LIPSEY'S NEW HEADQUARTERS

HIGHLAND ROAD
ST. GEORGE, LA 70809

VOLUME 1
ISSUED FOR BID



PROJECT DIRECTORY

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VICINITY MAP, N.T.S.



PROJECT DESCRIPTION

THE PROJECT NEW CONSTRUCTION FOR LIPSEY'S. THE PROJECT CONSISTS OF A ONE STORY WAREHOUSE BUILDING WITH AN ADJACENT, THREE STORY OFFICE BUILDING. BOTH BUIDLINGS ARE TYPE IIB CONSTRUCTION, FULLY SPRINKLERED, THE BUILDINGS ARE SEPARATED BY A 3 HOUR RATED FIRE WALL. THE WAREHOUSE UTILIZES TILT-WALL CONSTRUCTION AND THE OFFICE UTILIZES STRUCUTURAL STEEL FRAME CONSTRUCTION.

THE PROJECT IS DIVIDED INTO TWO VOLUMES. VOLUME 1 IS A SITE PACKAGE COMPRISED OF THE CIVIL AND STRUCTURAL SCOPE. VOLUME 2 IS A BUILDING PACKAGE AND IS COMPRISED OF ARCHITECTURAL (OFFICE AND WAREHOUSE), FIRE PROTECTION, PLUMBING, MECHANICAL, ELECTRICAL, AND LANDSCAPE.

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ARKEL Constructors

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KEY PLAN:

SEAL:

STEPHEN P. MAHER
REG. No. 32018
STATE OF LOUISIANA
REGISTERED ARCHITECT

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	11.18.2025	SD
	01.16.2026	100% DD
	02.27.2026	ISSUED FOR BID

LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

TITLE SHEET

G0.00.V1

GENERAL NOTES:

1. THE LOCATION OF ALL UNDERGROUND UTILITIES SHOWN ON THE PLANS IS APPROXIMATE ONLY. PRIOR TO BEGINNING ANY CONSTRUCTION, THE CONTRACTOR SHALL CONTACT "LOUISIANA ONE CALL", THE APPROPRIATE UTILITY COMPANIES, AND THE OWNER. CONTRACTOR SHALL BE CHARGED WITH THE RESPONSIBILITY OF PHYSICALLY VERIFYING THE EXACT LOCATION, DEPTH OR HEIGHT OF ALL UNDERGROUND AND OVERHEAD UTILITIES. THE CONTRACTOR SHALL CONTACT LOUISIANA ONE CALL OR THE APPROPRIATE UTILITY COMPANIES FOR THE LOCATION OF UNDERGROUND SERVICES A MINIMUM OF 48 HOURS PRIOR TO BEGINNING OF CONSTRUCTION THE WORK AREA.

2. THE CONTRACTOR IS WARNED THAT EXISTING OVERHEAD AND UNDERGROUND UTILITIES INCLUDING BUT NOT LIMITED TO ELECTRICAL LINES & POLES, DUCTS, CABLES, TELEPHONE CABLES, GAS LINES, WATER LINES, AND SANITARY SEWER LINES EXIST IN THE GENERAL AREA AND/OR RIGHTS OF WAY WHERE THE PROPOSED IMPROVEMENTS ARE TO BE CONSTRUCTED. THESE PLANS DO NOT WARRANT THE EXISTENCE OR NON-EXISTENCE OF UTILITIES OF CONFLICTS. IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO IDENTIFY, TO LOCATE AND TO PROTECT ALL THE EXISTING UTILITIES IN THE FIELD AND TO TAKE NECESSARY PRECAUTIONS TO PREVENT ANY DAMAGE OR INTERRUPTION IN SERVICE TO ANY UTILITIES OR CONFLICTS LOCATED WITHIN THE PROJECT LIMITS. ANY DAMAGE TO EXISTING CAUSED BY THE EXECUTION OF THE WORK UNDER THIS CONTRACT SHALL BE REPAIRED BY THE CONTRACTOR AT HIS EXPENSE. THE OWNER DOES NOT ASSUME OR IMPLY TO ANY LIABILITY FOR THE LOCATION, PROTECTION, AND/OR REPAIR OF EXISTING UTILITIES THAT MAY OCCUPY JOINT RIGHT-OF-WAY, THE WORK AREA, OR OTHERWISE CONFLICT WITH THE CONSTRUCTION OF THE WORK TO BE INSTALLED UNDER THIS CONTRACT.

3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING THE OWNER WITH PRECONSTRUCTION PHOTOGRAPHS OR VIDEOS OF ALL CONSTRUCTION AREAS PRIOR TO BEGINNING ANY WORK. ALL COSTS SHALL BE INCLUDED IN THE ITEM FOR MOBILIZATION.

4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL NECESSARY TEMPORARY CONSTRUCTION SIGNS, BARRICADES, AND RELATED DEVICES. ALL COSTS SHALL BE INCLUDED IN THE ITEM FOR MOBILIZATION.

5. THE CONTRACTOR SHALL COMPLY WITH ALL REQUIREMENTS OF THE OCCUPATIONAL SAFETY AND HEALTH (OSHA) REGULATIONS DURING THE PERFORMANCE OF THIS PROJECT. SAFETY IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE ENGINEER AND THE OWNER ARE NOT RESPONSIBLE FOR SAFETY IN, ON, OR ABOUT THE PROJECT SITE, NOR FOR COMPLIANCE BY THE APPROPRIATE PARTY WITH ANY REGULATIONS RELATION THERETO.

6. ALL CONSTRUCTION SHALL CONFORM TO THE EAST BATON ROUGE PARISH STANDARDS, PROJECT PLANS AND CONTRACT DOCUMENTS AND BE SUBJECT TO OBSERVATION BY THE OWNER'S REPRESENTATIVES. CONTRACTOR SHALL HAVE ONE (1) SIGNED COPY OF PROJECT PLANS AND A COMPLETE SET OF CONTRACT DOCUMENTS ON THE JOB SITE AT ALL TIMES.

7. THE CONTRACTOR SHALL NOTIFY FIBEROPTICS COMPANIES SEVEN (7) WORKING DAYS PRIOR TO ANY CONSTRUCTION ACTIVITY IN THEIR AREA. EXTREME CAUTION SHALL BE USED IN AREAS WHERE FIBEROPTIC CABLE IS LOCATED ADJACENT TO CONSTRUCTION ACTIVITY.

8. THE CONTRACTOR SHALL OBTAIN AT HIS EXPENSE ALL PERMITS AND INSPECTIONS THAT ARE REQUIRED TO PERFORM THE WORK. PERMITS SHALL BE OBTAINED AND INSPECTIONS SCHEDULED A MINIMUM OF 48 HOURS IN ADVANCE OF CONSTRUCTION

9. A PERMIT FOR DISCHARGES OF STORM WATER FROM CONSTRUCTION ACTIVITIES MAY BE REQUIRED FOR THIS PROJECT. PRIOR TO ANY CLEARING, GRUBBING, STRIPPING, STOCKPILING, EXCAVATION, EMBANKMENT OR OTHER GRADING ACTIVITIES, THE OWNER SHALL BE PROVIDED WITH AN LDEQ APPROVED COPY OF THE CONTRACTOR'S STORM WATER MANAGEMENT PLAN AND PERMIT FOR THE PROJECT IF REQUIRED.

10. SEDIMENT AND EROSION CONTROLS SHALL BE INSTALLED TO MEET THE PERMIT REQUIREMENTS, CITY/PARISH OF EAST BATON ROUGE SPECIFICATIONS AND STANDARD PLANS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO CHECK AND FIX THE DEVICES AFTER EVERY MAJOR STORM EVENT AND TO REMOVE THE TEMPORARY CONTROLS AFTER DISTURBED AREAS ARE FINALLY STABILIZED.

11. THE CONTRACTOR SHALL IMMEDIATELY REPAIR ALL DAMAGE TO ANY FACILITY, EQUIPMENT, CABLE, WIRE, PAVEMENT, OR OTHER STRUCTURE CAUSED BY HIS OPERATION WITH ACCEPTABLE SUITABLE MATERIALS, TO ORIGINAL (PRE-CONSTRUCTION) OR BETTER CONDITION AND TO THE SATISFACTION OF THE ENGINEER, AT NO COST TO THE OWNER.

12. ALL DISTURBED NON-PAVED AREAS NOT SHOWN ON THE LANDSCAPE PLANS SHALL BE SLAB SODDED AS DIRECTED BY ENGINEER. REFER TO THE LANDSACPE PLANS FOR ALL OTHER VEGETATIVE COVER.

13. WATER REQUIRED IN THIS PROJECT UNDER LADOTD SPECIFICATION SECTIONS 717, 718, 739 TO MAINTAIN THE SOD WILL NOT BE MEASURED OR PAID. (NO DIRECT PAY.)

14. ACCESS TO ALL EXISTING FACILITIES SHALL BE MAINTAINED AT ALL TIMES

15. OBSERVATIONS OF THE WORK IN PROGRESS AND/OR FIELD TESTING PERFORMED BY THE OWNER OR ENGINEER SHALL IN NO WAY EXCUSE THE CONTRACTOR FOR DEFECTS DISCOVERED IN HIS WORK.

16. COMPACTIONS TESTING MAY BE DONE PERIODICALLY BY THE OWNER. THESE TESTS ARE INTENDED TO PROVIDE THE OWNER A GREATER DEGREE OF ASSURANCE THAT THE CONTRACTOR IS COMPLYING WITH COMPACTION REQUIREMENTS. THE CONTRACTOR IS NOT TO RELY UPON THESE TESTS FOR FILL CONTROL, NOR ARE THESE TESTS TO BE CONSTRUED AS A GUARANTEE BY THE OWNER OF THE CONTRACTORS CONTRACTUAL OBLIGATION.

17. THE CONTRACTOR SHALL PROVIDE COPIES OF ALL TEST REPORTS AND INSPECTION RECORDS TO THE OWNER AND ENGINEER.

18. ALL UTILITIES SHALL BE MAINTAINED IN CONTINUOUS SERVICE THROUGHOUT CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGES OR INTERRUPTION OF SERIES RESULTING FROM CONSTRUCTION. THE CONTRACTOR WILL BE REQUIRED TO LOCATE AND MAINTAIN WATER AND SANITARY SEWER SERVICE TO EXISTING FACILITIES AT ALL TIMES AND TO RECONNECT EXISTING SERVICES TO NEW MAINS, INCLUDING PROVIDING ALL NECESSARY EQUIPMENT, VALVES, FITTINGS, PIPE AND APPURTENANCES REQUIRED TO DO SO.

19. THE CONTRACTOR SHALL PROVIDE ALL SIGNS, BARRICADES, FLAGGING, LIGHTING OF EXCAVATIONS, AND TRENCHES DURING THE PERFORMANCE OF THIS CONTRACT AS REQUIRED BY THE OWNER AT NO DIRECT PAY.

20. CONSTRUCTION STAKING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE PERFORMED BY A LOUISIANA LICENSED PROFESSIONAL LAND SURVEYOR.

21. CONTRACTOR SHALL NOT SCALE DRAWINGS FOR CONSTRUCTION. ANY MISSING DIMENSIONS OR DISCREPANCIES IN PLANS, FIELD STAKING OR PHYSICAL FEATURES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER. IF CONTRACTOR PROCEEDS WITH THE WORK WITHOUT NOTIFYING THE ENGINEER, HE DOES SO AT HIS OWN RISK.

22. THE CONTRACTOR SHALL AT ALL TIMES KEEP A SEPARATE, FULL SET OF CONTRACT DRAWINGS MARKED UP FULLY TO INDICATE AS BUILT CONDITIONS. SAID DRAWINGS SHALL BE PROVIDED TO THE OWNER & ENGINEER UPON COMPLETION OF THE WORK PRIOR TO FINAL ACCEPTANCE.

23. ANY ITEM NOT DESIGNATED TO BE REMOVED OR ANY EXISTING STRUCTURES, I.E. FENCE, GATE, PAVEMENT, LAWN, TREES, OR SHRUBBERY, AND ANY OTHER ITEM DISTURBED WITHIN PROJECT SITE SHALL BE REPLACED TO ORIGINAL CONDITION AT NO COST TO THE OWNER.

24. THE GEOTECHNICAL REPORT FOR EXISTING SOIL INFORMATION. ADDITIONAL SITE WORK AND PAVING REQUIREMENTS IS AVAILABLE UPON REQUEST.

GEOTECHNICAL REPORT NO: EH255135
TERRACON
BATON ROUGE, LA
DATED NOVEMBER 6, 2025

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KEY PLAN:

SEAL:



REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	11.18.2025	SD
	01.16.2026	100% DD
	02.27.2026	ISSUED FOR BID

LIPSEY'S NEW HEADQUARTERS

HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

GENERAL NOTES

C0.01

J:\01 C0.01 GENERAL NOTES.dwg | Mar 03, 2026 - 9:20am

LOT 9-E

TRACT A

SEC. 51
SEC. 54

WARD CREEK

TRACT B

TRACT
C-6-A-1

TRACT
D-5-C-1-B

LOT
11-B-6

LOT
11-B-5

LOT
11-B-4

LOT
11-B-3

LOT
11-B-2

LOT
11-B-1

HIGHLAND COMMERCIAL PARK

WETLANDS
(BEING MITIGATED)

LOT
9-D

325
54

REMOVE EX.
CONC. DRIVEWAY
(± 270 SF)

OLD PERKINS ROAD
(ROW VARIES)

LOT 9-C
TINA & TAO, INC
(NOW OR FORMERLY)

MONTANA ESTATES

LOT
10-A-2

CLEAR & GRUB
PER GEOTECH
SITE PREP

LOT
10-A-1

LOT
10-B

LOT 9-A

0 40' 80'

MAINTAIN ACCESS TO THE GAS METERING
STATION THROUGHOUT CONSTRUCTION
(COORDINATE CONSTRUTION ACTIVITIES WITH
DELTA UTILITIES + GULF SOUTH PIPELINE)

RELOCATE EX.
ENTERGY
POWER POLE

ADJUST EX. SSMH TOP TO
FINISH FLISH W. FINISHED PVMT

SAW CUT & REMOVE
EXISTING LADOTD
CONC. CURB
(± 255 LF)

SAW CUT & REMOVE
EXISTING LADOTD
CONC. CURB
(± 52 LF)

ADJUST EX. SSMH TOP TO
FINISH FLISH W. FINISHED PVMT

REMOVE EX.
LIMESTONE
DRIVEWAY
(± 700 SF)

HIGHLAND ROAD
(ROW VARIES)

CONVERT EX. LADOTD CURB
INLETS TO RETICULINE GRATE
INLETS PER LADOTD STND
(± 1 EA.)

CONVERT EX. LADOTD CURB
INLETS TO RETICULINE GRATE
INLETS PER LADOTD STND
(± 5 EA.)

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KEY PLAN:

SEAL:

STATE OF LOUISIANA
BRANDON S. HEBRON
License No. 38319
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
02.27.26

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	11.18.2025	SD
	01.16.2026	100% DD
	02.27.2026	ISSUED FOR BID

LIPSEY'S NEW
HEADQUARTERS

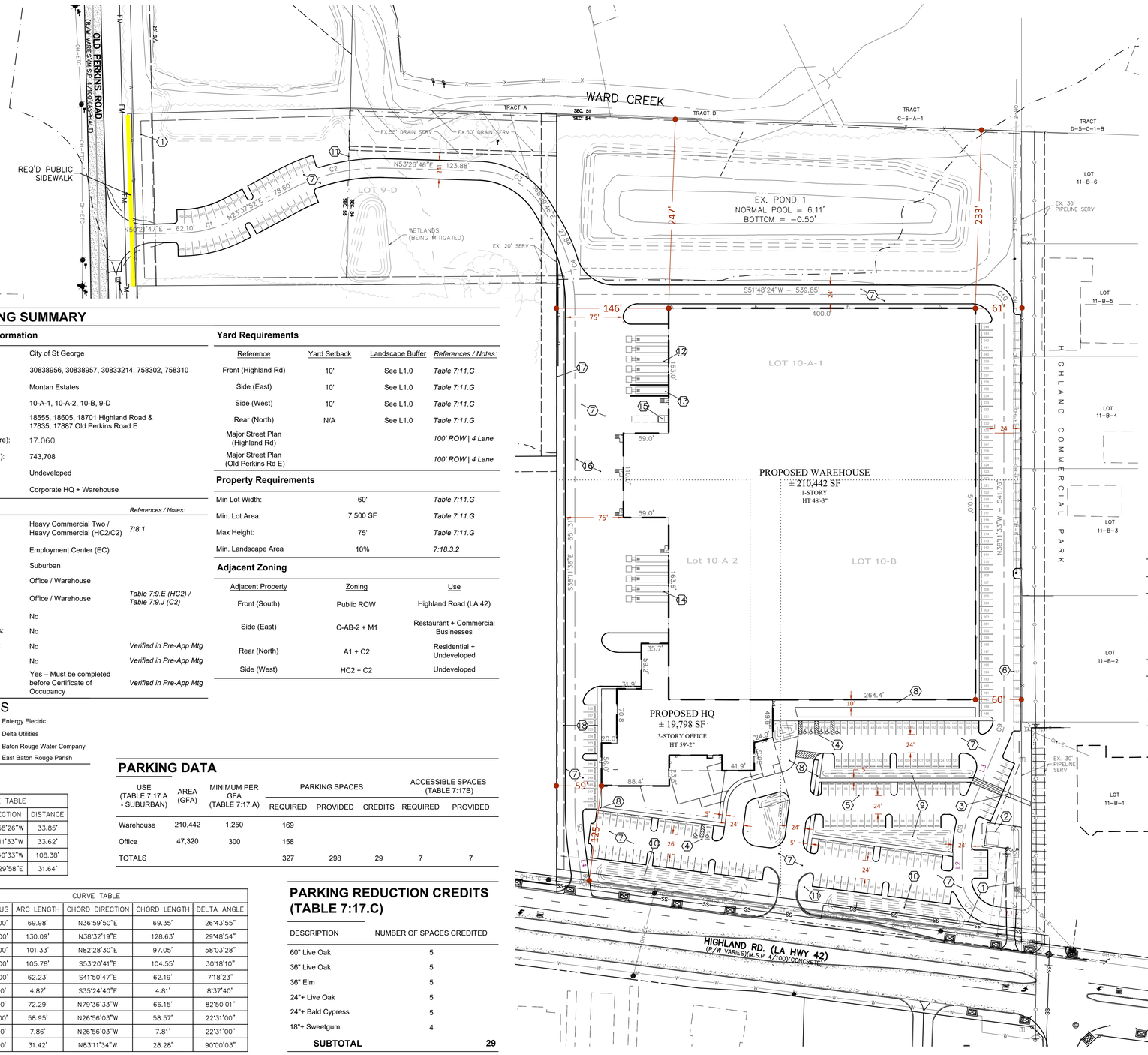
HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

DEMOLITION PLAN

C1.01

...02 C1.01 DEMOLITION PLAN.dwg | Feb 27, 2026 - 10:48am



PLANNING SUMMARY

Property Information

Jurisdiction:	City of St George
Parcel ID:	30838956, 30838957, 30833214, 758302, 758310
Subdivision	Montan Estates
Lots	10-A-1, 10-A-2, 10-B, 9-D
Site Address:	18555, 18605, 18701 Highland Road & 17835, 17887 Old Perkins Road E
Parcel Area (Acre):	17.060
Parcel Area (SF):	743,708
Existing Use:	Undeveloped
Proposed Use:	Corporate HQ + Warehouse

Zoning

		References / Notes:
Current Zoning:	Heavy Commercial Two / Heavy Commercial (HC2/C2)	*7:8.1*
Current FLU:	Employment Center (EC)	
Character Area	Suburban	
Proposed Use:	Office / Warehouse	
Permitted Use	Office / Warehouse	*Table 7:9.E (HC2) / Table 7:9.J (C2)*
PUD:	No	
Overlay Districts:	No	
Rezoning Reqd:	No	*Verified in Pre-App Mtg*
CU Reqd:	No	*Verified in Pre-App Mtg*
Platting Reqd:	Yes – Must be completed before Certificate of Occupancy	*Verified in Pre-App Mtg*

Yard Requirements

Reference	Yard Setback	Landscape Buffer	*References / Notes:*
Front (Highland Rd)	10'	See L1.0	*Table 7:11.G*
Side (East)	10'	See L1.0	*Table 7:11.G*
Side (West)	10'	See L1.0	*Table 7:11.G*
Rear (North)	N/A	See L1.0	*Table 7:11.G*
Major Street Plan (Highland Rd)			*100' ROW | 4 Lane*
Major Street Plan (Old Perkins Rd E)			*100' ROW | 4 Lane*

Property Requirements

Min Lot Width:	60'	*Table 7:11.G*
Min. Lot Area:	7,500 SF	*Table 7:11.G*
Max Height:	75'	*Table 7:11.G*
Min. Landscape Area	10%	*7:18.3.2*

Adjacent Zoning

Adjacent Property	Zoning	Use
Front (South)	Public ROW	Highland Road (LA 42)
Side (East)	C-AB-2 + M1	Restaurant + Commercial Businesses
Rear (North)	A1 + C2	Residential + Undeveloped
Side (West)	HC2 + C2	Undeveloped

UTILITIES

Electric	Entergy Electric
Natural Gas	Delta Utilities
Water	Baton Rouge Water Company
Sanitary Sewer	East Baton Rouge Parish

PARKING DATA

USE (TABLE 7:17.A - SUBURBAN)	AREA (GFA)	MINIMUM PER GFA (TABLE 7:17.A)	PARKING SPACES			ACCESSIBLE SPACES (TABLE 7:17B)	
			REQUIRED	PROVIDED	CREDITS	REQUIRED	PROVIDED
Warehouse	210,442	1,250	169				
Office	47,320	300	158				
TOTALS			327	298	29	7	7

LINE TABLE		
NUMBER	DIRECTION	DISTANCE
L1	S58°58'26"W	33.85'
L2	N38°11'33"W	33.62'
L3	N15°40'33"W	108.38'
L4	S45°29'58"E	31.64'

CURVE TABLE					
NUMBER	RADIUS	ARC LENGTH	CHORD DIRECTION	CHORD LENGTH	DELTA ANGLE
C1	150.00'	69.98'	N36°59'50"E	69.35'	26°43'55"
C2	250.00'	130.09'	N38°32'19"E	128.63'	29°48'54"
C3	100.00'	101.33'	N82°28'30"E	97.05'	58°03'28"
C4	200.00'	105.78'	S53°20'41"E	104.55'	30°18'10"
C5	488.00'	62.23'	S41°50'47"E	62.19'	7°18'23"
C6	32.00'	4.82'	S35°24'40"E	4.81'	8°37'40"
C7	50.00'	72.29'	N79°36'33"W	66.15'	82°50'01"
C8	150.00'	58.95'	N26°56'03"W	58.57'	22°31'00"
C9	20.00'	7.86'	N26°56'03"W	7.81'	22°31'00"
C10	20.00'	31.42'	N83°11'34"W	28.28'	90°00'03"

PARKING REDUCTION CREDITS (TABLE 7:17.C)

DESCRIPTION	NUMBER OF SPACES CREDITED
60" Live Oak	5
36" Live Oak	5
36" Elm	5
24"+ Live Oak	5
24"+ Bald Cypress	5
18"+ Sweetgum	4
SUBTOTAL	**29**

GENERAL NOTES:

1. ALL DIMENSIONS ARE TO FACE OF CURB, UNLESS NOTED OTHERWISE.
2. CONTRACTOR TO VERIFY BUILDING AND PARKING LOT LAYOUT WITH ARCHITECT AND ENGINEER PRIOR TO FORMING OF BUILDINGS, WALKS AND PARKING LOT AREAS.
3. ALL CURBS ARE BARRIER TYPE, UNLESS NOTED OTHERWISE, RE: CURB DETAIL
4. ALL NEWLY CUT AND/OR FILLED AREAS LACKING ADEQUATE VEGETATION SHALL BE SEEDED, FERTILIZED, MULCHED, AND/OR SODDED AS REQUIRED TO EFFECTIVELY PREVENT SOIL EROSION. CONTRACTOR SHALL MAINTAIN THESE AREAS UNTIL A HEALTHY STAND OF GRASS IS ACHIEVED.
5. THE CONTRACTOR MUST HAVE WRITTEN APPROVAL FROM THE CITY ENGINEER AND THE PROJECT ENGINEER BEFORE ANY CHANGE IN DESIGN IS MADE.
6. THE CONTRACTOR SHALL NOT ENTER UPON NOR CAUSE DAMAGE TO ANY ADJACENT PROPERTIES WITHOUT WRITTEN PERMISSION FROM SAID PROPERTY OWNERS.
7. ALL FILL AND COMPACTION SHALL BE EXECUTED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT AND THE GRADING PLAN.
8. THE CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR EXACT LOCATIONS AND DIMENSIONS OF VESTIBULE, SLOPE PAVING, SIDEWALKS, EXIT PORCHES, RAMPS, LOADING DOCKS, PRECISE BUILDING DIMENSIONS, AND EXACT BUILDING UTILITY ENTRANCE LOCATIONS.
9. CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR SITE LIGHTING ELECTRICAL PLAN, INCLUDING SITE LIGHTING LOCATIONS, TYPES, CONDUITS, ETC.
10. CONTRACTOR IS RESPONSIBLE FOR PROTECTION OF ALL AREAS INDICATED TO REMAIN UNDISTURBED OR TO REMAIN AS BUFFERS, ALL PROPERTY CORNERS, AND REPLACING ALL PINS DAMAGES DURING CONSTRUCTION.
11. CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE EXISTING BENCHMARK. BENCHMARK INFORMATION CAN BE FOUND ON THE GRADING PLAN AND/OR SURVEY PLAN.
12. PARKING STALLS MUST BE STRIPED WITH A 4" CONTRASTING STRIPE (YELLOW ON CONCRETE AND YELLOW OR WHITE ON ASPHALT PARKING LOTS).
13. CONTRACTOR SHALL COORDINATE WITH OWNER FOR SITE SIGNAGE.
14. PRIOR TO ANY DIGGING/TRENCHING/EXCAVATION IN THE RIGHT OF WAY THE CONTRACTOR SHALL CONTACT THE CITY PARISH TRAFFIC ENGINEERING DIVISION (CPTED) FOR LOCATION AND VERIFICATION OF THE EXISTING FIBER OPTIC LINES IN THE TRAFFIC SIGNAL NETWORK.
15. ALL WHEEL STOPS AND CONCRETE CURB SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE SITE DETAILS SHOWN IN THIS PLAN SET.
16. ALL PARKING SPACES ARE TO BE LAID OUT IN ACCORDANCE WITH THE TYPICAL DETAILS AS SHOWN ON THESE PLANS UNLESS OTHERWISE NOTED.

PROJECT DETAILS

OFFICE

BLDG Footprint (SF)	19,798
Floors (#)	3
BLDG GFA (SF)	47,320
BLDG Height (FT)	59'-2"

WAREHOUSE

BLDG Footprint (SF)	210,442
Floors (#)	1
BLDG Height (FT)	48'-3"

SITE DATA

DESCRIPTION	AREA (SF)	% OF SITE
TOTAL SITE AREA	**743,707**	**100%**
PRE-DEVELOPMENT		
PERVIOUS AREA:	**743,707**	**100.0**
EX. UNIMPROVED	741,251	99.7
EX. DELTA N.G. STATION	2,456	0.3
POST-DEVELOPMENT		
IMPERVIOUS AREA:	**453,099**	**60.9**
BLDG FOOTPRINT	232,656	31.3
VEHICULAR USE AREA	199,062	26.8
SIDEWALKS	18,925	2.5
EX. DELTA N.G. STATION	2,456	0.3
PERVIOUS AREA:	**290,608**	**39.1**

KEYNOTE LEGEND

#	KEYNOTE
1	MINIMUM YARD (TYP), TABLE 7:11.G
2	DELTA UTILITIES N.G. METERING STATION (TO REMAIN)
3	PEDESTRIAN ACCESS TO RUFFINOS, GATED
4	ACCESSIBLE PARKING (TYP)
5	STANDARD 90° PARKING STALL, 9'Wx18'D (TYP)
6	STANDARD PARALLEL PARKING STALL, 9'Wx22'D (TYP)
7	VEHICULAR USE AREA, CONCRETE (TYP)
8	PEDESTRIAN WALKWAY, CONCRETE (TYP)
9	RAIN GARDEN (TYP)
10	PRIVACY BERM (TYP)
11	SITE FENCING & GATES, REFER TO LANDSCAPE PLANS
12	LOADING BAYS - DOCK HIGH (X5)
13	LOADING BAY - RAMPED (X1)
14	LOADING BAYS - DOCK HIGH (X5)
15	REFUSE AREA + COMPACTOR
16	LOADING DOCKS AND SERVICE AREA, CONCRETE (TYP)
17	EX. RETAINING WALL W. TOP ELEVATION 19' (TO REMAIN)
18	PROPOSED PRIVATE UTILITY CORRIDOR

0 60' 120'

SCALE: 1" = 60'

RITTER MAHER ARCHITECTS
2987 Government Street | Baton Rouge, LA 70806
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KEY PLAN:

SEAL:
STATE OF LOUISIANA
BRANDON S. HEBRON
License No. 38319
PROFESSIONAL ENGINEER
IN
ENGINEERING
02.27.26

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	11.18.2025	SD
	01.16.2026	100% DD
	02.27.2026	ISSUED FOR BID

LIPSEY'S NEW HEADQUARTERS

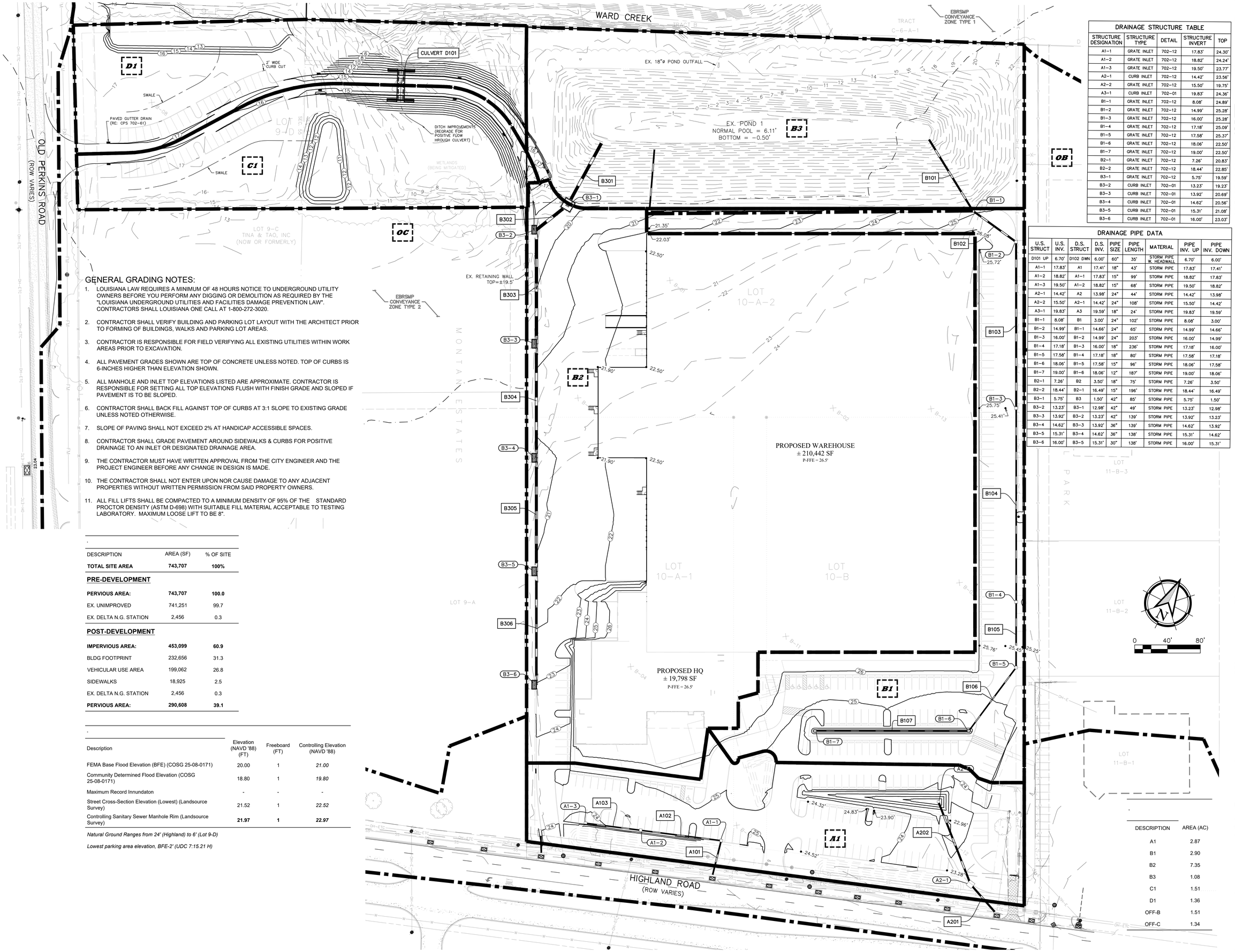
HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

SITE PLAN

C2.01

J:\03 C2.01 SITE PLAN.dwg | Feb 27, 2026 - 10:47am



GENERAL GRADING NOTES:

1. LOUISIANA LAW REQUIRES A MINIMUM OF 48 HOURS NOTICE TO UNDERGROUND UTILITY OWNERS BEFORE YOU PERFORM ANY DIGGING OR DEMOLITION AS REQUIRED BY THE "LOUISIANA UNDERGROUND UTILITIES AND FACILITIES DAMAGE PREVENTION LAW". CONTRACTORS SHALL LOUISIANA ONE CALL AT 1-800-272-3020.
2. CONTRACTOR SHALL VERIFY BUILDING AND PARKING LOT LAYOUT WITH THE ARCHITECT PRIOR TO FORMING OF BUILDINGS, WALKS AND PARKING LOT AREAS.
3. CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING ALL EXISTING UTILITIES WITHIN WORK AREAS PRIOR TO EXCAVATION.
4. ALL PAVEMENT GRADES SHOWN ARE TOP OF CONCRETE UNLESS NOTED. TOP OF CURBS IS 6-INCHES HIGHER THAN ELEVATION SHOWN.
5. ALL MANHOLE AND INLET TOP ELEVATIONS LISTED ARE APPROXIMATE. CONTRACTOR IS RESPONSIBLE FOR SETTING ALL TOP ELEVATIONS FLUSH WITH FINISH GRADE AND SLOPED IF PAVEMENT IS TO BE SLOPED.
6. CONTRACTOR SHALL BACK FILL AGAINST TOP OF CURBS AT 3:1 SLOPE TO EXISTING GRADE UNLESS NOTED OTHERWISE.
7. SLOPE OF PAVING SHALL NOT EXCEED 2% AT HANDICAP ACCESSIBLE SPACES.
8. CONTRACTOR SHALL GRADE PAVEMENT AROUND SIDEWALKS & CURBS FOR POSITIVE DRAINAGE TO AN INLET OR DESIGNATED DRAINAGE AREA.
9. THE CONTRACTOR MUST HAVE WRITTEN APPROVAL FROM THE CITY ENGINEER AND THE PROJECT ENGINEER BEFORE ANY CHANGE IN DESIGN IS MADE.
10. THE CONTRACTOR SHALL NOT ENTER UPON NOR CAUSE DAMAGE TO ANY ADJACENT PROPERTIES WITHOUT WRITTEN PERMISSION FROM SAID PROPERTY OWNERS.
11. ALL FILL LIFTS SHALL BE COMPACTED TO A MINIMUM DENSITY OF 95% OF THE STANDARD PROCTOR DENSITY (ASTM D-698) WITH SUITABLE FILL MATERIAL ACCEPTABLE TO TESTING LABORATORY. MAXIMUM LOOSE LIFT TO BE 8".

.

DESCRIPTION	AREA (SF)	% OF SITE
TOTAL SITE AREA	**743,707**	**100%**
PRE-DEVELOPMENT		
PERVIOUS AREA:	**743,707**	**100.0**
EX. UNIMPROVED	741,251	99.7
EX. DELTA N.G. STATION	2,456	0.3
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SIDEWALKS	18,925	2.5
EX. DELTA N.G. STATION	2,456	0.3
PERVIOUS AREA:	**290,608**	**39.1**

.

Description	Elevation (NAVD '88) (FT)	Freeboard (FT)	Controlling Elevation (NAVD '88)
FEMA Base Flood Elevation (BFE) (COSG 25-08-0171)	20.00	1	*21.00*
Community Determined Flood Elevation (COSG 25-08-0171)	18.80	1	*19.80*
Maximum Record Innundaton	-	-	-
Street Cross-Section Elevation (Lowest) (Landsource Survey)	21.52	1	*22.52*
Controlling Sanitary Sewer Manhole Rim (Landsource Survey)	**21.97**	**1**	**22.97**

Natural Ground Ranges from 24' (Highland) to 6' (Lot 9-D)

Lowest parking area elevation, BFE-2' (UDC 7:15.21 H)

DRAINAGE STRUCTURE TABLE

STRUCTURE DESIGNATION	STRUCTURE TYPE	DETAIL	STRUCTURE INVERT	TOP
A1-1	GRATE INLET	702-12	17.83'	24.30'
A1-2	GRATE INLET	702-12	18.82'	24.24'
A1-3	GRATE INLET	702-12	19.50'	23.77'
A2-1	CURB INLET	702-12	14.42'	23.56'
A2-2	GRATE INLET	702-12	15.50'	19.75'
A3-1	CURB INLET	702-01	19.83'	24.36'
B1-1	GRATE INLET	702-12	8.08'	24.89'
B1-2	GRATE INLET	702-12	14.99'	25.28'
B1-3	GRATE INLET	702-12	16.00'	25.28'
B1-4	GRATE INLET	702-12	17.18'	25.09'
B1-5	GRATE INLET	702-12	17.58'	25.37'
B1-6	GRATE INLET	702-12	18.06'	22.50'
B1-7	GRATE INLET	702-12	19.00'	22.50'
B2-1	GRATE INLET	702-12	7.26'	20.83'
B2-2	GRATE INLET	702-12	18.44'	22.85'
B3-1	GRATE INLET	702-12	5.75'	19.59'
B3-2	CURB INLET	702-01	13.23'	19.23'
B3-3	CURB INLET	702-01	13.92'	20.69'
B3-4	CURB INLET	702-01	14.62'	20.56'
B3-5	CURB INLET	702-01	15.31'	21.08'
B3-6	CURB INLET	702-01	16.00'	23.03'

DRAINAGE PIPE DATA

U.S. STRUCT	U.S. INV.	D.S. STRUCT	D.S. INV.	PIPE SIZE	PIPE LENGTH	MATERIAL	PIPE INV. UP	PIPE INV. DOWN
D101 UP	6.70'	D102 DWN	6.00'	60"	35'	STORM PIPE W. HEADWALL	6.70'	6.00'
A1-1	17.83'	A1	17.41'	18"	43'	STORM PIPE	17.83'	17.41'
A1-2	18.82'	A1-1	17.83'	15"	99'	STORM PIPE	18.82'	17.83'
A1-3	19.50'	A1-2	18.82'	15"	68'	STORM PIPE	19.50'	18.82'
A2-1	14.42'	A2	13.98'	24"	44'	STORM PIPE	14.42'	13.98'
A2-2	15.50'	A2-1	14.42'	24"	108'	STORM PIPE	15.50'	14.42'
A3-1	19.83'	A3	19.59'	18"	24'	STORM PIPE	19.83'	19.59'
B1-1	8.08'	B1	3.00'	24"	102'	STORM PIPE	8.08'	3.00'
B1-2	14.99'	B1-1	14.66'	24"	65'	STORM PIPE	14.99'	14.66'
B1-3	16.00'	B1-2	14.99'	24"	203'	STORM PIPE	16.00'	14.99'
B1-4	17.18'	B1-3	16.00'	18"	236'	STORM PIPE	17.18'	16.00'
B1-5	17.58'	B1-4	17.18'	18"	80'	STORM PIPE	17.58'	17.18'
B1-6	18.06'	B1-5	17.58'	15"	96'	STORM PIPE	18.06'	17.58'
B1-7	19.00'	B1-6	18.06'	12"	187'	STORM PIPE	19.00'	18.06'
B2-1	7.26'	B2	3.50'	18"	75'	STORM PIPE	7.26'	3.50'
B2-2	18.44'	B2-1	16.49'	15"	196'	STORM PIPE	18.44'	16.49'
B3-1	5.75'	B3	1.50'	42"	85'	STORM PIPE	5.75'	1.50'
B3-2	13.23'	B3-1	12.98'	42"	49'	STORM PIPE	13.23'	12.98'
B3-3	13.92'	B3-2	13.23'	42"	139'	STORM PIPE	13.92'	13.23'
B3-4	14.62'	B3-3	13.92'	36"	139'	STORM PIPE	14.62'	13.92'
B3-5	15.31'	B3-4	14.62'	36"	138'	STORM PIPE	15.31'	14.62'
B3-6	16.00'	B3-5	15.31'	30"	138'	STORM PIPE	16.00'	15.31'

.

DESCRIPTION	AREA (AC)
A1	2.87
B1	2.90
B2	7.35
B3	1.08
C1	1.51
D1	1.36
OFF-B	1.51
OFF-C	1.34

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STATE OF LOUISIANA
BRANDON S. HEBRON
License No. 36319
PROFESSIONAL ENGINEER
IN
ENGINEERING
02.27.26

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	11.18.2025	SD
	01.16.2026	100% DD
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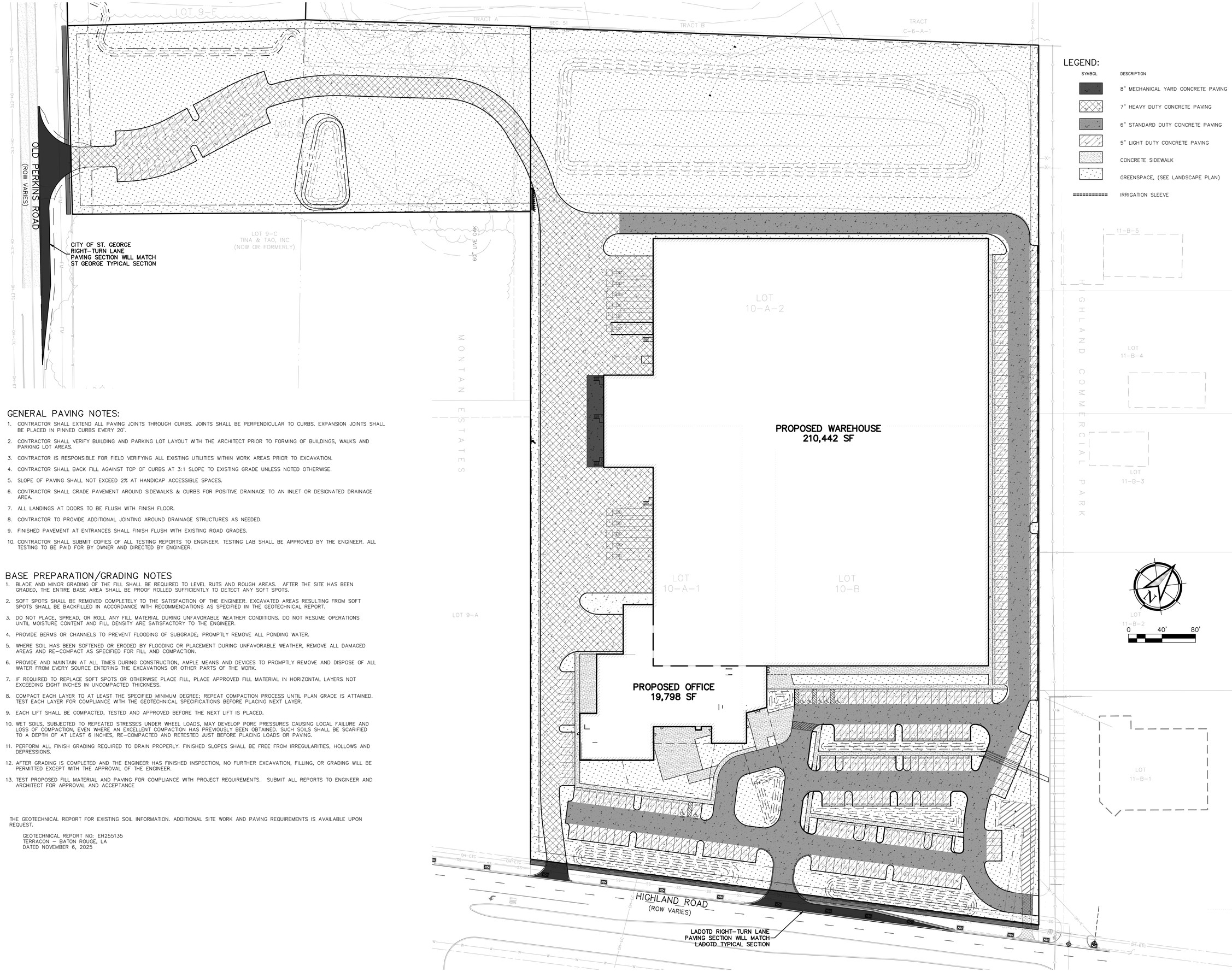
LIPSEY'S NEW HEADQUARTERS

HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

GRADING & DRAINAGE PLAN

C3.01



LEGEND:

SYMBOL	DESCRIPTION
	8" MECHANICAL YARD CONCRETE PAVING
	7" HEAVY DUTY CONCRETE PAVING
	6" STANDARD DUTY CONCRETE PAVING
	5" LIGHT DUTY CONCRETE PAVING
	CONCRETE SIDEWALK
	GREENSPACE, (SEE LANDSCAPE PLAN)
==========	IRRIGATION SLEEVE

GENERAL PAVING NOTES:

1. CONTRACTOR SHALL EXTEND ALL PAVING JOINTS THROUGH CURBS. JOINTS SHALL BE PERPENDICULAR TO CURBS. EXPANSION JOINTS SHALL BE PLACED IN PINNED CURBS EVERY 20'.
2. CONTRACTOR SHALL VERIFY BUILDING AND PARKING LOT LAYOUT WITH THE ARCHITECT PRIOR TO FORMING OF BUILDINGS, WALKS AND PARKING LOT AREAS.
3. CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING ALL EXISTING UTILITIES WITHIN WORK AREAS PRIOR TO EXCAVATION.
4. CONTRACTOR SHALL BACK FILL AGAINST TOP OF CURBS AT 3:1 SLOPE TO EXISTING GRADE UNLESS NOTED OTHERWISE.
5. SLOPE OF PAVING SHALL NOT EXCEED 2% AT HANDICAP ACCESSIBLE SPACES.
6. CONTRACTOR SHALL GRADE PAVEMENT AROUND SIDEWALKS & CURBS FOR POSITIVE DRAINAGE TO AN INLET OR DESIGNATED DRAINAGE AREA.
7. ALL LANDINGS AT DOORS TO BE FLUSH WITH FINISH FLOOR.
8. CONTRACTOR TO PROVIDE ADDITIONAL JOINTING AROUND DRAINAGE STRUCTURES AS NEEDED.
9. FINISHED PAVEMENT AT ENTRANCES SHALL FINISH FLUSH WITH EXISTING ROAD GRADES.
10. CONTRACTOR SHALL SUBMIT COPIES OF ALL TESTING REPORTS TO ENGINEER. TESTING LAB SHALL BE APPROVED BY THE ENGINEER. ALL TESTING TO BE PAID FOR BY OWNER AND DIRECTED BY ENGINEER.

BASE PREPARATION/GRADING NOTES

1. BLADE AND MINOR GRADING OF THE FILL SHALL BE REQUIRED TO LEVEL RUTS AND ROUGH AREAS. AFTER THE SITE HAS BEEN GRADED, THE ENTIRE BASE AREA SHALL BE PROOF ROLLED SUFFICIENTLY TO DETECT ANY SOFT SPOTS.
2. SOFT SPOTS SHALL BE REMOVED COMPLETELY TO THE SATISFACTION OF THE ENGINEER. EXCAVATED AREAS RESULTING FROM SOFT SPOTS SHALL BE BACKFILLED IN ACCORDANCE WITH RECOMMENDATIONS AS SPECIFIED IN THE GEOTECHNICAL REPORT.
3. DO NOT PLACE, SPREAD, OR ROLL ANY FILL MATERIAL DURING UNFAVORABLE WEATHER CONDITIONS. DO NOT RESUME OPERATIONS UNTIL MOISTURE CONTENT AND FILL DENSITY ARE SATISFACTORY TO THE ENGINEER.
4. PROVIDE BERMS OR CHANNELS TO PREVENT FLOODING OF SUBGRADE; PROMPTLY REMOVE ALL PONDING WATER.
5. WHERE SOIL HAS BEEN SOFTENED OR ERODED BY FLOODING OR PLACEMENT DURING UNFAVORABLE WEATHER, REMOVE ALL DAMAGED AREAS AND RE-COMPACT AS SPECIFIED FOR FILL AND COMPACTION.
6. PROVIDE AND MAINTAIN AT ALL TIMES DURING CONSTRUCTION, AMPLE MEANS AND DEVICES TO PROMPTLY REMOVE AND DISPOSE OF ALL WATER FROM EVERY SOURCE ENTERING THE EXCAVATIONS OR OTHER PARTS OF THE WORK.
7. IF REQUIRED TO REPLACE SOFT SPOTS OR OTHERWISE PLACE FILL, PLACE APPROVED FILL MATERIAL IN HORIZONTAL LAYERS NOT EXCEEDING EIGHT INCHES IN UNCOMPACTED THICKNESS.
8. COMPACT EACH LAYER TO AT LEAST THE SPECIFIED MINIMUM DEGREE; REPEAT COMPACTION PROCESS UNTIL PLAN GRADE IS ATTAINED. TEST EACH LAYER FOR COMPLIANCE WITH THE GEOTECHNICAL SPECIFICATIONS BEFORE PLACING NEXT LAYER.
9. EACH LIFT SHALL BE COMPACTED, TESTED AND APPROVED BEFORE THE NEXT LIFT IS PLACED.
10. WET SOILS, SUBJECTED TO REPEATED STRESSES UNDER WHEEL LOADS, MAY DEVELOP PORE PRESSURES CAUSING LOCAL FAILURE AND LOSS OF COMPACTION, EVEN WHERE AN EXCELLENT COMPACTION HAS PREVIOUSLY BEEN OBTAINED. SUCH SOILS SHALL BE SCARIFIED TO A DEPTH OF AT LEAST 6 INCHES, RE-COMPACTED AND RETESTED JUST BEFORE PLACING LOADS OR PAVING.
11. PERFORM ALL FINISH GRADING REQUIRED TO DRAIN PROPERLY. FINISHED SLOPES SHALL BE FREE FROM IRREGULARITIES, HOLLOWS AND DEPRESSIONS.
12. AFTER GRADING IS COMPLETED AND THE ENGINEER HAS FINISHED INSPECTION, NO FURTHER EXCAVATION, FILLING, OR GRADING WILL BE PERMITTED EXCEPT WITH THE APPROVAL OF THE ENGINEER.
13. TEST PROPOSED FILL MATERIAL AND PAVING FOR COMPLIANCE WITH PROJECT REQUIREMENTS. SUBMIT ALL REPORTS TO ENGINEER AND ARCHITECT FOR APPROVAL AND ACCEPTANCE

THE GEOTECHNICAL REPORT FOR EXISTING SOIL INFORMATION. ADDITIONAL SITE WORK AND PAVING REQUIREMENTS IS AVAILABLE UPON REQUEST.

