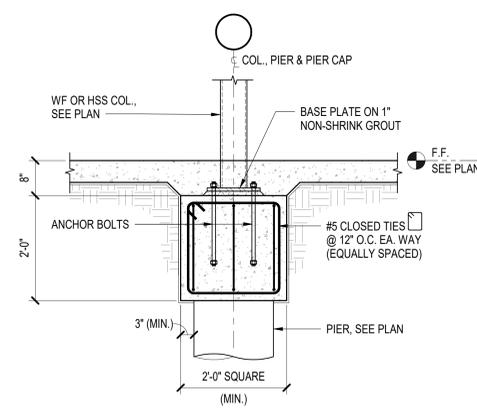
17 DETAIL AT HSS COLUMN
3/4" = 1'-0"



13 DETAIL AT EXTERIOR GRADE BEAM
3/4" = 1'-0"



6 DETAIL AT EXTERIOR HSS COLUMN
3/4" = 1'-0"

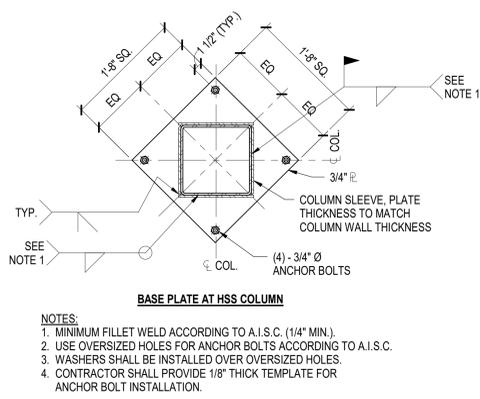


5 TYPICAL DETAIL
INTERIOR COLUMN WITH PIER CAP
3/4" = 1'-0"

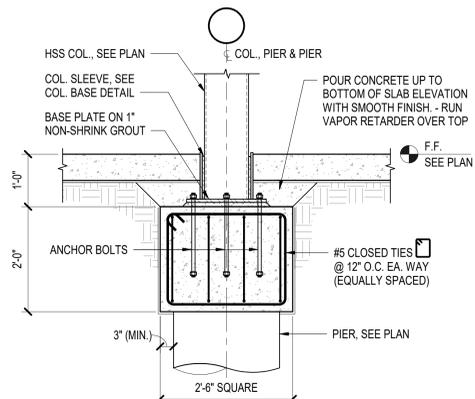
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KEY PLAN:

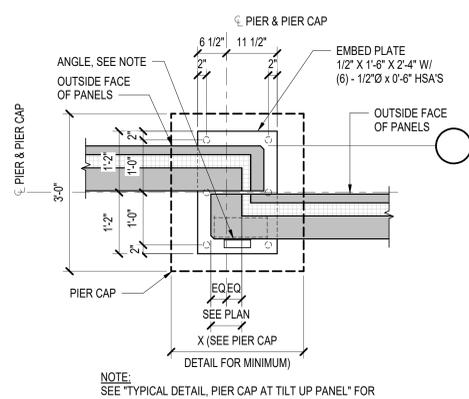
NO.	DATE	DESCRIPTION
12/15/25	50% DD	
01/14/26	100% DD	
02/27/26	ISSUE FOR BID	
1	03/16/26	ADDENDUM 1



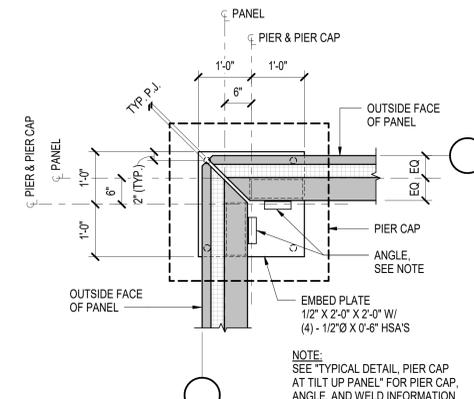
20 TYPICAL DETAIL
BASE PLATE AT HSS COLUMNS
3/4" = 1'-0"



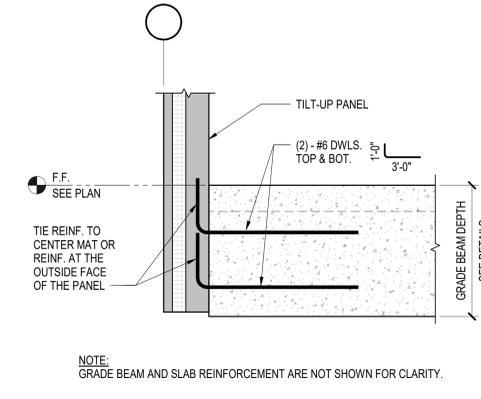
16 TYPICAL DETAIL
INTERIOR COLUMN WITH PIER CAP
3/4" = 1'-0"



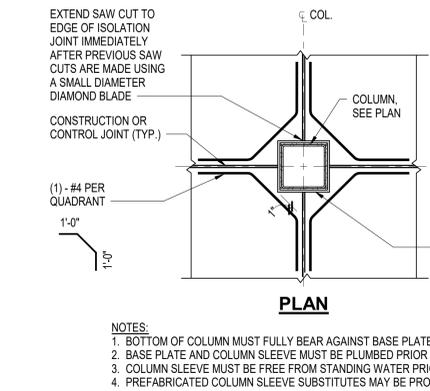
12 PLAN VIEW AT OVERLAPPING TILT-UP PANELS
3/4" = 1'-0"



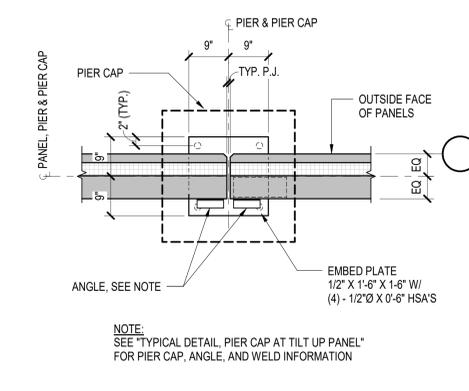
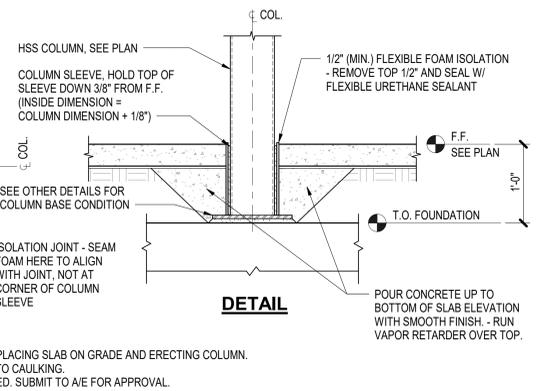
8 PLAN VIEW AT PANEL JOINT (MITRED CORNER)
3/4" = 1'-0"



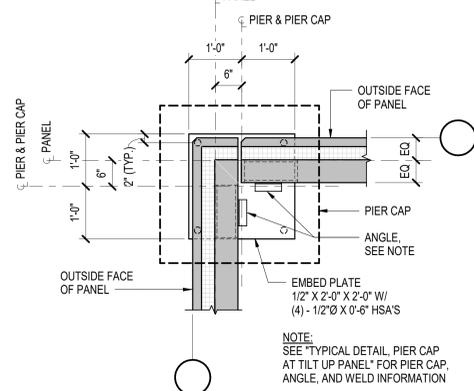
4 TYPICAL DETAIL
DOWELS TO TILT-UP PANEL AT GRADE BEAM
3/4" = 1'-0"



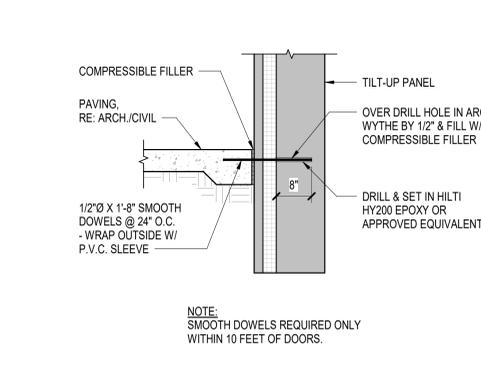
19 TYPICAL DETAIL - COLUMN BASE (SLAB-ON-GRADE) AT WAREHOUSE
3/4" = 1'-0"



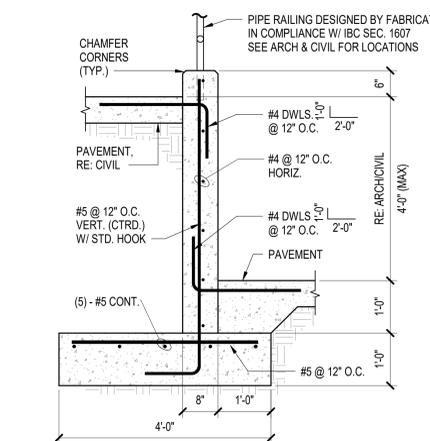
11 PLAN VIEW AT PANEL JOINT (INTERIOR)
3/4" = 1'-0"



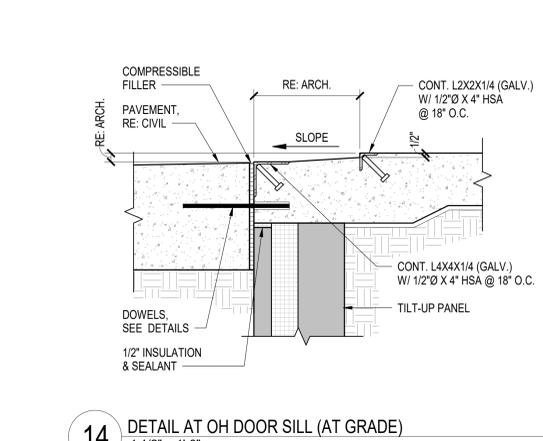
7 PLAN VIEW AT PANEL JOINT (CORNER)
3/4" = 1'-0"



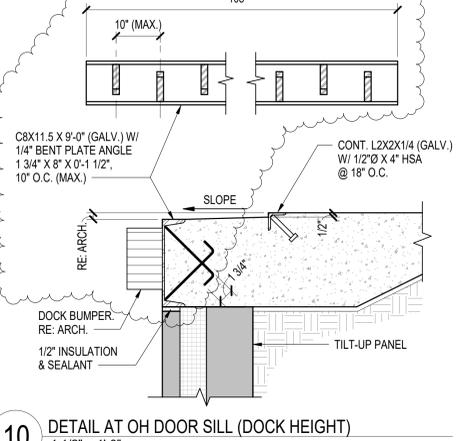
3 TYPICAL DETAIL
DOWELS TO SITE SLAB AT TILT-UP PANEL
3/4" = 1'-0"



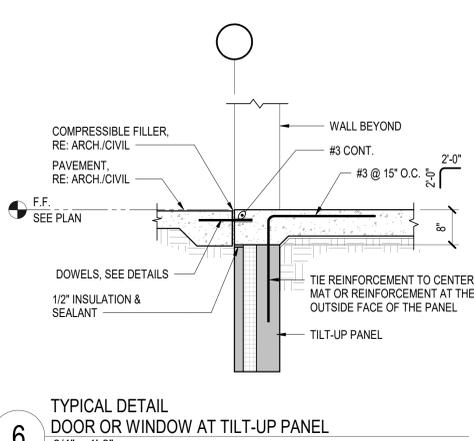
18 DETAIL AT RETAINING WALL AT TRUCK DOCK
3/4" = 1'-0"



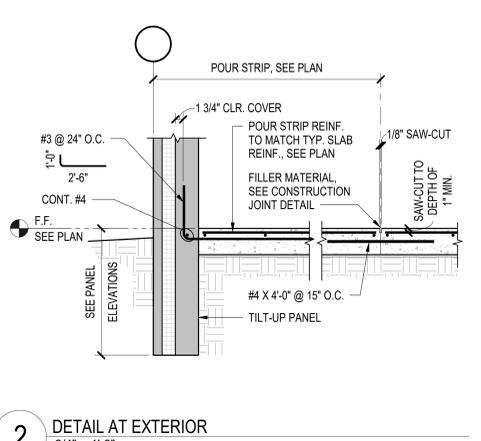
14 DETAIL AT OH DOOR SILL (AT GRADE)
1 1/2" = 1'-0"



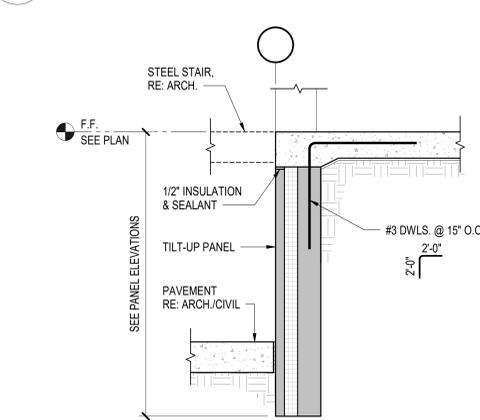
10 DETAIL AT OH DOOR SILL (DOCK HEIGHT)
1 1/2" = 1'-0"



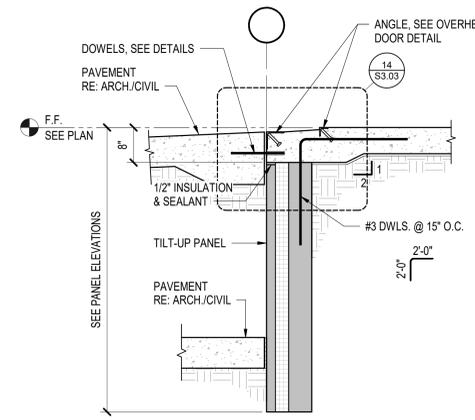
6 TYPICAL DETAIL
DOOR OR WINDOW AT TILT-UP PANEL
3/4" = 1'-0"



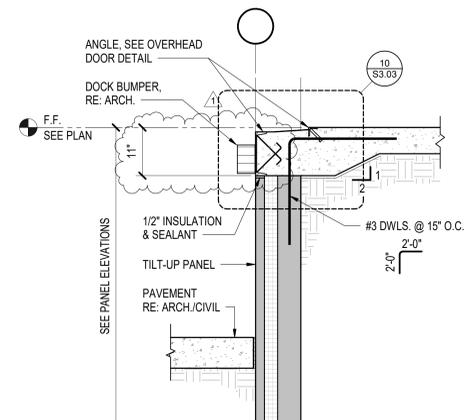
2 DETAIL AT EXTERIOR
3/4" = 1'-0"



17 TYPICAL DETAIL AT MAN DOOR AT DOCK
3/4" = 1'-0"



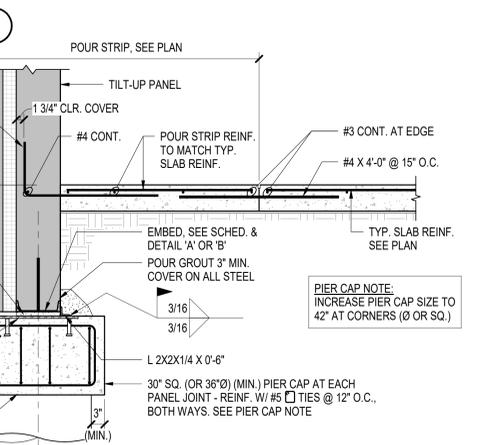
13 DETAIL AT OVERHEAD DOOR SILL (AT GRADE)
3/4" = 1'-0"



9 DETAIL AT OVERHEAD DOOR SILL (DOCK HEIGHT)
3/4" = 1'-0"

EMBED SCHEDULE	
PANEL THICKNESS	CHANNEL SIZE
5 1/2" - 5 3/4"	C5X6.7
6" - 6 3/4"	C6X8.2
7" - 7 3/4"	C7X9.8
8" - 8 3/4"	C8X11.5
9" - 9 3/4"	C9X13.4
10" - 10 3/4"	C10X15.3
> 11"	SEE DETAIL "B"

5 TYPICAL DETAIL - PIER CAP AT INSULATED TILT UP PANEL
3/4" = 1'-0"



REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
01	11/14/26	100% DD
02	27/26	ISSUE FOR BID
1	03/16/26	ADDENDUM 1

LIPSEY'S NEW HEADQUARTERS

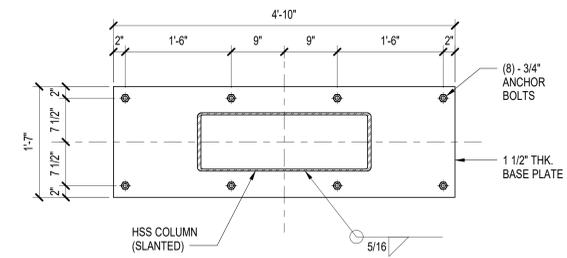
HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

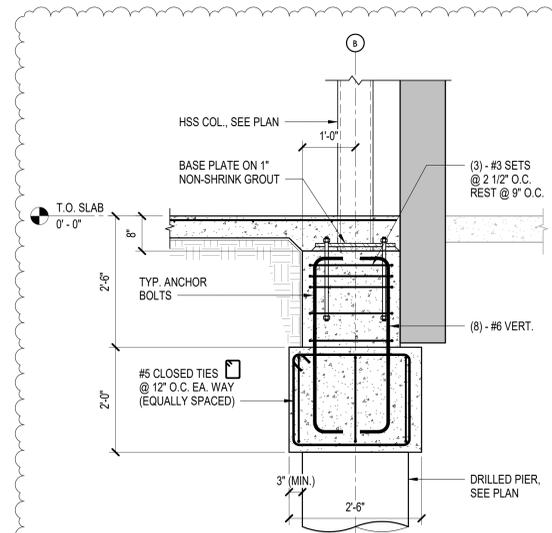
FOUNDATION DETAILS

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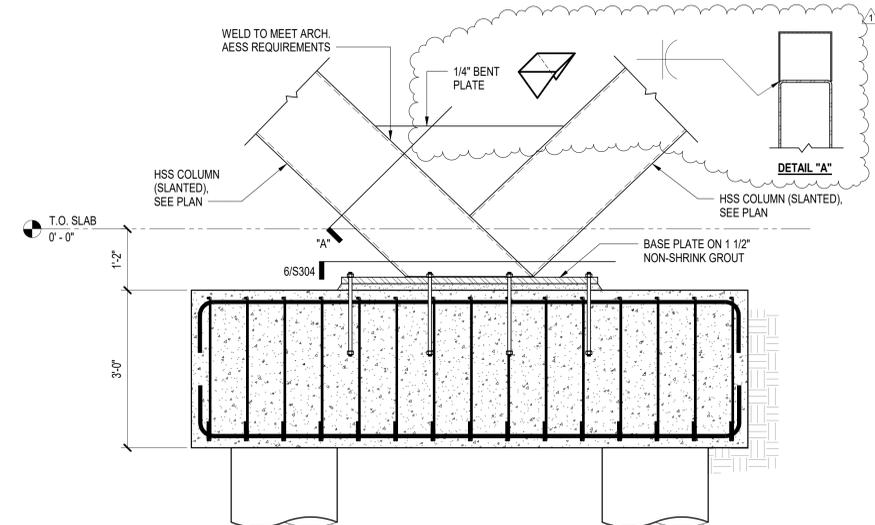
KEY PLAN:



6 DETAIL AT BASE PLATE AT "V" SHAPED COLUMN
1" = 1'-0"



9 DETAIL AT INTERIOR HSS COLUMN WITH PLINTH
3/4" = 1'-0"



5 DETAIL AT "V" SHAPED COLUMNS
3/4" = 1'-0"



REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
02/27/26		ISSUE FOR BID
1	03/16/26	ADDENDUM 1

LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

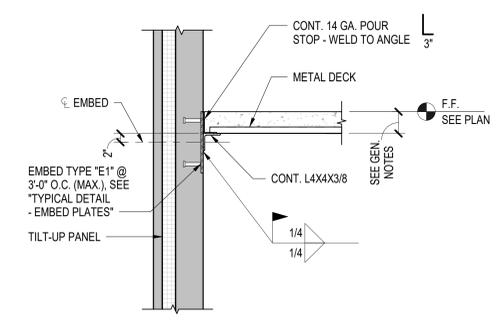
RMA PROJECT NO: 24.154

FOUNDATION DETAILS

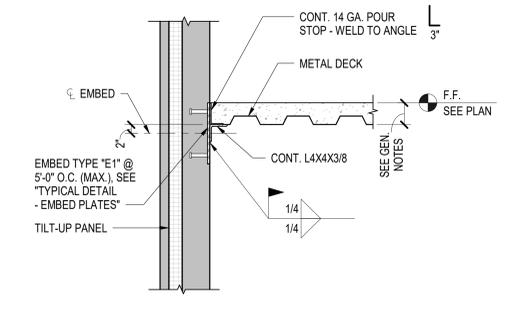
S3.04

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KEY PLAN:

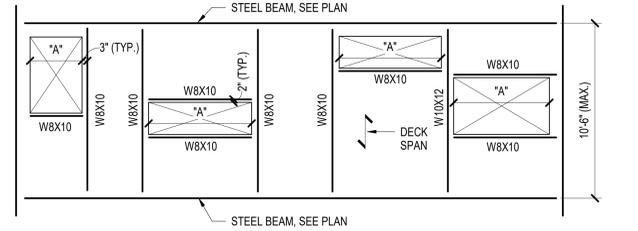


20 TYPICAL DETAIL - PERPENDICULAR DECK SUPPORT AT TILT-UP PANEL AT FLOOR
 3/4" = 1'-0"



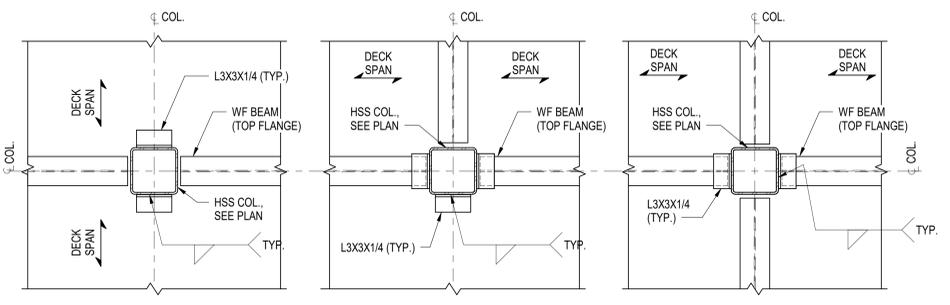
16 TYPICAL DETAIL - PARALLEL DECK SUPPORT AT TILT-UP PANEL AT FLOOR
 3/4" = 1'-0"

"A" = OPENING DIMENSION IN THE DIRECTION PERPENDICULAR TO DECK SPAN. USE THIS DETAIL FOR 2'-0" < "A" < 8'-0".

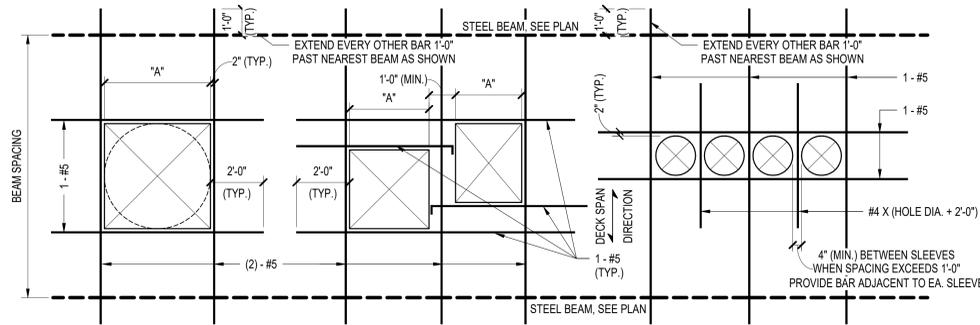


- NOTES:**
- COORDINATE OPENING SIZE AND LOCATIONS WITH ARCHITECTURAL DRAWINGS & MEP DRAWINGS.
 - THIS DETAIL SHOWS TYPICAL CONFIGURATIONS. VERIFY WITH ENGINEER FOR ALL OTHER CONDITIONS.
 - CONTACT ENGINEER OF RECORD IF DIMENSION "A" EXCEEDS 8'-0".
 - DECK MANUFACTURER TO PROVIDE 14 GAGE METAL EDGE CLOSURE AROUND EACH OPENING UNLESS DETAILED OTHERWISE.
 - IF AN OPENING EDGE IS WITHIN 12" OF THE CENTERLINE OF A COMPOSITE STEEL BEAM, CONTACT ENGINEER FOR GUIDANCE.
 - FOR OPENINGS ON BOTH SIDES OF AN ADDITIONAL FRAMING MEMBER, USE A W10X12 BEAM, AS SHOWN.
 - IF OPENING LOCATION RESULTS IN SINGLE-SPAN METAL DECK, CONTACT ENGINEER OF RECORD FOR GUIDANCE.

15 TYPICAL DETAIL - FRAMED OPENINGS IN COMPOSITE METAL DECK SLAB
 1/8" = 1'-0"

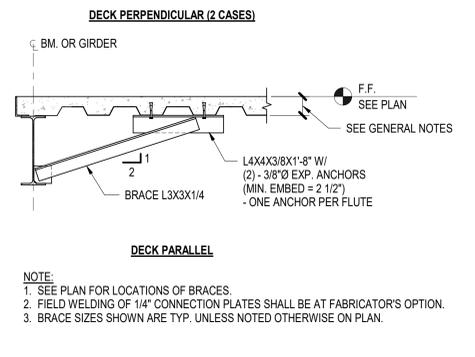
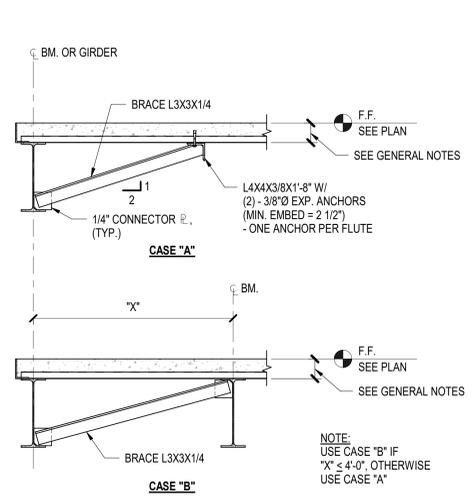


18 TYPICAL DETAIL - METAL DECK SUPPORT AT COLUMN
 1" = 1'-0"

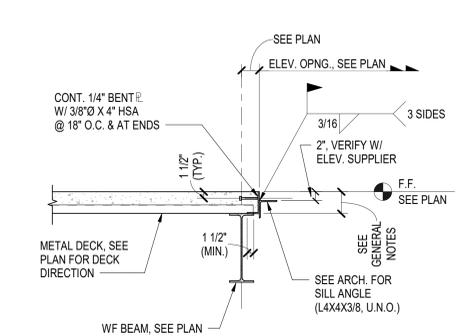


- NOTES:**
- COORDINATE OPENING SIZE & LOCATIONS WITH ARCHITECTURAL DRAWINGS & MEP DRAWINGS.
 - MAXIMUM DIMENSION "A" SHALL BE 2'-0". REFER TO PLAN OR DETAIL ON DRAWINGS FOR FRAMING OF OPENINGS GREATER THAN 2'-0".
 - SLAB REINFORCEMENT SHALL BE CUT AROUND FORMED OPENING & SHALL EXTEND TO WITHIN 2" OF OPENING ON ALL SIDES.
 - OPENINGS 10" & SMALLER DO NOT REQUIRE ADDITIONAL REINFORCEMENT.
 - FIELD CUT OPENINGS IN METAL DECK AFTER SURROUNDING CONCRETE SLAB HAS BEEN POURED & HAS ATTAINED MINIMUM OF 75% OF ITS SPECIFIED 28-DAY COMPRESSIVE STRENGTH.
 - REINFORCEMENT PERPENDICULAR TO DECK SPAN SHALL BE PLACED ON TOP OF DECK FLUTES.
 - REINFORCEMENT PARALLEL TO DECK SPAN SHALL BE PLACED IN THE LOWER FLUTE & SHALL BE CHAIRED TO PROVIDE 3/4" COVER.
 - THIS DETAIL SHOWS TYPICAL CONDITIONS. VERIFY REINFORCEMENT STEEL PLACEMENT WITH ENGINEER FOR ALL OTHER CONDITIONS.
 - PROVIDE REINFORCEMENT AS SHOWN FOR SQUARE, RECTANGULAR & ROUND OPENINGS.
 - IF AN OPENING EDGE IS WITHIN 12" OF THE CENTERLINE OF A STEEL BEAM SHOWN ON PLAN, CONTACT ENGINEER FOR GUIDANCE.

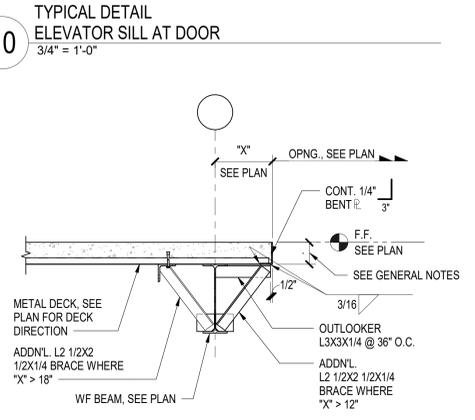
17 TYPICAL DETAIL - OPENINGS IN COMPOSITE METAL DECK SLAB, FLOOR OR ROOF
 3/8" = 1'-0"



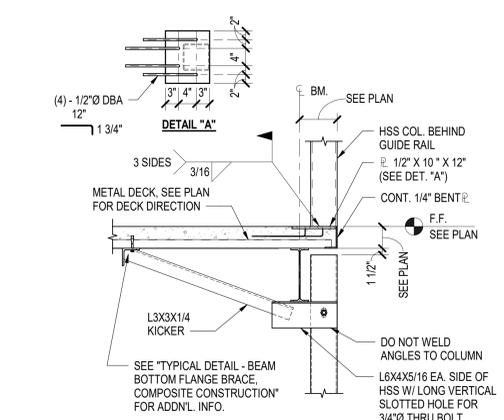
11 TYPICAL DETAIL - BM. BOT. FLANGE BRACE COMPOSITE CONSTRUCTION
 3/4" = 1'-0"



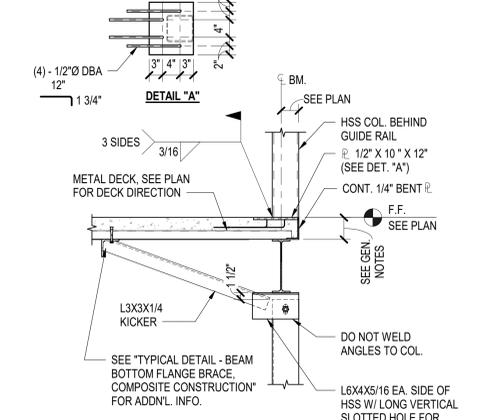
10 TYPICAL DETAIL - ELEVATOR SILL AT DOOR
 3/4" = 1'-0"



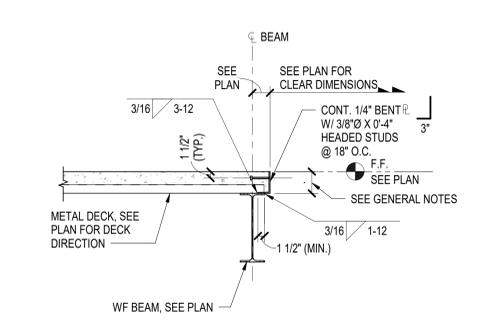
9 TYPICAL DETAIL - EDGE OF COMPOSITE METAL DECK AT OPENING
 3/4" = 1'-0"



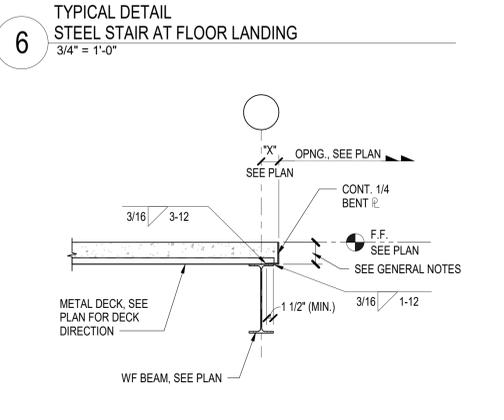
8 DETAIL AT HSS COLUMNS AT ELEVATOR AT COMPOSITE EXTENDED EDGE
 3/4" = 1'-0"



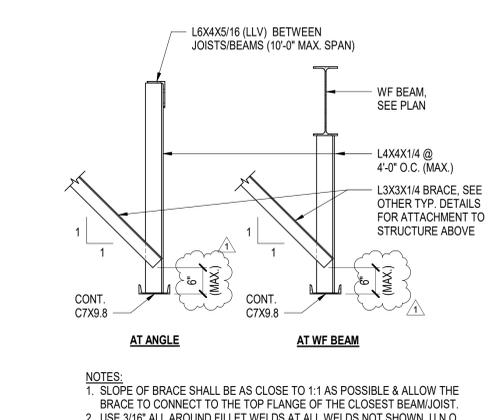
7 DETAIL AT HSS COLUMNS AT ELEVATOR AT COMPOSITE EDGE
 3/4" = 1'-0"



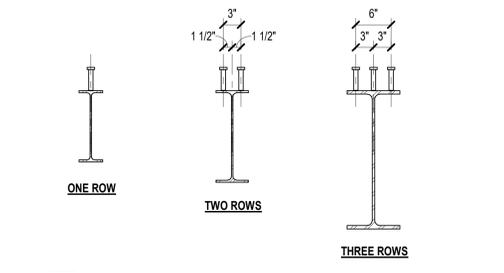
6 TYPICAL DETAIL - STEEL STAIR AT FLOOR LANDING
 3/4" = 1'-0"



5 TYPICAL DETAIL - EDGE OF COMPOSITE METAL DECK AT OPENING
 3/4" = 1'-0"

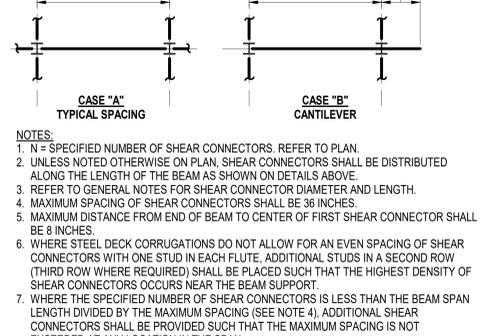


4 TYPICAL DETAIL - WINDOW WALL SUPPORT
 3/4" = 1'-0"

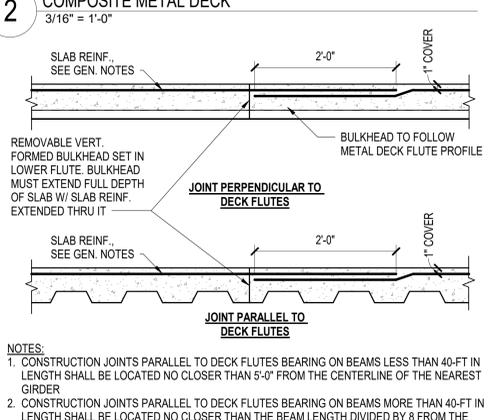


- NOTES:**
- SHEAR CONNECTORS IN ONE ROW SHALL BE PLACED DIRECTLY OVER THE BEAM WEB.
 - PLACE STUDS IN A SINGLE ROW WHERE SPACING REQUIREMENTS PERMIT. STUDS SHALL BE PLACED IN TWO ROWS OR THREE ROWS ONLY WHERE REQUIRED IN ORDER TO PLACE THE TOTAL NUMBER OF STUDS.
 - SEE TYPICAL DETAIL, SHEAR CONNECTOR PLACEMENT DIAGRAM.
 - PROVIDE MINIMUM 1" SPACING FROM EDGE OF FLANGE TO CENTERLINE OF STUD.
 - SUBMIT SHOP DRAWINGS SHOWING PLACEMENT OF SHEAR CONNECTORS FOR ENGINEER APPROVAL.

