BURBANK DR -
BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH
VSMsq Project Number: 26154

LV COLLECTIVE

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Sheet List - VSMsq

Sheet Number	Sheet Name
PS0.107	Loading Plan - Parking Level 07.5
PS1.101	Parking Level 01
PS2.107	Parking Level 07.5
PS2.108	Parking Level Roof
S0.000	Cover Sheet
S0.100	General Notes
S0.101	General Notes
S0.110	General Notes - Reinforcement
S0.111	General Notes - Reinforcement / Embeds
S0.120	PT General Notes
S0.121	Typical PT Details
S0.122	Typical PT Details
S0.123	Typical PT Details
S0.130	CMU General Notes / Typical Details
S0.131	CMU Typical Details
S0.140	Typical Metal Stud Framing General Notes / Details
S0.141	Typical Metal Stud Framing Details
S0.200	General Notes - Wood
S0.201	General Notes - Wood
S0.202	Typical Framing Details - Wood
S0.203	Typical Framing Details - Wood
S0.204	Typical Framing Details - Wood
S0.205	Stud / Post Schedule - Wood
S0.300	3D Views
S0.301	3D Views
S0.302	3D Views
S0.303	3D Views
S1.101	Full Site - Foundation Plan - Level 01
S1.102	Full Site - Forming Plan - Level 02
S1.103	Full Site - Wood Framing Plan - Level 03
S1.E101	Foundation Plan - East Building - Level 01 - OA
S1.E102	Forming Plan - East Building - Level 02 - OA
S1.E103	Wood Framing Plan - East Building - Level 03 - OA
S1.E104	Wood Framing Plan - East Building - Level 04 - OA
S1.E105	Wood Framing Plan - East Building - Level 05 - OA
S1.E106	Wood Framing Plan - East Building - Level 06 - OA
S1.E107	Wood Framing Plan - East Building - Roof - OA
S1.E108	Wood Shearwall Plan - East Building - OA
S1.W101	Foundation Plan - West Building - Level 01 - OA
S1.W102	Wood Framing Plan - West Building - Level 02 - OA
S1.W103	Wood Framing Plan - West Building - Level 03 - OA
S1.W104	Wood Framing Plan - West Building - Level 04 - OA
S1.W105	Wood Framing Plan - West Building - Level 05 - OA
S1.W106	Wood Framing Plan - West Building - Roof - OA
S1.W107	Wood Framing Plan - West Building - Roof - OA
S1.W108	Wood Shearwall Plan - West Building - OA
S2.E101a	Foundation Plan - East Building - Level 01 - Area A
S2.E101b	Foundation Plan - East Building - Level 01 - Area B
S2.E102a	Forming Plan - East Building - Level 02 - Area A
S2.E102b	Forming Plan - East Building - Level 02 - Area B
S2.E403a	Wood Framing Plan - East Building - Level 03 - Area A
S2.E403b	Wood Framing Plan - East Building - Level 03 - Area B
S2.E404a	Wood Framing Plan - East Building - Level 04 - Area A
S2.E404b	Wood Framing Plan - East Building - Level 04 - Area B
S2.E405a	Wood Framing Plan - East Building - Level 05 - Area A
S2.E405b	Wood Framing Plan - East Building - Level 05 - Area B
S2.E406a	Wood Framing Plan - East Building - Level 06 - Area A
S2.E406b	Wood Framing Plan - East Building - Level 06 - Area B
S2.E407a	Wood Framing Plan - East Building - Roof - Area A
S2.E407b	Wood Framing Plan - East Building - Roof - Area B
S2.W101a	Foundation Plan - West Building - Level 01 - Area A
S2.W101b	Foundation Plan - West Building - Level 01 - Area B
S2.W101c	Foundation Plan - West Building - Level 01 - Area C
S2.W101d	Foundation Plan - West Building - Level 01 - Area D
S2.W102B	Forming Plan - West Building - Level 02 - Area B
S2.W102C	Forming Plan - West Building - Level 02 - Area C
S2.W103C	Forming Plan - West Building - Level 03 - Area C
S2.W402a	Wood Framing Plan - West Building - Level 02 - Area A
S2.W402b	Wood Framing Plan - West Building - Level 02 - Area B
S2.W402d	Wood Framing Plan - West Building - Level 02 - Area D
S2.W403a	Wood Framing Plan - West Building - Level 03 - Area A
S2.W403b	Wood Framing Plan - West Building - Level 03 - Area B
S2.W403d	Wood Framing Plan - West Building - Level 03 - Area D
S2.W404a	Wood Framing Plan - West Building - Level 04 - Area A
S2.W404b	Wood Framing Plan - West Building - Level 04 - Area B
S2.W404c	Wood Framing Plan - West Building - Level 04 - Area C
S2.W404d	Wood Framing Plan - West Building - Level 04 - Area D
S2.W405a	Wood Framing Plan - West Building - Level 05 - Area A
S2.W405b	Wood Framing Plan - West Building - Level 05 - Area B
S2.W405c	Wood Framing Plan - West Building - Level 05 - Area C
S2.W405d	Wood Framing Plan - West Building - Level 05 - Area D
S2.W406a	Wood Framing Plan - West Building - Roof - Area A
S2.W406b	Wood Framing Plan - West Building - Level 06/ Roof - Area B
S2.W406c	Wood Framing Plan - West Building - Level 06/ Roof - Area C
S2.W406d	Wood Framing Plan - West Building - Roof - Area D
S2.W407b	Wood Framing Plan - West Building - Roof - Area B
S2.W407c	Wood Framing Plan - West Building - Roof - Area C
S3.000	Typical Foundation Details
S3.001	Typical Foundation Details
S3.010	PT Slab on Grade Foundation General Notes / Typical Details
S3.011	PT Slab on Grade Foundation Typical Details
S3.012	PT Slab on Grade Foundation Typical Details
S3.100	Foundations Sections
S4.000	Typical Concrete Connections
S4.001	Typical Concrete Connections
S4.021	Typical Steel Bar Joist Details
S4.100	Framing Sections
S4.110	Framing Sections - Wood Floor
S4.111	Framing Sections - Wood Floor
S4.112	Framing Sections - Wood Floor
S4.210	Stair Framing
S4.220	Elevator Framing
S4.310	Roof Framing Sections
S4.311	Roof Framing Sections
S4.400	Framing - Typical Steel K Bar Joist Roof Details
S4.420	Typical Steel Details
S5.100	CIP Concrete Column Schedule
S5.110	Typical CIP Concrete Column Details
S5.120	CIP Concrete Column Elevations
S5.130	Typical Stud Rail Details
S5.200	CIP Concrete Shear Wall Typical Details
S5.201	CIP Concrete Shearwall Typical Details
S5.202	CIP Concrete Shearwall Typical Details
S5.203	CIP Concrete Shearwall Typical Details
S5.300	CIP Concrete Beams
S5.310	CIP Concrete Beam Schedule
S5.410	Wood Shear Wall Schedule
S5.412	Wood Shear Wall Hold-Down Details - ATR / Rod System
S5.413	Wood Shear Wall Typical Details
S5.414	Wood Shear Wall Anchorage ATR / Rod System - Uniform Thickness Slab on Grade
S5.414A	Wood Shear Wall Anchorage ATR / Rod System - Uniform Thickness Podium Slab
S5.415	Embedded Rod Anchorage Option - Uniform Thickness Slab
S5.416	Embed Plate Anchorage Option - Uniform Thickness Slab
S5.417	Shear Wall Anchorage at Slab Steps
S5.420	Roof Girder Truss Uplift Details
S5.421	Roof Truss Uplift Details
S5.430	Wood Strapping Details

CONSTRUCTION ADMINISTRATION

- ALL STRUCTURAL SHOP DRAWINGS AND RFI'S SUBMITTED ELECTRONICALLY TO **VSM²** FOR REVIEW SHALL BE EMAILED TO: **ca@vsmsq.com**
- ALL FIELD REPORTS SUBMITTED ELECTRONICALLY TO **VSM²** SHALL BE EMAILED TO: **fieldreports@vsmsq.com**

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	100% DD	09/30/2026

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EAST BATON ROUGE PARISH

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SHEET NUMBER: S0.000

NOT RELEASED FOR CONSTRUCTION

GN1.0 GENERAL NOTES

1. THE STRUCTURAL DESIGN PRESENTED IN THESE DOCUMENTS CONFORMS TO THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE, 2021 EDITION WITH CITY OF BATON ROUGE AMENDMENTS.
2. THE GENERAL CONTRACTOR SHALL REVIEW SHOP DRAWINGS AND NOTE ALL DOCUMENT DISCREPANCIES PRIOR TO SUBMITTAL TO THE DESIGN TEAM FOR APPROVAL, AND SHALL COORDINATE SUBMITTAL RESPONSES BETWEEN SEPARATE SUBCONTRACTORS AS NEEDED.
3. THESE DOCUMENTS REPRESENT STRUCTURAL COMPONENTS IN THEIR FINISHED STATE AT COMPLETION OF THE PROJECT, AND METHODS OF CONSTRUCTION ARE NOT INCLUDED OR IMPLIED HEREIN. THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, AND SAFETY PRECAUTIONS REQUIRED TO COMPLETE THE WORK.
4. CONSTRUCTION VEHICLE LOADS SHALL NOT BE PERMITTED ON ELEVATED STRUCTURAL FRAMING WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
5. MATERIAL TESTING AND SPECIAL INSPECTIONS SHALL BE PERFORMED BY THE OWNER'S THIRD-PARTY TESTING LAB IN ACCORDANCE WITH THESE SPECIFICATIONS AND THE GOVERNING BUILDING CODE AS NOTED ABOVE. UNLESS NOTED OTHERWISE, PERIODIC SITE OBSERVATIONS BY FIELD REPRESENTATIVES OF THE STRUCTURAL ENGINEER DURING CONSTRUCTION ARE CURSORY IN SCOPE AND ARE NOT INTENDED TO BE A CHECK OF THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER ARE SOLELY FOR CONFIRMATION THAT THE WORK OF THE CONTRACTOR IS PROCEEDING IN ACCORDANCE WITH THE GENERAL INTENT OF THE STRUCTURAL CONTRACT DOCUMENTS. THE GENERAL CONTRACTOR SHALL VERIFY COMPLETENESS OF WORK BEFORE SCHEDULING SITE OBSERVATIONS BY THE ENGINEER.
6. DIMENSIONAL CONTROL FOR THE STRUCTURE IS DICTATED BY THE ARCHITECT. GENERAL CONTRACTOR SHALL CONFIRM DIMENSIONS FOR SLAB EDGES, DEPRESSIONS, EXPANSION JOINTS, CURBS, DRAINS, SLEEVES, INSERTS AND ALL OTHER OPENINGS WITH ARCHITECTURAL AND M/E/P DRAWINGS AND SHALL PROVIDE FOR ALL OPENINGS PRIOR TO PLACEMENT OF STRUCTURAL FRAMING. PENETRATIONS AND/OR DEPRESSIONS NOTED BY ARCHITECT OR M/E/P THAT ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED FOR APPROVAL PRIOR TO CONSTRUCTION.
7. SUSPENDED LOADS NOT SHOWN IN THE STRUCTURAL DRAWINGS THAT EXCEED 300 POUNDS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.
8. STRUCTURAL FRAMING MEMBERS SHALL BE CENTERED ON GRIDLINES AND EQUALLY SPACED UNLESS NOTED OTHERWISE.
9. PRODUCTS SUBMITTED FOR APPROVAL AS ALTERNATES TO THOSE SPECIFIED IN THE CONTRACT DOCUMENTS SHALL BE ACCOMPANIED BY BOTH A DOCUMENTED COST SAVINGS TO THE OWNER AND A CURRENT INTERNATIONAL CODE COUNCIL (ICC-ES) APPROVAL REPORT TO BE CONSIDERED FOR USE.
10. THE FOLLOWING SPECIFICATIONS OUTLINE THE MATERIAL PROPERTIES AND APPLICATIONS REQUIRED AS A MINIMUM TO CONSTRUCT THE STRUCTURAL COMPONENTS OF THE PROJECT. PRODUCT MANUFACTURER AND LOCAL CODE REQUIREMENTS SHALL CONTROL WHEN IN EXCESS OF THESE MINIMUMS.

GN1.1 GRAVITY LOAD CRITERIA

FLOOR LOADS*:

SUPERIMPOSED DEAD LOADS:

A. CEILING & MECHANICAL	5 PSF
B. FIXED PARTITIONS	10 PSF

LIVE LOADS:

A. UNITS (PRIVATE ROOOM & CORRIDORS SERVING THEM)	40 PSF
B. PUBLIC AREAS (PUBLIC ROOMS & CORRIDORS SERVING THEM)	100 PSF
C. BALCONIES	1.5 x LOAD OF AREA SERVED, 100 PSF MAX
D. LOBBY & 1ST FLOOR CORRIDORS	100 PSF / 2,000 LBS CONCENTRATED
E. OFFICES	50 PSF
F. OFFICE CORRIDORS ABOVE 1ST FLOOR	80 PSF
G. MOVEABLE PARTITIONS	15 PSF
H. PASSENGER VEHICLE PARKING	40 PSF / 3,000 LBS CONCENTRATED
I. LIGHT STORAGE	125 PSF
J. TRUCK DOCK	250 PSF / 8,000 LBS CONCENTRATED
K. GENERATOR / TRANSFORMER ROOM	125 PSF
L. MECHANICAL ROOM	80 PSF

ROOF LOADS:

SUPERIMPOSED DEAD LOADS:

A. CEILING & MECHANICAL	5 PSF
B. TYPICAL ROOF ASSEMBLY	15 PSF
C. ROOFTOP MECHANICAL EQUIPMENT	AS SPECIFIED BY MECH (30 PSF MIN)

LIVE LOADS:

A. STANDARD ROOF	20 PSF

*SEE AMENITY LOADING PLANS FOR MORE INFORMATION

GN1.2 WIND LOAD CRITERIA

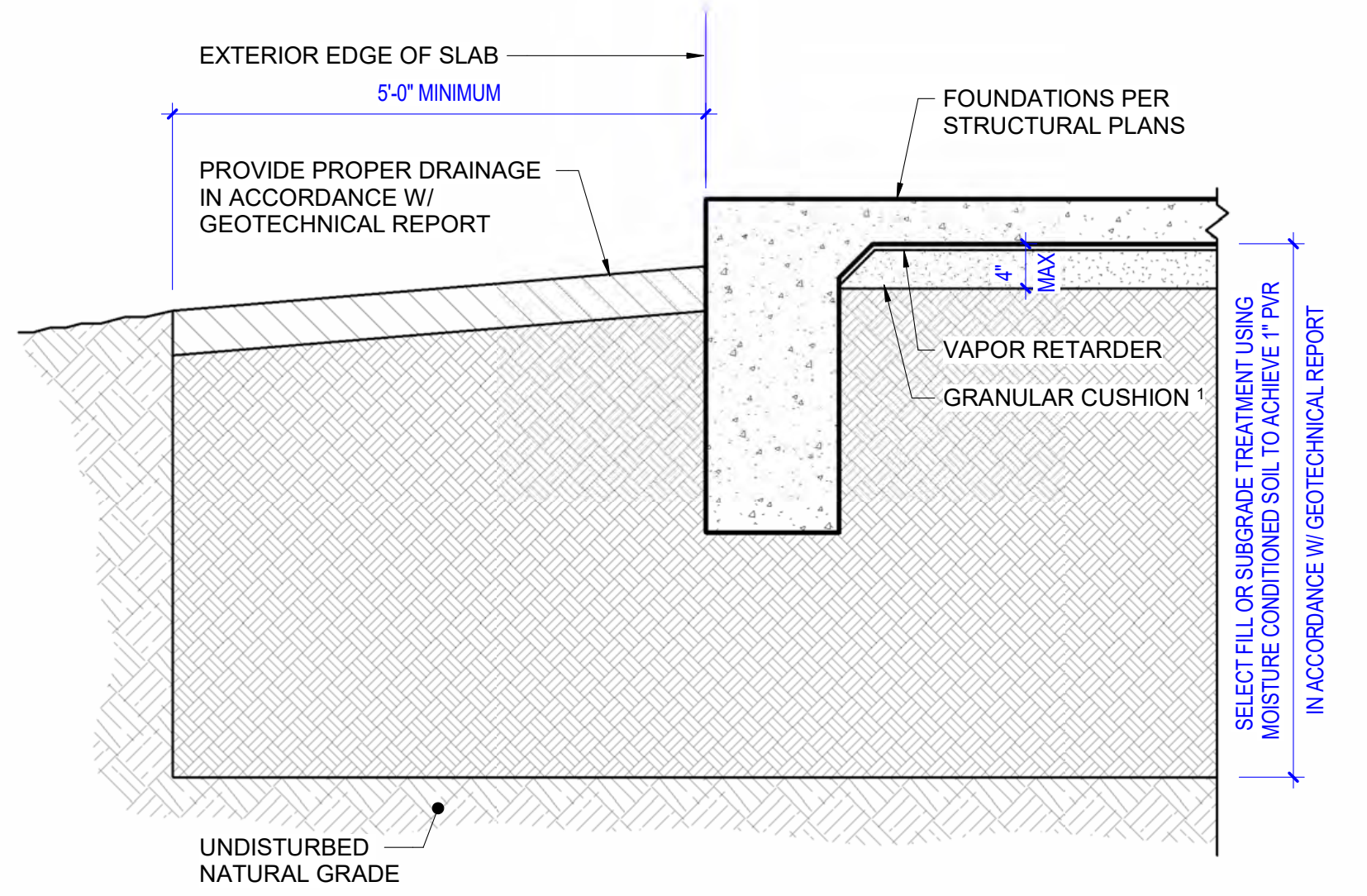
ULTIMATE BASIC WIND SPEED (3-SEC GUST), V_{ULT}	XX MPH
ALLOWABLE STRESS DESIGN WIND SPEED (3-SEC GUST), V_{ASD}	125 MPH
RISK CATEGORY	II
WIND EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT	+0.18
COMPONENT & CLADDING PRESSURES	SEE PRESSURE ZONE DETAILS

GN1.4 SEISMIC LOADING CRITERIA

SITE CLASSIFICATION	D
SEISMIC IMPORTANCE FACTOR	1.0
RISK CATEGORY	II
DESIGN SPECTRAL RESPONSE ACCELERATIONS	SS = 0.088 S1 = 0.057
SPECTRAL RESPONSE COEFFICIENTS	SDS = 0.094 SD1 = 0.090
SEISMIC DESIGN CATEGORY	B
SEISMIC FORCE RESISTING SYSTEM	ORDINARY CONCRETE SHEAR WALLS ORDINARY CONCRETE MOMENT FRAMES AND LIGHT FRAME WOOD SHEAR WALLS SHEATHED WITH WOOD STRUCTURAL PANEL OR OTHER MATERIALS
SEISMIC RESPONSE COEFFICIENT, C_S	ORDINARY CONCRETE SHEAR WALLS, Cs = 0.024 ORDINARY CONCRETE MOMENT FRAMES, R = 0.032 LIGHT FRAME WOOD SHEAR WALLS SHEATHED WITH WOOD STRUCTURAL PANEL OR OTHER MATERIALS, R = 0.047
RESPONSE MODIFICATION FACTOR, R	ORDINARY CONCRETE SHEAR WALLS, R=4 ORDINARY CONCRETE MOMENT FRAMES, R=3 LIGHT FRAME WOOD SHEAR WALLS SHEATHED WITH WOOD STRUCTURAL PANEL OR OTHER MATERIALS, R=2
DESIGN BASE SHEAR	X – DIRECTION = Cs * BUILDING WEIGHT Y – DIRECTION = Cs * BUILDING WEIGHT
DEFLECTION AMPLIFICATION FACTOR, C_d	ORDINARY CONCRETE SHEAR WALLS, Cd = 2.5 ORDINARY CONCRETE MOMENT FRAMES, Cd = 3 LIGHT FRAME WOOD SHEAR WALLS SHEATHED WITH WOOD STRUCTURAL PANEL OR OTHER MATERIALS, Cd = 2.5
ANALYSIS PROCEDURE (ELFP or MRSA)	ELFP

FN1.1 SPREAD FOOTING FOUNDATIONS (IMPROVED SOIL)

1. SPREAD FOOTING FOUNDATION DESIGN IS BASED ON GEOTECHNICAL ENGINEERING REPORT NO.26-0141 PREPARED BY PREMIER GEOTECH AND TESTING, LLC, DATED MAY 29, 2026, AND ALL ISSUED ADDENDUMS.
2. SOIL IMPROVEMENT DESIGN AND SPECIFICATIONS TO BE PERFORMED BY A SPECIALTY CONTRACTOR WITH EXPERIENCE IN GROUND IMPROVEMENT TECHNIQUES. ALL RECOMMENDATIONS NOTED IN THE GEOTECH REPORT ARE TO BE FOLLOWED. IMPROVED SOIL DESIGN AND SPECIFICATIONS TO BE SIGNED AND SEALED BY AN ENGINEERED LICENSED IN THE STATE WHERE THE PROJECT IS BEING BUILT. ALL DESIGN CRITERIA AND SHOP DRAWINGS TO BE SUBMITTED TO THE GEOTECHNICAL ENGINEER AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO THE START OF CONSTRUCTION.
3. SOIL IMPROVEMENT TECHNIQUES SHALL ACHIEVE A TOTAL SETTLEMENT OF 1" AND A DIFFERENTIAL IN 30' OF ¾". TOTAL SETTLEMENT SHALL BE MEASURED FROM THE BEGINNING OF CONSTRUCTION OF FOUNDATIONS.
4. BUILDING FOUNDATIONS HAVE BEEN DESIGNED TO MEET THE ABOVE SETTLEMENT CRITERIA AND THE BELOW BEARING CAPACITY ON IMPROVED SOILS:
   A. INDIVIDUAL FOUNDATIONS SUPPORTING CAST IN PLACE CONCRETE PODIUM HAVE BEEN DESIGNED TO A MAXIMUM ALLOWABLE BEARING PRESSURE OF 4,000 PSF WITH AN ALLOWABLE INCREASE OF 1/4 WHEN CONSIDERING LOAD COMBINATIONS WITH WIND OR SEISMIC AND SOIL MODULUS OF 25 PCI.
   B. INDIVIDUAL FOUNDATIONS SUPPORTING STEEL COLUMNS HAVE BEEN DESIGNED TO A MAXIMUM ALLOWABLE BEARING PRESSURE OF 2,000 PSF WITH AN ALLOWABLE INCREASE OF 1/3 WHEN CONSIDERING LOAD COMBINATIONS WITH WIND OR SEISMIC AND SOIL MODULUS OF 25 PCI.
   C. STRUCTURAL POST TENSIONED SLAB ON GRADE HAS BEEN DESIGNED TO A MAXIMUM ALLOWABLE BEARING PRESSURE OF 2,000 PSF WITH ALLOWABLE INCREASE OF 1/3 WHEN CONSIDERING LOAD COMBINATIONS WITH WIND OR SEISMIC AND A VARYING MODULUS OF SUBGRADE REACTION OF 80 PCI OVER A 3'-0" SQUARE AREA CENTERED OVER GROUND IMPROVEMENT ELEMENTS AND 20 PCI MODULUS OF SUBGRADE REACTION AT ALL OTHER LOCATIONS.
5. CONTRACTOR TO REVIEW GEOTECHNICAL REPORT AND GROND IMPROVEMENT DESIGN AND SPECIFICATIONS AND FOLLOW ALL RECOMMENDATIONS NOTED THEREIN.
6. FOOTING EXCAVATIONS SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER OR THE OWNER'S SPECIAL INSPECTOR PRIOR TO REINFORCEMENT AND CONCRETE PLACEMENT TO ENSURE BEARING SURFACES ARE CONSISTENT WITH DESIGN RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT.
7. FOOTING EXCAVATIONS SHALL BE FREE OF WATER PRIOR TO CONCRETE PLACEMENT. IF QUESTIONABLE SOILS ARE ENCOUNTERED, THE GEOTECHNICAL ENGINEER AND STRUCTURAL ENGINEER SHOULD BE NOTIFIED IMMEDIATELY.
8. FOOTING EXCAVATION SHALL BE NEAT UNLESS SPECIFICALLY NOTED IN THE GEOTECHNICAL REPORT AND THE CONTRACTOR HAS PLANNED FOR FORMED FOOTINGS.
9. FOOTINGS SHALL BE PLACED THE SAME DAY THAT EXCAVATION OCCURS. EXCAVATIONS SHALL NOT BE LEFT OPEN OVERNIGHT WITHOUT THE WRITTEN CONSENT OF THE GEOTECHNICAL ENGINEER.


EXTERIOR EDGE OF SLAB
5'-0" MINIMUM
FOUNDATIONS PER STRUCTURAL PLANS
PROVIDE PROPER DRAINAGE IN ACCORDANCE W/ GEOTECHNICAL REPORT
4" MAX
VAPOR RETARDER
GRANULAR CUSHION 1
SELECT FILL OR SUBGRADE TREATMENT USING MOISTURE CONDITIONED SOIL TO ACHIEVE 1" PVR IN ACCORDANCE W/ GEOTECHNICAL REPORT
UNDISTURBED NATURAL GRADE

SITE PREPARATION SCHEMATIC

NOTE:
1. GRANULAR CUSHION TO BE OMITTED ON SLAB ON GRADE CONDITIONS WITH VEHICULAR TRAFFIC.

GN2.0 ABBREVIATIONS (GENERAL):

@	AT	ID	INSIDE DIAMETER
&	AND	IN	INCH
= / EQ	EQUAL / EQUALS	INFO	INFORMATION
≠	NOT EQUAL	INT	INTERIOR
±	PLUS OR MINUS	JT	JOINT
° / DEG	DEGREES	LGTH	LENGTH
⌀ / DIA	DIAMETER	LNG	LONG
AB	ANCHOR BOLT	LP	LOW POINT
ADD'L	ADDITIONAL	MANUF	MANUFACTURE
ALT	ALTERNATE	MAX	MAXIMUM
ARCH	ARCHITECT	MDF	MEDIUM-DENSITY FIBERBOARD
ASD	ALLOWABLE STRESS DESIGN	MDO	MEDIUM-DENSITY OVERLAY
BLDG	BUILDING	MECH	MECHANICAL
BM	BEAM	MISC	MISCELLANEOUS
B.O.	BOTTOM OF	MIN.	MINIMUM
BOF	BOTTOM OF FOUNDATION / FOOTING	NIC	NOT IN CONTRACT
BOW	BOTTOM OF WALL	N.S.	NEAR SIDE
BRG	BEARING	# / NO.	NUMBER
CANT	CANTILEVER	O.A.	OVER ALL
℄ / C.L.	CENTERLINE	O.C.	ON CENTER
CJ	CONSTRUCTION JOINT	O.D.	OUTSIDE DIAMETER
CLR	CLEAR	O.H.	OPPOSITE HAND
COL	COLUMN	OPNG	OPENING
CONFIG	CONFIGURE / CONFIGURATION	OPP	OPPOSITE
CONSTR	CONSTRUCTION	PAR	PARALLEL
CONT	CONTINUOUS	PERP	PERPENDICULAR
COORD	COORDINATE	P.L.	PROPERTY LINE
CTRS	CENTERS	PLF	POUNDS PER LINEAR FOOT
DIM	DIMENSION	PRELIM	PRELIMINARY
DET	DETAIL	PROP	PROPERTY
DWG	DRAWING	QT /QTY	QUANTITY
EA	EACH	R / RAD.	RADIUS
EE	EACH END	RE / REF	REFERENCE
EJ	EXPANSION JOINT	REQ'D	REQUIRED
EL / ELEV	ELEVATION	REM	REMAINDER
EF	EACH FACE	REINF	REINFORCEMENT
ENGR	ENGINEER	RTU	ROOF TOP UNIT
EPXY	EPOXY	S.B.	SET BACK
ES	EACH SIDE	SCHED	SCHEDULE (D)
EOR	ENGINEER OF RECORD	SEOR	STRUCTURAL ENGINEER OF RECORD
EQ	EQUAL	SIM	SIMILAR
EW	EACH WAY	SOG	SLAB ON GRADE
EXP	EXPANSION	SPEC	SPECIFICATION
EXT	EXTERIOR	SQ.	SQUARE
F.D.	FLOOR DRAIN	STD	STANDARD
FIN	FINISH	STRUCT	STRUCTURE / STRUCTURAL
FLR	FLOOR	SYM	SYMMETRIC / SYMMETRICAL
FND	FOUNDATION	T.O.	TOP OF
FT	FOOT / FEET	TOF	TOP OF FOUNDATION / FOOTING
F.S.	FAR SIDE	TOW	TOP OF WALL
GC	GENERAL CONTRACTOR	UNO	UNLESS NOTED OTHERWISE
GR	GRADE	VERT	VERTICAL
HORIZ	HORIZONTAL	W/	WITH
HP	HIGH POINT	W/O	WITH OUT
HT	HEIGHT		

GN2.1 ABBREVIATIONS (STRUCTURAL):

db	BAR DIAMETER	LRFD	LOAD AND RESISTANCE FACTOR DESIGN
f'c	28 DAY CONCRETE STRENGTH	P	AXIAL LOAD
f'm	28 DAY MASONRY STRENGTH	PCF	POUNDS PER CUBIC FOOT
Fe	EFFECTIVE FORCE	PCY	POUNDS PER CUBIC YARD
Fy	YIELD STRENGTH	PSI	POUNDS PER SQUARE INCH
K	KIPS (1,000 LBS)	PSF	POUNDS PER SQUARE FOOT
KLF	KIPS PER LINEAR FOOT	T	TORSION
KSF	KIPS PER SQUARE FOOT		
KSI	KIPS PER SQUARE INCH		
ℓd / Ld	DEVELOPMENT LENGTH		
LB / LBS	POUND / POUNDS		

GN2.2 ABBREVIATIONS (STRUCTURALLY SOCIETIES / INSTITUTES):

ACI	AMERICAN CONCRETE INSTITUTE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
BIA	BRICK INSTITUTE of AMERICA
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
IBC	INTERVENTIONAL BUILDING CODE
ICC	INTERNATIONAL CODE COUNCIL
ICBO	INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS
PCI	PRECAST / PRESTRESSED CONCRETE INSTITUTE
SJI	THE STEEL JOIST INSTITUTE

GN2.3 ABBREVIATIONS (CONCRETE / MASONRY):

ACIP	AUGERED CAST-IN-PLACE PILES	N.W.	NORMAL WEIGHT
BOF	BOTTOM OF FOOTING / FOUNDATION	NWC	NORMAL WEIGHT CONCRETE
BOW	BOTTOM OF WALL	PT	POST TENSION
CIP	CAST IN PLACE	REINF	REINFORCEMENT
CONC	CONCRETE	RS	ROCK SOCKET
COV	COVER	SOG	SLAB ON GRADE
CMU	CONCRETE MASONRY UNIT	TOC	TOP OF CONCRETE
db	BAR DIAMETER	TOF	TOP OF FOOTING / FOUNDATION
DBA	DEFORMED BAR ANCHOR	TO PIL	TOP OF PILASTER
DWL	DOWEL	TOW	TOP OF WALL
EPXY	EPOXY	WP	WORKING POINT
FDN	FOUNDATION	WWF	WELDED WIRE FABRIC
HSA	HEADED STUD ANCHOR	WWM	WELDED WIRE MESH
ℓd / Ld	DEVELOPMENT LENGTH	WWR	WELDED WIRE REINFORCEMENT
L.W.	LIGHT WEIGHT		
LWC	LIGHT WEIGHT CONCRETE		