GEOTECHNICAL REPORT NO: EH255135
TERRACON – BATON ROUGE, LA
DATED NOVEMBER 6, 2025

RITTER MAHER ARCHITECTS
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KEY PLAN:

SEAL:

BRANDON S. HEBRON
NO. 39574
PROFESSIONAL ENGINEER
IN
LOUISIANA
02.27.26

REVISION NO:	DATE:	DESCRIPTION:
		PRELIMINARY PLAN ONLY NOT TO BE USED FOR CONSTRUCTION, BIDDING, RECORDATION, CONVEYANCE, SALES, OR AS A BASIS FOR THE ISSUANCE OF A PERMIT.

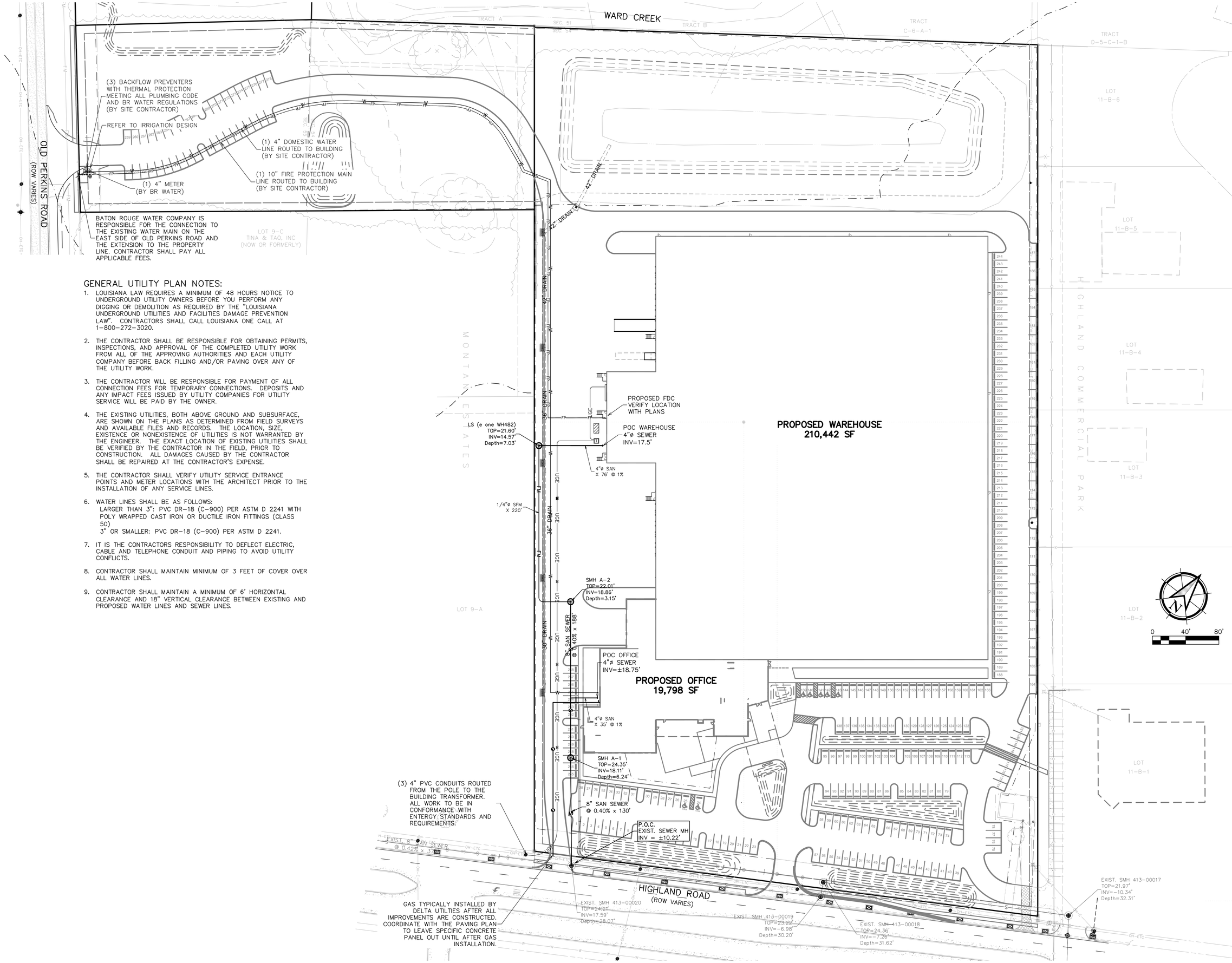
LIPSEY'S NEW HEADQUARTERS

HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

PAVING PLAN

C4.01



GENERAL UTILITY PLAN NOTES:

1. LOUISIANA LAW REQUIRES A MINIMUM OF 48 HOURS NOTICE TO UNDERGROUND UTILITY OWNERS BEFORE YOU PERFORM ANY DIGGING OR DEMOLITION AS REQUIRED BY THE "LOUISIANA UNDERGROUND UTILITIES AND FACILITIES DAMAGE PREVENTION LAW". CONTRACTORS SHALL CALL LOUISIANA ONE CALL AT 1-800-272-3020.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING PERMITS, INSPECTIONS, AND APPROVAL OF THE COMPLETED UTILITY WORK FROM ALL OF THE APPROVING AUTHORITIES AND EACH UTILITY COMPANY BEFORE BACK FILLING AND/OR PAVING OVER ANY OF THE UTILITY WORK.
3. THE CONTRACTOR WILL BE RESPONSIBLE FOR PAYMENT OF ALL CONNECTION FEES FOR TEMPORARY CONNECTIONS. DEPOSITS AND ANY IMPACT FEES ISSUED BY UTILITY COMPANIES FOR UTILITY SERVICE WILL BE PAID BY THE OWNER.
4. THE EXISTING UTILITIES, BOTH ABOVE GROUND AND SUBSURFACE, ARE SHOWN ON THE PLANS AS DETERMINED FROM FIELD SURVEYS AND AVAILABLE FILES AND RECORDS. THE LOCATION, SIZE, EXISTENCE OR NONEXISTENCE OF UTILITIES IS NOT WARRANTED BY THE ENGINEER. THE EXACT LOCATION OF EXISTING UTILITIES SHALL BE VERIFIED BY THE CONTRACTOR IN THE FIELD, PRIOR TO CONSTRUCTION. ALL DAMAGES CAUSED BY THE CONTRACTOR SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.
5. THE CONTRACTOR SHALL VERIFY UTILITY SERVICE ENTRANCE POINTS AND METER LOCATIONS WITH THE ARCHITECT PRIOR TO THE INSTALLATION OF ANY SERVICE LINES.
6. WATER LINES SHALL BE AS FOLLOWS:
   LARGER THAN 3": PVC DR-18 (C-900) PER ASTM D 2241 WITH POLY WRAPPED CAST IRON OR DUCTILE IRON FITTINGS (CLASS 50)
   3" OR SMALLER: PVC DR-18 (C-900) PER ASTM D 2241.
7. IT IS THE CONTRACTORS RESPONSIBILITY TO DEFLECT ELECTRIC, CABLE AND TELEPHONE CONDUIT AND PIPING TO AVOID UTILITY CONFLICTS.
8. CONTRACTOR SHALL MAINTAIN MINIMUM OF 3 FEET OF COVER OVER ALL WATER LINES.
9. CONTRACTOR SHALL MAINTAIN A MINIMUM OF 6' HORIZONTAL CLEARANCE AND 18" VERTICAL CLEARANCE BETWEEN EXISTING AND PROPOSED WATER LINES AND SEWER LINES.

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	11.18.2025	SD
	01.16.2026	100% DD
	02.27.2026	ISSUED FOR BID

LIPSEY'S NEW HEADQUARTERS

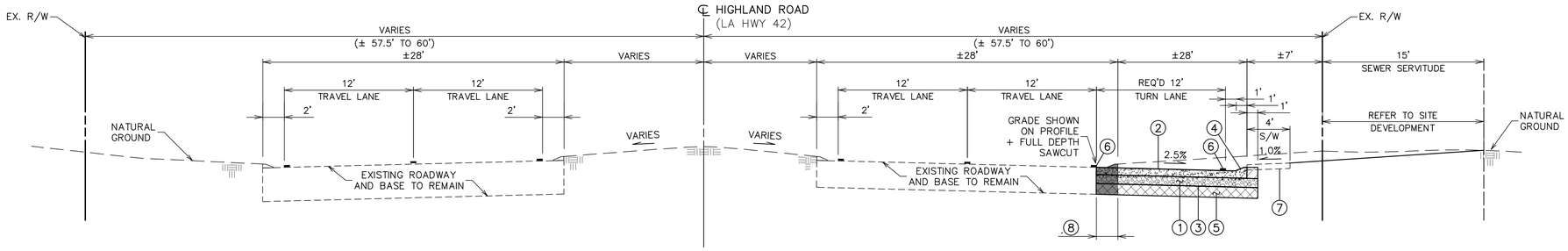
HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

UTILITY PLAN

C5.01

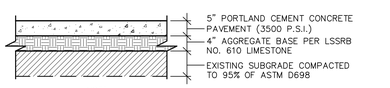
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HIGHLAND ROAD
(LA HWY 42)

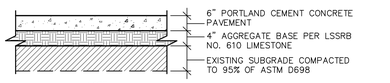
TYPICAL FINISHED TURNLANE CONCRETE SECTION
(STA. XXX+00 TO STA. XXX+00)

1. 10" THICK CLASS II BASE COURSE (CRUSHED STONE OR RECYCLED PCCP)
2. 8" PORTLAND CEMENT CONCRETE PAVEMENT (PCCP)
3. GEOTEXTILE FABRIC (INCLUDED IN CLASS II BASE COURSE)
4. INTEGRAL CONCRETE CURB (MOUNTABLE TYPE), RE:LADOTD CP-01
5. 12" THICK TYPE E LIME TREATMENT (9% BY VOLUME) (AS DIRECTED BY PROJECT ENGINEER)
6. PAVEMENT STRIPING & MARKERS, RE: LADOTD PM-01 THRU PM-09
7. 4" THICK CONCRETE SIDEWALK
8. REMOVE EXISTING CURB, SURFACING AND BASE



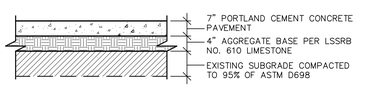
NOTE:
BASE MATERIAL SHALL MEET REQUIREMENTS AS SPECIFIED IN THE GEOTECHNICAL REPORT PRODUCED BY TERRACON DATED NOVEMBER 6 2025.
CONTRACTOR SHALL SUBMIT PROPOSED BASE MATERIAL SPECIFICATIONS TO ENGINEER FOR APPROVAL PRIOR TO ANY PAVING.

1 LIGHT DUTY PAVING SECTION
PARKING STALLS



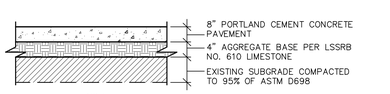
NOTE:
BASE MATERIAL SHALL MEET REQUIREMENTS AS SPECIFIED IN THE GEOTECHNICAL REPORT PRODUCED BY TERRACON DATED NOVEMBER 6 2025.
CONTRACTOR SHALL SUBMIT PROPOSED BASE MATERIAL SPECIFICATIONS TO ENGINEER FOR APPROVAL PRIOR TO ANY PAVING.

2 STANDARD DUTY PAVING SECTION
DRIVE AISLES



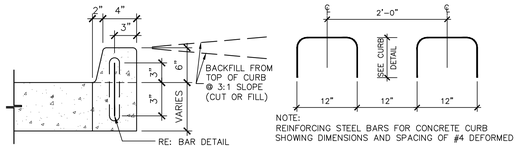
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CONTRACTOR SHALL SUBMIT PROPOSED BASE MATERIAL SPECIFICATIONS TO ENGINEER FOR APPROVAL PRIOR TO ANY PAVING.

3 HEAVY DUTY PAVING SECTION
ENTRANCES DRIVES & TRUCK ROUTES



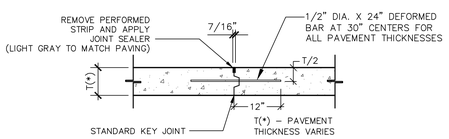
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4 HEAVY EQUIPMENT PAVING SECTION
MECHANICAL YARD & REFUSE AREAS

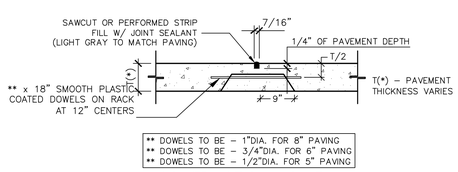


NOTE:
REINFORCING STEEL BARS FOR CONCRETE CURB SHOWING DIMENSIONS AND SPACING OF #4 DEFORMED BAR DETAIL

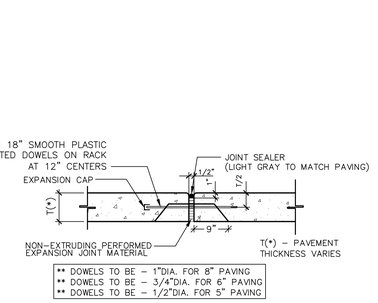
5 INTEGRAL CONCRETE CURB
N.T.S.



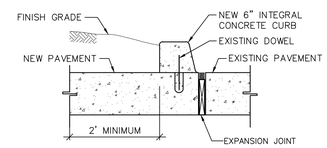
6 LONGITUDINAL (KEY) JOINT
N.T.S.



7 CONTRACTION JOINT
N.T.S.

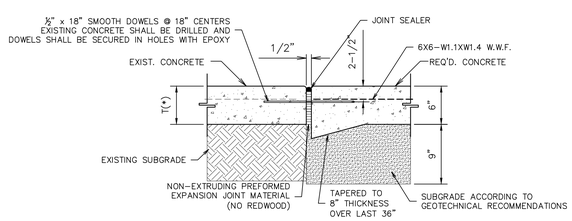


8 EXPANSION JOINT
N.T.S.



SECTION 'A-A'

9 DRIVEWAY CONNECTION
N.T.S.



10 NEW CONCRETE AT EXISTING CONCRETE
N.T.S.

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RITTERMAHER.COM

CONSULTANT:
HEBRON
ENGINEERING SERVICES, LLC
Brandon.Hebron@HESconnects.com | 225.964.0730

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STATE OF LOUISIANA
BRANDON S. HEBRON
License No. 36319
PROFESSIONAL ENGINEER
IN
ENGINEERING
02.27.26

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
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LIPSEY'S NEW HEADQUARTERS

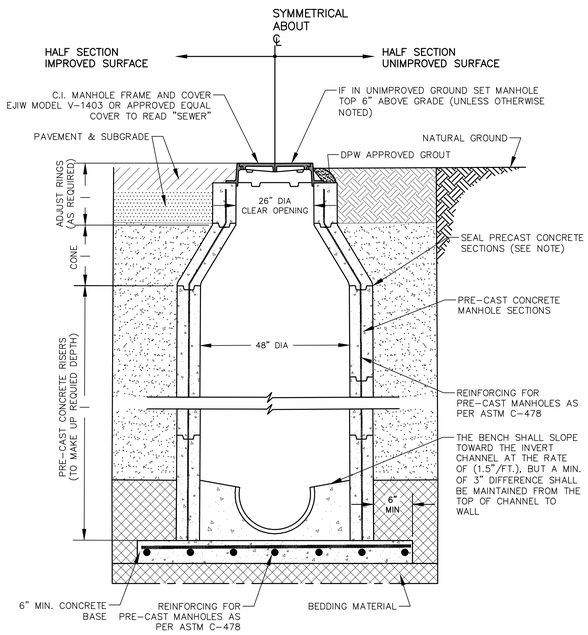
HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

SITE DETAILS

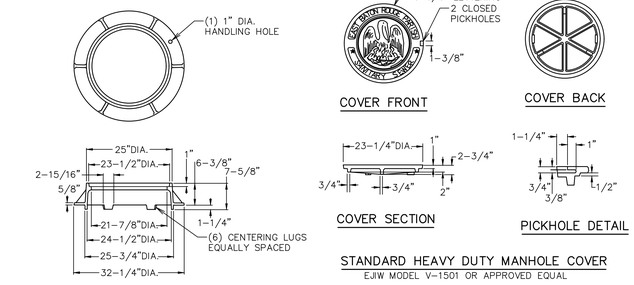
C6.01

/06 C6.01 SITE DETAILS.dwg | Mar 03, 2026 - 10:44am



NOTES:
1. ALL CAST IRON FRAMES AND COVERS SHALL BE TRAFFIC BEARING AND BE OF DOMESTIC ORIGIN. FRAME AND COVERS SHALL MEET OR EXCEED ALL REQUIREMENTS OF THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS DESIGNATION:M306 STANDARD SPECIFICATION FOR DRAINAGE, SEWER, UTILITY, AND RELATED CASTINGS. THEY SHALL HAVE AN ENVIRONMENTALLY SAFE, WATER-BASE ASPHALTIC COATING WHICH IS NONTOXIC, NONFLAMMABLE, COLORLESS, AND DRIES TO A HARD BLACK FINISH.
2. MANHOLE SECTIONS SHALL BE JOINED TOGETHER WITH FLEXIBLE WATERTIGHT RUBBER GASKETS CONFORMING TO EAST BATON ROUGE SPECIFICATIONS.
3. ALL PRE-CAST MANHOLES SHALL BE GIVEN AN ACCEPTABLE INTERIOR COATING.

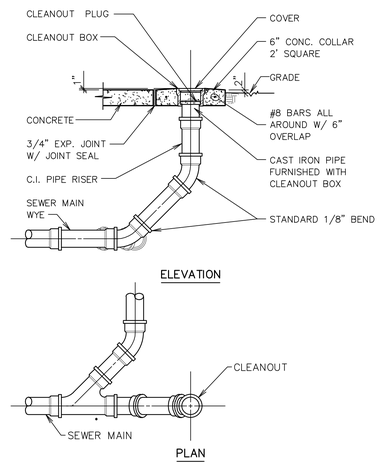
11 PRECAST SANITARY SEWER MANHOUSE
NTS



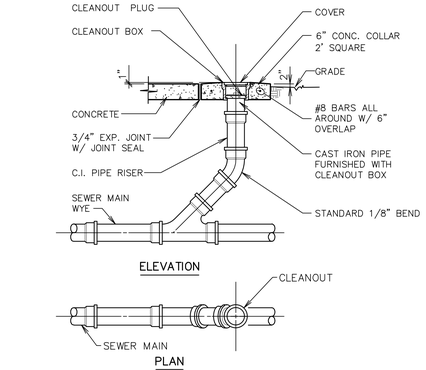
STANDARD MANHOLE FRAME WITH LUGS
EJIW MODEL V-1403 OR APPROVED EQUAL

STANDARD HEAVY DUTY MANHOLE COVER
EJIW MODEL V-1501 OR APPROVED EQUAL

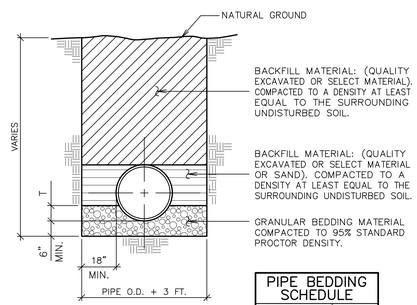
12 CLEANOUT AT WYE
NTS



13 CLEANOUT IN LINE
NTS



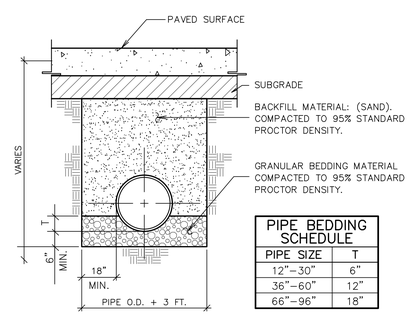
14 CAST IN PLACE INLET
NTS



OUTSIDE LIMITS OF PAVED SURFACES

PIPE BEDDING SCHEDULE	
PIPE SIZE	T
12"-30"	6"
36"-60"	12"
66"-96"	18"

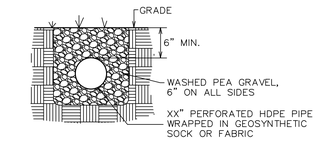
15 RIGID PIPE BEDDING
N.T.S.



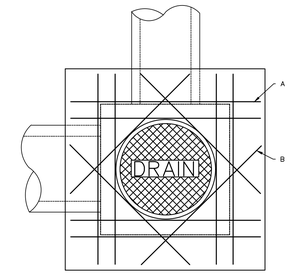
UNDER OR WITHIN 5' OF PAVED SURFACES

PIPE BEDDING SCHEDULE	
PIPE SIZE	T
12"-30"	6"
36"-60"	12"
66"-96"	18"

16 RIGID PIPE BEDDING
N.T.S.

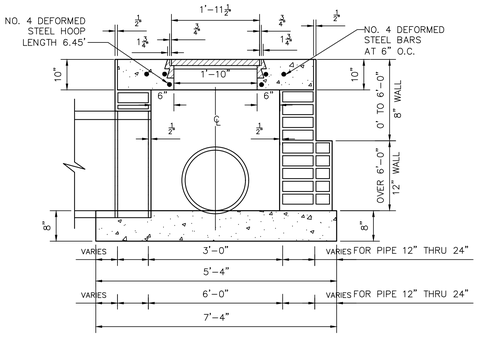


17 SOCK DRAIN
N.T.S.



5'-5" X 5'-5"			
BAR NO.	KIND	LENGTH	TOTAL
A	8 4 DEF.	4'-11"	39'-4"
B	4 4 DEF.	4'-6"	18'-0"

4'-5" X 4'-5"			
BAR NO.	KIND	LENGTH	TOTAL
A	8 4 DEF.	3'-11"	31'-4"
B	4 4 DEF.	3'-6"	14'-0"



18 STANDARD EAST BATON ROUGE PARISH TYPE 1 MANHOLE
N.T.S.

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	01.16.2026	100% DD
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LIPSEY'S NEW HEADQUARTERS

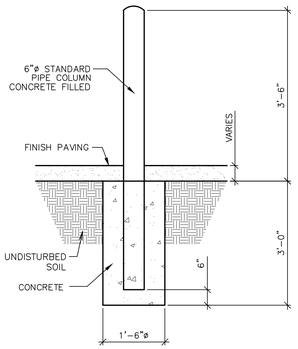
HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

SITE DETAILS

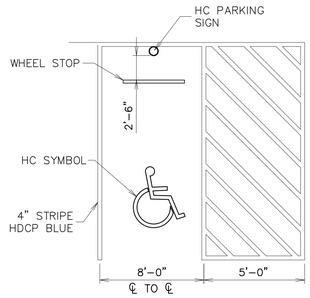
C6.02

J:\06 06.02 SITE DETAILS.dwg | Mar 03, 2026 - 10:50am



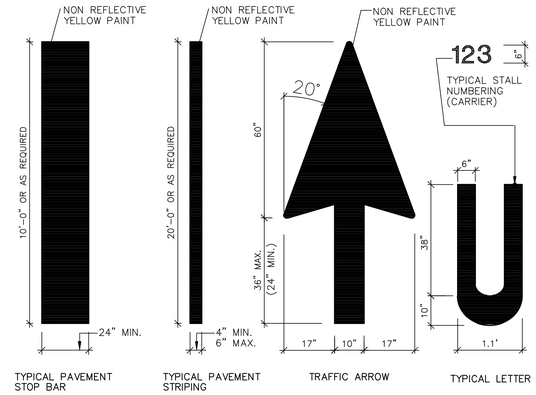
BOLLARD COLOR:
IN PAVED AREAS = YELLOW
POST INDICATOR VALVE / FIRE HYDRANT = RED

1 PIPE BOLLARD DETAIL
N.T.S.

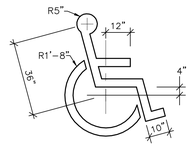


NOTES:
1. ALL SIGNS SHOWN SHALL CONFORM TO THE MANUAL FOR UNIFORM TRAFFIC CONTROL DEVICES.
2. ALL SIGNS SHOWN SHALL BE REFLECTORIZED TO SHOW THE SAME COLOR BY NIGHT AS BY DAY.
3. ALL SIGNS SHALL BE SECURELY MOUNTED ON GALVANIZED STEEL POSTS.

3 HANDICAP PARKING SPACE LAYOUT
N.T.S.

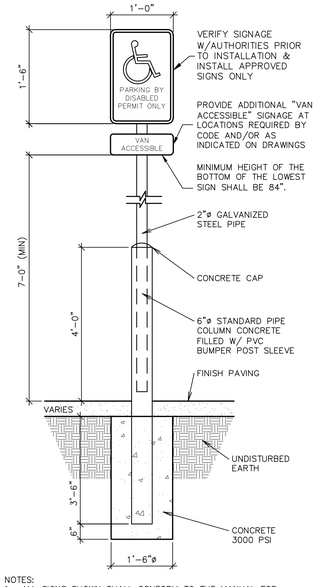


6 TYPICAL PAVEMENT MARKINGS
NTS



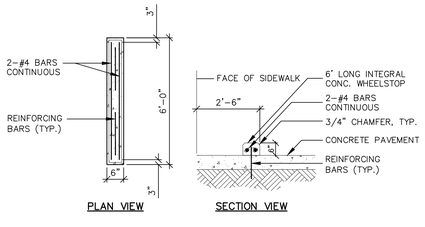
NOTE:
1. SYMBOL TO BE 4" WIDE HANDICAP BLUE TRAFFIC PAINT ON PAVEMENT. TYPICAL AT ALL HANDICAP STALLS
2. VERIFY SIGNAGE W/AUTHORITIES PRIOR TO INSTALLATION & INSTALL APPROVED SIGNS ONLY

4 ACCESSIBLE SYMBOL
NTS



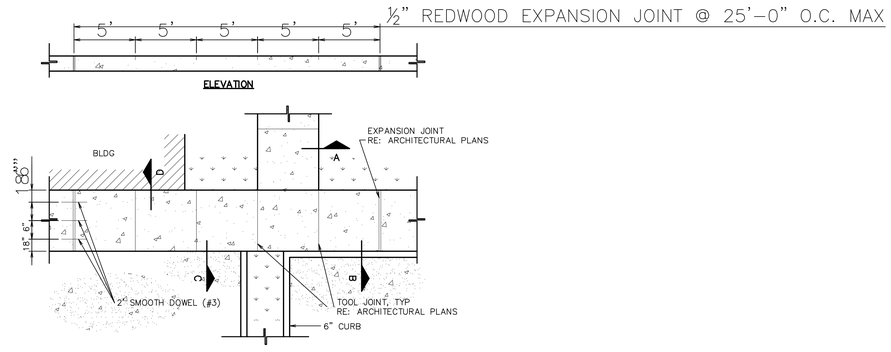
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2. ALL SIGNS SHOWN SHALL BE REFLECTORIZED TO SHOW THE SAME COLOR BY NIGHT AS BY DAY.
3. ALL SIGNS SHALL BE SECURELY MOUNTED ON GALVANIZED STEEL POSTS.
4. INSTALL QUANTITY AND AT LOCATIONS REQUIRED BY CODE AND/OR AS INDICATED ON DRAWINGS. CONFIRM ALL HANDICAPPED SIGNAGE CODES.

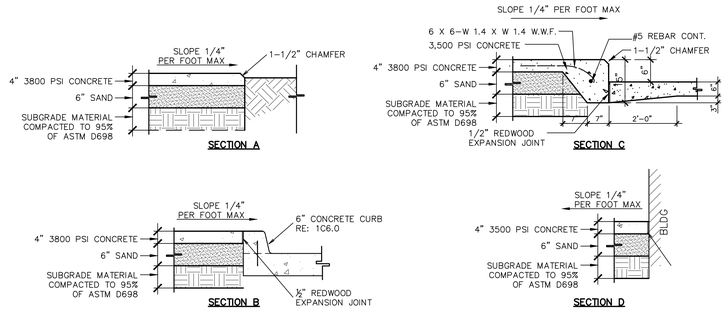
2 ACCESSIBLE PARKING SIGN
NTS



5 WHEEL STOP DETAIL
NTS

½" REDWOOD EXPANSION JOINT @ 25'–0" O.C. MAX





7 CONCRETE SIDEWALK
NTS

RITTER MAHER ARCHITECTS
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p 225.383.4321
RITTERMAHER.COM

CONSULTANT:
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LIPSEY'S NEW HEADQUARTERS

HIGHLAND ROAD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

SITE DETAILS

C6.03

C:\Users\brandon.hebron\Documents\24154_LIPSEY'S NEW HEADQUARTERS_brandon.hebron@HESconnects.com.rvt | 06.03 SITE DETAILS.dwg | Mar 03, 2026 - 10:52am

GENERAL NOTES

I. CODES AND SPECIFICATIONS

A. GENERAL BUILDING CODE
1. International Building Code 2021 with Louisiana Amendments
2. ASCE 7 – Minimum Design Loads for Buildings and Other Structures

B. CONCRETE CODES
1. ACI 318 – Building Code Requirements for Structural Concrete
2. ACI 301 – Specifications for Structural Concrete
3. ACI 315 – Details and Detailing of Concrete Reinforcement
4. AWS D1.4 – Structural Welding Code: Reinforcing Steel
5. CRSI – Manual of Standard Practice

C. STRUCTURAL STEEL CODES
1. AISC 360 – Specifications for Structural Steel Buildings
2. AISC 303 – Code of Standard Practice for Steel Buildings and Bridges
3. AWS D1.1 – Structural Welding Code: Steel
4. AWS D1.3 – Structural Welding Code: Sheet Steel
5. Research Council on Structural Connections – Specification for Structural Joints using High-Strength Bolts

D. MASONRY CODES
1. TMS 402 – Building Code Requirements for Masonry Structures
2. TMS 602 – Specification for Masonry Structures

E. COLD FORMED STEEL (LIGHT GAGE STEEL) CODE
1. AISI S100 – North American Specification for the Design of Cold Formed Steel Structural Members

F. CONFLICTS IN STRUCTURAL REQUIREMENTS
1. Where conflict exists between the various publications as specified herein, the strictest requirements of the various publications shall govern unless noted otherwise. Where conflict exists among the various parts of the Structural Contract Documents, (Structural Drawings, General Notes, Specifications) the strictest requirements shall govern.

All Codes and Specifications listed above shall include all amendments and addenda for the editions referenced in the International Building Code version listed in GENERAL BUILDING CODES, Paragraph I. A., as per Section 102.4. Otherwise, the latest edition of the Code or Specification shall be used.

II. TYPICAL DETAILS

A. Details labeled "Typical Details" on the Drawings shall apply to all situations on the Project that are the same or similar to those specifically detailed. Such details shall apply whether or not they are keyed in at each location. Questions regarding applicability of typical details shall be determined by the Engineer.

III. DESIGN CRITERIA

A. GENERAL
1. Dead loads: Dead load materials assumed in the design are shown on the Architectural and Structural Drawings. Any changes in construction materials from those shown on the Architectural or Structural Drawings shall be reported by the General Contractor to the Structural Engineer for verification of load carrying capacity of the structure.
2. Partitions: An allowance of 15 psf has been made for partitions as a uniformly distributed live load, wherever partitions are used and locations subject to change excluding public lobbies, corridors, restrooms, and places of public assembly.
3. Mechanical Rooms: Loadings for mechanical rooms are based on the minimum live loads as specified in Paragraph B below unless the weights of the actual equipment including housekeeping pads as shown on the Mechanical Drawings are larger, in which case, the actual loads are used. The General Contractor shall submit weights to the Structural Engineer for all equipment placed in mechanical rooms and rooftops for verification of loads used in the design and shall report any changes in location, number of pieces, and weight of equipment as shown on the Mechanical Drawings.

B. LIVE LOADS

CATEGORY	UNIFORM LOAD (PSF)	CONCENTRATED LOAD (#)
Roof	20	300
Grab Bars (Restrooms)	N/A	250
Guard Rails & Hand Rails	50 plf or 200 lbs. applied at top rail in any direction. Intermediate rails = 25 plf.	
Office Floors	50	2000
Corridors above First Floor:		
Offices	80	2000
Assembly Areas, Terraces, Lobbies	100	0
Auditorium-Movable Seating	100	0
Auditorium - Stage Area	150	0
Computer Rooms	150	2000
Corridors, First Floor	100	0
Mechanical Rooms, typical	150	Actual Equip. Wt.
Restrooms	60	2000
Roof – For Roof Gardens	100	0
Stairways, Exit Facilities	100	300
Storage - Heavy	250	0
Storage - Light	125	0

Live loads have been reduced on any member based on the Code cited in CODES AND SPECIFICATIONS, Paragraph I. A.

C. WIND LOADS
1. Wind pressure based on the requirements of Code cited in CODES AND SPECIFICATIONS, Paragraph I. A.
2. V = 127 mph, Exposure: C.
3. Risk Category **II**

D. ELEVATOR LOADS
1. Machine Beam, Car Buffer, Counterweight Buffer, and Guide Rail Loads. Assumed elevator loads to the supporting structure are shown on the Drawings, including machine beam reactions, car buffer reactions, counterweight buffer reactions, and horizontal and vertical guide rail loads. The General Contractor shall submit to the Structural Engineer final elevator shop drawings showing all loads to the structure prior to the installation of the elevators for verification of load carrying capacity.
2. Machine Room Live Load: Elevator machine rooms are designed for a uniform (unreduced) live load of 100 psf.

E. SNOW LOADS
1. Ground snow load, pg = 0 psf

F. RAIN LOADS
1. 15-min Precipitation Intensity = 8.64 in./hr
2. 60-min Precipitation Intensity = 4.51 in./hr

G. SEISMIC LOADS (per ASCE 7-10)

1. Site classification	D
2. Seismic importance factor	I
3. Occupancy category	II
4. Mapped spectral response accelerations	Ss = 0.107 S1 = 0.056
5. Spectral response coefficients	Sds=0.114 Sd1 = 0.089
6. Seismic design category	B
7. Basic Seismic Force Resisting system	**<PER ASCE 7>**
8. Design base shear	_____ kips
9. Seismic response coefficient (Cs)	_____
10. Response modification factor (R)	_____
11. Analysis procedure (ELFP or MRSA):	ELFP

IV. FOUNDATION - GENERAL

A. GEOTECHNICAL REPORT
Foundation design is based on the following geotechnical report:
Proposed "Geotechnical Engineering Report, Old Perkins Road Site, Geo-Prairieville, LA"
Prepared by: Terracon
Report No. EH25511235
Dated: 11/06/2025

All recommendations therein that relate to the work shown on these drawings shall be followed.

B. FOUNDATION APPROVAL AND INSPECTION BY AUTHORIZED INSPECTOR
1. Unless authorized otherwise by the Owner or Architect, the General Contractor shall notify the Geotechnical Engineer or other authorized inspector for review of foundation bearing surface, inspection of foundation installation, and foundation installation methods (including construction dewatering methods where required) and shall not place concrete prior to inspector's approval.

C. FOUNDATION REINFORCING STEEL INSPECTION
1. The Contractor shall notify the special inspector that the work is ready for inspection at least 24 hours before inspection.
2. The Contractor shall not pour any foundation concrete without inspection and approval by the special inspector, including but not limited to mix design verification, rebar placement, rebar welding, and concrete placement. Refer to the Special Inspection section of these General Notes for more information.

D. GRADE BEAMS
1. Grade Beam Side Forms.
   a. Form exposed faces of grade beams.
2. Grade beam bottom steel shall be chaired at 5 foot maximum centers using beam bolsters providing 3 inch bottom cover to reinforcing steel. Beam bolsters used shall be intended for support on soil.
3. Bottom bars of grade beam shall be spliced (where required) at footings only. Top bars shall be spliced (where required) at the midspan of the grade beams. Lap length shall be 36 times bar diameter unless specified otherwise. Provide corner bars to match top and bottom steel.

E. SLAB-ON-GRADE CONSTRUCTION
1. Slab on grade construction shall follow the recommendations of Guide for Concrete Floor and Slab Construction ACI 302.1R and ACI 302.2R.
   a. Floor Flatness / Floor Levelness (FF/FL) = 25/20 Office.
   b. Floor Flatness / Floor Levelness (FF/FL) = 45/35 Warehouse.
2. Subgrade Preparation shall conform to the soils report noted above. If report does not specify the amount of select fill, provide fill as required above to reduce PVR to 1 inch.
3. Cranes for erecting tilt wall panels are not allowed on the floor slab.

F. VAPOR RETARDER
1. Vapor retarder shall: have no more than 0.01 perms when tested in accordance with ASTM E-96; meet or exceed the requirements of ASTM E-1745, Class A; (U.N.O.) in accordance with ACI 302.2R.
   Acceptable products include:
   a. Stego Wrap, 15 mils, by Stego Industries LLC
   b. Husky Yellow Guard, 15 mils, by Poly-America
   c. Approved equal conforming to performance requirements
2. All seams shall be overlapped a minimum of 6 inches and taped with polyethylene tape. All penetrations, block-outs, and openings shall be sealed using a combination of the vapor retarder and polyethylene tape. Tape shall be provided by vapor retarder vendor.
3. Review of the installed product by a representative of the supplier is recommended.

V. DRILLED PIER FOUNDATION

A. DESIGN SOIL PRESSURES
1. Total allowable pier capacities were calculated using the ultimate capacities per the geotechnical report (shown below) and an ASD factor of safety of 2.0.

DRILLED SHAFT AXIAL RESISTANCE SUMMARY		
DRILLED SHAFT DIAMETER (INCHES)	ULTIMATE COMPRESSION CAPACITY (KIPS)	ULTIMATE TENSION CAPACITY (KIPS)
24	280	230
30	370	260
36	470	340

2. Static load test to be performed per the geotechnical recommendations.

B. FIELD INSPECTION OF BEARING STRATUM
1. The bearing stratum of each drilled pier shall be inspected and approved by the Geotechnical Engineer or other authorized inspector prior to pouring of concrete.

C. BEARING ELEVATION
1. The bottom elevation of piers is shown on the drawings for bid purposes. The actual required bearing elevation may vary as required to provide proper capacity as determined by the Geotechnical Engineer. Footings shall be poured immediately after excavation.

D. PIER CAPS AND WIDENED GRADE BEAMS
1. Refer to Drawings for requirements at pier caps and widened grade beams.

VI. CONCRETE

A. SPECIFICATIONS
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. Concrete shall comply with the requirements of ACI 318 and ACI 301.
3. Type I/II Portland Cement conforming to ASTM C150 shall be used, unless noted otherwise. The use of alternative cement types (such as Type IL Cement) shall be submitted to the Engineer-of-Record for approval prior to producing a concrete mix design submittal. See note 4 for Type IL Cement requirements.
4. Type IL Blended Cement shall conform to ASTM C595. If concrete mix designs utilize Type IL Cement, they shall have a different mix number from Type I/II mix designs and the data for mix verification (historical data or trial batches) shall be from mixes utilizing Type IL Cement.
5. Fly ash conforming to ASTM C618, Type C or F may be used. The maximum amount of fly ash shall be 25% of the total cementitious material by weight.
6. Aggregates shall conform to ASTM C33 for normal-weight concrete and ASTM C330 for lightweight concrete.
7. Water used in mixing concrete shall conform to ASTM C94.

B. CLASSES OF CONCRETE
1. All concrete shall conform to the requirements as specified in the table below unless noted otherwise on the Drawings:

USAGE	28 DAY COMP. CONC. STRENGTH (PSI)	TYPE	NOMINAL MAX. AGGREGATE SIZE
Grade Beams	4000	NW	1 1/2"
Slab-on-Grade	4000	NW	1 1/2"
Drilled Piers	3000	NW	1"
Elevator Pit Walls and Slab	4000	NW	1 1/2"
Steel Deck Slabs	3000	NW	1"
Topping Slab	3000	NW	3/4"
Retaining Walls	4000	NW	1"
Tilt Up Walls	4000	NW	1"
Shallow Spread and Strip Footings	3000	NW	1 1/2"

Note: NW = Normal-weight concrete

2. Concrete for slab-on-grade shall have a maximum water-cement ratio of 0.50.
3. Admixtures used shall be compatible with floor treatments.
4. Concrete at interior slabs shall not have air-entraining admixtures.
5. There shall be no horizontal cold joint in any concrete pour.

C. CONCRETE MIX DESIGNS
1. Concrete mix designs must be submitted a minimum of 15 days prior to the start of the work for Engineer and Owner's testing laboratory approval prior to placement of concrete in the field.
2. Any adjustments in approved mix designs including changes in admixtures must be submitted in writing to the Engineer and Owner's testing laboratory for approval prior to use in the field.
3. Concrete designed to be pumped shall be noted so on the mix designs and shall have mix proportions compatible with the pumping process.
4. Mix designs shall be proportioned based upon trial batching or experience as required by ACI 318.

D. EPOXY ADHESIVES
1. Typical epoxy adhesive for drilling and setting reinforcing into previously-cast, or existing, concrete shall be Hilti HY 200 system, or approved equivalent. Reference manufacturer specifications for installation procedures.

VII. REINFORCING STEEL

A. SPECIFICATION
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. Reinforcement shall comply with the requirements of ACI 315 and ACI 318.
3. All steel reinforcement shall conform to ASTM A615, Grade 60.
4. All welded reinforcing steel shall conform to ASTM A706.
5. All welded smooth wire fabric shall conform to ASTM A1064 (Yield strength = 65,000 psi).
6. All welded deformed wire fabric shall conform to ASTM A1064 (Yield strength = 70,000 psi).
7. All welded wire fabric shall be furnished in flat sheets only.
8. Welding shall comply with the requirements of AWS D1.4.

B. DETAILING AND BAR SUPPORTS
1. Detailing of and bar supports for reinforcing steel shall be in accordance with the ACI Standard Details and Detailing of Concrete Reinforcement as reported by ACI Committee 315. All continuous reinforcing steel shall be lapped using a Class B lap splice, unless specified otherwise.

C. MANUAL OF CONCRETE PRACTICE
1. Unless noted otherwise, methods of estimating, detailing, fabricating, placing and contracting for reinforcing materials shall follow the Manual of Standard Practice as published by the Concrete Reinforcing Steel Institute.

D. PLACEMENT OF WELDED WIRE FABRIC
1. Welded wire fabric shall be continuous across the entire concrete surface and not be interrupted by beams or girders and properly lapped one cross wire spacing plus 2 inches.

E. REINFORCING STEEL COVERAGE
Reinforcing steel coverage should conform to the requirements specified below. The reinforcing steel detailer shall adjust reinforcing steel cage sizes at intersecting structural members as required to allow clearance for intersecting reinforcing bar layers maintaining minimum specified cover. Cover in structural members not specified below shall conform to the requirements of ACI 318 Section 7.7 unless specified otherwise on the drawings.

1. Foundation Members

a. Grade Beams	1 1/2" top, 3" bottom, 2" sides (3" sides if cast against soil)
b. Drilled Piers	3" sides
c. Interior Slab-on-Grade	1 1/2" top cover for one layer of steel (u.n.o. on contraction joint typical detail) 1 1/2" top cover (u.n.o. on contraction joint typical detail), 3" bottom cover for two layers of steel.
d. Exterior Slab-on-Grade	2" top cover for one layer of steel. 2" top cover, 3" bottom cover for two layers of steel.
e. Basement Walls	2" dirt face, 3/4" interior face
f. Foundation Retaining Walls	2" both faces
g. Sump Walls, Pit Walls	2" both faces
h. Shallow Spread and Strip Footings	3" bottom and sides, 2" top
i. Industrial Slab-on-Grade	2" top cover for one layer of steel
j. Tilt-Up Panels	1" outside face from back of reveals, 3/4" interior face, 1" edges

VIII. POST-INSTALLED ANCHORS

A. GENERAL
1. Except where indicated on the drawings, post-installed anchors shall consist of the following anchor types as provided by Hilti, Inc. or approved equal.

B. ANCHORAGE TO CONCRETE
1. Adhesive Anchors
   a. Adhesive anchors for cracked and uncracked concrete use:
      i. Hilti HIT-HY 200 V3 Safe Set System with the Hilti HIT-Z rod per ICC ESR-4868 or approved equal
      ii. Hilti HIT-HY 200 V3 Safe Set System with Hilti hollow drill bit and VC 150/300 with Hilti HAS threaded rod per ICC ESR-4868 or approved equal
      iii. Hilti HIT-RE 500 V3 Safe Set System with Hilti hollow drill bit and VC 150/300 with Hilti HAS threaded rod per ICC ESR-3814 or approved equal
      iv. Hilti HIT-RE 500 V3 Safe Set System with Hilti HIT-RT roughening tool with Hilti HAS threaded rod per ICC ESR-3814 for diamond cored holes or approved equal
   b. Basis of design includes the following design parameters:
      i. Cracked concrete
      ii. Water-saturated concrete
      iii. Base material temperature of 23 to 104 degrees Fahrenheit
      iv. Allowable with hammer-drill, hollow drill bit system, and core drilling methods
2. Mechanical Anchors
   a. Medium duty mechanical anchors for cracked and uncracked concrete use:
      i. Hilti Kwik Bolt-TZ2 expansion anchors Safe Set System with hollow drill bit and VC 150/300 and SI-AT-A22 Adaptive Torque Module per ICC ESR-4266 or approved equal
      ii. Hilti Kwik Bolt 3 expansion anchors safe set system with hollow drill bit and VC 150/300 and SI-AT-A22 with adaptive torque (uncracked concrete only) per ICC ESR-3027 or approved equal
      iii. Hilti Kwik HUS EZ and Kwik HUS-EZ-I/E screw anchors Safe Set System with hollow drill bit and VC 150/300 per ICC ESR-3027 or approved equal
   b. Heavy duty mechanical anchors for cracked and uncracked concrete use:
      i. Hilti HDA undercut anchors per ICC ESR-1546 or approved equal
      ii. Hilti HSL-3 expansion anchors per ICC ESR-1545 or approved equal
   c. Mechanical anchors specified as carbon steel may only be used in dry hole conditions. It is the responsibility of the contractor to provide dry hole conditions for installation, or they shall contact the engineer of record for a stainless steel anchor alternative.

C. GENERAL REQUIREMENTS
1. Anchor capacity used in design shall be based on the technical data published by Hilti or such other method as approved by the structural engineer of record. Substitution requests for alternate products must be approved in writing by the structural engineer of record prior to use. Contractor shall provide calculations that have been sealed by another licensed engineer demonstrating that the substituted product is capable of meeting the performance of the specified product. Substitutions will be evaluated by their having an ICC ESR showing compliance with the relevant building code for seismic uses, load resistance, installation category, and availability of comprehensive installation instructions. Adhesive anchor evaluation will also consider creep, in-service temperature, installation temperature, moisture condition of concrete, and drilling methods.
2. Install anchors per the Manufacturer Printed Installation Instructions (MPII), as included in the anchor packaging.
3. Adhesive anchors in upwardly-inclined orientation and/or embedment depths greater than 10 inches must be installed using the Hilti Profis Piston Plug System.
4. The contractor shall arrange an anchor manufacturer's representative to provide onsite installation training for all of the anchoring products specified. The structural engineer of record must receive documented confirmation that all personnel who install anchors are trained prior to the commencement of anchor installation.
5. Anchor capacity is dependent upon spacing between adjacent anchors and proximity of anchors to edge of concrete. Install anchors in accordance with spacing and edge clearances indicated on the drawings.
6. Existing reinforcing bars in the concrete structure may conflict with specific anchor locations. Unless noted on the drawings that the bars can be cut, the contractor shall review the existing structural drawings and shall undertake to locate the position of the reinforcing bars at the locations of the concrete anchors by ferroscan, GPR, x-ray, or other means.
7. All hole conditions are assumed to be dry or water-saturated, unless noted otherwise. The contractor shall be responsible for removing water in order to avoid water-filled hole or submerged hole conditions before installing anchors.

IX. POST-INSTALLED REBAR

A. GENERAL
1. Except where indicated on the drawings, post-installed rebar shall use the following products as provided by Hilti, Inc. or approved equal.