3 TYPICAL DETAIL - SHEAR CONNECTOR SPACING CRITERIA
 1" = 1'-0"



2 TYPICAL DETAIL - SHEAR CONNECTOR PLACEMENT DIAGRAMS COMPOSITE METAL DECK
 3/16" = 1'-0"



1 TYPICAL DETAIL - CONSTRUCTION JOINT CONCRETE SLAB ON METAL DECK
 1" = 1'-0"

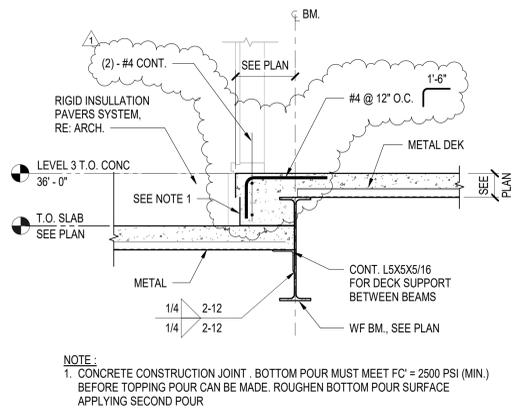


NO.	DATE	DESCRIPTION
01/14/26	100% DD	
02/27/26	ISSUE FOR BID	
03/16/26	ADDENDUM 1	

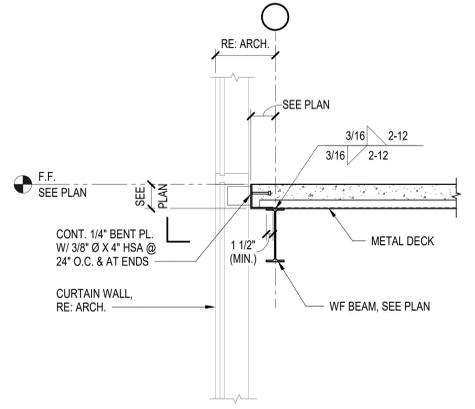
LIPSEY'S NEW HEADQUARTERS
 HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO.: 24.154

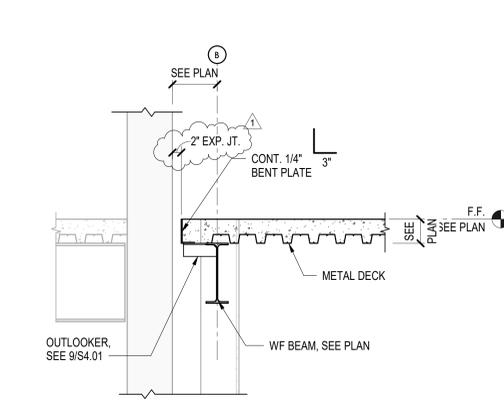
FRAMING DETAILS



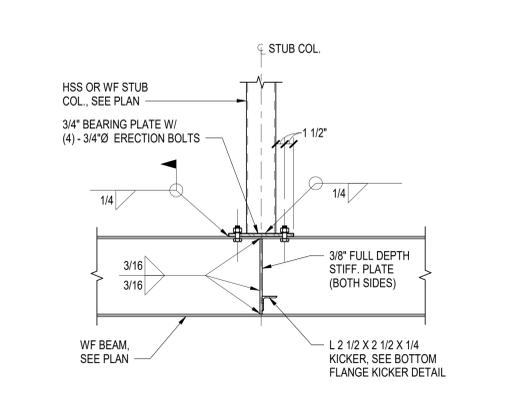
16 DETAIL AT BALCONY
3/4" = 1'-0"



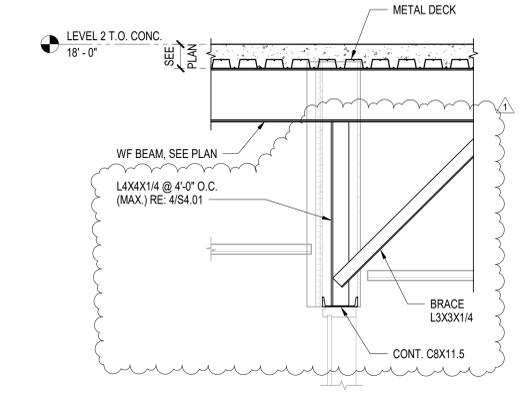
12 DETAIL AT EXTERIOR
3/4" = 1'-0"



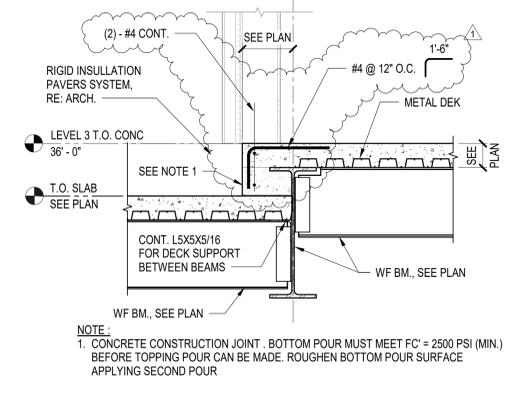
8 DETAIL AT EDGE OF SLAB
3/4" = 1'-0"



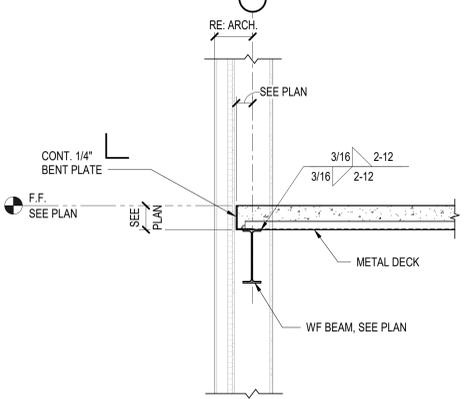
4 TYPICAL DETAIL - STUB COLUMN
1" = 1'-0"



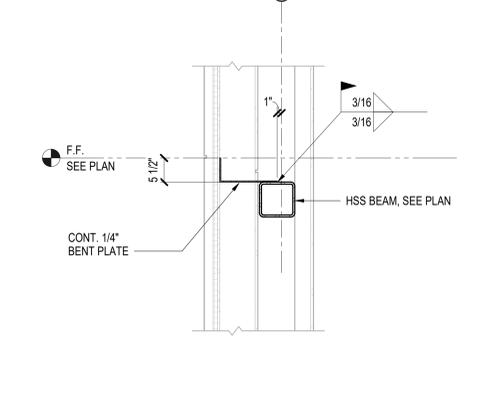
19 DETAIL AT EXTERIOR
3/4" = 1'-0"



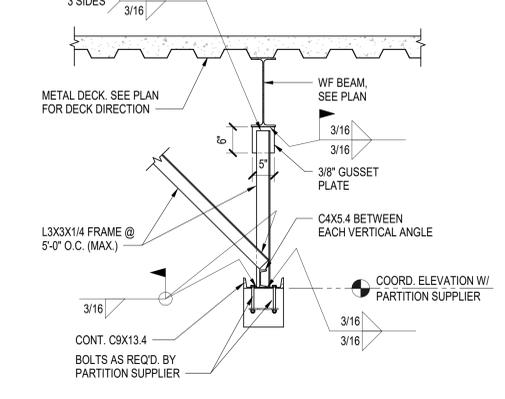
15 DETAIL AT BALCONY
3/4" = 1'-0"



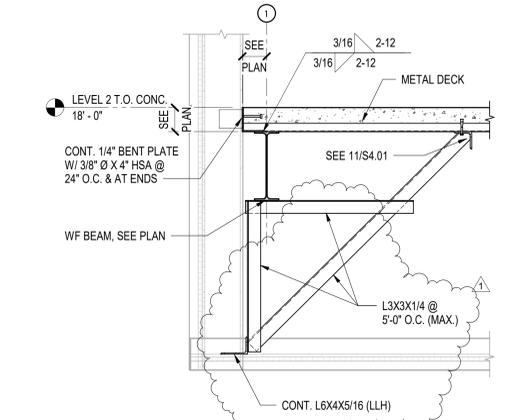
11 DETAIL AT EXTERIOR
3/4" = 1'-0"



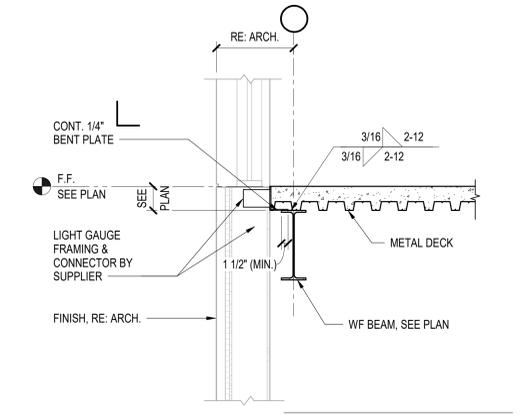
7 DETAIL AT EXTERIOR
3/4" = 1'-0"



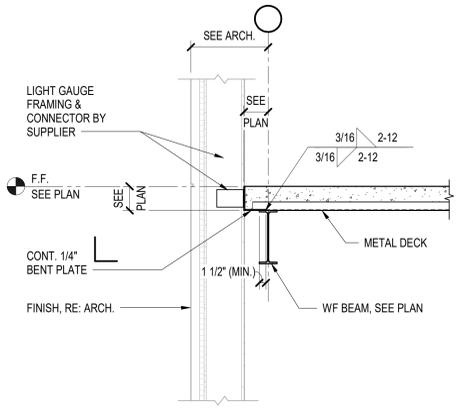
3 TYPICAL DETAIL OPERABLE PARTITION AT COMPOSITE FLOOR FRAMING
3/4" = 1'-0"



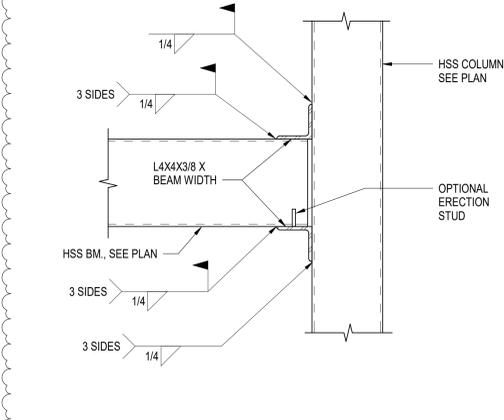
18 DETAIL AT EXTERIOR
3/4" = 1'-0"



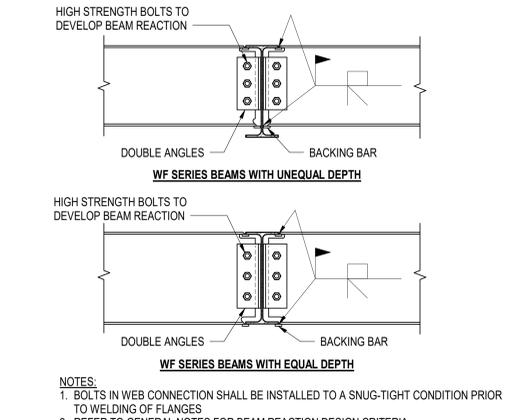
14 DETAIL AT EXTERIOR
3/4" = 1'-0"



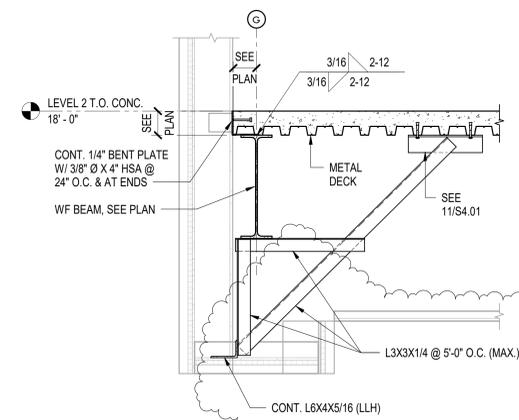
10 DETAIL AT EXTERIOR
3/4" = 1'-0"



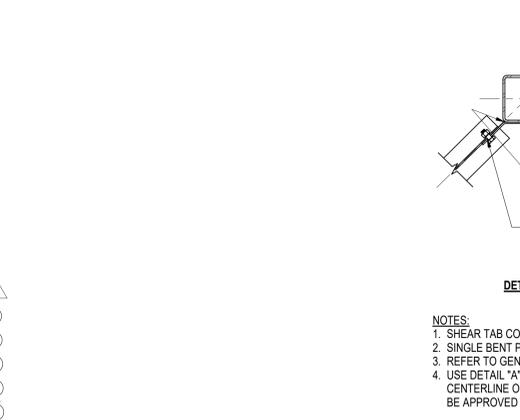
6 TYPICAL DETAIL HSS BEAM TO HSS COLUMN CONNECTION
1 1/2" = 1'-0"



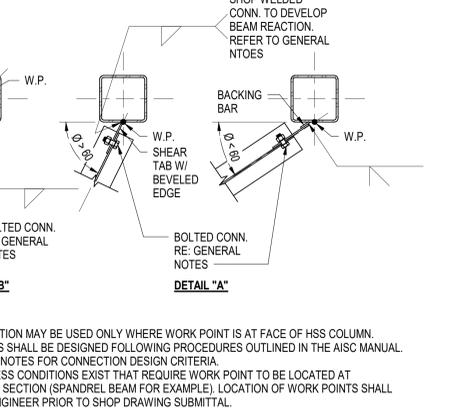
2 TYPICAL DETAIL - WF BEAM TO WF BEAM MOMENT CONNECTION
1" = 1'-0"



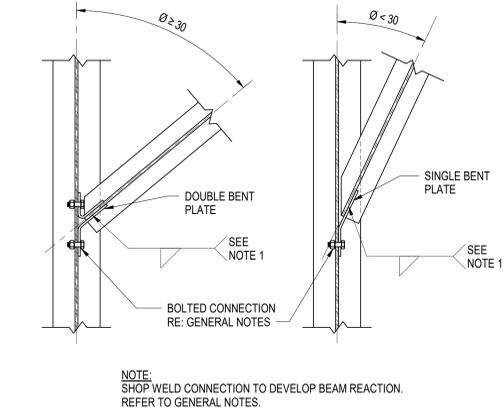
17 DETAIL AT EXTERIOR
3/4" = 1'-0"



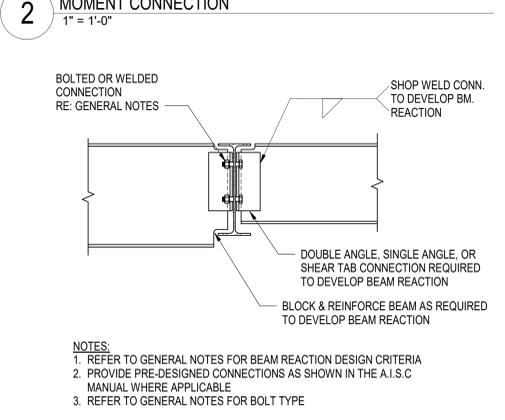
13 DETAIL AT EXTERIOR
3/4" = 1'-0"



9 TYPICAL DETAIL - SKEWED WF BEAM TO HSS COLUMN SHEAR CONNECTION
1" = 1'-0"



5 TYPICAL DETAIL - SKEWED WIDE FLANGE BEAM TO WIDE BEAM CONNECTION SHEAR CONNECTION
1" = 1'-0"



1 TYPICAL DETAIL AT WF BEAM TO WF BEAM CONNECTION
1" = 1'-0"



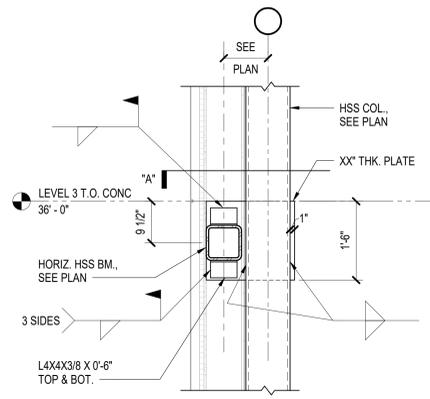
REVISION SCHEDULE	
NO.	DESCRIPTION
01/14/26	100% DD
02/27/26	ISSUE FOR BID
03/16/26	ADDENDUM 1

LIPSEY'S NEW HEADQUARTERS

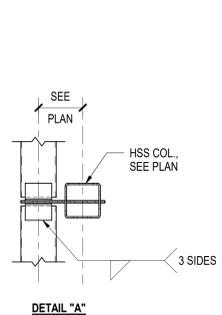
HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

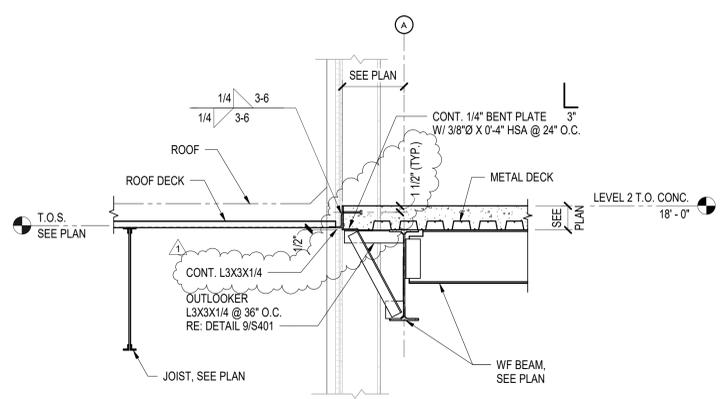
FRAMING DETAILS



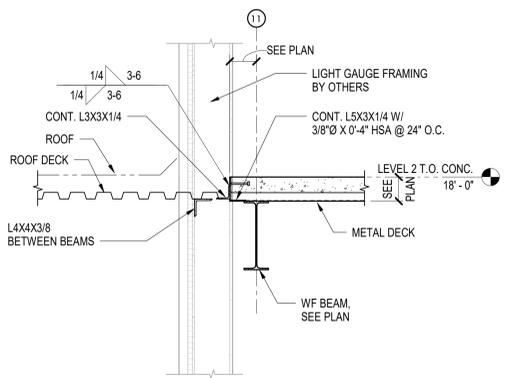
16 DETAIL AT EXTERIOR
3/4" = 1'-0"



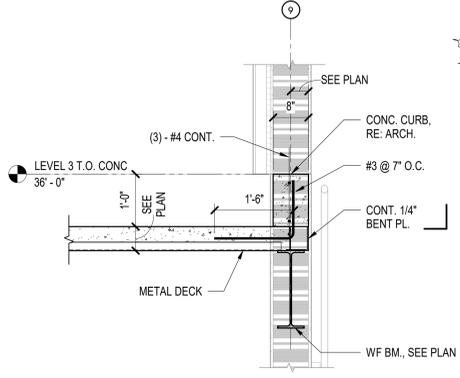
12 DETAIL AT BALCONY
3/4" = 1'-0"



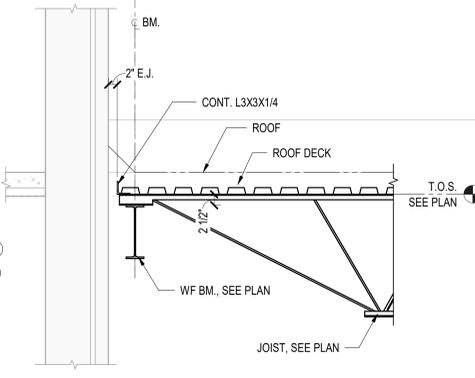
8 DETAIL AT SLAB EDGE
3/4" = 1'-0"



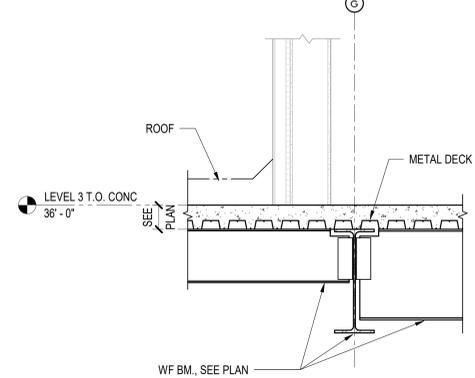
15 DETAIL AT CANOPY AND FLOOR
3/4" = 1'-0"



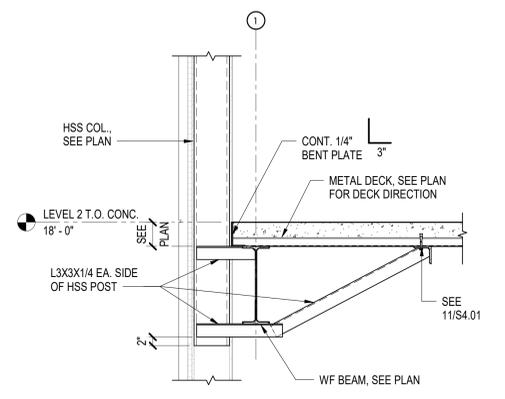
11 DETAIL AT BALCONY
3/4" = 1'-0"



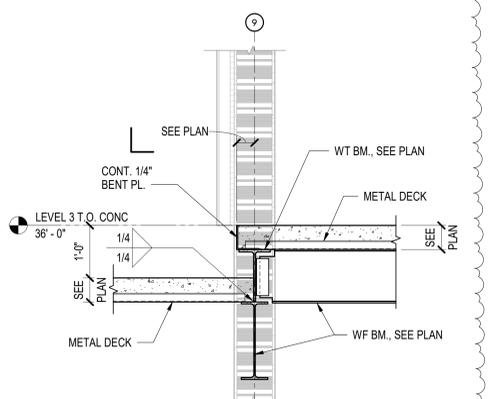
7 DETAIL AT EXTERIOR
3/4" = 1'-0"