GN2.4 ABBREVIATIONS (STEEL / LIGHT GAUGE FRAMING):

BB	BACK TO BACK	MC	MOMENT CONNECTION
BOS	BOTTOM OF STEEL	MTL	METAL
COMP	COMPOSITE	PAF	POWER ACTUATED FASTENER
DCW	DEMAND CRITICAL WELD	PJP	PARTIAL JOINT PENETRATION
FP	FULL PENETRATION	PL	PLATE
GALV	GALVANIZED	SS	STAINLESS STEEL
HSA	HEADED STUD ANCHOR	STIF	STIFFENER
HSS	HOLLOW STRUCTURAL SECTION	STL	STEEL
KB	KNEE BRACE	TOS	TOP OF STEEL
L	ANGLE	TS	TUBE SECTION
LLBB	LONG LEG BACK TO BACK	WF	WIDE FLANGE
LLH	LONG LEG HORIZONTAL	WP	WORKING POINT
LLV	LONG LEG VERTICAL	WT	WIDE FLANGE TEE
LG	LIGHT GAUGE	XS	EXTRA STRONG
LSH	LONG SIDE HORIZONTAL	XXS	DOUBLE EXTRA STRONG
LSV	LONG SIDE VERTICAL		

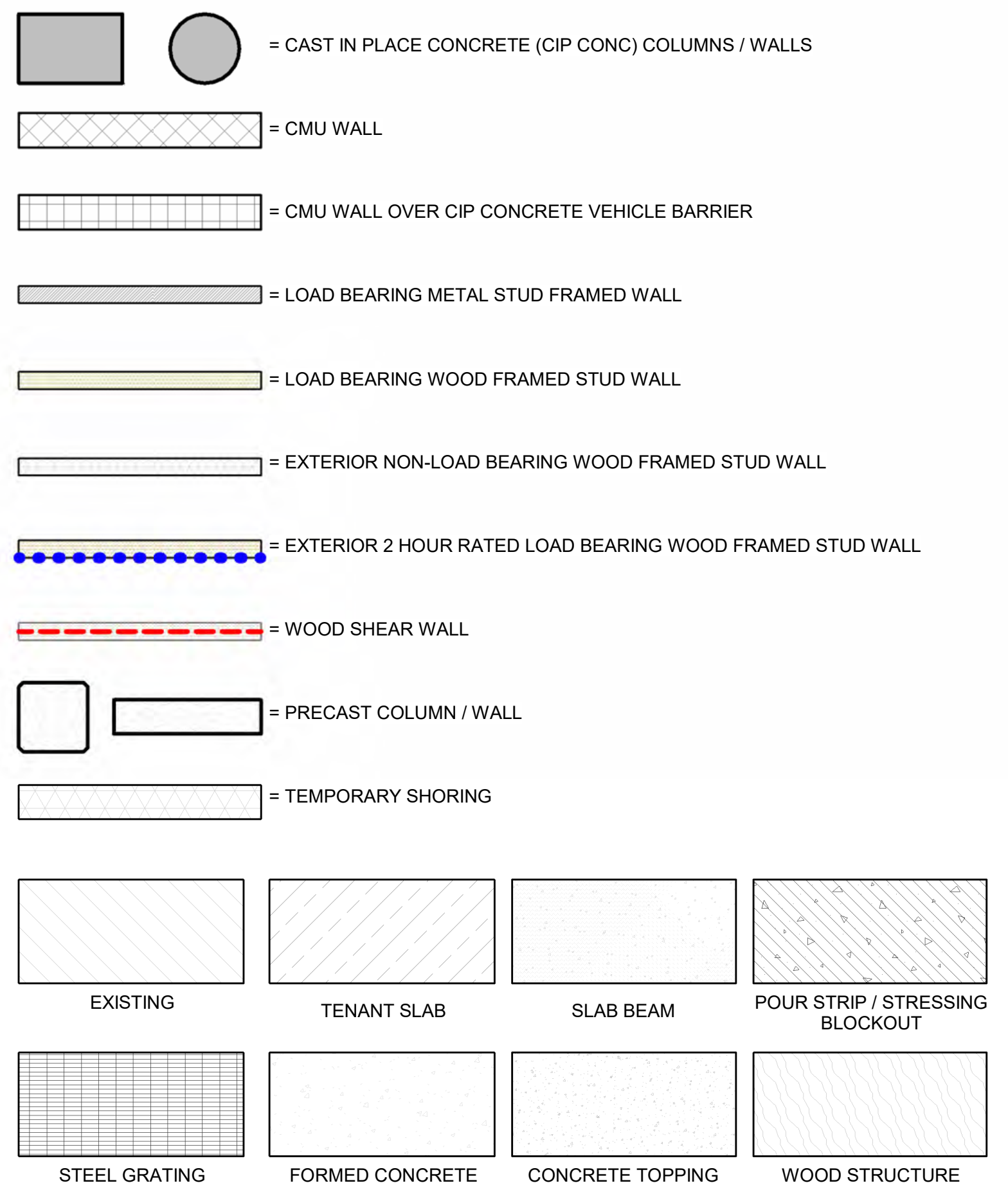
GN2.5 ABBREVIATIONS (WOOD FRAMING):

B.N.	BOUNDARY NAILING	LVL	LAMINATED VENEER LUMBER
CLT	CROSS LAMINATED TIMBER	NLBW	NON-LOAD BEARING WALL
D.B.	DROP BEAM	OSB	ORIENTED STRAND BOARD
DFL	DOUGLAS FIR LARCH	PLY	PLYWOOD
E.N.	EDGE NAILING	PSL	PARALLEL STRAND LUMBER
F.B.	FLUSH BEAM	PT	PRESSURE TREATED
FRT	FIRE RETARDANT TREATED	SS	SELECT STRUCTURAL
GL	GLUED LAMINATED TIMBER (GLU-LAM)	SW	SHEAR WALL
GT	GIRDER TRUSS	SWL	SHEAR WALL LENGTH
GWB	GYPSUM WALL BOARD	SYP	SOUTHER YELLOW PINE
GYP	GYPSUM	WSP	WOOD STRUCTURAL PANEL
HDR	HEADER		
HNGR	HANGER		
LBW	LOAD BEARING WALL		
LSL	LAMINATED STRAND LUMBER		

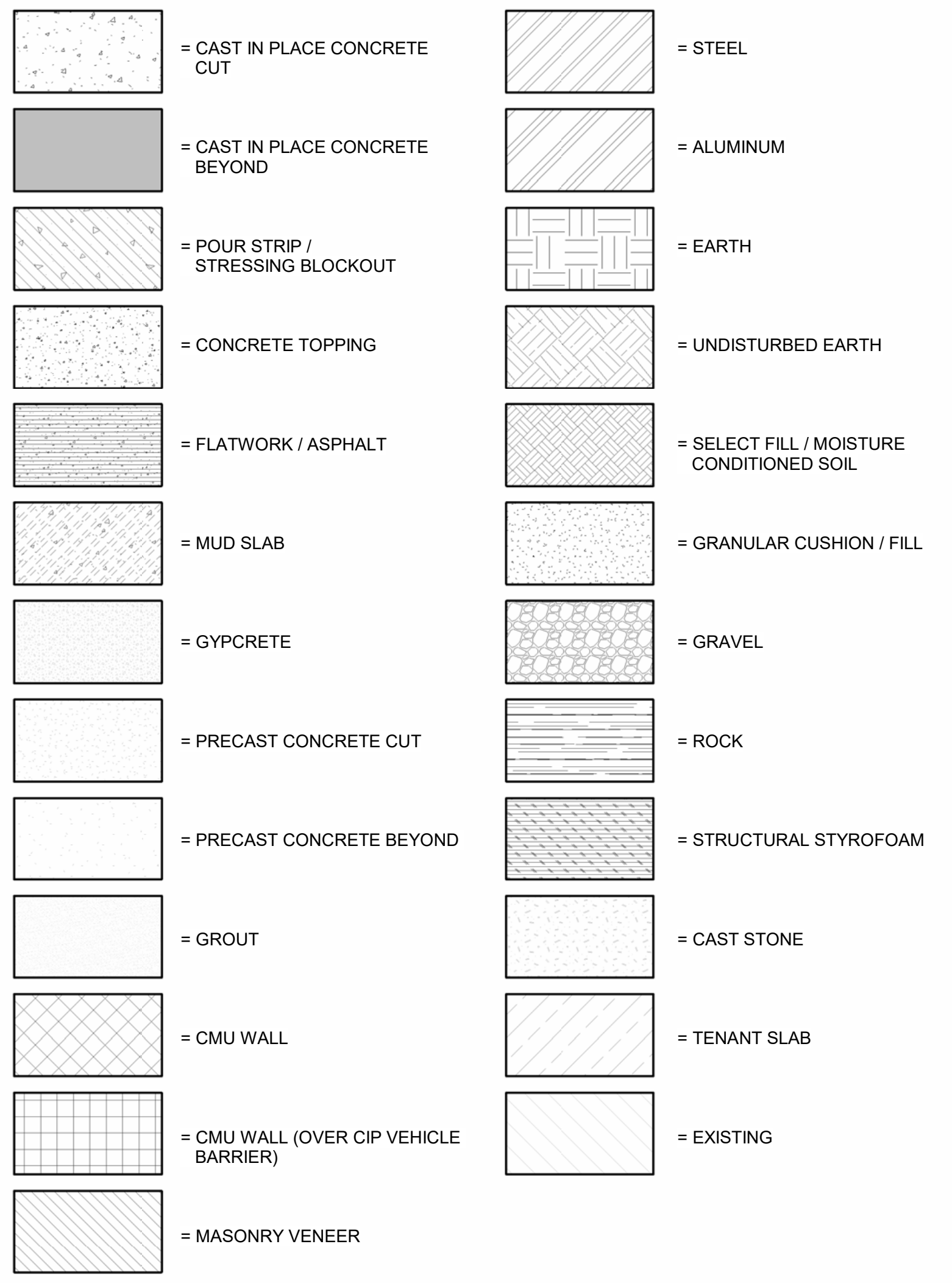
GN1.5 DEFERRED SUBMITTALS

1. DESIGN AND DETAILING OF THE BUILDING COMPONENTS LISTED BELOW ARE DELEGATED TO THE GENERAL CONTRACTOR'S SPECIALTY ENGINEER AND SHALL BE CONSIDERED DEFERRED SUBMITTALS FOR PERMITTING PURPOSES IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE SECTION 107.3.4.1.
2. DESIGN DOCUMENTS SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE JURISDICTION OF THE PROJECT AND SUBMITTED TO THE ARCHITECT AND ENGINEER FOR REVIEW AND COORDINATION PRIOR TO SUBMISSION TO THE BUILDING OFFICIAL FOR THEIR REVIEW AND APPROVAL. DESIGN DOCUMENTS SHALL INCLUDE DETAILED DRAWINGS AS WELL AS ENGINEERING CALCULATIONS.
3. DEFERRED SUBMITTAL ITEMS INCLUDE BUT MAY NOT BE LIMITED TO THE FOLLOWING:
   A. HANDRAILS
   B. STRUCTURAL STEEL CONNECTIONS
   C. CURTAIN WALL AND WINDOW WALL SYSTEMS
   D. COLD-FORMED METAL STUD (CFMS) FRAMING (NON-LOAD BEARING WALLS, SOFFITS, ARCHITECTURAL SECTIONS, ETC.)
   E. PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES
   F. ALL OTHER SYSTEMS NOT SPECIFICALLY SHOWN IN THE STRUCTURAL DOCUMENTS
   G. ALL SITE STRUCTURES (RETAINING WALLS, FENCING, TRELLISES, LANDSCAPE AND/OR CIVIL STRUCTURES, ETC.)
   H. SPECIALTY PERFORMANCE SPECIFIED FOUNDATION SYSTEMS (RAMMED AGGREGATE PIERS, PRECAST PILES, MICROPILES, ETC.)
   I. STEEL STAIRS

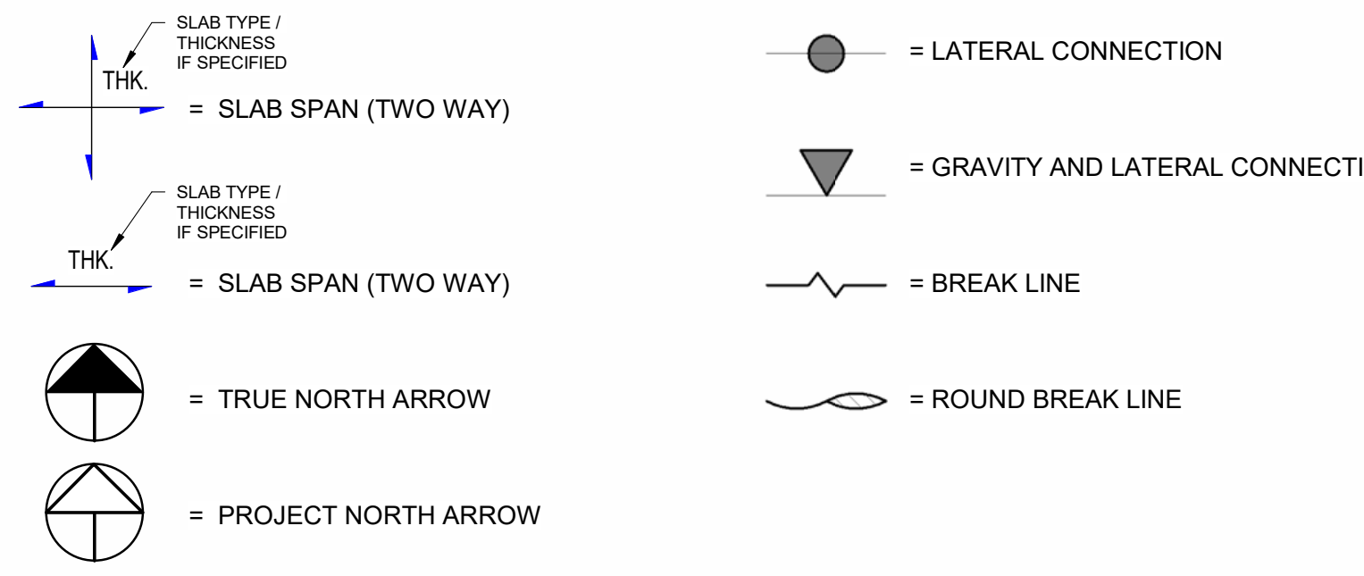
GN3.0 PLAN SYMBOLOGY:


= CAST IN PLACE CONCRETE (CIP CONC) COLUMNS / WALLS
= CMU WALL
= CMU WALL OVER CIP CONCRETE VEHICLE BARRIER
= LOAD BEARING METAL STUD FRAMED WALL
= LOAD BEARING WOOD FRAMED STUD WALL
= EXTERIOR NON-LOAD BEARING WOOD FRAMED STUD WALL
= EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
= WOOD SHEAR WALL
= PRECAST COLUMN / WALL
= TEMPORARY SHORING
EXISTING
TENANT SLAB
SLAB BEAM
POUR STRIP / STRESSING BLOCKOUT
STEEL GRATING
FORMED CONCRETE
CONCRETE TOPPING
WOOD STRUCTURE

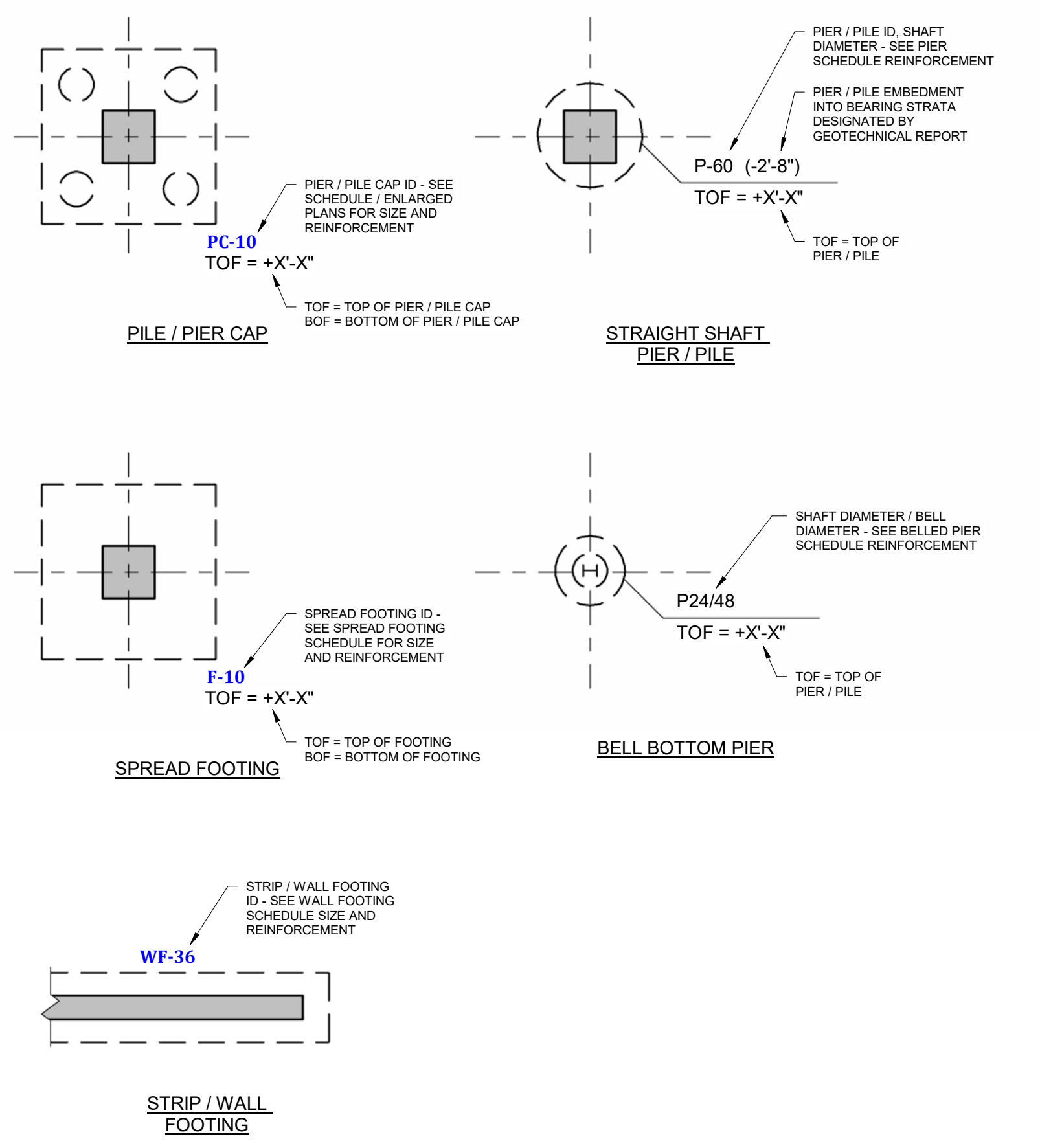
GN3.1 DETAIL / ELEVATION SYMBOLOGY:



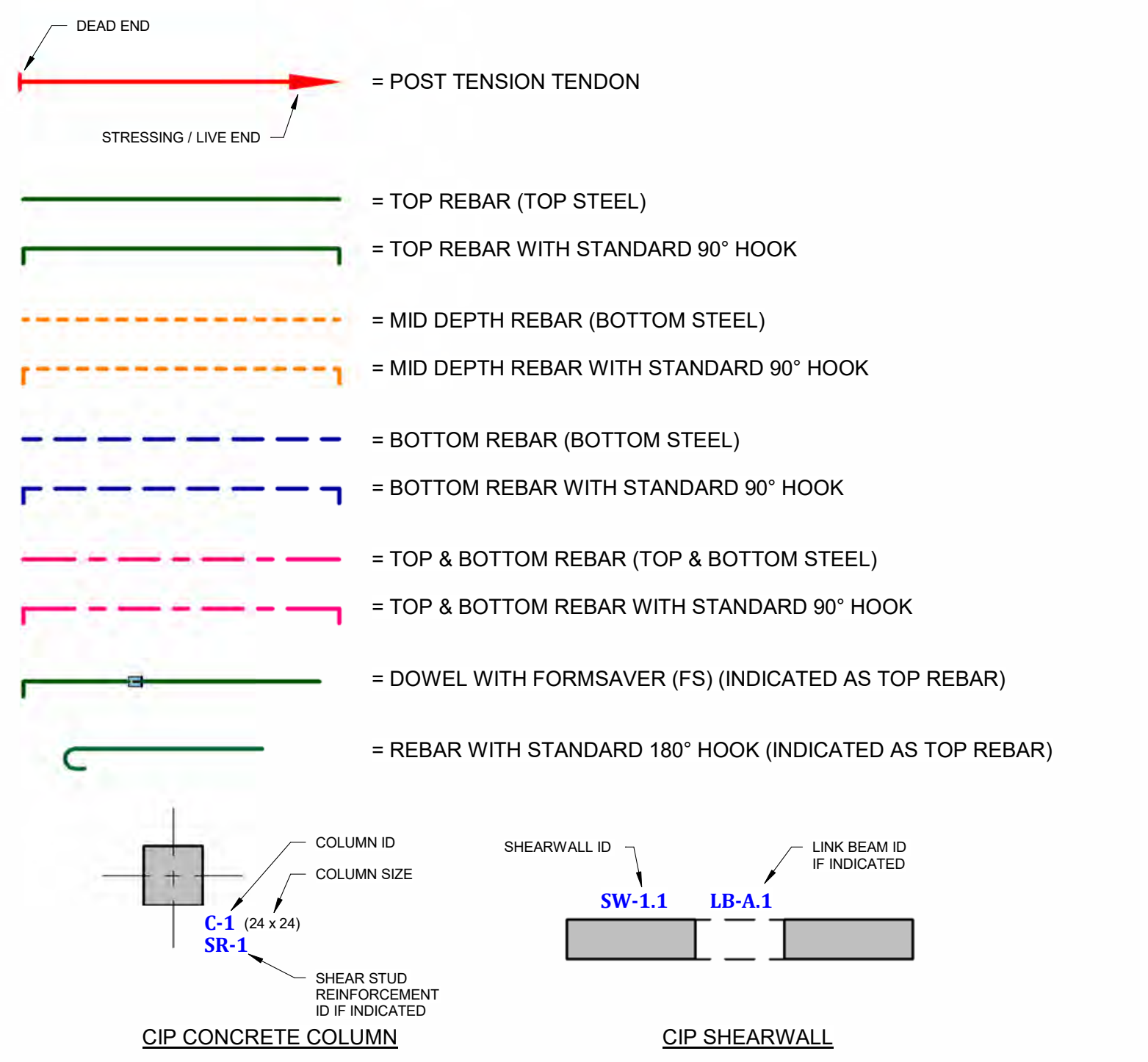
GN3.2 GENERAL SYMBOLS:



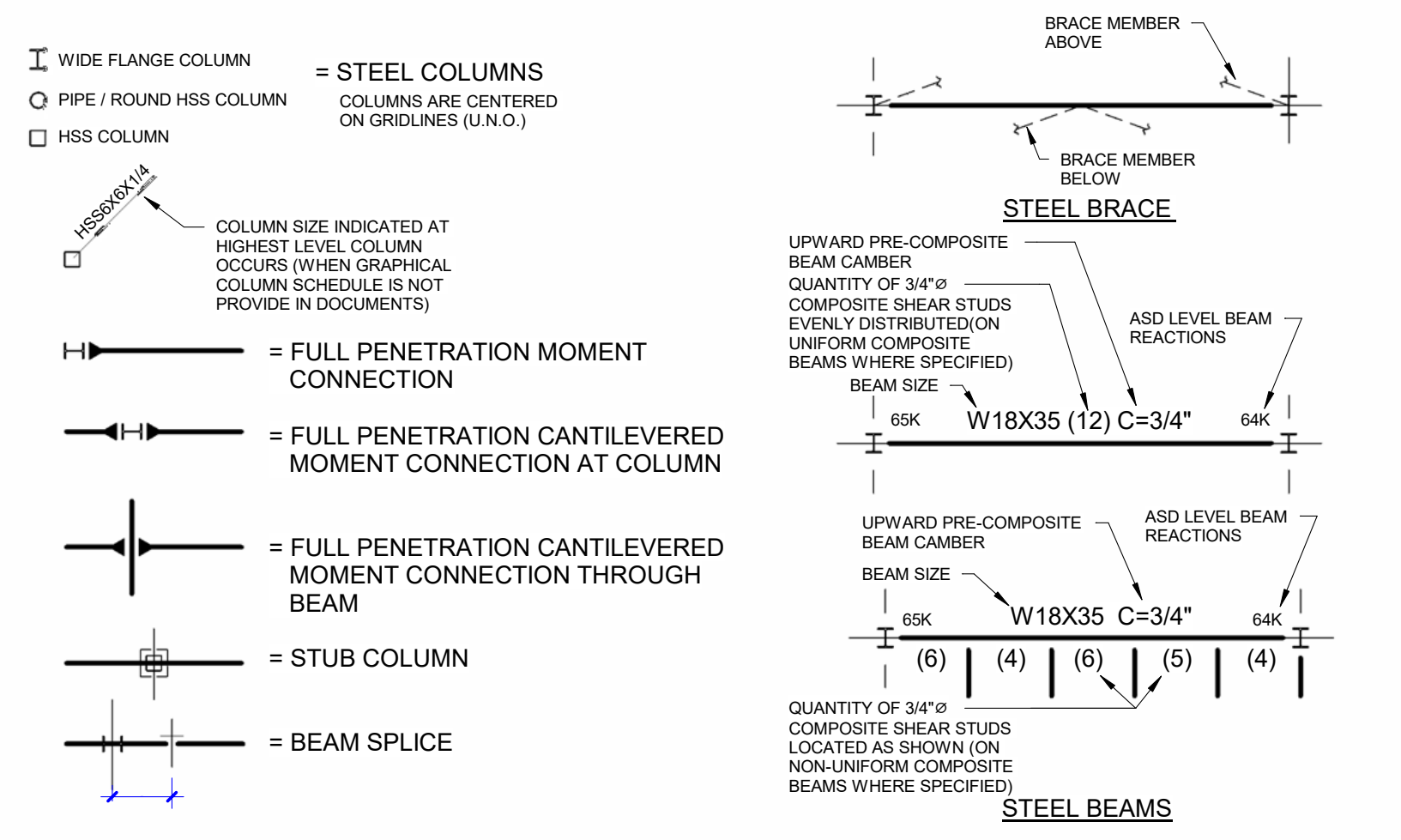
GN3.5 FOUNDATION SYMBOLS:


PIER / PILE CAP ID - SEE SCHEDULE / ENLARGED PLANS FOR SIZE AND REINFORCEMENT
PC-10
TOF = +X'-X"
TOF = TOP OF PIER / PILE CAP
BOF = BOTTOM OF PIER / PILE CAP
PILE / PIER CAP
PIER / PILE ID, SHAFT DIAMETER - SEE PIER SCHEDULE REINFORCEMENT
PIER / PILE EMBEDMENT INTO BEARING STRATA DESIGNATED BY GEOTECHNICAL REPORT
P-60 (-2'-8")
TOF = +X'-X"
TOF = TOP OF PIER / PILE
STRAIGHT SHAFT PIER / PILE
SPREAD FOOTING ID - SEE SPREAD FOOTING SCHEDULE FOR SIZE AND REINFORCEMENT
F-10
TOF = +X'-X"
TOF = TOP OF FOOTING
BOF = BOTTOM OF FOOTING
SPREAD FOOTING
SHAFT DIAMETER / BELL DIAMETER - SEE BELLED PIER SCHEDULE REINFORCEMENT
P24/48
TOF = +X'-X"
TOF = TOP OF PIER / PILE
BELL BOTTOM PIER
STRIP / WALL FOOTING ID - SEE WALL FOOTING SCHEDULE SIZE AND REINFORCEMENT
WF-36
STRIP / WALL FOOTING

GN3.3 CIP CONCRETE PLAN SYMBOLS:


DEAD END
STRESSING / LIVE END
= POST TENSION TENDON
= TOP REBAR (TOP STEEL)
= TOP REBAR WITH STANDARD 90° HOOK
= MID DEPTH REBAR (BOTTOM STEEL)
= MID DEPTH REBAR WITH STANDARD 90° HOOK
= BOTTOM REBAR (BOTTOM STEEL)
= BOTTOM REBAR WITH STANDARD 90° HOOK
= TOP & BOTTOM REBAR (TOP & BOTTOM STEEL)
= TOP & BOTTOM REBAR WITH STANDARD 90° HOOK
= DOWEL WITH FORMSAVER (FS) (INDICATED AS TOP REBAR)
= REBAR WITH STANDARD 180° HOOK (INDICATED AS TOP REBAR)
COLUMN ID
COLUMN SIZE
C-1 (24 x 24)
SR-1
SHEAR STUD REINFORCEMENT ID IF INDICATED
CIP CONCRETE COLUMN
SHEARWALL ID
LINK BEAM ID IF INDICATED
SW-1.1
LB-A.1
CIP SHEARWALL

GN3.4 STEEL PLAN SYMBOLOGY:


WIDE FLANGE COLUMN
PIPE / ROUND HSS COLUMN
HSS COLUMN
= STEEL COLUMNS
COLUMNS ARE CENTERED ON GRIDLINES (U.N.O.)
HSS6X6X1/4
COLUMN SIZE INDICATED AT HIGHEST LEVEL COLUMN OCCURS (WHEN GRAPHICAL COLUMN SCHEDULE IS NOT PROVIDE IN DOCUMENTS)
= FULL PENETRATION MOMENT CONNECTION
= FULL PENETRATION CANTILEVERED MOMENT CONNECTION AT COLUMN
= FULL PENETRATION CANTILEVERED MOMENT CONNECTION THROUGH BEAM
= STUB COLUMN
= BEAM SPLICE
BRACE MEMBER ABOVE
BRACE MEMBER BELOW
STEEL BRACE
UPWARD PRE-COMPOSITE BEAM CAMBER
QUANTITY OF 3/4"⌀ COMPOSITE SHEAR STUDS EVENLY DISTRIBUTED(ON UNIFORM COMPOSITE BEAMS WHERE SPECIFIED)
ASD LEVEL BEAM REACTIONS
BEAM SIZE
65K
W18X35 (12) C=3/4"
64K
UPWARD PRE-COMPOSITE BEAM CAMBER
ASD LEVEL BEAM REACTIONS
BEAM SIZE
65K
W18X35 C=3/4"
64K
(6)
(4)
(6)
(5)
(4)
QUANTITY OF 3/4"⌀ COMPOSITE SHEAR STUDS LOCATED AS SHOWN (ON NON-UNIFORM COMPOSITE BEAMS WHERE SPECIFIED)
STEEL BEAMS

VSM²

PROJECT # :	26154
DRAWN BY:	EG
CHECKED BY:	QB

NILES BOLTON ASSOCIATES

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No.	Description	Date
	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

LV COLLECTIVE

DOCUMENTS ARE NOT TO BE USED FOR PRICING OR CONSTRUCTION PURPOSES. DOCUMENTS ARE ISSUED FOR THE PURPOSES OF:
- 100% DD

DOCUMENTS ARE ISSUED UNDER THE AUTHORITY OF:
- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
General Notes

SHEET NUMBER:
S0.100

NOT RELEASED FOR CONSTRUCTION

9/30/2026 7:30:00 PM

GN5.0 SPECIAL INSPECTIONS:

1. ALL ITEMS NOTED IN TABLE BELOW AS REQUIRED SHALL BE INSPECTED IN ACCORDANCE WITH IBC SECTION 1704 BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED TESTING AGENCY.
2. ALL ITEMS TO BE TESTED AT THE INTERVALS INDICATED IN THE TABLE BELOW UNDER TESTING.
3. THE TESTING AGENCY SHALL PROVIDE COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS TO THE EOR, ARCHITECT, OWNER, BUILDING DEPARTMENT (IF REQUIRED), AND CONTRACTOR.
4. ANY AND ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OFR THE CONTRACTOR FOR CORRECTION. ALL ITEMS LEFT UNCORRECTED SHALL BE BROUGHT TO THE ATTENTION OF THE EOR, ARCHITECT AND BUILDING DEPARTMENT (IF REQUIRED)
5. UPON COMPLETION OF THE PROJECT, THE TESTING AGENCY SHALL PROVIDE EOR, ARCHITECT, AND OWNER A STATEMENT SIGNED AND DATED BY A LICENSED PROFESSIONAL ENGINEER, THAT TO THE BEST OF THEIR KNOWLEDGE, ALL REQUIRED INSPECTIONS WHERE MADE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
6. FOR MATERIAL SAMPLING / TESTING REQUIREMENTS, REFER TO **GN5.1 MATERIAL SAMPLING** TABLE BELOW.