B. REBAR DOWELING INTO CONCRETE
1. Adhesive Anchors
   a. Adhesive anchors for cracked and uncracked concrete use:
      i. Hilti HIT-HY 200 V3 Safe Set System with the Hilti hollow drill bit and VC 150/300 with continuously deformed rebar per ICC ESR-4868 or approved equal
      ii. Hilti HIT-RE 500 V3 Safe Set System with Hilti hollow drill bit and VC 150/300 with continuously deformed rebar per ICC ESR-3814 or approved equal
      iii. Hilti HIT-RE 500 V3 Safe Set System with Hilti HIT-RT roughening tool with continuously deformed rebar per ICC ESR-3814 in diamond cored holes or approved equal
   b. Basis of design includes the following design parameters:
      i. Cracked concrete
      ii. Water-saturated concrete
      iii. Base material temperature of 23 to 104 degrees Fahrenheit
      iv. Allowable with hammer-drill, hollow drill bit system, and core drilling methods
      v. Current ICC-ES report with approval for development of bar using ACI provisions for embedment depths greater than 20 bar diameters

C. GENERAL REQUIREMENTS
1. Anchor capacity used in design shall be based on the technical data published by Hilti or such other method as approved by the structural engineer of record. Substitution requests for alternate products must be approved in writing by the structural engineer of record prior to use. Contractor shall provide calculations that have been sealed by another licensed engineer demonstrating that the substituted product is capable of meeting the performance of the specified product. Substitutions will be evaluated by their having an ICC ESR showing compliance with the relevant building code for seismic uses, load resistance, installation category, and availability of comprehensive installation instructions. Adhesive anchor evaluation will also consider creep, in-service temperature, installation temperature, moisture condition of concrete, and drilling methods.
2. Install anchors per the Manufacturer Printed Installation Instructions (MPII), as included in the anchor packaging.
3. Adhesive anchors in upwardly-inclined orientation and/or embedment depths greater than 10 inches must be installed using the Hilti Profi Piston Plug System.
4. The contractor shall arrange an anchor manufacturer's representative to provide onsite installation training for all of the anchoring products specified. The structural engineer of record must receive documented confirmation that all personnel who install anchors are trained prior to the commencement of anchor installation.
5. Anchor capacity is dependent upon spacing between adjacent anchors and proximity of anchors to edge of concrete. Install anchors in accordance with spacing and edge clearances indicated on the drawings.
6. Existing reinforcing bars in the concrete structure may conflict with specific anchor locations. Unless noted on the drawings that the bars can be cut, the contractor shall review the existing structural drawings and shall undertake to locate the position of the reinforcing bars at the locations of the concrete anchors by ferroscan, GPR, x-ray, or other means.
7. All hole conditions are assumed to be dry or water-saturated, unless noted otherwise. The contractor shall be responsible for removing water in order to avoid water-filled hole or submerged hole conditions before installing anchors.

X. CONCRETE FORMWORK

A. RESPONSIBILITY
1. The design, construction, and safety of all formwork shall be the responsibility of the General Contractor. All forms, shores, backshores, falsework, bracing, and other temporary supports shall be engineered to support all loads imposed including the wet weight of concrete, construction equipment, live loads, lateral loads due to wind and wet concrete imbalance. The Contractor shall also be responsible for determining when temporary supports, shores, backshores, and other bracing may be safely removed.

B. SUBMITTALS
1. The General Contractor shall submit for Owner's record only, formwork shop drawings. Formwork shop drawings shall include all items described in Paragraph A, including calculations. Formwork shop drawings shall be sealed by a registered Engineer in the state that the project is located.

XI. NON-SHRINK GROUT FOR BASE PLATES AND BEARING PLATES

A. SPECIFICATIONS
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. Non-shrink grout shall conform to Corps of Engineers Specification for Non-Shrink Grout, CE-CRD-C621.
3. Twenty-eight day compressive strength shall be 5,000 psi, as determined by grout cube tests.
4. Minimum thickness of grout under all base plates and bearing plates shall be 1 inch.

B. TYPE
1. Grout for base plates and bearing plates shall be a non-metallic, shrinkage resistant, premixed, non-corrosive, non-staining product containing Portland cement, silica sands, shrinkage compensating agents, and fluidity improving compounds.

C. PLACEMENT
1. Grout shall be placed in a fluid, flowable state under base plates that have a form built around them for grout confinement.
2. Grout shall be cured according to manufacturer's recommendations.

XII. STRUCTURAL STEEL

A. MATERIAL
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. Structural steel shall comply with the requirements of AISC 360 and AISC 303.
3. All hot rolled steel plates, shapes and bars shall be new steel conforming to ASTM Specification A6.
4. All Wide Flange W-Shapes shall conform to ASTM A992, Grade 50.
5. All Wide Flange M- and S- Shapes shall conform to ASTM A36.
6. All Wide Flange HP- Shapes shall conform to ASTM 572, Grade 50.
7. All Hollow Structural Shapes (HSS) shall conform to ASTM A500, Grade C.
8. All Pipe Sections shall conform to ASTM A53, Grade B.
9. All Channels, Angles and Plates shall conform to ASTM A36.
10. All anchor rods shall conform to ASTM F1554, Grade 36.
11. All nuts shall conform to ASTM A563.
12. All washers shall conform to ASTM F436.
13. All headed stud anchors shall conform to ASTM A108, Type B (Yield Strength = 51 ksi).
14. All deformed bar anchors shall conform to ASTM A1064, Type C (Yield Strength = 70 ksi).

B. CONNECTIONS
1. Typical connection details are indicated on the Drawings.
2. The design of all steel connections shall be performed under the direct supervision of a registered professional engineer in the state where the project is located, employed by the fabricator. Calculations sealed by the fabricator's professional engineer must be submitted, if requested.
3. It is the intention of the plans and specifications that shop connections be welded or bolted and that field connections be bolted, unless detailed otherwise on the Drawings.
4. All typical beam simple connections shall be standard double angle or single angle framed beam connections. Shear tab connections may be used at locations where double angle connections are not possible. Seated beam connections shall not be used unless indicated in the Drawings. Provide full depth shear tab if beam frames on only one side of a girder.
5. The beam connections shall be designed for the reactions listed below. Any connection capacities that do not meet or exceed the reactions listed below shall be clearly indicated on the beam piece marks and submitted for review as part of the shop drawing review process.
   a. Non-Composite beams: Design connections to support a reaction R (unless specified otherwise) equal to one half the total uniform load capacity from the table of Uniform Load Constants in the AISC Manual.
   b. Composite beams: Support a reaction R (unless specified otherwise) equal to a multiplier, defined below, times one-half the total uniform load capacity from the table of Max Total Uniform Load.

Beam Size	Multiplier	Beam Size	Multiplier
W8	2.75	W21	1.75
W10	2.55	W24	1.70
W12	2.35	W27	1.60
W14	2.00	W30	1.50
W16	1.90	W33	1.45
W18	1.85	W36	1.40

Add to the reaction listed above, any loads or reactions of members supported by the beam within three feet of beam end and the vertical components of forces in brace members framing into the beams.

6. Bracing connections shall develop full tensile forces at each end of the bracing member unless bracing forces are specified on the Drawings.
7. All connections designated in the documents as moment connections shall be designed to develop the full flexural capacity of the member that has the lesser flexural capacity of the members at the connection. In cases where multiple beams are moment connected to a single column, each beam-to-column interface must be designed for the full flexural capacity of the member that has the lesser flexural capacity.
8. All continuous deck edge angles and bent plates shall use full penetration butt welds at splices.
9. Steel-to-Aluminum Connections
   a. Provide Neoprene washers to fully separate aluminum and steel materials. Use only stainless steel fasteners with complete separation of steel from aluminum.

C. GALVANIZING
1. All steel exposed to weather or outside the building's waterproofing, such as brick shelf angles, shall be hot-dipped galvanized after fabrication.
2. All steel surfaces to be hot dip galvanized shall be prepared as specified by the Steel Structures Painting Council (SSPC).
3. The zinc coating for steel shapes and plates shall average not less than 2.3 oz. with no individual thickness less than 2.0 oz.
4. Galvanize all nuts, bolts, and washers used in the connection of galvanized steel.
5. Protect all field welded connections with "Z.R.C. Cold Galvanizing Compound" as manufactured by Z.R.C. Product Company or approved equal.

D. STAIRS, HANDRAILS, AND ACCESS LADDERS
1. Stairs, handrails, guardrails and access ladders (including attachment to structure) shall be designed by the fabricator for the loads indicated in IBC Chapter 16, OSHA, and other requirements noted in the architectural drawings. For access ladders not supported by concrete walls, all landings and steel columns shall be included in the fabricator's scope of work. Shop drawings shall be sealed by a Professional Engineer, representing the fabricator, in the state where the project is located.

E. ALUMINUM CANOPIES
1. Steel elements supporting aluminum canopies (designed by others) have been designed based on preliminary assumptions of canopy reactions. The canopy structure, and connection of canopy to the steel, shall be designed by the canopy supplier's engineer. GC to provide structural engineer with final canopy reactions prior to fabricating steel, to confirm final structural design is sufficient for canopy support.

XIII. STRUCTURAL BOLTS AND THREADED FASTENERS

A. SPECIFICATION
1. All bolts shall conform to ASTM F3125, Grade A325, Type 1, unless noted otherwise in the drawings.
2. All nuts shall conform to ASTM A563.
3. All washers shall conform to ASTM F436.

B. DESIGN
1. Minimum bolt diameter shall be 3/4 inch.
2. Connection Type: Unless noted otherwise on the Drawings or in these General Notes, all bolted connections shall be bearing type connections using standard holes (hole diameter nominally 1/16 inch in excess of nominal bolt diameter with) threads included in the shear planes.

C. INSTALLATION
1. Fastener Tension: High strength bearing bolts shall be tightened using an impact wrench to a snug tight condition. The snug tight condition is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench.
2. All bolts shall be new and shall not be re-used.

XIV. WELDING OF STRUCTURAL STEEL

A. WELDER CERTIFICATION
1. All shop and field welders shall be certified according to AWS procedures for the welding process and welding position used.

B. MINIMUM SIZE AND STRENGTH
1. Fillet Welds: Minimum size of fillet welds shall be as specified in the AISC Manual.
2. Partial Penetration Groove Welds: The minimum effective throat thickness of partial penetration groove welds shall be as specified in the AISC Manual.
3. Minimum Strength of Welded Connections: Unless noted otherwise on the drawings, all shop and field welds shall develop the full tensile strength of the member or elements jointed.
   a. All connections designated in the documents as moment connections shall be designed to develop the full flexural capacity of the member that has the lesser flexural capacity of the members at the connection, unless noted otherwise in the Drawings. In cases where multiple beams are moment connected to a single column, each beam-to-column interface must be designed for the full flexural capacity of the member that has the lesser flexural capacity, unless noted otherwise in the Drawings.
4. Connection of all miscellaneous steel shall consist of 1/4" fillet welds all-around (minimum) if no other connection information is provided on the structural drawings.
5. At slotted connections, and anywhere a gap may exist between base metal and connecting material, weld size shall be increased to account for gap width (per AWS recommendations).

C. FILLER METAL REQUIREMENTS
1. Strength: Weld shall be as specified in the AISC Manual.
2. Electrodes. Electrodes for various welding processes shall be as specified below:
   a. SMAW: E70XX low hydrogen
   b. SAW: F7X-EXXX

D. WELDING
1. All welding shall comply with the requirements of AWS.
2. All full penetration welds shall be tested to verify compliance, unless noted otherwise.
3. All fillet welds shall be visually inspected, unless noted otherwise.

XV. OPEN WEB STEEL JOISTS

A. SPECIFICATIONS
1. Open web steel joists and joist girders shall be designed, manufactured and erected per the Steel Joist Institute (SJI) Specifications.
2. Joists and joist girders shall be cambered for dead load. Provide standard SJI camber unless specified otherwise on the Drawings.
3. Provide top chord extensions where shown on the Drawings.
4. Provide flat bearing for all joists and joist girders. Provide minimum end bearing of 2 1/2 inches for K-series joists and 4 inches for LH and DLH joists. If sufficient bearing does not exist, stagger the end of each joist and center bearing over center of support.
5. Provide horizontal and diagonal bridging as required by SJI Standards and Specifications.
6. Provide standard SJI bearing depth for all joists and joist girders. When K-series joists bear on the same beam or joist girder as LH- or DLH-series joists, seat depth shall match.
7. Top chord of joists shall be double angles or tees.
8. Provide minimum end anchorage to steel and masonry, as required by SJI Standards and Specifications.
9. Joists and joist girders shall be shop painted per Architectural specifications.
10. End connections for joists and joist girders with bottom chord extensions shall only be completed after the full dead load is applied.

B. DESIGN
1. The design of all open web joists shall conform to the codes and specifications cited above.
2. All joists shall carry the design loads as specified in the SJI load tables as the minimum requirement. Additionally, the joists shall be designed to carry any other load types and patterns as indicated on the structural drawings. Examine the structural drawings for any other loads required to be carried by any joist.
3. Roof joists shall be designed for a net service level uplift pressure = 30 PSF, unless noted otherwise in the wind pressure diagrams.
4. Increase size as required to conform with Factory Mutual and U.L. requirements.
5. Design shall include loads for HVAC units suspended from or supported on the roof. See Architectural and Mechanical Drawings for locations of units.
6. Joist extensions shall match joist capacity, unless noted otherwise in the drawings.
7. For roof joists with slopes greater than 1/4-inch per foot, the joist seat shall be designed for a 3,000 lb longitudinal point load parallel to the joist applied at the joist bearing elevation (unless noted otherwise in roof framing details).

C. SUBMITTAL
1. Joist manufacturer shall submit the following for Engineer's review and approval:
   a. Produce certification letter that joists and joist girders comply with all documents noted in these general notes.
   b. Shop drawings showing joists layout, bridging and other accessories.
   c. Design calculations for all joists showing load carrying capacity. Calculations shall be signed and sealed by a registered Engineer. Calculations are for Owner's record only.

D. OSHA REQUIREMENTS
1. Comply with all OSHA requirements. Provide additional steel, bridging, etc. as required. Provide vertical plates shop welded to structural steel columns and/or beams and bottom chord extensions at joists at columns, as required by OSHA.

XVI. STEEL ROOF DECK (WITH RIGID INSULATION BOARD)

A. SPECIFICATIONS
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. The design, fabrication, and erection of all roof deck shall conform to the Steel Deck Institute (SDI) Standards.
3. Galvanized steel deck shall conform to ASTM A653 Structural Quality, with a minimum yield strength of 80 ksi. Steel deck shall be galvanized with a protective zinc coating conforming to ASTM A653 G90 class.
4. Steel deck accessories shall conform to ASTM A653 Structural Quality.

B. ROOF DECK SCHEDULE
1. Roof deck shall be 1.5B (22 gauge, unless noted otherwise on plan), as manufactured by Vulcraft, or roof deck with equivalent section properties.
2. Roof deck shall be Toris 5.5A (20 gauge) as manufactured by Epic Metals, or roof deck with equivalent section properties.

C. ATTACHMENT
1. Roof deck shall be attached to structural supports as follows unless noted otherwise on plan:

For use with 1.5B deck suppoted by joists and WF beams

Attachment Type	Hilti X-HSN 24
Attachment Pattern	36/7
Sidelap fastener	Hilti SLC
Sidelap spacing	12" o.c.
Perimeter spacing	6" o.c.

For use with Toris 5.5A deck suppoted by joists and WF beams

Attachment Type	3/4" puddle
Attachment Pattern	24/3
Sidelap fastener	#10 TEK screws
Sidelap spacing	36" o.c.
Perimeter spacing	36" o.c.

   a. Welds shall penetrate all layers of deck material at end laps and side joints and shall have good fusion to the supporting members.
   b. Welding washers shall be used when welding steel deck less than 0.028 inches thick.
2. Deck supported on cold-formed light gage framing (C-Joists or trusses) shall be attached to each support member using #12 TEK sheet screws at all ribs where sides lap and each rib in between.
3. Provide a minimum end bearing of 2 inches over supports.
4. End laps of sheets shall be a minimum of two inches and shall occur over supports.
5. Provide roof deck construction which is listed and conforms to the UL Fire Resistance Directory and Building Materials Directory. Deck attachment shall be increased above the minimum specified herein as required to achieve a Class 90 uplift rating.

D. DECK SPANS
1. Deck spans shall not exceed the maximum center to center spans as required by SDI criteria. Where possible, all steel deck shall extend over three or more supports. Two span deck shall be used only where deck layout does not permit the use of three spans. Single span deck is not permitted.

E. ACCESSORIES
1. Use the following accessories as required to provide a finished surface for the application of insulation and roofing:
   a. Ridge, hip and valley plates (1/4 inch)
   b. Cant strips (20 gauge)
   c. Sump pans (14 gauge)
   d. Inside or outside closure channels (20 gauge)
   e. Butt strips at change of deck directions (20 gauge)
   f. Filler sheets (20 gauge)
   g. Rubber closures

F. ROOF OPENINGS
1. Roof openings less than 6 inch square or diameter require no reinforcement. Openings 6 inch to 10 inch shall be reinforced with a 20 gauge galvanized plate welded to the deck at each corner and 6 inch maximum centers with a 5/8 inch diameter puddle weld or sheet steel screws.
2. Unless indicated otherwise on the drawings, openings over 10 inch wide or diameter shall be reinforced with an angle L 2 1/2 x 2 1/2 x 1/4 framing each side of the opening and spanning between supports for spans 4 feet or less and L 3 x 3 x 1/4 for spans greater than 4 feet but less than 6 feet unless specified otherwise on drawings.
3. Larger openings shall be referred to the Engineer for framing.

G. SUBMITTALS
1. The steel roof deck manufacturer shall submit the following:
   a. Product specifications and installation instructions for each type of deck.
   b. A certificate of product compliance with SDI Standards and these General Notes.
   c. Shop drawings showing complete deck lay

XVII. COMPOSITE STEEL FLOOR DECK

A. SPECIFICATION
1. All specifications shall be as noted below, unless noted otherwise in the drawings.
2. The design, fabrication, and erection of all roof deck shall conform to the Steel Deck Institute (SDI) Standards.
3. Galvanized steel deck shall conform to ASTM A653 Structural Quality, with a minimum yield strength of 50 ksi (or an Engineer approved equal). Steel deck shall be galvanized with a protective zinc coating conforming to ASTM A653 G60 class.
4. Steel deck accessories shall conform to ASTM A653 Structural Quality.
5. Refer to CONCRETE section for concrete requirements.
6. Refer to REINFORCING STEEL section for steel reinforcement requirements.

B. FLOOR DECK AND SLAB SCHEDULE
1. Provide, as specified at each location scheduled in the table below, composite steel deck with the indicated depth of deck, minimum deck thickness, concrete type, total slab thickness, and slab reinforcement.

Location	Deck Depth	Min. Deck Thk.	Concrete Type	Total Slab Thk.	Slab Reinforcing
Typ floor	2 in	18 ga.	NW	5 1/2 in	#3 @ 15" O.C.

NW = Normal-weight Concrete

2. The average rib width to depth of deck ratio shall be greater than or equal to 2.0.
3. Refer to DESIGN CRITERIA section of General Notes for the total design superimposed loads.
4. The deck thickness specified above shall be considered the minimum thickness. The deck manufacturer shall be responsible to select the required deck thickness, if greater than shown in the table above, to carry the design superimposed live load indicated plus 25 psf dead load for the spans shown on the drawings and meeting all performance criteria as specified by the Steel Deck Institute.
5. Reinforcing shall be located at the top of the concrete slab.

C. ATTACHMENT
1. Floor deck units shall be welded to each structural support member using 5/8 inch diameter puddle welds at each end of sheet and each intermediate support beginning at edge rib and at a maximum spacing of 12 inch centers, conforming to a 36/4 pattern (or an equivalent pattern for decking with alternate widths to Vulcraft).
   a. Welds shall penetrate all layers of deck material at end laps and side joints and shall have good fusion to the supporting members.
   b. Welding washers shall be used when welding steel deck less than 0.028 inches thick.
2. Side laps of adjacent units shall be fastened by seam weld or button punching so that spacing between supports and fasteners does not exceed 36 inches.
3. Provide a minimum end bearing of 2 inches over supports.
4. Sheets shall not be lapped at ends, unless noted otherwise.

D. DECK SPANS
1. Deck spans shall not exceed the maximum center to center spans as required by SDI standards. Where possible, all steel deck shall extend over three or more supports. Two span deck shall be used only where deck layout does not permit the use of three spans. Single span deck is not permitted. All deck shall be designed as unshored construction unless noted otherwise.
2. The deck manufacturer shall be responsible to select the required deck thickness, if greater than shown in the table above, so that the allowable construction clear span (as specified by the Steel Deck Institute) meets or exceeds the spans shown on the drawings.

E. CHLORIDE ADMIXTURES
1. The use of admixtures in concrete containing chloride salts shall not be permitted for concrete slab.

F. EXTRA CONCRETE REQUIRED BY DECK DEFLECTION
1. The General Contractor shall include in his bid additional concrete required for steel deck slabs to account for deck deflection.

G. SHEET CLOSURES
1. Steel deck manufacturer shall furnish sheet closures at all slab edges, columns, walls, and openings unless steel angles or plates are specified in details on the drawings.
2. Closures shall be minimum 14 gage cold formed steel welded to edge supports with 1 inch long weld at 12" maximum centers. Provide 2" minimum bearing over steel support.
3. Closures and support welds shall be designed to support the wet concrete weight and a 150 pound concentrated load acting at the slab edge without exceeding a stress of 0.8 Fy; and the wet concrete weight alone without exceeding a stress of 0.6 Fy and a deflection of 3/4" maximum.
4. Provide sheet steel closures for open ends of panels where deck stops or changes direction.

H. FLOOR OPENINGS
1. For all openings in composite deck not framed with structural steel and greater than 10 inches in width in either direction, provide 2 - # 5 x opening width plus 2'-0" in a direction perpendicular to deck ribs each side of opening with bars bearing to deck ribs each side of opening with bars bearing on top of ribs and 2 - # 5 x deck span plus 1'-0" each side of opening chaired 3/4" up from bottom of nearest deck rib running beside the opening from support to support unless heavier reinforcement is indicated on the drawings.
2. Provide blockout in slab for opening with deck uncut. Cut deck at opening after concrete has reached 75% of its design strength.

I. TOPPING SLABS
1. Any additional concrete topping specified over the composite deck slab shall be placed after the slab has reached 75% of its 28-day strength.

J. SUBMITTALS
1. The composite steel deck manufacturer shall submit the following:
   a. Product specifications and installation instructions for each type of deck.
   b. A certificate of product compliance with SDI Standards and these General Notes.
   c. Design calculations sealed by a registered engineer in the state where the project is located verifying compliance for all span conditions with SDI Standards and these General Notes.
   d. Shop drawings showing complete deck layout, attachment details, accessories, and all other details for Engineer review and approval prior to the start of erection.

XVIII. HEADED STUD ANCHORS

A. PROPERTIES
1. All headed stud anchors shall conform to ASTM A108, Type B (Yield Strength = 51 ksi).
2. Stud diameter and length shall be 3/4 inch diameter x 3-7/8 inch long before welding.
3. Stud diameter and length for embed plates shall be as noted on the drawings.

B. INSTALLATION
1. Composite beam shear studs shall be welded in the field (not the shop) using automatically timed stud welding equipment.
2. The top flange of the beam must be unpainted and free of heavy rust, mill scale, and other foreign materials.
3. Headed stud anchors for embed plates shall be welded in the shop using automatically timed stud welding equipment.

XIX. SUBMITTALS

A. SHOP DRAWINGS
1. The General Contractor shall submit for Engineer review shop drawings for the following items:
   a. Concrete Mix Designs
   b. Reinforcing Steel
   c. Structural Steel (*)
   d. Miscellaneous Steel
   e. Steel Deck
   f. Bar Joists (*)
   g. Guard Rails and Hand Rails (*)
   h. Steel Stair (*)
   i. Tilt Wall (Lifting / Bracing / Erection) (*,#)
   j. Light-gage steel (*,#)
   k. Concrete Masonry Product Data
   l. Curtain Wall (*,#)
   m. Aluminum Canopies (*,#)
   n. Steel Trusses (*)
   o. Skylight (#)
   p. Grating
   q. Fire Sprinkler System (#)

   Items marked (*) shall have shop drawings sealed by a registered engineer in the state where the project is located. Items marked (#) shall be submitted to Engineer for Owner's record only and will not have Engineer's shop drawing stamp.

2. All shop drawings must be reviewed and sealed by the General Contractor prior to submittal.
3. The omission from the shop drawings of any material required by the Contract Documents to be furnished shall not relieve the contractor of the responsibility of furnishing and installing such materials, regardless of whether the shop drawings have been reviewed and approved.

B. MANUFACTURER'S LITERATURE
1. Submit manufacturer's literature for all materials and products used in construction on the project.

C. REPRODUCTION
1. The use of reproductions of these Contract Documents by any contractor, subcontractor, erector, fabricator, or material supplier in lieu of preparation of shop drawings signifies his acceptance of all information shown herein as correct, and obligates himself to any job expense, real or implied, arising due to any errors that may occur hereon.

XX. MISCELLANEOUS

A. CONTRACT DOCUMENTS
1. It is the responsibility of the General Contractor to obtain all Contract Documents and latest addenda and to submit such documents to all subcontractors and material suppliers prior to the submittal of shop drawings, fabrication of any structural members, and erection in the field.

B. DRAWING CONFLICTS
1. The General Contractor shall compare the Architectural and Structural drawings and report any discrepancy between each set of drawings and within each set of drawings to the Architect and Engineer prior to the fabrication and installation of any structural members.

C. EXISTING CONDITIONS
1. The General Contractor shall verify all dimensions and existing conditions at the job site and report any discrepancies from assumed conditions shown on the drawings to the Architect and Engineer prior to the fabrication and erection of any members.

D. RESPONSIBILITY OF THE CONTRACTOR FOR STABILITY OF THE STRUCTURE DURING CONSTRUCTION
1. All structural elements of the project have been designed by the Structural Engineer to resist the required code vertical and lateral forces that could occur in the final completed structure only. It is the responsibility of the Contractor to provide all required bracing during construction to maintain the stability and safety of all structural elements during the construction process until the structure is tied together and completed.

E. HORIZONTAL CONSTRUCTION JOINTS IN CONCRETE POURS
1. There shall be no horizontal construction joints in any concrete pours unless shown on the drawings. All deviations or additional joints shall be approved in writing by the Architect/Engineer.

XXI. SITE OBSERVATION BY THE STRUCTURAL ENGINEER

A. GENERAL
1. The contract structural drawings and specifications represent the finished structure, and except where specifically shown, do not indicate the method or means of construction. The Contractor shall supervise and direct the work and shall be solely responsible for all construction means, methods, and procedures, techniques, and sequence.
2. The Engineer shall not have control or charge of, and shall not be responsible for, construction means, methods, techniques, sequences, or procedures, for safety precautions and programs in connection with the work, for the acts or omission of the Contractor, Subcontractor, or any other persons performing any of the work, or for the failure of any of them to carry out the work in accordance with the contract documents.
3. Periodic site observations by field representatives are solely for the purpose of becoming generally familiar with the progress and quantity of the work completed and determining, in general, if the work observed is being performed in a manner indicating that the work, when fully completed, will be accordance with the structural contract documents. This limited site observation should not be construed as exhaustive or continuous to check the quality or quantity of the work, but rather periodic in an effort to keep the Owner reasonably informed about the progress and quality of the portion of the structure completed.

XXII. SPECIAL INSPECTION

A. GENERAL
1. Special inspection is required per Chapter 17 of the International Building Code (IBC).
2. The special inspector shall be a qualified person who shall demonstrate competence to the building official for inspection of the particular type of construction requiring special inspection.
3. Prior to the start of construction, a pre-construction meeting with the architect, engineer, building official, contractor, and special inspectors shall be called to review the special inspection requirements.

B. DUTIES OF THE SPECIAL INSPECTOR
1. Abide by the special inspection and testing agreement provided by the local jurisdiction.
2. Observe the work for conformance with the approved construction documents. All discrepancies shall be brought to the attention of the contractor for correction. Then, if left uncorrected by the contractor, discrepancies shall be brought to the structural engineer, architect, and building department (if required).
3. Furnish inspection reports for each inspection to the building official, architect, engineer, contractor and other designated parties, in a timely manner, as established at the pre-construction meeting.
4. Submit a final report documenting required special inspections and correction of any discrepancies noted in the inspections. The final report shall be signed and sealed by a Professional Engineer responsible for the special inspections.

C. DUTIES OF THE CONTRACTOR
1. Notify the special inspector that the work is ready for inspection at least 24 hours before inspection.
2. The construction or work for which special inspection is required shall remain accessible and exposed for special inspection purposes until completion of the required special inspections.
3. Provide the special inspector with access to approved construction documents at the jobsite.
4. Keep records of all special inspection reports at the jobsite.

D. DEFINITIONS
1. Continuous special inspection – Special inspection by the special inspector who is continuously present when and where the work to be inspected is being performed.
2. Periodic special inspection – Special inspection by the special inspector who is intermittently present where the work to be inspected has been or is being performed.

E. WORK THAT REQUIRES SPECIAL INSPECTION

ITEM	REMARKS
Grading, Excavations, Fill	Refer to Geotechnical Report
Bearing Stratum Verification	Refer to Geotechnical Report
Concrete - Verify Proper Mix Design	Periodic
Concrete - Concrete Placement	Continuous
Concrete - Rebar Placement	Inspect Final Placement
Concrete - Anchor Bolts and Plates	Inspect Final Placement
Concrete - Expansion Anchors	Periodic
Concrete - Epoxy	Periodic
Steel - Verify Cold-Formed Deck Material	Periodic
Steel - Deck & Reinforcement Welding	Periodic
Open Web Steel Joists and Joist Girders – End Connections – Welded or Bolted	Periodic
Open Web Steel Joists and Joist Girders – Bridging	Periodic
Structural Steel - Fillet Welds (Visual Inspection of All Welded Joints)	Periodic
Structural Steel - Partial or Full Penetration Welds (Non Destructive Testing of All Welded Joints)	Periodic
Structural Steel - Bearing Bolts	Periodic
Structural Steel - Composite Shear Studs	Periodic
Structural Steel - Size and Location of Structural Elements	Periodic
Structural Steel - Slip Critical Connections	Periodic
Structural Steel - Inspection of Frame Joint Details for Compliance with Approved Construction Documents	Periodic

XXIII. TILT UP WALL PANELS

A. GENERAL
1. Concrete Strength: Refer to General Notes.
2. Panels are viewed from the interior.
3. Panels reinforced as follows:

Panel Thickness	Architectural Wythe + Insulation	Panel Reinforcing
7 1/4"	3" + 1"	#5 @ 12 inch o.c. vert. and #4 @ 12 inch o.c. horiz at mid-depth
10 1/4"	3" + 1"	#4 @ 10 inch o.c. vert. and #4 @ 18 inch o.c. horiz. at each face
10 1/4"	N.A.	#4 @ 10 inch o.c. vert. and #4 @ 18 inch o.c. horiz at each face

4. All panels greater than 10-inches thick must have vertical and horizontal reinforcing in each face. This requirement supersedes any scheduled reinforcement.
5. In addition, panels shall have the following additional reinforcing, unless noted otherwise:
   a. One # 5 continuous bar (at each mat of reinforcing steel) at all edges, with one 2'-0" x 2'-0" # 5 corner bar (at each mat of reinforcing steel) at all corners.
   b. Two # 5 continuous bars at all sides of openings. Extend these bars 2'-0" (minimum) past corner of openings and provide two # 5 x 4'-0" diagonal bars at all corners of each opening (extending 2 feet past each side of corner).
6. All reinforcement shown on panel elevations are in addition to main reinforcements noted in Item 3 above.
7. See Details for dowels, connections to steel frames, etc.
8. Contractor shall submit for Owner's record the method of lifting panels prior to pouring panels, and shall furnish and install lifting inserts and any additional reinforcement required by the lifting method.
9. See Architectural, Mechanical, Electrical, and Structural Drawings and Details for all bolts, plates, dowels, reglets, chamfers, reveals, finishes, conduit, boxes, etc., to be cast into panels. Contractor shall submit shop drawings coordinating the placement of all such materials, including reinforcing steel to be cast into panels.
10. All steel exposed to exterior shall be galvanized and all welds shall be coated with zinc rich paint.
11. Coordinate top of panel dimensions with Architectural details.
12. Verify door and window openings in panels with Architectural Drawings.
13. Coordinate location of embed plates shown with steel fabricator.
14. See Architectural Drawings for exterior finish of the panels.
15. Panels supporting aluminum canopies (designed by others) have been designed based on preliminary assumptions of canopy reactions. The canopy structure, and connection of canopy to the panel, shall be designed by the canopy supplier's engineer. GC to provide structural engineer with final canopy reactions prior to fabricating panel, to confirm final panel design is sufficient for canopy support.

B. RELEASING AGENT (CURING COMPOUND/BOND BREAKER)
1. The releasing agent used for separating panels from the casting bed shall be a solution of organic esters and silicones containing no oil, waxes, paraffin, or other material which could affect bond of subsequent finishes or the natural appearance of the exposed concrete surface.

C. ERECTION OF PANELS
1. Panels have been designed for final, in-service building conditions only.
2. The General Contractor shall submit tilt wall lifting and bracing shop drawings that are signed and sealed by a Professional Engineer licensed in the state where the project is located. The lifting and bracing engineer shall note that the edge reinforcement and opening reinforcement required in section A.4 of these notes are not required to be placed in each face of the panel and the lifting and bracing calculations shall use clear cover dimensions assuming these bars are located at the reinforcing steel mat.
3. Lifting: Additional reinforcement, strong-backs, or other temporary reinforcement methods may be required during the lifting of the panels. The general contractor is responsible for the panel lifting design and methodology.
4. Bracing: After placing panels, provide temporary braces and supports to securely hold panels in position according to the approved shop drawings. The general contractor is responsible for the panel bracing. Maintain temporary braces in place until final framing, connections, decking, and slab leave-out construction is completed.
5. Thermal Movements: During construction, the structure is susceptible to potentially large thermal movements when the steel frame and roof deck experiences rapid heating from direct exposure to the sun and rapid cooling over night. If the tilt-up panels are supported by erection braces during these thermal fluctuations they can restrain the panels, resulting in cracked panels at brace points or at joist, beam, or girder connections. The General Contractor shall be responsible for the erection methodology to limit the risks due to thermal movement.
6. Welding: Precautions shall be taken to insulate concrete from thermal stresses caused by welding.

XXIV. SPRINKLER PIPES

1. An allowance of 5 psf has been taken for the weight of the sprinkler system. Support locations shall be determined so that loads applied to joists, joist girders, and beams do not exceed the equivalent of 5 psf.
2. The method of supporting sprinkler pipes and the selection of hangers and their attachment to the structure shall be the responsibility of the sprinkler contractor.
3. Hangers and headers shall be located at top chord panel points where possible. Where this is not possible, additional web members shall be required as per the typical detail for concentrated loads not at panel points that exceed 100 pounds.
4. 8" diameter pipes running parallel to joists shall be supported at 8 feet on center maximum by headers supported at top chord panel points at two joists. 8" diameter pipes running perpendicular to joists shall be supported at top chord panel points of joists at 6 feet on center maximum.
5. 5" and 6" diameter pipes running parallel to joists shall be supported at 12 feet on center maximum by headers supported at top chord panel points at two joists. 5" and 6" diameter pipes running perpendicular to joist shall be supported at top chord panel points of joists at 13 feet on center maximum.
6. 4" diameter pipes and smaller shall be supported from top chord panel points as required by applicable codes.
7. Where alternative pipe layouts are not possible, A/E shall be notified to review conditions where loading exceeds the 5 psf allowance noted above.

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KEY PLAN:

SEAL:

STATE OF LOUISIANA
HEATH H. MICHEL
License No. 39291
PROFESSIONAL ENGINEER
IN
DIGITALLY SIGNED: 03/02/2026
PINNACLE STRUCTURAL ENGINEERS
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	12/15/25	50% DD
	01/14/26	100% DD
	02/27/26	ISSUE FOR BID

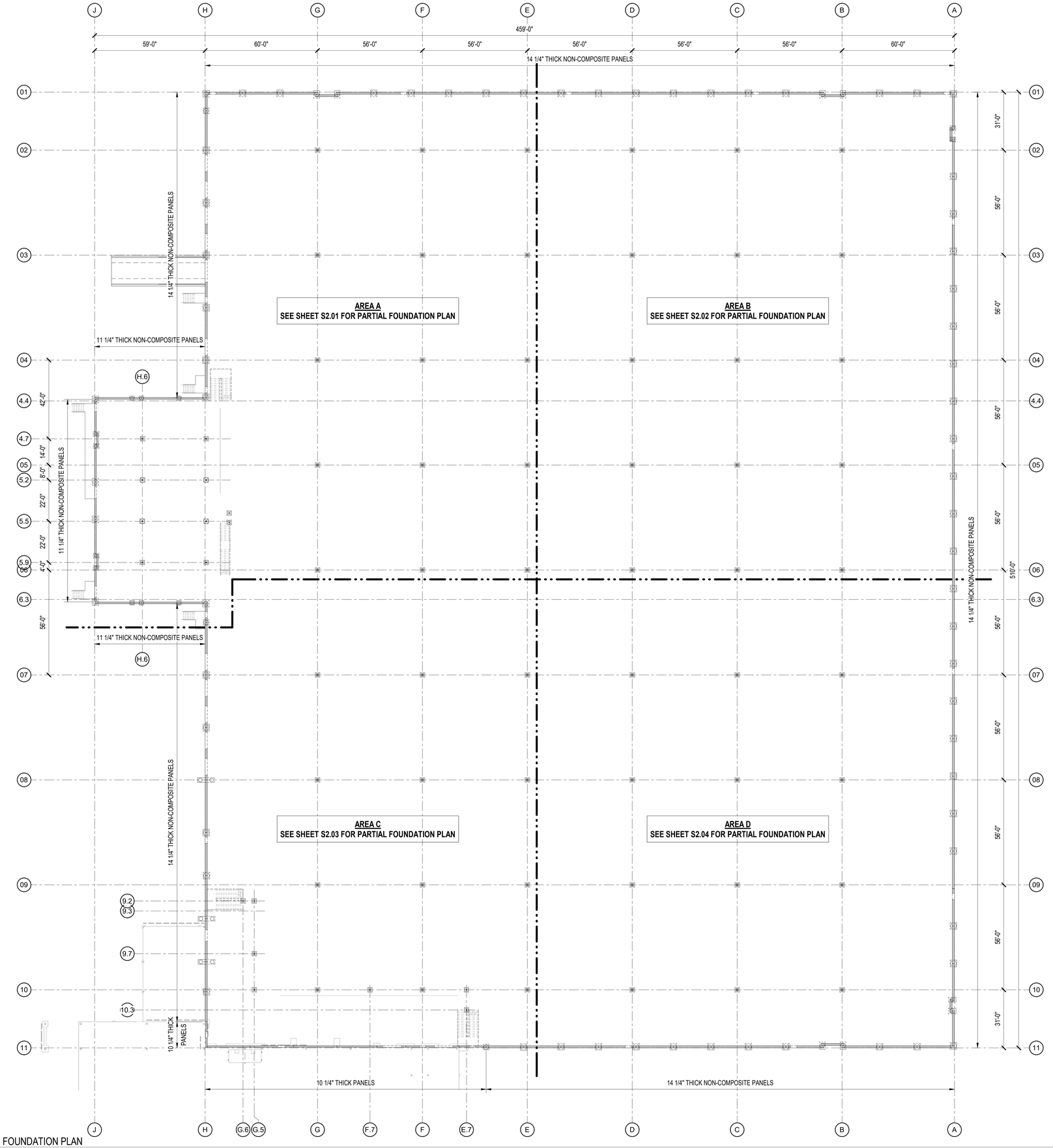
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

GENERAL NOTES AND WIND DIAGRAMS

S0.02



1 OVERALL FOUNDATION PLAN
3/64" = 1'-0"

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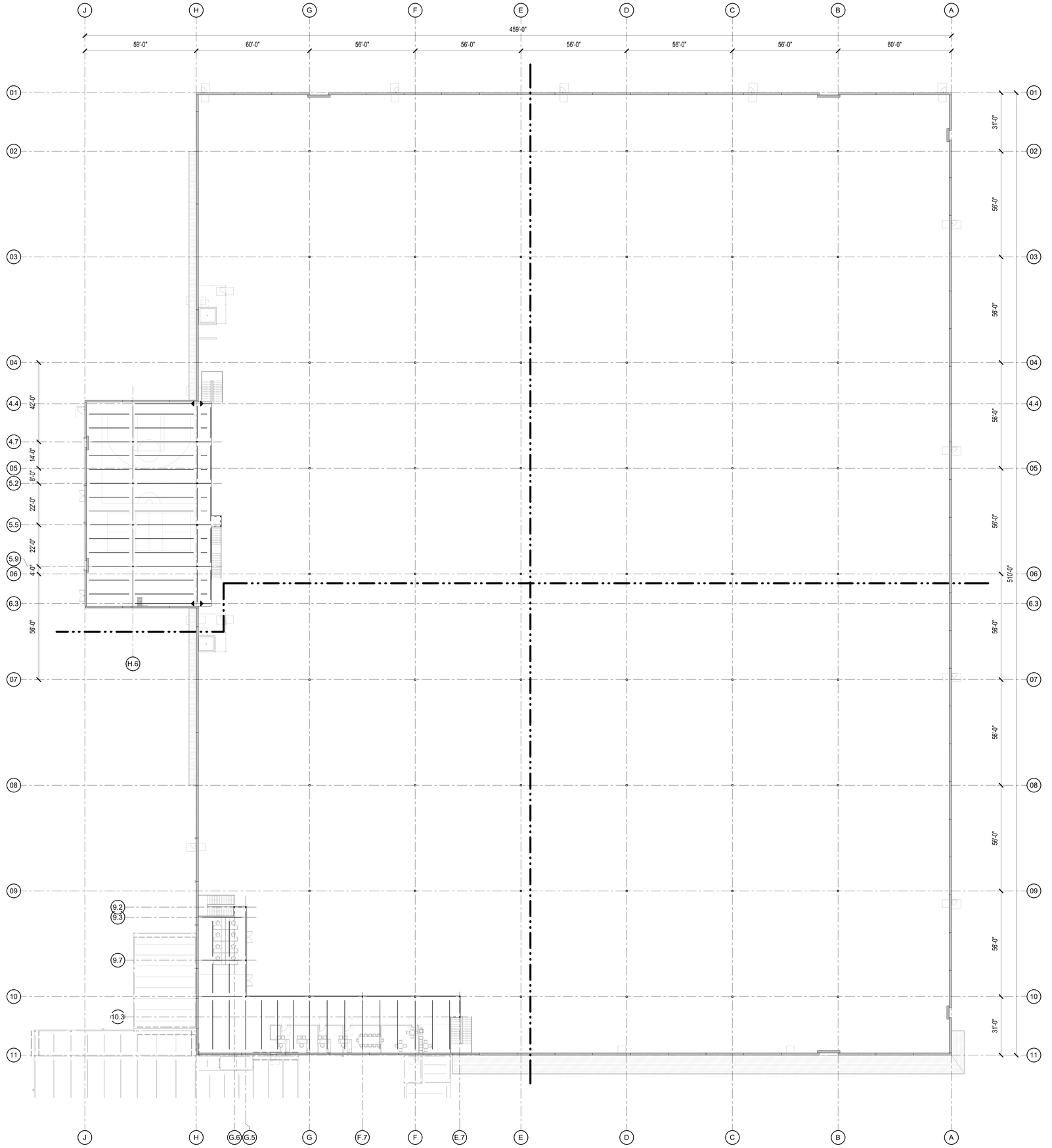
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PROJECT ADDRESS

RMA PROJECT NO: 251172

OVERALL FOUNDATION PLAN

S1.01



1 OVERALL MEZZANINE FLOOR FRAMING PLAN
3/64" = 1'-0"

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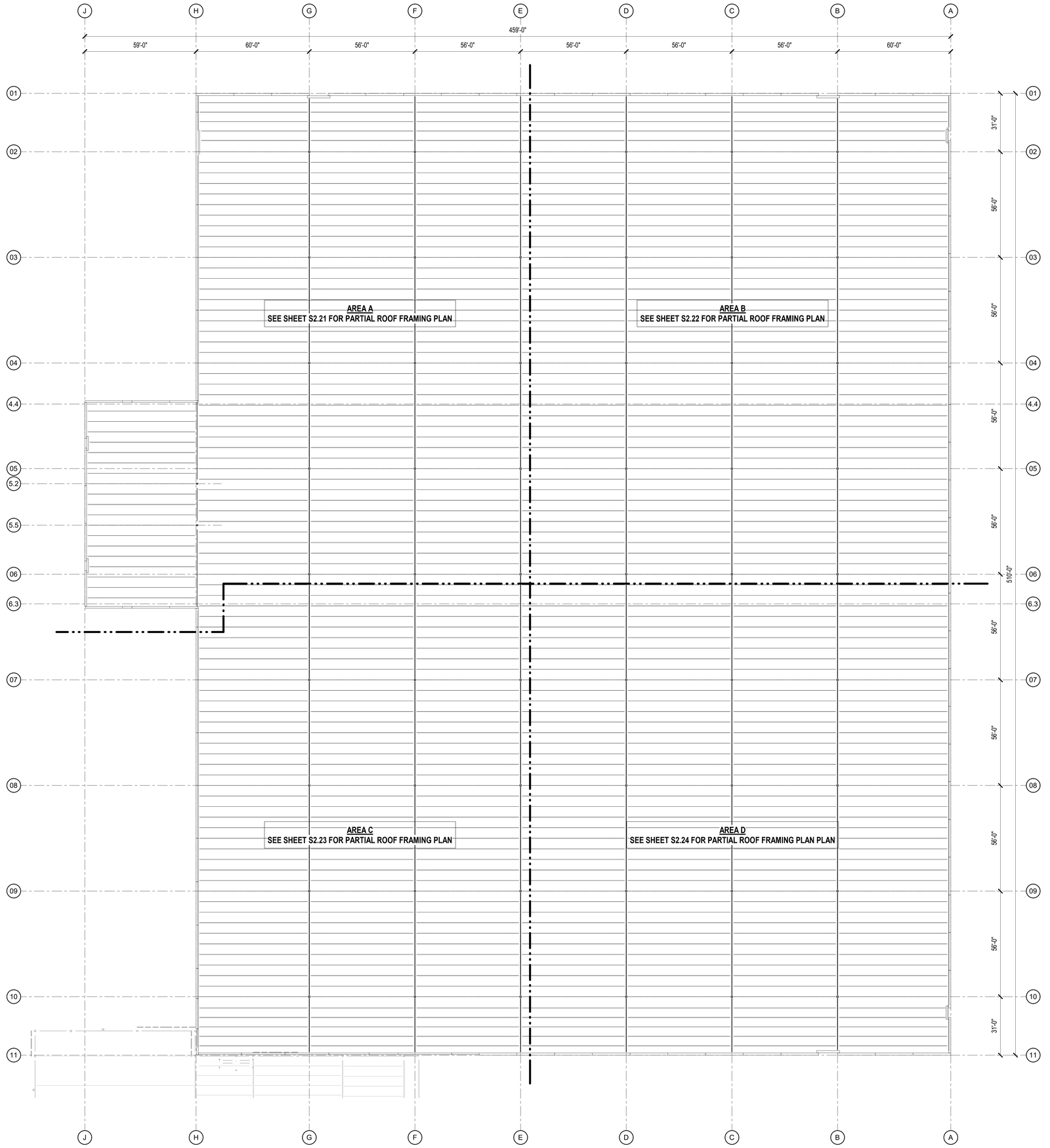
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RMA PROJECT NO: 251172

OVERALL MEZZANINE FLOOR FRAMING PLAN

S1.02



1 OVERALL ROOF FRAMING PLAN
3/64" = 1'-0"

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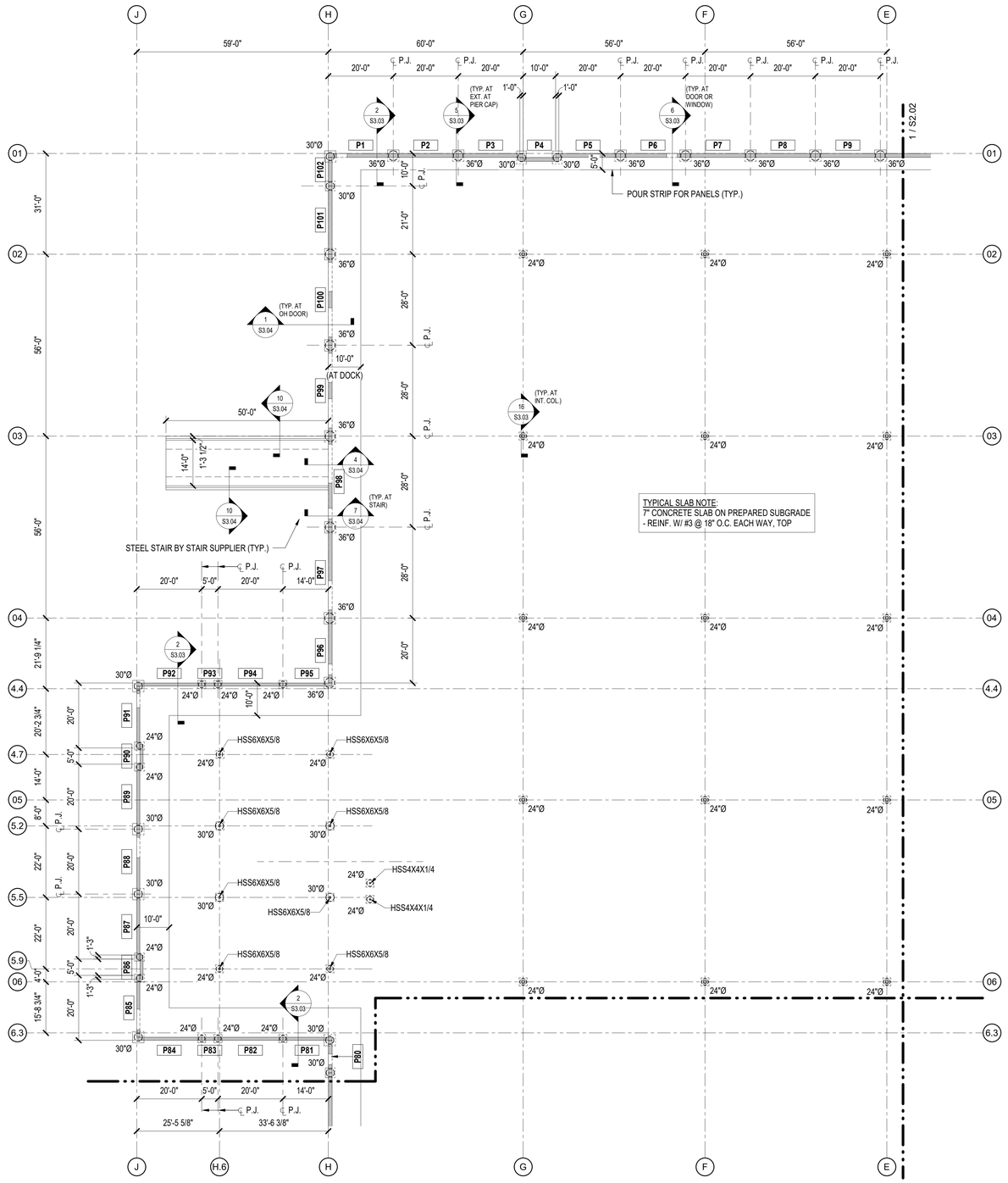
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PROJECT ADDRESS

RMA PROJECT NO: 251172

OVERALL ROOF FRAMING PLAN

S1.03



1 PARTIAL FOUNDATION PLAN
1/16" = 1'-0"

GENERAL NOTES

1. ALL STRAIGHT SHAFTS UNDER TILT-UP PANELS SHALL BE CENTERED UNDER PANELS AND CENTERED ON PANEL JOINTS, U.N.O.
2. STRAIGHT SHAFTS NOTED THUS: 24"Ø INDICATE THE A 24" DIAMETER STRAIGHT SHAFT.
3. STRAIGHT SHAFTS UNDER GRADE BEAMS (WITHOUT COLUMNS) SHALL BE CENTERED UNDER GRADE BEAMS, U.N.O.
4. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
5. ALL COLUMNS ARE HSS12X12X1/4 TYP., U.N.O.
6. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.
7. SEE DETAIL 8/S3.02 FOR HSS BASE PLATES.