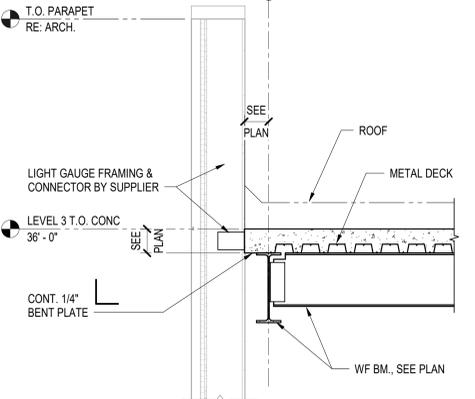
3 DETAIL AT FLOOR
3/4" = 1'-0"



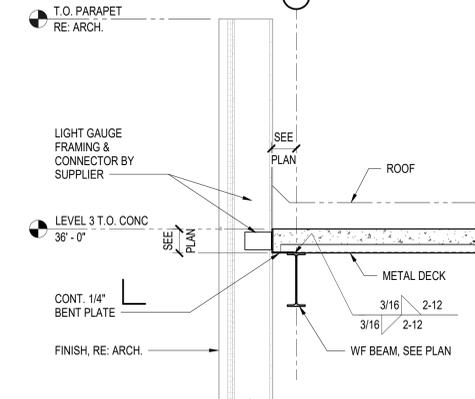
14 DETAIL AT EXTERIOR
3/4" = 1'-0"



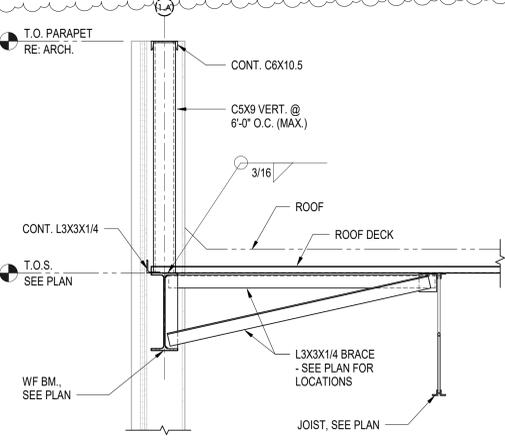
10 DETAIL AT BALCONY
3/4" = 1'-0"



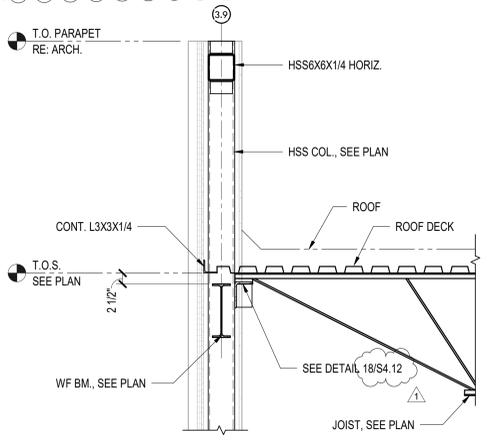
6 DETAIL AT EXTERIOR
3/4" = 1'-0"



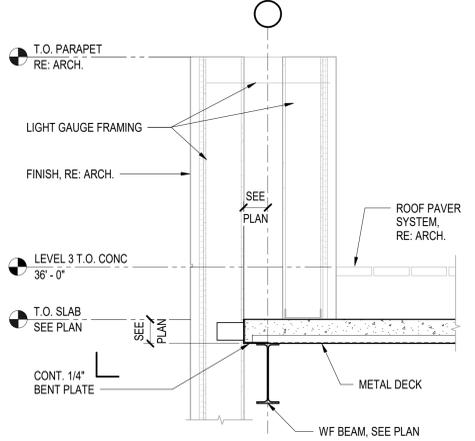
2 DETAIL AT EXTERIOR
3/4" = 1'-0"



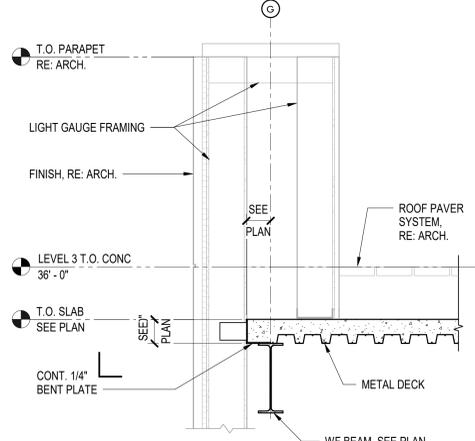
13 DETAIL AT EXTERIOR
3/4" = 1'-0"



9 DETAIL AT EXTERIOR
3/4" = 1'-0"



5 DETAIL AT EXTERIOR
3/4" = 1'-0"



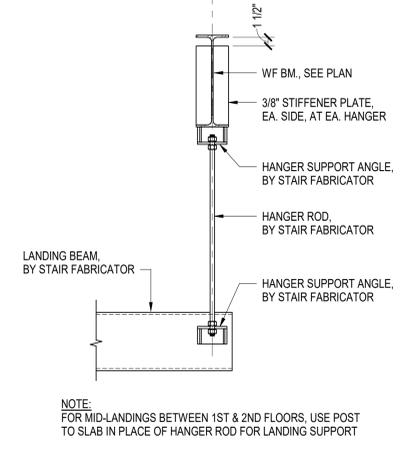
1 DETAIL AT EXTERIOR
3/4" = 1'-0"



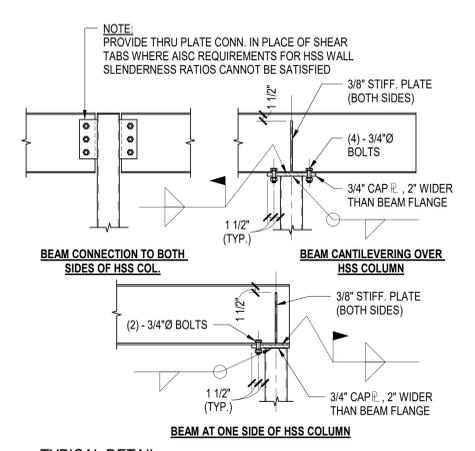
REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
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1	03/16/26	ADDENDUM 1

LIPSEY'S NEW HEADQUARTERS
HIGHLAND RD, ST. GEORGE, LA
RMA PROJECT NO.: 24.154

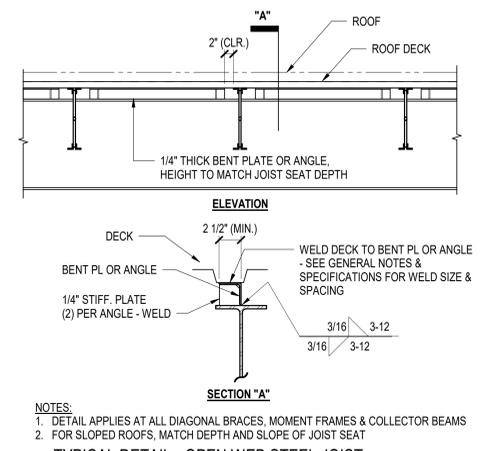
FRAMING DETAILS



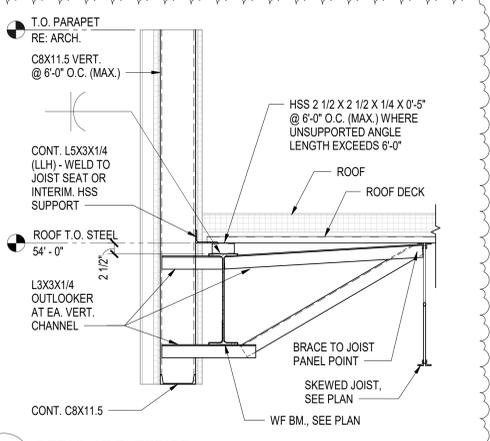
12 TYPICAL DETAIL - STAIR HANGER
3/4" = 1'-0"



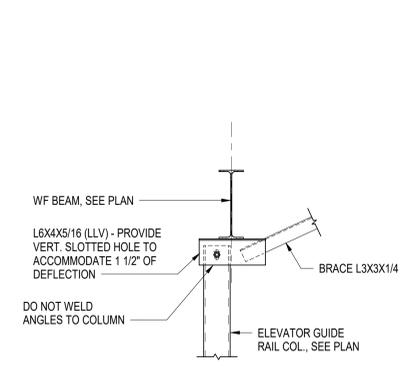
8 TYPICAL DETAIL - STEEL BEAM TO HSS COLUMN CONNECTION
3/4" = 1'-0"



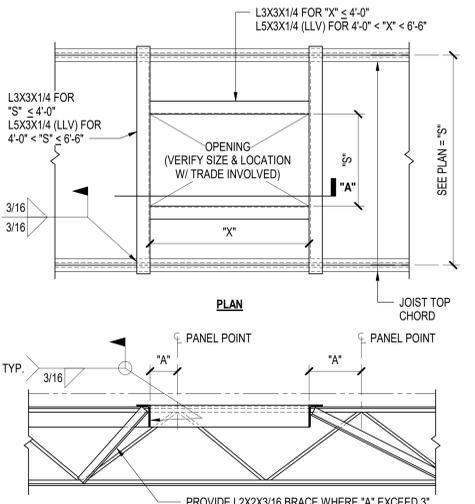
4 TYPICAL DETAIL - OPEN WEB STEEL JOIST DECK DIAPHRAGM CONN. TO STEEL BEAM
3/4" = 1'-0"



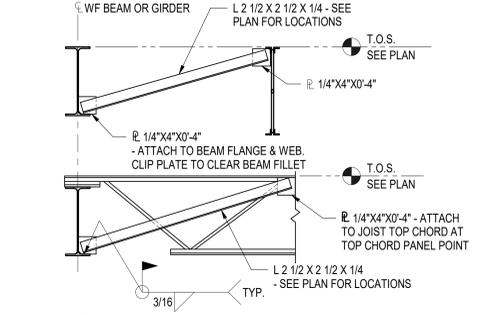
19 DETAIL AT EXTERIOR
3/4" = 1'-0"



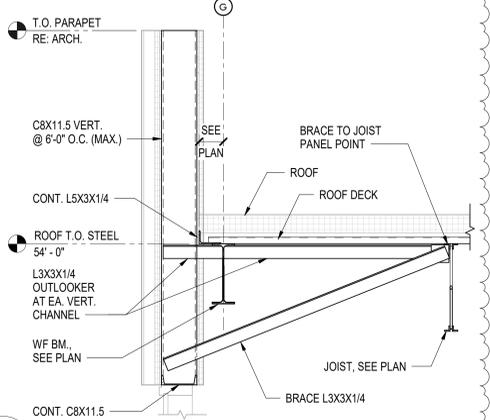
11 DETAIL AT HSS COLUMN ELEVATOR
3/4" = 1'-0"



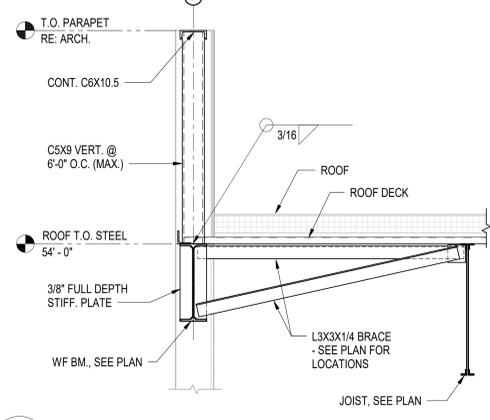
6 TYPICAL DETAIL - OPEN WEB STEEL JOIST ROOF OPENING
3/4" = 1'-0"



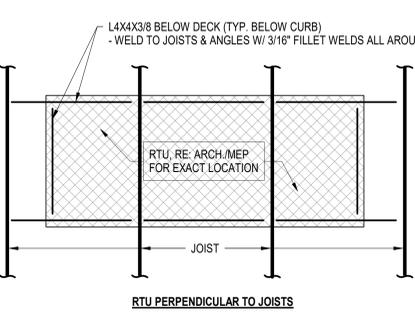
3 TYPICAL DETAIL - BEAM BOT. FLANGE BRACE JOIST CONSTRUCTION
3/4" = 1'-0"



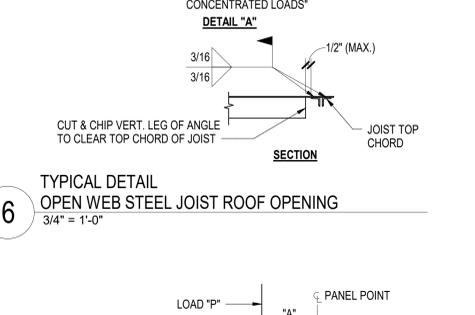
18 DETAIL AT EXTERIOR
3/4" = 1'-0"



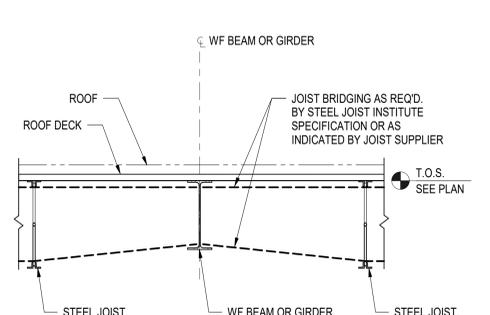
14 DETAIL AT EXTERIOR
3/4" = 1'-0"



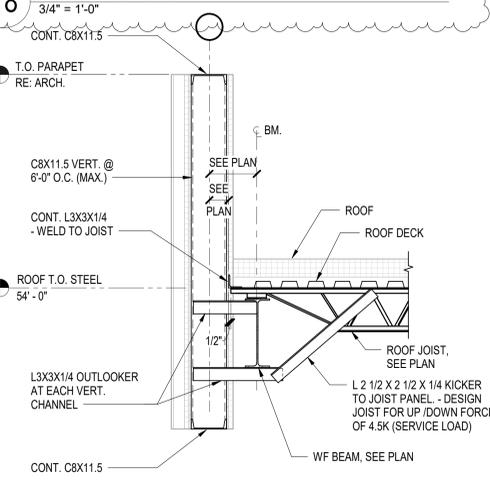
9 TYPICAL DETAIL - RTU SUPPORT AT ROOF
3/8" = 1'-0"



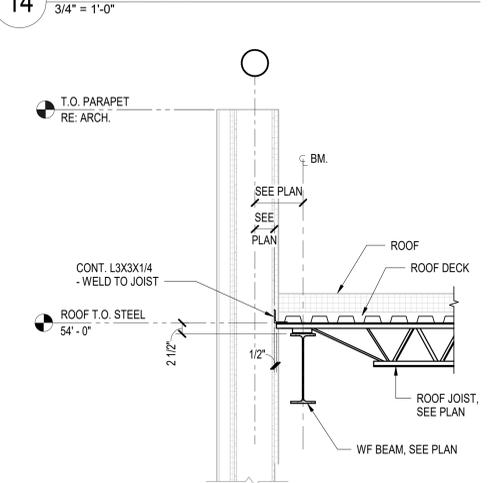
5 TYPICAL DETAIL - STIFFENING OF STEEL JOIST FOR CONCENTRATED LOADS
3/4" = 1'-0"



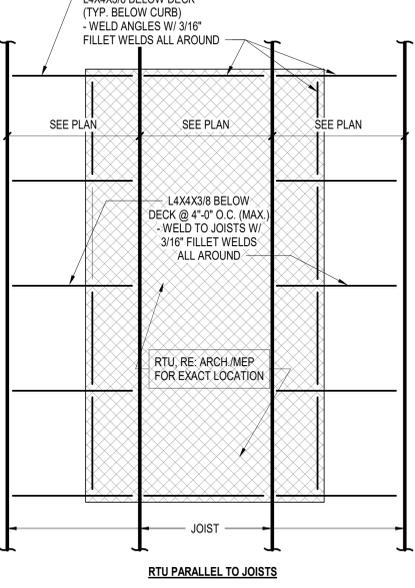
2 TYPICAL DETAIL - OPEN WEB STEEL JOIST AT INTERIOR TIE BEAM, ROOF
3/4" = 1'-0"



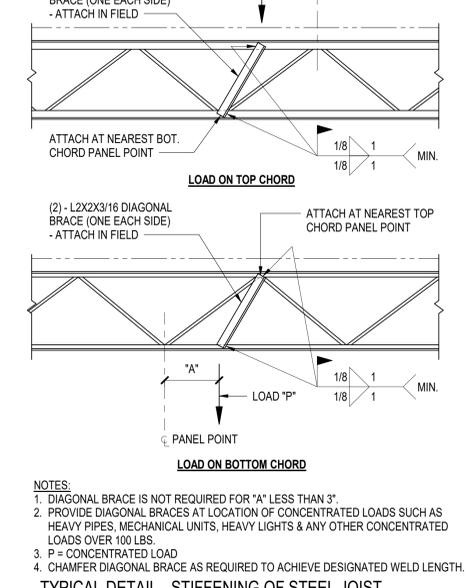
17 DETAIL AT EXTERIOR
3/4" = 1'-0"



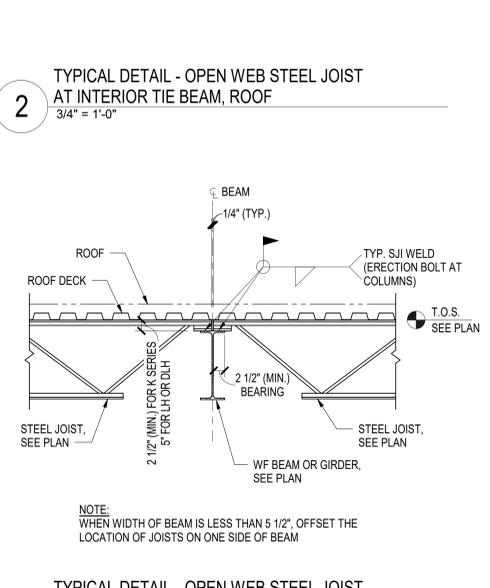
13 DETAIL AT EXTERIOR
3/4" = 1'-0"



9 TYPICAL DETAIL - RTU SUPPORT AT ROOF
3/8" = 1'-0"



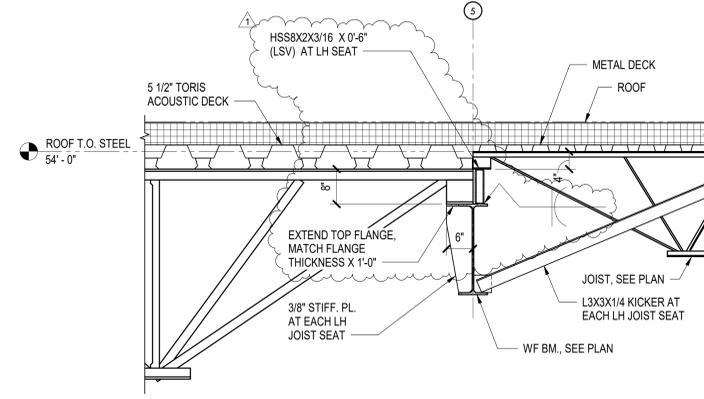
5 TYPICAL DETAIL - STIFFENING OF STEEL JOIST FOR CONCENTRATED LOADS
3/4" = 1'-0"