ITEM	REQUIRED	TESTING
FOUNDATIONS:		
GRADING, EXCAVATION, AND FILL	●	BY GEOTECHNICAL EOR
FOOTINGS AND PIER / PILE CAPS	●	CONTINUOUS
DRILLED PIERS / PILES		CONTINUOUS
CONCRETE:		
VERIFY USE OF PROPER MIX DESIGN	●	PERIODIC
BATCH PLANT INSPECTION	●	PERIODIC
REBAR PLACEMENT	●	FINAL PLACEMENT
REBAR WELDING	●	CONTINUOUS
ANCHOR BOLTS AND EMBED PLATES	●	FINAL PLACEMENT
POST - TENSION REINFOCEMENT PLACEMENT	●	FINAL PLACEMENT
POST - TENSION STRESSING AND ENCAPSULATION SYSTEM GREASE CAP INSTALLATION	●	CONTINUOUS
CONCRETE PLACEMENT	●	CONTINUOUS
SHOTCRETE PLACEMENT		CONTINUOUS
EPOXY	●	PERIODIC
POST-INSTALLED ANCHORS (MECHANICAL / ADHESIVE)	●	PERIODIC
PRE-CAST CONCRETE ERECTION		PERIODIC
MASONRY:		
PROTECTION OF MASONRY WHEN TEMPERATURES EXCEED 90° AND BELOW 40°	●	PERIODIC
VERIFICATION OF PROPORTIONS OF SITE PREPARED MORTAR	●	PERIODIC
SIZE AND LOCATION OF STRUCTURAL ELEMENTS	●	PERIODIC
SIZE, GRADE, AND TYPE OF REBAR	●	PERIODIC
VERIFICATION OF MORTOR JOINTS	●	PERIODIC
ANCHORS SIZE, TYPE, AND LOCATION	●	PERIODIC
VERIFICATION OF CLEAN GROUT SPACE PRIOR TO REINF AND GROUT PLACEMENT	●	PERIODIC
REBAR WELDING	●	PERIODIC
REBAR PLACEMENT	●	PERIODIC
GROUT PLACEMENT	●	PERIODIC
LOCATION OF REBAR AND CONNECTORS VERIFICATION	●	PERIODIC
COMPLIANCE WITH REQ'D INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPOVED SUBMITTALS VERIFIED	●	PERIODIC
LIGHT GAUGE FRAMING:		
SHOP AND FIELD WELDING		PERIODIC
STUD PLACEMENT		PERIODIC
STRUCTURAL STEEL:		
SHOP WELDING - FILLET WELDS	●	PERIODIC
SHOP WELDING - PARTIAL OR FULL PENETRATION WELDS	●	CONTINUOUS
FIELD WELDING - FILLET WELDS	●	PERIODIC
FIELD WELDING - PARTIAL OR FULL PENETRATION WELDS	●	CONTINUOUS
BOLTED CONNECTIONS	●	PERIODIC
STRUCTURAL WOOD FRAMING:		
NAILING AND BOLTING	●	PERIODIC
UPLIFT ANCHORS AND HOLD-DOWNS	●	PERIODIC
SHEARWALL, DIAPHRAGMS, DRAG STRUTS, AND BRACES	●	PERIODIC

GN5.1 MATERIAL SAMPLING:

1. ALL ITEMS NOTED IN TABLE BELOW AS REQUIRED SHALL BE SAMPLED AND TESTED BY A CERTIFIED INSPECTOR FROM AN ESTABLISHED TESTING AGENCY IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, GENERAL NOTES OR PREVAILING BUILDING CODE (WICH EVER IS MORE STRINGENT).
2. ALL MATERIAL SAMPLING AND TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM REQUIREMENTS
3. FOR ADDITIONAL INFORMATION ON MATERIAL SAMPLING AND TESTING, REFERS TO PROJECT SPECIFICATIONS AND GENERAL NOTES.
4. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING REPORTS DIRECTLY TO THE STRUCTURAL EOR, ARCHITECT, OWNER, BUILDING DEPARTMENT (IF REQUIRED), AND CONTRACTOR.
5. ANY MATERIALS THAT FAIL TO MEET THE PROJECT SPECIFICATIONS SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE STRUCTURAL EOR.

ITEM	REQUIRED	TESTING
CONCRETE:		
DESIGN MIX	●	CONTINUOUS
CYLINDERS	●	AS REQD BY SPECIFICATIONS
SAMPLING FRESH CONCRETE AND PERFORMING SLUMP TESTING, AIR CONTENT TESTING, DETERMING THE TEMERATURE AT THE FRESH CONCRETE AT THE TIME OF MACKING SPECIMENS FOR STRENGTH TESTS.	●	CONTINUOUS
REBAR	●	MILL CERTIFICATES
POST - TENSION TENDONS	●	TWO 5'-0" MIN. SAMPLES PER REEL. (ONE FROM EACH END
MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND CURING TECHNIQUES	●	PERIODIC
VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF POST - TENSION TENDONS IN POSITIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	●	CONTINUOUS
LARGE CONCRETE MAT TEMPERATURE MONITORING DURING INITIAL CURING	●	MONITOR TEMPERATURE DURING CURING
POST-INSTALLED ANCHORS (MECHANICAL / ADHESIVE)	●	COORD W/STRUCTURAL EOR FOR FREQUENCY OF TESTING
MASONRY:		
PREPERATION OF ANY REQD GROUP SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	●	PERIODIC
STRUCTURAL STEEL:		
ULTRASONIC TESTING	●	-
BEND TEST ON WELDED STUDS	●	-

GN6.0 MASONRY VENEER LOOSE LINTEL SCHEDULE:

LOOSE STEEL LINTEL SCHEDULE - MASONRY VENEER [1]

MAXIMUM CLEAR SPAN[4] (FT)	MIN. LINTEL[2] SIZE (LLV)	MINIMUM BEARING LENGTH[3]
4'-0"	L3 1/2 x 3 1/2 x 1/4	4"
6'-0"	L4 x 3 1/2 x 1/4	4"
8'-0"	L4 x 3 1/2 x 3/8	8"
10'-0"	L6 x 3 1/2 x 3/8	8"
12'-0"	L8 x 4 x 7/16	12"

LINTEL SCHEDULE NOTES:

1. SCHEDULED LINTELS SHALL BE USED WITH 4" NOMINAL WIDTH BRICK MASONRY ONLY.
2. ALL STEEL LINTELS SHALL BE HOT DIPPED GALVANIZED, INSTALLED LONG LEG VERTICAL (LLV).
3. MINIMUM BEARING LENGTH MEASURED FROM FACE OF OPENING TO END OF LINTEL.
4. MAXIMUM CLEAR SPAN MEASURED FACE TO FACE OF SUPPORTED OPENING.
5. PROVIDE GALVANIZED BRICK TIES @ 24" O.C. MAX. WITH (1) TIE MIN. PER 2.67 SQ FEET OF WALL AREA.
6. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND SIZE OF BRICK EXPANSION JOINTS WHERE OCCURS, TYP.
7. PROVIDE WEEPHOLES @ 24" O.C. MAX. IN HEAD JOINTS IMMEDIATELY ABOVE FLASHING. REFER TO ARCHITECTURAL DRAWINGS FOR MORE INFO.
8. REFER TO PROJECT SPECIFICATIONS FOR ALL OTHER INFORMATION, TYP.
9. ALL OUT OF PLANE LATERAL FORCES RESISTED BY BACKUP WALL FRAMING, SEE PLANS FOR MORE INFORMATION.
10. BRICK SHALL NOT OVERHANG THE TOE OF THE STEEL ANGLE BY MORE THAN 1 INCH.

ST1.0 STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL SHALL CONFORM TO THE AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC 303 “CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES”, LATEST APPROVED EDITIONS REFERENCED IN GOVERNING BUILDING CODE.
2. THE STRUCTURAL DRAWINGS REPRESENT TYPICAL FINAL CONDITIONS. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL REQUIRED ERECTION AIDS, TEMPORARY BRACING, AND ENSURING THAT FABRICATION AND ERECTION OF STRUCTURAL STEEL COMPLIES WITH APPLICABLE OSHA REQUIREMENTS.
3. GENERAL CONTRACTOR SHALL SUBMIT DETAILED SHOP AND INSTALLATION DRAWINGS SHOWING SHOP AND ERECTION DETAILS INCLUDING MEMBER SIZES, GRADES OF MATERIALS, DETAILS OF FABRICATION AND ERECTION, AND MEMBER CONNECTIONS. PROCUREMENT AND/OR FABRICATION OF MATERIAL PRIOR TO SHOP DRAWINGS APPROVAL SHALL BE AT THE CONTRACTOR’S RISK.
4. MEMBER CONNECTIONS SHALL BE DELEGATED AS NOTED IN “STRUCTURAL STEEL CONNECTION NOTES”.
5. MEMBER CONNECTIONS SHALL BE FULLY COMPLETED AND INSPECTED PRIOR TO CASTING CONCRETE SLAB.
6. MEMBER SPLICES, PENETRATIONS, AND OTHER MODIFICATIONS OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT SPECIFIC APPROVAL FROM STRUCTURAL ENGINEER FOR EACH LOCATION AND CONNECTION TYPE UNLESS NOTED OTHERWISE IN THE STRUCTURAL DRAWINGS.
7. VERIFY THE SIZE AND LOCATION OF FLOOR AND ROOF OPENINGS FOR MECHANICAL AND ELECTRICAL REQUIREMENTS AND COORDINATE WITH MECHANICAL AND ELECTRICAL CONTRACTOR PRIOR TO FABRICATION OF MATERIALS.
8. STRUCTURAL STEEL MATERIALS SHALL BE NEW STEEL AND CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS U.N.O.:
   - WIDE FLANGES________ASTM A992 Gr. 50
   - OTHER ROLLED SHAPES, PLATES, BARS________ASTM A36
   - PIPE________ASTM A53 Gr. B
   - HOLLOW STRUCTURAL SECTIONS -HSS________ASTM A500 Gr. C
   - BOLTS________ASTM F3125 Grade A325 or A490
   - ANCHOR BOLTS________ASTM F1554 Grade 36
   - HIGH STRENGTH ANCHOR BOLTS________ASTM F1554 Grade 55 (S1) or 105
   - WELDING ELECTRODES________E70XX
   - STUD SHEAR CONNECTORS________ASTM A108 & AWS D1.1 TYPE B (NELSON FLUX FILLED OR APPROVED EQUIVALENT)
   - DEFORMED BAR ANCHORS________ASTM A1064 Grade 70 (NELSON D2L OR APPROVED EQUIVALENT)
9. STEEL THAT IS NOT EXPOSED TO VIEW SHALL BE CLEANED PER SSPC-SP2 AND PAINTED WITH ONE SHOP COAT OF RED OXIDE PRIMER, MINIMUM OF 1.5 MILS (DRY FILM THICKNESS). DO NOT PAINT STRUCTURAL STEEL AND ANCHOR RODS THAT ARE TO BE EMBEDDED IN CONCRETE OR TO RECEIVE FIREPROOFING. STEEL AT OR BELOW FINISHED GRADE OR BELOW FLOOR SLAB SHALL RECEIVE 2 COATS OF BITUMINOUS PAINT OR 3” MINIMUM CONCRETE COVER. STRUCTURAL STEEL SHALL BE TOUCHED UP AFTER ERECTION WITH PRIMER/PAINT TO MATCH SHOP PREP.
10. EXPOSED TO VIEW STEEL FINISH AND QUALITY STANDARD NOTED IN THESE DOCUMENTS ARE DEFINED BASED ON AISC 303-16 INCLUDING AESS1, AESS2, AESS3 AND AESS4. REFER TO THE DRAWINGS FOR SPECIFIC MEMBERS REQUIRED TO MEET ONE OF THESE STANDARDS AND REFER TO AISC 303-16 FOR ADDITIONAL INFORMATION. IF ARCHITECTURALLY EXPOSED STRUCTURAL STEEL STANDARDS NOTED IN THESE DOCUMENTS DIFFERS FROM THOSE NOTED IN THE ARCHITECTURAL DOCUMENTS, THE MORE STRINGENT STANDARD SHALL BE FOLLOWED.
11. ALL STRUCTURAL STEEL THAT WILL BE PERMANENTLY EXPOSED TO WEATHER SHALL BE PROTECTED AGAINST CORROSION IN ACCORDANCE WITH THE REQUIREMENTS OF AISC 360 AND THE PROJECT SPECIFICATIONS. PROTECTIVE COATINGS SHALL BE SELECTED AND APPLIED TO PROVIDE A DURABLE BARRIER CONSISTENT WITH THE ANTICIPATED SERVICE ENVIRONMENT. HOT-DIP GALVANIZING IN ACCORDANCE WITH ASTM A123 OR OTHER APPROVED PROTECTIVE COATING SYSTEMS IS THE PREFERRED METHOD FOR LONG-TERM CORROSION PROTECTION OF EXPOSED STEEL. WHERE STRUCTURAL STEEL IS EXPOSED IN THE FIELD DUE TO ERECTION PROCEDURES, HANDLING, OR DAMAGE TO THE ORIGINAL COATING, FIELD-APPLIED GALVANIZATION SHALL BE PERFORMED IN ACCORDANCE WITH ASTM A780. ACCEPTABLE REPAIR METHODS INCLUDE ZINC-RICH PAINT, ZINC SOLDER, OR ZINC THERMAL SPRAY, WITH THE COATING THICKNESS RESTORED TO MATCH THE SURROUNDING GALVANIZED COATING. ALL FIELD-COATED AREAS SHALL BE PREPARED BY CLEANING TO BARE METAL AND APPLIED IN STRICT ACCORDANCE WITH THE COATING MANUFACTURER’S RECOMMENDATIONS TO ENSURE CONTINUITY OF CORROSION PROTECTION.
12. THE CONTRACTOR SHALL REVIEW SHOP AND FIELD WELD REQUIREMENTS FOR COMPATIBILITY WITH THE CONSTRUCTION SEQUENCE. PROPOSED REVISIONS FROM SHOP TO FIELD WELDS OR FROM FIELD TO SHOP WELDS SHALL BE IDENTIFIED BY THE CONTRACTOR ON THE SHOP DRAWINGS.
13. ANCHOR RODS TO BE EMBEDDED IN CONCRETE SHALL BE SET USING A STEEL TEMPLATE THAT ENSURES BOLTS MAINTAIN PROPER POSITIONING DURING CONCRETE PLACEMENT. ANCHOR RODS THAT ARE OMITTED OR MISPLACED MAY NOT BE SUITABLE FOR POST INSTALLED SUBSTITUTES AND MAY REQUIRE DEMOLITION AND REPLACEMENT OF CONCRETE SECTION.
14. GROUT FOR LEVELING OF BASE PLATES SHALL BE NON-SHRINK, NON METALLIC, AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 8,000 PSI AT 28 DAYS AND CONFORM TO ASTM C1107 U.N.O..
15. MECHANICAL UNITS (SUCH AS A/C UNITS, HEATER UNITS, ETC.) ARE NOT TO BE HUNG FROM STRUCTURE WITHOUT THE ENGINEER'S APPROVAL, UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
16. EDGE ANGLES SUPPORTING FLOOR AND ROOF DECK SHALL BE SPLICED ONLY OVER SUPPORTS.
17. FABRICATE STEEL BEAMS WITH CAMBER UP U.N.O..
18. GENERAL CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER IN WRITING OF FABRICATION OR ERECTION ERRORS AND RECEIVE WRITTEN APPROVAL BEFORE TAKING CORRECTIVE ACTION INVOLVING MODIFICATIONS TO STRUCTURAL MEMBERS OR FRAMING.
19. GENERAL CONTRACTOR SHALL PROVIDE STRCUTURAL STEEL AS SHOWN IN THE FULL SET OF PROJECT CONTRACT DOCUMENTS (INCLUDING ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, ETC...). DISCREPANCIES BETWEEN THE STRUCTURAL DRAWINGS AND OTHER CONSULTANTS’ SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO FABRICATION.

ST1.1 STRUCTURAL STEEL CONNECTION NOTES:

1. STEEL MEMBER END CONNECTION DESIGN ARE DELEGATED TO THE GENERAL CONTRACTOR OR THE GENERAL CONTRACTOR'S CHOSEN STEEL FABRICATOR PER SECTION "GN1.5 - DEFERRED SUBMITTALS" AND AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES". MEMBER END CONNECTION DETAILS SHOWN IN THE STRUCTURAL DRAWINGS ARE GENERALIZED TO CONVEY FRAMING INTENT ONLY, AND ARE NOT MEANT FOR FABRICATION UNLESS SPECIFICALLY NOTED AS FULLY DESIGNED.
2. CONNECTIONS, SPLICES AND ERECTION PIECES SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR’S STRUCTURAL ENGINEER LICENSED IN THE JURISDICTION OF THE PROJECT UNLESS INDICATED AS BEING FULLY DESIGNED ON THE STRUCTURAL DRAWINGS. THIS INCLUDES THE DESIGN, DETAIL, FURNISHING AND INSTALLATION OF STIFFENERS, CONTINUITY PLATES, DOUBLER PLATES, OR OTHER NECESSARY ADDITIONAL PARTS AS REQUIRED FOR LOCAL MEMBER STRENGTHENING. SHOP DRAWINGS AND CORRESPONDING CALCULATIONS, BOTH BEARING THE CONNECTION ENGINEER’S SEAL AND SIGNATURE, SHALL BE SUBMITTED FOR REVIEW FOR GENERAL CONFORMANCE WITH CONTRACT DOCUMENTS PRIOR TO FABRICATION.
3. SHOP AND FIELD CONNECTIONS SHALL BE MADE WITH HIGH STRENGTH BOLTS OR WELDS. HIGH STRENGTH BOLTS AND NUTS SHALL BE CLEARLY MARKED AS REQUIRED BY AISC SPECIFICATIONS. BOLTS SHALL BE A MINIMUM DIAMETER OF ¾” U.N.O.. FILLET WELD SIZES SHALL BE 1/16” SMALLER THAN THICKNESS OF THINNER MATERIAL BEING JOINED U.N.O..
4. DESIGN SIMPLE SHEAR GRAVITY CONNECTIONS FOR FORCES INDICATED ON THE DRAWINGS. CONNECTION DESIGN FORCES INDICATED ON THE DRAWINGS ARE UNFACTORED U.N.O..
   - FOR NON-COMPOSITE BEAM CONNECTIONS WHERE REACTIONS ARE OMITTED FROM THE DRAWINGS, DESIGN EACH END CONNECTION FOR 55% OF THE MAXIMUM TOTAL UNIFORM LOAD AS DEFINED IN THE AISC STEEL CONSTRUCTION MANUAL, TABLE 3-6.
   - FOR COMPOSITE BEAM CONNECTIONS WHERE REACTIONS ARE OMITTED FROM THE DRAWINGS, DESIGN EACH END CONNECTION FOR 100% OF THE MAXIMUM TOTAL UNIFORM LOAD AS DEFINED IN THE AISC STEEL CONSTRUCTION MANUAL, TABLE 3-6 FOR BEAMS WITH WEIGHT EQUAL TO OR LESS THAN 40 LBS/FT, AND 85% OF THE MAXIMUM TOTAL UNIFORM LOAD FOR BEAMS LARGER THAN 40 LBS/FT.
   - FOR MOMENT CONNECTIONS WHERE MOMENT REACTION IS OMITTED FROM THE DRAWINGS, DESIGN FOR THE FULL PLASTIC MOMENT CAPACITY OF THE BEAM IN ADDITION TO THE SHEAR VALUES NOTED ABOVE.
5. BRACE AND / OR TRUSS CONNECTIONS SHALL BE DESIGNED TO DEVELOP BOTH THE MAXIMUM TENSILE AND COMPRESSIVE CAPACITY OF THE MEMBER SIZES SHOWN, UNLESS CONNECTION DESIGN FORCES ARE SPECIFICALLY NOTED IN THE STRUCTURAL DRAWINGS. BRACE AND / OR TRUSS DESIGN FORCES PROVIDED ARE UNFACTORED U.N.O..
6. STEEL MEMBERS 6” DEEP AND GREATER SHALL HAVE NO CONNECTION CONSIST OF LESS THAN (2) ¾” DIA. A325-N BOLTS OR WELDS DEVELOPING LESS THAN 10 KIPS ALLOWABLE CAPACITY. MINIMUM WELD SIZE SHALL BE A 3/16” FILLET WELD, OR MIN. PER ACIS D1.1, WHICHEVER IS LARGER.
7. MEMBER WORK POINTS FOR CONNECTION DESIGN AND DETAILING OCCUR AT INTERSECTIONS OF MEMBER CENTERLINES, U.N.O..
8. OVERSIZED OR SLOTTED BOLT HOLES SHALL NOT BE USEDFOR CONNECTIONS UNLESS SPECIFICALLY INDICATED IN THE STRUCTURAL CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
9. CONNECTIONS OCCURING THROUGH EXPANSION JOINTS SHALL PROVIDE FREE MOVEMENT AS INDICATED IN THE STRUCTURAL DETAILS. BOLTS IN SLOTTED HOLES SHALL BE UTILIZED AND SHALL BE NO MORE THAN FINGER TIGHTENED WITH THREADS CRIMPED U.N.O..
10. SLIP CRITICAL BOLTS AND BOLTS SUBJECT TO DIRECT TENSION SHALL UTILIZE DIRECT TENSION INDICATORS OR APPROVED EQUIVALENT TO ENSURE CORRECT PRE-TENSION IS APPLIED PER AISC “MANUAL OF STEEL CONSTRUCTION”.
11. SHOP AND FIELD CONNECTIONS ARE TO BE INSPECTED BY AN INDEPENDENT TESTING LABORATORYFOR PROPER MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH SECTION "GN5.0 - SPECIAL INSPECTIONS" AND THE GOVERNING BUILDING CODE.
12. BOLTED BEAM CONNECTIONS SHALL USE ASTM F3125 GR. A325 OR A490 BOLTS AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS". SIMPLE FRAMED SHEAR CONNECTIONS SHALL CONSIST OF "SNUG-TIGHTENED JOINTS" WHICH ENSURE THAT THE PLIES OF CONNECTED MATERIAL HAVE BEEN BROUGHT INTO FIRM CONTACT AND SHALL BE INSTALLED AND VISUALLY INSPECTED PER SECTIONS 8.1 AND 9.1 RESPECTIVELY OF THE "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS." VISUAL INSPECTION OF BOLTED CONNECTIONS SHALL BE PERFORMED BY AN INDEPENDENT TESTING AGENCY. DIRECT TENSION INDICATOR (DTI) BOLTS SHALL BE PERMITTED.
13. WELDING INCLUDING WELDING ELECTRODES, WELDING PROCESS, MINIMUM PREHEAT AND INTERPASS TEMPERATURES SHALL BE IN ACCORDANCE WITH THE AISC AND AWS SPECIFICATIONS. ANY STRUCTURAL STEEL DAMAGED IN WELDING IS TO BE REPLACED OR REINFORCED AS APPROVED BY THE STRUCTURAL ENGINEER. WELDERS SHALL HOLD VALID CERTIFICATES IN ACCORDANCE WITH SECTION 4 OF THE AWS D1.1 "STRUCTURAL WELDING CODE-STEEL" AND HAVE CURRENT EXPERIENCE IN THE TYPE OF WELDS AS SHOWN IN THE STRUCTURAL CONTRACT DOCUMENTS. THE STRUCTURAL ENGINEER MAY REQUEST EVIDENCE OF SUCH QUALIFICATIONS AT ANY TIME DURING THE PROJECT. WELDS SHALL BE PERFORMED USING E70XX SERIES LOW HYDROGEN RODS U.N.O.. WELDS SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY IN ACCORDANCE WITH THE AISC AND AWS SPECIFICATIONS.
14. COMPLETE JOINT PENETRATION WELDS SHALL HAVE ULTRASONIC TESTING PERFORMED BY THE OWNER’S QUALITY ASSURANCE INSPECTOR IN ACCORDANCE WITH AWS D1.1. FAILURES SHALL BE REPAIRED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
15. REMOVE WELD BACKER BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED U.N.O.
16. WELD LENGTHS SHALL BE CONTINUOUS U.N.O..
17. GROOVE WELDS NOTED IN STRUCTURAL CONTRACT DOCUMENTS SHALL BE COMPLETE JOINT PENETRATION U.N.O..
18. DIRECT CONTACT JOINTS BETWEEN TWO MEMBERS SHALL BE WELDED TO DEVELOP THE ALLOWABLE TENSILE CAPACITY OF THE SMALLER MEMBER U.N.O..

GN7.0 STRUCTURAL STYROFOAM FILL:

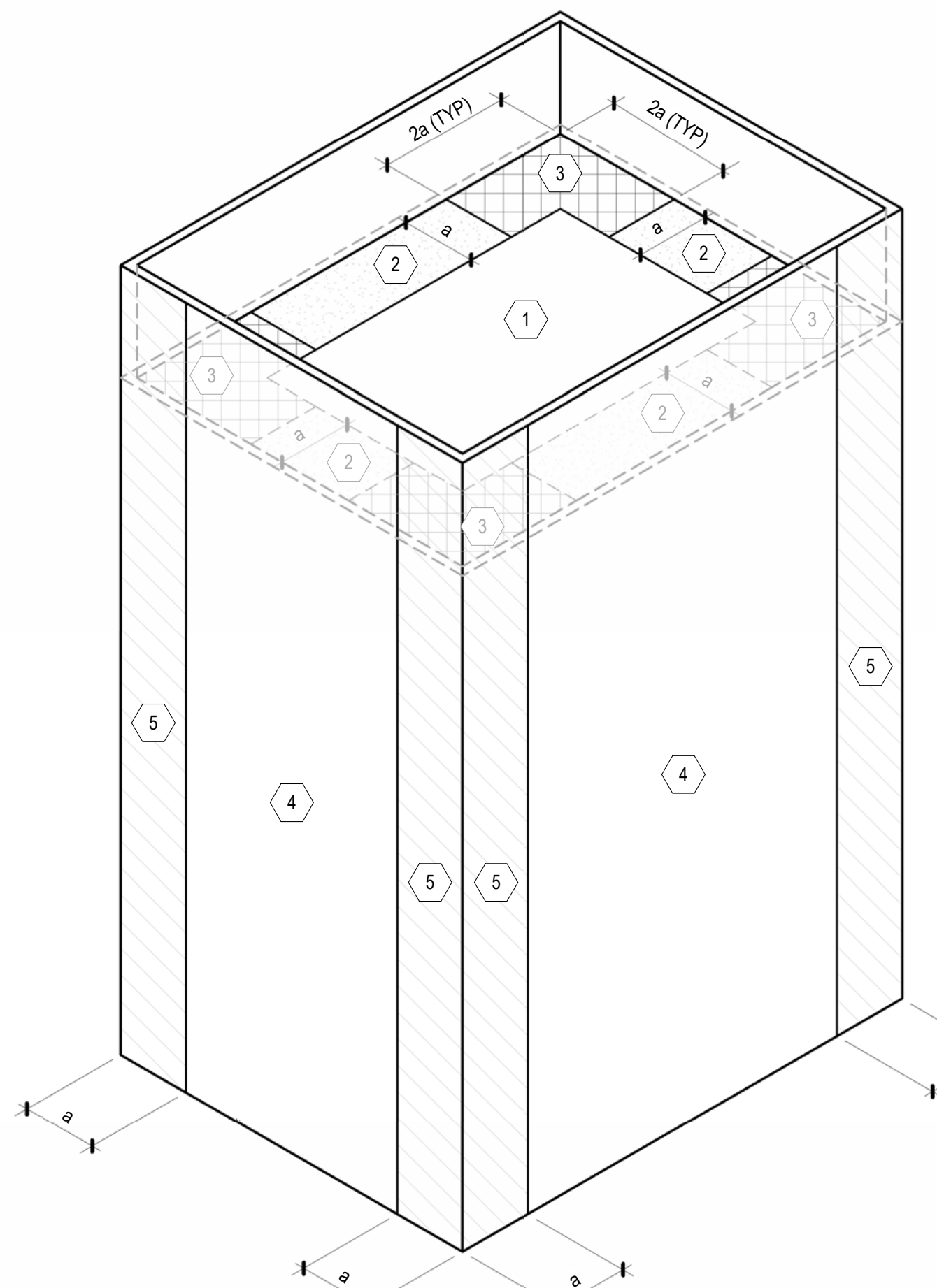
1. STRUCTURAL STYROFOAM FILL NOTED IN DRAWINGS SHALL CONSIST OF EXTRUDED POLYSTYRENE (XPS) CONFORMING TO MINIMUM PHYSICAL PROPERTY REQUIREMENTS GIVEN IN ASTM C578, TABLE 1 FOR TYPE IV CLASSIFICATION.