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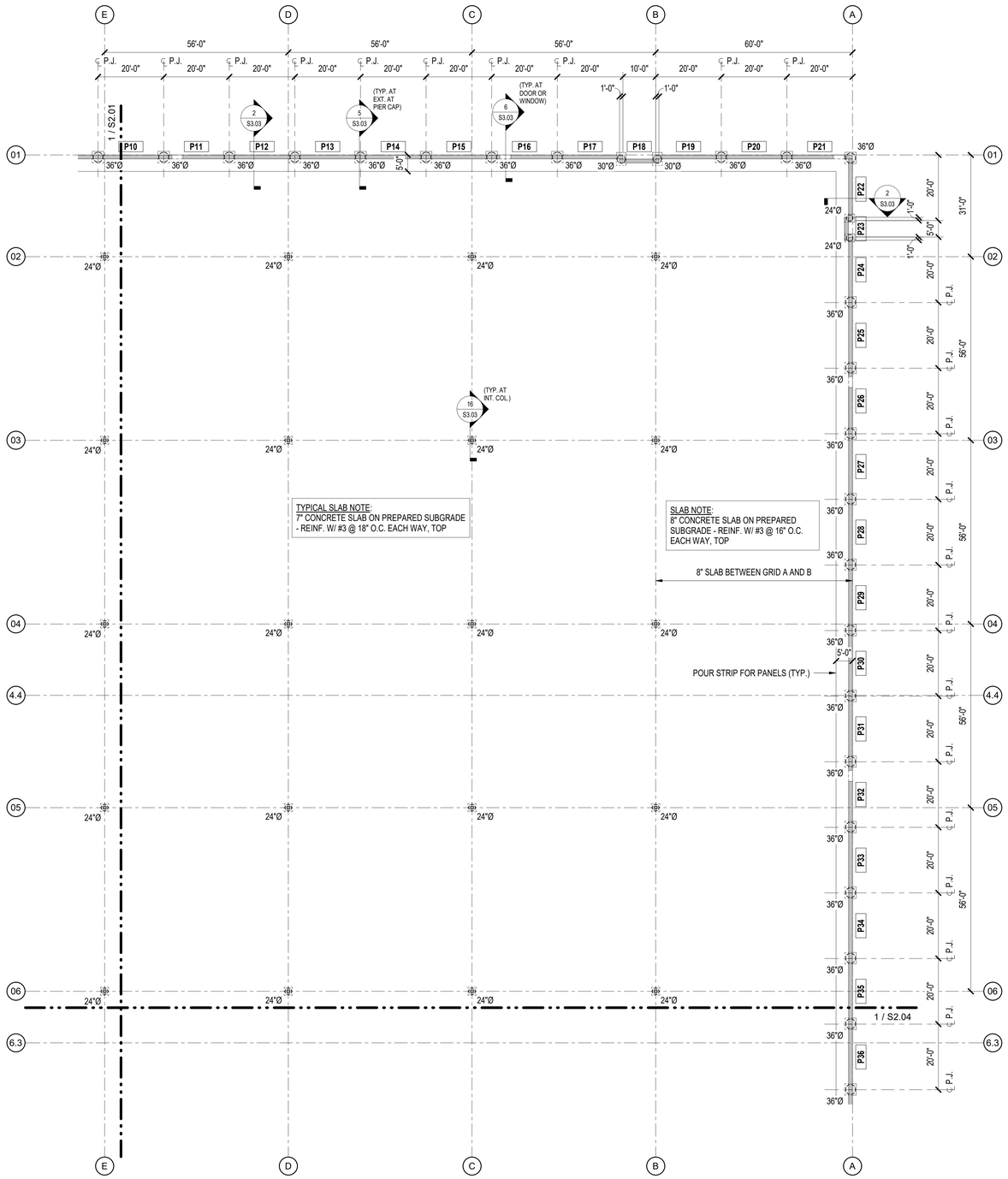
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PARTIAL FOUNDATION PLAN - AREA A

S2.01



1 PARTIAL FOUNDATION PLAN
1/16" = 1'-0"

GENERAL NOTES

1. **ALL STRAIGHT SHAFTS UNDER TILT-UP PANELS SHALL BE CENTERED UNDER PANELS AND CENTERED ON PANEL JOINTS, U.N.O.**
2. **STRAIGHT SHAFTS NOTED THUS: 24"Ø INDICATE THE A 24" DIAMETER STRAIGHT SHAFT.**
3. **STRAIGHT SHAFTS UNDER GRADE BEAMS (WITHOUT COLUMNS) SHALL BE CENTERED UNDER GRADE BEAMS, U.N.O.**
4. **LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.**
5. **ALL COLUMNS ARE HSS12X12X1/4 TYP., U.N.O.**
6. **SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.**
7. **SEE DETAIL 8/S3.02 FOR HSS BASE PLATES.**

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HEATH H. MICHEL
License No. 39291
PROFESSIONAL ENGINEER
IN
ENGINEERING
DIGITALLY SIGNED: 03/02/2026

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LOUISIANA FIRM LICENSE #4141

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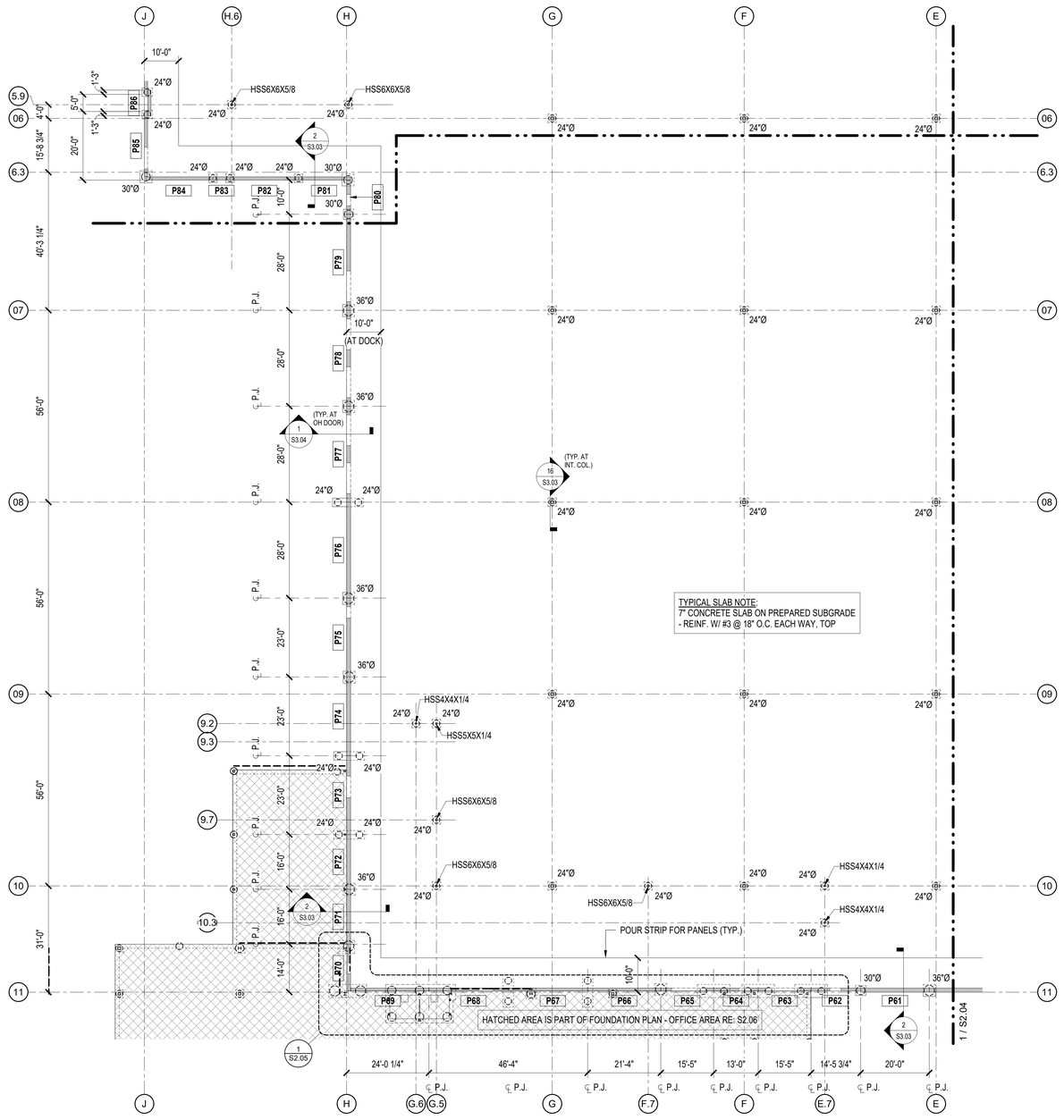
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PROJECT ADDRESS

RMA PROJECT NO: 251172

PARTIAL FOUNDATION PLAN - AREA B

S2.02



1 PARTIAL FOUNDATION PLAN
1/16" = 1'-0"

GENERAL NOTES

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2. **STRAIGHT SHAFTS NOTED THUS: 24"Ø INDICATE THE A 24" DIAMETER STRAIGHT SHAFT.**
3. **STRAIGHT SHAFTS UNDER GRADE BEAMS (WITHOUT COLUMNS) SHALL BE CENTERED UNDER GRADE BEAMS, U.N.O.**
4. **LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.**
5. **ALL COLUMNS ARE HSS12X12X1/4 TYP., U.N.O.**
6. **SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.**
7. **SEE DETAIL 8/S3.02 FOR HSS BASE PLATES.**

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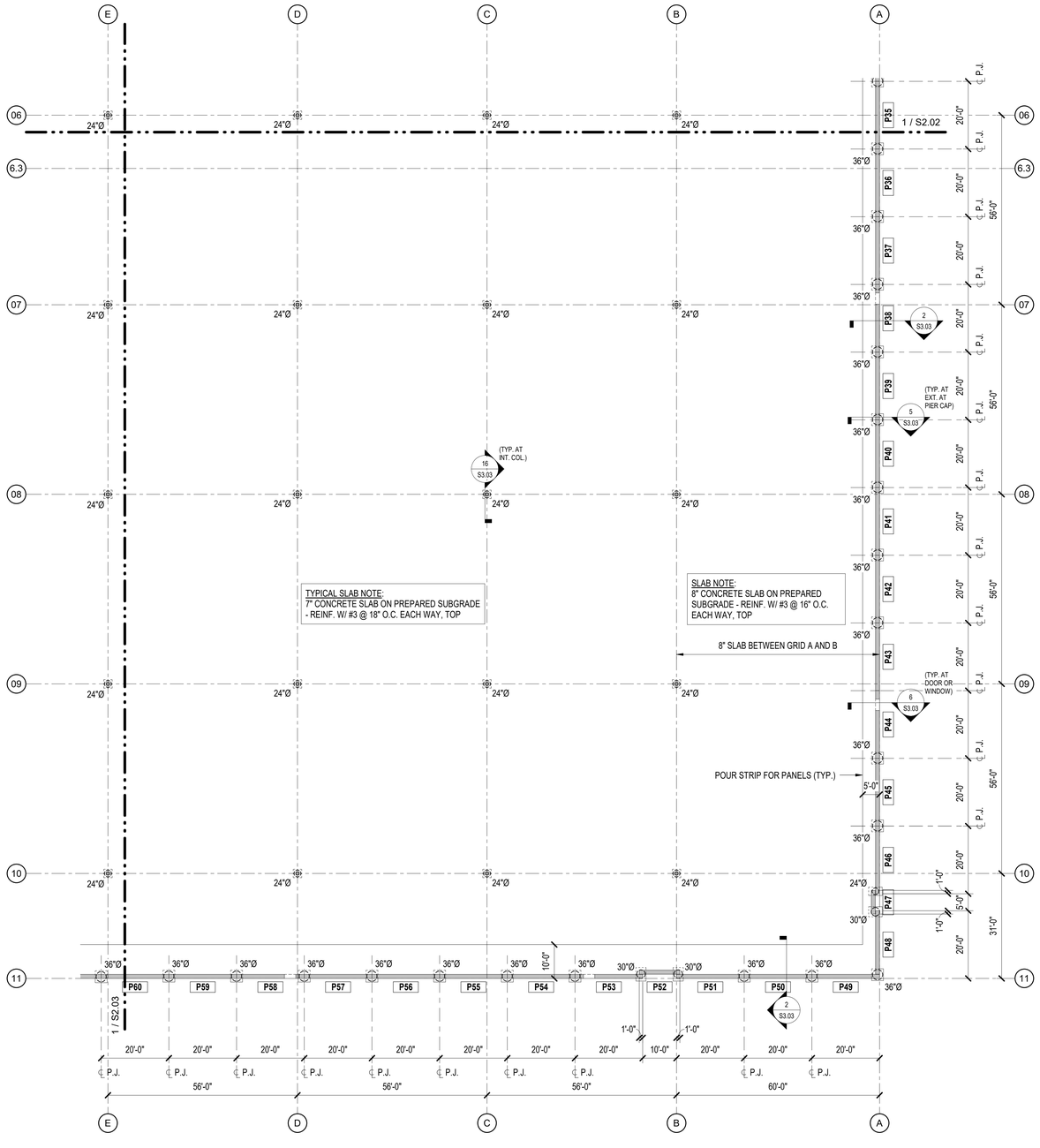
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RMA PROJECT NO: 251172

PARTIAL FOUNDATION PLAN - AREA C

S2.03



1 PARTIAL FOUNDATION PLAN
1/16" = 1'-0"

GENERAL NOTES

1. **ALL STRAIGHT SHAFTS UNDER TILT-UP PANELS SHALL BE CENTERED UNDER PANELS AND CENTERED ON PANEL JOINTS, U.N.O.**
2. **STRAIGHT SHAFTS NOTED THUS: 24"Ø INDICATE THE A 24" DIAMETER STRAIGHT SHAFT.**
3. **STRAIGHT SHAFTS UNDER GRADE BEAMS (WITHOUT COLUMNS) SHALL BE CENTERED UNDER GRADE BEAMS, U.N.O.**
4. **LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.**
5. **ALL COLUMNS ARE HSS12X12X1/4 TYP., U.N.O.**
6. **SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.**
7. **SEE DETAIL 8/S3.02 FOR HSS BASE PLATES.**

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RMA PROJECT NO: 251172

PARTIAL FOUNDATION PLAN - AREA D

S2.04

NOT FOR CONSTRUCTION

1. ALL STRAIGHT SHAFT UNDER TILT-UP PANELS SHALL BE CENTERED UNDER PANELS AND CENTERED ON PANEL JOINTS, U.N.O.
2. STRAIGHT SHAFT NOTED THUS: 24"Ø INDICATE THE PIER SHAFT AS 24" IN DIAMETER.
3. STRAIGHT SHAFTS UNDER GRADE BEAMS (WITHOUT COLUMNS) SHALL BE CENTERED UNDER GRADE BEAMS, U.N.O.
4. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
5. ALL COLUMNS ARE HSS6X6X1/4 TYP., U.N.O.
6. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF ALL FLOOR SLOPES, DEPRESSIONS, ETC.
7. SEE DETAIL 8/S302 FOR HSS BASE PLATES.

AREA C
SEE SHEET S2.03 FOR PARTIAL FOUNDATION PLAN

TYPICAL SLAB NOTE:
5" CONCRETE SLAB ON PREPARED SUBGRDE
- REINF. W/ #3 @ 18" O.C. EACH WAY, TOP

1 FOUNDATION PLAN - OFFICE
1/8" = 1'-0"

NOT FOR CONSTRUCTION

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LIPSEY'S NEW HEADQUARTERS

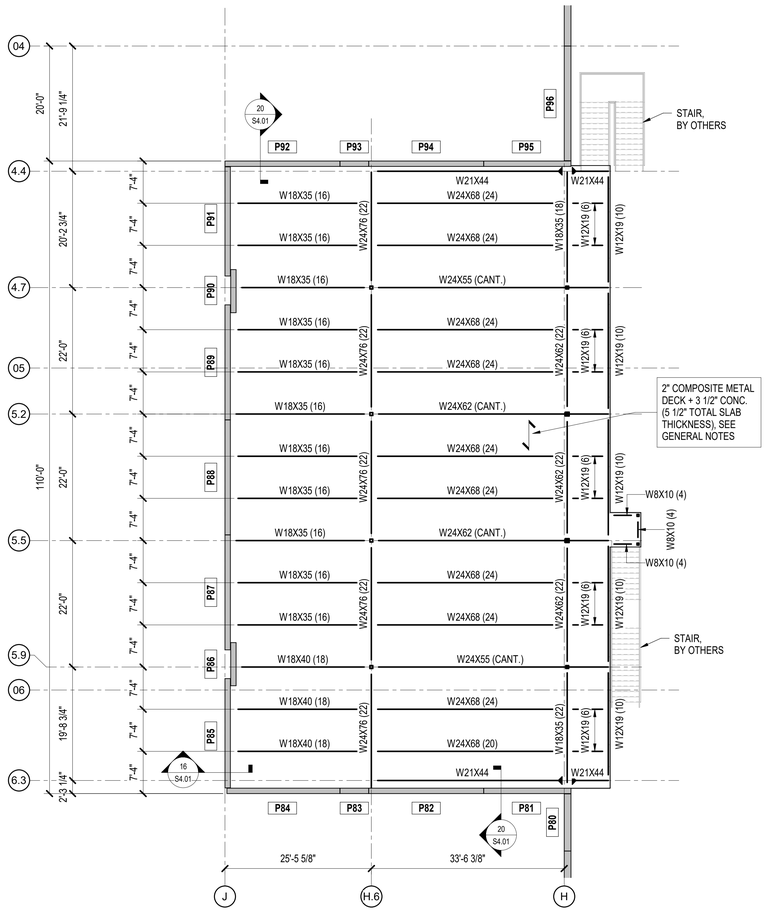
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RMA PROJECT NO: 24.154

FOUNDATION PLAN - OFFICE

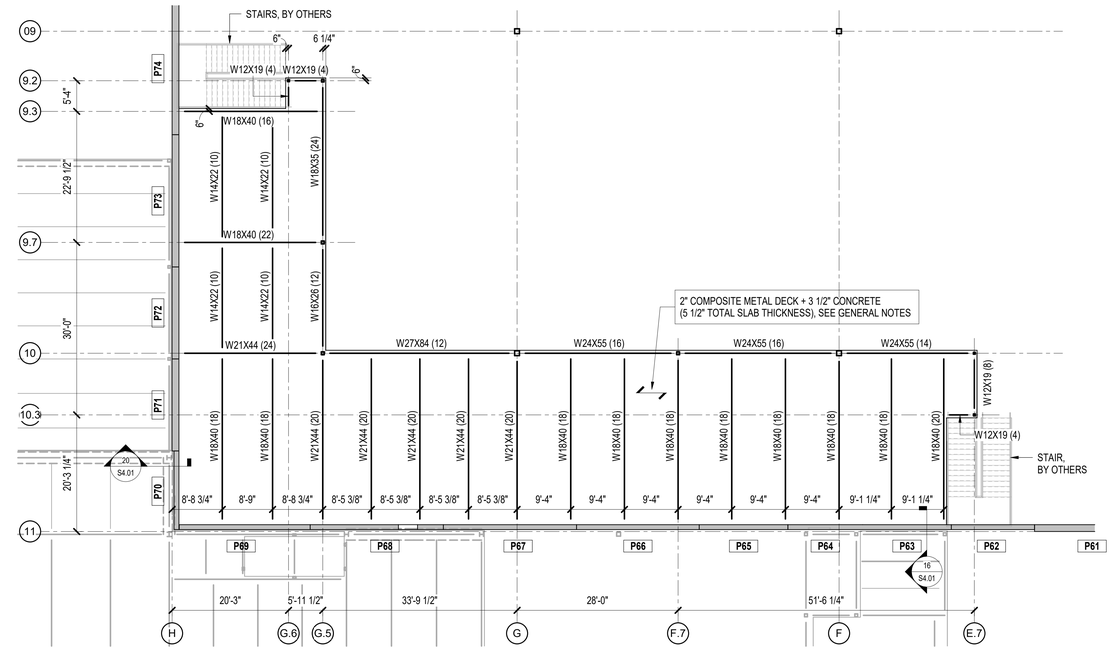
S2.05

NOT FOR CONSTRUCTION



1. FINISHED FLOOR ELEVATION = 14'-6" ABOVE FINISHED FIRST FLOOR.
2. LOCATIONS SHOWN THUS: W16X31 (12) C = 3/4" INDICATES A W16X31 WITH THE NUMBER OF SHEAR CONNECTORS AS 12 AND UPWARD CAMBER AT MID SPAN OF BEAM AS 3/4".
3. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
4. SEE SHEET S2.01 FOR COLUMN SIZES.

2 FRAMING PLAN - WEST MEZZANINE
3/32" = 1'-0"



1 SOUTH MEZZANINE
3/32" = 1'-0"

1. FINISHED FLOOR ELEVATION = 18'-0" ABOVE FINISHED FIRST FLOOR.
2. LOCATIONS SHOWN THUS: W16X31 (12) C = 3/4" INDICATES A W16X31 WITH THE NUMBER OF SHEAR CONNECTORS AS 12 AND UPWARD CAMBER AT MID SPAN OF BEAM AS 3/4".
3. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
4. SEE SHEET S2.03 FOR COLUMN SIZES.

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RMA PROJECT NO: 251172

MEZZNINE FOOR FRAMING PLANS

S2.10

GENERAL NOTES

1. FINISHED FLOOR ELEVATIONS = 18'-0" ABOVE FINISH FIRST FLOOR.
2. LOCATIONS SHOWN THUS: W16X31 (12) C = 3/4" INDICATES A W16X31 WITH THE NUMBER OF SHEAR CONNECTORS AS 12 AND UPWARD CAMBER AT MID-SPAN OF BEAM AS 3/4".
3. SPACE WF BEAMS EQUALLY, U.N.O.
4. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
5. SEE SHEET S2.01 FOR ALL COLUMNS SIZES NOT SHOWN.
6. SEE SHEET S5.01 FOR BRACE ELEVATIONS.
7. LOCATIONS NOTED THUS: INDICATES A MOMENT CONNECTION.

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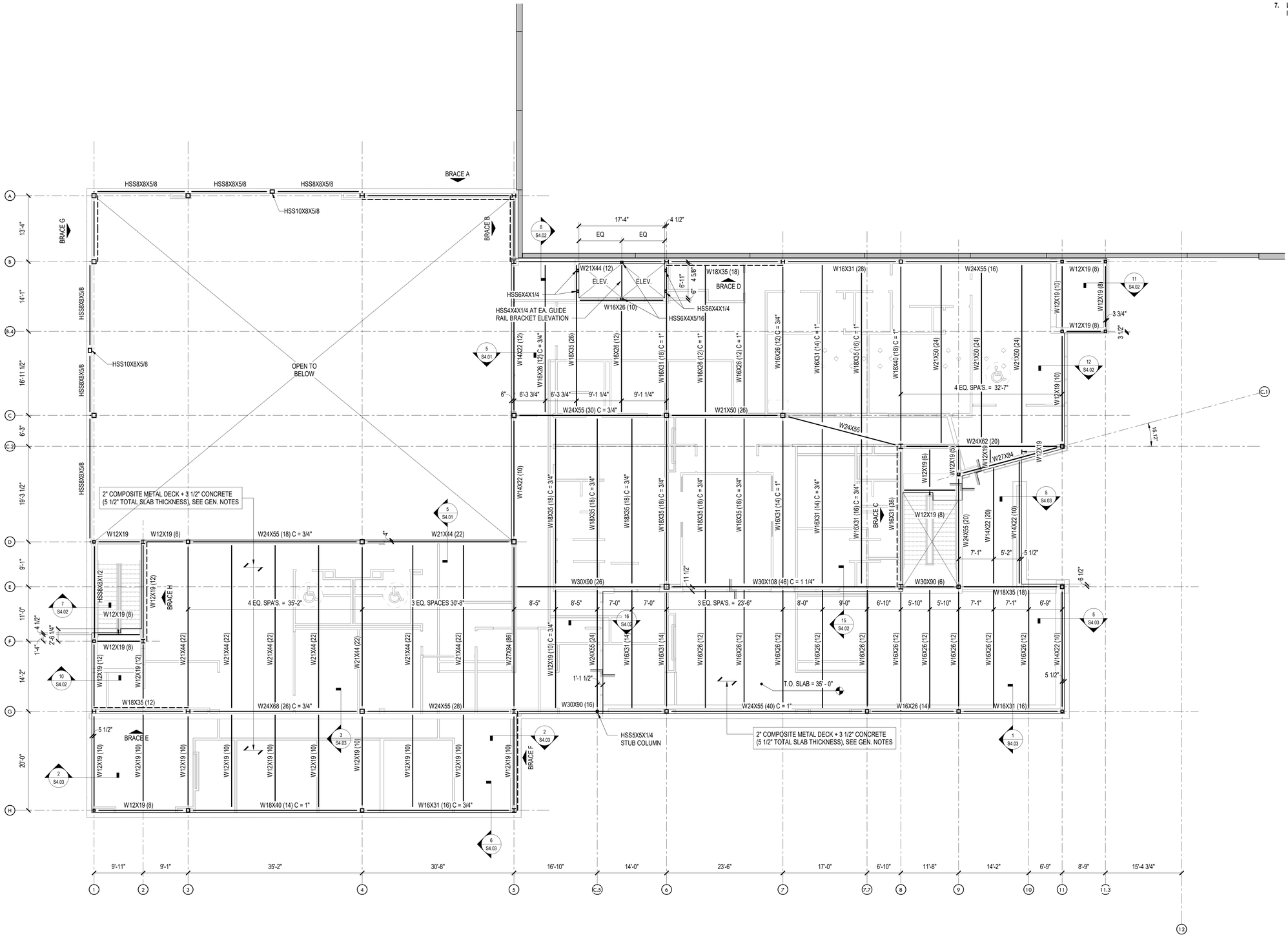
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RMA PROJECT NO: 24.154

SECOND FLOOR FRAMING PLAN - OFFICE

S2.11

1 SECOND FLOOR FRAMING PLAN - OFFICE
1/8" = 1'-0"



1 THIRD FLOOR FRAMING PLAN
1/8" = 1'-0"

GENERAL NOTES

1. FINISHED FLOOR ELEVATIONS = 36'-0" ABOVE FINISH FIRST FLOOR.
2. LOCATIONS SHOWN THUS: W16X31 (12) C = 3/4" INDICATES A W16X31 WITH THE NUMBER OF SHEAR CONNECTORS AS 12 AND UPWARD CAMBER AT MID-SPAN OF BEAM AS 3/4".
3. SPACE WF BEAMS EQUALLY, U.N.O.
4. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS FOR PANEL ELEVATIONS & DETAILS.
5. SEE SHEET S2.01 FOR ALL COLUMNS SIZES NOT SHOWN.
6. SEE SHEET S5.01 FOR BRACE ELEVATIONS.
7. LOCATIONS NOTED THUS: INDICATES A MOMENT CONNECTION.

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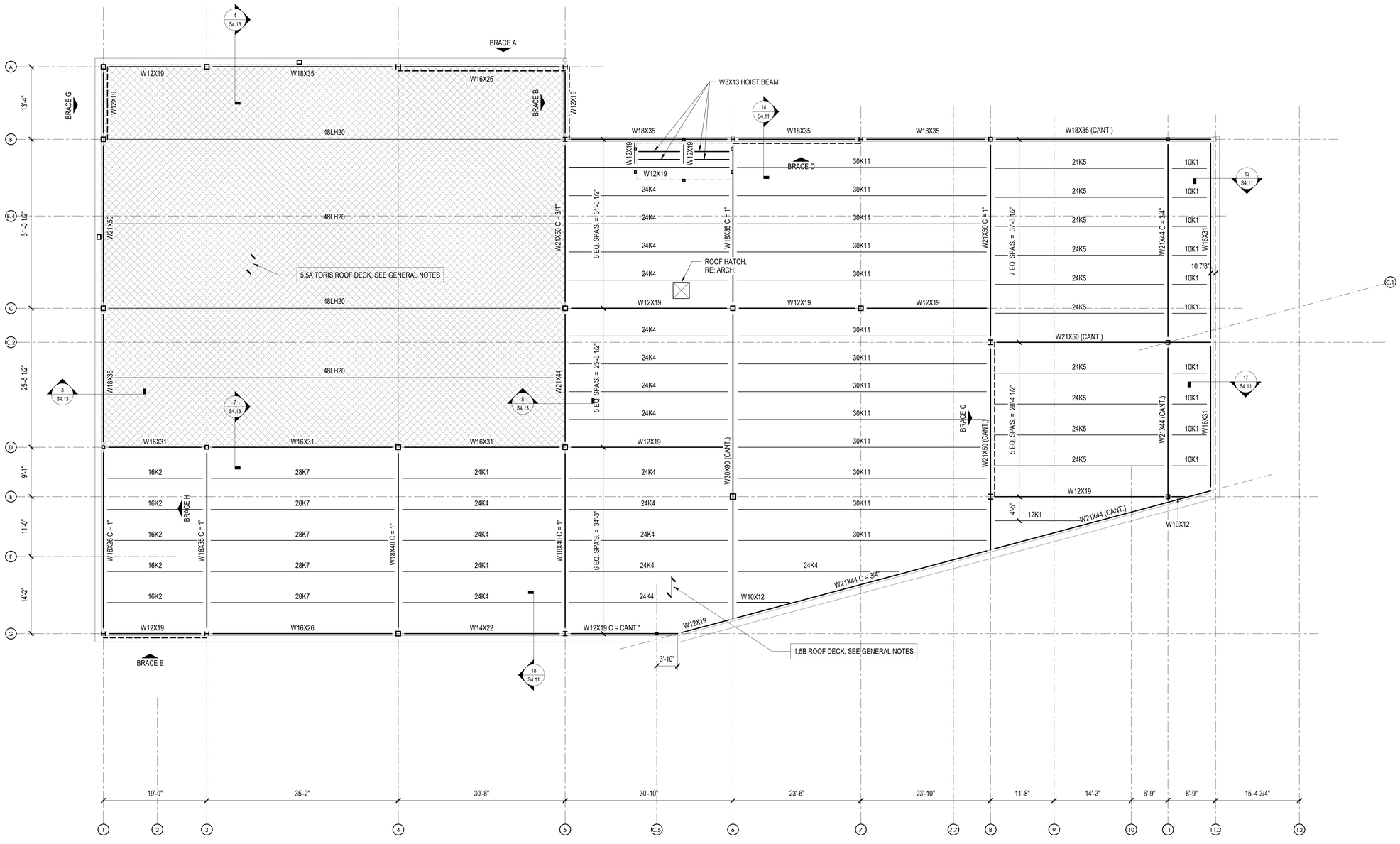
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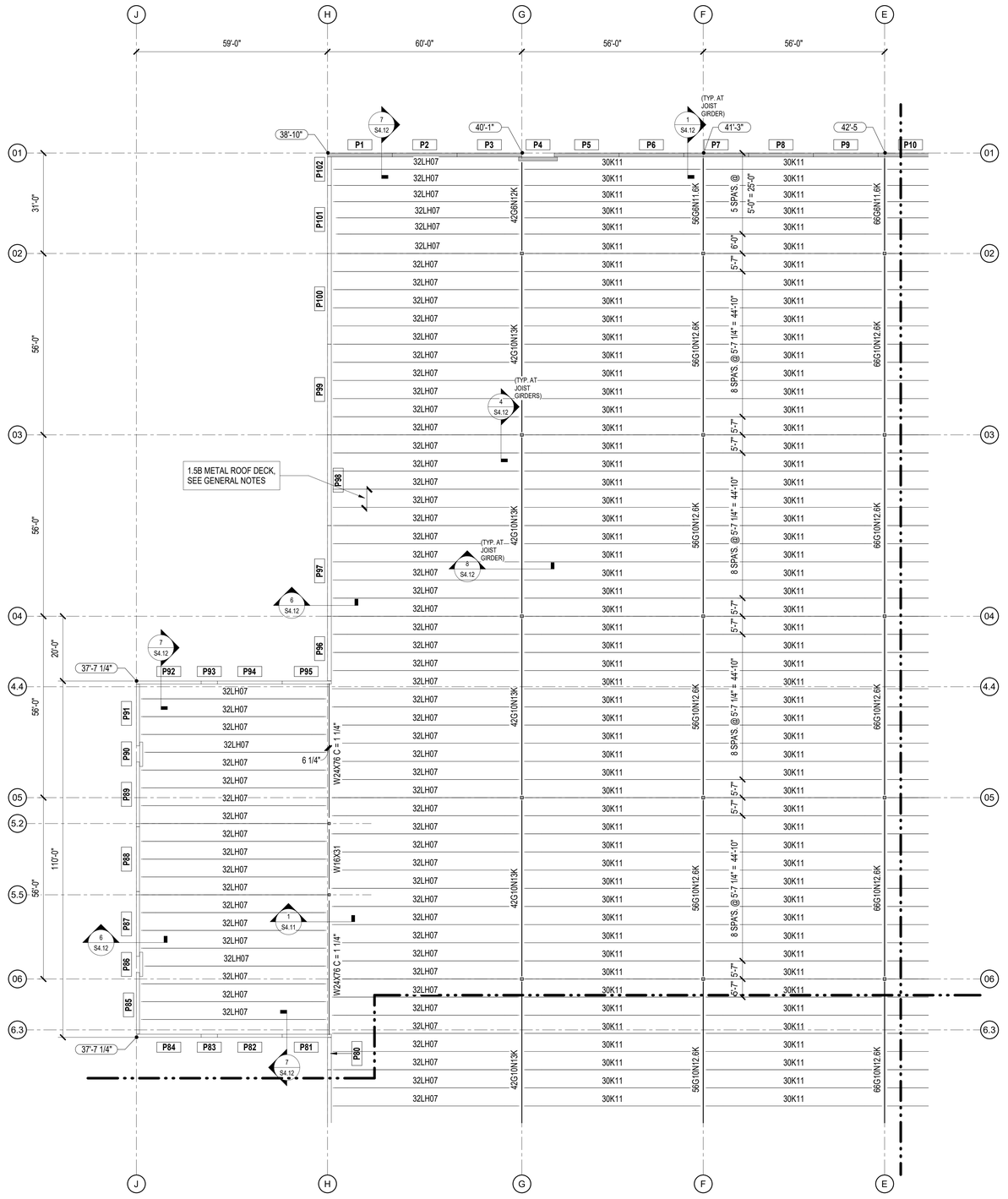
THIRD FLOOR FRAMING PLAN - OFFICE

S2.12

GENERAL NOTES

1. LOCATIONS SHOWN THUS: (XX'-XX") INDICATES TOP OF STEEL (T.O.S.) ELEVATION (TOP OF STEEL = TOP OF JOIST OR BOTTOM OF DECK).
2. JOISTS SHALL BE DESIGNED FOR ADDITIONALLOADS RESULTING FROM RTU'S AND OTHER MECHANICAL EQUIPMENT ON THE ROOF OR SUSPENDED FROM THE ROOF STRUCTURE. SEE ARCH'TL. AND MEP DRAWINGS FOR LOCATIONS.
3. SPACE JOISTS EQUALLY (6'-0" O.C. MAX.), U.N.O.
4. REFER TO ARCHITECTURAL DRAWINGS / MEP FOR LOCATION OF ROOF DRAINS, OVERFLOW DRAINS & TAPERED INSULATION.
5. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
6. SEE SHEET S2.01 FOR COLUMN SIZES.
7. JOISTS SHALL BE DESIGNED FOR ALL POINT AND LINE LOADS SHOWN ON THE ROOF PLANS WITH AN "L = " DESIGNATION AS PART OF THE ROOF LIVE LOAD CASE.


BRACE A
BRACE B
BRACE C
BRACE D
BRACE E
BRACE G
BRACE H
W8X13 HOIST BEAM
W12X19
W18X35
W16X26
W12X18
W12X19
48LH20
W18X35
W18X35 (CANT.)
W12X19
W12X19
W12X19



1 ENLARGED ROOF FRAMING PLAN
1/16" = 1'-0"

GENERAL NOTES

1. LOCATIONS SHOWN THUS: (XX-XX) INDICATES TOP OF STEEL (T.O.S.) ELEVATION (TOP OF STEEL = TOP OF JOIST OR BOTTOM OF DECK).
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3. SPACE JOISTS EQUALLY (6'-0" O.C. MAX.)
4. REFER TO ARCHT'L. DRAWINGS / MEP FOR LOCATION OF ROOF DRAINS, OVERFLOW DRAINS & TAPERED INSULATION.
5. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
6. SEE SHEET S2.01-S2.04 FOR COLUMN SIZES.
7. JOISTS SHALL BE DESIGNED FOR ALL POINT AND LINE LOADS SHOWN ON THE ROOF PLANS WITH AN "L = " DESIGNATION AS PART OF THE ROOF LIVE LOAD CASE.

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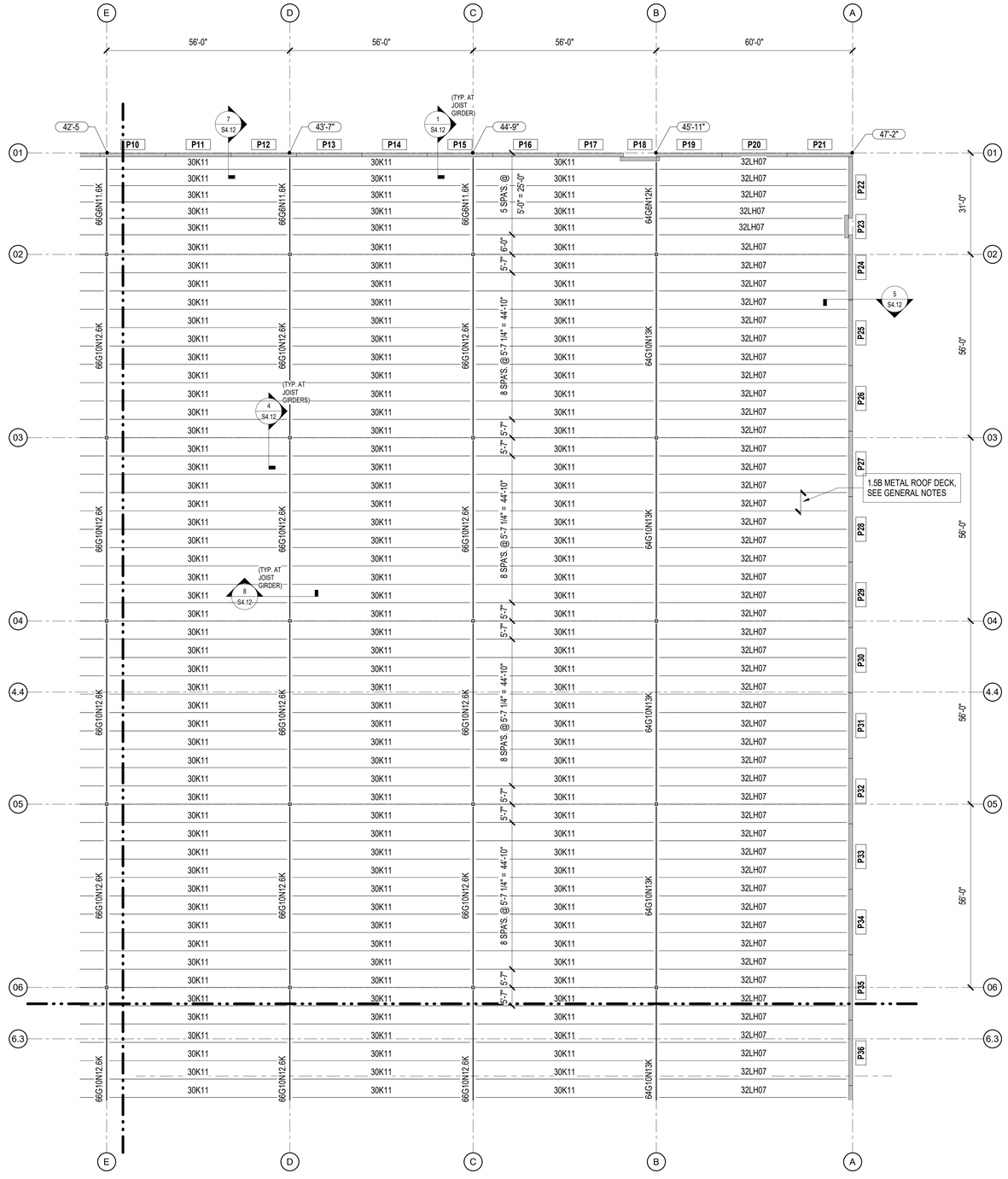
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RMA PROJECT NO: 251172

PARTIAL ROOF FRAMING PLAN - AREA A

S2.21



1 ENLARGED ROOF FRAMING PLAN
1/16" = 1'-0"

GENERAL NOTES

1. LOCATIONS SHOWN THUS: (XX-XX") INDICATES TOP OF STEEL (T.O.S.) ELEVATION (TOP OF STEEL = TOP OF JOIST OR BOTTOM OF DECK).
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3. SPACE JOISTS EQUALLY (6'-0" O.C. MAX.)
4. REFER TO ARCHT'L. DRAWINGS / MEP FOR LOCATION OF ROOF DRAINS, OVERFLOW DRAINS & TAPERED INSULATION.
5. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
6. SEE SHEET S2.01-S2.04 FOR COLUMN SIZES.
7. JOISTS SHALL BE DESIGNED FOR ALL POINT AND LINE LOADS SHOWN ON THE ROOF PLANS WITH AN "L = " DESIGNATION AS PART OF THE ROOF LIVE LOAD CASE.

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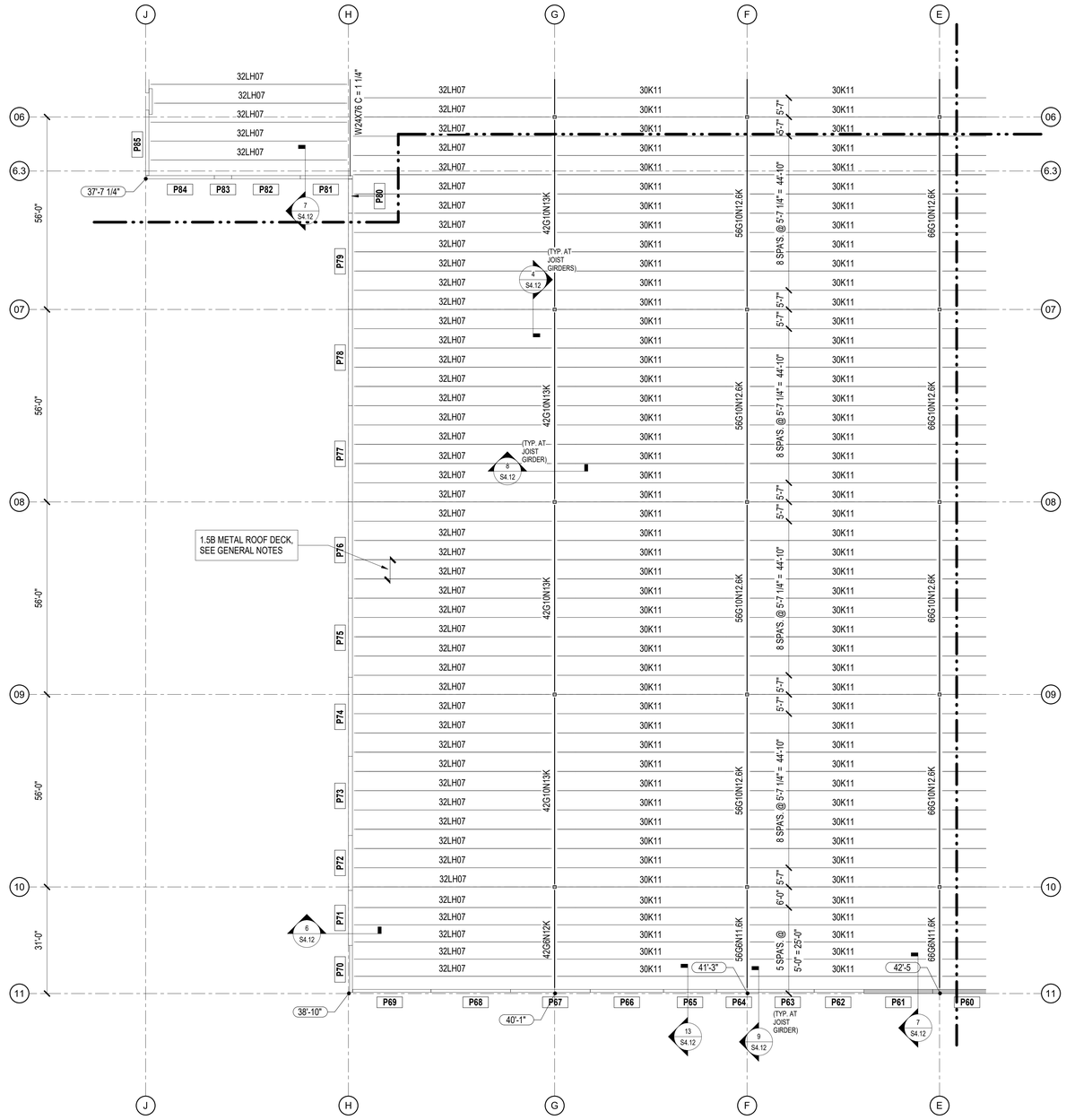
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RMA PROJECT NO: 251172

PARTIAL ROOF FRAMING PLAN - AREA B

S2.22



1 ENLARGED ROOF FRAMING PLAN
1/16" = 1'-0"

GENERAL NOTES

1. LOCATIONS SHOWN THUS: (XX-XX") INDICATES TOP OF STEEL (T.O.S.) ELEVATION (TOP OF STEEL = TOP OF JOIST OR BOTTOM OF DECK).
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6. SEE SHEET S2.01-S2.04 FOR COLUMN SIZES.
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KEY PLAN:

SEAL:
STATE OF LOUISIANA
HEATH H. MICHEL
License No. 39291
PROFESSIONAL ENGINEER
IN
DIGITALLY SIGNED: 03/02/2026
PINNACLE STRUCTURAL ENGINEERS
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	12/15/25	50% DD
	01/13/26	100% DD
	02/27/26	ISSUE FOR BID

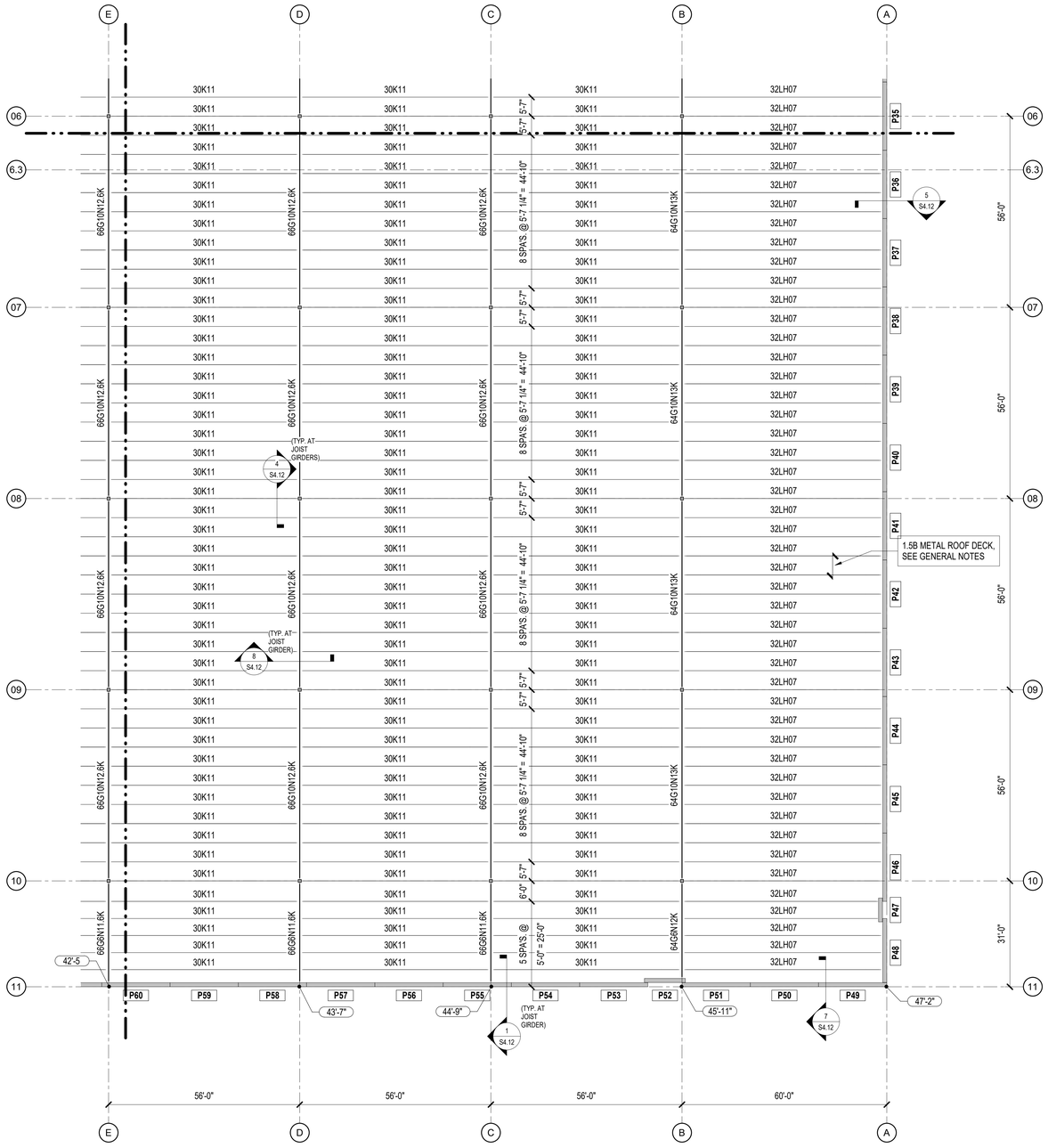
HIGHLAND RD

PROJECT ADDRESS

RMA PROJECT NO: 251172

PARTIAL ROOF FRAMING PLAN - AREA C

S2.23



1 ENLARGED ROOF FRAMING PLAN
1/16" = 1'-0"

GENERAL NOTES

1. LOCATIONS SHOWN THUS: (XX-XX) INDICATES TOP OF STEEL (T.O.S.) ELEVATION (TOP OF STEEL = TOP OF JOIST OR BOTTOM OF DECK).
2. JOISTS SHALL BE DESIGNED FOR ADDITIONALLOADS RESULTING FROM RTU'S AND OTHER MECHANICAL EQUIPMENT ON THE ROOF OR SUSPENDED FROM THE ROOF STRUCTURE. SEE ARCHT'L. AND MEP DRAWINGS FOR LOCATIONS.
3. SPACE JOISTS EQUALLY (6'-0" O.C. MAX.)
4. REFER TO ARCHT'L. DRAWINGS / MEP FOR LOCATION OF ROOF DRAINS, OVERFLOW DRAINS & TAPERED INSULATION.
5. LOCATIONS NOTED THUS: PX INDICATES PANEL NUMBER, SEE S5 SERIES SHEETS PANEL ELEVATIONS AND DETAILS.
6. SEE SHEET S2.01-S2.04 FOR COLUMN SIZES.
7. JOISTS SHALL BE DESIGNED FOR ALL POINT AND LINE LOADS SHOWN ON THE ROOF PLANS WITH AN "L = " DESIGNATION AS PART OF THE ROOF LIVE LOAD CASE.