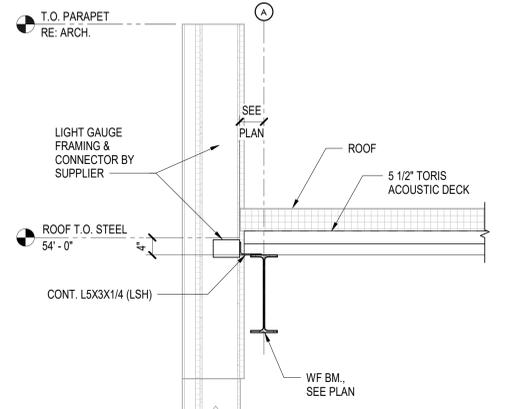
1 TYPICAL DETAIL - OPEN WEB STEEL JOIST AT INTERIOR JOIST GIRDER, ROOF
3/4" = 1'-0"

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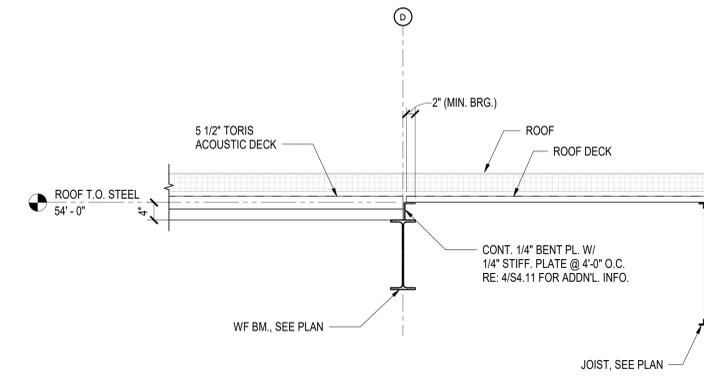
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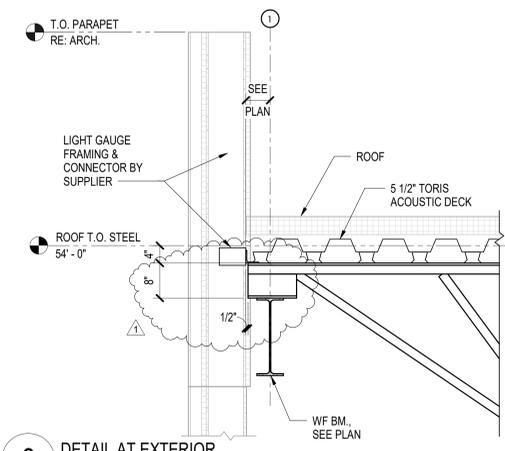
8 DETAIL AT ROOF
3/4" = 1'-0"



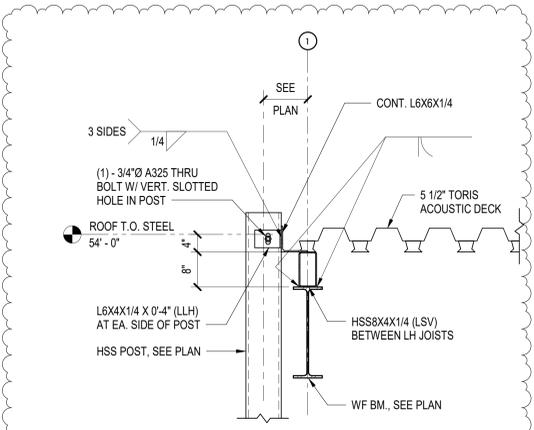
4 DETAIL AT EXTERIOR
3/4" = 1'-0"



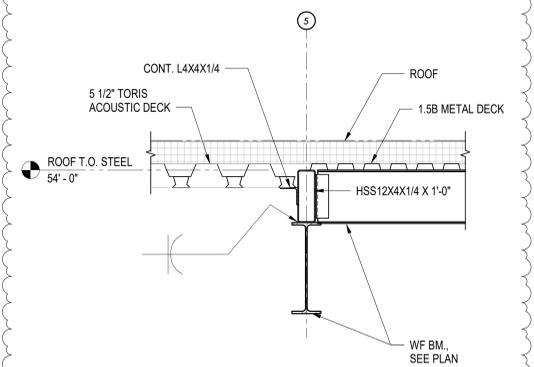
7 DETAIL AT ROOF
3/4" = 1'-0"



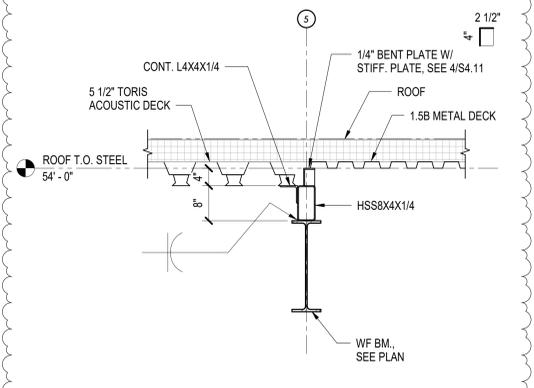
3 DETAIL AT EXTERIOR
3/4" = 1'-0"



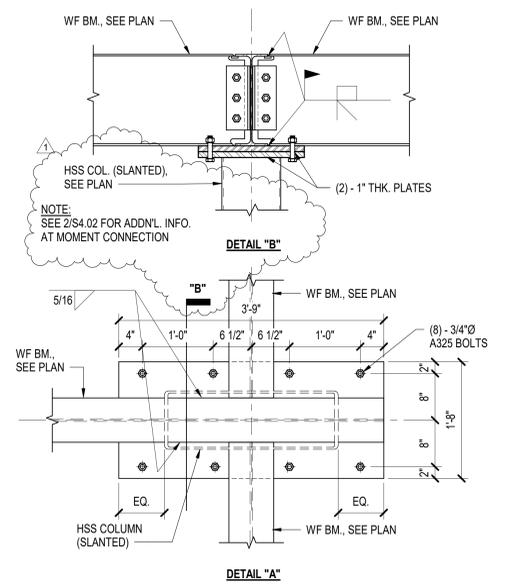
19 DETAIL AT EXTERIOR
3/4" = 1'-0"



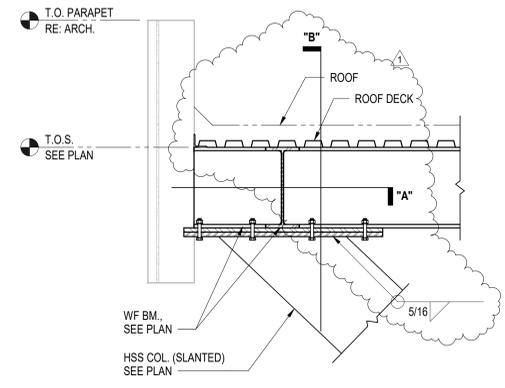
18 DETAIL AT ROOF
3/4" = 1'-0"



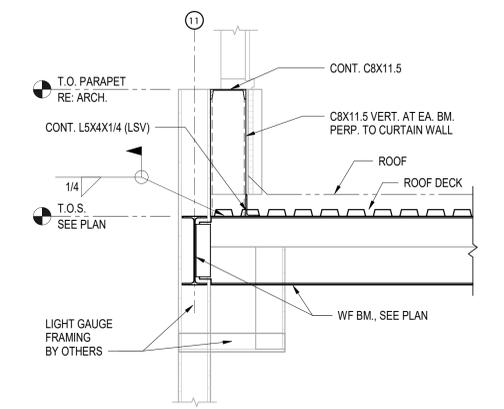
17 DETAIL AT ROOF
3/4" = 1'-0"



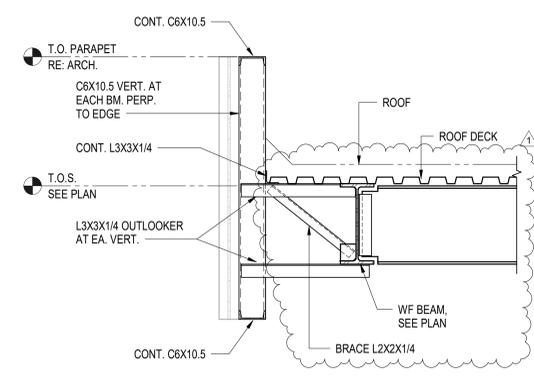
13 DETAIL AT CANOPY
3/4" = 1'-0"



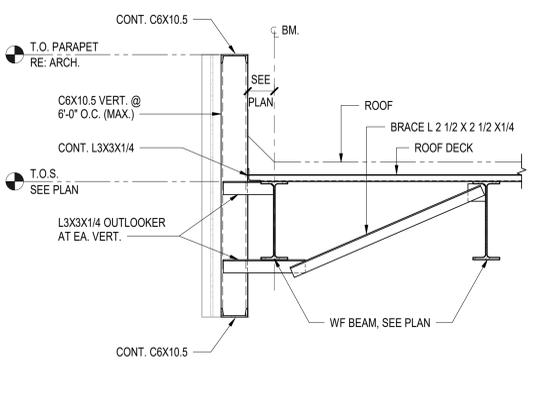
5 DETAIL AT CANOPY
3/4" = 1'-0"



6 DETAIL AT CANOPY
3/4" = 1'-0"



2 DETAIL AT CANOPY
3/4" = 1'-0"



1 DETAIL AT CANOPY
3/4" = 1'-0"



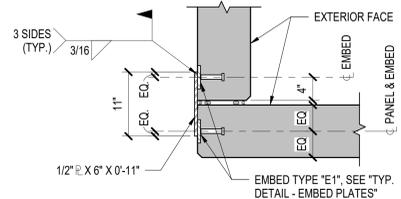
REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
02/27/26		ISSUE FOR BID
1	03/16/26	ADDENDUM 1

LIPSEY'S NEW HEADQUARTERS

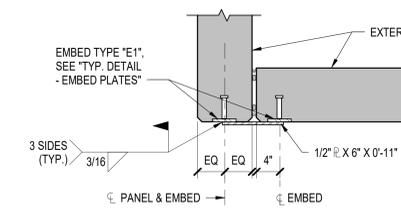
HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO.: 24.154

FRAMING DETAILS



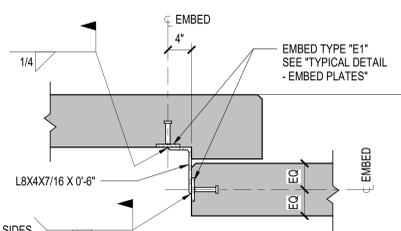
PLAN VIEW AT INTERIOR CORNER



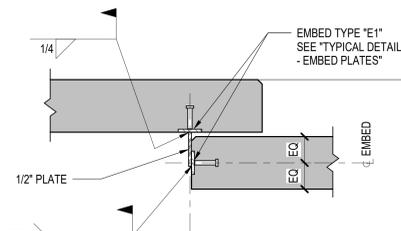
PLAN VIEW AT INTERIOR CORNER

- NOTES:
1. CONNECT PANELS AT 3'-0" BELOW ROOF AND AT PANEL MID-HEIGHT, U.N.O.
2. MAKE CONNECTION PRIOR TO REMOVING STRONG-BACKS AND BRACES.

11 TYPICAL DETAIL CONNECTION OF TILT-UP PANELS AT OFFSET CORNER 1" = 1'-0"



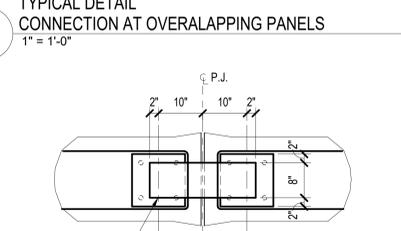
PLAN VIEW AT INTERIOR CORNER



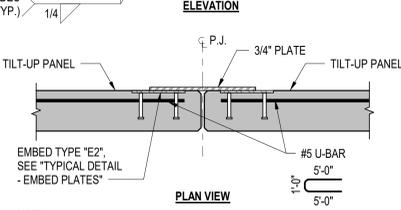
PLAN VIEW OVERLAPPING PANELS

- NOTES:
1. CONNECT PANELS AT 3'-0" BELOW ROOF AND AT PANEL MID-HEIGHT, U.N.O.
2. MAKE CONNECTION PRIOR TO REMOVING STRONG-BACKS AND BRACES.

10 TYPICAL DETAIL CONNECTION AT OVERLAPPING PANELS 1" = 1'-0"



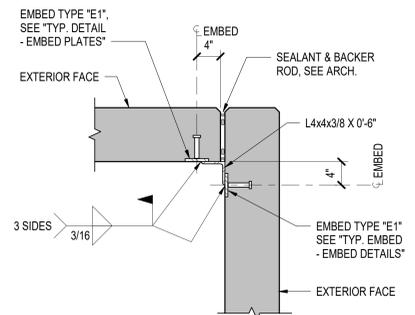
ELEVATION



PLAN VIEW

- NOTE:
PROVIDE PANEL TO PANEL CONNECTION AS INDICATED IN THE PANEL ELEVATIONS

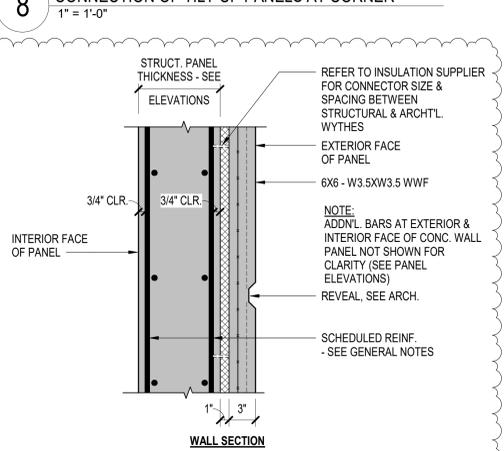
9 PANEL TO PANEL CONNECTION 3/4" = 1'-0"



PLAN VIEW AT CORNER

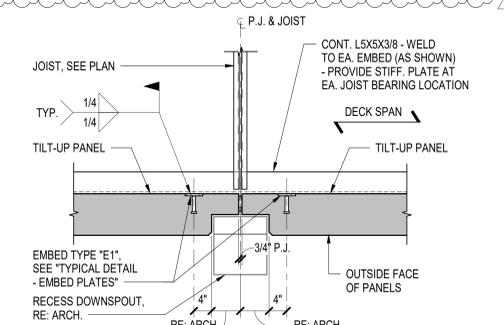
- NOTES:
1. CONNECT PANELS AT 3'-0" BELOW ROOF AND AT PANEL MID-HEIGHT, U.N.O.
2. MAKE CONNECTION PRIOR TO REMOVING STRONG-BACKS AND BRACES.

8 TYPICAL DETAIL CONNECTION OF TILT-UP PANELS AT CORNER 1" = 1'-0"



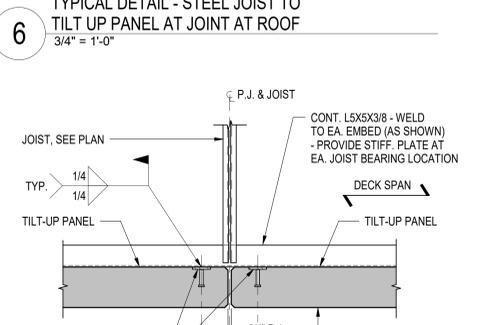
WALL SECTION

7 TYPICAL DETAIL - INSULATED TILT-UP PANEL REINFORCEMENT & PLACEMENT 1 1/2" = 1'-0"



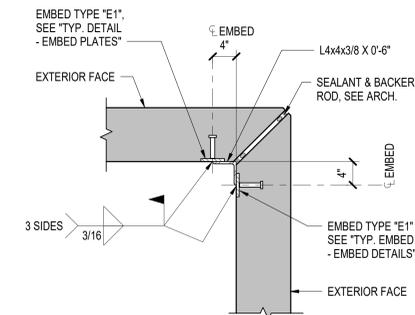
- NOTE:
1. L5x5x3/8 SHALL NOT BE SPICED AT JOISTS AT PANEL JOINTS.
2. EDGE ANGLE NOT SHOWN FOR CLARITY.
3. THIS DETAIL TO BE USED FOR ALL JOIST WITHIN 6" OF A PANEL JOINT.
4. BEARING ANGLE SHALL BE DISCONTINUOUS AT ROOF EXPANSION JOINT LOCATIONS ONLY, U.N.O..

6 TYPICAL DETAIL - STEEL JOIST TO TILT UP PANEL AT JOINT AT ROOF 3/4" = 1'-0"



- NOTE:
1. L5x5x3/8 SHALL NOT BE SPICED AT JOISTS AT PANEL JOINTS.
2. EDGE ANGLE NOT SHOWN FOR CLARITY.
3. THIS DETAIL TO BE USED FOR ALL JOIST WITHIN 6" OF A PANEL JOINT.
4. BEARING ANGLE SHALL BE DISCONTINUOUS AT ROOF EXPANSION JOINT LOCATIONS ONLY, U.N.O..

5 TYPICAL DETAIL - STEEL JOIST TO TILT UP PANEL AT JOINT AT ROOF 3/4" = 1'-0"



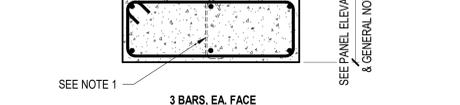
PLAN VIEW AT MITERED CORNER

- NOTES:
1. CONNECT PANELS AT 3'-0" BELOW ROOF AND AT PANEL MID-HEIGHT, U.N.O.
2. MAKE CONNECTION PRIOR TO REMOVING STRONG-BACKS AND BRACES.

4 TYPICAL DETAIL CONNECTION OF TILT-UP PANELS AT CORNER 1" = 1'-0"



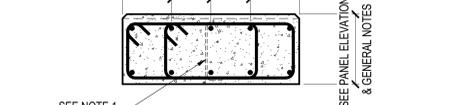
3 BARS, EA. FACE



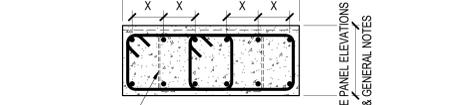
4 BARS, EA. FACE



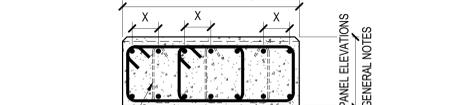
5 BARS, EA. FACE



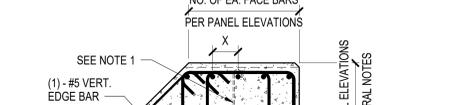
6 BARS, EA. FACE



7 BARS, EA. FACE



NO. OF EA. FACE BARS PER PANEL ELEVATIONS

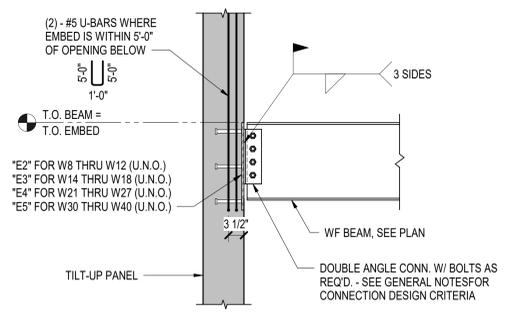


MITERED PANEL LEG

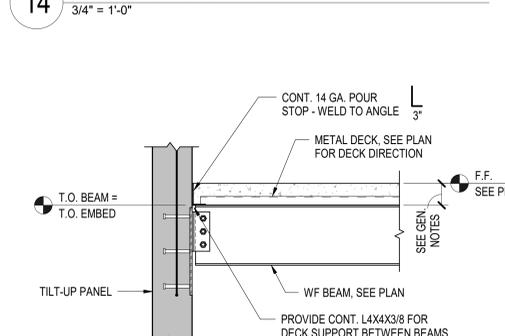


- NOTE:
1. CROSS TIES MUST BE INCLUDED WHEN X > 6".
2. ALL REINFORCING SHOWN ON THESE PLAN DETAILS IS IN ADDITION TO REINFORCING REQUIREMENTS SHOWN IN THE GENERAL NOTES U.N.O.
3. FOR MINIMUM COVER TO TIES, SEE GENERAL NOTES.
4. ALTERNATE HOOKS WHEN PLACING SUCCESSIVE SETS OF CROSS TIES.
5. ALL TIES ARE #3 @ 36" O.C. U.N.O. ON PANEL ELEVATIONS.