GN8.0 STAIR & HANDRAIL NOTES:

1. STAIRS AND HANDRAILS SHALL BE DESIGNED BY AN ENGINEER LICENSED IN THE JURISDICTION OF THE PROJECT IN ACCORDANCE WITH THE GOVERNING CODE AND THE NOTES BELOW. SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND COORDINATION PRIOR TO CONSTRUCTION.
2. SHOP DRAWINGS SHALL INCLUDE DESIGN CRITERIA, FRAMING SIZES AND DETAILING FOR ALL MEMBERS AND CONDITIONS, AND CONNECTIONS TO SUPPORT STRUCTURE.
3. CONNECTIONS TO SUPPORT STRUCTURE SHALL BE DETAILED SUCH THAT THEY DO NOT INDUCE TORSION AND/OR LOAD ECCENTRICITY TO SUPPORTING FRAMING MEMBERS. PROVIDE ADDITIONAL BRACING AS REQUIRED TO MITIGATE EFFECTS OF TORSION ON INDIVIDUAL SUPPORT MEMBERS.
4. STAIRS SHALL BE DESIGNED TO SUPPORT 100 POUNDS PER SQUARE FOOT LOAD OVER ENTIRE AREA. TREADS SHALL ALSO BE DESIGNED TO SUPPORT 300 POUNDS CONCENTRATED LOAD LOCATED SO AS TO PRODUCE THE MAXIMUM LOAD EFFECT IN THE MEMBERS.
5. HANDRAILS AND GUARDS SHALL BE DESIGNED AT A MINIMUM TO WITHSTAND A UNIFORM LOAD OF 50 PLF OR A CONCENTRATED LOAD OF 200 POUNDS APPLIED IN ANY DIRECTION SO AS TO PRODUCE THE MAXIMUM LOAD EFFECT IN THE MEMBERS. INTERMEDIATE RAILS, PANEL FILLERS, AND THEIR CONNECTIONS SHALL BE DESIGNED TO WITHSTAND A LOAD OF 50LBS ON AN AREA NOT TO EXCEED 12" X 12" APPLIED HORIZONTALLY, INCLUDING OPENINGS AND SPACES BETWEEN RAILS AND LOCATED SO AS TO PRODUCE THE MAXIMUM LOAD EFFECTS.
6. REFER TO ARCHITECTURAL DRAWINGS FOR STAIR AND HANDRAIL LAYOUT AND DIMENSIONS.

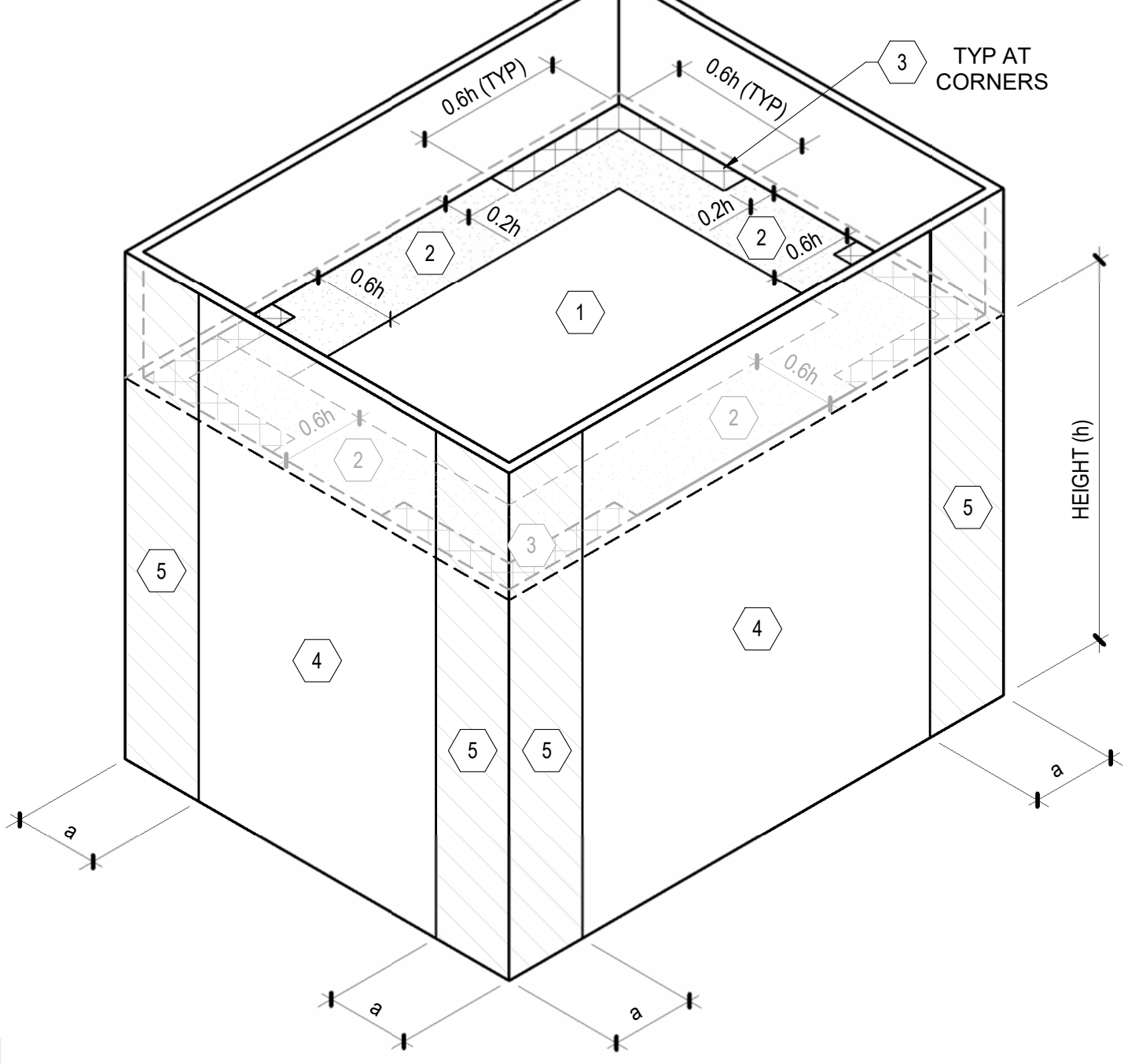
GN10.0 COMPONENT AND CLADDING FOR WIND PRESSURE DIAGRAMS:

1. PRESSURES SHOWN ARE BASED ON SERVICE LEVEL (3-SEC) WIND VELOCITY, (V_{ASD}). FOR ULTIMATE LOADS, DIVIDE PRESSURES GIVEN BY 0.6.
2. PARAPET DESIGN PRESSURES CONSIST OF APPLYING THE APPLICABLE POSITIVE PRESSURE TO ONE SIDE OF THE PARAPET AND NEGATIVE PRESSURE TO THE OPPOSITE SIDE.
3. REFER TO BUILDING CODE FOR IMPACT RESISTANT GLAZING REQUIREMENTS IN HURRICANE PRONE REGIONS.



BUILDING WITH FLAT ROOF
MEAN ROOF HEIGHT OVER 60'-0"

TRIB AREA FT²	DESIGN WIND PRESSURE - FLAT ROOFS (LOADS INDICATED ARE SERVICE LOADS)		
	ZONE 1	ZONE 2	ZONE 3
10	+___PSF - 38 PSF	+___PSF - 60 PSF	+___PSF - 81 PSF
50	+___PSF - 33 PSF	+___PSF - 53 PSF	+___PSF - 72 PSF
100	+___PSF - 31 PSF	+___PSF - 50 PSF	+___PSF - 69 PSF



BUILDING WITH FLAT ROOF
MEAN ROOF HEIGHT 60'-0" OR UNDER

TRIB AREA FT²	DESIGN WIND PRESSURE WALLS (LOADS INDICATED ARE SERVICE LOADS)	
	ZONE 4	ZONE 5
10	+ 26 PSF - 26 PSF	+ 26 PSF - 48 PSF
50	+ 25 PSF - 25 PSF	+ 24 PSF - 42 PSF
100	+ 23 PSF - 25 PSF	+ 23 PSF - 38 PSF

ZONE LENGTHS

ZONE	LENGTH
a	30 '- 0 "

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
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CC1.0 CONCRETE AND CONCRETE REINFORCEMENT:

1. STRUCTURAL CONCRETE SHALL BE LABORATORY DESIGNED AND SHALL CONFORM TO THE LATEST EDITION OF ACI 318 AND ACI 301, BY THE AMERICAN CONCRETE INSTITUTE. MIX DESIGNS FOR EACH TYPE AND STRENGTH SHALL BE ESTABLISHED BY THE SUPPLIER AND SUBMITTED FOR APPROVAL A MINIMUM OF 15 BUSINESS DAYS PRIOR TO THE COMMENCEMENT OF WORK. ADJUSTMENTS MADE TO ALREADY APPROVED MIX DESIGNS, INCLUDING CHANGES IN ADMIXTURES, SHALL BE SUBMITTED IN WRITING FOR APPROVAL PRIOR TO FIELD USE. CONCRETE DESIGNED TO BE PUMPED SHALL BE NOTED AS SUCH IN MIX DESIGNS SUBMITTED FOR REVIEW.
2. CONCRETE TO BE USED IN POST-TENSIONED CONSTRUCTION OF ELEMENTS CARRYING ONE FLOOR OF LOAD (SLABS, BEAMS, ETC.) SHALL BE PROPORTIONED TO REACH 60% (MINIMUM) OF SPECIFIED 28-DAY STRENGTH WITHIN 72 HOURS AFTER CONCRETE PLACEMENT TO ALLOW FOR TIMELY STRESSING OF TENDONS. CONCRETE TO BE USED IN POST-TENSIONED CONSTRUCTION OF ELEMENTS CARRYING TWO OR MORE FLOORS OF LOAD (PODIUMS, TRANSFER GIRDERS, ETC.) SHALL BE PROPORTIONED TO REACH 75% (MINIMUM) OF SPECIFIED 28-DAY STRENGTH WITHIN 72 HOURS AFTER CONCRETE PLACEMENT. CONTRACTOR SHALL SUBMIT HISTORICAL BREAK DATA AS NEEDED TO CONFIRM EARLY STRENGTH GAIN IN ADDITION TO DATA REQUIRED BY ACI 318, CHAPTER 5.
3. FORMWORK, SHORING AND RESHORING SHALL BE DESIGNED, ERECTED AND MAINTAINED ACCORDING TO ACI 301 AND ACI 347 TO WITHSTAND NATURAL AND CONSTRUCTION LOADS APPLIED PRIOR TO CONCRETE STRUCTURE REACHING ITS DESIGN STRENGTH. SHOP DRAWINGS SHOWING LAYOUT AND PROCEDURE OF FORMWORK, SHORING AND RESHORING, AS WELL AS CORRESPONDING CALCULATIONS, EACH BEARING THE SEAL AND SIGNATURE OF THE FORMWORK/SHORING ENGINEER LICENSED IN THE JURISDICTION OF THE PROJECT, SHALL BE SUBMITTED FOR REVIEW PRIOR TO CONSTRUCTION. RESHORING SHALL EXTEND A MINIMUM OF TWO FLOORS BELOW FLOOR SUPPORTING FALSEWORK (OR GROUND FLOOR).
4. CONCRETE REINFORCEMENT SHALL BE NEW DEFORMED BILLET STEEL AND SHALL BE FREE OF DIRT, OIL OR OTHER BOND INHIBITING MATERIAL U.N.O..
   A.
      - AND SHALL CONFORM TO ASTM A 615, GRADE 60 FOR #6 AND SMALLER, AND GRADE 75/80 FOR #7 AND LARGER. WELDABLE REINFORCING BARS SHALL CONFORM TO ASTM A706, GR. 60 U.N.O..
5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, GRADE 70.
6. EPOXY COATED REINFORCING BARS NOTED IN THE STRUCTURAL DRAWINGS SHALL CONFORM TO ASTM A775.
7. DEFORMED BAR ANCHORS NOTED IN THE STRUCTURAL DRAWINGS SHALL CONFORM TO ASTM A1064 GRADE 70 U.N.O..
8. DETAIL REINFORCING BARS AND PROVIDE BAR SUPPORTS AND SPACERS IN ACCORDANCE WITH ACI 315. SUBMIT SHOP DRAWINGS FOR APPROVAL WHICH ACCURATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT AS SPECIFIED IN THE STRCUTURAL DRAWINGS. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT CLARIFYING SECTIONS, ELEVATIONS, AND DETAILS SHALL NOT ACCEPTED.
9. REINFORCING BAR BENDS SHALL BE MADE COLD. FIELD BENDING SHALL NOT BE PERMITTED U.N.O.. NO WELDING OF REINFORCING BARS OR TORCHING TO BEND REINFORCING BARS SHALL BE PERMITTED U.N.O..
10. REINFORCING SHALL BE PROPERLY CHAIRED AND TIED PER ACI 315 (SP66) AND CRSI (PLACING REINFORCING BARS) PRIOR TO PLACING CONCRETE.
11. CONCRETE SHALL BE NORMAL WEIGHT STONE AGGREGATE CONCRETE UNLESS NOTED OTHERWISE. AGGREGATE SHALL MEET ASTM C33 REQUIREMENTS, AND SHALL BE 3/4" TO 1 1/2" NOMINAL AGGREGATE SIZE. CONCRETE ON METAL DECK IS TO UTILIZE 3/8" PEA GRAVEL COURSE AGGREGATE. PROVIDE ADMIXTURES AS REQUIRED TO IMPROVE WORKABILITY. THE GENERAL CONTRACTOR SHALL COORDINATE SLUMP REQUIREMENTS UNLESS NOTED OTHERWISE IN STRUCTURAL DOCUMENTS. PLASTIC CONCRETE TEMPERATURE SHALL NOT EXCEED 90 DEGREES PRIOR TO PLACEMENT. THE WATER:CEMENT RATIO SHALL NOT EXCEED THE RATIOS SHOWN BELOW. THE FLYASH CONTENT SHALL NOT EXCEED THE PERCENTAGE OF CEMENTITIOUS MATERIALS SHOWN BELOW.
12. CONCRETE CONSTRUCTION SHALL MEET THE FOLLOWING REQUIREMENTS U.N.O.:

STRUCTURAL ELEMENT	MIN. COVER	MIN COMPRESSIVE STRENGTH (psi)	TEST AGE (DAYS)	MAX. W/CM RATIO	MAX FLY ASH CONTENT	ELASTIC MODULUS	SHRINKAGE LIMIT	ADDITIONAL MIX NOTES
FOUNDATIONS:								
• PT SLAB ON GRADE	SEE PT SOG NOTES	5,000	56	SEE TABLE	20%%	XXXX	0.04%	3,5
• FOOTINGS	3" ALL SURFACES	4,000	28		50%	XXXX	XXXX	-
• GRADE BEAMS	1 ½" TOP / 3" ELSE	5,000	56		20%%	XXXX	XXXX	-
• NON-STRUCTURAL SLAB ON GRADE	1 ½" TOP & BOT	3,500	28		20%	XXXX	XXXX	-
• RETAINING WALL	2" EXT / 3/4" INT	4,000	28		30%	XXXX	XXXX	-
ELEVATED STRUCTURE:								
• BEAMS	1 ½" EA. FACE	5,000	56	SEE TABLE	20%	XXXX	0.04%	3,5
• COLUMNS	1 5/8" EA. FACE	5,000	56		30%	XXXX	XXXX	2,4
• SHEAR WALLS	1 ½" EA. FACE	5,000	56		30%	XXXX	XXXX	2,4
• INTERIOR SLABS	¾" TOP / 1" BOT	5,000	56		20%	XXXX	0.04%	3,5
• EXTERIOR SLABS	1 ½" TOP / 1" BOT	5,000	56		20%	XXXX	0.04%	3,5

MIX NOTES

1. AT CONTRACTOR'S OPTION, THIS MIX TO CONTAIN POLYPROPYLENE OR NYLON FIBROUS REINFOREMENT AT THE MANUFACTURE'S RECOMMENDED DOSAGE AND SHALL MATCH OR EXCEED MINIMUM BONDED STEEL REINFORCING BAR SHOWN ON DRAWINGS.
2. THIS MIX SHALL BE HIGH SLUMP OR SELF CONSOLIDATING CONCRETE (SCC) TO FACILITATE PLACEMENT.
3. THIS MIX SHALL HAVE A SHRINKAGE LIMIT OF 0.040% AT 28 DAYS. CONFIRM TO REQUIREMENTS LISTED UNDER SHRINKAGE..
4. REFERENCE ARCHITECTURAL ELEVATIONS FOR APPLICABLE LOCATIONS. SEE NOTE **CC1.3 EXPOSED CONCRETE** FOR CONCRETE REQUIREMENT.
5. THIS MIX SHALL ACHIEVE A MINIMUM TESTED COMPRESSIVE STRENGTH OF 3,000 PSI @ 3 DAYS FOR STRESSING OF PT TENDONS AND 5,000 PSI @ 56 DAYS FOR FINAL MINIMUM COMPRESSIVE STRENGTH

CC1.0 CONCRETE AND CONCRETE REINFORCEMENT (CONTINUED):

MAX. WATER / CEMENT RATION (W/CM) FOR NW CONCRETE SCHEDULE	
CONCRETE f'c	NON-AIR ENTRAINED CONCRETE
3,000 PSI	0.55
4,000 PSI	0.50
4,500 PSI	0.45
5,000 - 10,000 PSI	0.40

NOTE: CONCRETE EXPOSED TO FREEZE / THAW CYCLES AND / OR CORROSIVE ENVIORNMENTS SHALL BE AIR ENTRAINED WITH A MAXIMUM W/ CM OF 0.40 REFER TO ACI 318 TABLE 19.3.3.1 EXPOSURE F2 FOR TARGET AIR CONTENT.

13. CONCRETE SHRINKAGE SHALL BE MEASURED AND RECORDED IN ACCORDANCE WITH ASTM C157 WITH 7-DAY WATER CURE AND 21-DAY AIR DRY CURE. FINAL LENGTH CHANGE MEASUREMENT SHALL BE TAKEN 28 DAYS AFTER 7-DAY WATER CURE FOR A TOTAL 35-DAY TEST DURATION. TOTAL SHRINKAGE SHALL BE LESS THAN THE SPECIFIED SHRINKAGE LIMIT IN TABLE ABOVE AND GENERAL **NOTE CC1.2 CONCRETE SHRINKAGE**
14. CLOSURE POUR CONCRETE SHALL HAVE A SHRINKAGE LIMIT OF 0.025% AT 28 DAYS. USE 1.0 GAL / CU YD OF ECLIPSE SHRINKAGE REDUCING ADMIXTURE BY WR GRACE, OR APPROVED EQUAL ALTERNATE. (ALTERNATE TO BE SUBMITTED TO EOR FOR APPOVAL)
15. PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I OR II.
16. FLYASH SHALL CONFORM TO ASTM C618, TYPE C OR F.
17. WATER SHALL NOT BE ADDED TO THE CONCRETE AT THE JOBSITE UNLESS EXPLICITLY NOTED ON BATCH TICKET AS BEING WITHHELD AT BATCH PLANT. FINAL WATER CONTENT AT PLACEMENT SHALL NOT EXCEED WATER CONTENT SPECIFIED IN APPROVED MIX DESIGNS.
18. MECHANICAL AND ELECTRICAL CONDUIT SHALL NOT BE PLACED IN BEAMS PARALLEL TO BEAM REINFORCING. PROVIDE A MINIMUM OF 1 1/2" CLEAR BETWEEN CONDUIT AND PARALLEL REINFORCING. DO NOT "BUNDLE" CONDUITS. CONDUITS SHALL BE PLACED IN THE MIDDLE ONE THIRD OF THE SLAB THICKNESS OR BEAM DEPTH. SEE TYPICAL DETAILS FOR ADDITIONAL INFORMATION.
19. SET AND BUILD INTO FORM WORK ANCHORAGE DEVICES AND OTHER EMBEDDED ITEMS REQUIRED FOR OTHER WORK THAT IS ATTACHED TO OR SUPPORTED BY CAST-IN-PLACE CONCRETE. REBAR PROJECTING FROM CONCRETE SHALL BE SECURED IN PLACE PRIOR TO PLACING CONCRETE.
20. IF NOT SHOWN ON PLAN, THE CONTRACTOR SHALL SUBMIT A PROPOSED CONSTRUCTION JOINT LAYOUT FOR REVIEW. MAXIMUM SQUARE FOOTAGE PER POUR SHALL NOT EXCEED 15,000 UNLESS APPROVED BY ENGINEER.
21. UNLESS NOTED OTHERWISE ON PLAN, SLAB-ON-GRADE CONTROL JOINTS SHALL BE LOCATED AT APPROXIMATELY 400 SQ FT GRIDS. ACTUAL LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR FOR REVIEW AND APPROVAL PRIOR TO PLACEMENT. SEE TYPICAL DETAILS FOR ADDITIONAL INFORMATION.
22. THE OWNER'S INDEPENDENT TESTING LABORATORY SHALL VERIFY AND PROVIDE WRITTEN DOCUMENTATION OF THE FOLLOWING:
   A. BATCH TICKETS FOR EACH TRUCK MATCHES THE APPROVED MIX DESIGNS,
   B. MEASURED SLUMP CONFORMS TO THAT NOTED IN APPROVED MIX DESIGNS,
   C. PLACEMENT OPERATIONS ARE IN ACCORDANCE WITH ACI SPECIFICATIONS,
   D. CONTROL JOINTS ARE INSTALLED WITHIN ACI TIME ALLOWANCE,
   E. PROPER CURING METHODS ARE UTILIZED.
23. CONSTRUCTION VEHICLE LOADS SHALL NOT BE PERMITTED ON ELEVATED SLABS WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
24. ALUMINUM SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE, EITHER ELEVATED OR GRADE SUPPORTED.
25. HORIZONTAL CONSTRUCTION JOINTS ARE NOT PERMITTED IN ANY CONCRETE CONSTRUCTION UNLESS SPECIFICALLY NOTED IN THE STRUCTURAL DRAWINGS.
26. FLOOR CHASES, SLEEVES AND SHAFTS ARE TO BE FORMED PRIOR TO CONCRETE PLACEMENT. CORING AND SAW CUTTING OF CONCRETE AFTER PLACEMENT SHALL NOT BE PERMITTED.
27. UNDER NORMAL CONDITIONS, REINFORCED CONCRETE AND POST-TENSIONED CONCRETE DEVELOPS CRACKS. THE CRACKS ARE DUE TO THE INHERENT SHRINKAGE OF CONCRETE, CREEP, AND RESTRAINING EFFECTS OF WALLS AND OTHER STRUCTURAL ELEMENTS.
28. CURING
   A. TO CONFORM TO:
      - ACI 301, SECTION 5
      - ACI 302, GUIDE TO CONCRETE FLOOR AND SLAB CONSTRUCTION
      - ACI 308, STANDARD SPECIFICATION FOR CURING
   B. CURING PROCEDURES TO BE IMPLIMINTED IMEDIATELY TO MAINTAIN CONCRETE IN A MOIST CONDITION AFTER PLACEMENT
   C. LIQUID MEMBRANE-FORMING CURING COMPOUNDS SHALL CONFORM TO ASTM C309 TYPE 1-D. CURING COMPOUNDS APPLIED TO SURFACES INTENDED TO RECIEVE FLOOR COVERINGS, PROTECTIVE COATINGS OR SEALERS, SHALL BE COMPATIBLE WITH THOSE PRODUCTS. APPLY COMPOUNDS IN ACCORDANCE WITH MANUFACTURE RECOMMENDATIONS.
   D. POLYETHYLENE SHEETING AND BURLAP-POLYETHYLENE SHEETING SHALL CONFORM TO ASTM C171.
   E. PROVIDE CURING AS FOLLOWS:

CURING	DEFINITION
C1	IMMEDIATELY AFTER SCREEDING CONCRETE, APPLY AN EVAPORATIVE REDUCER TO ALL EXPOSED SURFACES IN ACCORDANCE WITH MANUFACTURE'S RECOMMENDATIONS
C2	IMMEDIATELY AFTER STRIPPING FORMS, APPLY A LIQUID MEMBRANE-FORMING CURING COMPOUND TO ALL EXPOSED SURFACES. ALTERNATIVELY, COVER WITH POLYETHYLENE SHEETING OR BURLAP-POLYETHYLENE SHEETING IN DIRECT CONTACT WITH CONCRETE. FOR FORMED SURFACES, THESE REQUIREMENTS MAY BE OMMITED IF FORMS REMAIN IN CONTACT WITH CONCRETE FOR 7 DAYS OR MORE.
C3	IMMEDIATELY AFTER FINISHING, APPLY A LIQUID MEMBRANE-FORMING CURING COMPOUND TO TOP SURFACE OF MAT AND COVER WITH BLANKETS FOR 7 DAYS MINIMUM.
C4	NO SPECIAL CURING PROCEDURES REQUIRED

C1 = SLAB ON GRADE AND ELEVATED SLABS
C2 = COLUMNS AND WALLS
C3 = MAT FOUNDATION
C4 = ALL OTHER CONCRETE

CC1.1 CONCRETE SAMPLING:

1. A SET OF CYLINDERS FOR COMPRESSIVE STRENGTH TESTS SHALL BE MOLDED AND LAB CURED IN ACCORDANCE WITH ASTM C 31 BY THE OWNER'S INDEPENDENT TESTING LAB AT THE FREQUENCY SPECIFIED IN THESE DRAWINGS, OR AS NOTED IN ACI 318 AND THE GOVERNING BUILDING CODE, WHICHEVER IS GREATER. EACH SET OF COMPRESSION TESTS SHALL CONSIST OF A MINIMUM OF FOUR STANDARD TEST CYLINDERS U.N.O.. COMPRESSION STRENGTH TESTS SHALL BE PERFORMED IN ACCORDANCE WITH ASTM C39 AND RESULTS SHALL BE REPORTED TO THE ENGINEER WITHIN 24 HOURS OF TESTING.
2. FIELD SAMPLES FOR STRENGTH TESTS SHALL BE TAKEN IN ACCORDANCE WITH ASTM C172 AND SHALL CONSIST OF A MINIMUM NUMBER OF STANDARD TEST CYLINDERS PER SET TAKEN AT FREQUENCY AS NOTED BELOW:

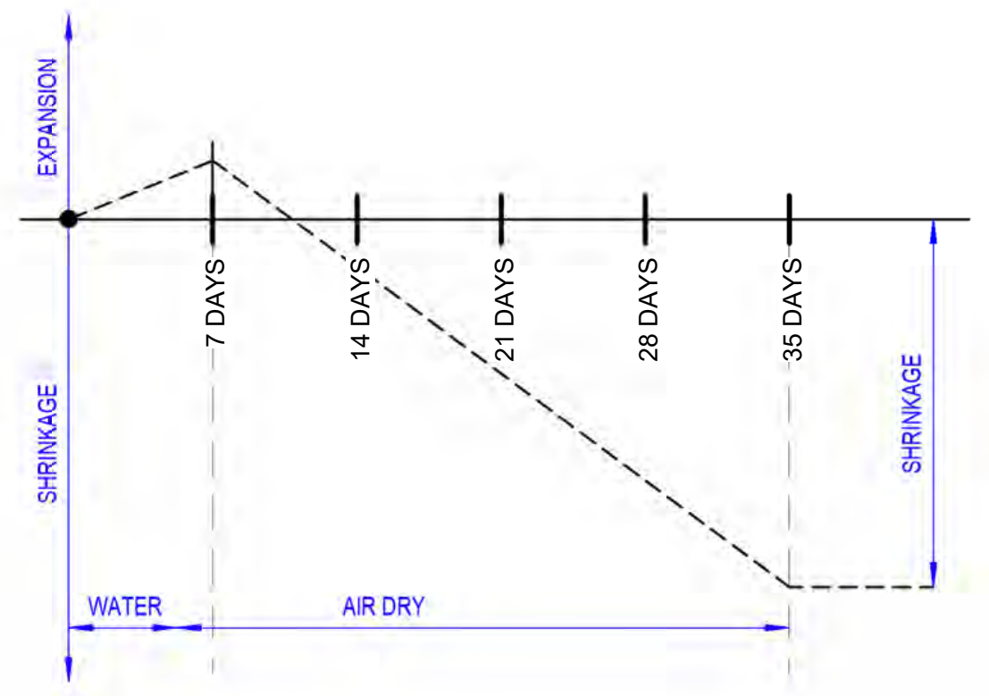
ELEMENT	# CYLINDERS	SAMPLE SET FREQUENCY
• MAT FNDN	4 PER SET U.N.O.	PER 250 CU YDS
• PIERS	4 PER SET U.N.O.	PER 50 CU YDS
• PILES	4 PER SET U.N.O.	PER 50 CU YDS/PER PILE GROUP BENEATH EA. COL / WALL
• BASEMENT WALL	4 PER SET U.N.O.	PER 150 CU YDS
• FOOTING & PILE CAP	4 PER SET U.N.O.	PER 50 CU YDS
• SLAB ON GRADE	4 PER SET U.N.O.	PER 150 CU YDS / PER 5000 SQ FT FLOOR AREA
• MILD SLAB	4 PER SET U.N.O.	PER 150 CU YDS / PER 5000 SQ FT FLOOR AREA
• P.T. SLAB	6 PER SET U.N.O.	PER 150 CU YDS / PER 5000 SQ FT FLOOR AREA
• COL/SW (28 DAY f'c)	4 PER SET U.N.O.	PER 50 CU YDS / MIN. 2 PER FLOOR
• COL/SW (56 DAY f'c)	6 PER SET U.N.O.	PER 50 CU YDS / MIN. 2 PER FLOOR
• OTHERS	4 PER SET U.N.O.	PER 150 CU YDS / MIN. 1 SET PER CLASS PER DAY

3. NO MORE THAN ONE SET OF CYLINDERS AT A TIME SHALL BE MADE FROM ANY SINGLE TRUCK.
4. A MINIMUM OF FIVE SETS OF CYLINDERS SHALL BE TAKEN AND TESTED FOR EACH CLASS OF CONCRETE.
5. IF MORE THAN ONE BATCH PLANT IS USED TO SOURCE CONCRETE, THE FREQUENCIES CITED ABOVE SHALL APPLY FOR EACH PLANT USED.