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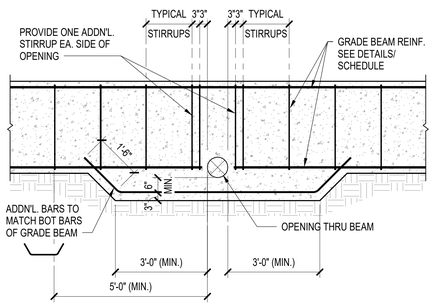
HIGHLAND RD

PROJECT ADDRESS

RMA PROJECT NO: 251172

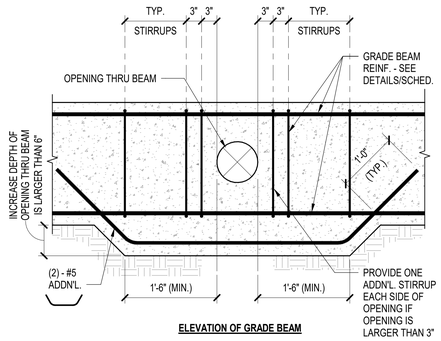
PARTIAL ROOF FRAMING PLAN - AREA D

S2.24



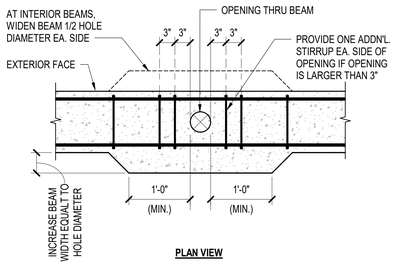
ELEVATION OF GRADE BEAM

20 TYPICAL DETAIL
OPENING THRU GRADE BEAM THAT CONFLICTS WITH BOTTOM REINF.
1/2" = 1'-0"



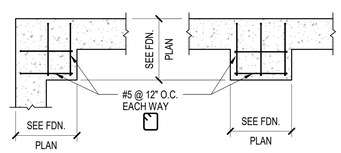
ELEVATION OF GRADE BEAM

19 TYPICAL DETAIL
OPENING THRU GRADE BEAM
1" = 1'-0"



PLAN VIEW

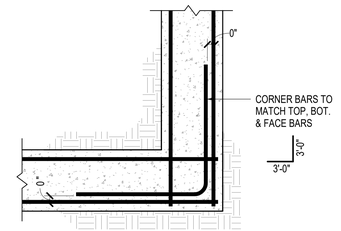
18 TYPICAL DETAIL
VERTICAL HOLE THRU GRADE BEAM LARGER THAN 3" DIAMETER
1" = 1'-0"



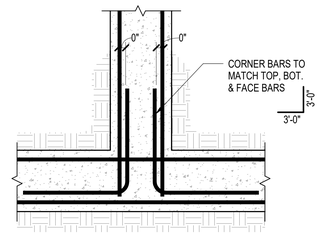
PLAN VIEW

NOTES:
1. PLAN VIEWS FOR TYPICAL WIDENED GRADE BEAMS ARE SHOWN. FOR SIMILAR CONDITIONS NOT SHOWN, PROVIDE ADDN'L. #5 TIES @ 12" O.C. (MAX.) IN WIDENED SECTION.
2. SEE DETAIL SHEETS FOR GRADE BEAM WIDTHS.

17 TYPICAL DETAIL - ADDITIONAL BARS AT WIDENED GRADE BEAM
1/2" = 1'-0"



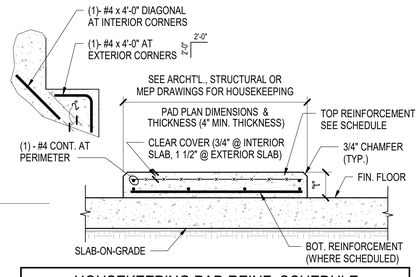
PLAN VIEW AT CORNER



PLAN VIEW AT "T" INTERSECTION

NOTE:
GRADE BEAMS SHALL BE POURED MONOLITHICALLY AT INTERSECTION. SEE TYPICAL GRADE BEAM CONSTRUCTION JOINT DETAIL FOR ACCEPTABLE CONSTRUCTION JOINT LOCATIONS.

15 TYPICAL DETAIL
GRADE BEAM "T" INTERSECTION TOP BAR PLACEMENT DIAGRAM
1" = 1'-0"

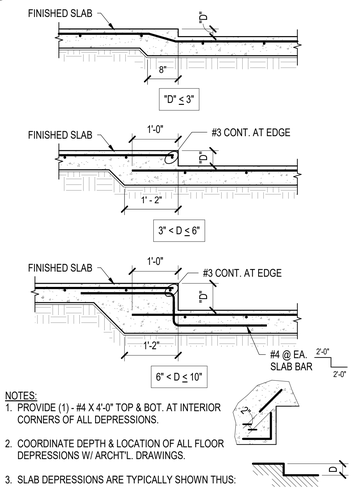


HOUSEKEEPING PAD REINF. SCHEDULE

PAD THICKNESS	TOP REINFORCEMENT	BOT. REINFORCEMENT
t ≤ 4"	6 X 6 - W2.9 X W2.9	NONE
4 < t ≤ 6"	4 X 4 - W4.0 X W4.0	NONE
6 < t ≤ 8"	4 X 4 - W5.5 X W5.5	NONE
8 < t ≤ 12"	#4 @ 12" O.C. EACH WAY	#3 @ 18" O.C. EACH WAY
12 < t ≤ 16"	#4 @ 12" O.C. EACH WAY	#4 @ 12" O.C. EACH WAY

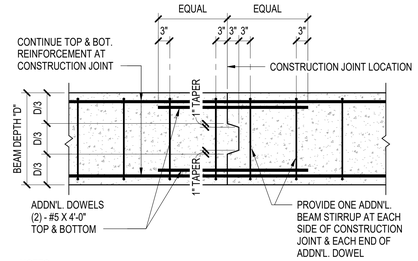
NOTES:
1. GENERAL CONTRACTOR TO COORDINATE WITH MECHANICAL DRAWINGS & SPECIFICATIONS TO DETERMINE REQUIREMENTS FOR HOUSEKEEPING PADS OVER SLAB-ON-GRADE & PROVIDE WHERE REQUIRED WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT. COORDINATE DIMENSIONS & OTHER SPECIAL REQUIREMENTS WITH EQUIPMENT MANUFACTURERS AS REQUIRED.
2. GENERAL CONTRACTOR SHALL NOTIFY ENGINEER OF ALL HOUSEKEEPING PAD LOCATION & THICKNESS PRIOR TO INSTALLATION.

14 TYPICAL DETAIL - CONC. ISLAND OR HOUSEKEEPING PAD (SLAB-ON-GRADE)
1" = 1'-0"



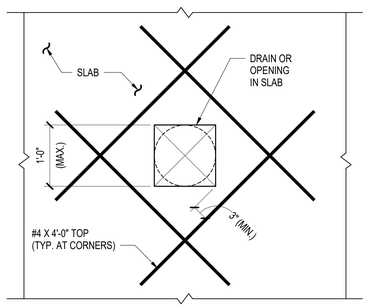
NOTES:
1. PROVIDE (1) - #4 X 4'-0" TOP & BOT. AT INTERIOR CORNERS OF ALL DEPRESSIONS.
2. COORDINATE DEPTH & LOCATION OF ALL FLOOR DEPRESSIONS W/ ARCHT'L. DRAWINGS.
3. SLAB DEPRESSIONS ARE TYPICALLY SHOWN THUS:

13 TYPICAL DETAIL AT FLOOR DEPRESSIONS
3/4" = 1'-0"

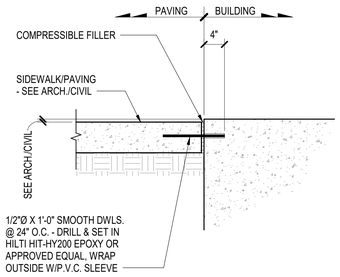


NOTES:
1. GENERAL CONTRACTOR SHALL SUBMIT DESIRED CONSTRUCTION JOINT LAYOUT AS A SHOP DRAWING FOR APPROVAL A MINIMUM OF TWO WEEKS PRIOR TO POUR.
2. CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED WITH REINFORCING STEEL SUPPLIER & ERECTOR.
3. JOINT LOCATIONS SHALL BE WITHIN TWO FEET EITHER SIDE OF BEAM MIDSPAN.
4. FOR JOINT LOCATIONS OTHER THAN WITHIN TWO FEET EITHER SIDE OF BEAM MIDSPAN, CONTRACTOR SHALL COORDINATE REQUIRED ADDITIONAL REINFORCEMENT WITH THE ENGINEER ON THE SHOP DRAWINGS.

12 TYPICAL DETAIL
GRADE BEAM CONSTRUCTION JOINT
3/4" = 1'-0"



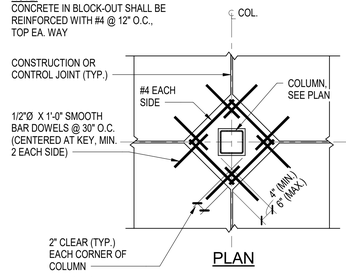
11 TYPICAL DETAIL - ADDITIONAL REINFORCEMENT OPENING AT SLAB-ON-GRADE
1" = 1'-0"



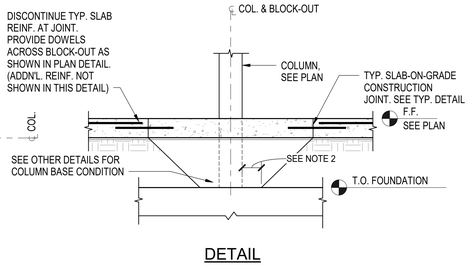
NOTE:
SMOOTH DOWELS REQUIRED ONLY WITHIN 10 FEET OF DOORS

7 TYPICAL DETAIL - DOWELS TO FLATWORK
1" = 1'-0"

NOTE:
CONCRETE IN BLOCK-OUT SHALL BE REINFORCED WITH #4 @ 12" O.C., TOP EA. WAY



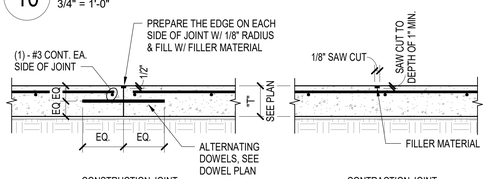
PLAN



DETAIL

NOTES:
1. GENERAL CONTRACTOR TO COORDINATE REQUIRED SIZE OF BLOCK-OUT FOR COLUMNS. SUBMIT THE DESIRED BLOCK-OUT SIZE TO ARCHITECT FOR APPROVAL.
2. PROVIDE 3" MINIMUM CONCRETE COVER ALL AROUND COLUMN.
3. AT G.C. OPTION, BLOCK-OUTS MAY BE CIRCULAR. MAINTAIN EDGE DISTANCE MINIMUMS AS SHOWN ABOVE, WITH (8) SMOOTH DOWELS (MIN.)
4. INCREASE BLOCK-OUT SIZE AS REQUIRED AT BRACES.

10 TYPICAL DETAIL
BLOCK-OUT AROUND COLUMN (SLAB-ON-GRADE)
3/4" = 1'-0"

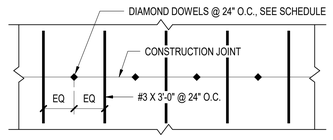


CONSTRUCTION JOINT

CONTRACTION JOINT

DOWEL SCHEDULE	
SLAB DEPTH	DOWEL SIZE
5" - 6"	1/4" X 4 1/2 X 4 1/2
7" - 8"	3/8" X 4 1/2 X 4 1/2

NOTE:
CONTACT ENGINEER OF RECORD FOR SLAB DEPTHS NOT LISTED IN THIS SCHEDULE



DOWEL PLAN

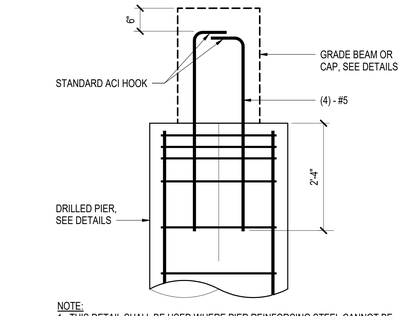
CONSTRUCTION JOINT NOTES:
1. SLAB REINFORCEMENT SHALL BE CHAIRED BY SOIL SUPPORTED SLAB BOLSTERS.
2. BREAK BOND BETWEEN NEW & PREVIOUSLY PLACED SLAB BY SPRAYING OR PAINTING EXPOSED SIDE OF KEY & DOWEL WITH A CURING COMPOUND, ASPHALTIC EMULSION OR FORM OIL.
3. SUBGRADE SHALL BE FREE OF STANDING WATER AT THE TIME OF CONCRETE PLACEMENT.

JOINT SPACING NOTE:
1. PROVIDE CONTROL AND/OR CONSTRUCTION JOINTS AT EVERY COLUMN CENTERLINE & AT A MAXIMUM SPACING OF 36 TIMES THE SLAB THICKNESS, NOT TO EXCEED 15'-0" U.N.O.

SAW CONTRACTION SPACING JOINT NOTES:
1. MAKE SAW CUT WITH SOFF-CUT AS SOON AS SLAB IS ABLE TO SUPPORT WEIGHT OF WORKERS & SAWING EQUIPMENT WITHOUT DAMAGE TO FINISH SURFACE OF SLAB.
2. ALL JOINTS SHALL BE CONTINUOUS. DO NOT OFFSET OR STAGGER JOINTS.
3. CLEAN JOINT PRIOR TO FILLING JOINT.

JOINT FILLER MATERIAL NOTES:
1. FILLER MATERIAL USED SHALL HAVE A MINIMUM SHORE "A" HARDNESS OF 80, & SHALL CONFORM TO ASTM D2240. JOINT FILLER SHALL BE APPROVED BY ENGINEER PRIOR TO APPLICATION. JOINT FILLER SHALL BE EUCO 700 BY EUCLID PRODUCTS OR APPROVED EQUAL.
2. WHERE POSSIBLE, FILLER MATERIAL SHALL BE APPLIED WHEN BUILDING IS UNDER PERMANENT TEMPERATURE CONTROL. THIS SHALL BE EITHER AT THE END OF CONSTRUCTION OF THE COMPLETE BUILDING SHELL, OR A MINIMUM OF 90 DAYS AFTER SLAB CONSTRUCTION.
3. FOLLOW STRICTLY THE MANUFACTURER'S RECOMMENDED PROCEDURES FOR APPLYING THE JOINT FILLER.
4. ALL EXPOSED SLAB JOINTS SHALL BE FILLED WITH FILLER MATERIAL, U.N.O. ON ARCH. DOCUMENTS

9 TYPICAL DETAIL - CONSTRUCTION AND CONTRACTION JOINTS (SLAB-ON-GRADE)
1" = 1'-0"

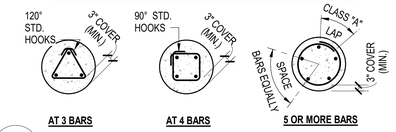


NOTE:
1. THIS DETAIL SHALL BE USED WHERE PIER REINFORCING STEEL CANNOT BE ACCOMMODATED WITHIN WIDTH OF GRADE BEAM/CAP.
2. SEE TYPICAL DRILLED PIER DETAIL FOR ADDITIONAL INFORMATION.

4 TYPICAL DETAIL
SHAFT SIZE LARGER THAN GRADE BEAM /CAP
3/4" = 1'-0"

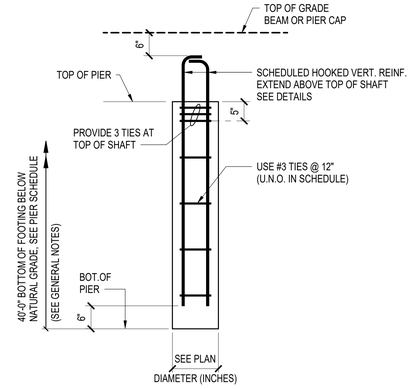
DRILLED PIER VERTICAL REINFORCEMENT SCHEDULE

SHAFT DIAMETER (INCHES)	VERTICAL REINFORCEMENT	TIES
12	3 - #5	#3 @ 12"
14, 16	4 - #5	#3 @ 12"
18, 20	4 - #6	#3 @ 12"
22, 24, 26	5 - #7	#3 @ 12"
28, 30	5 - #8	#3 @ 12"
36	5 - #9	#3 @ 12"
42	7 - #9	#3 @ 12"
48	7 - #10	#3 @ 12"
54	9 - #10	#3 @ 12"
60	9 - #11	#4 @ 12"
66	11 - #11	#4 @ 12"
72	14 - #11	#4 @ 12"

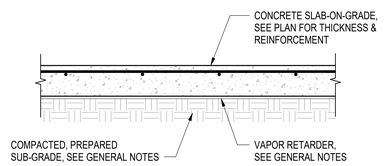


AT 3 BARS **AT 4 BARS** **5 OR MORE BARS**

3 SHAFT BAR PLACEMENT PLANS
3/4" = 1'-0"



2 TYPICAL DETAIL - DRILLED PIER
3/4" = 1'-0"



NOTE:
UNLESS SPECIFIED ELSEWHERE, VAPOR RETARDER SHALL BE A MINIMUM OF 15 MILS THICK MEETING OR EXCEEDING ASTM E-1745, CLASS A, & HAVE NO MORE THAN 0.01 PERMS WHEN TESTED IN ACCORDANCE WITH ASTM E-96.

1 TYPICAL DETAIL SLAB-ON-GRADE CONSTRUCTION SUBGRADE PREPARATION
1" = 1'-0"

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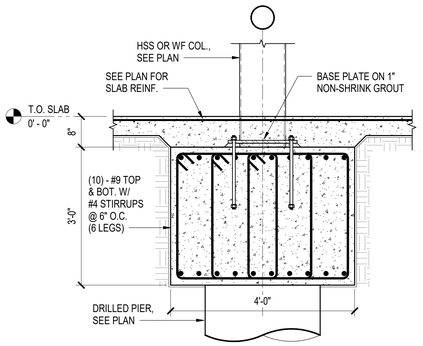
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

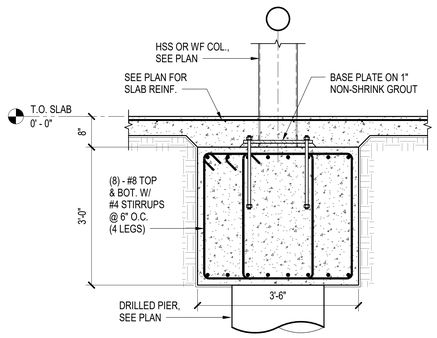
RMA PROJECT NO: 24.154

FOUNDATION DETAILS

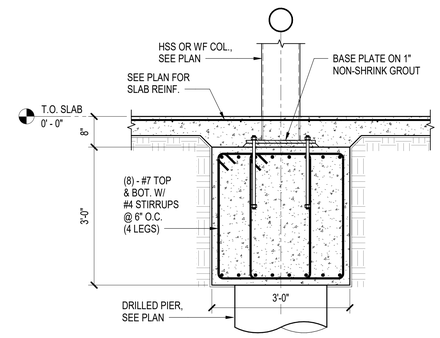
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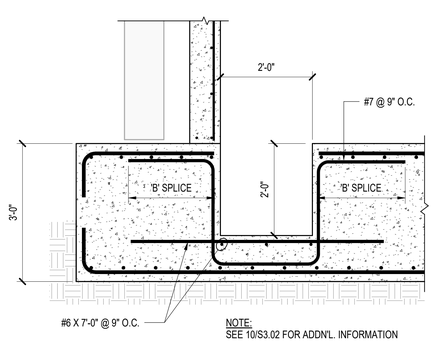
20 DETAIL AT INTERIOR GRADE BEAM AND COLUMN
3/4" = 1'-0"



16 DETAIL AT INTERIOR GRADE BEAM AND COLUMN
3/4" = 1'-0"

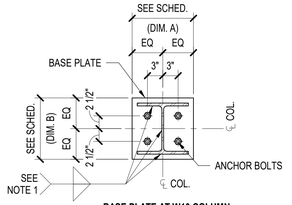


12 DETAIL AT INTERIOR GRADE BEAM AND COLUMN
3/4" = 1'-0"



NOTE:
SEE 10/S3.02 FOR ADDN'L. INFORMATION

8 DETAIL AT ELEVATOR SUMP PIT
3/4" = 1'-0"

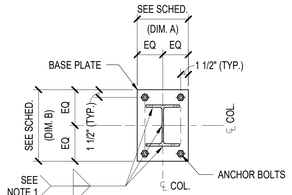


BASE PLATE AT W10 COLUMN

COLUMN SCHEDULE		
COLUMN	BASE PLATE A X B X THICKNESS	ANCHOR BOLTS
W 10 X 49	12" x 12" x 3/4"	(4) - 3/4"Ø
W 10 X 54	14" x 12" x 3/4"	(4) - 3/4"Ø
W 10 X 68	14" x 12" x 3/4"	(4) - 3/4"Ø

NOTES:
1. MINIMUM FILLET WELD ACCORDING TO A.I.S.C. (1/4" MIN.).
2. USE OVERSIZED HOLES FOR ANCHOR BOLTS ACCORDING TO A.I.S.C.
3. WASHERS SHALL BE INSTALLED OVER OVERSIZED HOLES.
4. CONTRACTOR SHALL PROVIDE 1/8" THICK TEMPLATE FOR ANCHOR BOLT INSTALLATION.

4 TYPICAL DETAIL BASE PLATE AT WIDE FLANGE COLUMNS
1" = 1'-0"

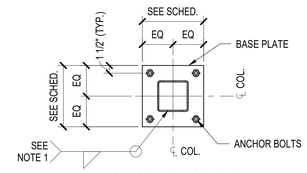


BASE PLATE AT W8 COLUMN

COLUMN SCHEDULE		
COLUMN	BASE PLATE A X B X THICKNESS	ANCHOR BOLTS
W 8 X 35	16" x 10" x 3/4"	(4) - 3/4"Ø
W 8 X 40	16" x 10" x 3/4"	(4) - 3/4"Ø
W 8 X 48	16" x 10" x 3/4"	(4) - 3/4"Ø
W 8 X 58	16" x 12" x 3/4"	(4) - 3/4"Ø
W 8 X 67	16" x 12" x 3/4"	(4) - 3/4"Ø

NOTES:
1. MINIMUM FILLET WELD ACCORDING TO A.I.S.C. (1/4" MIN.).
2. USE OVERSIZED HOLES FOR ANCHOR BOLTS ACCORDING TO A.I.S.C.
3. WASHERS SHALL BE INSTALLED OVER OVERSIZED HOLES.
4. CONTRACTOR SHALL PROVIDE 1/8" THICK TEMPLATE FOR ANCHOR BOLT INSTALLATION.

3 TYPICAL DETAIL BASE PLATE AT WIDE FLANGE COLUMNS
1" = 1'-0"

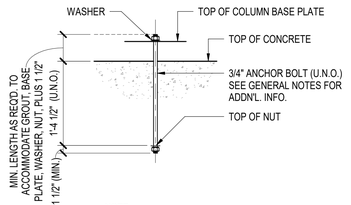


BASE PLATE AT HSS COLUMN

COLUMN SCHEDULE		
COLUMN	BASE PLATE	ANCHOR BOLTS
HSS 6 X 6	12" X 12" X 3/4"	(4) - 3/4"Ø
HSS 8 X 8	14" X 14" X 3/4"	(4) - 3/4"Ø
HSS 10 X 10	16" X 16" X 3/4"	(4) - 3/4"Ø
HSS 12 X 12	18" X 18" X 3/4"	(4) - 3/4"Ø

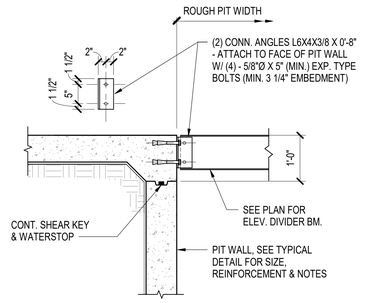
NOTES:
1. MINIMUM FILLET WELD ACCORDING TO A.I.S.C. (1/4" MIN.).
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4. CONTRACTOR SHALL PROVIDE 1/8" THICK TEMPLATE FOR ANCHOR BOLT INSTALLATION.

2 TYPICAL DETAIL BASE PLATE AT HSS COLUMNS
1" = 1'-0"



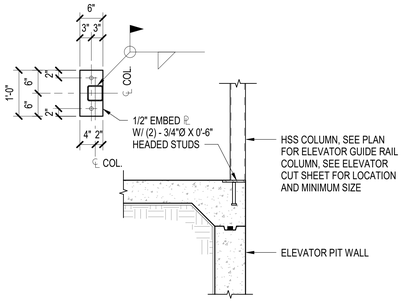
NOTE:
ALL ANCHOR RODS SHALL CONFORM TO ASTM F1554, GR. 36,U.N.O.

1 TYPICAL DETAIL - ANCHOR BOLTS
1" = 1'-0"



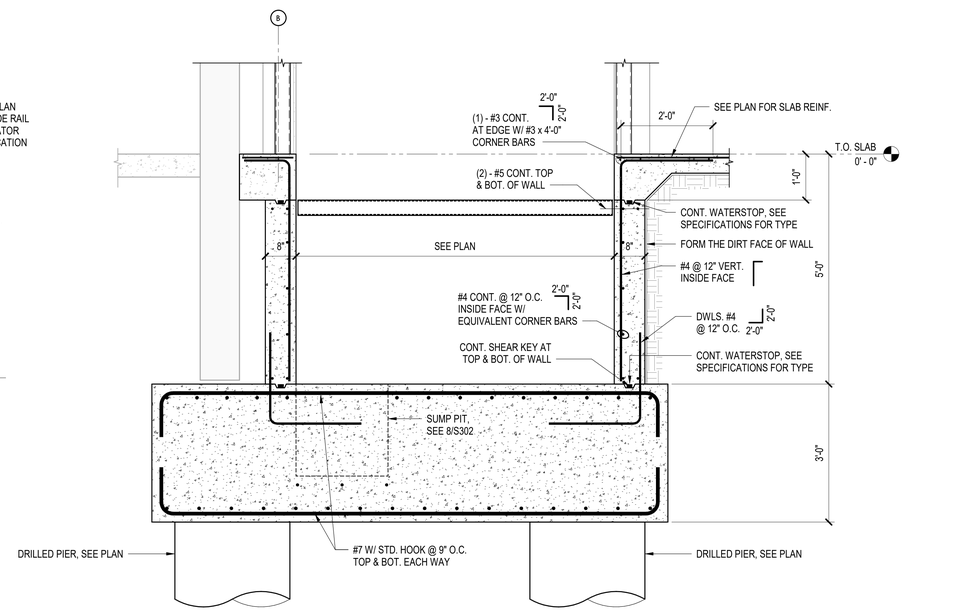
NOTES:
INSTALL EXPANSION BOLTS ACCORDING TO MANUFACTURER'S REQUIREMENTS.

19 TYPICAL DETAIL ELEVATOR DIVIDER BEAM CONNECTION TO PIT WALL
3/4" = 1'-0"



NOTES:
SEE "TYP. DETAIL - ELEVATOR PIT" FOR ADDN'L. INFO. NOT SHOWN

15 DETAIL AT HSS COLUMN BEHIND ELEVATOR RAIL
3/4" = 1'-0"

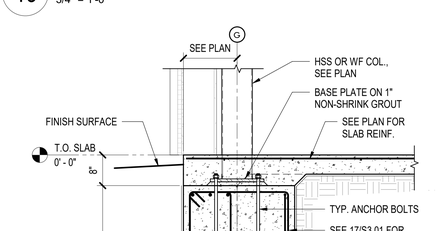


NOTES:
1. BRACE WALL AS REQUIRED UNTIL UPPER SLAB IS POURED. & HAS REACHED 2,500 PSI MIN.
2. SEE PLAN FOR SUMP PIT, WHERE REQUIRED. SLOPE PIT BOTTOM SLAB 1" TO SUMP PIT.
3. SEE OTHER DETAILS FOR ELEVATOR SILL DETAIL.
4. GENERAL CONTRACTOR TO COORDINATE ELEVATOR PIT SIZE, DEPTH, SILL DETAIL & BLOCK-OUT REQUIREMENTS WITH ELEVATOR MANUFACTURER'S APPROVED SHOP DRAWINGS.
5. WHERE ELEVATOR PIT SLAB IS BLOCKED OUT TO ALLOW FOR PLACEMENT OF HYDRAULIC ELEVATOR JACK, FILL BLOCK-OUT WITH CONCRETE AFTER JACK IS SET.

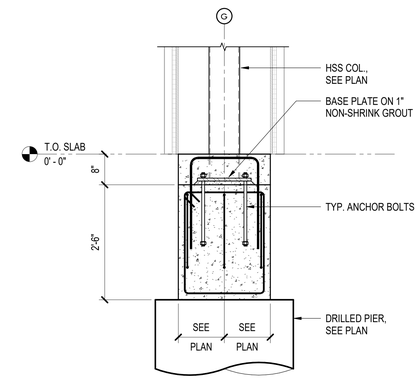
10 DETAIL AT ELEVATOR PIT
3/4" = 1'-0"

NOTES:
1. PROVIDE CONTINUOUS ELEVATOR SILL ANGLE AT THRESHOLD OF ELEVATOR PIT. SEE ARCHT'L. DRAWINGS FOR LOCATION.
2. INSTALL EXPANSION BOLTS ACCORDING TO MANUFACTURER'S REQUIREMENTS.

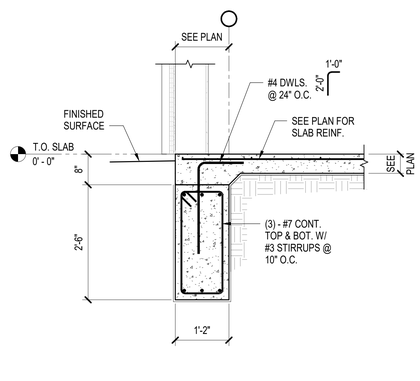
14 TYPICAL DETAIL ELEVATOR PIT SILL ANGLE CONNECTION
3/4" = 1'-0"



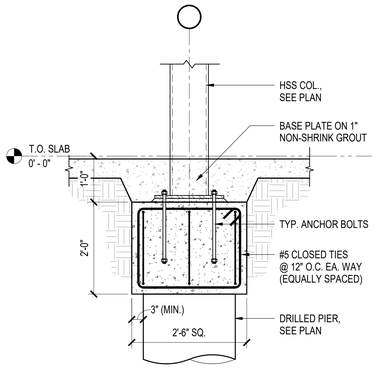
18 DETAIL AT EXTERIOR GRADE BEAM AT HSS COLUMN
3/4" = 1'-0"



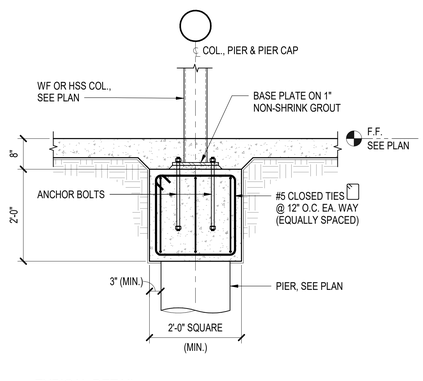
17 DETAIL AT HSS COLUMN
3/4" = 1'-0"



13 DETAIL AT EXTERIOR GRADE BEAM
3/4" = 1'-0"



6 DETAIL AT EXTERIOR HSS COLUMN
3/4" = 1'-0"



5 TYPICAL DETAIL INTERIOR COLUMN WITH PIER CAP
3/4" = 1'-0"

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PROFESSIONAL ENGINEER
IN
DIGITALLY SIGNED: 03/02/2026
PINNACLE STRUCTURAL ENGINEERS
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	12/15/25	50% DD
	01/14/26	100% DD
	02/27/26	ISSUE FOR BID

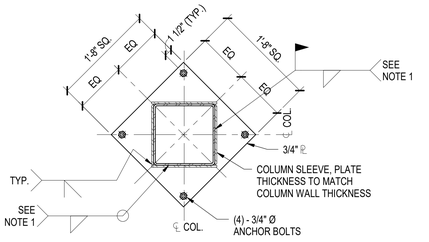
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

FOUNDATION DETAILS

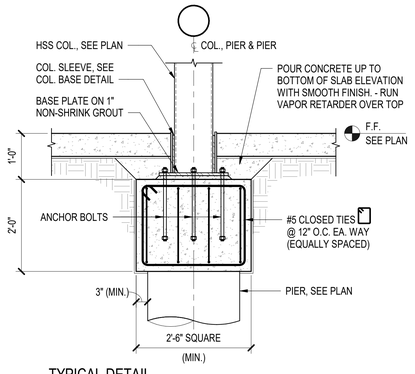
S3.02



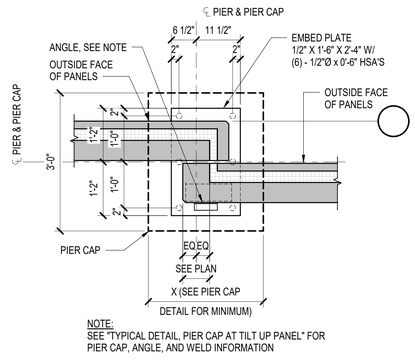
BASE PLATE AT HSS COLUMN

NOTES:
1. MINIMUM FILLET WELD ACCORDING TO A.I.S.C. (1/4" MIN.).
2. USE OVERSIZED HOLES FOR ANCHOR BOLTS ACCORDING TO A.I.S.C.
3. WASHERS SHALL BE INSTALLED OVER OVERSIZED HOLES.
4. CONTRACTOR SHALL PROVIDE 1/8" THICK TEMPLATE FOR ANCHOR BOLT INSTALLATION.

20 TYPICAL DETAIL
BASE PLATE AT HSS COLUMNS
1" = 1'-0"

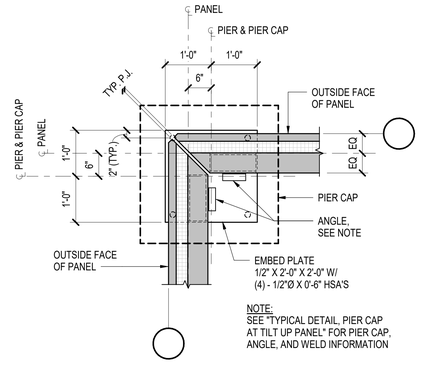


16 TYPICAL DETAIL
INTERIOR COLUMN WITH PIER CAP
3/4" = 1'-0"



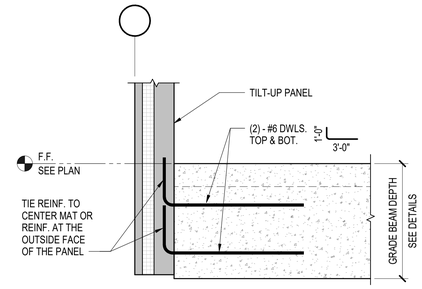
NOTE:
SEE "TYPICAL DETAIL, PIER CAP AT TILT UP PANEL" FOR PIER CAP, ANGLE, AND WELD INFORMATION

12 PLAN VIEW AT OVERLAPPING TILT-UP PANELS
3/4" = 1'-0"



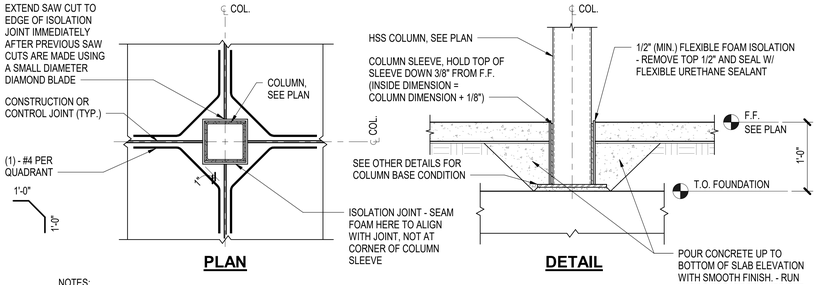
NOTE:
SEE "TYPICAL DETAIL, PIER CAP AT TILT UP PANEL" FOR PIER CAP, ANGLE, AND WELD INFORMATION

8 PLAN VIEW AT PANEL JOINT (MITRED CORNER)
3/4" = 1'-0"



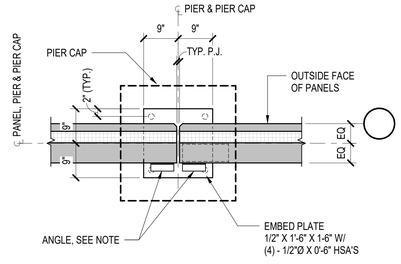
NOTE:
GRADE BEAM AND SLAB REINFORCEMENT ARE NOT SHOWN FOR CLARITY.

4 TYPICAL DETAIL
DOWELS TO TILT-UP PANEL AT GRADE BEAM
3/4" = 1'-0"



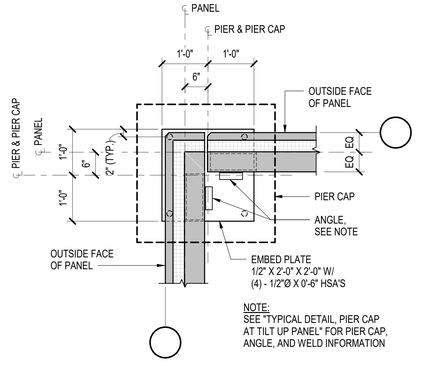
NOTES:
1. BOTTOM OF COLUMN MUST FULLY BEAR AGAINST BASE PLATE.
2. BASE PLATE AND COLUMN SLEEVE MUST BE PLUMBED PRIOR TO PLACING SLAB ON GRADE AND ERECTING COLUMN.
3. COLUMN SLEEVE MUST BE FREE FROM STANDING WATER PRIOR TO CAULKING.
4. PREFABRICATED COLUMN SLEEVE SUBSTITUTES MAY BE PROVIDED. SUBMIT TO A/E FOR APPROVAL.

19 TYPICAL DETAIL - COLUMN BASE (SLAB-ON-GRADE)
3/4" = 1'-0"



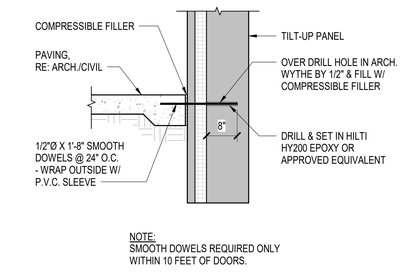
NOTE:
SEE "TYPICAL DETAIL, PIER CAP AT TILT UP PANEL" FOR PIER CAP, ANGLE, AND WELD INFORMATION

11 PLAN VIEW AT PANEL JOINT (INTERIOR)
3/4" = 1'-0"



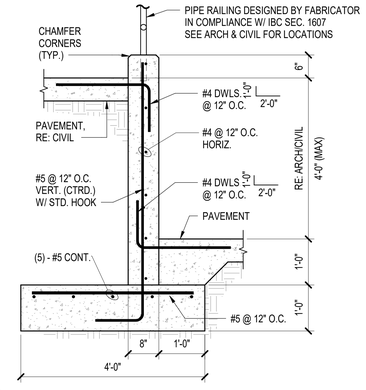
NOTE:
SEE "TYPICAL DETAIL, PIER CAP AT TILT UP PANEL" FOR PIER CAP, ANGLE, AND WELD INFORMATION

7 PLAN VIEW AT PANEL JOINT (CORNER)
3/4" = 1'-0"

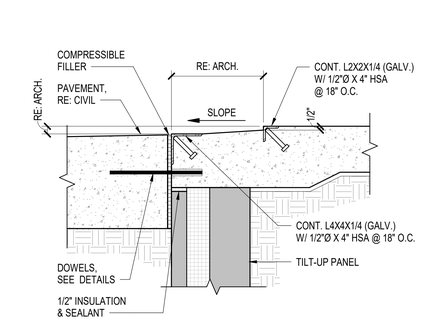


NOTE:
SMOOTH DOWELS REQUIRED ONLY WITHIN 10 FEET OF DOORS.

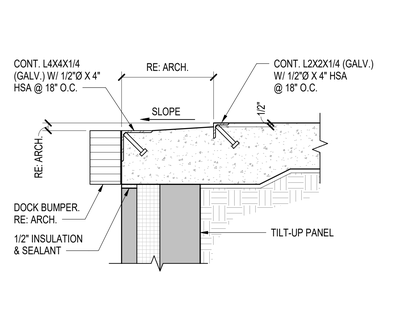
3 TYPICAL DETAIL
DOWELS TO SITE SLAB AT TILT-UP PANEL
3/4" = 1'-0"



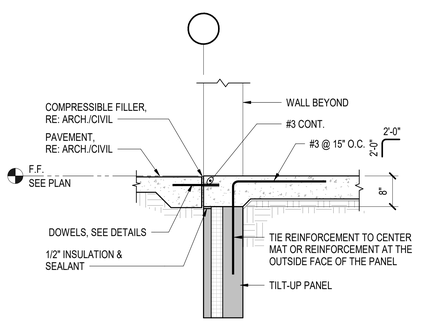
18 DETAIL AT RETAINING WALL AT TRUCK DOCK
3/4" = 1'-0"



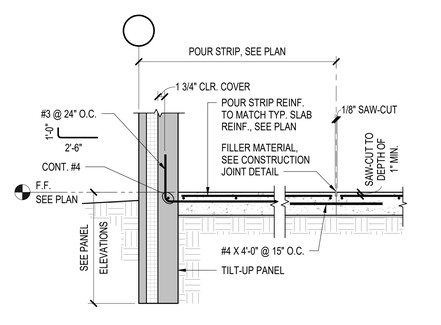
14 DETAIL AT OH DOOR SILL (AT GRADE)
1 1/2" = 1'-0"



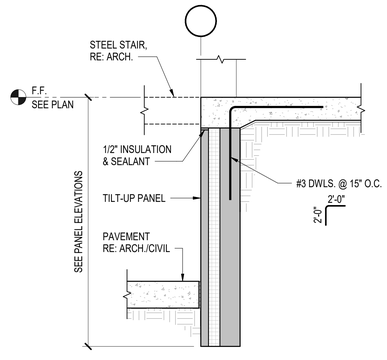
10 DETAIL AT OH DOOR SILL (DOCK HEIGHT)
1 1/2" = 1'-0"



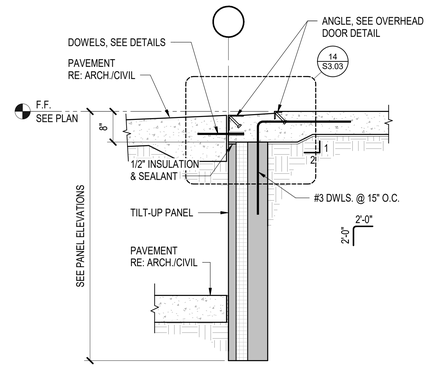
6 TYPICAL DETAIL
DOOR OR WINDOW AT TILT-UP PANEL
3/4" = 1'-0"



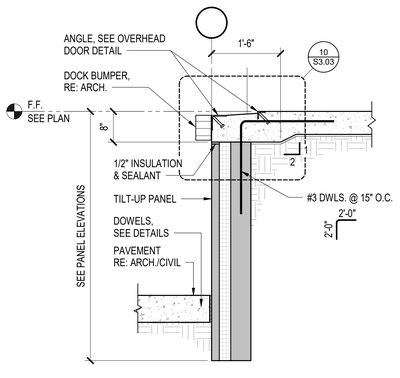
2 DETAIL AT EXTERIOR
3/4" = 1'-0"



17 TYPICAL DETAIL AT MAN DOOR AT DOCK
3/4" = 1'-0"

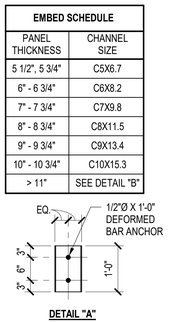


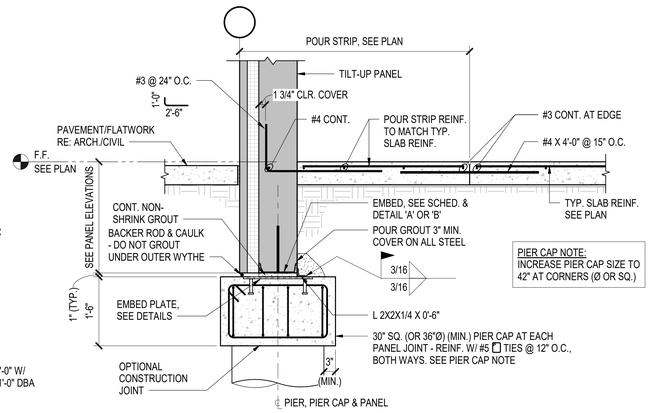
13 DETAIL AT OVERHEAD DOOR SILL (AT GRADE)
3/4" = 1'-0"



9 DETAIL AT OVERHEAD DOOR SILL (DOCK HEIGHT)
3/4" = 1'-0"

EMBED SCHEDULE	
PANEL THICKNESS	CHANNEL SIZE
5 1/2", 5 3/4"	C5X6.7
6" - 6 3/4"	C6X8.2
7" - 7 3/4"	C7X9.8
8" - 8 3/4"	C8X11.5
9" - 9 3/4"	C9X13.4
10" - 10 3/4"	C10X15.3
> 11"	SEE DETAIL "B"





5 TYPICAL DETAIL - PIER CAP AT INSULATED TILT UP PANEL
3/4" = 1'-0"

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License No. 39291
PROFESSIONAL ENGINEER
IN
DIGITALLY SIGNED: 03/02/2026
PINNACLE STRUCTURAL ENGINEERS
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	01/14/26	100% DD
	02/27/26	ISSUE FOR BID

LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

FOUNDATION DETAILS

S3.03

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ARCHITECTS

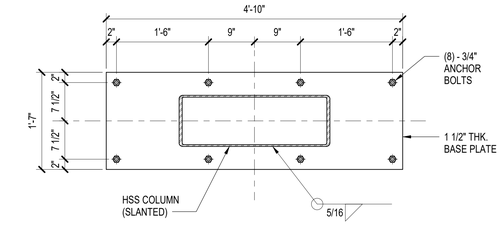
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p 225.383.4321

RITTERMAHER.COM

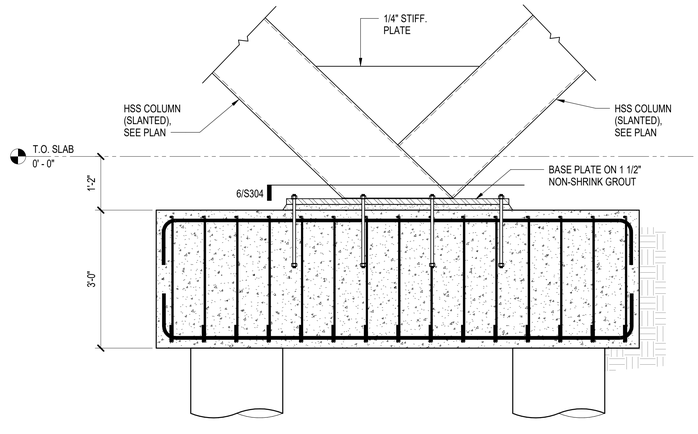
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KEY PLAN:



6 DETAIL AT BASE PLATE AT "V" SHA[PED COLUMN
1" = 1'-0"



4 DETAIL AT "V" SHAPED COLUMNS
3/4" = 1'-0"

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	02/27/26	ISSUE FOR BID

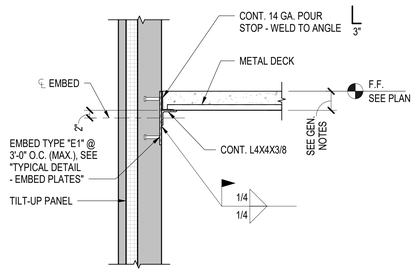
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

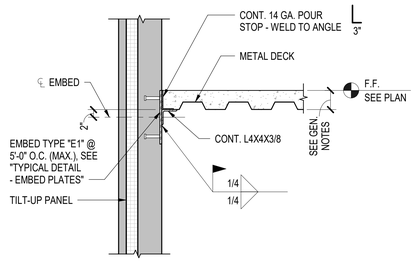
RMA PROJECT NO: 24.154

FOUNDATION DETAILS

S3.04

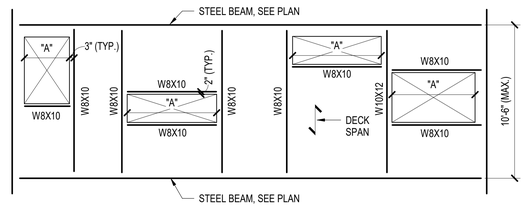


20 TYPICAL DETAIL - PERPENDICULAR DECK SUPPORT AT TILT-UP PANEL AT FLOOR
3/4" = 1'-0"



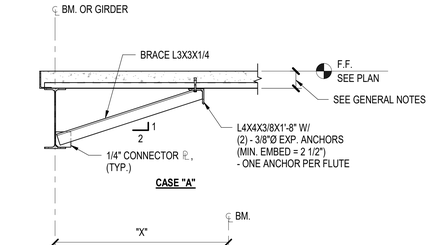
16 TYPICAL DETAIL - PARALLEL DECK SUPPORT AT TILT-UP PANEL AT FLOOR
3/4" = 1'-0"

"A" = OPENING DIMENSION IN THE DIRECTION PERPENDICULAR TO DECK SPAN. USE THIS DETAIL FOR 2'-0" < "A" < 8'-0".