1 TYPICAL DETAIL ADDITIONAL REINFORCING AT PANEL LEGS 1" = 1'-0"



14 TYPICAL DETAIL WF BEAM TO TILT-UP PANEL 3/4" = 1'-0"



13 TYPICAL DETAIL WF BEAM TO TILT-UP PANEL AT FLOOR 3/4" = 1'-0"



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Louisiana Firm License #4141

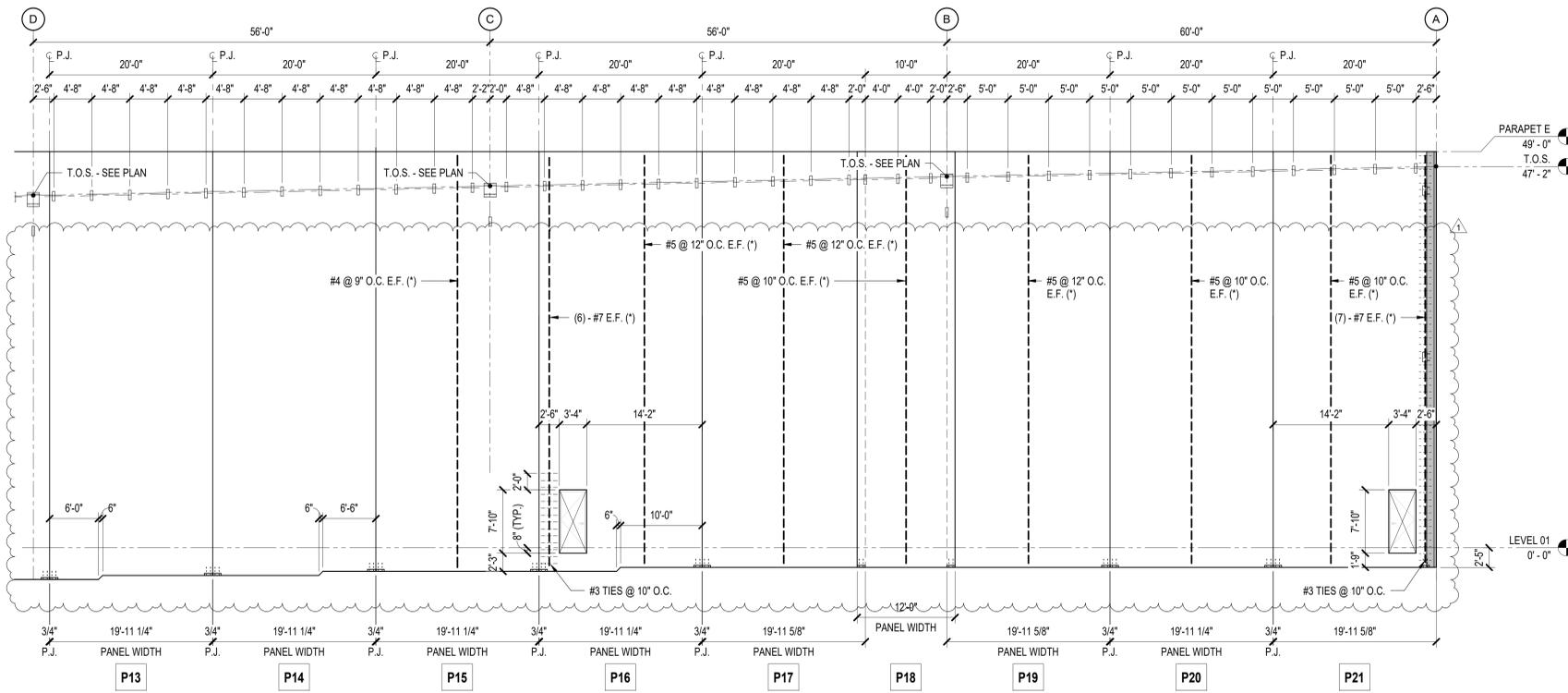
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1	03/16/26	ADDENDUM 1

LIPSEY'S NEW HEADQUARTERS

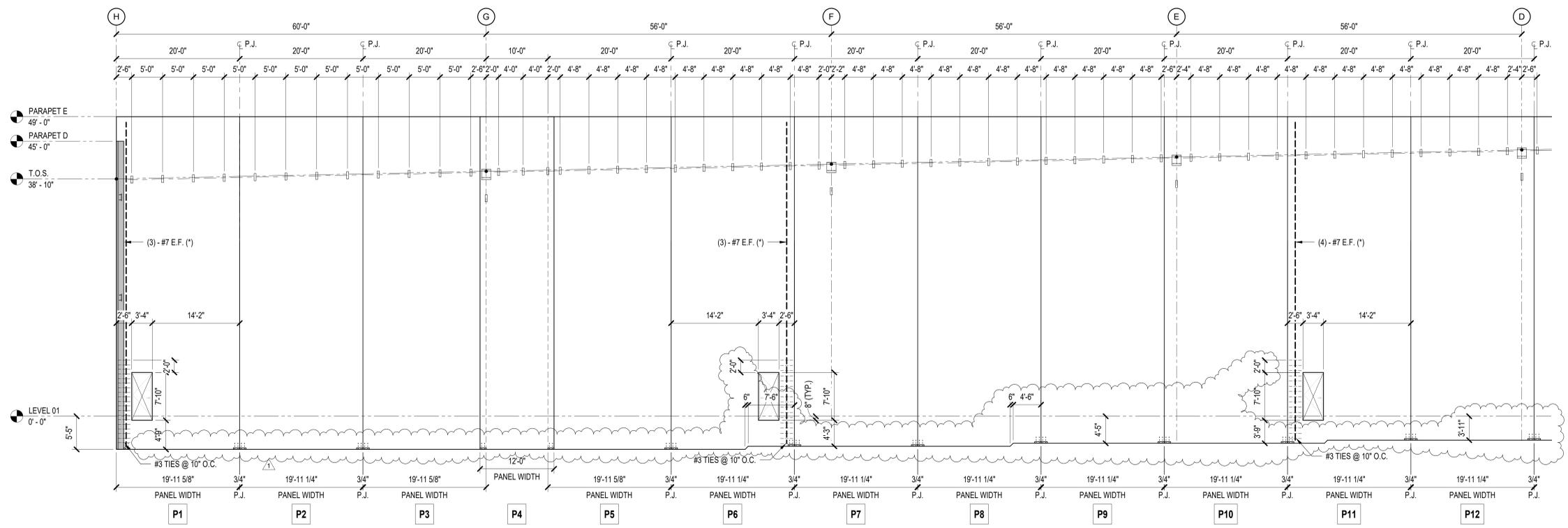
HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

PANEL DETAILS



4 PANEL ELEVATION
1/8" = 1'-0"



3 PANEL ELEVATION
1/8" = 1'-0"

- GENERAL NOTES
1. ALL EMBED PLATES ARE TYPE "E1", U.N.O.
 2. SEE DETAIL 4/S501 FOR EMBED PLATE SIZE & DIMENSIONS.
 3. VERIFY ALL OPENING W/ ARCHT'L DRAWINGS.
 4. SEE TILT-UP WALL PANEL SECTION OF THE GENERAL NOTES FOR TYPICAL PANEL REINFORCING.
 5. INSTALL EMBEDS SHOWN IN DETAILS & ON PANEL ELEVATIONS.
 6. PROVIDE GALV. STEEL EMBED AT O.H. DOORS, SEE 6/S501 FOR INFORMATION.
 7. SEE SHEET S503 AND S504 FOR REINFORCEMENT PLACEMENT.
 8. TYPICAL NON-COMPOSITE PANEL THICKNESS IS 14 1/4", U.N.O.
 9. LOCATIONS NOTED THUS: (*) INDICATES THE VERTICAL REINFORCING NOTED SHALL REPLACE THE TYPICAL VERTICAL REINFORCING REQUIRED PER THE GENERAL NOTES.
 10. TYPICAL REINFORCING SHALL RUN THROUGH FUTURE KNOCK-OUT OPENINGS.

RITTER MAHER ARCHITECTS
2987 Government Street (Baton Rouge, LA 70806
p 225.383.4321
RITTERMAHER.COM

Pinnacle STRUCTURAL ENGINEERS
3120 Southwest Freeway, Suite 410
Houston, TX 77098
713.807.8811 voice
25105

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KEY PLANS:

SEAL:

HEATH H. MICHEL
License No. 39281
PROFESSIONAL ENGINEER
IN
LOUISIANA
Pinnacle Structural Engineers
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO.	DATE	DESCRIPTION
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1	03/16/26	ADDENDUM 1

HIGHLAND RD
PROJECT ADDRESS

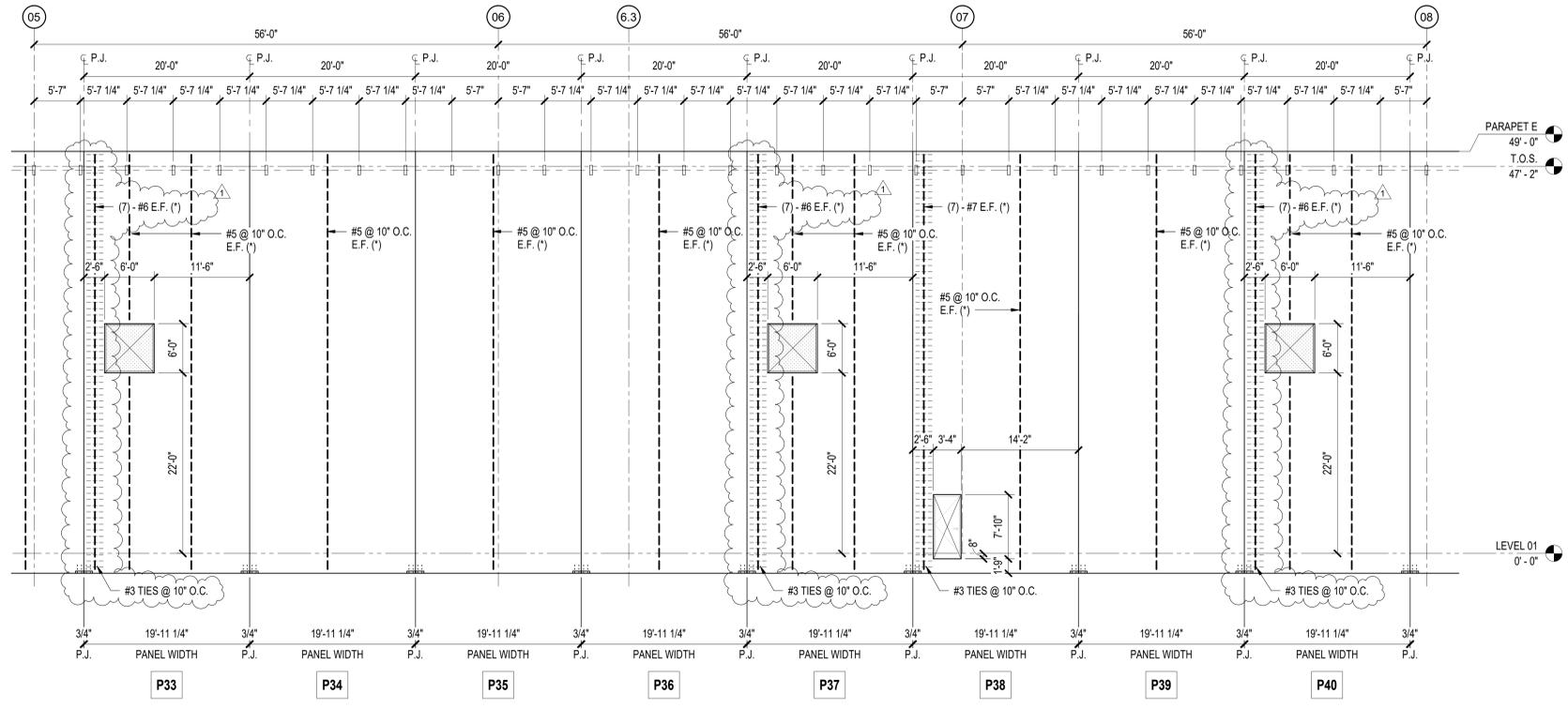
RMA PROJECT NO: 251172

PANEL ELEVATIONS

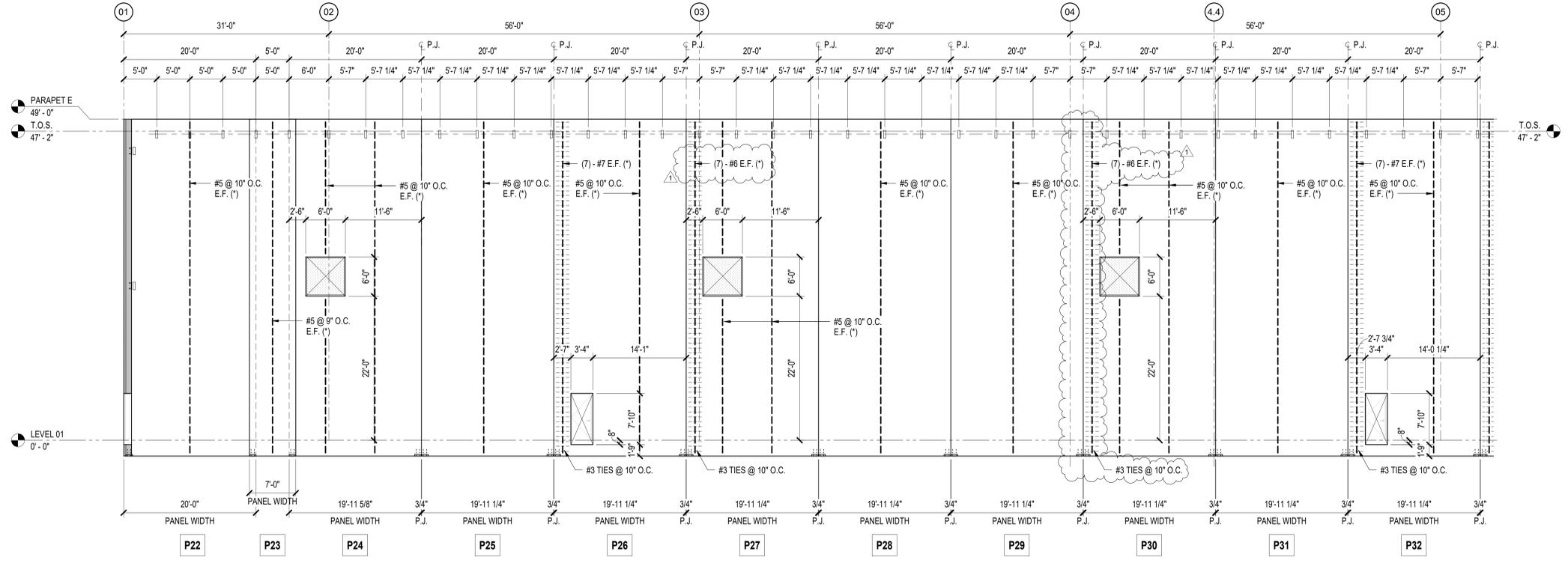
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 10. TYPICAL REINFORCING SHALL RUN THROUGH FUTURE KNOCK-OUT OPENINGS.



4 PANEL ELEVATION
1/8" = 1'-0"



3 PANEL ELEVATION
1/8" = 1'-0"



REVISION SCHEDULE		
NO.	DATE	DESCRIPTION
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HIGHLAND RD

PROJECT ADDRESS

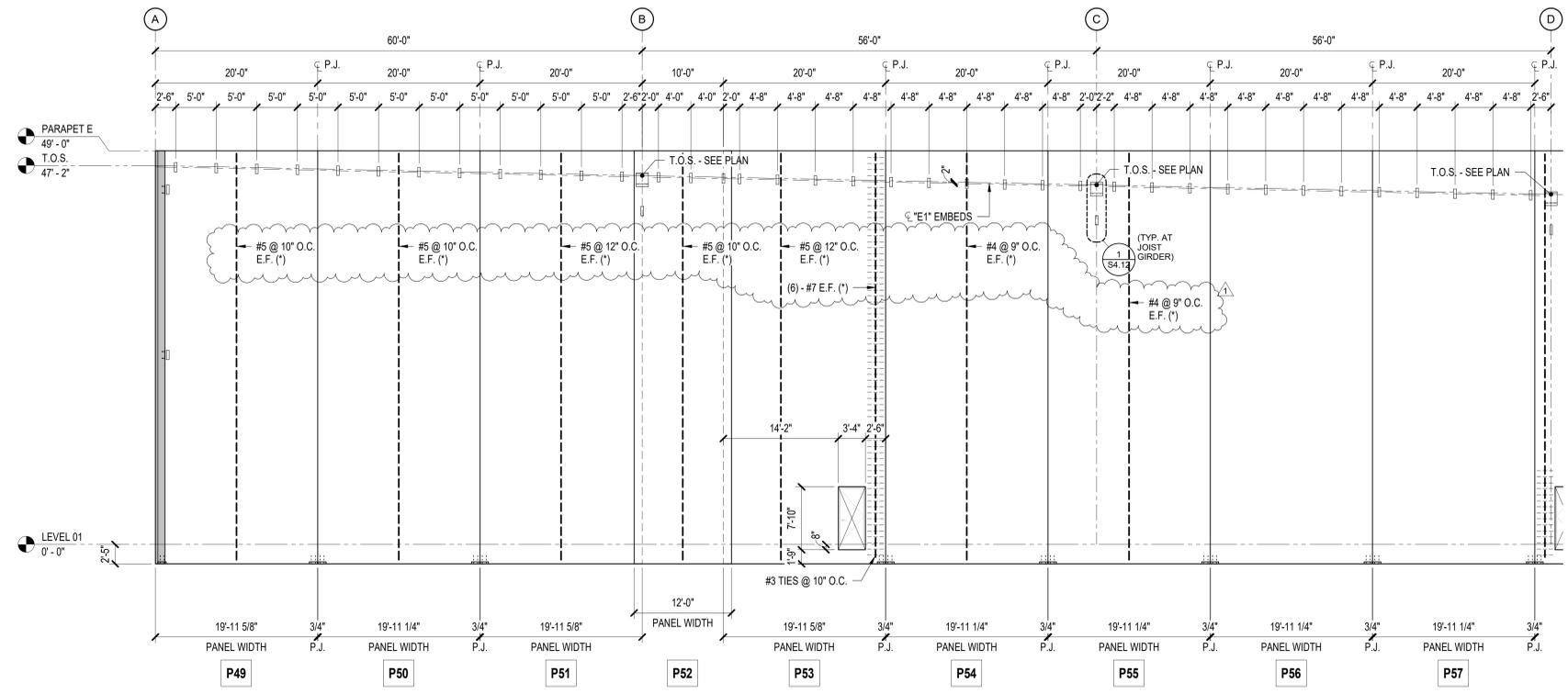
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PANEL ELEVATIONS