CC1.2 CONCRETE SHRINKAGE:

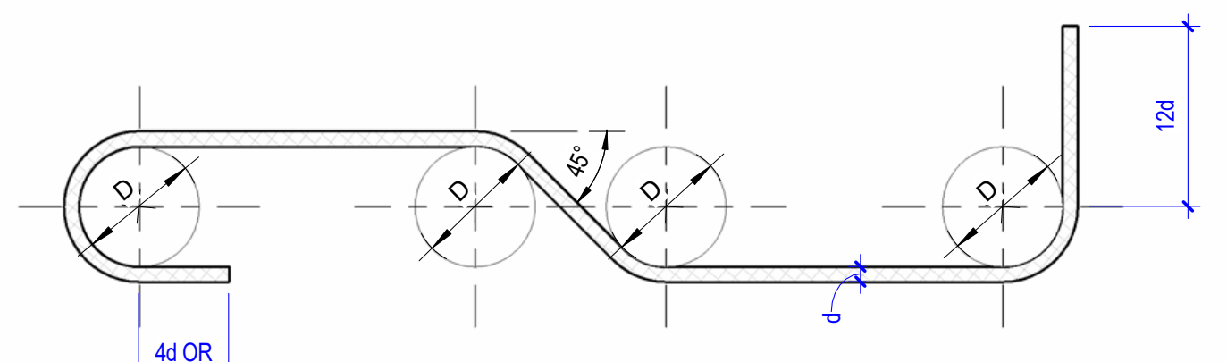
1. ALTERNATIVELY, CONFIRM TO THE FOLLOWING MAXIMUM ALLOWABLE WATER CONTENT IN LIEU OF SHRINKAGE TESTING:
   - FOR INTERMEDIATE VALUES, USE LINEAR INTERPOLATION.
   - MAXIMUM ALLOWABLE WATER CONTENT SHALL BE TOTAL WATER IN THE MIX AT THE TIME OF PLACING, INCLUDING WATER ADDED AT THE SITE.
   - FOR SPECIFIED 28 DAY SHRINKAGE LIMITS BELWO 0.030%, A SHRINKAGE REDUCING ADMIXTURE IS REQUIRED:

SPECIFIED 28 DAY SHRINKAGE LIMIT	MAXIMUM ALLOWABLE WATER CONTENT (LBS PER CUBIC YARD)
0.035%	235
0.040%	245
0.045%	255

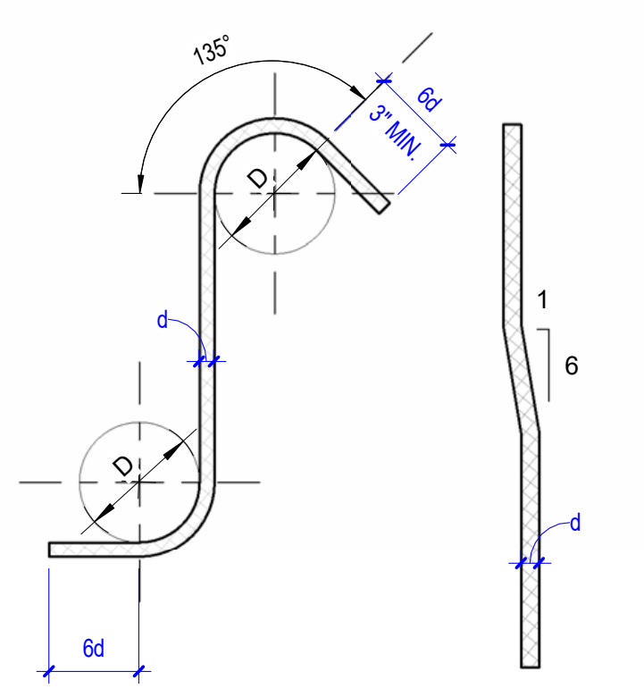


CC1.3 EXPOSED CONCRETE NOTES:

1. NO FLY ASH SHALL BE USED IN ARCHITECTURALLY EXPOSED CONCRETE WITHOUT PRIOR APPROVAL FROM ARCHITECT.
2. PROVIDE A FINISH OF "SF-2.0" OR BETTER AS DEFINED BY ACI FOR ARCHITECTURALLY EXPOSED CONCRETE AND SEAL WITH A WATER BASED ACRYLIC SEALER APPROVED BY THE OWNER / ARCHITECT.
3. ARCHITECTURALLY EXPOSED CONCRETE SHALL BE FORMED WITH MDO BOARD WHICH IS REPLACED AT SUFFICIENT INTERVALS TO ENSURE CONSISTENT QUALITY AND FINISH. CEILINGS SHALL HAVE TIGHT BUTT JOINTS. JOINTS SHALL NOT BE GROUND UNLESS MISALIGNMENT EXCEEDS A.C.I. STANDARDS AND OWNER IS CONSULTED AND APPROVES.
4. ARCHITECTURALLY EXPOSED CONCRETE COLUMNS, COLUMN TRANSITION CAPS, BEAMS, SLAB EDGES, AND WALLS SHALL BE FORMED WITH A 3/4" CHAMFER AT EACH EDGE.
5. ALL EXPOSED TO VIEW CONCRETE ELEMENTS TO HAVE A 28 DAY SHRINKAGE LIMIT OF 0.040%
6. EXPOSED TO VIEW CONCRETE ELEMENTS POURED WITH HIGHER STRENGTH CONCRETE THAN 4,000 PSI SHALL UTILIZE AN APPROVED SHRINKAGE REDUCING ADMIXTURE PER MANUFACTURES RECOMMENDATIONS TO LIMIT SHRINKAGE TO 0.040% AS NOTED ABOVE.



BAR BENDING FOR REINFORCING BAR STANDARD HOOKS



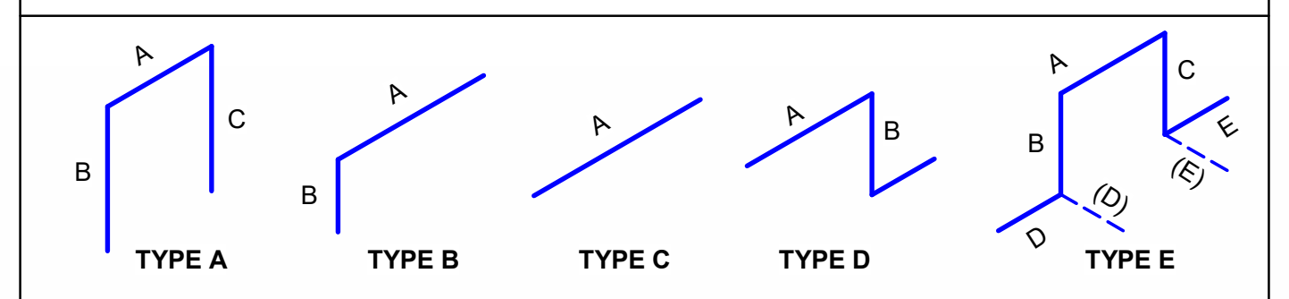
BAR BENDING FOR STIRRUPS AND TIE HOOKS

BAR BENDING FOR REINFORCING BAR OFFSET

BAR BENDING TABLE			
BAR	d	D STIRRUPS / TIES	D REINF BAR STD HOOKS
#3	3/8"	1 1/2"	2 1/4"
#4	1/2"	2"	3"
#5	5/8"	2 1/2"	3 3/4"
#6	3/4"	4 1/2"	4 1/2"
#7	7/8"	5 1/4"	5 1/2"
#8	1"	6"	6"
#9	1 1/8"	-	9 1/2"
#10	1 1/4"	-	10 3/4"
#11	1 3/8"	-	12"
#14	1 3/4"	-	18 1/4"
#18	2 1/4"	-	24"

REBAR BENDING DIAGRAM

DOWEL SCHEDULE



DOWEL MARK	SIZE	CONFIG	A	B	C	D	E
DWL-A1	#5	TYPE A	48"	24"	24"	-	-
DWL-A2	#3	TYPE A	8"	24"	24"	-	-
DWL-A3	#3	TYPE A	S	24"	24"	-	-
DWL-B1	#5	TYPE B	24"	24"	-	-	-
DWL-B2	#4	TYPE B	24"	8"	-	-	-
DWL-B3	#4	TYPE B	36"	8"	-	-	-
DWL-B4	#4	TYPE B	48"	8"	-	-	-
DWL-C1	#4	TYPE C	48"	-	-	-	-
DWL-C2	#5	TYPE C	48"	-	-	-	-

NOTE:
1. S = SLAB / WALL THICKNESS MINUS REQUIRED COVERAGE

Class B SPLICE (ℓd = 1.3) - TO BE USED U.N.O.

GRADE 60 **UNCOATED** BARS IN NORMAL WEIGHT CONCRETE (BLACK / GALVANIZED)

BAR SIZE	CONCRETE STRENGTH (f`c=PSI)									
	3,000	4,000	5,000	6,000	7,000	8,000	9,000			
#3	1'-10"	1'-7"	1'-5"	1'-4"	1'-4"	1'-4"	1'-4"			
#4	2'-5"	2'-1"	1'-11"	1'-9"	1'-7"	1'-6"	1'-5"			
#5	3'-0"	2'-7"	2'-4"	2'-2"	2'-0"	1'-10"	1'-9"			
#6	3'-7"	3'-1"	2'-10"	2'-7"	2'-4"	2'-3"	2'-1"			
#7	5'-3"	4'-6"	4'-1"	3'-9"	3'-5"	3'-3"	3'-0"			
#8	6'-0"	5'-2"	4'-8"	4'-3"	3'-11"	3'-8"	3'-6"			
#9	6'-9"	5'-10"	5'-3"	4'-9"	4'-5"	4'-2"	3'-11"			
#10	7'-7"	6'-7"	5'-11"	5'-4"	5'-0"	4'-8"	4'-5"			
#11	8'-5"	7'-3"	6'-6"	5'-11"	5'-6"	5'-2"	4'-10"			
#14	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR			
#18	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR			

Class A SPLICE (ℓd = 1.0) - TO BE USED ONLY WHEN SPECIFIED

GRADE 60 **UNCOATED** BARS IN NORMAL WEIGHT CONCRETE (BLACK / GALVANIZED)

BAR SIZE	CONCRETE STRENGTH (f`c=PSI)									
	3,000	4,000	5,000	6,000	7,000	8,000	9,000			
#3	1'-5"	1'-3"	1'-1"	1'-0"	1'-0"	1'-0"	1'-0"			
#4	1'-10"	1'-7"	1'-5"	1'-4"	1'-3"	1'-2"	1'-1"			
#5	2'-4"	2'-0"	1'-10"	1'-8"	1'-6"	1'-5"	1'-4"			
#6	2'-9"	2'-5"	2'-2"	2'-0"	1'-10"	1'-9"	1'-7"			
#7	4'-0"	3'-6"	3'-2"	2'-10"	2'-8"	2'-6"	2'-4"			
#8	4'-7"	4'-0"	3'-7"	3'-3"	3'-0"	2'-10"	2'-8"			
#9	5'-2"	4'-6"	4'-0"	3'-8"	3'-5"	3'-2"	3'-0"			
#10	5'-10"	5'-1"	4'-6"	4'-2"	3'-10"	3 '-7"	3'-5"			
#11	6'-6"	5'-7"	5'-0"	4'-7"	4'-3"	4'-0"	3'-9"			
#14	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR			
#18	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR			

Class B SPLICE (ℓd = 1.3) - TO BE USED U.N.O.

GRADE 80 **UNCOATED** BARS IN NORMAL WEIGHT CONCRETE (BLACK / GALVANIZED)

BAR SIZE	CONCRETE STRENGTH (f`c=PSI)									
	3,000	4,000	5,000	6,000	7,000	8,000	9,000			
#3	2'-9"	2'-5"	2'-2"	2'-0"	1'-10"	1'-9"	1'-7"			
#4	3'-8"	3'-2"	2'-10"	2'-7"	2'-5"	2'-3"	2'-2"			
#5	4'-7"	4'-0"	3'-7"	3'-3"	3'-0"	2'-10"	2'-8"			
#6	5'-6"	4'-9"	4'-3"	3'-11"	3'-7"	3'-5"	3'-2"			
#7	8'-0"	6'-11"	6'-2"	5'-8"	5'-3"	4'-11"	4'-8"			
#8	9'-2"	7'-11"	7'-1"	6'-6"	6'-0"	5'-7"	5'-4"			
#9	10'-4"	8'-11"	8'-0"	7'-4"	6'-9"	6'-4"	6'-0"			
#10	11'-7"	10'-1"	9'-0"	8'-3"	7'-7"	7'-1"	6'-9"			
#11	12'-10"	11'-2"	10'-0"	9'-1"	8'-5"	7'-11"	7'-5"			
#14	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR			
#18	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR			

Class A SPLICE (ℓd = 1.0) - TO BE USED ONLY WHEN SPECIFIED

GRADE 80 **UNCOATED** BARS IN NORMAL WEIGHT CONCRETE (BLACK / GALVANIZED)

BAR SIZE	CONCRETE STRENGTH (f`c=PSI)									
	3,000	4,000	5,000	6,000	7,000	8,000	9,000			
#3	2'-2"	1'-10"	1'-8"	1'-6"	1'-5"	1'-4"	1'-3"			
#4	2'-10"	2'-6"	2'-3"	2'-0"	1'-10"	1'-9"	1'-8"			
#5	3'-6"	3'-1"	2'-9"	2'-6"	2'-4"	2'-2"	2'-1"			
#6	4'-3"	3'-8"	3'-4"	3'-0"	2'-9"	2'-7"	2'-6"			
#7	6'-2"	5'-4"	4'-9"	4'-4"	4'-1"	3'-10"	3'-7"			
#8	7'-0"	6'-1"	5'-6"	5'-0"	4'-7"	4'-4"	4'-1"			
#9	7'-11"	6'-11"	6'-2"	5'-7"	5'-3"	4'-11"	4'-7"			
#10	8'-11"	7'-9"	6'-11"	6'-4"	5'-10"	5'-6"	5'-2"			
#11	9'-11"	8'-7"	7'-8"	7'-0"	6'-6"	6'-1"	5'-9"			
#14	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR			
#18	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR	MECH CPLR			

MINIMUM EMBEDMENT WITH STANDARD 90° HOOK

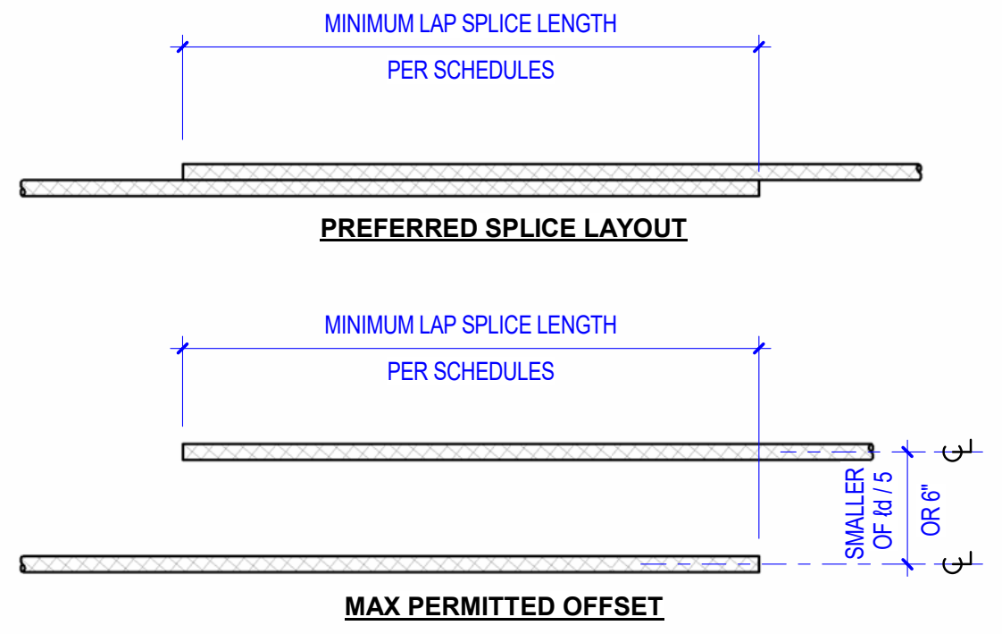
GRADE 60 **UNCOATED** BARS IN NORMAL WEIGHT CONCRETE (BLACK / GALVANIZED)

BAR SIZE	f`c=3,000 PSI	f`c=4,000 PSI	f`c=5,000 PSI	f`c=6,000 PSI	f`c=7,000 PSI	f`c=8,000 PSI	f`c=9,000 PSI	
#3	0'-9"	0'-8"	0'-7"	0'-6"	0'-6"	0'-6"	0'-6"	
#4	1'-0"	0'-10"	0'-9"	0'-8"	0'-8"	0'-7"	0'-7"	
#5	1'-2"	1'-0"	0'-11"	0'-10"	0'-10"	0'-9"	0'-8"	
#6	1'-5"	1'-3"	1'-1"	1'-0"	1'-0"	0'-11"	0'-10"	
#7	1'-8"	1'-5"	1'-3"	1'-2"	1'-1"	1'-0"	1'-0"	
#8	1'-10"	1'-7"	1'-5"	1'-4"	1'-3"	1'-2"	1'-1"	
#9	2'-1"	1'-10"	1'-8"	1'-6"	1'-5"	1'-4"	1'-3"	
#10	2'-4"	2'-1"	1'-10"	1'-8"	1'-7"	1'-6"	1'-5"	
#11	2'-7"	2'-3"	2'-0"	1'-10"	1'-9"	1'-7"	1'-6"	
#14	3'-2"	2'-9"	2'-5"	2'-3"	2'-1"	1'-11"	1'-10"	
#18	4'-2"	3'-7"	3'-3"	2'-11"	2'-9"	2'-7"	2'-6"	

MINIMUM EMBEDMENT WITH STANDARD 90° HOOK

GRADE 75-80 **UNCOATED** BARS IN NORMAL WEIGHT CONCRETE (BLACK / GALVANIZED)

BAR SIZE	f`c=3,000 PSI	f`c=4,000 PSI	f`c=5,000 PSI	f`c=6,000 PSI	f`c=7,000 PSI	f`c=8,000 PSI	f`c=9,000 PSI	
#3	1'-0"	0'-10"	0'-9"	0'-8"	0'-8"	0'-8"	0'-8"	
#4	1'-3"	1'-1"	1'-0"	0'-10"	0'-10"	0'-9"	0'-9"	
#5	1'-6"	1'-3"	1'-2"	1'-1"	1'-1"	1'-0"	0'-10"	
#6	1'-10"	1'-7"	1'-5"	1'-3"	1'-3"	1'-2"	1'-1"	
#7	2'-1"	1'-10"	1'-7"	1'-6"	1'-5"	1'-3"	1'-3"	
#8	2'-4"	2'-0"	1'-10"	1'-8"	1'-7"	1'-5"	1'-5"	
#9	2'-8"	2'-4"	2'-1"	1'-11"	1'-10"	1'-7"	1'-7"	
#10	2'-11"	2'-8"	2'-4"	2'-1"	2'-0"	1'-10"	1'-10"	
#11	3'-3"	2'-10"	2'-6"	2'-4"	2'-3"	1'-11"	1'-11"	
#14	4'-0"	3'-6"	3'-1"	2'-10"	2'-8"	2'-4"	2'-4"	
#18	5'-3"	4'-6"	4'-1"	3'-8"	3'-6"	3'-3"	3'-2"	



PREFERRED SPLICE LAYOUT

MAX PERMITTED OFFSET

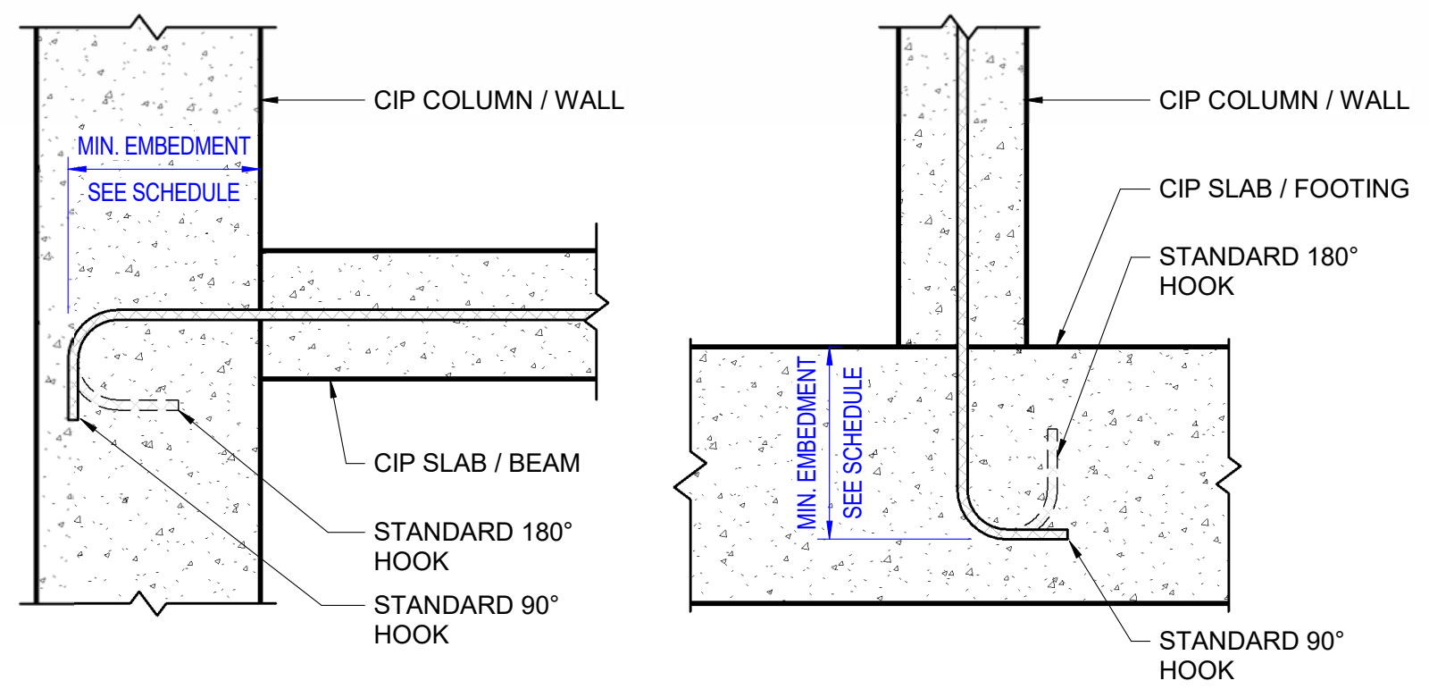
TYPICAL REBAR LAP SPLICE DETAIL

ADDITIONAL NOTES AT BUNDLED BARS

1. REINFORCEMENT DEVELOPMENT LENGTH SHOWN IN TABLE SHALL BE INCREASED BY 20 PERCENT WHEN SHOWN BUNDLED IN A GROUP OF THREE.
2. REINFORCEMENT DEVELOPMENT LENGTH SHOWN IN TABLE SHALL BE INCREASED BY 33 PERCENT WHEN SHOWN BUNDLED IN A GROUP OF FOUR.
3. BUNDLES LARGER THAN FOUR BARS ARE NOT PERMITTED.

ADDITIONAL NOTES FOR LIGHT WEIGHT CONCRETE

1. REINFORCEMENT DEVELOPMENT LENGTH SHOWN IN TABLE SHALL BE INCREASED BY 33 PERCENT



TYPICAL HOOKED BAR EMBEDMENT

VSM

PROJECT # : 26154
DRAWN BY: Author
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General Notes - Reinforcement

SHEET NUMBER:
S0.110

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CC2.0 POST-INSTALLED ANCHORS IN CONCRETE:

CC2.1 MATERIALS:

1. THE INSTALLATION OF POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN PLACE ANCHORS SHALL BE APPROVED BY THE ENGINEER OF RECORD.
2. SUBMITTAL OF ALL PROPOSED PRODUCTS, WITH TECHNICAL DATA AND CURRENT ICC-ESR REPORTS IS REQUIRED FOR REVIEW AND APPROVAL BY ENGINEER OF RECORD.
3. MECHANICAL ANCHORS SHALL BE TESTED AND ASSESSED IN ACCORDANCE WITH THE MOST RECENT EDITION OF ACI 355.2 *QUALIFICATION OF POST-INSTALLED ADHESIVE ANCHORS IN CONCRETE (355.4) AND COMMENTARY* AND ICC ES AC193 CONSIDERING CRACKED AND UNCRACKED CONCRETE. PRE-APPROVED MECHANICAL ANCHORS ARE AS FOLLOWS:
   A. HILTI:
      a. HILTI KWIK HUS EZ AND KWIK HUS EZ-I SCREW ANCHOR (ICC ESR-3027)
      b. HILTI HSL-3 EXPANSION ANCHOR (ICC ESR-1545)
      c. HILTI HDA UNDERCUT ANCHOR (ICC ESR-1546)
      d. HILTI KWIK BOLT TZ2 (ICC ESR-4266)
      e. HILTI KWIK BOLT 1 (IAPMO ER-678)
   B. SIMPSON:
      a. SIMPSON STONG-TIE TITEN HD AND TITEN-HD ROD HANGER (ICC ESR-2713)
      b. SIMPSON STRONG TIE STRONG-BOLT (ICC ESR-1771)
      c. SIMPSON STRONG-TIE STRONG-BOLT 2 (ICC ESR-3037)
   C. DEWALT:
      a. DEWALT SCREW-BOLT+ AND HANGERMATE+ (ICC ESR-3889)
      b. DEWALT POWER-STUD+ SD1 (ICC ESR-2818)
      c. DEWALT POWER-STUD+ SD2 (ICC ESR-2502)
      d. DEWALT POWER-BOLT+ (ICC ESR-3260)
      e. DEWALT CCU+ (ICC-ESR 4810)
4. ADHESIVE ANCHOR SYSTEMS SHALL BE TESTED AND ASSESSED IN ACCORDANCE WITH THE MOST RECENT EDITION OF ACI 355.*4 QUALIFICATION OF POST-INSTALLED ADHESIVE ANCHORS IN CONCRETE (355.4) AND COMMENTARY* AND ICC ES AC308 CONSIDERING CRACKED AND UNCRACKED CONCRETE. PRE-APPROVED ADHESIVE ANCHORS ARE AS FOLLOWS:
   A. HILTI:
      a. HILTI HIT-HY 200 V3 WITH HIT-Z OR HAS THREADED RODS (ICC ESR-4866)
      b. HILTI RE 500 V3 (ICC ESR-3814)
      c. HILTI KWIK-X DUAL ACTION ANCHOR SYSTEM WITH KHC CAPSULE ADHESIVE AND KWIK-HUS EZ (ICC ESR-5065)
   B. SIMPSON:
      a. SIMPSON STONG-TIE SET-3G (ICC ESR-4057)
   C. DEWALT:
      a. DEWALT AC200+ (ICC ESR-4027)
      b. DEWALT PURE 110+ (ICC ESR-3298)
      c. DEWALT PURE 220+ (ICC ESR-5144)
5. ADHESIVE ANCHORS SELECTED FROM TEM 2 ABOVE SHALL BE SUPPLIED AS AN ENTIRE SYSTEM. THE SYSTEM SHALL INCLUDE, BUT IS NOT LIMITED TO, MANUFACTURERS PRINTED INSTALLATION INSTRUCTIONS (MPII) AS SUPPLIED WITH THE ADHESIVE, ADHESIVE CARTRIDGE, MIXING NOZZLE, EXTENSION TUBE, DISPENSER, AND ALL REQUIRED EQUIPMENT FOR PROPERLY CLEANING THE DRILLED HOLE.
6. ANCHOR DESIGN SHALL BE IN ACCORDANCE WITH CHAPTER 17 OF ACI 318-19. FOR ADHESIVE ANCHORS, THE FOLLOWING MINIMUM DESIGN VALUES FOR BOND STRESS WERE ASSUMED FOR THE DESIGN USING THE ABOVE ADHESIVE ANCHOR ASSEMBLIES FOR IMPACT- OR ROCKDRILLED HOLES:
   a. OUTDOOR/EXPOSED APPLICATION: Tcr = 200 PSI, Tuncr = 650 PSI
   b. INDOOR/DRY APPLICATION: Tcr = 300 PSI, Tuncr = 1000 PSI
7. ALL-THREADED ROD (EYEBOLTS, THREADED STUDS, INTERNAL THREADED PARTS) TO BE USED IN ADHESIVE ANCHOR ASSEMBLIES SHALL CONFORM TO ASTM A36, A193 (GRADE B7), F1554 OR OTHER APPROVED ANCHOR ASSEMBLY TYPES. STAINLESS STEEL ANCHOR RODS SHALL BE AISI TYPE 304 OR TYPE 316. THREADS SHALL BE UNC COARSE THREADS, UNLESS NOTED OTHERWISE. COMPATIBLE NUTS AND WASHERS SHALL BE FURNISHED WITH THE ALL-THREAD ROD AND CONSIDERED PART OF THE ASSEMBLY. WITH HOT-DIPPED GALVANIZED RODS, PROVIDE OVERSIZED TAPPED, HOT-DIPPED GALVANIZED NUTS.
8. NUTS, WASHERS, AND OTHER HARDWARE USED WITH AN ALL-THREADED BAR ADHESIVE ANCHOR SYSTEM OR WITH A MECHANICAL EXPANSION ANCHOR SHALL HAVE A MATERIAL OR AN ALLOY DESIGNATION THAT IS COMPATIBLE WITH THE ANCHOR ROD/ALLOY. GALVANIZED ASSEMBLIES SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A153 CLASS C. ELECTROPLATE GALVANIZING IS NOT ACCEPTABLE. DISSIMILAR METAL ASSEMBLIES SHALL BE SEPARATED BY NYLON, EPDM, OR OTHER APPROVED NON-METALLIC WASHERS.
9. REINFORCING BARS TO BE USED IN ADHESIVE ANCHOR ASSEMBLIES (E.G., AS ANCHOR REINFORCEMENT) OR AS POST-INSTALLED REINFORCING SHALL CONFORM TO ASTM A615 OR A706.