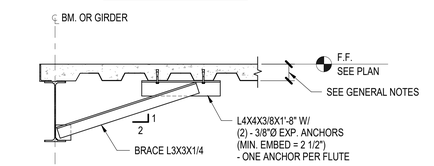


NOTES:
1. COORDINATE OPENING SIZE AND LOCATIONS WITH ARCHITECTURAL AND MEP DRAWINGS.
2. THIS DETAIL SHOWS TYPICAL CONFIGURATIONS. VERIFY WITH ENGINEER FOR ALL OTHER CONDITIONS.
3. CONTACT ENGINEER OF RECORD IF DIMENSION "A" EXCEEDS 8'-0".
4. DECK MANUFACTURER TO PROVIDE 14 GAGE METAL EDGE CLOSURE AROUND EACH OPENING UNLESS DETAILED OTHERWISE.
5. IF AN OPENING EDGE IS WITHIN 12" OF THE CENTERLINE OF A COMPOSITE STEEL BEAM, CONTACT ENGINEER FOR GUIDANCE.
6. FOR OPENINGS ON BOTH SIDES OF AN ADDITIONAL FRAMING MEMBER, USE A W10X12 BEAM, AS SHOWN.
7. IF OPENING LOCATION RESULTS IN SINGLE-SPAN METAL DECK, CONTACT ENGINEER OF RECORD FOR GUIDANCE.

15 TYPICAL DETAIL FRAMED OPENINGS IN COMPOSITE METAL DECK SLAB
1/8" = 1'-0"



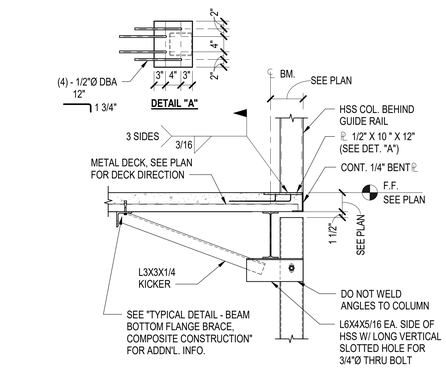
DECK PERPENDICULAR (2 CASES)



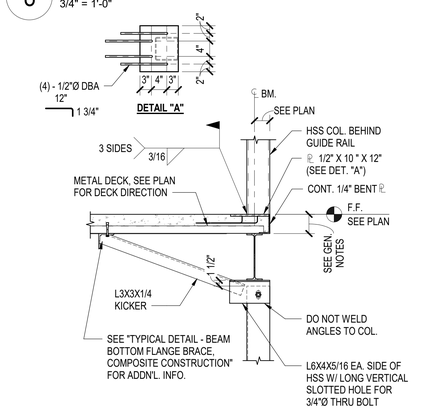
DECK PARALLEL

NOTE:
1. SEE PLAN FOR LOCATIONS OF BRACES.
2. FIELD WELDING OF 1/4" CONNECTION PLATES SHALL BE AT FABRICATOR'S OPTION.
3. BRACE SIZES SHOWN ARE TYP. UNLESS NOTED OTHERWISE ON PLAN.

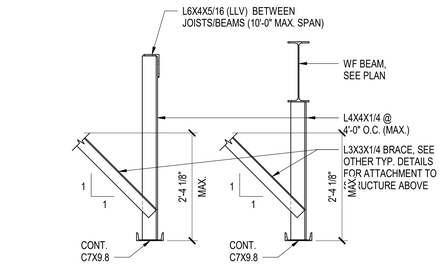
11 TYPICAL DETAIL - BM. BOT. FLANGE BRACE COMPOSITE CONSTRUCTION
3/4" = 1'-0"



8 DETAIL AT HSS COLUMNS AT ELEVATOR AT COMPOSITE EXTENDED EDGE
3/4" = 1'-0"

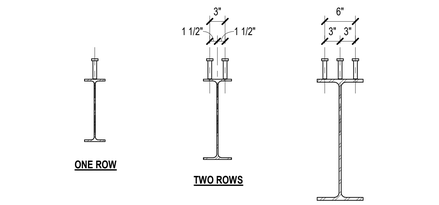


7 DETAIL AT HSS COLUMNS AT ELEVATOR AT COMPOSITE EDGE
3/4" = 1'-0"



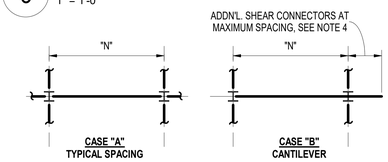
NOTES:
1. SLOPE OF BRACE SHALL BE AS CLOSE TO 1:1 AS POSSIBLE & ALLOW THE BRACE TO CONNECT TO THE TOP FLANGE OF THE CLOSEST BEAM/JOIST.
2. USE 3/16" ALL AROUND FILLET WELDS AT ALL WELDS NOT SHOWN, U.N.O.

4 TYPICAL DETAIL - WINDOW WALL SUPPORT
3/4" = 1'-0"



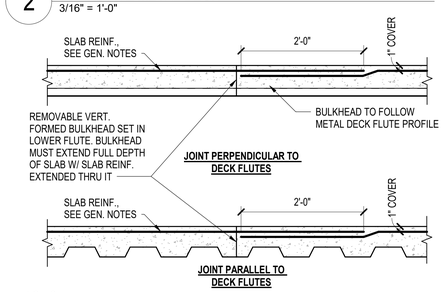
NOTES:
1. SHEAR CONNECTORS IN ONE ROW SHALL BE PLACED DIRECTLY OVER THE BEAM WEB.
2. PLACE STUDS IN A SINGLE ROW WHERE SPACING REQUIREMENTS PERMIT. STUDS SHALL BE PLACED IN TWO ROWS OR THREE ROWS ONLY WHERE REQUIRED IN ORDER TO PLACE THE TOTAL NUMBER OF STUDS. SEE TYPICAL DETAIL, SHEAR CONNECTOR PLACEMENT DIAGRAM.
3. PROVIDE MINIMUM 1" SPACING FROM EDGE OF FLANGE TO CENTERLINE OF STUD.
4. SUBMIT SHOP DRAWINGS SHOWING PLACEMENT OF SHEAR CONNECTORS FOR ENGINEER APPROVAL.

3 TYPICAL DETAIL SHEAR CONNECTOR SPACING CRITERIA
1" = 1'-0"



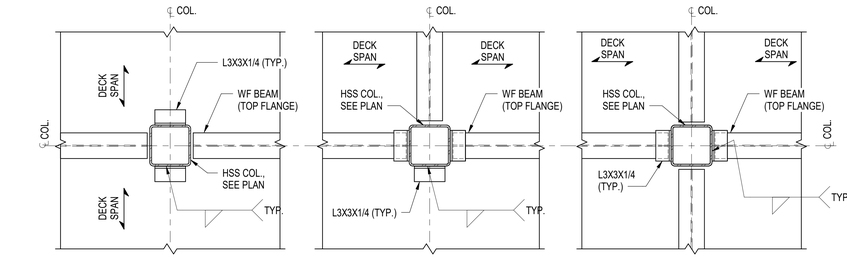
NOTES:
1. N = SPECIFIED NUMBER OF SHEAR CONNECTORS. REFER TO PLAN.
2. UNLESS NOTED OTHERWISE ON PLAN, SHEAR CONNECTORS SHALL BE DISTRIBUTED ALONG THE LENGTH OF THE BEAM AS SHOWN ON DETAILS ABOVE.
3. REFER TO GENERAL NOTES FOR SHEAR CONNECTOR DIAMETER AND LENGTH.
4. MAXIMUM SPACING OF SHEAR CONNECTORS SHALL BE 36 INCHES.
5. MAXIMUM DISTANCE FROM END OF BEAM TO CENTER OF FIRST SHEAR CONNECTOR SHALL BE 8 INCHES.
6. WHERE STEEL DECK CORRUGATIONS DO NOT ALLOW FOR AN EVEN SPACING OF SHEAR CONNECTORS WITH ONE STUD IN EACH FLUTE, ADDITIONAL STUDS IN A SECOND ROW (THIRD ROW WHERE REQUIRED) SHALL BE PLACED SUCH THAT THE HIGHEST DENSITY OF SHEAR CONNECTORS OCCURS NEAR THE BEAM SUPPORT.
7. WHERE THE SPECIFIED NUMBER OF SHEAR CONNECTORS IS LESS THAN THE BEAM SPAN LENGTH DIVIDED BY THE MAXIMUM SPACING (SEE NOTE 4), ADDITIONAL SHEAR CONNECTORS SHALL BE PROVIDED SUCH THAT THE MAXIMUM SPACING IS NOT EXCEEDED AT ANY LOCATION IN THE SPAN.
8. SUBMIT SHOP DRAWING PLACEMENT OF SHEAR CONNECTORS FOR ENGINEER'S APPROVAL.

2 TYPICAL DETAIL SHEAR CONNECTOR PLACEMENT DIAGRAMS COMPOSITE METAL DECK
3/16" = 1'-0"

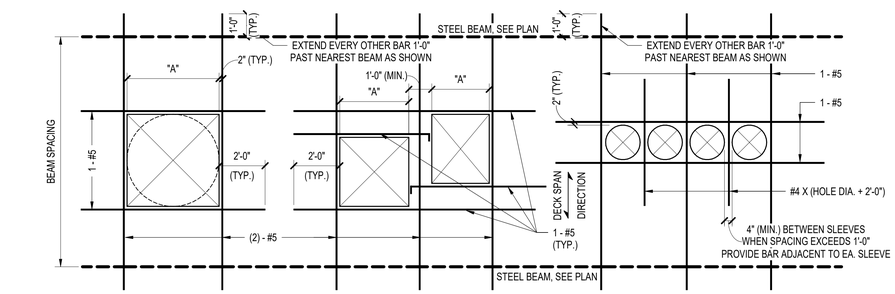


NOTES:
1. CONSTRUCTION JOINTS PARALLEL TO DECK FLUTES BEARING ON BEAMS LESS THAN 40-FT IN LENGTH SHALL BE LOCATED NO CLOSER THAN 5'-0" FROM THE CENTERLINE OF THE NEAREST GIRDER
2. CONSTRUCTION JOINTS PARALLEL TO DECK FLUTES BEARING ON BEAMS MORE THAN 40-FT IN LENGTH SHALL BE LOCATED NO CLOSER THAN THE BEAM LENGTH DIVIDED BY 8 FROM THE CENTERLINE OF THE NEAREST GIRDER
3. CONSTRUCTION JOINTS PERPENDICULAR TO DECK FLUTES SHALL BE LOCATED AT THE CENTER OF THE METAL DECK SPAN
4. AT CONTRACTOR'S OPTION, PREFORMED PERMANENT METAL SHEAR KEY FOLLOWING CONTOUR OF METAL DECK FLUTES MAY BE USED WITH SLAB REINFORCEMENT STOPPING EACH SIDE & #3 DOWELS X 2'-0" CENTERED @ 18" O.C.

1 TYPICAL DETAIL - CONSTRUCTION JOINT CONCRETE SLAB ON METAL DECK
1" = 1'-0"



18 TYPICAL DETAIL - METAL DECK SUPPORT AT COLUMN
1" = 1'-0"

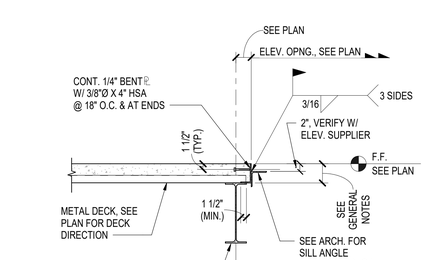


NOTES:
1. COORDINATE OPENING SIZE & LOCATIONS WITH ARCHITECTURAL DRAWINGS & MEP DRAWINGS.
2. MAXIMUM DIMENSION "A" SHALL BE 2'-0. REFER TO PLAN OR DETAIL ON DRAWINGS FOR FRAMING OF OPENINGS GREATER THAN 2'-0".
3. SLAB REINFORCEMENT SHALL BE CUT AROUND FORMED OPENING & SHALL EXTEND TO WITHIN 2" OF OPENING ON ALL SIDES.
4. OPENINGS 10" & SMALLER DO NOT REQUIRE ADDITIONAL REINFORCEMENT.
5. FIELD CUT OPENINGS IN METAL DECK AFTER SURROUNDING CONCRETE SLAB HAS BEEN POURED & HAS ATTAINED MINIMUM OF 75% OF ITS SPECIFIED 28-DAY COMPRESSIVE STRENGTH.
6. REINFORCEMENT PERPENDICULAR TO DECK SPAN SHALL BE PLACED ON TOP OF DECK FLUTES.
7. REINFORCEMENT PARALLEL TO DECK SPAN SHALL BE PLACED IN THE LOWER FLUTE & SHALL BE CHAIRED TO PROVIDE 3/4" COVER.
8. THIS DETAIL SHOWS TYPICAL CONDITIONS. VERIFY REINFORCING STEEL PLACEMENT WITH ENGINEER FOR ALL OTHER CONDITIONS.
9. PROVIDE REINFORCEMENT AS SHOWN FOR SQUARE, RECTANGULAR & ROUND OPENINGS.
10. IF AN OPENING EDGE IS WITHIN 12" OF THE CENTERLINE OF A STEEL BEAM SHOWN ON PLAN, CONTACT ENGINEER FOR GUIDANCE.

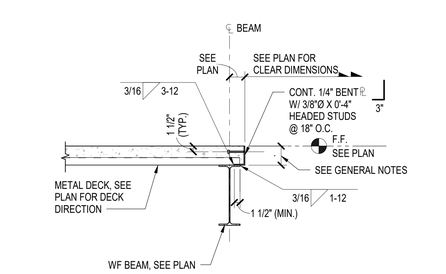
"A" = OPENING DIMENSION IN THE DIRECTION PERPENDICULAR TO DECK SPAN. USE THIS DETAIL FOR "A" ≤ 2'-0".

NOTE: WHEN SPACING BETWEEN MULTIPLE OPENINGS IS ≤ 2'-0", REINFORCEMENT SHOULD BE PLACED AS SHOWN IN SINGLE OPENING CONDITION.

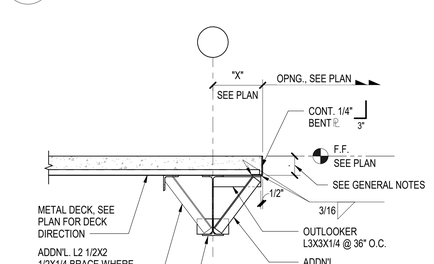
17 TYPICAL DETAIL - OPENINGS IN COMPOSITE METAL DECK SLAB, FLOOR OR ROOF
3/8" = 1'-0"



10 TYPICAL DETAIL ELEVATOR SILL AT DOOR
3/4" = 1'-0"

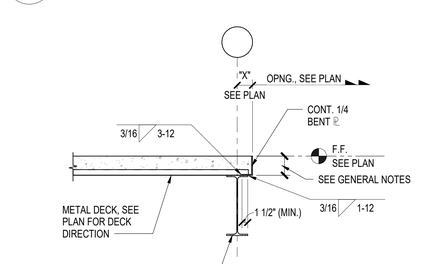


6 TYPICAL DETAIL STEEL STAIR AT FLOOR LANDING
3/4" = 1'-0"



NOTES:
1. THIS DETAIL APPLIES AT ALL CONDITIONS WHERE X > 6" U.N.O.
2. EDGE ANGLES MAY BE FIELD WELDED TO BEAMS AT THE CONTRACTORS OPTION.

9 TYPICAL DETAIL EDGE OF COMPOSITE METAL DECK AT OPENING
3/4" = 1'-0"



NOTES:
1. THIS DETAIL APPLIES AT ALL CONDITIONS WHERE X < 6" U.N.O.
2. EDGE ANGLES MAY BE FIELD WELDED TO BEAMS AT THE CONTRACTORS OPTION.

5 TYPICAL DETAIL EDGE OF COMPOSITE METAL DECK AT OPENING
3/4" = 1'-0"

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25105

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KEY PLAN:

SEAL:
STATE OF LOUISIANA
HEATH H. MICHEL
License No. 39291
PROFESSIONAL ENGINEER
IN
ENGINEERING
DIGITALLY SIGNED: 03/22/2026
PINNACLE STRUCTURAL ENGINEERS
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	01/14/26	100% DD
	02/27/26	ISSUE FOR BID

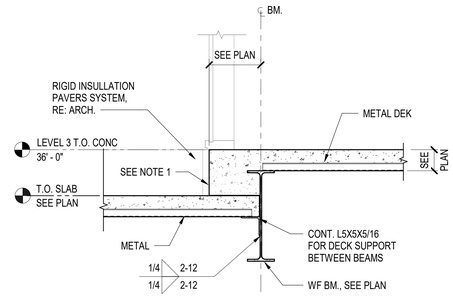
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

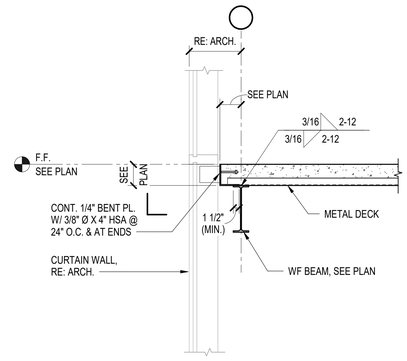
FRAMING DETAILS

S4.01

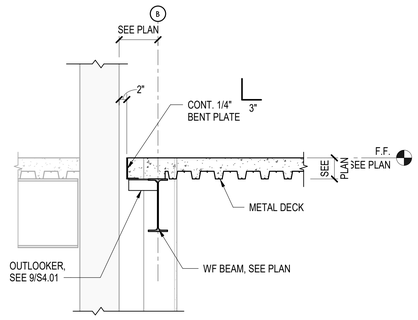


NOTE :
1. CONCRETE CONSTRUCTION JOINT . BOTTOM POUR MUST MEET FC' = 2500 PSI (MIN.) BEFORE TOPPING POUR CAN BE MADE. ROUGHEN BOTTOM POUR SURFACE APPLYING SECOND POUR

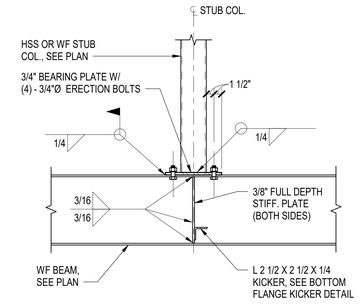
16 DETAIL AT BALCONY
3/4" = 1'-0"



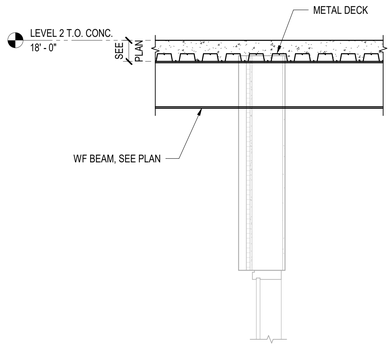
12 DETAIL AT EXTERIOR
3/4" = 1'-0"



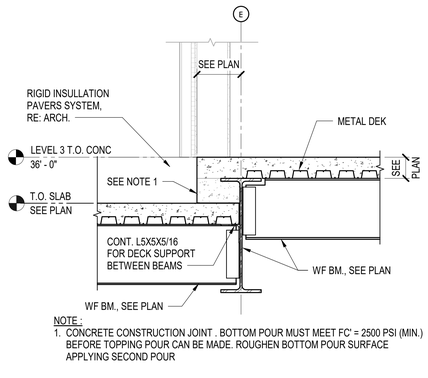
8 DETAIL AT EDGE OF SLAB
3/4" = 1'-0"



4 TYPICAL DETAIL - STUB COLUMN
1" = 1'-0"

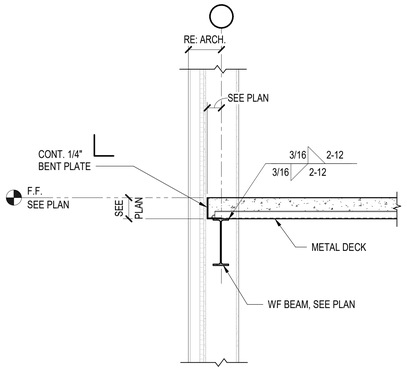


19 DETAIL AT EXTERIOR
3/4" = 1'-0"

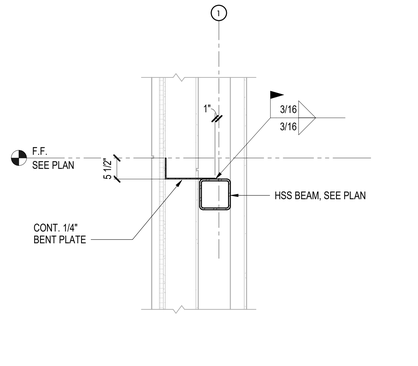


NOTE :
1. CONCRETE CONSTRUCTION JOINT . BOTTOM POUR MUST MEET FC' = 2500 PSI (MIN.) BEFORE TOPPING POUR CAN BE MADE. ROUGHEN BOTTOM POUR SURFACE APPLYING SECOND POUR

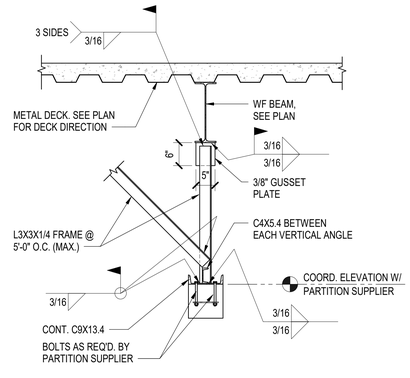
15 DETAIL AT BALCONY
3/4" = 1'-0"



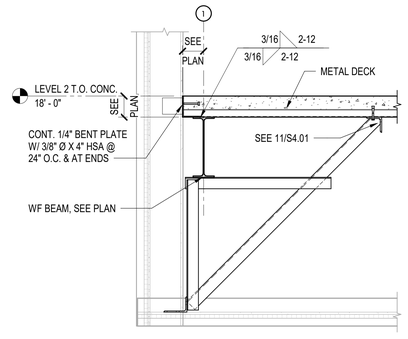
11 DETAIL AT EXTERIOR
3/4" = 1'-0"



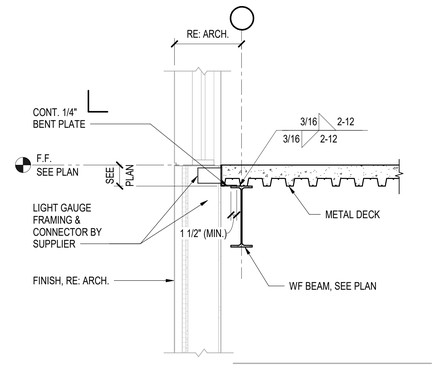
7 DETAIL AT EXTERIOR
3/4" = 1'-0"



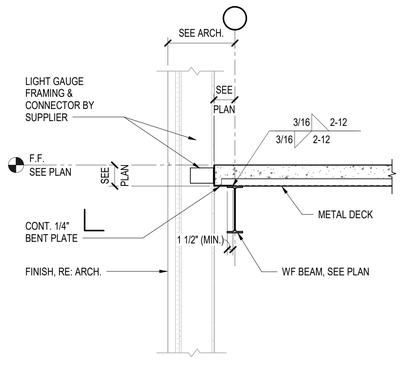
TYPICAL DETAIL OPERABLE PARTITION AT COMPOSITE FLOOR FRAMING
3/4" = 1'-0"



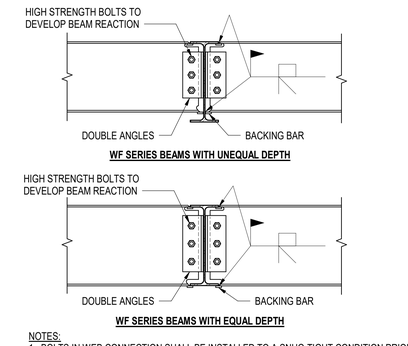
18 DETAIL AT EXTERIOR
3/4" = 1'-0"



14 DETAIL AT EXTERIOR
3/4" = 1'-0"



10 DETAIL AT EXTERIOR
3/4" = 1'-0"

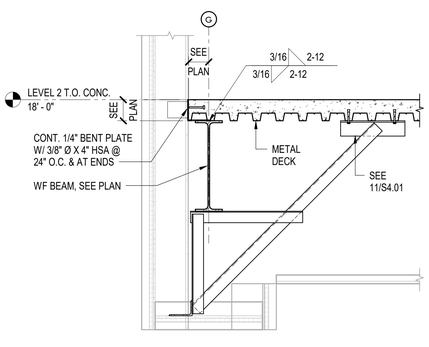


WF SERIES BEAMS WITH UNEQUAL DEPTH

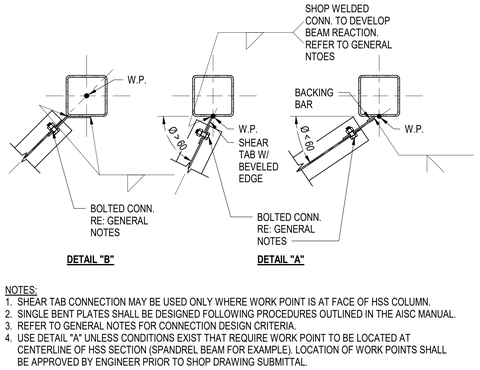
WF SERIES BEAMS WITH EQUAL DEPTH

NOTES:
1. BOLTS IN WEB CONNECTION SHALL BE INSTALLED TO A SNUG-TIGHT CONDITION PRIOR TO WELDING OF FLANGES
2. REFER TO GENERAL NOTES FOR BEAM REACTION DESIGN CRITERIA
3. PROVIDE PRE-DESIGNED CONNECTIONS AS SHOWN IN AISC MANUAL WHERE APPLICABLE
4. REFER TO GENERAL NOTES FOR BOLT TYPE

2 TYPICAL DETAIL - WF BEAM TO WF BEAM MOMENT CONNECTION
1" = 1'-0"



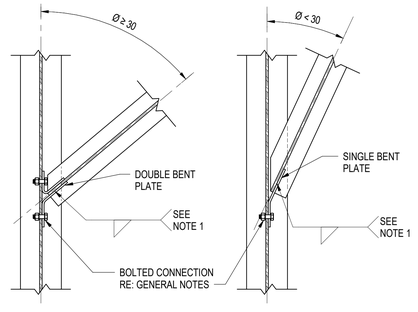
17 DETAIL AT EXTERIOR
3/4" = 1'-0"



DETAIL "B" **DETAIL "A"**

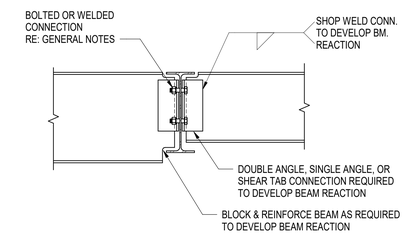
NOTES:
1. SHEAR TAB CONNECTION MAY BE USED ONLY WHERE WORK POINT IS AT FACE OF HSS COLUMN.
2. SINGLE BENT PLATES SHALL BE DESIGNED FOLLOWING PROCEDURES OUTLINED IN THE AISC MANUAL.
3. REFER TO GENERAL NOTES FOR CONNECTION DESIGN CRITERIA.
4. USE DETAIL "A" UNLESS CONDITIONS EXIST THAT REQUIRE WORK POINT TO BE LOCATED AT CENTERLINE OF HSS SECTION (SPANDREL BEAM FOR EXAMPLE). LOCATION OF WORK POINTS SHALL BE APPROVED BY ENGINEER PRIOR TO SHOP DRAWING SUBMITTAL.

9 TYPICAL DETAIL - SKEWED WF BEAM TO HSS COLUMN SHEAR CONNECTION
1" = 1'-0"



NOTE:
SHOP WELD CONNECTION TO DEVELOP BEAM REACTION. REFER TO GENERAL NOTES.

5 TYPICAL DETAIL - SKEWED WIDE FLANGE BEAM TO WIDE BEAM CONNECTION SHEAR CONNECTION
1" = 1'-0"



NOTES:
1. REFER TO GENERAL NOTES FOR BEAM REACTION DESIGN CRITERIA
2. PROVIDE PRE-DESIGNED CONNECTIONS AS SHOWN IN THE A.I.S.C MANUAL WHERE APPLICABLE
3. REFER TO GENERAL NOTES FOR BOLT TYPE

1 TYPICAL DETAIL AT WF BEAM TO WF BEAM CONNECTION
1" = 1'-0"

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25105

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HEATH H. MICHEL
License No. 39291
PROFESSIONAL ENGINEER
IN
DIGITALLY SIGNED: 03/02/2026
PINNACLE STRUCTURAL ENGINEERS
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	01/14/26	100% DD
	02/27/26	ISSUE FOR BID

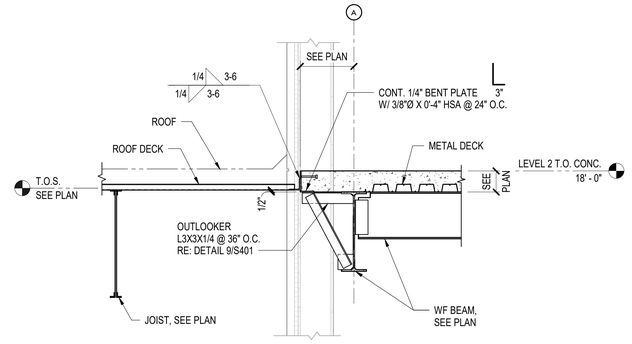
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

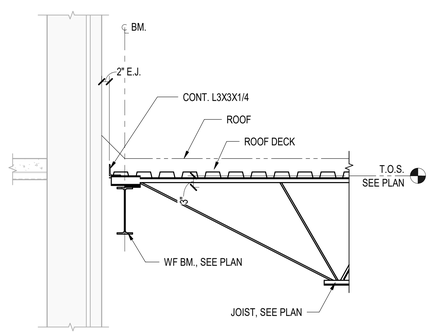
RMA PROJECT NO: 24.154

FRAMING DETAILS

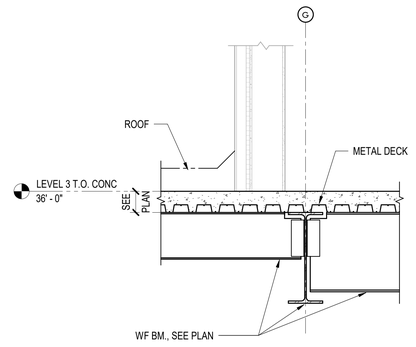
S4.02



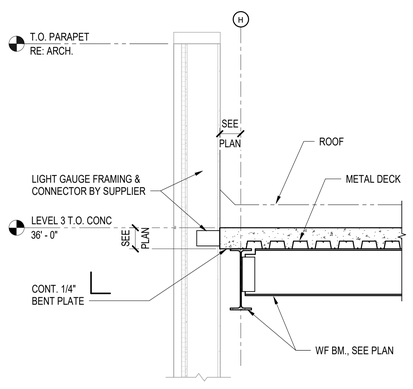
8 DETAIL AT SLAB EDGE
3/4" = 1'-0"



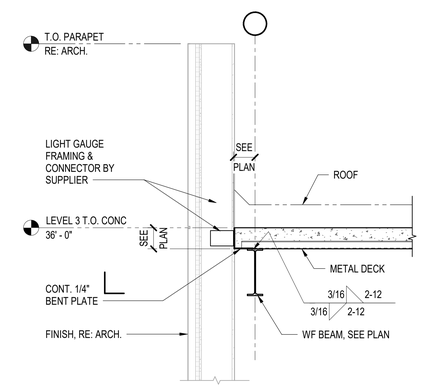
7 DETAIL AT EXTERIOR
3/4" = 1'-0"



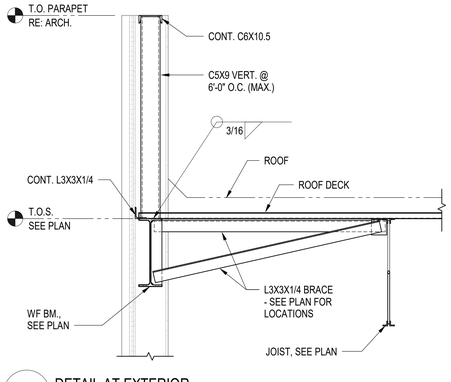
3 DETAIL AT FLOOR
3/4" = 1'-0"



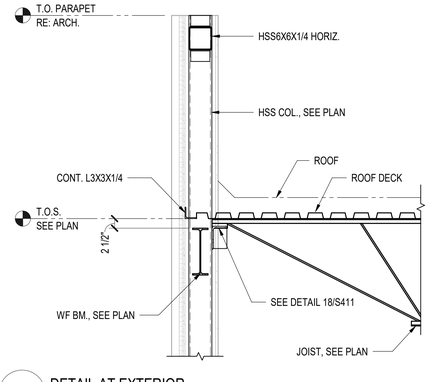
6 DETAIL AT EXTERIOR
3/4" = 1'-0"



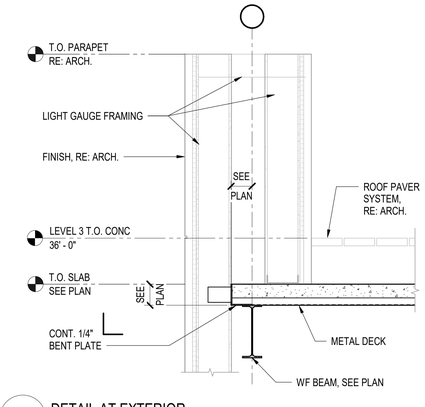
2 DETAIL AT EXTERIOR
3/4" = 1'-0"



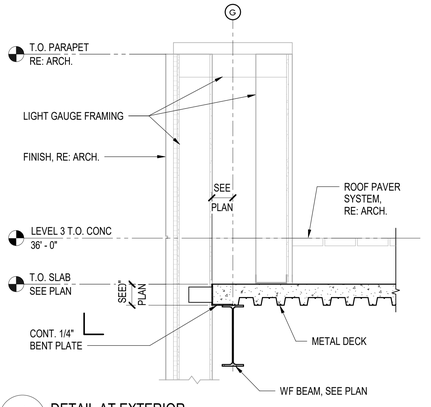
13 DETAIL AT EXTERIOR
3/4" = 1'-0"



9 DETAIL AT EXTERIOR
3/4" = 1'-0"



5 DETAIL AT EXTERIOR
3/4" = 1'-0"



1 DETAIL AT EXTERIOR
3/4" = 1'-0"

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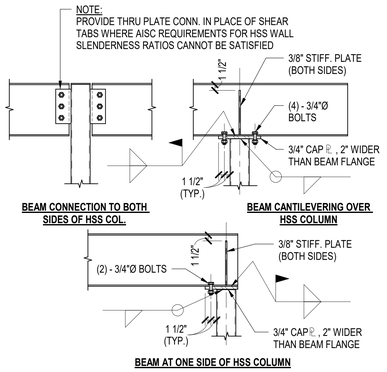
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HIGHLAND RD, ST. GEORGE, LA

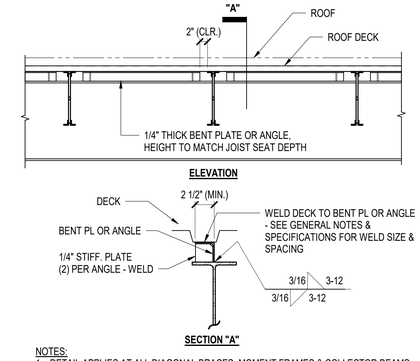
RMA PROJECT NO: 24.154

FRAMING DETAILS

S4.03

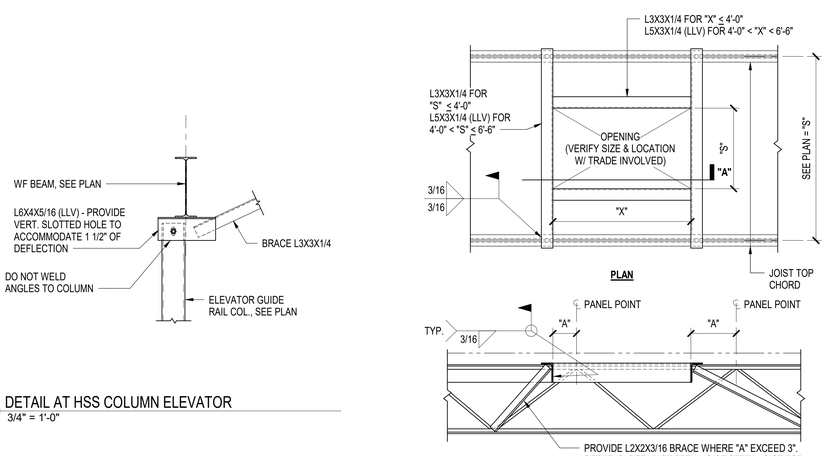


8 TYPICAL DETAIL
STEEL BEAM TO HSS COLUMN CONNECTION
3/4" = 1'-0"

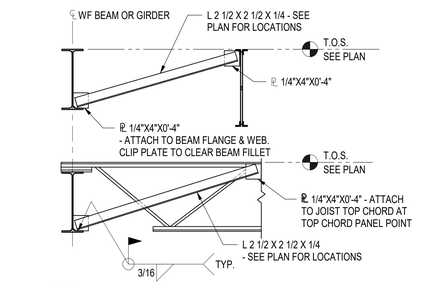


NOTES:
1. DETAIL APPLIES AT ALL DIAGONAL BRACES, MOMENT FRAMES & COLLECTOR BEAMS
2. FOR SLOPED ROOFS, MATCH DEPTH AND SLOPE OF JOIST SEAT

4 TYPICAL DETAIL - OPEN WEB STEEL JOIST
DECK DIAPHRAGM CONN. TO STEEL BEAM
3/4" = 1'-0"



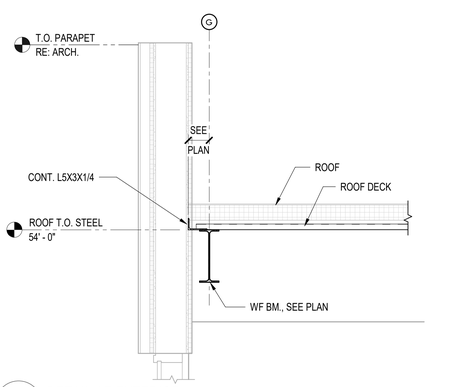
6 TYPICAL DETAIL
OPEN WEB STEEL JOIST ROOF OPENING
3/4" = 1'-0"



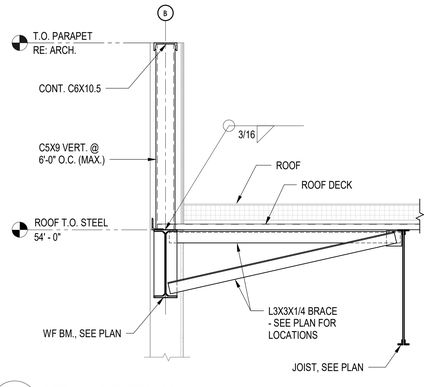
NOTES:
1. LOCATE ANGLE BRACES AT ALL EXTERIOR BEAMS & MOMENT FRAMES @ 6'-0" O.C. (MAX.).
2. FIELD WELDING OF 1/4" CONNECTION PLATES SHALL BE AT FABRICATOR'S OPTION.
3. ALL BRACING SHALL BE IN PLACE & WELDED BEFORE DECK INSTALLATION.
4. BRACE SIZE SHOWN HERE ARE TYPICAL U.N.O. ON PLAN.
5. SEE GENERAL NOTES FOR DECK CONSTRUCTION.

3 TYPICAL DETAIL - BEAM BOT. FLANGE BRACE
JOIST CONSTRUCTION
3/4" = 1'-0"

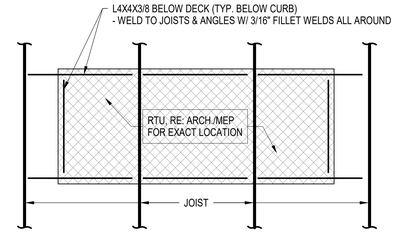
11 DETAIL AT HSS COLUMN ELEVATOR
3/4" = 1'-0"

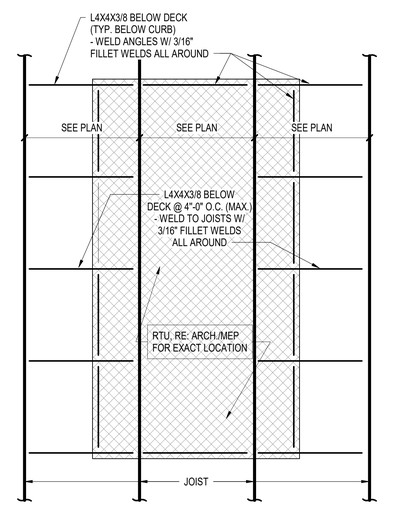


18 DETAIL AT EXTERIOR
3/4" = 1'-0"

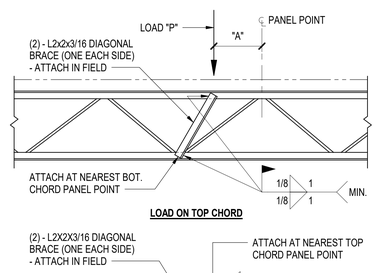


14 DETAIL AT EXTERIOR
3/4" = 1'-0"



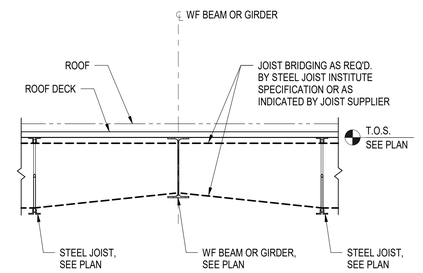


9 TYPICAL DETAIL - RTU SUPPORT AT ROOF
3/8" = 1'-0"

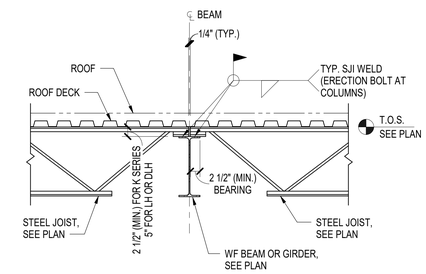


NOTES:
1. DIAGONAL BRACE IS NOT REQUIRED FOR "A" LESS THAN 3".
2. PROVIDE DIAGONAL BRACES AT LOCATION OF CONCENTRATED LOADS SUCH AS HEAVY PIPES, MECHANICAL UNITS, HEAVY LIGHTS & ANY OTHER CONCENTRATED LOADS OVER 100 LBS.
3. P = CONCENTRATED LOAD
4. CHAMFER DIAGONAL BRACE AS REQUIRED TO ACHIEVE DESIGNATED WELD LENGTH.

5 TYPICAL DETAIL - STIFFENING OF STEEL JOIST
FOR CONCENTRATED LOADS
3/4" = 1'-0"

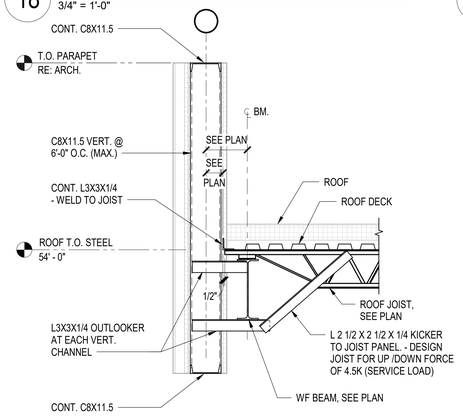


2 TYPICAL DETAIL - OPEN WEB STEEL JOIST
AT INTERIOR TIE BEAM, ROOF
3/4" = 1'-0"

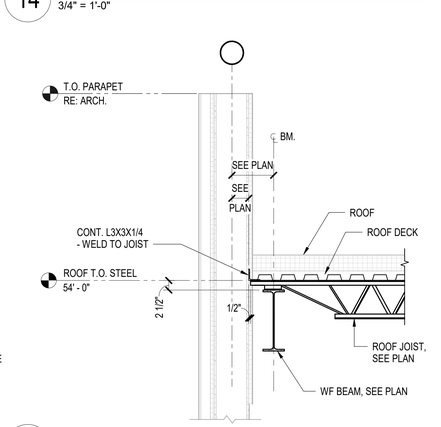


NOTE:
WHEN WIDTH OF BEAM IS LESS THAN 5 1/2", OFFSET THE LOCATION OF JOISTS ON ONE SIDE OF BEAM

1 TYPICAL DETAIL - OPEN WEB STEEL JOIST
AT INTERIOR JOIST GIRDER, ROOF
3/4" = 1'-0"



17 DETAIL AT EXTERIOR
3/4" = 1'-0"



13 DETAIL AT EXTERIOR
3/4" = 1'-0"

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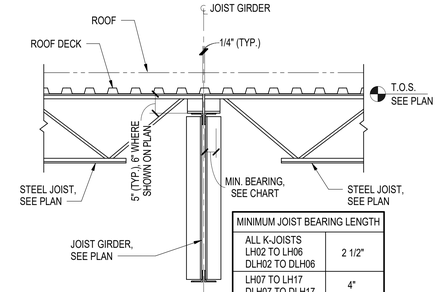
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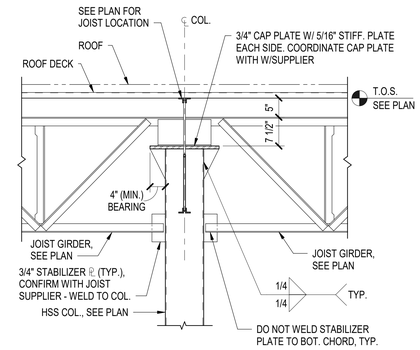
FRAMING DETAILS

S4.11

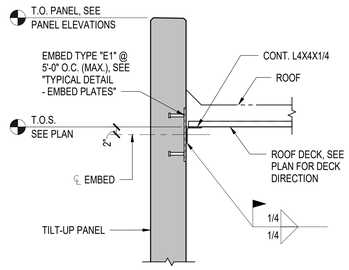


MINIMUM JOIST BEARING LENGTH	
ALL K-JOISTS LH02 TO LH06 DLH02 TO DLH06	2 1/2"
LH07 TO LH17 DLH07 TO DLH17	4"
LH18 TO LH25 DLH18 TO DLH25	6"

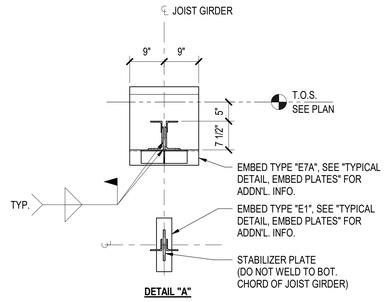
8 TYPICAL DETAIL - OPEN WEB STEEL JOIST AT INTERIOR JOIST GIRDER, ROOF
3/4" = 1'-0"



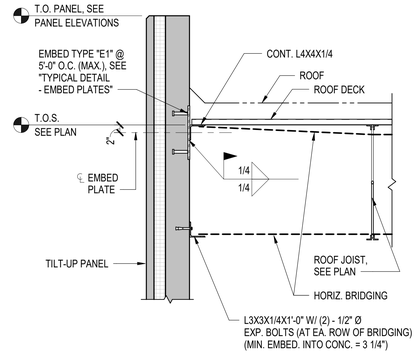
4 TYPICAL DETAIL SUPPORT OF JOIST GIRDERS OVER HSS COLUMN
3/4" = 1'-0"



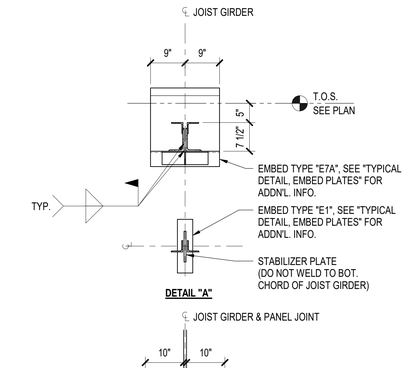
15 TYPICAL DETAIL DECK SUPPORT AT EXTERIOR TILT-UP PANEL
3/4" = 1'-0"



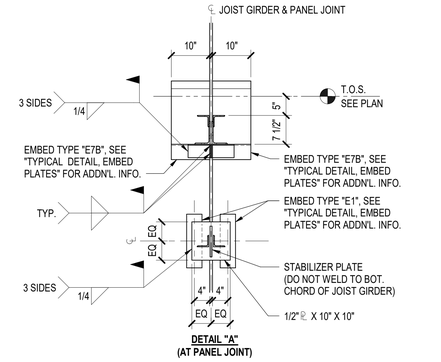
DETAIL "A"



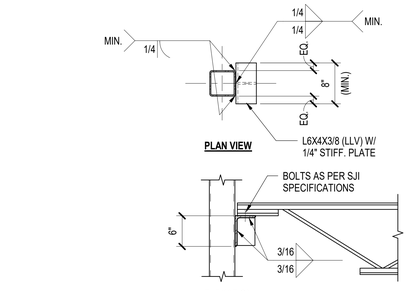
7 TYPICAL DETAIL INSULATED TILT-UP PANEL PARALLEL TO JOIST AT ROOF
3/4" = 1'-0"



DETAIL "A"



DETAIL "A"
(AT PANEL JOINT)

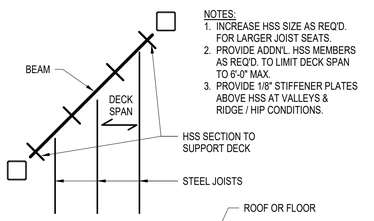


PLAN VIEW

ELEVATION

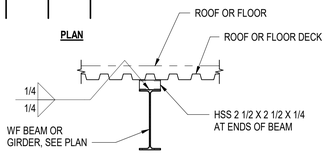
NOTES:
1. PROVIDE BOLTED CONNECTION AS PER SJI SPECIFICATIONS.
2. ALL WELDS AS PER AISC MINIMUM OR AS REQUIRED TO DEVELOP JOIST REACTION WHICHEVER IS GREATER.
3. PROVIDE L5X5X3/8 AT LHO7 JOISTS.