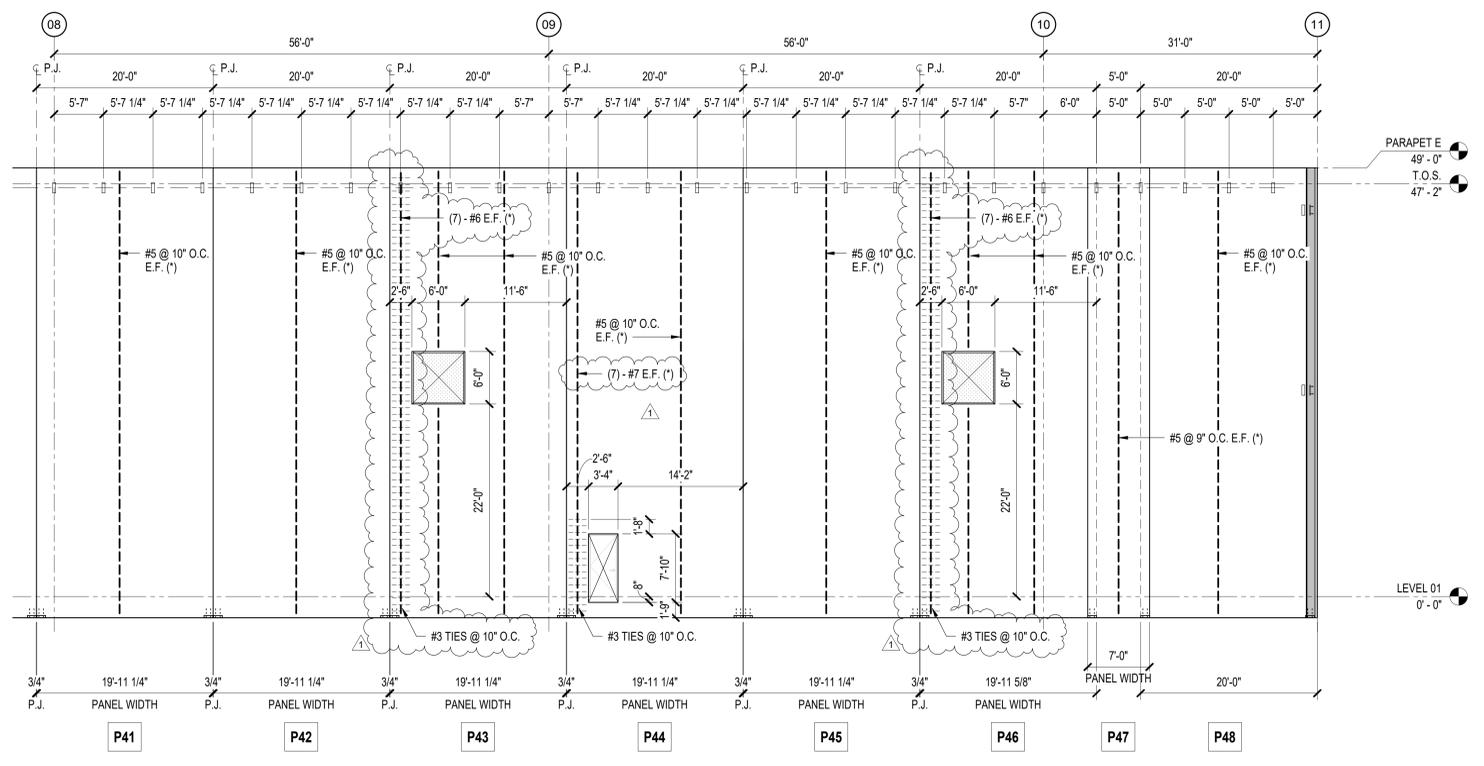
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4 PANEL ELEVATION
1/8" = 1'-0"



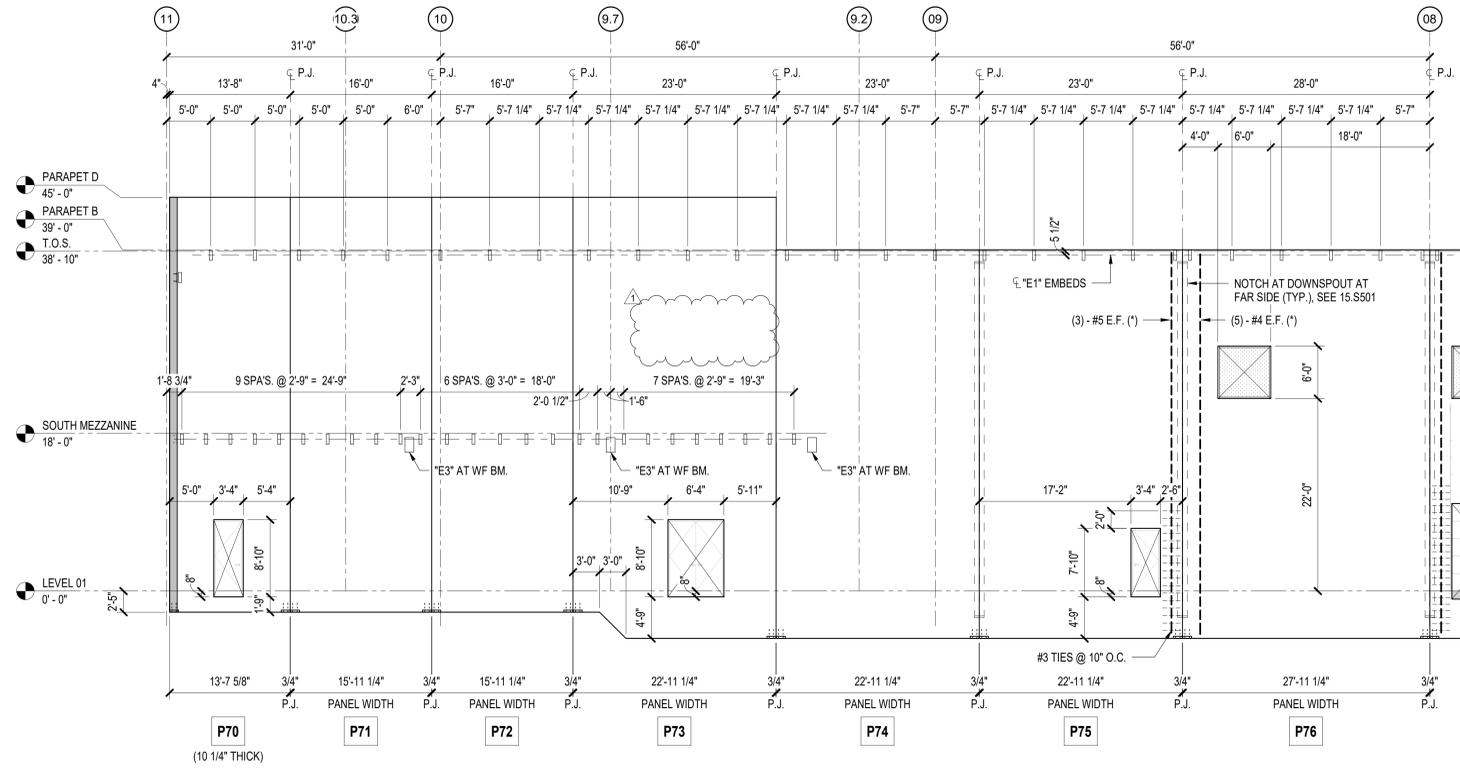
3 PANEL ELEVATION
1/8" = 1'-0"



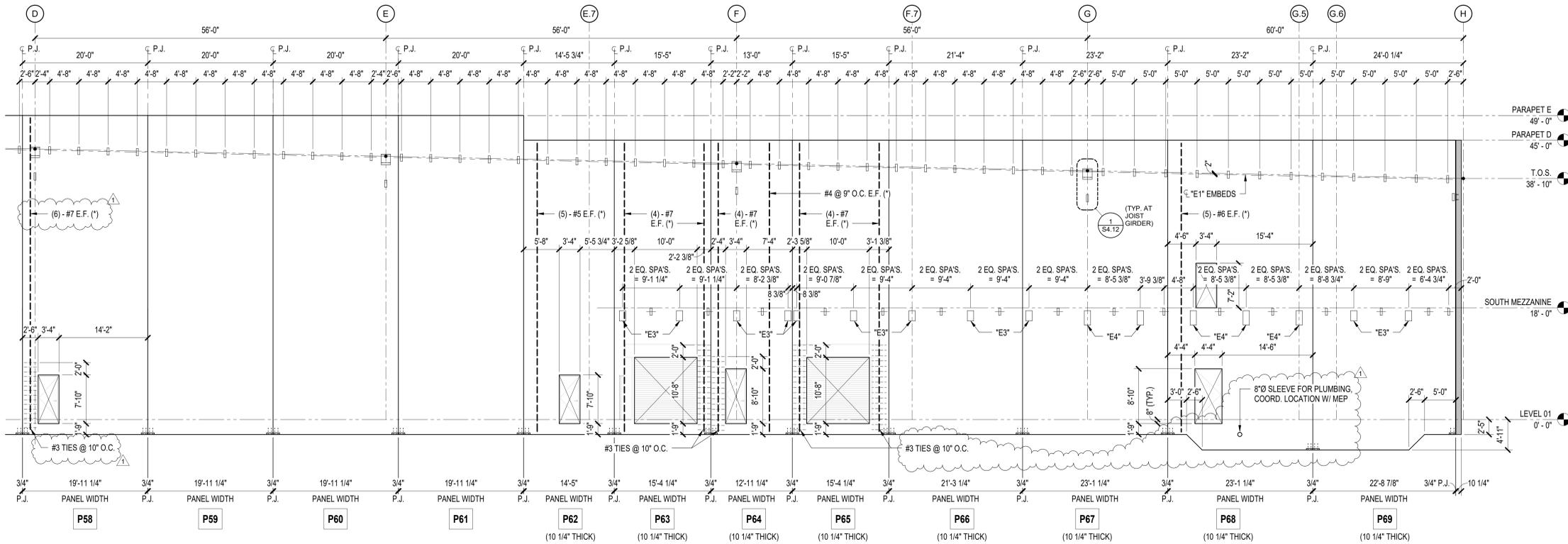
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HIGHLAND RD
PROJECT ADDRESS
RMA PROJECT NO: 251172

PANEL ELEVATIONS



4 PANEL ELEVATION
1/8" = 1'-0"



3 PANEL ELEVATION
1/8" = 1'-0"

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KEY PLANS

SEAL

STATE OF LOUISIANA
HEATH H. MICHEL
License No. 39291
PROFESSIONAL ENGINEER
IN
STRUCTURAL ENGINEERING
Pinnacle Structural Engineers
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

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HIGHLAND RD
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RMA PROJECT NO: 251172

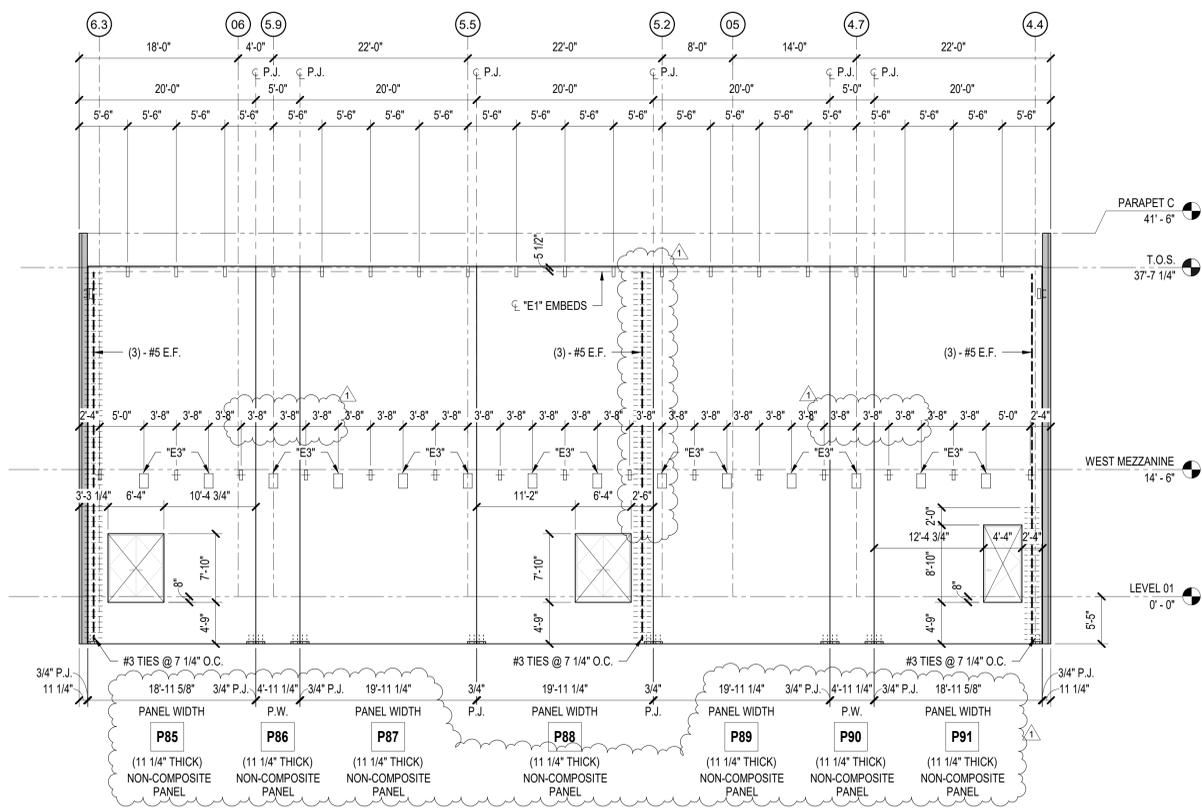
PANEL ELEVATIONS

GENERAL NOTES

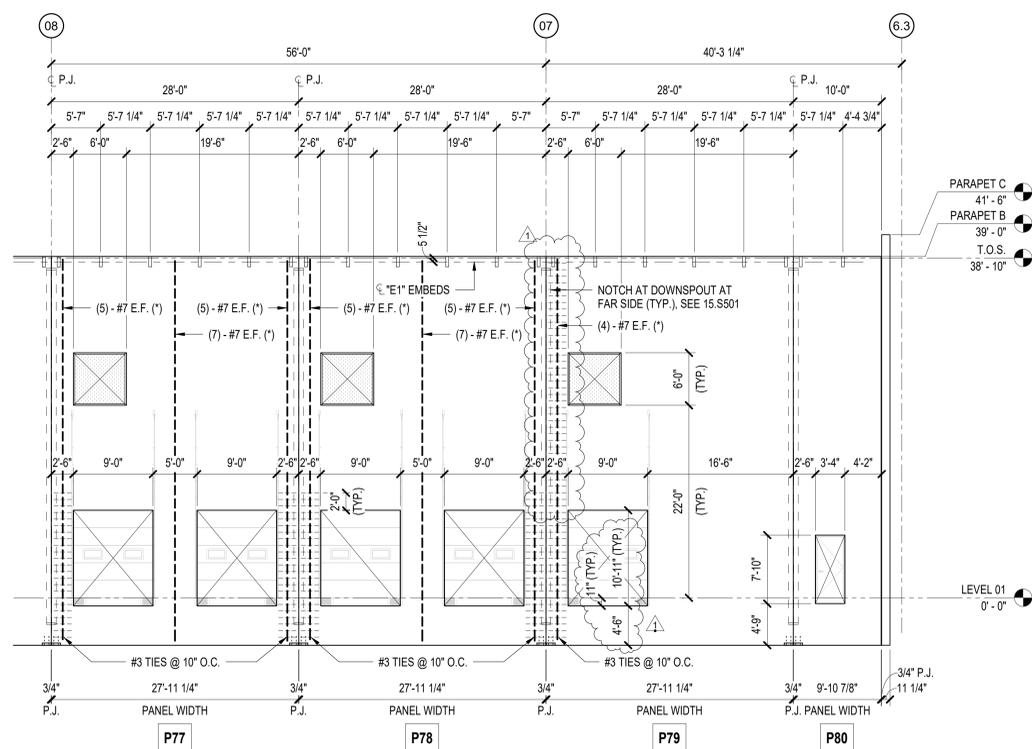
1. ALL EMBED PLATES ARE TYPE "E1", U.N.O.
2. SEE DETAIL 4/S501 FOR EMBED PLATE SIZE & DIMENSIONS.
3. VERIFY ALL OPENING W/ ARCHT'L DRAWINGS.
4. SEE TILT-UP WALL PANEL SECTION OF THE GENERAL NOTES FOR TYPICAL PANEL REINFORCING.
5. INSTALL EMBEDS SHOWN IN DETAILS & ON PANEL ELEVATIONS.
6. PROVIDE GALV. STEEL EMBED AT O.H. DOORS, SEE 6/S501 FOR INFORMATION.
7. SEE SHEET S503 AND S504 FOR REINFORCEMENT PLACEMENT.
8. TYPICAL NON-COMPOSITE PANEL THICKNESS IS 14 1/4", U.N.O.
9. LOCATIONS NOTED THUS: (*) INDICATES THE VERTICAL REINFORCING NOTED SHALL REPLACE THE TYPICAL VERTICAL REINFORCING REQUIRED PER THE GENERAL NOTES.
10. TYPICAL REINFORCING SHALL RUN THROUGH FUTURE KNOCK-OUT OPENINGS.