CC2.2 GENERAL INSTALLATION GUIDELINES:

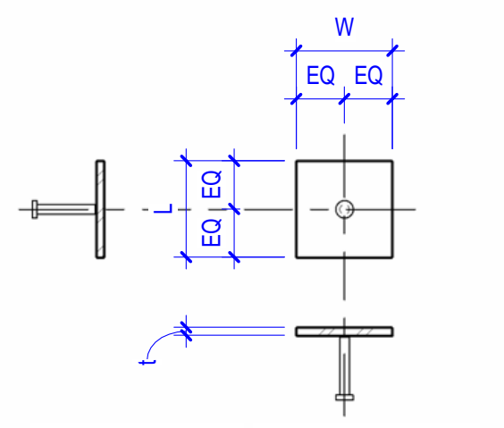
1. ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) IN CONJUNCTION WITH EDGE DISTANCE, SPACING AND EMBEDMENT DEPTH AS INDICATED ON THE DRAWINGS.
2. DRILLED HOLES FOR POST-INSTALLED ANCHORS IN CONCRETE AND MASONRY SHALL BE COMPLETED PER OSHA 29 CFR 1926.1153 - RESPIRABLE CRYSTALLINE SILICA USING A TESTED SYSTEM SUCH AS DEWALT DUSTX+ EXTRACTOR AND DRILL BIT SYSTEM.
3. CONCRETE SHALL HAVE AN ABSOLUTE MINIMUM COMPRESSIVE STRENGTH (f'c) OF 2,500 PSI AT THE TIME OF ADHESIVE ANCHOR INSTALLATION.
4. CONCRETE AT TIME OF ADHESIVE ANCHOR INSTALLATION SHALL HAVE A MINIMUM AGE OF 21 DAYS. FOR INSTALLATION OF ADHESIVE ANCHORS IN CONCRETE HAVING AN AGE LESS THAN 21 DAYS, TESTS SHALL BE CONDUCTED TO VERIFY THE PERFORMANCE OF THE PRODUCT IN ACCORDANCE WITH ACI 355.4.
5. THE CONCRETE TEMPERATURE AT THE TIME OF ADHESIVE ANCHOR INSTALLATION SHALL BE AT LEAST 50°F UNLESS TESTING HAS BEEN CONDUCTED IN ACCORDANCE WITH RECOGNIZED CRITERIA TO VERIFY PERFORMANCE IN CONCRETE AT LOWER TEMPERATURES.
6. EMBEDMENT DEPTH AND MINIMUM ANCHOR PROJECTION OF THE ANCHOR ELEMENT FROM THE CONCRETE SURFACE SHALL BE AS SHOWN ON THE DRAWINGS OR DETAIL FOR THE PARTICULAR ANCHOR OR GROUP OF ANCHORS BEING INSTALLED. IN THE ABSENCE OF A SPECIFIC EMBEDMENT DEPTH OR ANCHOR PROJECTION , CONTRACTOR TO REQUEST SAID INFORMATION FROM THE ENGINEER PRIOR TO ANCHOR INSTALLATION.
7. ADHESIVE CARTRIDGES SHALL BE STORED UNDER CONDITIONS IN COMPLIANCE WITH MANUFACTURER RECOMMENDATIONS REGARDING TEMPERATURE, EXPOSURE TO SUNLIGHT, ETC. AND EVIDENCE OF COMPLIANCE SHALL BE MADE AVAILABLE UPON REQUEST. THE USE OF EXPIRED ADHESIVE, AS INDICATED BY THE EXPIRATION DATE ON THE CARTRIDGE, IS PROHIBITED.

CC2.3 INSTALLATION:

1. ADHESIVE ANCHORS SHALL BE INSTALLED BY QUALIFIED PERSONNEL TRAINED TO INSTALL ADHESIVE ANCHORS IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. BOTH POST-INSTALLED EXPANSION AND ADHESIVE ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).
2. ADHESIVE ANCHORS WITH DIAMETER GREATER THAN 3/8- INCH INSTALLED IN ORIENTATIONS FROM HORIZONTAL TO VERTICAL SHALL EMPLOY A PISTON PLUG FOR THE ADHESIVE INJECTION.
3. INSTALLATION OF ADHESIVE ANCHORS IN ORIENTATIONS FROM HORIZONTAL TO VERTICAL TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY THE ACI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM OR EQUIVALENT. THESE ANCHORS ARE DESIGNATED WITH A (CERT) AFTER THE ANCHOR CALLOUT. NOTE: SOME DOWN-HOLE INSTALLATIONS SHOWN ON DRAWINGS SUPPORT SUSTAINED TENSION LOADS AND ARE SO DESIGNATED WITH A (CERT) AFTER THE ANCHOR CALLOUT.
4. THE INSTALLER'S QUALIFICATIONS SHALL BE SUBMITTED AND APPROVED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS)
5. THE CONTRACTOR SHALL PROVIDE ALL EQUIPMENT REQUIRED TO INSTALL THE EXPANSION AND/OR ADHESIVE ANCHOR INCLUDING, BUT NOT LIMITED TO, DRILLS, SETTING TOOLS, CLEAN-OUT BRUSHES, BLOWOUT BULBS, OIL-FREE COMPRESSED AIR, VACUUMS, WRENCHES, ETC.
6. UNLESS OTHERWISE SPECIFIED, ANCHORS SHALL BE INSTALLED IN HOLES DRILLED WITH A ROTARY IMPACT HAMMER DRILL OR, WHERE NOT OTHERWISE PROSCRIBED, A ROCK DRILL. WHERE SPECIFIED AND WHERE PERMITTED BY THE MPII, HOLES MAY BE DRILLED WITH A DIAMOND CORE DRILL. IN ALL CASES, THE BIT DIAMETER SHALL BE IN ACCORDANCE WITH THE MPII.
7. ANCHOR HOLES SHALL BE THOROUGHLY CLEANED IN ACCORDANCE WITH THE PROCEDURES SPECIFIED IN THE MPII PRIOR TO ADHESIVE INJECTION. AT A MINIMUM, CLEAN WITH OIL-FREE AND MOISTURE-FREE COMPRESSED AIR USING A NOZZLE EXTENDED TO THE BOTTOM HOLE; SUPPLEMENT WITH A BRUSH OR OTHER TOOL CLEANING TO REMOVE ALL CONCRETE DUST AND LOOSE MATERIAL AND FOLLOW WITH A SECOND COMPRESSED AIR CLEANING.
8. DRILLED AND CLEANED ANCHOR HOLES SHALL BE PROTECTED FROM CONTAMINATION AND WATER (E.G. RAIN) UNTIL THE ADHESIVE IS INSTALLED.
9. A DRILLED ANCHOR HOLE SHALL BE RE-CLEANED JUST PRIOR TO ADHESIVE INJECTION IF, IN THE OPINION OF THE ENGINEER, INSPECTOR, OR OWNER'S REPRESENTATIVE, THE HOLE HAS BECOME CONTAMINATED AFTER INITIAL CLEANING.
10. ADHESIVE SHALL BE INJECTED IN ACCORDANCE WITH THE MPII USING EQUIPMENT AND PROCEDURES AS SPECIFIED THEREIN FOR THE SPECIFIC CONDITIONS ASSOCIATED WITH THE INJECTION. THIS SHOULD BE CLEARLY SPECIFIED IN THE MPII, IF NOT, ANOTHER PRODUCT SHOULD BE SPECIFIED.
11. ANCHOR ELEMENTS TO BE INSTALLED IN THE ADHESIVE SHALL BE CLEAN, OIL-FREE, AND FREE OF LOOSE RUST, PAINT, OR OTHER COATINGS. THREADS ON THE PROJECTING PORTION OF THE ANCHOR ELEMENT SHALL BE PROTECTED FROM ADHESIVE CONTAMINATION.
12. INSTALLED ADHESIVE ANCHORS SHALL BE SECURELY FIXED IN-PLACE TO PREVENT DISPLACEMENT WHILE THE ADHESIVE CURES. UNLESS SHOWN OTHERWISE ON THE DRAWINGS, ANCHORS SHALL BE INSTALLED PERPENDICULAR TO THE CONCRETE SURFACE. ANCHORS DISPLACED BEFORE FULL ADHESIVE CURE SHALL BE CONSIDERED DAMAGED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
13. POST-INSTALLED REINFORCING BARS OR ALL-THREADED BARS SHALL NOT BE BENT AFTER BEING INSTALLED UNLESS SPECIFICALLY PERMITTED BY THE SEOR OR ON THE CONTRACT DOCUMENTS.

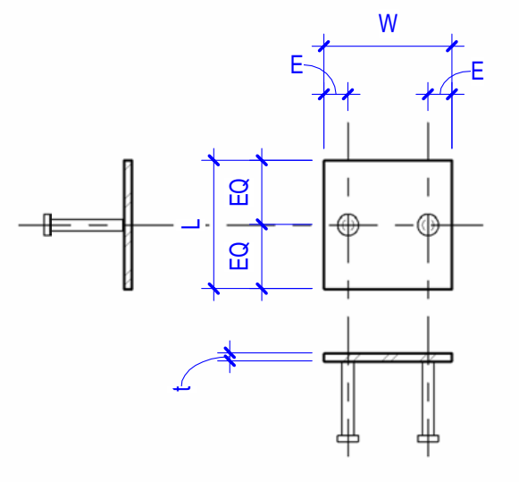
CC2.4 FIELD QUALITY CONTROL:

1. THE IBC REQUIRES SPECIAL INSPECTION OF ALL POST-INSTALLED ANCHORS. ACI 318-19 SECTIONS 26.13.1.5 AND 26.13.1.6 REQUIRE THAT ALL INSPECTIONS OF MECHANICAL AND ADHESIVE ANCHORS, RESPECTIVELY, ARE PERFORMED BY A CERTIFIED FIELD INSPECTOR SPECIFICALLY APPROVED FOR THAT PURPOSE BY THE SEOR AND THE BUILDING OFFICIAL. CERTIFICATION IS ESTABLISHED THROUGH AN INDEPENDENT ASSESSMENT SUCH AS THE ACI POST-INSTALLED CONCRETE ANCHOR INSTALLATION INSPECTOR PROGRAM OR SIMILAR PROGRAM WITH EQUIVALENT REQUIREMENTS. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY A CERTIFIED INSPECTOR. ACI 318-19 26.13.3.2(E) REQUIRES THESE ANCHOR INSTALLATIONS TO BE INSPECTED BY A CERTIFIED SPECIAL INSPECTOR WHO IS CONTINUOUSLY PRESENT WHEN AND WHERE THE INSTALLATIONS ARE BEING PERFORMED. AS REQUIRED BY ACI 318-19, THE SPECIAL INSPECTOR SHALL SUBMIT A REPORT TO THE SEOR AND BUILDING OFFICIAL DESCRIBING THE WORK COVERED BY THE REPORT AND STATING THE WORK HAS BEEN PERFORMED, THE MATERIALS AND THE INSTALLATION PROCEDURES USED CONFORM TO THE APPROVED CONTRACT DOCUMENTS, AND THE WORK WAS INSTALLED ACCORDING TO THE MPII.
2. POST-INSTALLED EXPANSION AND ADHESIVE ANCHORS SHALL BE PROOF-LOADED AS REQUIRED BY THE CONTRACT DOCUMENTS. A MINIMUM 10 PERCENT OF EACH TYPE AND SIZE OF EACH TYPE OF SIZE OF ANCHOR ASSEMBLY SHALL BE PROOF LOADED IN TENSION BY AN INDEPENDENT TESTING LABORATORY UNLESS OTHERWISE INDICATED ON THE CONTRACT DOCUMENTS.
   a. PROOF LOADING SHALL BE CONDUCTED AS CONFINED TENSION TESTING FOR ADHESIVE ANCHORS USING A CENTER-HOLE CYLINDER.
   b. THE INDEPENDENT TESTING LABORATORY SHALL SUBMIT AN ANCHORAGE TESTING PLAN TO THE LICENSED DESIGN PROFESSIONAL TO ENSURE THE TESTING REQUIREMENTS ARE FULFILLED.
3. POST-INSTALLED EXPANSION AND ADHESIVE ANCHORS SHALL BE PROOF-LOADED TO LOAD LEVELS AS SHOWN ON THE CONTRACT DOCUMENTS. PROOF LOADING OF ADHESIVE ANCHORS SHALL BE PERFORMED AFTER A MINIMUM CURING PERIOD SPECIFIED BY THE MANUFACTURER. MINIMUM ADHESIVE ANCHOR PROOF LOADS SHALL BE 50 PERCENT OF EXPECTED ADHESIVE ULTIMATE BOND STRENGTH OR 80 PERCENT OF STEEL YIELD STRENGTH OF THE ANCHOR ROD, WHICHEVER IS LOWER.
4. ANCHORS SHALL HAVE NO VISIBLE INDICATIONS OF DISPLACEMENT OR DAMAGE DURING OR AFTER PROOF LOAD APPLICATION. CONCRETE CRACKING IN THE VICINITY OF THE ANCHOR AFTER LOADING SHALL BE CONSIDERED A FAILURE.
5. IF MORE THAN 10 PERCENT OF THE TESTED ANCHORS FAIL TO ACHIEVE THE SPECIFIED PROOF LOAD WITHIN THE LIMITS DEFINED ON THESE DRAWINGS, 100 PERCENT OF THE ANCHORS OF THE SAME DIAMETER AND TYPE AS THE FAILED ANCHOR SHALL BE PROOF TESTED, UNLESS OTHERWISE DIRECTED BY THE SEOR.



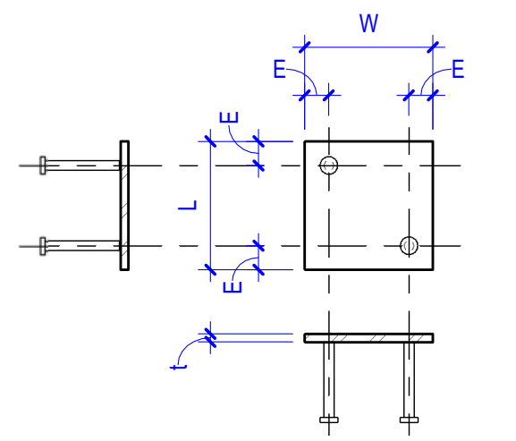
Embed Plate - Type A (WP-A)

MARK	PLATE				ANCHORS (HSA)		
	W	L	t	E	DIA.	LENGTH	QUANTITY
WP-A4X4.1	4"	4"	3/8"	-	1/2"	4"	1
WP-A6X6.1	6"	6"	1/2"	-	5/8"	4"	1



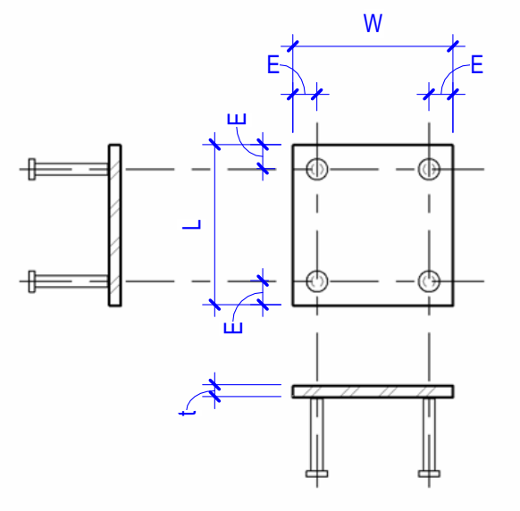
Embed Plate - Type B (WP-B)

MARK	PLATE				ANCHORS (HSA)		
	W	L	t	E	DIA.	LENGTH	QUANTITY
WP-B8X5.1	8"	5"	1/2"	1 1/2"	5/8"	4"	2
WP-B8X8.1	8"	8"	1/2"	1 1/2"	5/8"	4"	2
WP-B8X8.2	8"	8"	1/2"	1 1/2"	3/4"	5"	2
WP-B8XS.1	8"	S	1/2"	1 1/2"	5/8"	4"	2
WP-B8XS.2	8"	S	1/2"	1 1/2"	3/4"	5"	2
WP-B12X6.1	12"	6	1/2"	1 1/2"	3/4"	5"	2



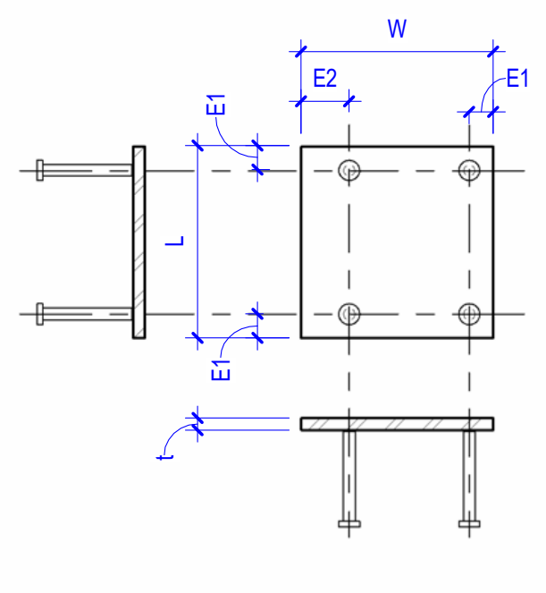
Embed Plate - Type C (WP-C)

MARK	PLATE				ANCHORS (HSA)		
	W	L	t	E	DIA.	LENGTH	QUANTITY
WP-C6X6.1	6"	6"	3/8"	1 1/2"	1/2"	4"	2
WP-C8X8.1	8"	8"	1/2"	1 1/2"	5/8"	5"	2



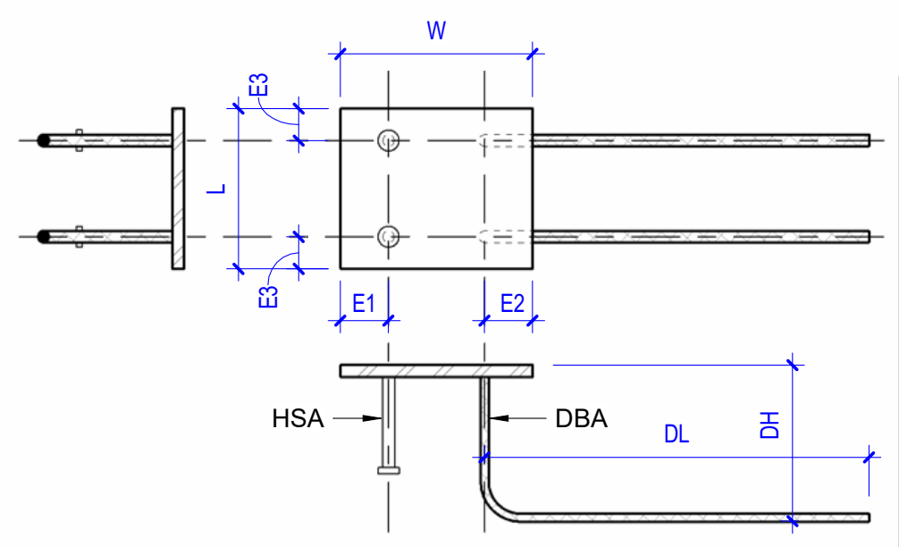
Embed Plate - Type D (WP-D)

MARK	PLATE				ANCHORS (HSA)		
	W	L	t	E	DIA.	LENGTH	QUANTITY
WP-D8X8.1	8"	8"	1/4"	1 1/2"	1/2"	6"	4
WP-D8X8.2	8"	8"	1/2"	1 1/2"	5/8"	4"	4
WP-D8X8.3	8"	8"	1/2"	1 1/2"	5/8"	5"	4
WP-D8X12.1	8"	12"	3/4"	1 1/2"	3/4"	6"	4
WP-D10X10.1	10"	10"	3/4"	1 1/2"	3/4"	4"	4
WP-D10X10.2	10"	10"	3/4"	1 1/2"	3/4"	5"	4
WP-D12X12.1	12"	12"	1/2"	1 1/2"	1/2"	3 1/2"	4
WP-D12X12.2	12"	12"	3/4"	1 1/2"	3/4"	4"	4
WP-D12X12.3	12"	12"	3/4"	1 1/2"	3/4"	6"	4
WP-D12X12.4	12"	12"	3/4"	1 1/2"	3/4"	5"	4
WP-D12X12.5	12"	12"	3/8"	1 1/2"	1/2"	4"	4
WP-D14X14.1	14"	14"	3/4"	1 1/2"	3/4"	4"	4
WP-D14X14.2	14"	14"	3/4"	1 1/2"	3/4"	5"	4



Embed Plate - Type E (WP-E)

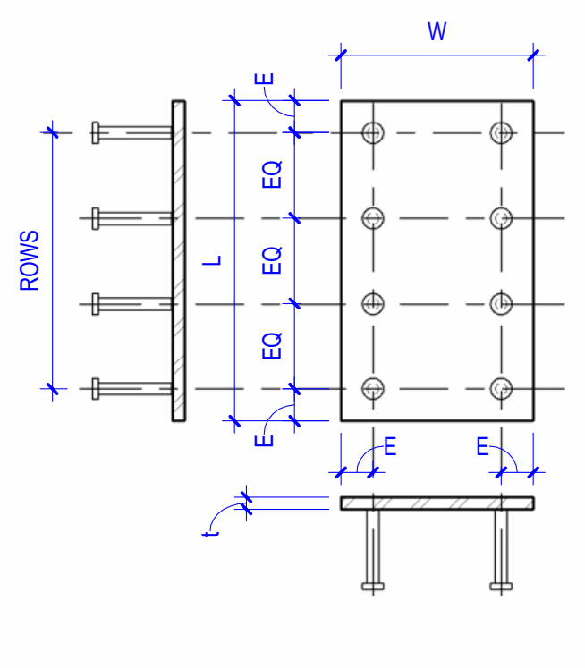
MARK	PLATE					ANCHORS (HSA)		
	W	L	t	E1	E2	DIA.	LGTH	QTY
WP-E8X8.1	8"	8"	1/2"	1 1/2"	3"	5/8"	4"	4
WP-E8X8.2	8"	8"	1/2"	1 1/2"	3"	5/8"	5"	4
WP-E8X10.1	8"	10"	1/2"	1 1/2"	3"	5/8"	4"	4
WP-E10X10.1	10"	10"	3/4"	1 1/2"	3"	3/4"	4"	4
WP-E10X10.2	10"	10"	3/4"	1 1/2"	3"	3/4"	5"	4
WP-E12X12.1	12"	12"	3/4"	1 1/2"	3"	3/4"	4"	4
WP-E12X12.2	12"	12"	3/4"	1 1/2"	3"	3/4"	6"	4
WP-E12X12.3	12"	12"	3/4"	1 1/2"	3"	3/4"	5"	4
WP-E12X12.4	12"	12"	3/8"	1 1/2"	3"	1/2"	4"	4
WP-E14X14.1	14"	14"	3/4"	1 1/2"	3"	3/4"	4"	4
WP-E14X14.2	14"	14"	3/4"	1 1/2"	3"	3/4"	5"	4



Embed Plate - Type F (WP-F)

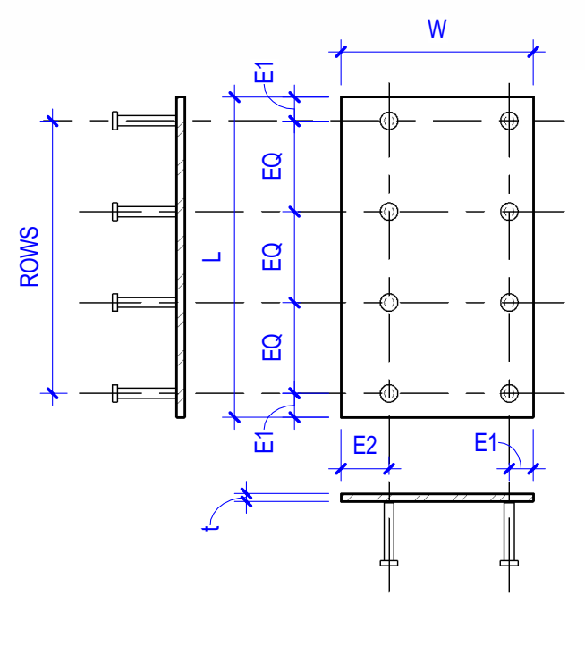
MARK	PLATE						ANCHORS (HSA)			ANCHORS (DBA)			
	W	L	t	E1	E2	E3	DIA.	LGTH	QTY	DIA.	DL	DH	QTY
WP-F12X12.1	12"	10"	3/4"	3"	3"	3"	3/4"	6"	2	3/4"	24"	C1	2

NOTE:
1. S = SLAB THICKNESS
2. C1= TOTAL SLAB THICKNESS - BOTT REBAR COVER
3. C2= COLUMN / WALL WIDTH - 2"



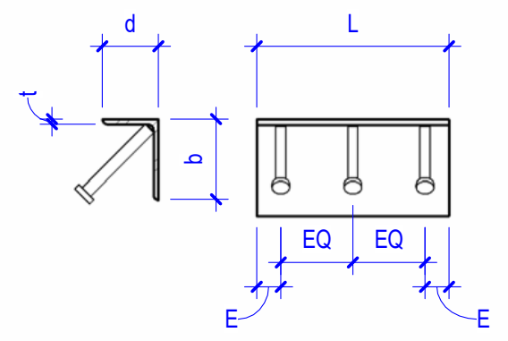
Embed Plate - Type G (WP-G)

MARK	PLATE				ANCHORS (HSA)			
	W	L	t	E	ROWS	DIA.	LENGTH	QUANTITY
WP-G12X16.1	12"	16"	1/2"	1 1/2"	3	5/8"	4"	6
WP-G12X18.1	12"	18"	1/2"	1 1/2"	3	5/8"	4"	6
WP-G12X20.1	12"	20"	1/2"	1 1/2"	4	5/8"	4"	8
WP-G12X22.1	12"	22"	1/2"	1 1/2"	4	5/8"	4"	8
WP-G12X24.1	12"	24"	1/2"	1 1/2"	5	5/8"	4"	10
WP-G12X28.1	12"	28"	1/2"	1 1/2"	5	5/8"	4"	10
WP-G12X30.1	12"	30"	1/2"	1 1/2"	6	5/8"	4"	12
WP-G12X30.1	12"	33"	1/2"	1 1/2"	6	5/8"	4"	12
WP-G12X36.1	12"	36"	3/4"	1 1/2"	7	3/4"	4"	14
WP-F12X40.1	12"	40"	3/4"	1 1/2"	7	3/4"	4"	14
WP-F12X48.1	12"	48"	3/4"	1 1/2"	8	3/4"	4"	16



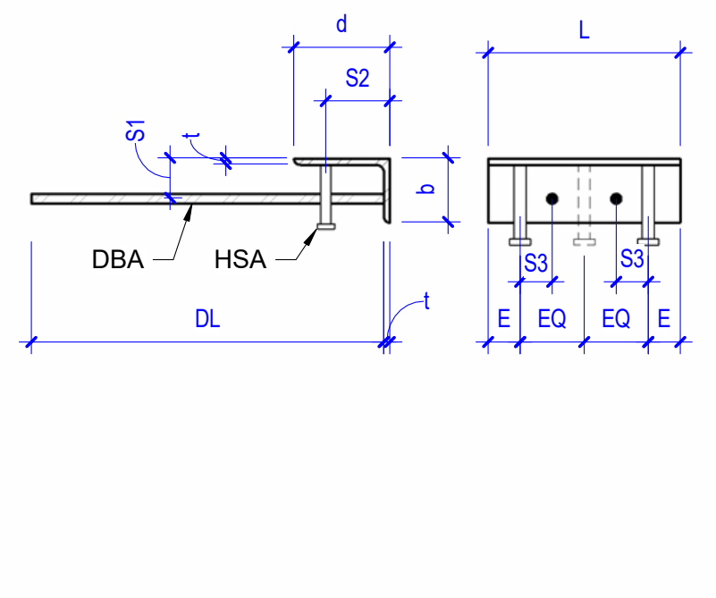
Embed Plate - Type H (WP-H)

MARK	PLATE					ANCHORS (HSA)			
	W	L	t	E1	E2	ROWS	DIA.	LGTH	QTY
WP-H8X14.1	8"	14"	1/2"	1 1/2"	3"	3	1/2"	4"	6
WP-H12X16.1	12"	16"	1/2"	1 1/2"	3"	3	1/2"	4"	6
WP-H12X18.1	12"	18"	1/2"	1 1/2"	3"	3	1/2"	4"	6
WP-H12X20.1	12"	20"	1/2"	1 1/2"	3"	4	5/8"	4"	8
WP-H12X22.1	12"	22"	1/2"	1 1/2"	3"	4	5/8"	4"	8
WP-H12X24.1	12"	24"	1/2"	1 1/2"	3"	5	5/8"	4"	10
WP-H12X28.1	12"	28"	1/2"	1 1/2"	3"	5	5/8"	4"	10
WP-H12X30.1	12"	30"	1/2"	1 1/2"	3"	6	5/8"	4"	12
WP-H12X33.1	12"	33"	1/2"	1 1/2"	3"	6	5/8"	4"	12
WP-H12X36.1	12"	36"	3/4"	1 1/2"	3"	7	3/4"	4"	14
WP-H12X40.1	12"	40"	3/4"	1 1/2"	3"	7	3/4"	4"	14
WP-H12X48.1	12"	48"	3/4"	1 1/2"	3"	8	3/4"	4"	16



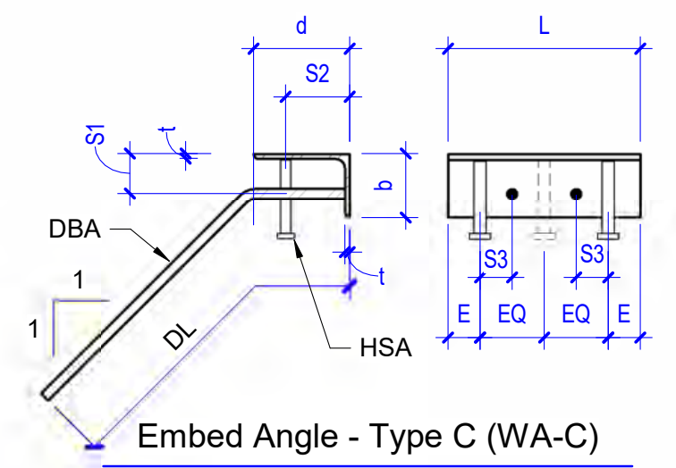
Embed Angle - Type A (WA-A)

MARK	ANGLE					ANCHORS (HSA)		
	b	d	t	L	E	DIA.	LGTH	QUANTITY
WA-A21/2x2 1/2.1	2 1/2"	2 1/2"	1/4"	CONT	1 1/2"	5/8"	4"	12" O.C.
WA-A3X3.1	3"	3"	1/4"	10"	1 1/2"	5/8"	4"	3
WA-A4X4.1	4"	4"	5/16"	1'-0"	1 1/2"	5/8"	4"	3
WA-A5X3 1/2.1	5"	3 1/2"	5/16"	CONT	1 1/2"	3/4"	5"	12" O.C.
WA-A5X5.1	5"	5"	3/8"	1'-0"	1 1/2"	3/4"	5"	3
WA-A6X6.1	6"	4"	3/8"	1'-0"	1 1/2"	3/4"	6"	3



Embed Angle - Type B (WA-B)

MARK	ANGLE					ANCHORS (DBA)				ANCHORS (HSA)				
	b	d	t	L	E	DIA.	DL	QTY	S1	DIA.	LGTH	S2	S3	QTY
WA-B6X4.12	4"	6"	5/16"	1'-0"	2"	5/8"	2'-0"	2	2 1/2"	5/8"	4"	4"	2"	2
WA-B6X4.12	4"	4"	5/16"	1'-0"	2"	5/8"	C2	2	2 1/2"	5/8"	4"	4"	2"	2
WA-B5X5.12	5"	5"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3"	3/4"	5"	3"	2"	2
WA-B6X6.12	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.16	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.18	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.20	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.22	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.24	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.28	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.30	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.33	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.36	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.40	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2
WA-B6X6.48	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2



Embed Angle - Type C (WA-C)