18 TYPICAL DETAIL JOIST SEAT AT HSS COLUMN
1" = 1'-0"



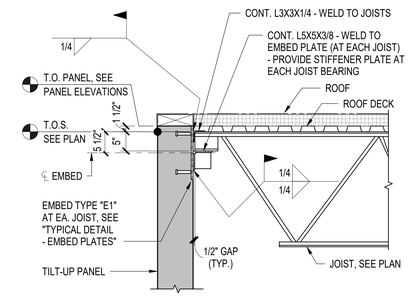
PLAN

NOTES:
1. INCREASE HSS SIZE AS REQ'D. FOR LARGER JOIST SEATS.
2. PROVIDE ADDN'L. HSS MEMBERS AS REQ'D. TO LIMIT DECK SPAN TO 6'-0" MAX.
3. PROVIDE 1/8" STIFFENER PLATES ABOVE HSS AT VALLEYS & RIDGE / HIP CONDITIONS.



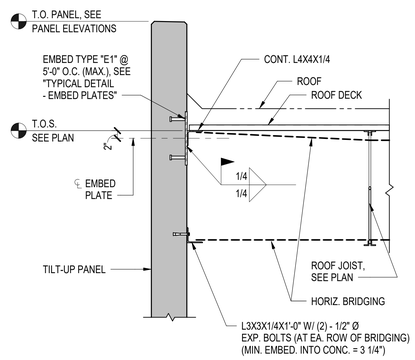
SECTION

17 TYPICAL DETAIL - DECK SUPPORT AT SKEWED BEAM
3/4" = 1'-0"

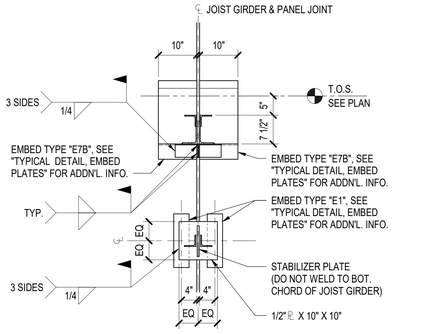


NOTE:
CONT. L5X5X3/8 SHALL ONLY BE SPLICED AT EMBED PLATES.

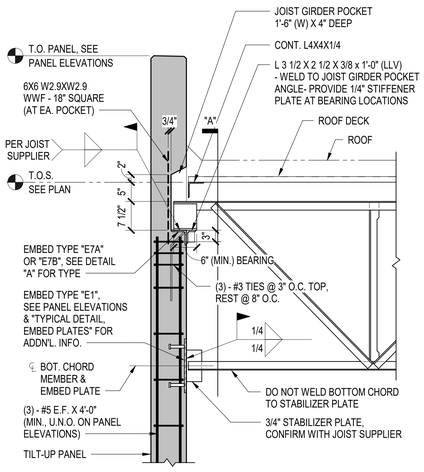
14 TYPICAL DETAIL TILT-UP PANEL PERPENDICULAR TO JOIST
3/4" = 1'-0"



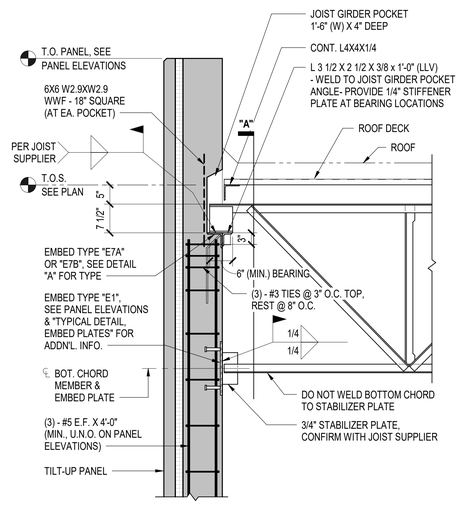
13 TYPICAL DETAIL TILT-UP PANEL PARALLEL TO JOIST AT ROOF
3/4" = 1'-0"



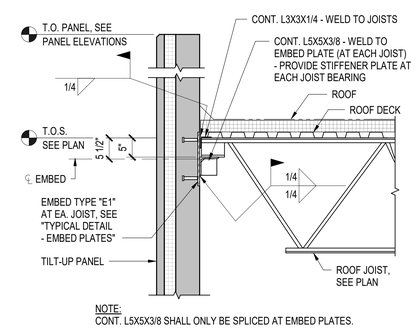
DETAIL "A"
(AT PANEL JOINT)



9 TYPICAL DETAIL JOIST GIRDER CONNECTION TO TILT-UP PANEL
3/4" = 1'-0"



1 TYPICAL DETAIL JOIST GIRDER CONNECTION TO INSULATED TILT-UP PANEL
3/4" = 1'-0"



NOTE:
CONT. L5X5X3/8 SHALL ONLY BE SPLICED AT EMBED PLATES.

5 TYPICAL DETAIL INSULATED TILT-UP PANEL PERPENDICULAR TO JOIST
3/4" = 1'-0"

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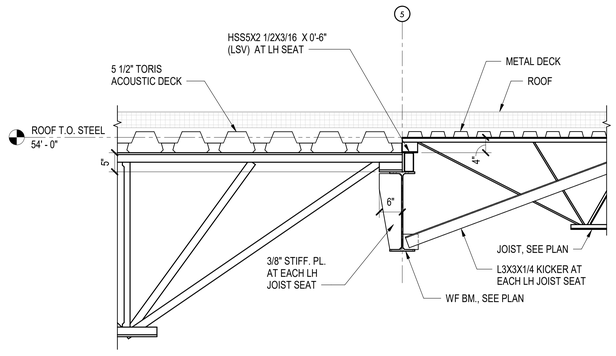
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

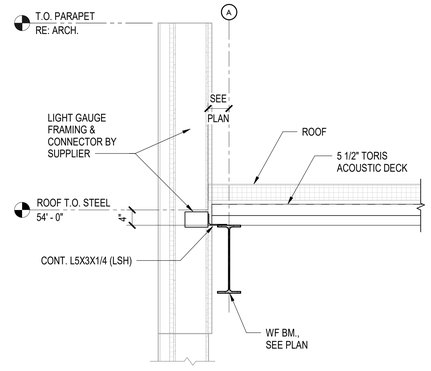
RMA PROJECT NO: 24.154

FRAMING DETAILS

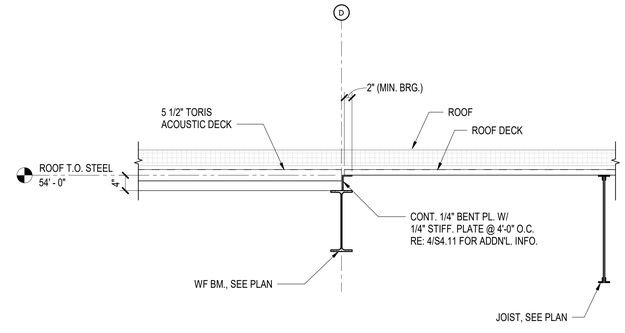
S4.12



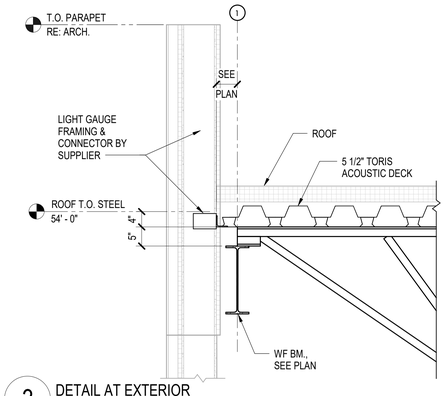
8 DETAIL AT ROOF
3/4" = 1'-0"



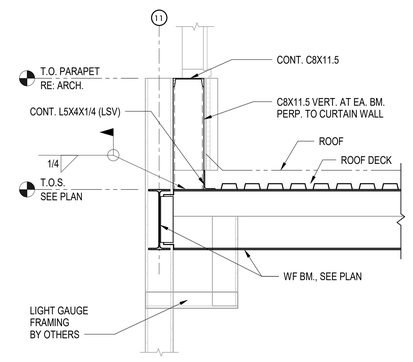
4 DETAIL AT EXTERIOR
3/4" = 1'-0"



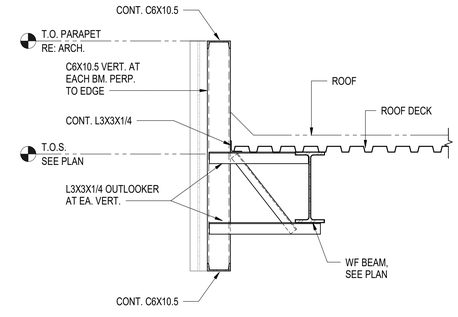
7 DETAIL AT ROOF
3/4" = 1'-0"



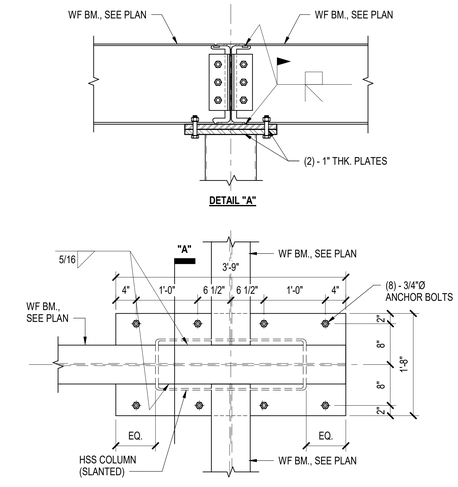
3 DETAIL AT EXTERIOR
3/4" = 1'-0"



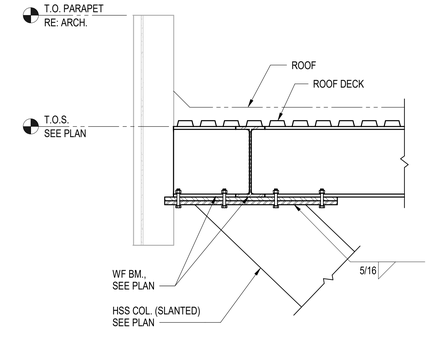
6 DETAIL AT CANOPY
3/4" = 1'-0"



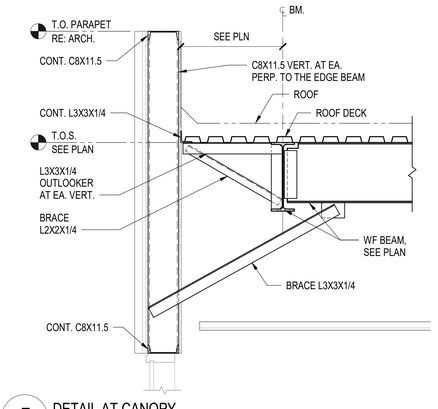
2 DETAIL AT CANOPY
3/4" = 1'-0"



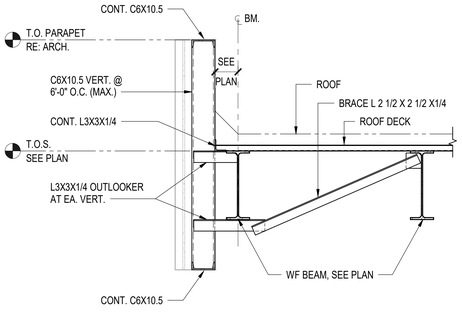
13 DETAIL AT HSS SLANTED COLUMN CONNECTION
1" = 1'-0"



9 DETAIL AT CANOPY
3/4" = 1'-0"



5 DETAIL AT CANOPY
3/4" = 1'-0"



1 DETAIL AT CANOPY
3/4" = 1'-0"

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License No. 39291
PROFESSIONAL ENGINEER
IN
DIGITALLY SIGNED: 03/02/2026

PINNACLE STRUCTURAL ENGINEERS
LOUISIANA FIRM LICENSE #4141

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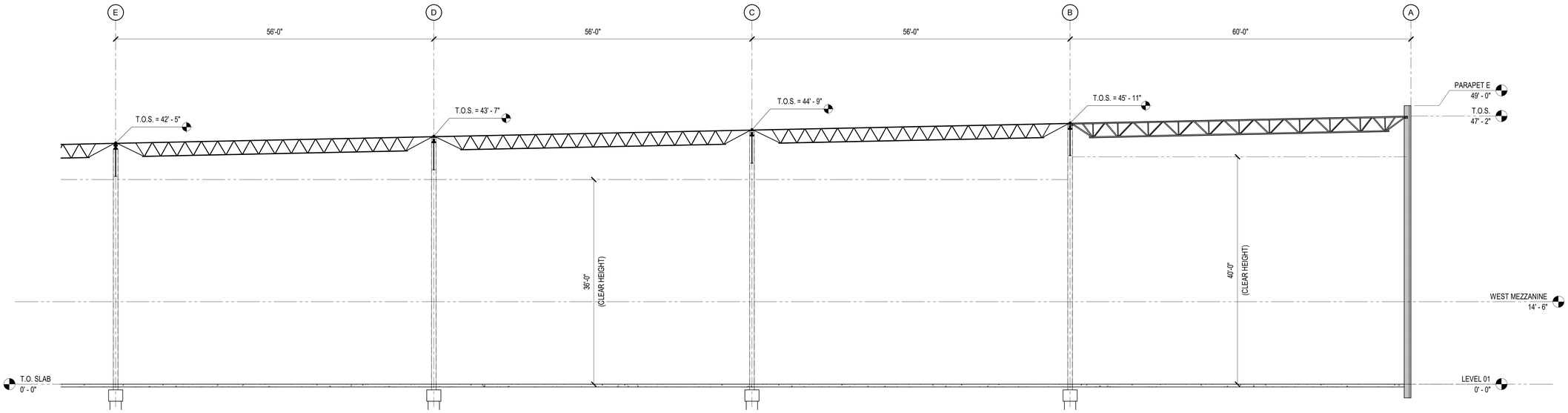
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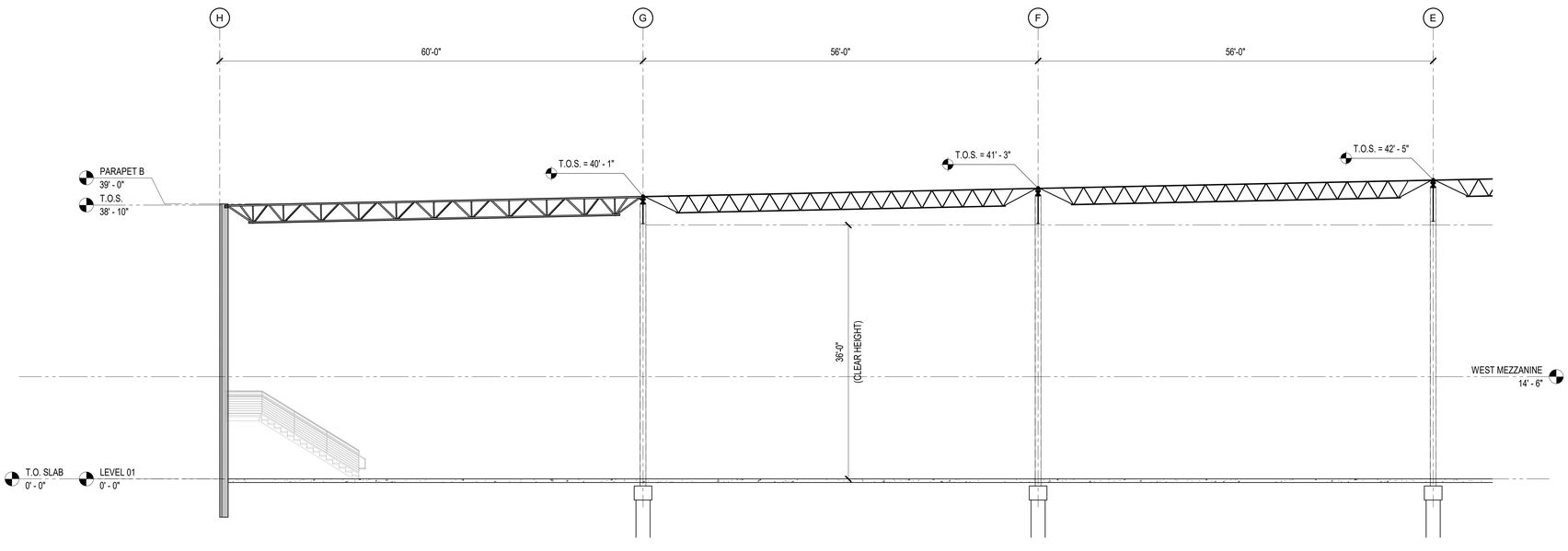
RMA PROJECT NO: 24.154

FRAMING DETAILS

S4.13



2 TYPICAL BUILDING SECTION
1/8" = 1'-0"



1 TYPICAL BUILDING SECTION
1/8" = 1'-0"

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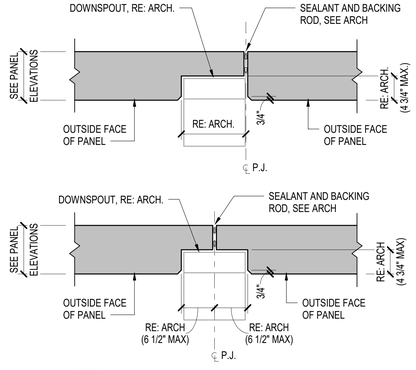
HIGHLAND RD

PROJECT ADDRESS

RMA PROJECT NO: 251172

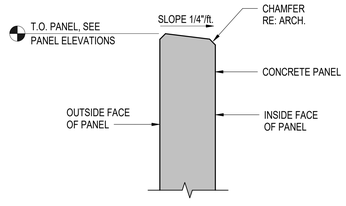
TYPICAL BUILDING SECTIONS

S4.21

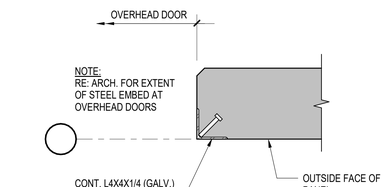


NOTES:
SEE ARCHT'L. DRAWINGS FOR LOCATIONS WHERE DETAIL APPLIES.

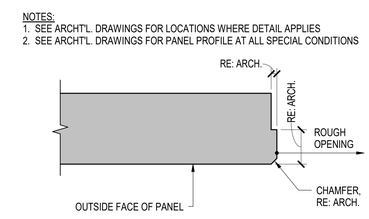
15 TYPICAL DETAIL DOWNSPOUT AT TILT-UP PANELS
1" = 1'-0"



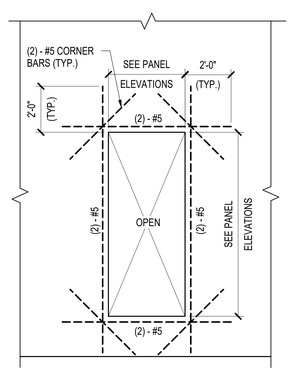
11 TYPICAL DETAIL - TOP OF CONCRETE PANEL
1 1/2" = 1'-0"



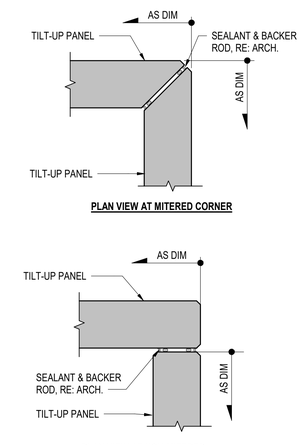
6 TYPICAL DETAIL TILT WALL PANEL AT OVERHEAD DOOR
1 1/2" = 1'-0"

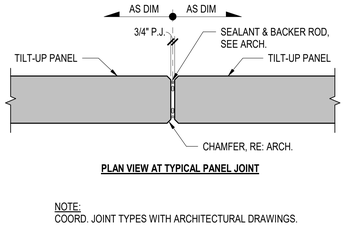


3 TYPICAL DETAIL - TILT-UP PANEL NOTCH AT WINDOW JAMB / SILL / HEADER
1 1/2" = 1'-0"



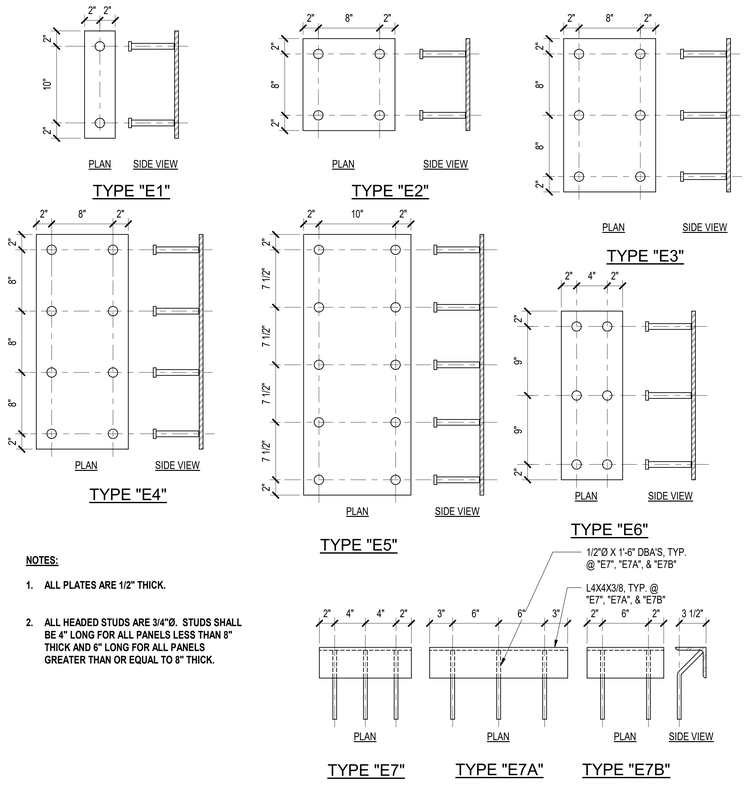
14 TYPICAL DETAIL ADDITIONAL REINFORCING AT OPENING
3/8" = 1'-0"





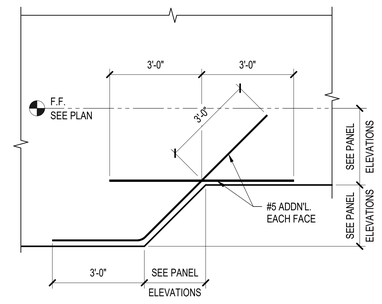
NOTE:
COORD. JOINT TYPES WITH ARCHITECTURAL DRAWINGS.

9 TYPICAL DETAIL - TILT-UP PANEL JOINT
1" = 1'-0"



NOTES:

1. ALL PLATES ARE 1/2" THICK.
2. ALL HEADED STUDS ARE 3/4"Ø. STUDS SHALL BE 4" LONG FOR ALL PANELS LESS THAN 8" THICK AND 6" LONG FOR ALL PANELS GREATER THAN OR EQUAL TO 8" THICK.



13 TYPICAL DETAIL - STEP IN TILT-UP PANEL
1/2" = 1'-0"

4 TYPICAL DETAIL - EMBED PLATES
1 1/2" = 1'-0"

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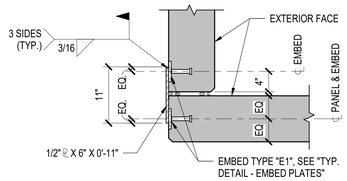
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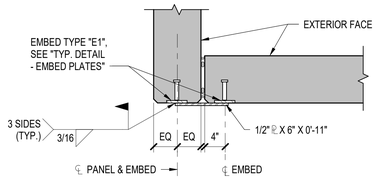
RMA PROJECT NO: 24.154

PANEL DETAILS

S5.01



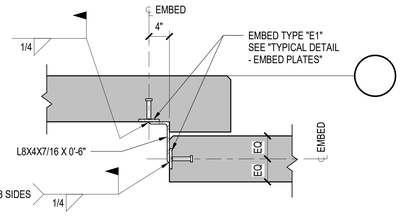
PLAN VIEW AT INTERIOR CORNER

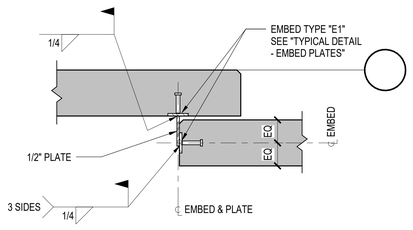


PLAN VIEW AT INTERIOR CORNER

NOTES:
1. CONNECT PANELS AT 3'-0" BELOW ROOF AND AT PANEL MID-HEIGHT, U.N.O.
2. MAKE CONNECTION PRIOR TO REMOVING STRONG-BACKS AND BRACES.

11 TYPICAL DETAIL
CONNECTION OF TILT-UP PANELS AT OFFSET CORNER
1" = 1'-0"

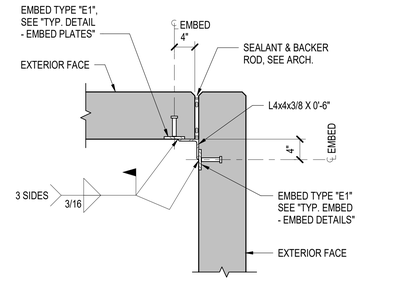




PLAN VIEW
OVERLAPPING PANELS

NOTES:
1. CONNECT PANELS AT 3'-0" BELOW ROOF AND AT PANEL MID-HEIGHT, U.N.O.
2. MAKE CONNECTION PRIOR TO REMOVING STRONG-BACKS AND BRACES.

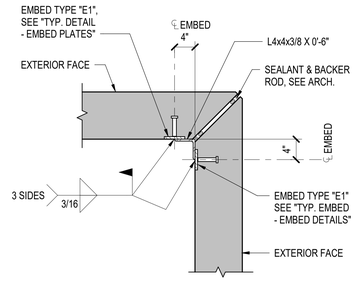
10 TYPICAL DETAIL
CONNECTION AT OVERALAPPING PANELS
1" = 1'-0"



NOTES:
1. CONNECT PANELS AT 3'-0" BELOW ROOF AND AT PANEL MID-HEIGHT, U.N.O.
2. MAKE CONNECTION PRIOR TO REMOVING STRONG-BACKS AND BRACES.

PLAN VIEW AT CORNER

8 TYPICAL DETAIL
CONNECTION OF TILT-UP PANELS AT CORNER
1" = 1'-0"

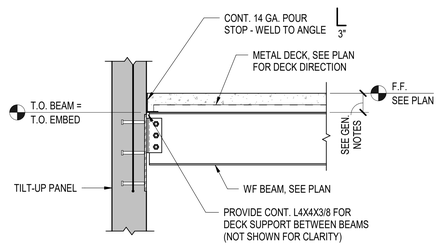


NOTES:
1. CONNECT PANELS AT 3'-0" BELOW ROOF AND AT PANEL MID-HEIGHT, U.N.O.
2. MAKE CONNECTION PRIOR TO REMOVING STRONG-BACKS AND BRACES.

PLAN VIEW AT MITERED CORNER

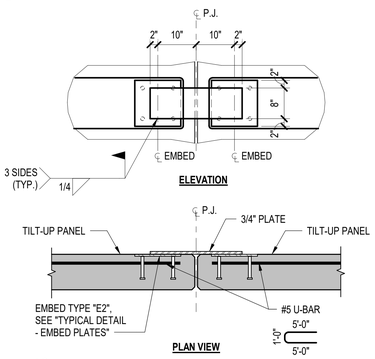
4 TYPICAL DETAIL
CONNECTION OF TILT-UP PANELS AT CORNER
1" = 1'-0"

14 TYPICAL DETAIL
WF BEAM TO TILT-UP PANEL
3/4" = 1'-0"



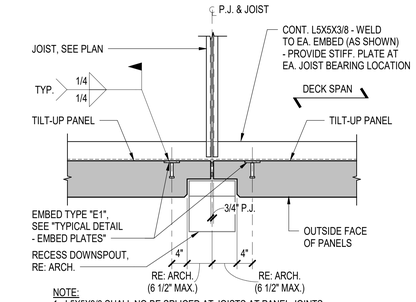
NOTE:
SEE TYPICAL DETAIL, "WF BEAM TO TILT-UP PANEL" FOR ADDN'L INFORMATION.

13 TYPICAL DETAIL
WF BEAM TO TILT-UP PANEL AT FLOOR
3/4" = 1'-0"



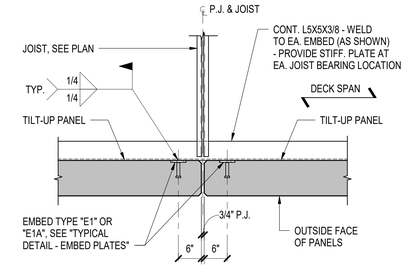
NOTE:
PROVIDE PANEL TO PANEL CONNECTION AS INDICATED IN THE PANEL ELEVATIONS

9 PANEL TO PANEL CONNECTION
3/4" = 1'-0"



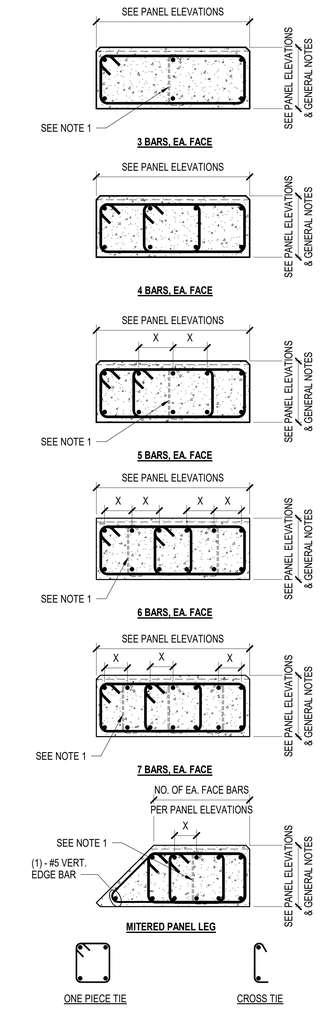
NOTE:
1. L5X5X3/8 SHALL NO BE SPLICED AT JOISTS AT PANEL JOINTS.
2. EDGE ANGLE NOT SHOWN FOR CLARITY.
3. THIS DETAIL TO BE USED FOR ALL JOIST WITHIN 6" OF A PANEL JOINT.
4. BEARING ANGLE SHALL BE DISCONTINUOUS AT ROOF EXPANSION JOINT LOCATIONS ONLY, U.N.O..

6 TYPICAL DETAIL - STEEL JOIST TO
TILT UP PANEL AT JOINT AT ROOF
3/4" = 1'-0"



NOTE:
1. L5X5X3/8 SHALL NO BE SPLICED AT JOISTS AT PANEL JOINTS.
2. EDGE ANGLE NOT SHOWN FOR CLARITY.
3. THIS DETAIL TO BE USED FOR ALL JOIST WITHIN 6" OF A PANEL JOINT.
4. BEARING ANGLE SHALL BE DISCONTINUOUS AT ROOF EXPANSION JOINT LOCATIONS ONLY, U.N.O..

5 TYPICAL DETAIL - STEEL JOIST TO
TILT UP PANEL AT JOINT AT ROOF
3/4" = 1'-0"



NOTES:
1. CROSS TIES MUST BE INCLUDED WHEN X > 6".
2. ALL REINFORCING SHOWN ON THESE PLAN DETAILS IS IN ADDITION TO REINFORCING REQUIREMENTS SHOWN IN THE GENERAL NOTES U.N.O.
3. FOR MINIMUM COVER TO TIES, SEE GENERAL NOTES.
4. ALTERNATE HOOKS WHEN PLACING SUCCESSIVE SETS OF CROSS TIES.
5. ALL TIES ARE #3 @ 36" O.C. U.N.O. ON PANEL ELEVATIONS.

1 TYPICAL DETAIL
ADDITIONAL REINFORCING AT PANEL LEGS
1" = 1'-0"

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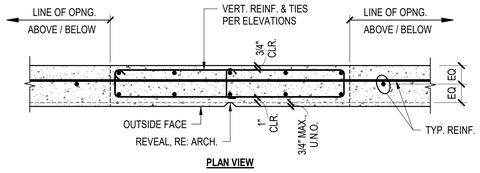
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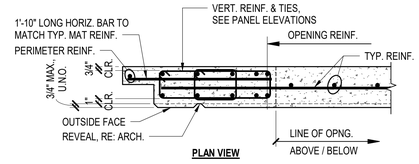
PANEL DETAILS

S5.02



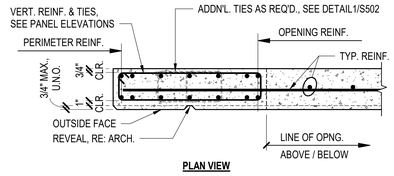
PLAN VIEW

12 AT PANEL BETWEEN OPENINGS ABOVE / BELOW (BARS EACH FACE)
1" = 1'-0"



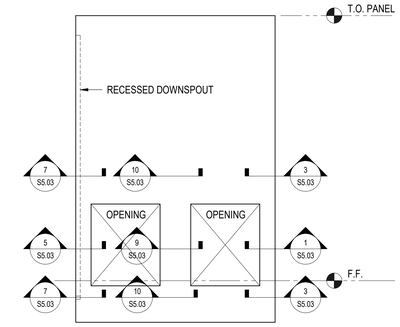
PLAN VIEW

8 AT PANEL LEG ABOVE / BELOW OPENING (BARS AT EACH FACE) W/ RECESSED DOWNSPOUT
1" = 1'-0"

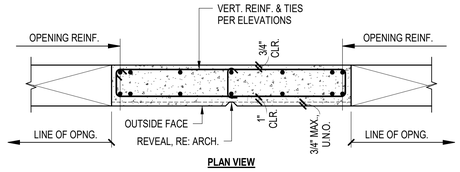


PLAN VIEW

4 AT PANEL LEG ABOVE / BELOW OPENING (BARS AT EACH FACE)
1" = 1'-0"

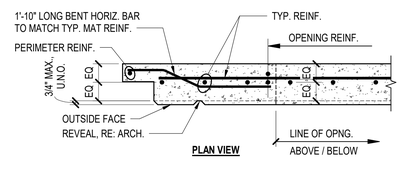


15 TYPICAL PANEL WITH BASE REINF. PANEL THICKNESS 10" OR LESS
1/8" = 1'-0"



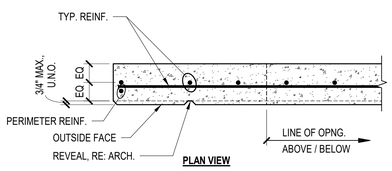
PLAN VIEW

11 AT PANEL BETWEEN OPENINGS (BARS EACH FACE)
1" = 1'-0"



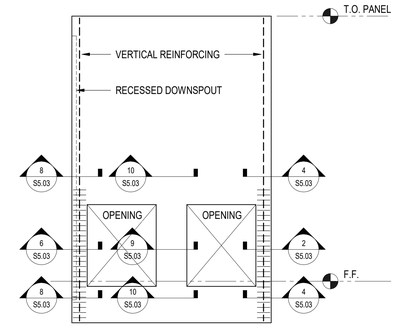
PLAN VIEW

7 AT PANEL LEG ABOVE / BELOW OPENING WITH RECESSED DOWNSPOUT
1" = 1'-0"

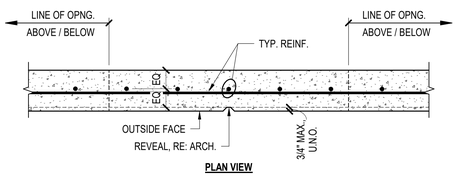


PLAN VIEW

3 AT PANEL LEG ABOVE / BELOW OPENING
1" = 1'-0"

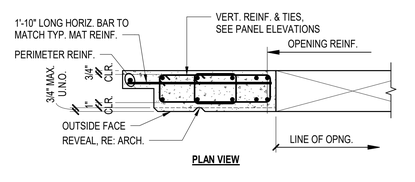


14 TYPICAL PANEL WITH PANEL LEG REINF. AT EACH FACE PANEL THICKNESS 10" OR LESS
1/8" = 1'-0"



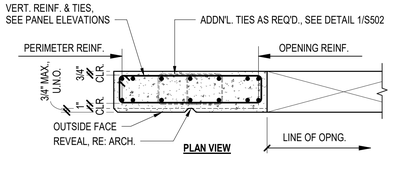
PLAN VIEW

10 AT PANEL BETWEEN OPENINGS ABOVE / BELOW
1" = 1'-0"



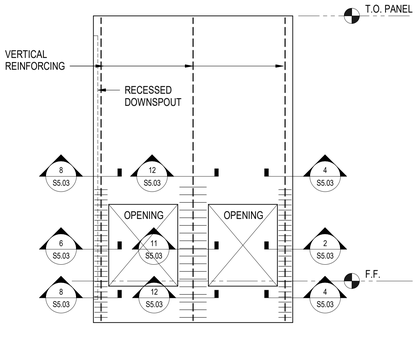
PLAN VIEW

6 AT PANEL LEG AT OPENING (BARS AT EACH FACE) W/ RECESSED DOWNSPOUT
1" = 1'-0"

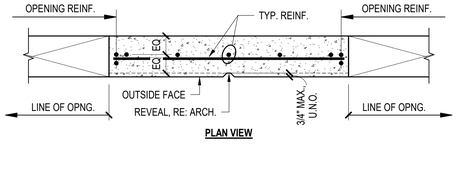


PLAN VIEW

2 PANEL LEG AT OPENING (BARS EACH FACE)
1" = 1'-0"

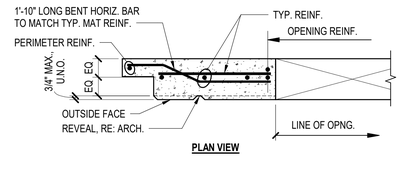


13 TYPICAL PANEL WITH PANEL LEG REINF. AT EACH FACE PANEL THICKNESS 10" OR LESS
1/8" = 1'-0"



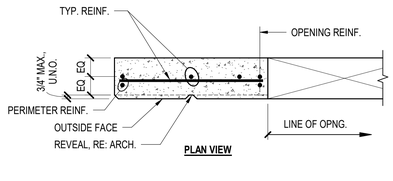
PLAN VIEW

9 AT PANEL BETWEEN OPENINGS
1" = 1'-0"



PLAN VIEW

5 AT PANEL LEG AT OPENING WITH RECESSED DOWNSPOUT
1" = 1'-0"



PLAN VIEW

1 AT PANEL LEG AT OPENING
1" = 1'-0"

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IN
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	02/27/26	ISSUE FOR BID

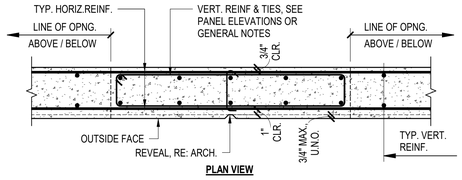
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

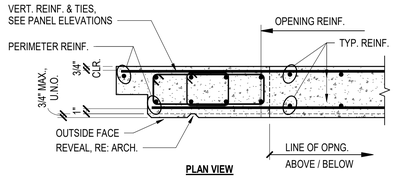
RMA PROJECT NO: 24.154

PANEL DETAILS

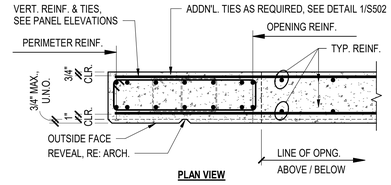
S5.03



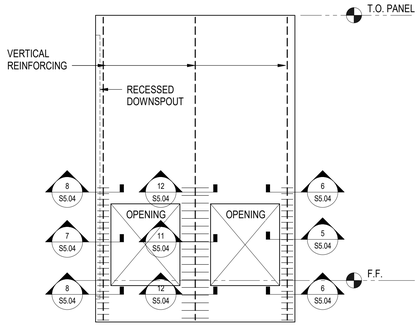
12 AT PANEL BETWEEN OPENINGS ABOVE / BELOW
1" = 1'-0"



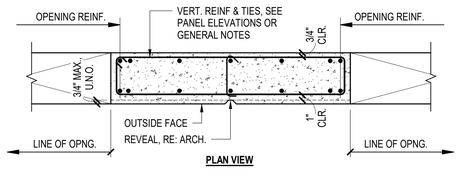
8 AT PANEL LEG ABOVE / BELOW OPENING (BARS AT EACH FACE) W/ RECESSED DOWNSPOUT
1" = 1'-0"



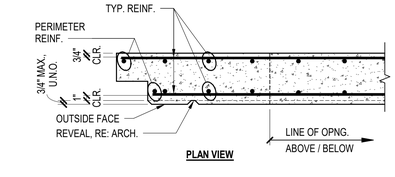
4 AT PANEL LEG ABOVE / BELOW OPENING (BARS AT EACH FACE)
1" = 1'-0"



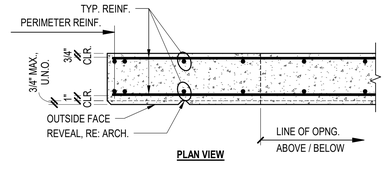
15 TYPICAL PANEL W/ PANEL LEG REINF. AT EACH FACE PANEL THICKNESS MORE THAN 10"
1/8" = 1'-0"



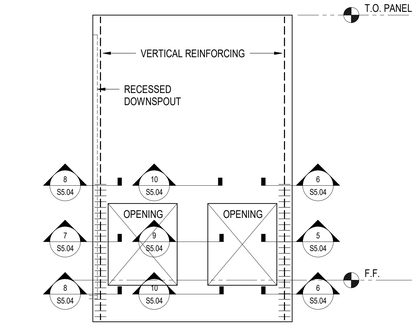
11 AT PANEL BETWEEN OPENINGS
1" = 1'-0"



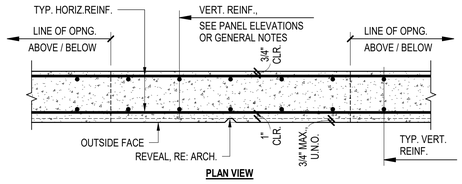
7 AT PANEL LEG ABOVE / BELOW OPENING W/ RECESSEDD DOWNSPOUT
1" = 1'-0"



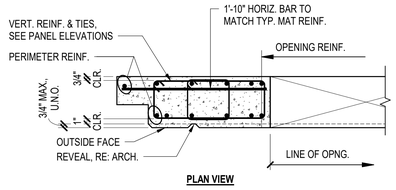
3 AT PANEL LEG ABOVE / BELOW OPENING
1" = 1'-0"



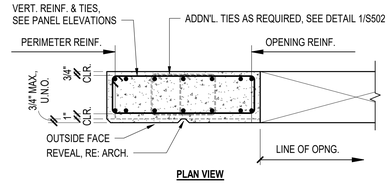
14 TYPICAL PANEL W/ PANEL LEG REINF. AT EACH FACE PANEL THICKNESS MORE THAN 10"
1/8" = 1'-0"



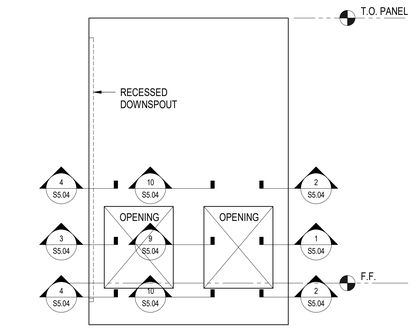
10 AT PANEL BETWEEN OPENINGS ABOVE / BELOW
1" = 1'-0"



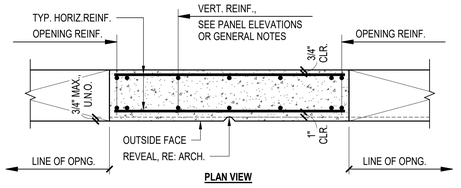
6 AT PANEL LEG ABOVE / BELOW OPENING (BARS AT EACH FACE) W/ RECESSED DOWNSPOUT
1" = 1'-0"



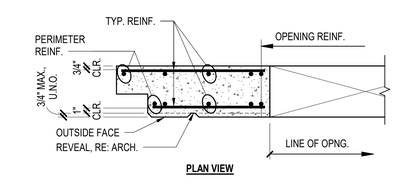
2 AT PANEL LEG AT OPENING (BARS AT EACH FACE)
1" = 1'-0"



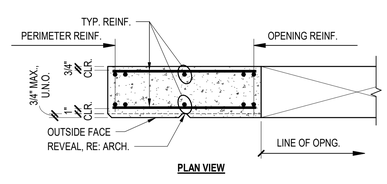
13 TYPICAL PANEL W/ PANEL LEG REINF. AT EACH FACE PANEL THICKNESS MORE THAN 10"
1/8" = 1'-0"



9 AT PANEL BETWEEN OPENINGS
1" = 1'-0"



5 AT PANEL LEG AT OPENING W/ RECESSED DOWNSPOUT
1" = 1'-0"



1 AT PANEL LEG AT OPENING
1" = 1'-0"

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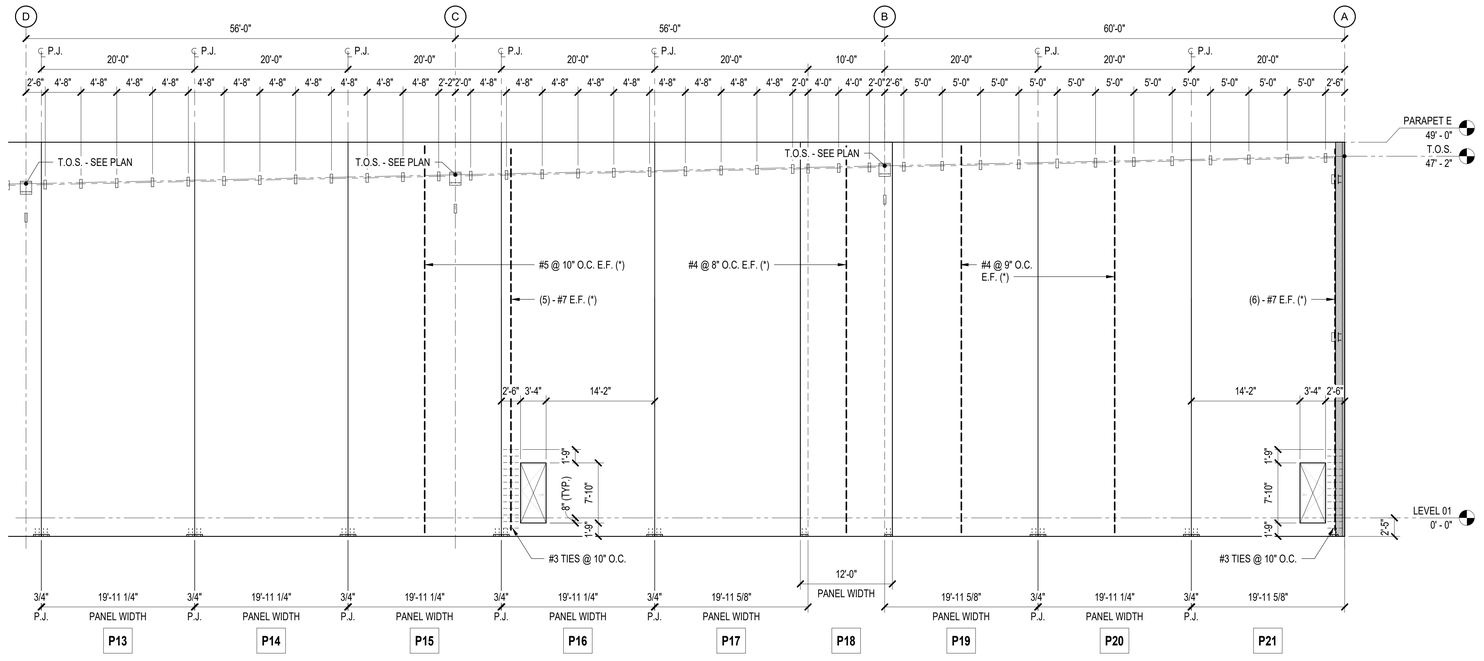
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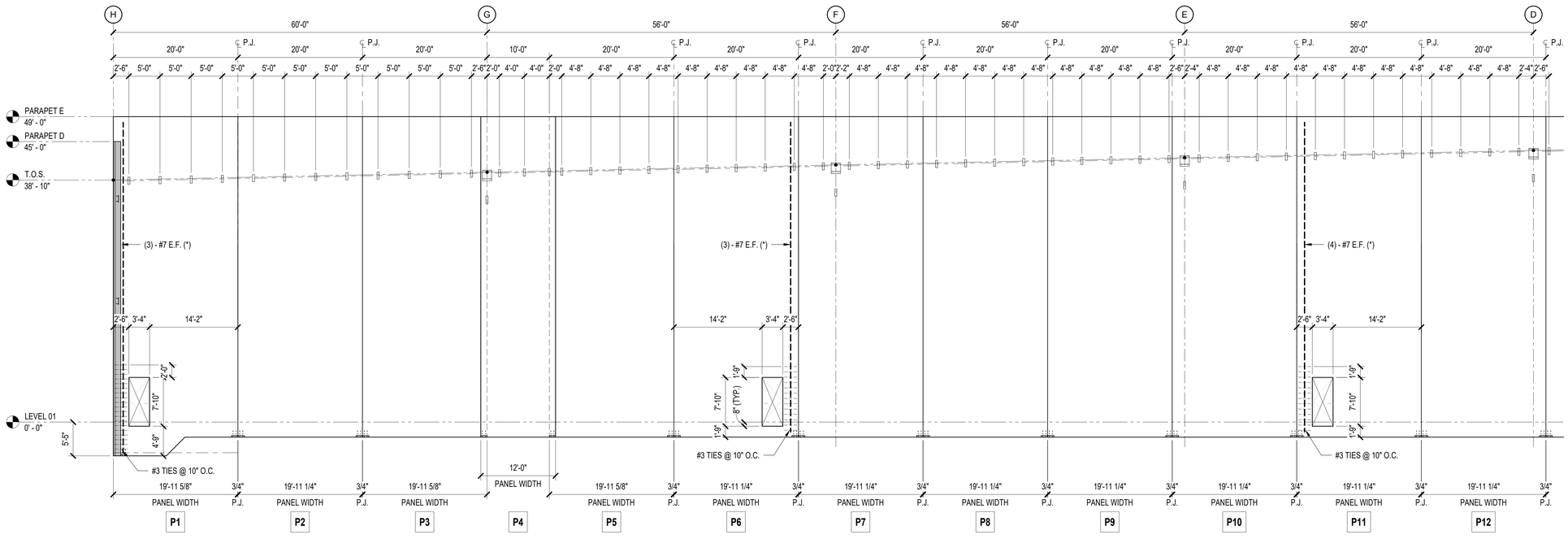
RMA PROJECT NO: 24.154

PANEL DETAILS

S5.04



4 PANEL ELVATION
1/8" = 1'-0"



3 PANEL ELVATION
1/8" = 1'-0"

GENERAL NOTES

1. ALL EMBED PLATES ARE TYPE "E1", U.N.O.
2. SEE DETAIL 4/S501 FOR EMBED PLATE SIZE & DIMENSIONS.
3. VERIFY ALL OPENING W/ ARCHT'L. DRAWINGS.
4. SEE TILT-UP WALL PANEL SECTION OF THE GENERAL NOTES FOR TYPICAL PANEL REINFORCING.
5. INSTALL EMBEDS SHOWN IN DETAILS & ON PANEL ELEVATIONS.
6. PROVIDE GALV. STEEL EMBED AT O.H. DOORS, SEE 6/S501 FOR INFORMATION.
7. SEE SHEET S503 AND S504 FOR REINFORCEMENT PLACEMENT.
8. TYPICAL NON-COMPOSITE PANEL THICKNESS IS 14 1/4", U.N.O.
9. LOCATIONS NOTED THUS: (*) INDICATES THE VERTICAL REINFORCING NOTED SHALL REPLACE THE TYPICAL VERTICAL REINFORCING REQUIRED PER THE GENERAL NOTES.
10. TYPICAL REINFORCING SHALL RUN THROUGH FUTURE KNOCK-OUT OPENINGS.