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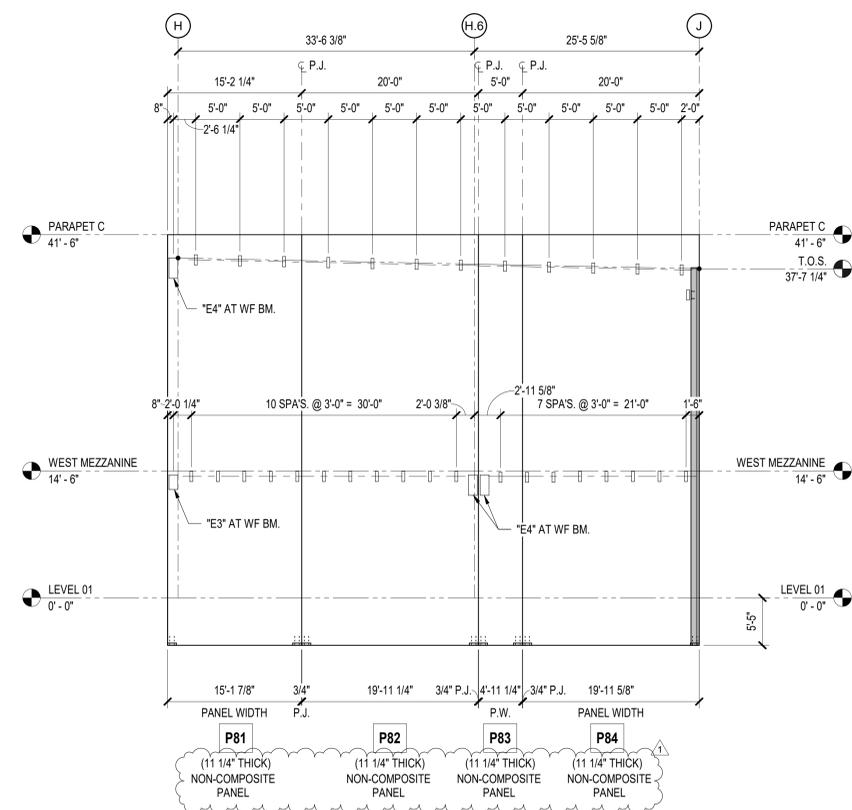
KEY PLANS



4 PANEL ELEVATION
1/8" = 1'-0"



3 PANEL ELEVATION
1/8" = 1'-0"



1 PANEL ELEVATION
1/8" = 1'-0"



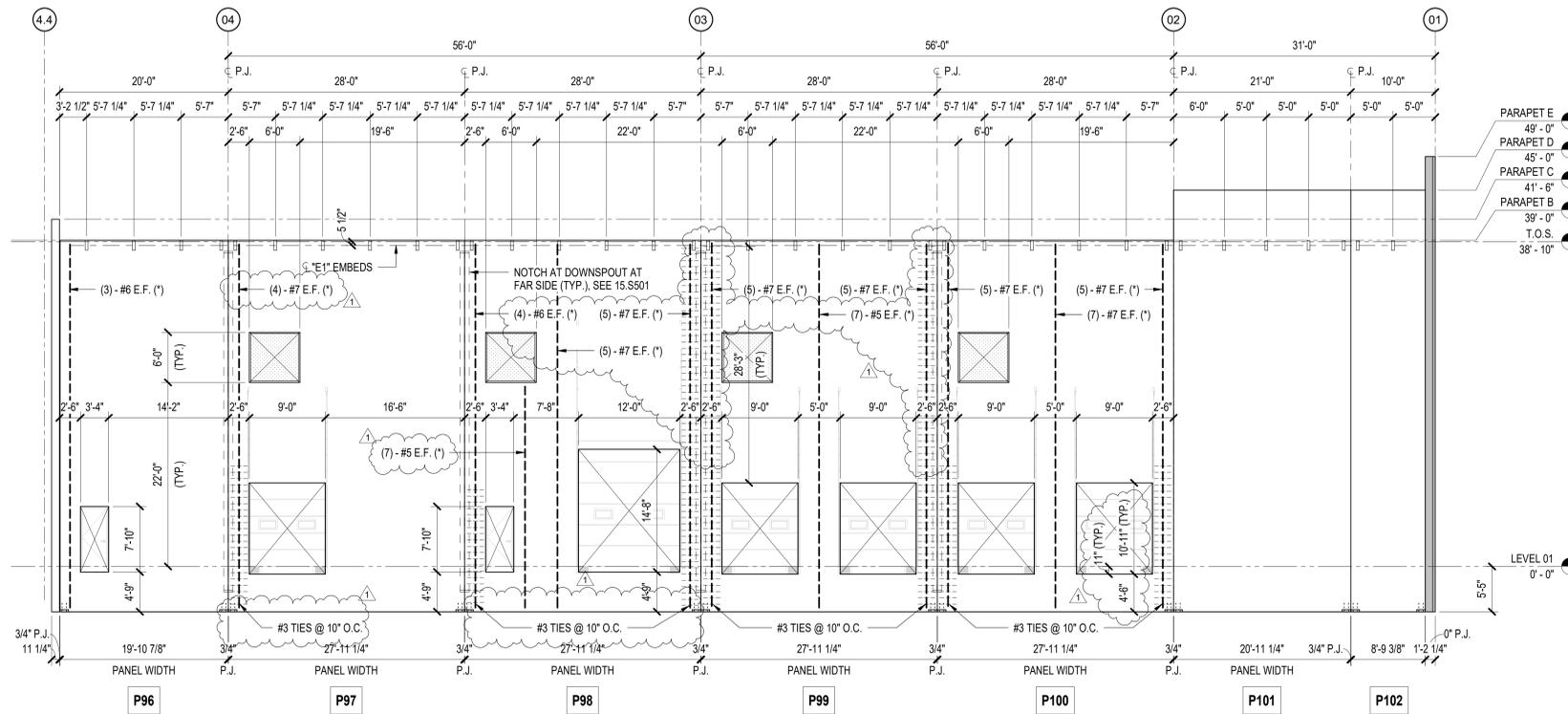
REVISION SCHEDULE	
NO.	DATE DESCRIPTION
02/27/26	ISSUE FOR BID
1	03/16/26 ADDENDUM 1

HIGHLAND RD

PROJECT ADDRESS

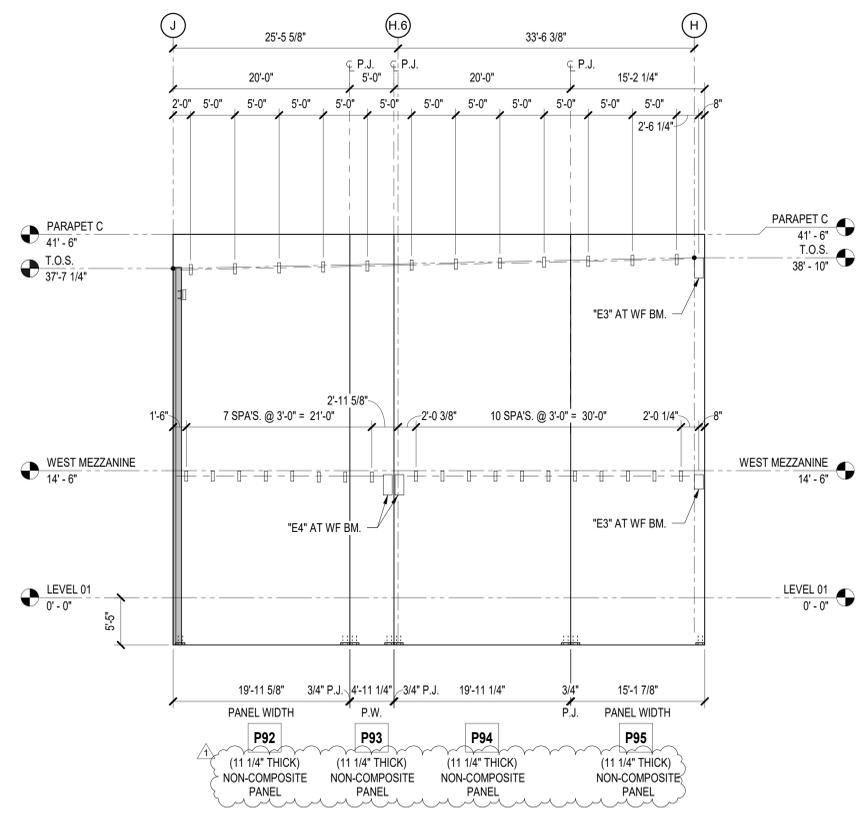
RMA PROJECT NO: 251172

PANEL ELEVATIONS



- PARAPET E 49'-0"
- PARAPET D 45'-0"
- PARAPET C 41'-6"
- PARAPET B 39'-0"
- T.O.S. 38'-10"
- LEVEL 01 0'-0"

4 PANEL ELEVATION
1/8" = 1'-0"



- PARAPET C 41'-6"
- T.O.S. 37'-7 1/4"
- PARAPET C 41'-6"
- T.O.S. 38'-10"
- LEVEL 01 0'-0"

1 PANEL ELEVATION
1/8" = 1'-0"

- GENERAL NOTES
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 2. SEE DETAIL 4/S501 FOR EMBED PLATE SIZE & DIMENSIONS.
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 10. TYPICAL REINFORCING SHALL RUN THROUGH FUTURE KNOCK-OUT OPENINGS.

RITTER MAHER ARCHITECTS
2987 Government Street (Baton Rouge, LA 70806)
p 225.383.4321
RITTERMAHER.COM

Pinnacle STRUCTURAL ENGINEERS
3120 Southwest Freeway, Suite 410
Houston, TX 77098
713.807.8911 voice
25105

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KEY PLANS:

SEAL:

HEATH H. MICHEL
License No. 39291
PROFESSIONAL ENGINEER
IN
LOUISIANA
Pinnacle Structural Engineers
LOUISIANA FIRM LICENSE #4141

NO.	DATE	DESCRIPTION
02/27/26		ISSUE FOR BID
1	03/16/26	ADDENDUM 1

HIGHLAND RD
PROJECT ADDRESS
RMA PROJECT NO: 251172

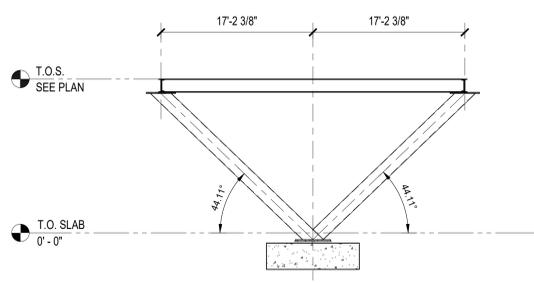
PANEL ELEVATIONS

GENERAL NOTES

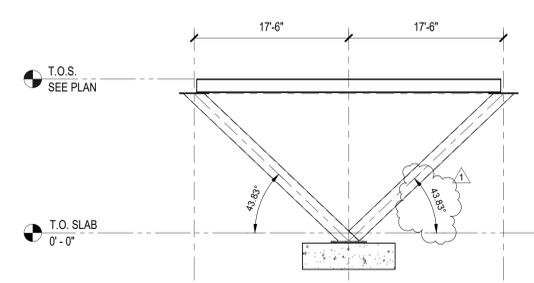
- REFER TO BRACE ELEVATIONS FOR BRACE SIZES & FORCES.
- IF NOT PROVIDED, U.N.O., THE BRACE FORCE SHALL BE THE MAXIMUM TENSION AND/OR COMPRESSION CAPACITY OF THE BRACE, IF BRACE FORCES ARE SHOWN, THEY ARE FACTORED LOADS (LRFD).
- GUSSET PLATES SHALL BE CENTERED ON STEEL BEAM & DESIGNED TO DEVELOP BRACE FORCE.
- BOLTS & WELDS SHALL BE DESIGNED BY THE FABRICATOR FOR THE VERTICAL & THE HORIZONTAL COMPONENTS OF THE BRACE FORCE INCLUDING THE EFFECT OF THE ECCENTRICITY. THE MINIMUM SIZE FILLETWELD SHALL BE 1/4".

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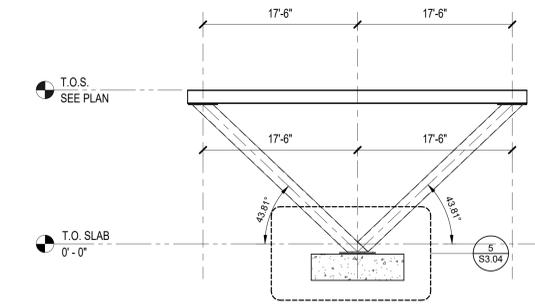
KEY PLAN:



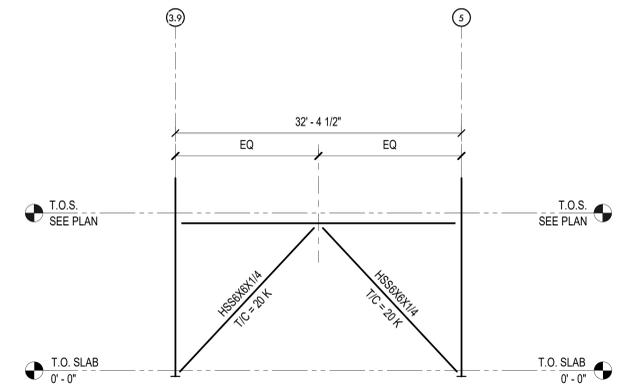
16 SLANTED COLUMN ELEVATION
1/8" = 1'-0"



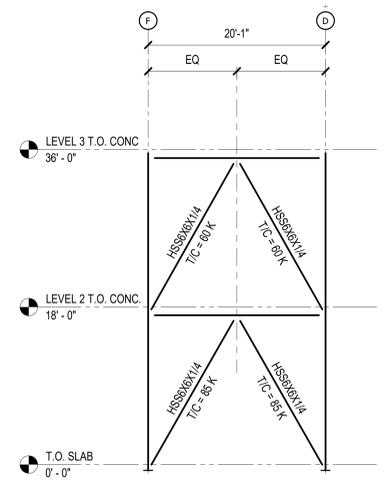
12 SLANTED COLUMN ELEVATION
1/8" = 1'-0"



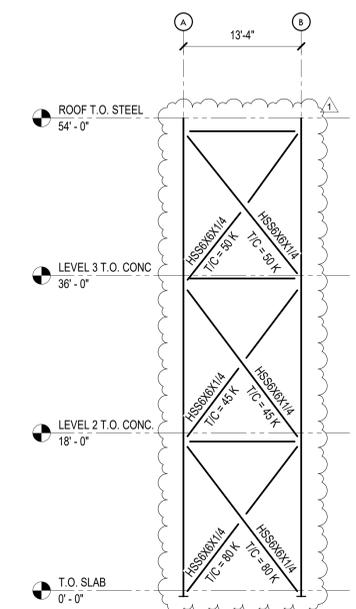
8 SLANTED COLUMN ELEVATION
1/8" = 1'-0"



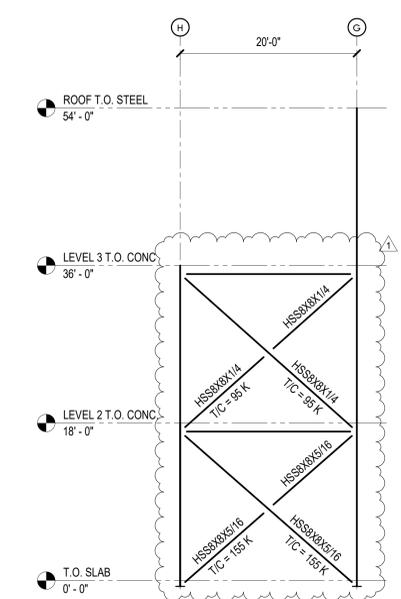
15 BRACE J
1/8" = 1'-0"



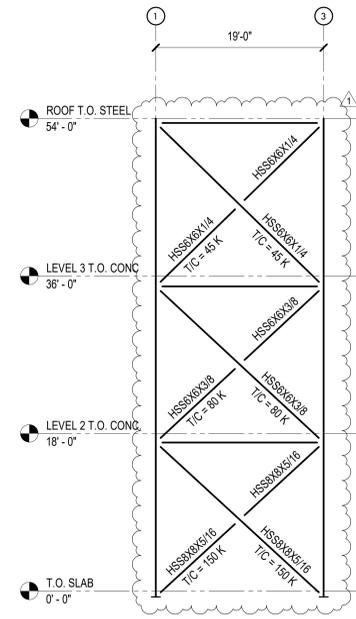
11 BRACE H
1/8" = 1'-0"



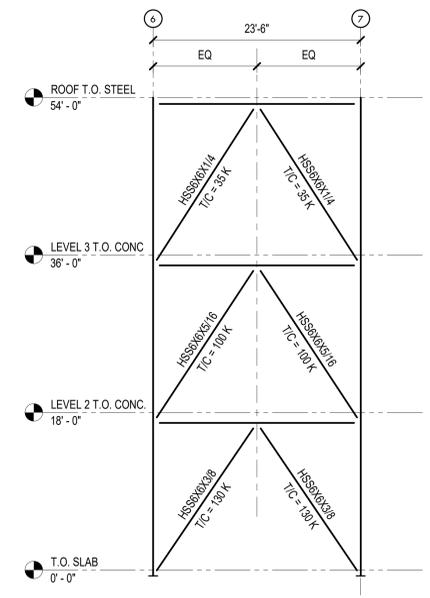
6 BRACE G
1/8" = 1'-0"



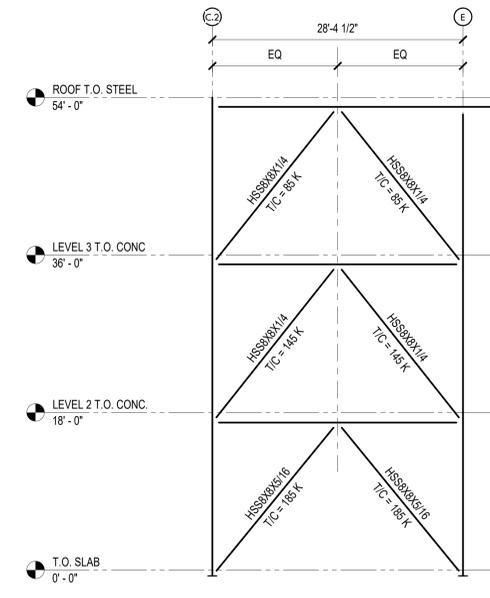
3 BRACE F
1/8" = 1'-0"



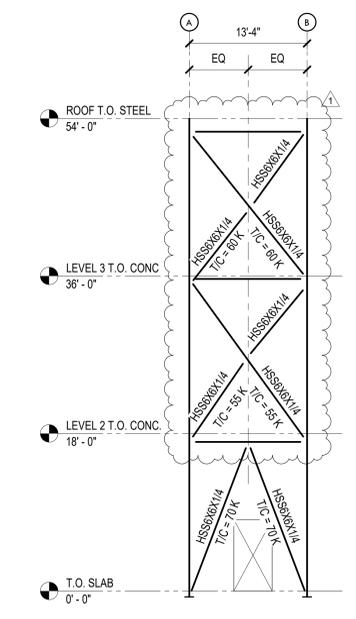
17 BRACE E
1/8" = 1'-0"



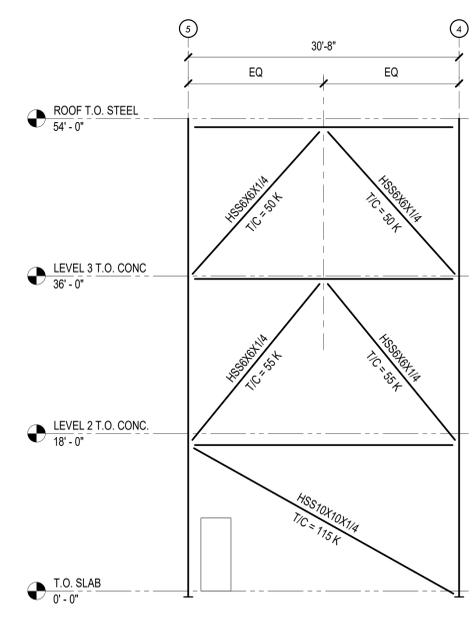
13 BRACE D
1/8" = 1'-0"



9 BRACE C
1/8" = 1'-0"



4 BRACE B
1/8" = 1'-0"



1 BRACE A
1/8" = 1'-0"



NO.	DATE	DESCRIPTION
12/15/25	50% DD	
01/14/26	100% DD	
02/27/26	ISSUE FOR BID	
1 03/16/26	ADDENDUM 1	

LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

BRACE ELEVATIONS & DETAILS