MARK	ANGLE					ANCHORS (DBA)				ANCHORS (HSA)				
	b	d	t	L	E	DIA.	DL	QTY	S1	DIA.	LGTH	S2	S3	QTY
WA-C6X4.1	4"	6"	5/16"	1'-0"	2"	5/8"	2'-0"	2	2 1/2"	5/8"	4"	4"	2"	2
WA-C6X4.2	4"	6"	5/16"	1'-0"	2"	5/8"	1'-6"	2	2 1/2"	5/8"	4"	4"	2"	2
WA-C5X5.1	5"	5"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3"	3/4"	5"	3"	2"	2
WA-C6X6.1	6"	6"	3/8"	1'-0"	2"	3/4"	2'-0"	2	3 1/2"	3/4"	6"	4"	2"	2

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POST TENSION CONCRETE GENERAL NOTES:

PT1.0 GENERAL:

1. MATERIALS, INSTALLATION, STRESSING AND FINISHING SPECIFICATIONS SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF PTI's *SPECIFICATION FOR UNBONDED SINGLE STRAND TENDONS* OR ACI 423.7, WHICH EVER IS MORE RESTRICTIVE.
2. INSTALLATION DRAWINGS, THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND CORRESPONDING ELONGATION CALCULATIONS, BOTH BEARING THE SEAL AND SIGNATURE OF THE ENGINEER THAT PREPARED THEM, TO THE SEOR FOR REVIEW AND APPROVAL A MINIMUM OF (4) WEEKS PRIOR TO SCHEDULED FABRICATION OF THE TENDONS. INSTALLATION DRAWINGS SHALL SHOW THE TENDON LAYOUT, ALL ANCHORAGE LOCATIONS AND TENDON SUPPORTS WITH ALL DETAILS NECESSARY TO ENSURE PROPER INSTALLATION. THE REVIEW OF THE INSTALLATION DRAWINGS BY THE SEOR IS ONLY FOR GENERAL CONFORMANCE WITH THE INTENT OF THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.
3. MARKING OF TENDON LOCATIONS. IF DESIRED BY OWNERSHIP, THE HORIZONTAL POSITION OF SLAB TENDONS SHALL BE MARKED ON THE SOFFIT OF THE SLAB ACCOMPLISHED BY ATTACHING MARKERS TO THE SLAB FORM WORK BY SPRAY PAINTING THE FORM WORK ALONG THE TENDON PATH JUST PRIOR TO PLACEMENT OF CONCRETE. THE PAINT MARKS WILL TRANSFER TO THE CONCRETE SOFFIT TO PERMANENTLY MARK TENDON LOCATIONS AND CAN BE HIDDEN BY SUSPENDED CEILING SYSTEMS.
4. EMBEDMENTS. WHERE PERMANENT FIXTURES SUCH AS CURTAIN WALL SYSTEMS, HANDRAILS, FIRE PROTECTION EQUIPMENT, LIGHTS AND SECURITY DEVICES MUST BE CONNECTED TO POST-TENSIONED SLABS OR BEAMS, THEY SHALL BE ATTACHED USING APPROPRIATE EMBEDMENTS. NO DISSIMILAR METALS SHALL BE IN CONTACT.
5. FASTENERS AND INSERTS. FASTENERS OR INSERTS SHALL NOT BE SHOT OR DRILLED INTO POST-TENSIONED SLABS AFTER THE CONCRETE IS PLACED UNLESS THERE IS WRITTEN AUTHORIZATION FROM THE SEOR. DRILLED OR POWDER-DRIVEN FASTENERS ARE ONLY PERMITTED IF THEY ARE LOCATED TO AVOID TENDONS AND ANCHORAGES AND CAN BE SHOWN TO NOT SPALL CONCRETE. SPECIAL ATTENTION SHALL BE GIVEN TO BRACES FOR COLUMN AND WALL FORMS TO ENSURE THEY ARE LOCATED BETWEEN TENDON GROUPS.
6. OPENINGS. OPENING, PENETRATION AND INSERT LOCATIONS SHALL BE DETERMINED TO THE FULLEST EXTENT POSSIBLE PRIOR TO TENDON LAYOUT. NO CHANGES SHALL BE MADE IN THE FIELD AND NO CORE DRILLING SHALL BE ALLOWED WITHOUT PRIOR WRITTEN APPROVAL OF THE SEOR.
7. FORM WORK. FOR MULTI-LEVEL STRUCTURES, THE FORM WORK SHALL EXTEND BEYOND THE SLAB EDGE OR SCAFFOLDING SHALL BE PROVIDED TO ALLOW ADEQUATE ROOM FOR STRESSING OPERATIONS.
8. FIELD INSTALLATION REVIEW. THE TENDON, MILD REINFORCING AND SLAB EMBEDMENT INSTALLATION SHALL BE MADE AVAILABLE FOR REVIEW BY EOR AS WELL AS INSPECTED BY OWNER'S SPECIAL INSPECTOR PRIOR TO CONCRETE PLACEMENT. THE SEOR SHALL BE NOTIFIED A MINIMUM (7) DAYS IN ADVANCE OF THE DATE ALL INSTALLATIONS OF TENDONS, MILD REINFORCING AND SLAB EMBEDMENTS WILL BE COMPLETED.
9. SPECIAL INSPECTION. SPECIAL INSPECTION SHALL BE CARRIED OUT IN ACCORDANCE WITH THE IBC OR APPLICABLE STATE OR LOCAL BUILDING CODE.
10. STRUCTURAL OBSERVATION. STRUCTURAL OBSERVATION SHALL BE PERFORMED BY THE SEOR OR DESIGNATED REPRESENTATIVE IF REQUIRED BY GOVERNING STATE OR LOCAL BUILDING CODE.
11. FIELD FOREMAN. THE FIELD FOREMAN RESPONSIBLE FOR THE INSTALLATION, STRESSING AND FINISHING OF ALL POST-TENSIONED MATERIAL (INCLUDING ALL MILD REINFORCEMENT NECESSARY FOR PROPER INSTALLATION OF THE POST-TENSIONING TENDONS) SHALL BE PTI-CERTIFIED WITH A MINIMUM 5 YEARS EXPERIENCE IN THIS CAPACITY FOR THIS TYPE OF CONSTRUCTION.

PT1.1 MATERIALS:

1. STRAND QUALITY. POST TENSIONING TENDONS SHALL USE LOW-RELAXATION STRAND CONFORMING TO THE FOLLOWING:
   A. SEVEN WIRE STRAND ASTM A416
   B. MINIMUM ULTIMATE STRESS: 270 KSI
   C. ½" DIAMETER STRAND AREA: 0.153 SQUARE INCHES
      - ONE SAMPLE OF EACH REEL OR HEAT SHALL BE TESTED BY AN APPROVED LABORATORY. MILL CERTIFICATES MAY BE SUBSTITUTED IN LIEU OF INDEPENDENT TESTING IF APPROVED BY THE SEOR. TEST RESULTS OR MILL CERTIFICATES SHALL BE SUBMITTED TO THE SEOR BEFORE ANY TENDONS ARE FABRICATED OR INSTALLED.
2. POST TENSIONING HARDWARE QUALITY. ALL ANCHORAGES, COUPLERS AND MISCELLANEOUS HARDWARE SHALL BE STANDARD PRODUCTS APPROVED BY GOVERNING AGENCIES AND THE SEOR.
3. SHEATHING. UNBONDED TENDONS SHALL BE ENCASED IN A SLIPPAGE SHEATHING THAT SHALL BE MANUFACTURED BY A PROCESS THAT PROVIDES WATERTIGHT ENCASEMENT OF THE CORROSION INHIBITING COATING MATERIAL (POST TENSIONING COATING) SO AS TO PREVENT THE INTERNAL MIGRATION OF ANY WATER AND/OR MOISTURE. SHEATHING SHALL BE OF SUFFICIENT STRENGTH AND DURABILITY TO RESIST DAMAGE DURING NORMAL FABRICATION, TRANSPORTATION, INSTALLATION AND CONCRETE PLACEMENT OPERATIONS. MINIMUM SHEATHING THICKNESS SHALL BE 0.050 INCHES. TEARS IN THE SHEATHING SHALL BE REPAIRED BY REPLACING THE POST TENSIONING COATING AND RESTORING WATER-TIGHTNESS. TENDONS SHALL BE PROTECTED DURING SHIPPING AND HANDLING TO AVOID DAMAGE TO THE TENDON SHEATHING DURING TRANSPORTATION AND OFF-LOADING AT THE PROJECT SITE, AND AVOID EXPOSURE TO DEICING SALTS OR ANY OTHER SIMILAR CORROSIVE ELEMENT.
4. CONCRETE. STRENGTH AND MATERIAL DATA FOR ALL CONCRETE MIXTURE PROPORTIONS TO BE USED ON THE PROJECT SHALL BE PROVIDED TO THE SEOR IN A TIMELY MANNER PRIOR TO THEIR USE.

PT1.2 INSTALLATION:

1. INSTALLATION OF UNBONDED TENDONS. THE CONTRACTOR RESPONSIBLE FOR THE POST TENSIONING SUPPLIER WHO INSTALLS THEIR POST TENSIONING MATERIALS OR FOR THE INDEPENDENT POST TENSIONING INSTALLER SHALL ENSURE THAT THE INSTALLATION CREW MEETS THE STANDARDS SET FORTH IN THE CONTRACT DOCUMENTS. ACI 301 REQUIRES THAT FOR UNBONDED POST TENSIONING, PERSONNEL SHALL BE CERTIFIED IN ACCORDANCE WITH PTI LEVEL 1 UNBONDED PT FIELD INSTALLATION PROGRAM. FOR BONDED POST TENSIONING, PERSONNEL SHALL BE CERTIFIED IN ACCORDANCE WITH PTI LEVEL 1 BONDED PT FIELD PT FIELD INSTALLATION PROGRAM. IN ADDITION, THE CREW FOREMAN SHALL BE CERTIFIED AS LEVEL 2 UNBONDED PT IRONWORKER AND/OR INSPECTOR FOR UNBONDED POST TENSIONING AND LEVEL 2 BONDED PT FIELD SPECIALIST FOR BONDED POST TENSIONING, RESPECTIVELY. IF THE POST TENSIONING SUPPLIER DOES NOT INSTALL POST TENSIONING MATERIALS, THEY SHALL PROVIDE INITIAL JOB SITE TECHNICAL ASSISTANCE TO INSTRUCT THE INSTALLER ON ANY SPECIAL REQUIREMENTS OF THEIR SYSTEM TO ENSURE PROPER INSTALLATION, STRESSING AND FINISHING OF ALL POST TENSIONING MATERIALS.
2. TENDONS. TENDONS SHALL BE SHOP FABRICATED IN A PTI CERTIFIED PLANT WITH PRE-ASSEMBLED FIXED END ANCHORAGES. PLASTIC POCKET FORMERS SHALL BE USED AT ALL STRESSING ENDS TO RECESS THE ANCHORAGE CASTINGS SO THAT THE REQUIRED COVER IS ACHIEVED.
3. TENDON INSTALLATION. FIELD PERSONNEL SHALL BE CERTIFIED IN ACCORDANCE WITH PTI CERTIFICATION LEVEL 1 (UNBONDED PT) FIELD INSTALLATION PROGRAM. CARE SHALL BE TAKEN SO THAT TENDONS ARE LOCATED AND HELD IN THEIR DESIGNATED POSITIONS AS SHOWN ON THE APPROVED INSTALLATION DRAWINGS. EXCEPT AS NOTED ON THE CONSTRUCTION DOCUMENTS OR APPROVED BY THE SEOR, TOLERANCES FOR THE VERTICAL LOCATION OF THE PRE-STRESSING STEEL SHALL BE:
   A. +/- ¼" FOR SLAB THICKNESS LESS THAN 8"
   B. +/- 3/8" FOR CONCRETE THICKNESS BETWEEN 8" AND 24"
   C. +/- ½" FOR CONCRETE THICKNESS GREATER THAN 24"
      - ACCESS SHALL BE PROVIDED TO STRESSING ENDS.
4. TENDON GROUPS (TENDONS GROUPS SHALL BE PLACED TO PROVIDE ADEQUATE CLEARANCE TO MILD REINFORCING AND FACILITATE CONCRETE PLACEMENT):
   A. SLABS LESS THAN 14" THICK = MAXIMUM (4) STRANDS PER FLAT BUNDLE
   B. SLABS 14" THICK TO LESS THAN 24" = MAXIMUM (5) STRANDS PER FLAT BUNDLE
   C. SLABS 24" OR MORE = MAXIMUM (6) STRANDS PER FLAT BUNDLE
   D. BEAMS = MAXIMUM (6) STANDS PER BUNDLE
5. TENDONS OVER COLUMNS. IN TWO-WAY SLAB CONSTRUCTION, A MINIMUM OF (2) TENDONS IN EACH DIRECTION SHALL PASS THROUGH THE CRITICAL SHEAR SECTION AND BETWEEN THE VERTICAL BARS OF THE COLUMNS. IT IS PERMISSIBLE TO REPLACE THE TENDONS WITH BOTTOM NON-PRESTRESSED REINFORCEMENT BETWEEN THE VERTICAL BARS OF THE COLUMN IN EACH DIRECTION. REFER TO TYPICAL DETAILS FOR MORE INFORMATION.
6. TENDON ADJUSTMENTS. SMALL DEVIATIONS IN THE HORIZONTAL SPACING OF THE SLAB TENDONS WILL BE PERMITTED WHEN REQUIRED TO AVOID OPENINGS, INSERTS AND DOWELS WITH SPECIFIC LOCATION REQUIREMENTS. WHERE TENDON LOCATIONS INTERFERE WITH EACH OTHER, ONE TENDON MAY BE MOVED HORIZONTALLY TO AVOID THE INTERFERENCE.
7. TWISTING. TWISTING OR ENTWINING OF INDIVIDUAL TENDONS WITHIN A GROUP IS NOT PERMITTED. ENTWINING GROUPS WITHIN A BEAM IS NOT PERMITTED.
8. VERTICAL PROFILES. PROFILES SHALL CONFORM TO CONTROL POINTS SHOWN ON THE DRAWINGS AND SHALL HAVE AN APPROXIMATE PARABOLIC DRAPE BETWEEN SUPPORTS UNLESS NOTED OTHERWISE. LOW POINTS SHALL BE AT MID-SPAN UNLESS NOTED OTHERWISE. HARPED TENDONS SHALL BE STRAIGHT BETWEEN LOW AND HIGH POINTS.
9. HORIZONTAL PROFILES. IF TENDONS MUST BE CURVED HORIZONTALLY TO AVOID OPENINGS OR OTHER OBSTRUCTIONS, TENDON GROUPS SHALL BE FLARED SUCH THAT A MINIMUM OF 2 INCHES OF CLEAR SEPARATION IS MAINTAINED BETWEEN EACH INDIVIDUAL TENDON. TENDONS SHALL BE FLARED A MAXIMUM OF 1:6 (MINIMUM RADIUS OF 20 FEET). IF TENDONS ARE FLARED AT MORE THAN 1:12, HAIRPINS SHALL BE USED TO TRANSFER THE HORIZONTAL RADIAL FORCE OVER THE CURVED LENGTH TO THE CONCRETE, UNLESS NOTED OTHERWISE ON THE APPROVED INSTALLATION DRAWINGS. REFER TO TYPICAL DETAILS FOR MORE INFORMATION.
10. TENDON COVER. ALL DIMENSIONS SHOWING THE LOCATION OF POST TENSIONING TENDONS ARE TO THE CENTER OF GRAVITY OF THE TENDON (CGS) REFERENCED FROM THE SLAB OR BEAM SOFFIT, UNLESS NOTED OTHERWISE. MINIMUM COVER IS AS SPECIFIED IN THE GENERAL NOTES SECTION OF THESE STRUCTURAL DRAWINGS. IN THE ABSENCE OF SUCH SPECIFICATION, PROVIDE MINIMUM 1 INCH CONCRETE COVER TOP AND 3/4 INCH COVER BOTTOM AT SLABS AND 3 INCH COVER TOP AND BOTTOM AT BEAMS, TYPICAL.
11. MINIMUM CHAIRING FOR SLAB TENDONS. TENDONS SHALL BE SUPPORTED ON NON PRESTRESSED REINFORCEMENT SPACED A MAXIMUM 4 FEET ON CENTER AND SHALL BE SECURED TO POSITIONING DEVICES AT EACH TENDON / SUPPORT BAR INTERSECTION TO ENSURE CORRECT VERTICAL AND HORIZONTAL LOCATION IS MAINTAINED DURING CONCRETE PLACEMENT. SUPPORTS SHALL BE INSTALLED TO PREVENT EXCESSIVE MOVEMENT DURING CONCRETE PLACEMENT.
12. SUPPORT BARS. SUPPORTS SHALL BE MINIMUM #4 BARS. SUPPORT BARS SPANNING ACROSS COLUMN CAPITALS SHALL BE OF ADEQUATE SIZE AND GENERALLY PLACED PARALLEL TO BANDED TENDON GROUPS AND BELOW THE LOWER LAYER OF SCHEDULED TOP REINFORCEMENT. ALL SUPPORT BARS SHALL HAVE 24 INCH MINIMUM SPLICE LENGTH.
13. ANCHORAGES. ANCHORAGE DEVICES SHALL BE RECESSED MINIMUM (2) INCHES WITH (2) CONTINUOUS #4 BACKUP BARS PLACED BEHIND ALL ANCHORAGES UNLESS NOTED OTHERWISE.

PT1.2 INSTALLATION (CONTINUED):

14. BLOCK-OUTS AND STRESSING POCKETS. ALL BLOCK-OUT OR STRESSING POCKETS REQUIRED FOR ACCESS TO ANCHORAGES IN BEAMS OR SLABS SHALL BE ADEQUATELY REINFORCED SO AS TO NOT DECREASE THE STRENGTH OF THE STRUCTURE. ALL BLOCK-OUT AND STRESSING POCKETS SHALL BE SEALED IN SUCH A MANNER AS TO ELIMINATE WATER LEAKAGE THROUGH OR INTO THE BLOCK-OUT POCKET. THE LOCATION OF ALL BLOCK-OUTS AND STRESSING POCKETS SHALL BE REVIEWED AND APPROVED BY THE SEOR.
15. PIPES. PLASTIC OR METAL CONDUITS MAY BE EMBEDDED IN THE SLAB PROVIDING THE FOLLOWING CRITERIA ARE MET:
   A. THE OUTSIDE DIAMETER OF THE CONDUIT DOES NOT EXCEED ¼ THE SLAB THICKNESS OR 2 INCHES, WHICHEVER IS LESS;
   B. CONDUITS SHALL BE LOCATED WITHIN THE MIDDLE 1/3 OF THE SLAB THICKNESS WITH NO EXCEPTIONS;
   C. CENTER TO CENTER SPACING OF THE CONDUITS IS NOT LESS THAN 3X THE DIAMETER OF THE LARGEST CONDUIT WITH NO MORE THAN (3) CONDUITS PER (6) FOOT WIDTH OF SLAB;
   D. CONDUITS DO NOT CONTACT, INTERRUPT OR DISPLACE POST TENSIONING TENDONS OR MILD STEEL REINFORCING;
   E. NO CONDUIT MAY BE PLACED WITHIN A DISTANCE EQUAL TO THE LARGEST COLUMN DIMENSION MEASURED FROM FACE OF COLUMN UNLESS APPROVED IN WRITING BY THE SEOR.
      - IT IS UNDESIRABLE TO HAVE A LARGE NUMBER OF CONDUITS ENTERING THE SLAB AT ONE LOCATION. IF THIS OCCURS, CONDUITS MUST BE FANNED OUT IMMEDIATELY. ADDITIONAL MILD REINFORCEMENT SHALL BE ADDED TOP AND BOTTOM UNTIL ITEM (d) ABOVE IS MET. AT A MINIMUM, PROVIDE ADDED #4@8 EACH WAY (T). FINAL ADDED REINFORCEMENT TO BE PROVIDED ON A CASE BY CASE BASIS BY THE SEOR BASED ON ACTUAL FIELD CONDITIONS.
16. PENETRATIONS. PENETRATIONS SHALL NOT BE PERMITTED IN BEAMS OR DROP CAPS UNLESS SPECIFICALLY INDICATED ON POST TENSIONING SHOP DRAWINGS OR TYPICAL DETAILS AND APPROVED BY THE SEOR.

PT1.3 CONCRETE PLACEMENT:

1. CONCRETE PLACEMENT. PRIOR TO CONCRETE PLACEMENT, FORMS SHALL BE CLEANED OF ALL DEBRIS AND DIRT. WHEN CONCRETE IS PLACED IN POST TENSIONED SLABS, SPECIAL CARE SHALL BE TAKEN AT COLUMN DROP CAPS AND DROP PANELS. THE PUMP HOSE SHALL BE INSERTED INTO THE DROP CAP OR PANEL BELOW THE REINFORCEMENT AND FILLED UNTIL THE CONCRETE REACHES THE TOP REINFORCING LAYER. CONCRETE ELEVATION SHALL BE MONITORED TO AVOID FLOTATION OF TOP REINFORCING. AFTER THE DROP PANEL OR DROP CAP IS FULL OF CONCRETE, CONCRETE SHALL BE PLACED OVER THE TOP REINFORCING LAYER TO THE SPECIFIED SLAB THICKNESS. CONCRETE SHALL BE ADEQUATELY VIBRATED IN AND AROUND COLUMN DROP CAPS AND DROP PANELS.
2. PUMPED CONCRETE. IF CONCRETE IS PLACED WITH PUMPS, HOSES SHALL NOT BE ALLOWED TO REST ON REINFORCEMENT BE IT POST TENSIONING TENDONS OR NON-PRESTRESSED REINFORCEMENT.
3. CONCRETE CONSOLIDATION. THE CONTRACTOR SHALL TAKE PRECAUTIONS TO ENSURE COMPLETE CONSOLIDATION AND DENSIFICATION OF CONCRETE BEHIND ALL POST-TENSIONING ANCHORAGES. PROPER CARE SHALL BE TAKEN NOT TO ALLOW THE VIBRATOR TO COME INTO CONTACT OF THE NON-PRESTRESSED REINFORCING OR POST TENSIONING TENDONS.
4. CHLORIDES. MAXIMUM WATER SOLUBLE CHLORIDE ION CONTENT WITHIN THE CONCRETE SHALL NOT EXCEED 0.06% BY WEIGHT OF CEMENT PER ACI 318 LATEST EDITION.

PT1.4 STRESSING OF TENDONS:

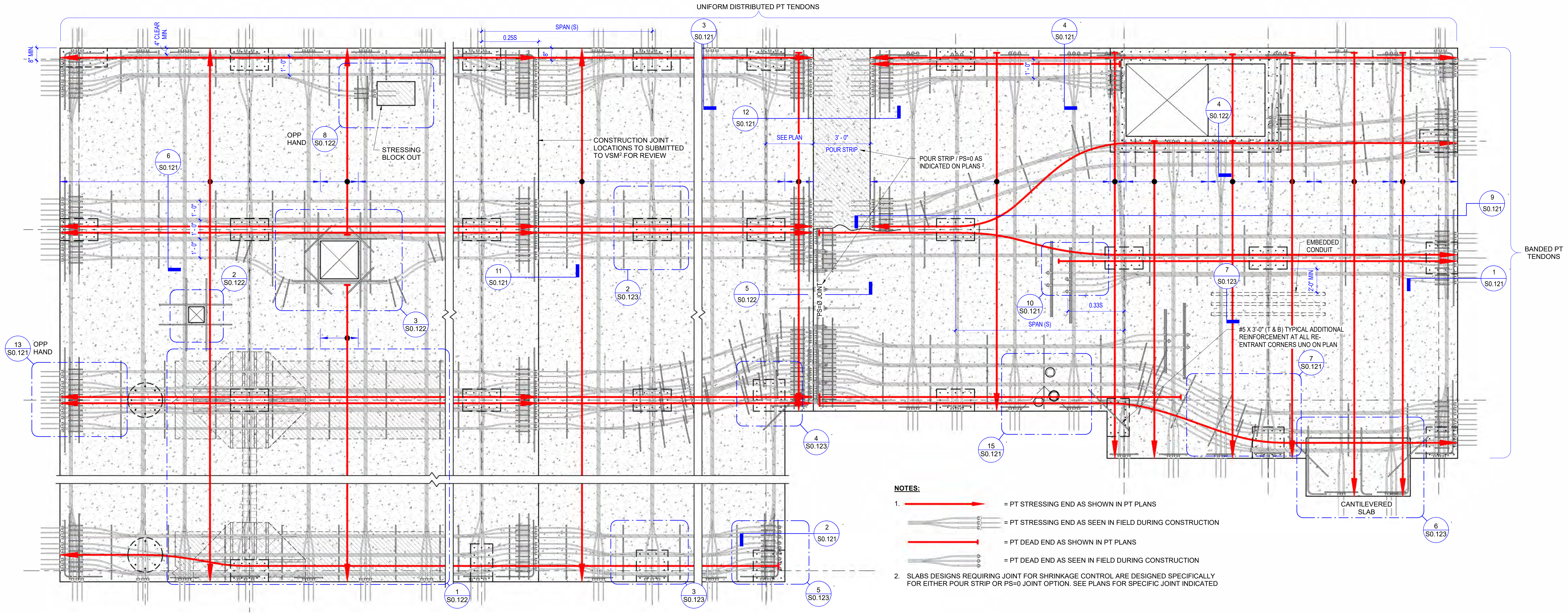
1. TENDON STRESSES SHALL CONFORM TO THE FOLLOWING MAXIMUM VALUES:
   A. TENDON JACKING STRESS: $0.94f_{py} < 0.80f_{pu}$
   B. TENDON STRESS IMMEDIATELY AFTER PRE-STRESS TRANSFER: $0.82f_{py} < 0.74f_{pu}$
   C. TENDON STRESS AT ANCHORAGES AND COUPLERS AFTER ANCHORAGE SET: $0.70f_{pu}$
2. FINAL EFFECTIVE FORCE. FORCES SHOWN ON THE STRUCTURAL DRAWINGS ARE FINAL EFFECTIVE FORCES AFTER SHORT AND LONG TERM LOSSES. THE POST TENSIONING SUPPLIER SHALL PROVIDE SHORT AND LONG TERM LOSS CALCULATIONS WITH THE ASSOCIATED POST TENSIONING SHOP DRAWING SUBMITTAL PACKAGE FOR REVIEW BY THE SEOR.
3. CONCRETE AT STRESSING: CONCRETE SHALL REACH A MINIMUM AVERAGE COMPRESSIVE STRENGTH OF 3,000 PSI PRIOR TO STRESSING. THE MINIMUM CONCRETE STRENGTH SHALL BE ESTABLISHED BY BREAKING A MINIMUM OF (2) 6x12 OR (3) 4x8 CONCRETE TEST CYLINDERS CURED AT THE JOBSITE UNDER CONDITIONS SIMILAR TO THE CURING OF POST-TENSIONED ELEMENTS WITH AN AVERAGE OF 3,000 PSI WITH NO SINGLE TEST RESULT FALLING BELOW 2,800 PSI. STRESSING SHALL NOT COMMENCE UNTIL THE CONCRETE REACHES THE SPECIFIED STRENGTH. TENDONS SHOULD BE STRESSED WITHIN 72 HOURS AFTER CONCRETE PLACEMENT TO MINIMIZE EARLY AGE CONCRETE CRACKING. THIS MAY NOT APPLY TO ELEMENTS THAT ARE STAGE STRESSED WHERE ONLY A PORTION OF TOTAL POST TENSIONING FORCE IS APPLIED WITHIN THE 72 HOURS.
4. CALIBRATION. EACH STRESSING JACK AND GAUGE COMBINATION SHALL BE INDIVIDUALLY IDENTIFIED AND CALIBRATED AS A UNIT TO KNOWN STANDARDS AT INTERVALS NOT EXCEEDING (6) MONTHS. COPIES OF CALIBRATION CERTIFICATES FOR EACH JACK AND GAUGE COMBINATION BEING USED SHALL BE SUBMITTED TO THE SEOR FOR REVIEW AND REVIEWED COPIES KEPT AT THE JOBSITE TO BE MADE AVAILABLE UPON REQUEST.
5. TENDON STRESSING. THE STRESSING OPERATION SHALL BE UNDER THE IMMEDIATE CONTROL OF A PERSON WHO IS A PTI-CERTIFIED INSTALLER EXPERIENCE IN THIS TYPE OF WORK. CONTINUOUS INSPECTION AND RECORDING AND RECORDING OF ELONGATIONS AND STRESSING EQUIPMENT GAUGE PRESSURES BY AN INDEPENDENT THIRD PARTY INSPECTOR HIRED BY THE OWNER IS REQUIRED DURING ALL STRESSING OPERATIONS. THE INDEPENDENT INSPECTOR SHALL BE CERTIFIED UNDER THE PTI *CERTIFIED INSPECTOR PROGRAM*.