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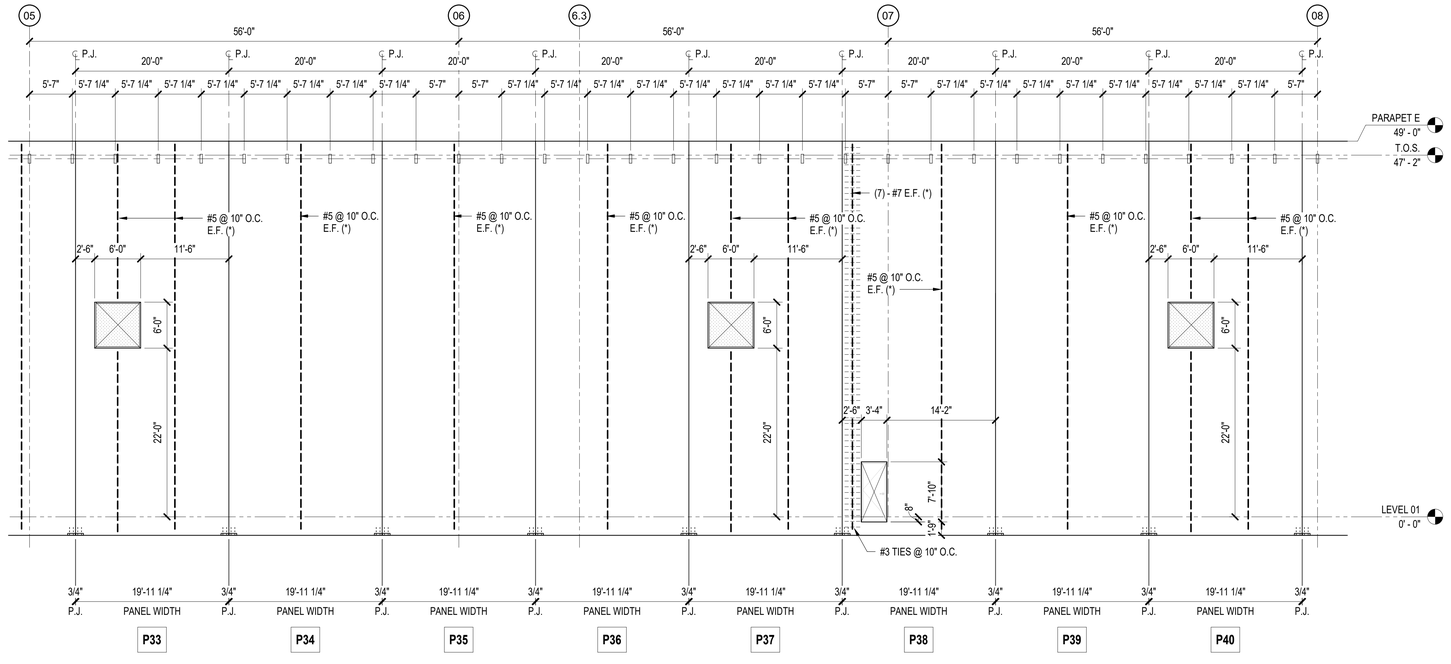
HIGHLAND RD

PROJECT ADDRESS

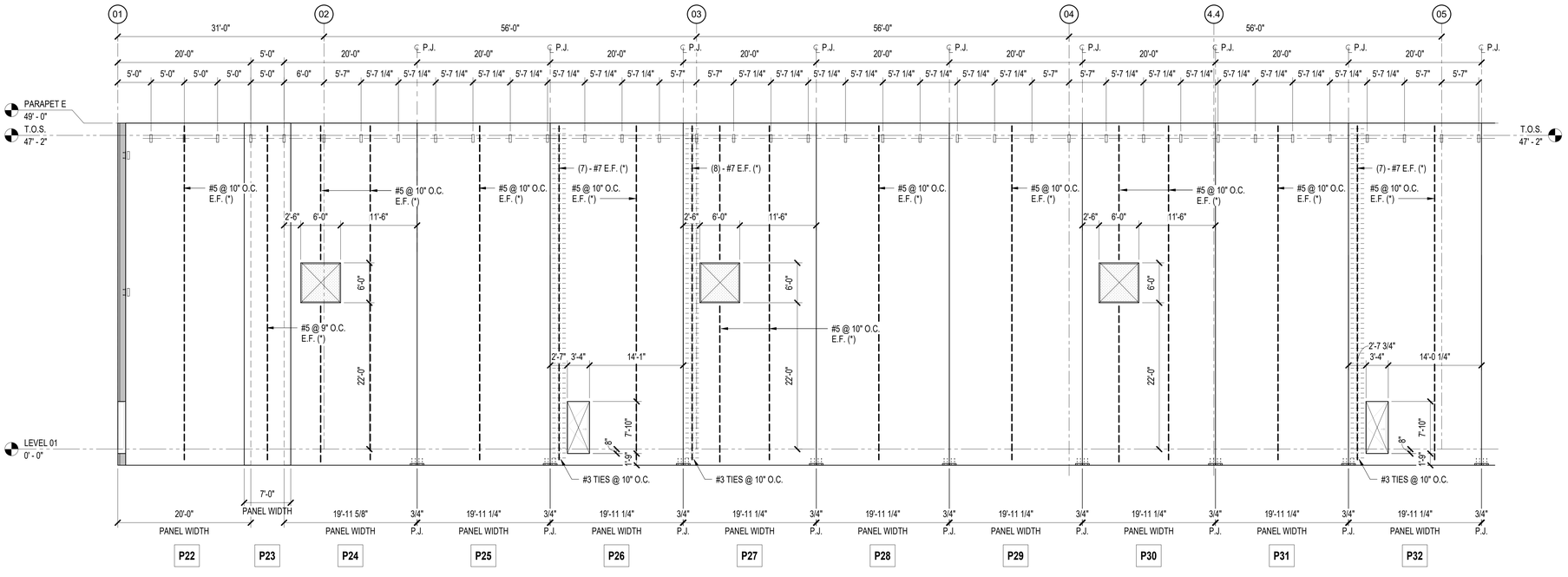
RMA PROJECT NO: 251172

PANEL ELEVATIONS

S5.11



4 PANEL ELVATION
1/8" = 1'-0"



3 PANEL ELVATION
1/8" = 1'-0"

GENERAL NOTES

1. ALL EMBED PLATES ARE TYPE "E1", U.N.O.
2. SEE DETAIL 4/S501 FOR EMBED PLATE SIZE & DIMENSIONS.
3. VERIFY ALL OPENING W/ ARCHT'L. DRAWINGS.
4. SEE TILT-UP WALL PANEL SECTION OF THE GENERAL NOTES FOR TYPICAL PANEL REINFORCING.
5. INSTALL EMBEDS SHOWN IN DETAILS & ON PANEL ELEVATIONS.
6. PROVIDE GALV. STEEL EMBED AT O.H. DOORS, SEE 6/S501 FOR INFORMATION.
7. SEE SHEET S503 AND S504 FOR REINFORCEMENT PLACEMENT.
8. TYPICAL NON-COMPOSITE PANEL THICKNESS IS 14 1/4", U.N.O.
9. LOCATIONS NOTED THUS: (*) INDICATES THE VERTICAL REINFORCING NOTED SHALL REPLACE THE TYPICAL VERTICAL REINFORCING REQUIRED PER THE GENERAL NOTES.
10. TYPICAL REINFORCING SHALL RUN THROUGH FUTURE KNOCK-OUT OPENINGS.

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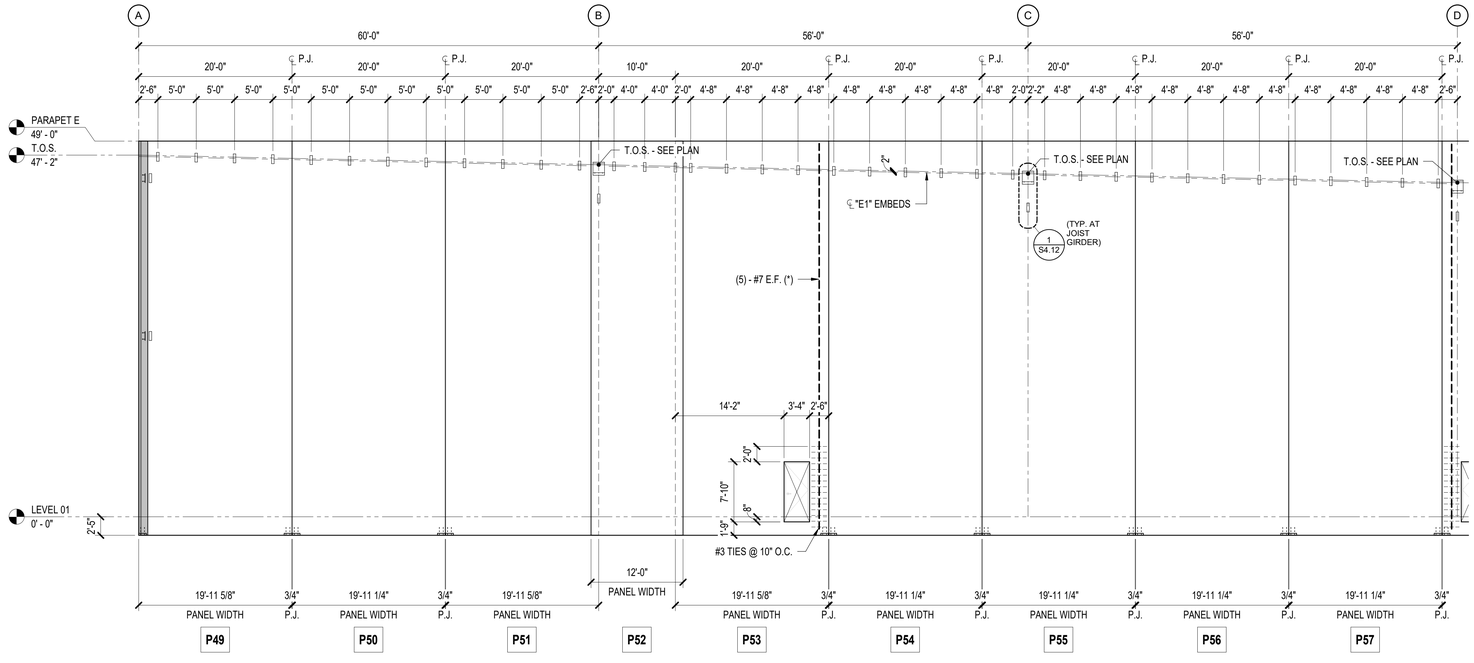
HIGHLAND RD

PROJECT ADDRESS

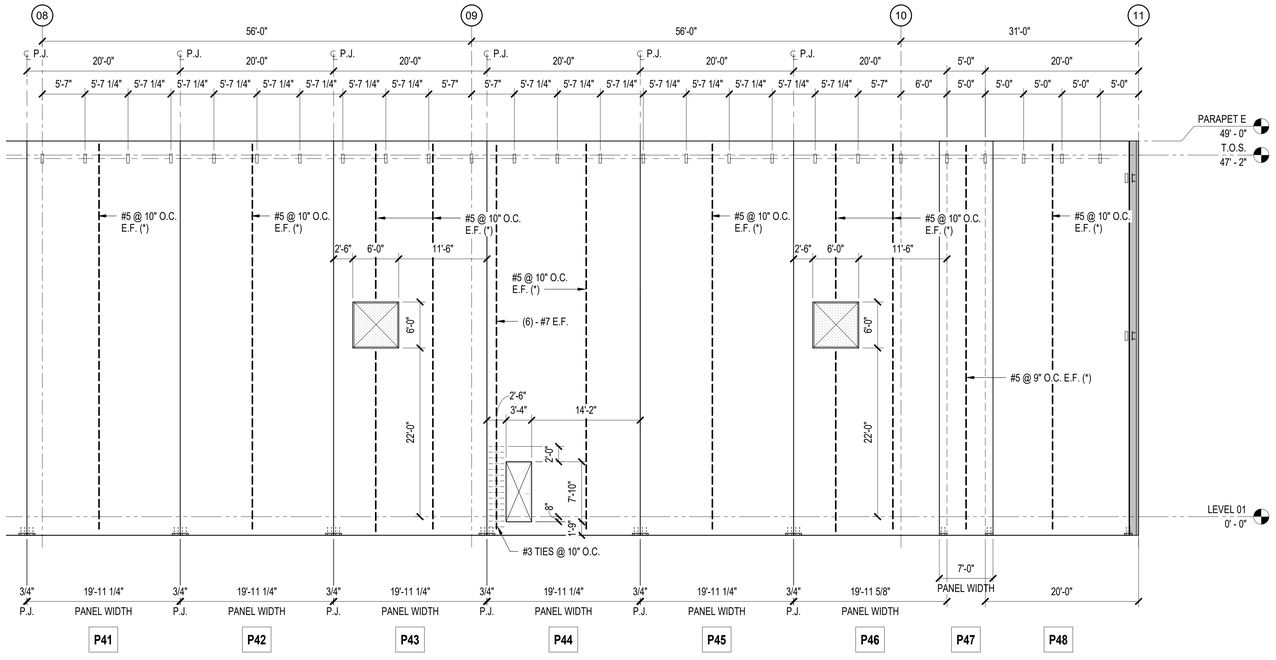
RMA PROJECT NO: 251172

PANEL ELEVATIONS

S5.12



4 PANEL ELVATION
1/8" = 1'-0"



3 PANEL ELVATION
1/8" = 1'-0"

GENERAL NOTES

1. ALL EMBED PLATES ARE TYPE "E1", U.N.O.
2. SEE DETAIL 4/S501 FOR EMBED PLATE SIZE & DIMENSIONS.
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4. SEE TILT-UP WALL PANEL SECTION OF THE GENERAL NOTES FOR TYPICAL PANEL REINFORCING.
5. INSTALL EMBEDS SHOWN IN DETAILS & ON PANEL ELEVATIONS.
6. PROVIDE GALV. STEEL EMBED AT O.H. DOORS, SEE 6/S501 FOR INFORMATION.
7. SEE SHEET S503 AND S504 FOR REINFORCEMENT PLACEMENT.
8. TYPICAL NON-COMPOSITE PANEL THICKNESS IS 14 1/4", U.N.O.
9. LOCATIONS NOTED THUS: (*) INDICATES THE VERTICAL REINFORCING NOTED SHALL REPLACE THE TYPICAL VERTICAL REINFORCING REQUIRED PER THE GENERAL NOTES.
10. TYPICAL REINFORCING SHALL RUN THROUGH FUTURE KNOCK-OUT OPENINGS.

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KEY PLAN:

SEAL:

STATE OF LOUISIANA
HEATH H. MICHEL
License No. 39291
PROFESSIONAL ENGINEER
IN
DIGITALLY SIGNED: 03/02/2026
PINNACLE STRUCTURAL ENGINEERS
LOUISIANA FIRM LICENSE #4141

REVISION SCHEDULE

NO:	DATE:	DESCRIPTION:
	02/27/26	ISSUE FOR BID

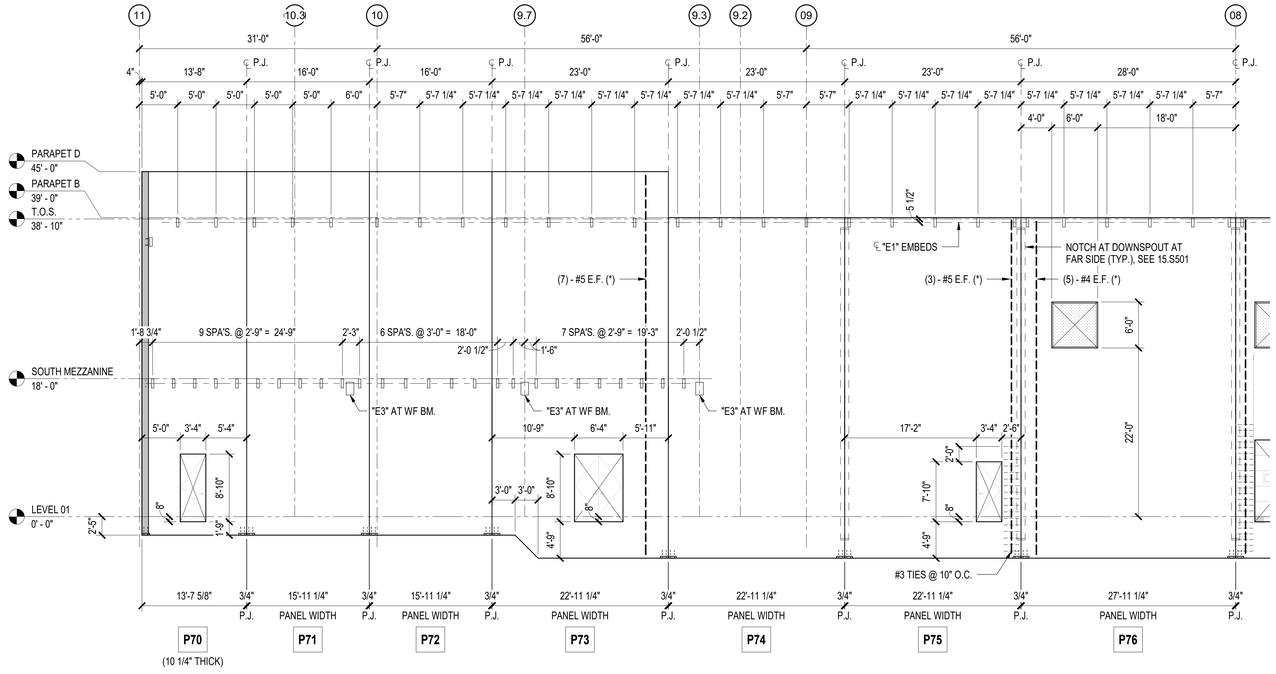
HIGHLAND RD

PROJECT ADDRESS

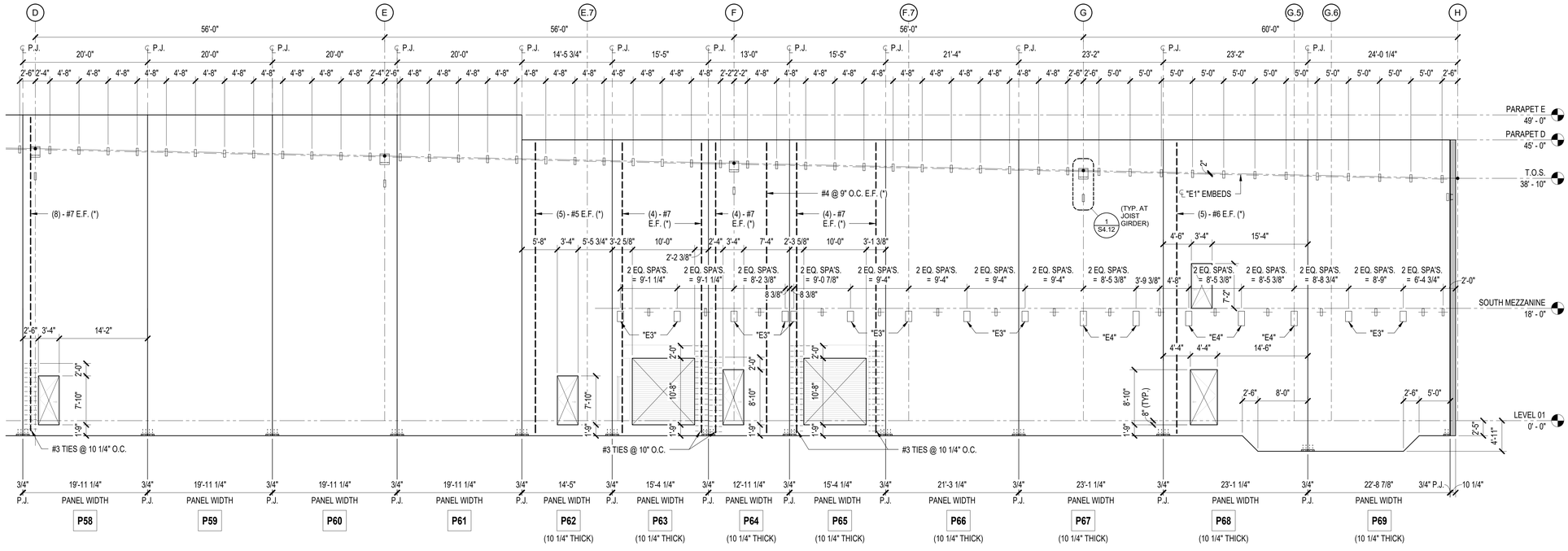
RMA PROJECT NO: 251172

PANEL ELEVATIONS

S5.13



4 PANEL ELVATION
1/8" = 1'-0"



3 PANEL ELVATION
1/8" = 1'-0"

GENERAL NOTES

1. ALL EMBED PLATES ARE TYPE "E1", U.N.O.
2. SEE DETAIL 4/S501 FOR EMBED PLATE SIZE & DIMENSIONS.
3. VERIFY ALL OPENING W/ ARCHT'L. DRAWINGS.
4. SEE TILT-UP WALL PANEL SECTION OF THE GENERAL NOTES FOR TYPICAL PANEL REINFORCING.
5. INSTALL EMBEDS SHOWN IN DETAILS & ON PANEL ELEVATIONS.
6. PROVIDE GALV. STEEL EMBED AT O.H. DOORS, SEE 6/S501 FOR INFORMATION.
7. SEE SHEET S503 AND S504 FOR REINFORCEMENT PLACEMENT.
8. TYPICAL NON-COMPOSITE PANEL THICKNESS IS 14 1/4", U.N.O.
9. LOCATIONS NOTED THUS: (*) INDICATES THE VERTICAL REINFORCING NOTED SHALL REPLACE THE TYPICAL VERTICAL REINFORCING REQUIRED PER THE GENERAL NOTES.
10. TYPICAL REINFORCING SHALL RUN THROUGH FUTURE KNOCK-OUT OPENINGS.

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3120 Southwest Freeway, Suite 410
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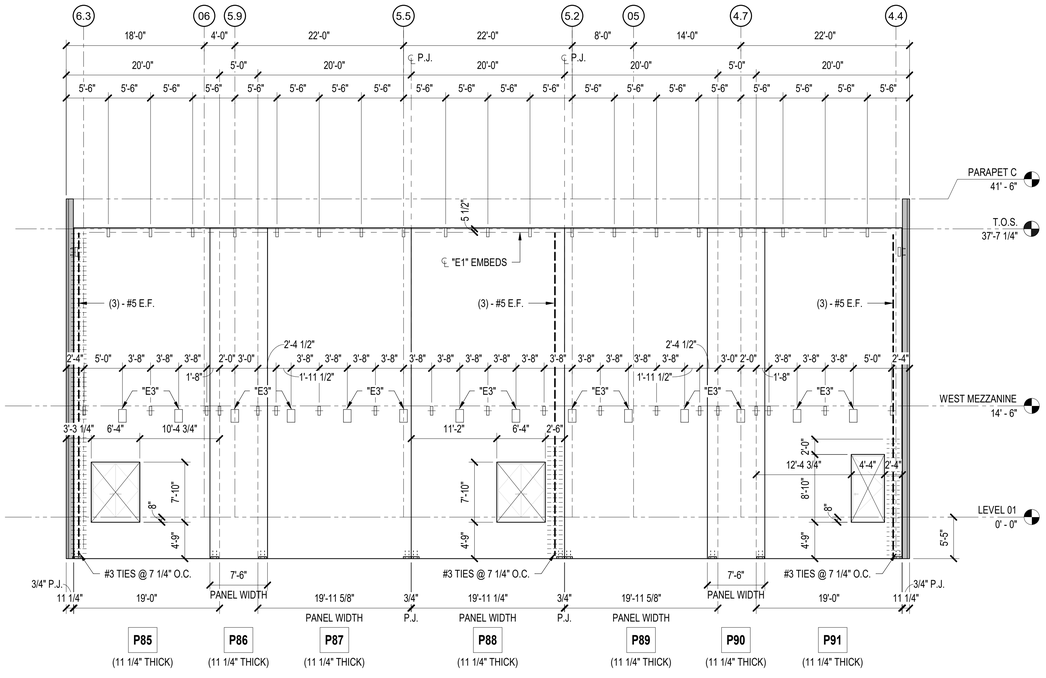
HIGHLAND RD

PROJECT ADDRESS

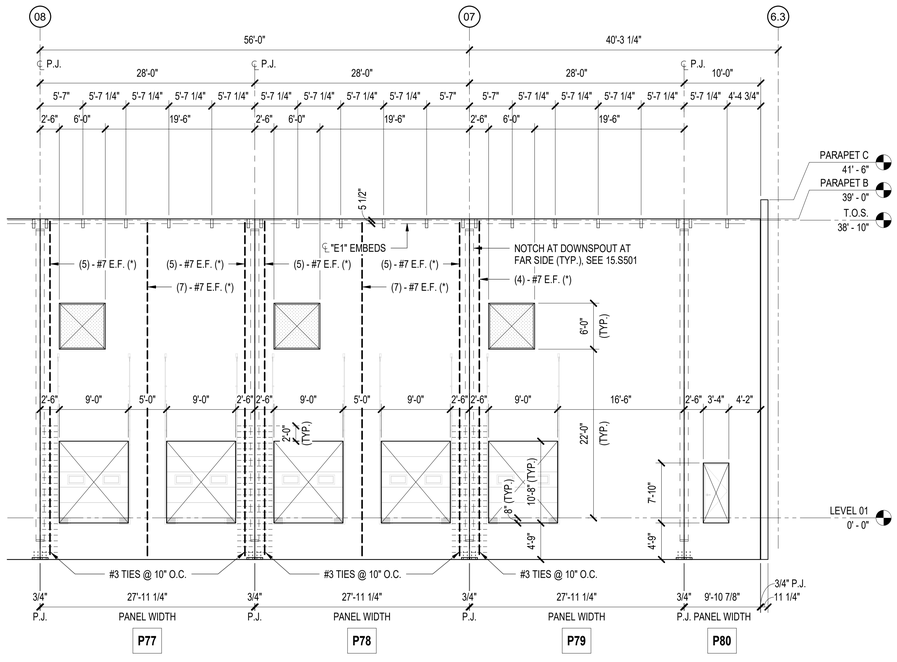
RMA PROJECT NO: 251172

PANEL ELEVATIONS

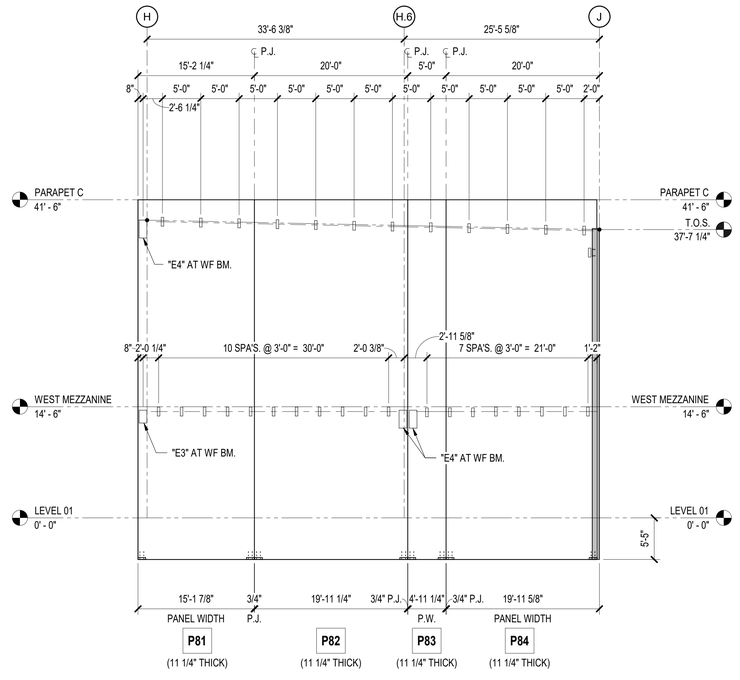
S5.14



4 PANEL ELVATION
1/8" = 1'-0"



3 PANEL ELVATION
1/8" = 1'-0"



1 PANEL ELVATION
1/8" = 1'-0"

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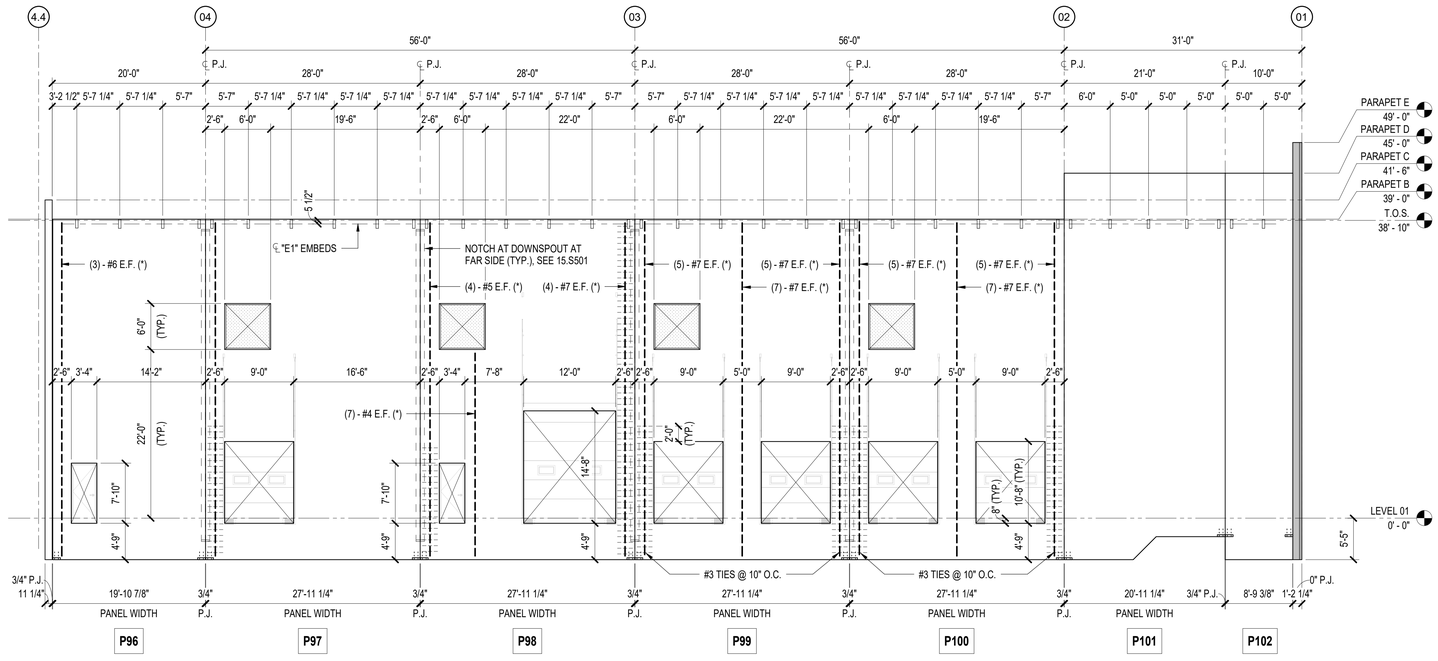
HIGHLAND RD

PROJECT ADDRESS

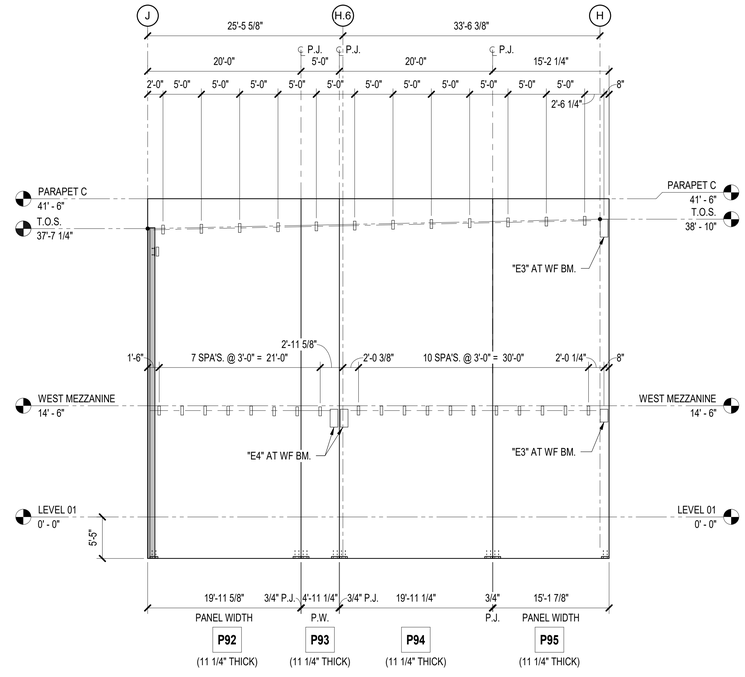
RMA PROJECT NO: 251172

PANEL ELEVATIONS

S5.15



4 PANEL ELVATION
1/8" = 1'-0"



1 PANEL ELVATION
1/8" = 1'-0"

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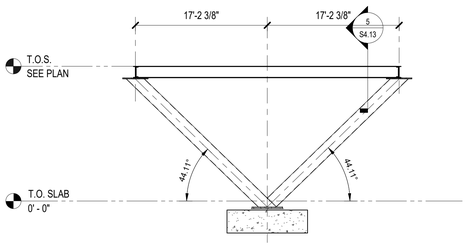
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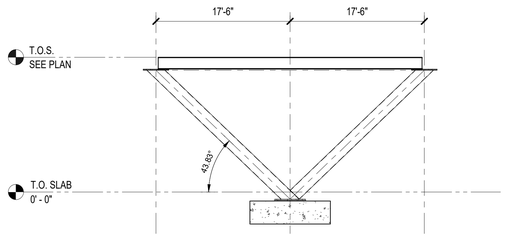
RMA PROJECT NO: 251172

PANEL ELEVATIONS

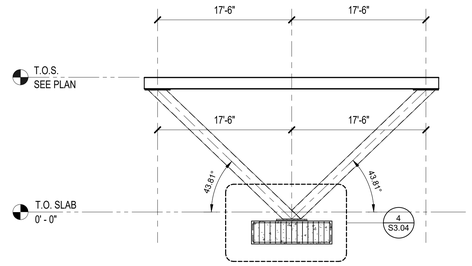
S5.16



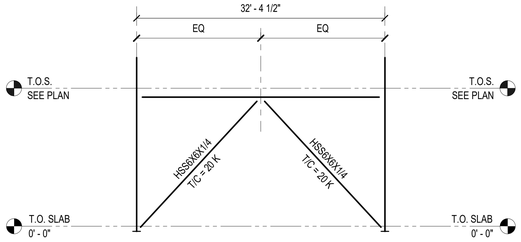
16 SLANTED COLUMN ELEVATION
1/8" = 1'-0"



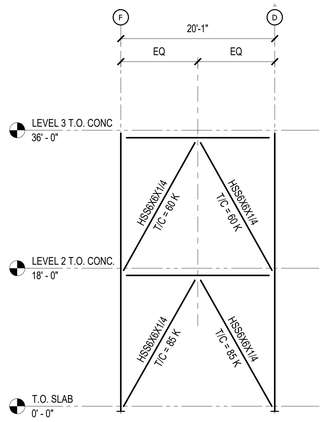
12 SLANTED COLUMN ELEVATION
1/8" = 1'-0"



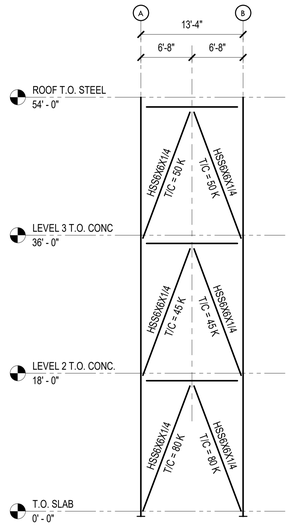
8 SLANTED COLUMN ELEVATION
1/8" = 1'-0"



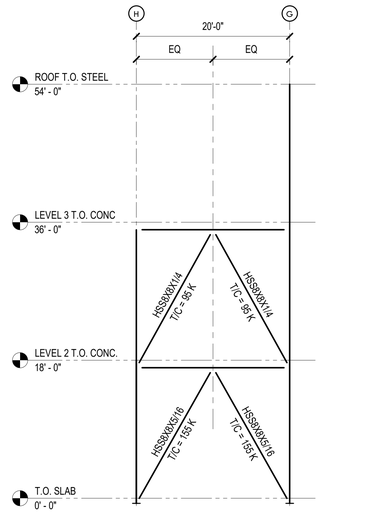
15 BRACE J
1/8" = 1'-0"



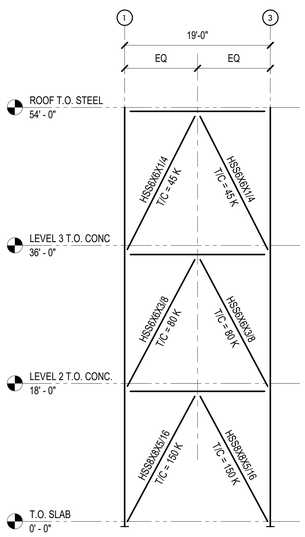
11 BRACE H
1/8" = 1'-0"



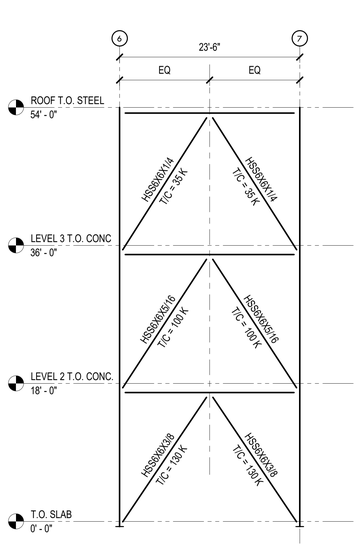
6 BRACE G
1/8" = 1'-0"



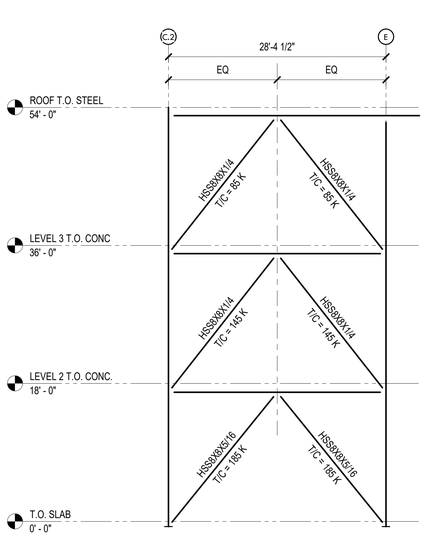
3 BRACE F
1/8" = 1'-0"



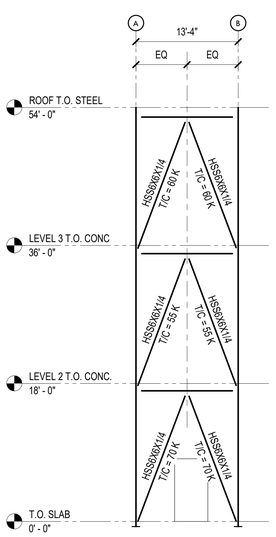
17 BRACE E
1/8" = 1'-0"



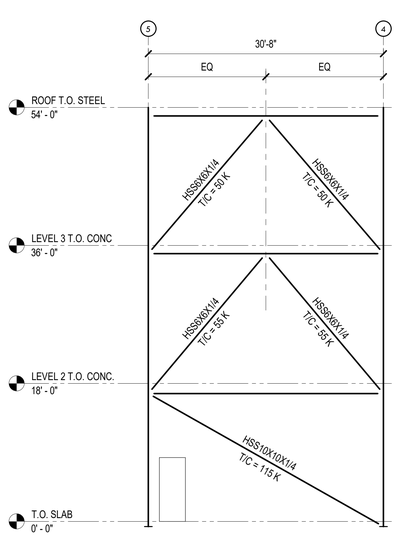
13 BRACE D
1/8" = 1'-0"



9 BRACE C
1/8" = 1'-0"



4 BRACE B
1/8" = 1'-0"



1 BRACE A
1/8" = 1'-0"

GENERAL NOTES

1. REFER TO BRACE ELEVATIONS FOR BRACE SIZES & FORCES.
2. IF NOT PROVIDED, U.N.O., THE BRACE FORCE SHALL BE THE MAXIMUM TENSION AND / OR COMPRESSION CAPACITY OF THE BRACE. IF BRACE FORCES ARE SHOWN, THEY ARE FACTORED LOADS (LRFD).
3. GUSSET PLATES SHALL BE CENTERED ON STEEL BEAM & DESIGNED TO DEVELOP BRACE FORCE.
4. BOLTS & WELDS SHALL BE DESIGNED BY THE FABRICATOR FOR THE VERTICAL & THE HORIZONTAL COMPONENTS OF THE BRACE FORCE INCLUDING THE EFFECT OF THE ECCENTRICITY. THE MINIMUM SIZE FILLETWELD SHALL BE 1/4".

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	01/14/26	100% DD
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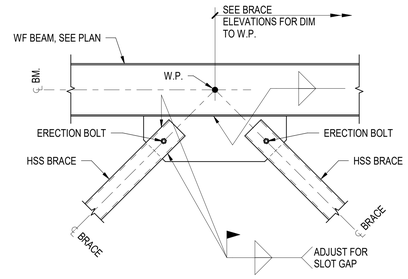
LIPSEY'S NEW HEADQUARTERS

HIGHLAND RD, ST. GEORGE, LA

RMA PROJECT NO: 24.154

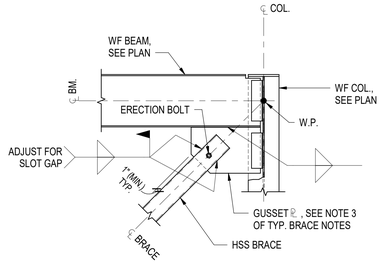
BRACE ELEVATIONS & DETAILS

S6.01

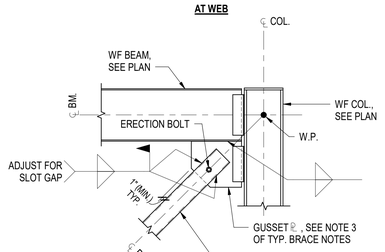


NOTE:
SEE SHEET S6.01 FOR TYPICAL BRACE NOTES.

4 TYPICAL DETAIL - BRACE CONNECTION
3/4" = 1'-0"



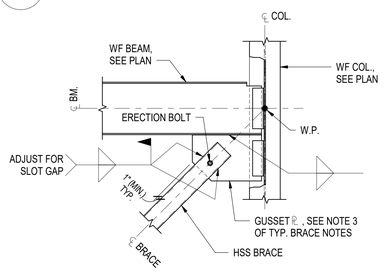
AT WEB



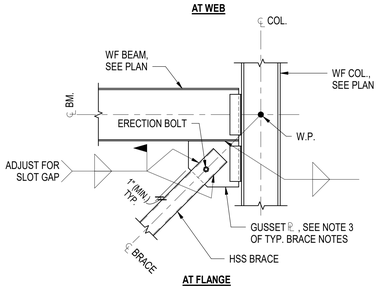
AT FLANGE

NOTE:
1. SEE SHEET S6.01 FOR TYPICAL BRACE NOTES.
2. BEAM & GUSSET MAY BE BOLTED OR FIELD WELDED TO THE COLUMN.

6 TYPICAL DETAIL - BRACE CONNECTION
3/4" = 1'-0"



AT WEB

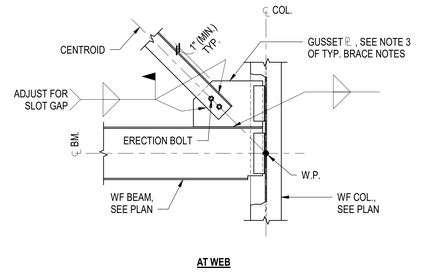


AT FLANGE

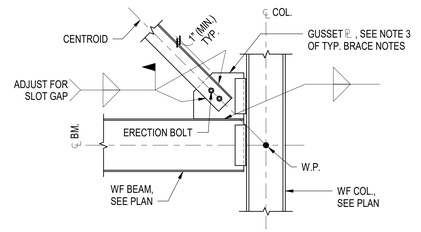
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5 TYPICAL DETAIL - BRACE CONNECTION
3/4" = 1'-0"

2 TYPICAL DETAIL - BRACE CONNECTION
3/4" = 1'-0"

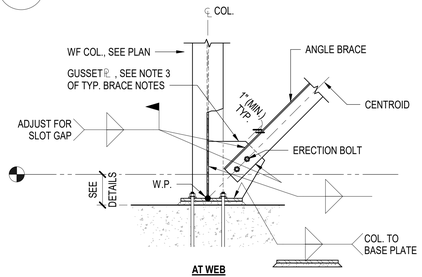


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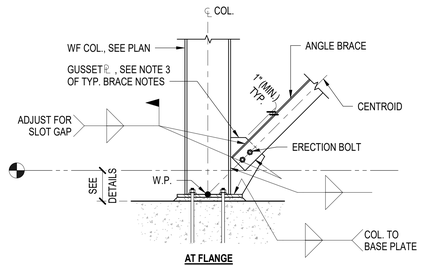


AT FLANGE

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2. BEAM & GUSSET MAY BE BOLTED OR FIELD WELDED TO THE COLUMN.



AT WEB



AT FLANGE

NOTE:
SEESHEET S6.01 FOR TYPICAL BRACE NOTES.

1 TYPICAL DETAIL - BRACE CONNECTION
3/4" = 1'-0"

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RMA PROJECT NO: 24.154

BRACE DETAILS

S6.02