PT1.4 STRESSING OF TENDONS (CONTINUED):

6. STRESSING SEQUENCE. TO PREVENT OVERLOADING OF THE FORMING SYSTEM DURING STRESSING OPERATIONS, THE TYPE OF FORM-WORK SYSTEM BEING USED SHALL BE CONSIDERED WHEN DETERMINING THE APPROPRIATE STRESSING SEQUENCE. IN GENERAL, UNIFORMLY DISTRIBUTED TENDONS WILL BE STRESSED BEFORE BANDED TENDONS IN TWO-WAY SLAB CONSTRUCTION. SLAB TENDONS SHALL BE STRESSED BEFORE BEAM TENDONS IN ONE-WAY SLAB CONSTRUCTION. SPECIAL CONSIDERATION SHALL BE GIVEN TO TRANSFER GIRDERS. ADDITIONAL STRESSING SEQUENCING REQUIREMENTS SHALL BE SPECIFIED AS FOLLOWS:
   A. TWO-WAY SLAB SEQUENCE
      a. STRESS CONTINUOUS DISTRIBUTED TENDONS
      b. STRESS CONTINUOUS BANDED TENDONS
      c. STRESS ADDED DISTRIBUTED TENDONS
      d. STRESS ADDED BANDED TENDONS
   B. ONE-WAY SLAB AND BEAM SEQUENCE
      a. STRESS TEMPERATURE TENDONS
      b. STRESS CONTINUOUS UNIFORM SLAB TENDONS
      c. STRESS BEAM TENDONS
7. ELONGATIONS. FIELD READINGS OF ELONGATIONS AND/OR STRESSING FORCES SHALL NOT VARY BY MORE THAN +/-7% FROM CALCULATED VALUES SHOWN ON THE INSTALLATION DRAWINGS. IF THE MEASURED ELONGATIONS VARY FROM CALCULATED VALUES BY MORE THAN +/- 7% FOR MORE THAN (2) CONSECUTIVE READINGS, THE STRESSING OPERATION SHALL BE SUSPENDED UNTIL THE CAUSE OF THE VARIATION FROM SPECIFIED ELONGATION IS DETERMINED AND CORRECTED TO THE SATISFACTION OF THE SEOR. ELONGATION REPORTS SHALL BE SUBMITTED THE SAME DAY THE STRESSING OPERATION IS COMPLETED AND THE ELONGATION REPORT SHALL BE APPROVED OR REJECTED WITHIN 96 HOURS AFTER STRESSING.
8. MEMBER FORCES. THE POST TENSIONING FORCE PROVIDED IN THE FIELD FOR EACH STRUCTURAL MEMBER SHALL NOT BE LESS THAN THE REQUIREMENTS SHOWN ON THE STRUCTURAL DRAWINGS. IN THIS CONTEXT, STRUCTURAL MEMBERS ARE BEAMS OR SLABS EACH SERVING THEIR RESPECTIVE TRIBUTARY AREA.
9. TENDON TAILS. TENDON TAILS WITHIN A SECTION OF THE PROJECT SHALL NOT BE CUT UNTIL ALL POST TENSIONING TENDONS IN THAT SECTION HAVE BEEN SATISFACTORILY STRESSED AND THE SEOR HAS GRANTED APPROVAL IN WRITING. IN ADDITION, ALL TENDON TAILS SHALL BE PROTECTED WITH A PT COATING FILLED CAP WITHIN (24) HOURS OF CUTTING OFF TENDON TAILS. CONNECTION OF THE CAP TO THE ANCHORAGE SHALL BE WATERTIGHT.
10. GROUTING OF ANCHORAGE POCKETS. TO MINIMIZE MOISTURE ACCESS TO TENDON STEEL, ANCHORAGE POCKETS SHALL BE FILLED WITH NON-SHRINK NON-METALLIC GROUT AS SOON AS PRACTICAL AFTER STRESSING BUT NOT MORE THAN (8) HOURS AFTER CUTTING TENDON TAILS. GROUT CONTAINING CHLORIDES OR OTHER CHEMICALS KNOWN TO BE DELETERIOUS TO PRE-STRESSING STEEL SHALL NOT BE USED. PRIOR TO INSTALLING GROUT, THE INSIDE SURFACE OF THE POCKET SHALL BE COATED OR SPRAYED WITH AN EPOXY RESIN BONDING AGENT. CARE SHALL BE TAKEN TO PREVENT CONTAMINATION OF THE ANCHORAGE RECESS CONCRETE SURFACE WHICH MAY REDUCE THE BONDING CAPACITY OF THE GROUT.
11. TENDON FINISHING REPORTS. A TENDON FINISHING REPORT(S) SHALL BE COMPLETED BY THE INDEPENDENT THIRD PARTY INSPECTOR WITH OFFICIAL RECORD SUBMITTED TO THE SEOR FOR ALL TENDONS CONTAINING, AT A MINIMUM, THE FOLLOWING INFORMATION:
   A. PROJECT IDENTIFICATION NAME AND NUMBER
   B. INSPECTION DATE
   C. INSPECTOR'S NAME AND TITLE
   D. TENDON MARK
   E. VERIFICATION THAT PROCEDURES (9) AND (10) ABOVE HAVE BEEN COMPLETED.
12. SHORING. THE SHORING SUPPORTING SLABS AND BEAMS MAY BE STRIPPED WHEN ALL TENDONS HAVE BEEN SATISFACTORILY STRESSED AND THE SEOR'S HAS GRANTED APPROVAL IN WRITING. RE-SHORE IN ACCORDANCE WITH APPROVED SHORING PLANS. SHORING REQUIREMENTS FOR STAGE STRESSED TRANSFER GIRDERS SHALL BE AS NOTED ON THE STRUCTURAL DRAWINGS AND/OR APPROVED SHORING DRAWINGS.
13. IN AREAS SUPPORTING A PARTIAL SPAN SUCH AS NEAR POUR STRIPS OR CONSTRUCTION JOINTS, THE SHORING IN THE PARTIAL SPAN SHALL STAY IN PLACE UNTIL THE REMAINING SECTION OF THE SPAN HAS BEEN PLACED AND STRESSED. IN CASES WHERE THE CONSTRUCTION JOINT OR POUR STRIP ARE LOCATED AT OR NEAR THE ¼ SPAN OR THE SLAB, BEAM, OR BOTH, THE IMMEDIATE BACK-SPAN SHALL REMAIN SHORED UNTIL THE ADJACENT SPAN IS COMPLETED. REFER TO THE CONSTRUCTION DOCUMENTS AND THE POST TENSIONING INSTALLATION DRAWINGS FOR SUCH LOCATIONS.

PT1.5 INSPECTION FOR PRE-STRESSING REINFORCEMENT:

CONTINUOUS SPECIAL INSPECTION SHALL BE PROVIDED DURING REINFORCING STEEL AND POST TENSIONING TENDON PLACEMENT FOR ALL STRUCTURAL CONCRETE. TENDON INSTALLATION AND INTEGRITY SHALL BE INSPECTED PRIOR TO CONCRETE PLACEMENT. DURING ALL STRESSING OF POST TENSION CONCRETE, THE SPECIAL INSPECTION SHALL INCLUDE RECORDING THE FIELD MEASURED ELONGATION AND JACKING FORCE FOR EACH TENDON. FOR QUALITY CONTROL, INDEPENDENT THIRD PARTY INSPECTION OF TENDON TAILS, END CAP INSTALLATION AND GROUTING OF ANCHORAGE POCKETS IS REQUIRED.



NOTES:

1. ⟶ = PT STRESSING END AS SHOWN IN PT PLANS
   ⟶ = PT STRESSING END AS SEEN IN FIELD DURING CONSTRUCTION
   ⊣ = PT DEAD END AS SHOWN IN PT PLANS
   ⊣ = PT DEAD END AS SEEN IN FIELD DURING CONSTRUCTION
2. SLABS DESIGNS REQUIRING JOINT FOR SHRINKAGE CONTROL ARE DESIGNED SPECIFICALLY FOR EITHER POUR STRIP OR PS=0 JOINT OPTION. SEE PLANS FOR SPECIFIC JOINT INDICATED

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VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
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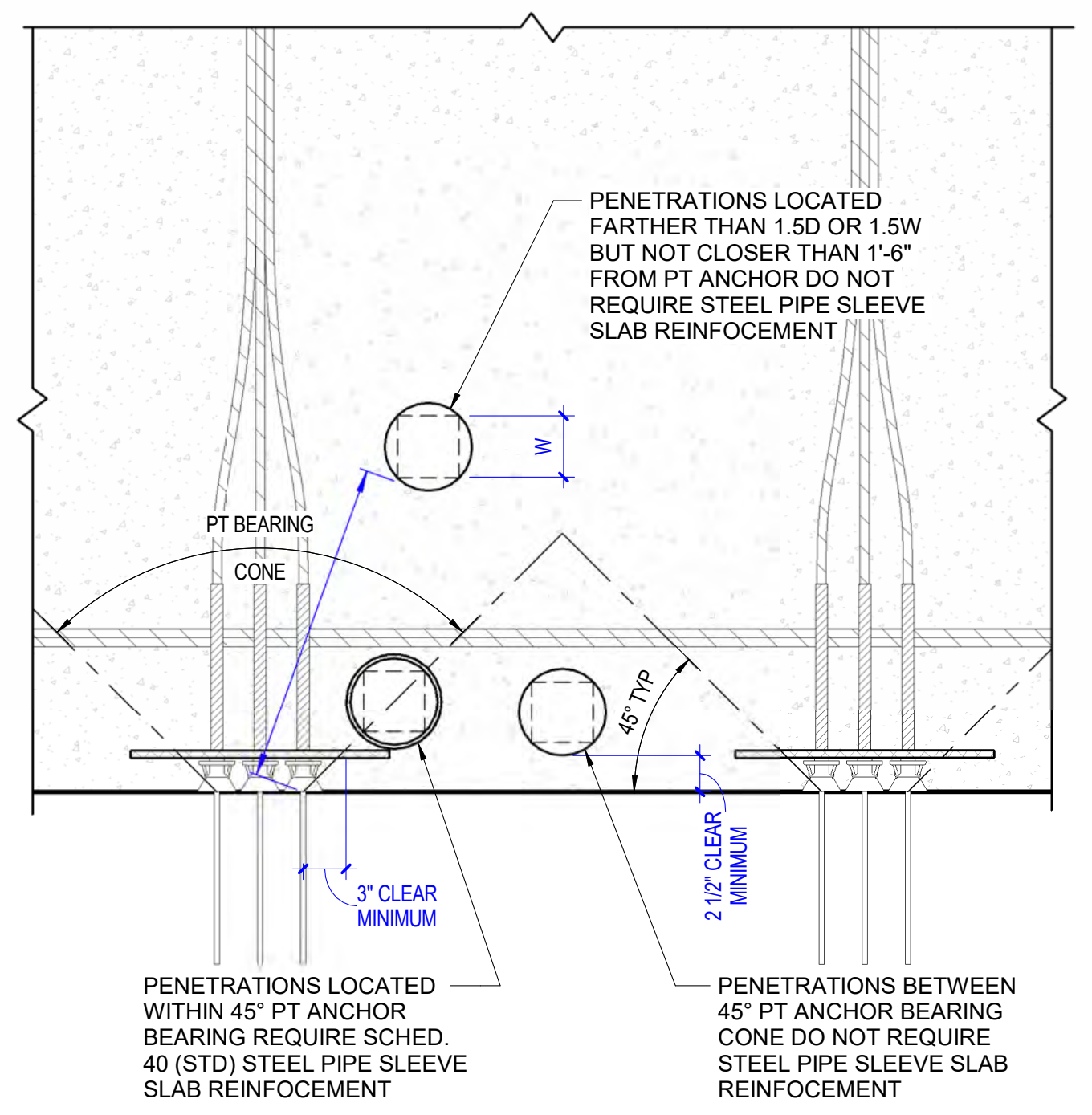
SHEET TITLE:
PT General Notes

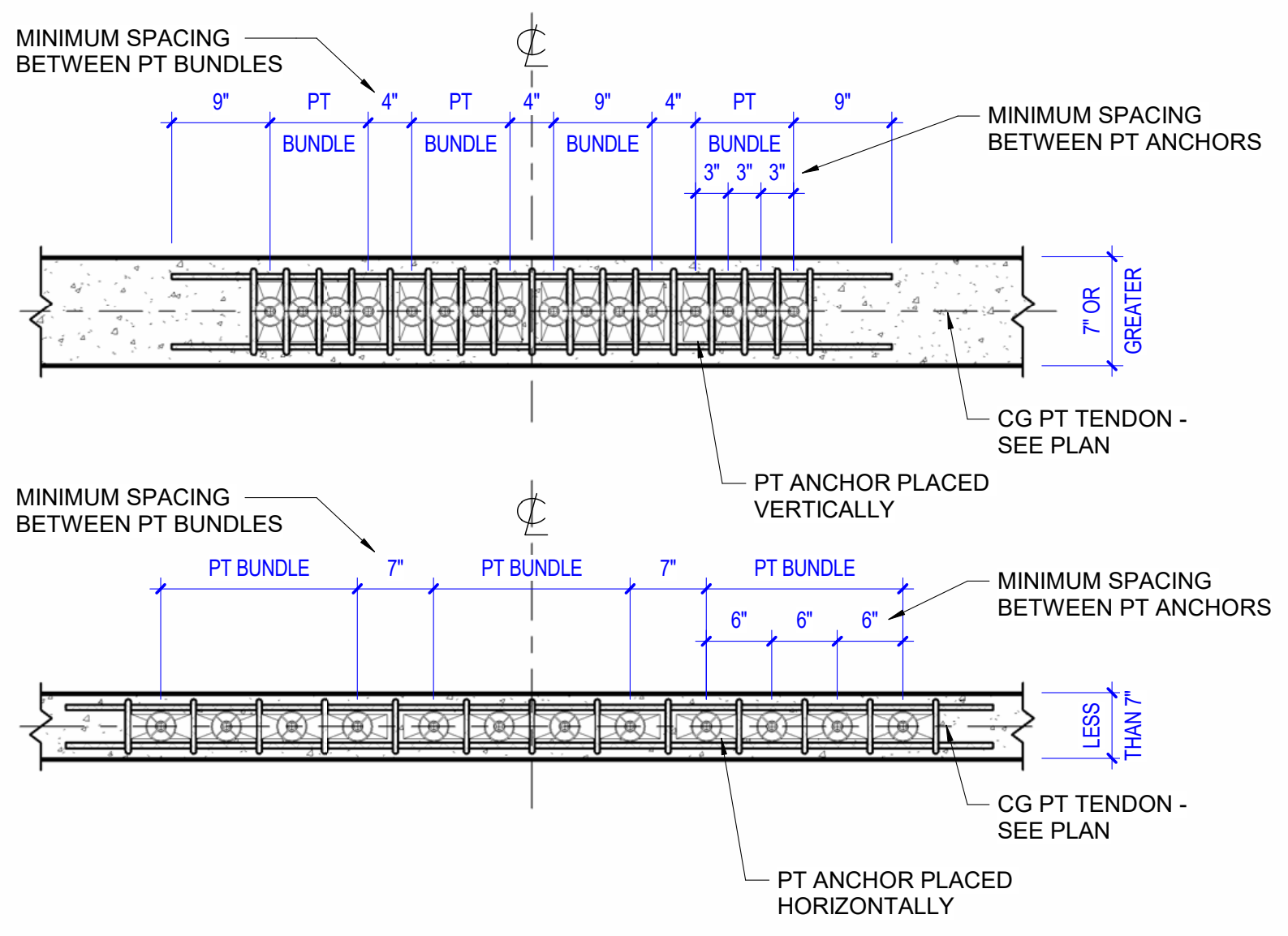
SHEET NUMBER:
S0.120

NOT RELEASED FOR CONSTRUCTION


PENETRATIONS LOCATED FARTHER THAN 1.5D OR 1.5W BUT NOT CLOSER THAN 1'-6" FROM PT ANCHOR DO NOT REQUIRE STEEL PIPE SLEEVE SLAB REINFOCEMENT
W
PT BEARING
CONE
45° TYP
3" CLEAR MINIMUM
2 1/2" CLEAR MINIMUM
PENETRATIONS LOCATED WITHIN 45° PT ANCHOR BEARING REQUIRE SCHED. 40 (STD) STEEL PIPE SLEEVE SLAB REINFOCEMENT
PENETRATIONS BETWEEN 45° PT ANCHOR BEARING CONE DO NOT REQUIRE STEEL PIPE SLEEVE SLAB REINFOCEMENT

15 Penetration Near Tendon Anchorage Zone

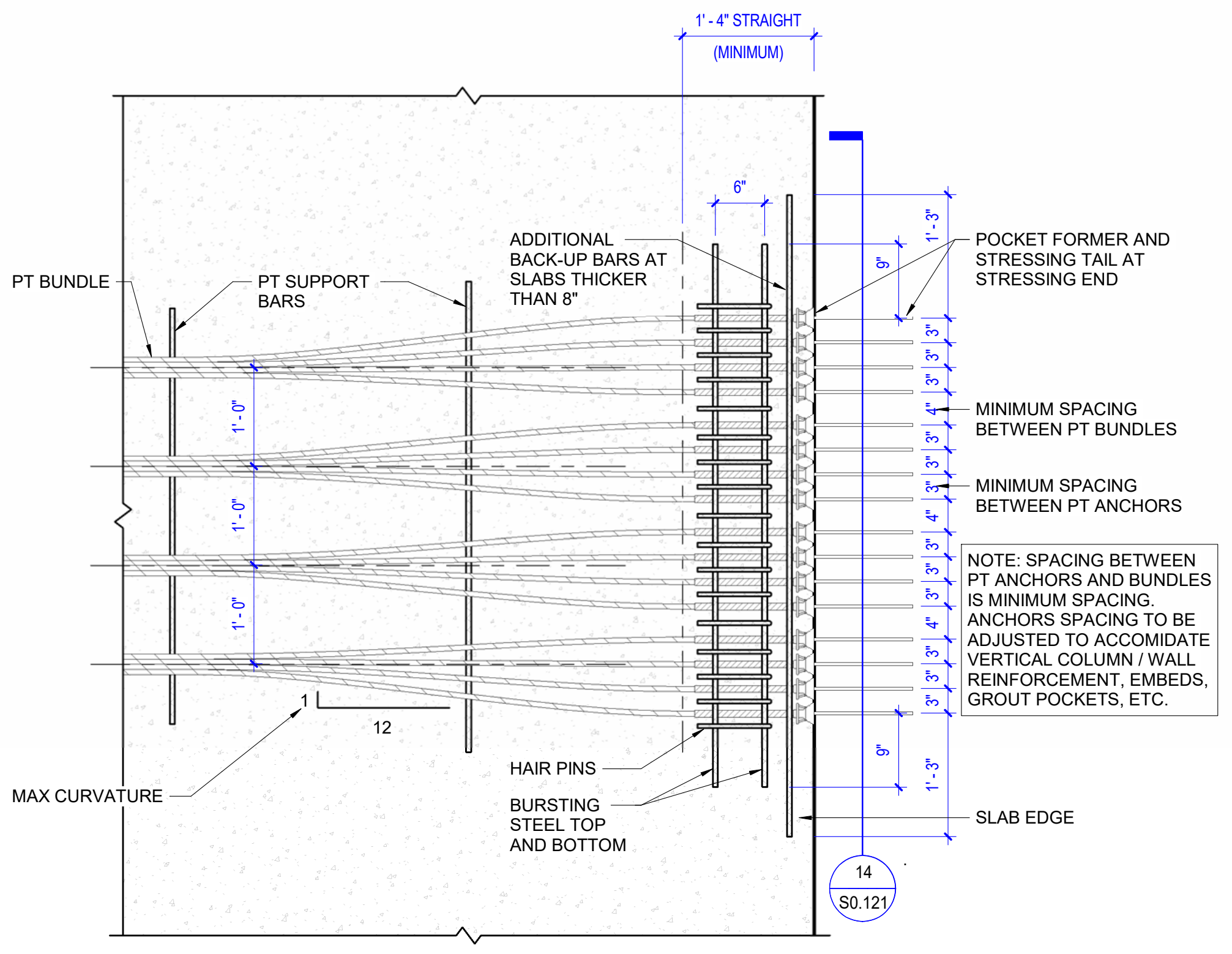
1" = 1'-0"


MINIMUM SPACING BETWEEN PT BUNDLES
9"
PT BUNDLE
4"
PT BUNDLE
4"
9"
BUNDLE
4"
PT BUNDLE
9"
3" 3" 3"
MINIMUM SPACING BETWEEN PT ANCHORS
7" OR GREATER
CG PT TENDON - SEE PLAN
PT ANCHOR PLACED VERTICALLY
MINIMUM SPACING BETWEEN PT BUNDLES
PT BUNDLE
7"
PT BUNDLE
7"
PT BUNDLE
6" 6" 6"
MINIMUM SPACING BETWEEN PT ANCHORS
LESS THAN 7"
CG PT TENDON - SEE PLAN
PT ANCHOR PLACED HORIZONTALLY

NOTE:
1. STRESSING SEQUENCE; START AT CENTER OF STRAND GROUP THEN GO TO RIGHT AND LEFT OF CENTER PER DIAGRAM ABOVE. CONTINUE THIS SEQUENCE UNTIL ALL TENDONS ARE STRESSED.
2. MINIMUM SPACING BETWEEN PT ANCHORS AND PT BUNDLES TO BE INCREASED AS REQUIRED TO AVOID CONFLICT WITH VERTICAL COLUMN REINFORCEMENT, EMBEDS, ETC.

14 Banded PT Elevation at Slab Edge

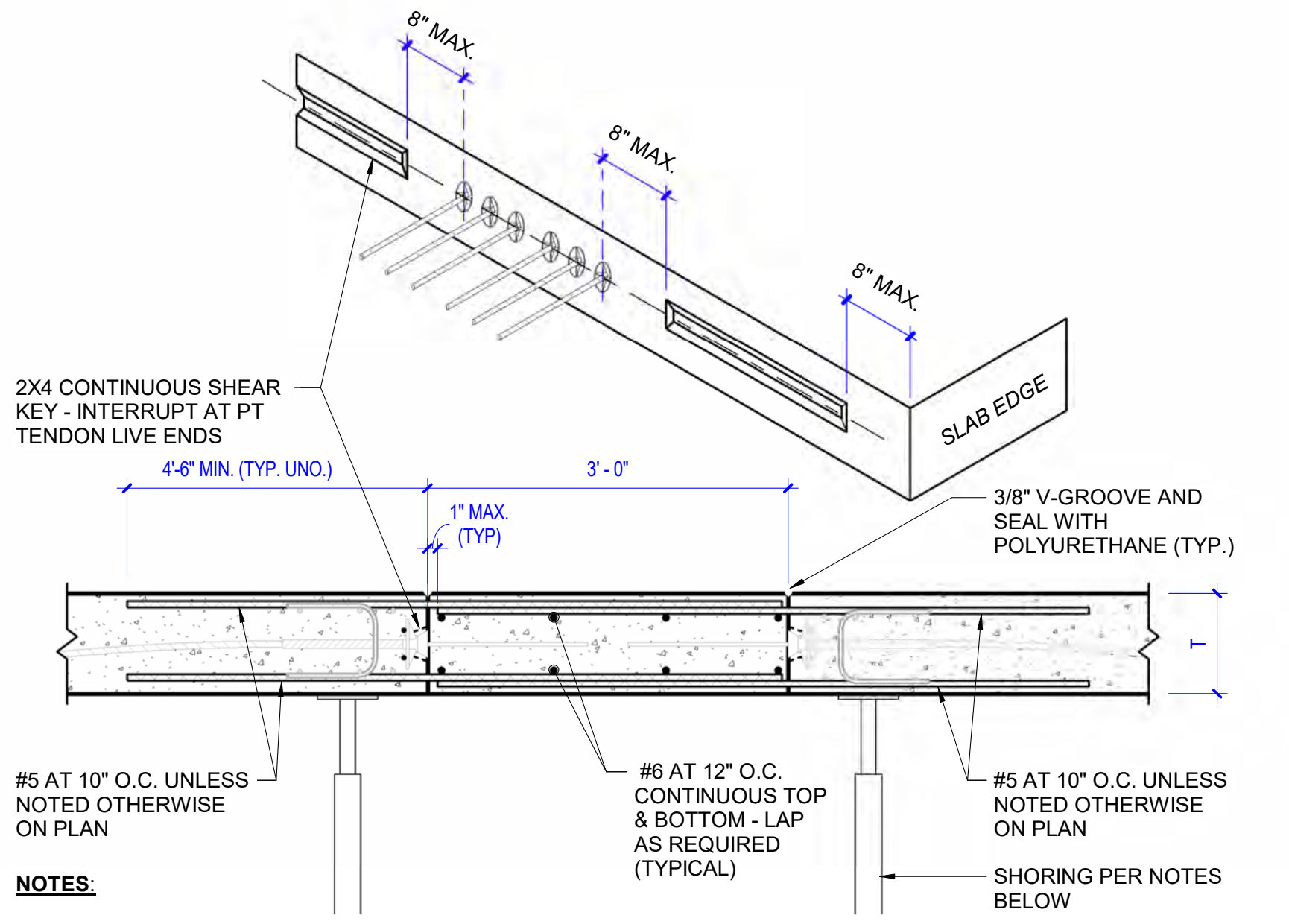
3/4" = 1'-0"


1' - 4" STRAIGHT
(MINIMUM)
6"
PT BUNDLE
PT SUPPORT BARS
ADDITIONAL BACK-UP BARS AT SLABS THICKER THAN 8"
POCKET FORMER AND STRESSING TAIL AT STRESSING END
1' - 3"
9"
3"
4"
MINIMUM SPACING BETWEEN PT BUNDLES
MINIMUM SPACING BETWEEN PT ANCHORS
1' - 0"
1' - 0"
1' - 0"
1
12
MAX CURVATURE
HAIR PINS
BURSTING STEEL TOP AND BOTTOM
SLAB EDGE
14
S0.121

NOTE: SPACING BETWEEN PT ANCHORS AND BUNDLES IS MINIMUM SPACING. ANCHORS SPACING TO BE ADJUSTED TO ACCOMIDATE VERTICAL COLUMN / WALL REINFORCEMENT, EMBEDS, GROUT POCKETS, ETC.

13 Flaring of Banded Tendons at Slab Edge

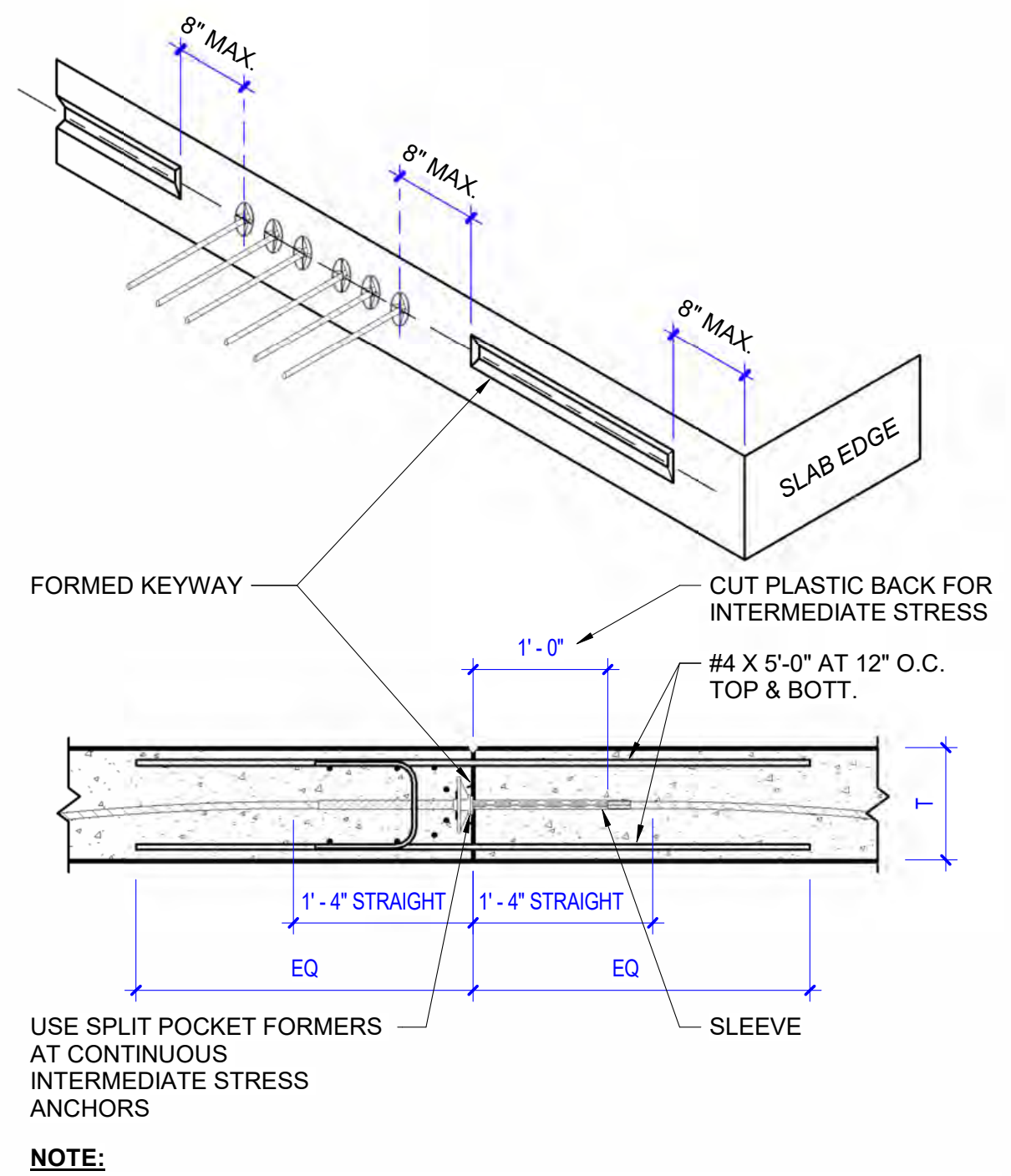
3/4" = 1'-0"


8" MAX.
8" MAX.
8" MAX.
SLAB EDGE
2X4 CONTINUOUS SHEAR KEY - INTERRUPT AT PT TENDON LIVE ENDS
4'-6" MIN. (TYP. UNO.)
3' - 0"
1" MAX. (TYP)
3/8" V-GROOVE AND SEAL WITH POLYURETHANE (TYP.)
T
#5 AT 10" O.C. UNLESS NOTED OTHERWISE ON PLAN
#6 AT 12" O.C. CONTINUOUS TOP & BOTTOM - LAP AS REQUIRED (TYPICAL)
#5 AT 10" O.C. UNLESS NOTED OTHERWISE ON PLAN
SHORING PER NOTES BELOW

NOTES:
1. RETAIN SHORING UNTIL POUR STRIP CONCRETE HAS REACHED SPECIFIED DESIGN COMPRESSIVE STRENGTH, TYP.
2. CONTRACTOR TO NOTE SPECIAL SHORING CONDITIONS ON EITHER SIDE OF THE POUR STRIP AND PROVIDE ADDITIONAL SHORING AS REQUIRED. MORE SPECIFICALLY, IN AREAS SUPPORTING A PARTIAL SPAN NEAR POUR STRIPS, SHORING IN THE PARTIAL SPAN STAY IN PLACE UNTIL THE REMAINING SECTION OF THE SPAN HAS BEEN PLACED AND STRESSED. IN CASES WHERE THE POUR STRIP IS LOCATED AT OR NEAR THE 1/4 SPAN OF THE SLAB, BEAM, OR BOTH, THE IMMEDIATE BACK-SPAN SHALL REMAIN SHORED UNTIL THE POUR STRIP IS PLACED AND HAS REACHED SPECIFIED DESIGN COMPRESSIVE STRENGTH. REFER TO THE CONSTRUCTION DOCUMENTS AND THE POST TENSIONING INSTALLATION DRAWINGS FOR SUCH LOCATIONS.
3. POUR STRIP TO BE PLACED WITH LOW-SHRINK CONCRETE NOT LESS THAN 28 DAYS AFTER STRESSING ON BOTH SIDES. REFER TO GENERAL NOTES FOR LOW-SHRINK CONCRETE REQUIREMENTS.
4. ROUGHEN, CLEAN AND PREPARE JOINTS TO A SATURATED SURFACE DRY CONDITION PRIOR TO POUR STRIP CONCRETE PLACEMENT, TYP.
5. ELIMINATE ACCIDENTAL MISALIGNMENT BETWEEN EDGE OF SLABS THAT ARE TO BE JOINED WITH POUR STRIP. USE MECHANICAL METHODS SUCH AS JACKING AS NECESSARY.
6. PROVIDE WATERPROOFING MEMBRANE IF REQUIRED AND REF. ARCHITECTURAL DRAWINGS FOR MORE INFO.
7. CUT STRESSED TENDON TAILS WITHIN (24) HOURS AFTER RECEIVING EOR APPROVAL AND GROUT STRESSING POCKETS WITHIN (8) HOURS OF CUTTING STRESSED TENDON TAILS, TYP. REFER TO GENERAL NOTES FOR ADDITIONAL REQUIREMENTS AND INFORMATION WHEN THESE CONDITIONS CANNOT BE MET.

12 Section at Pour Strip

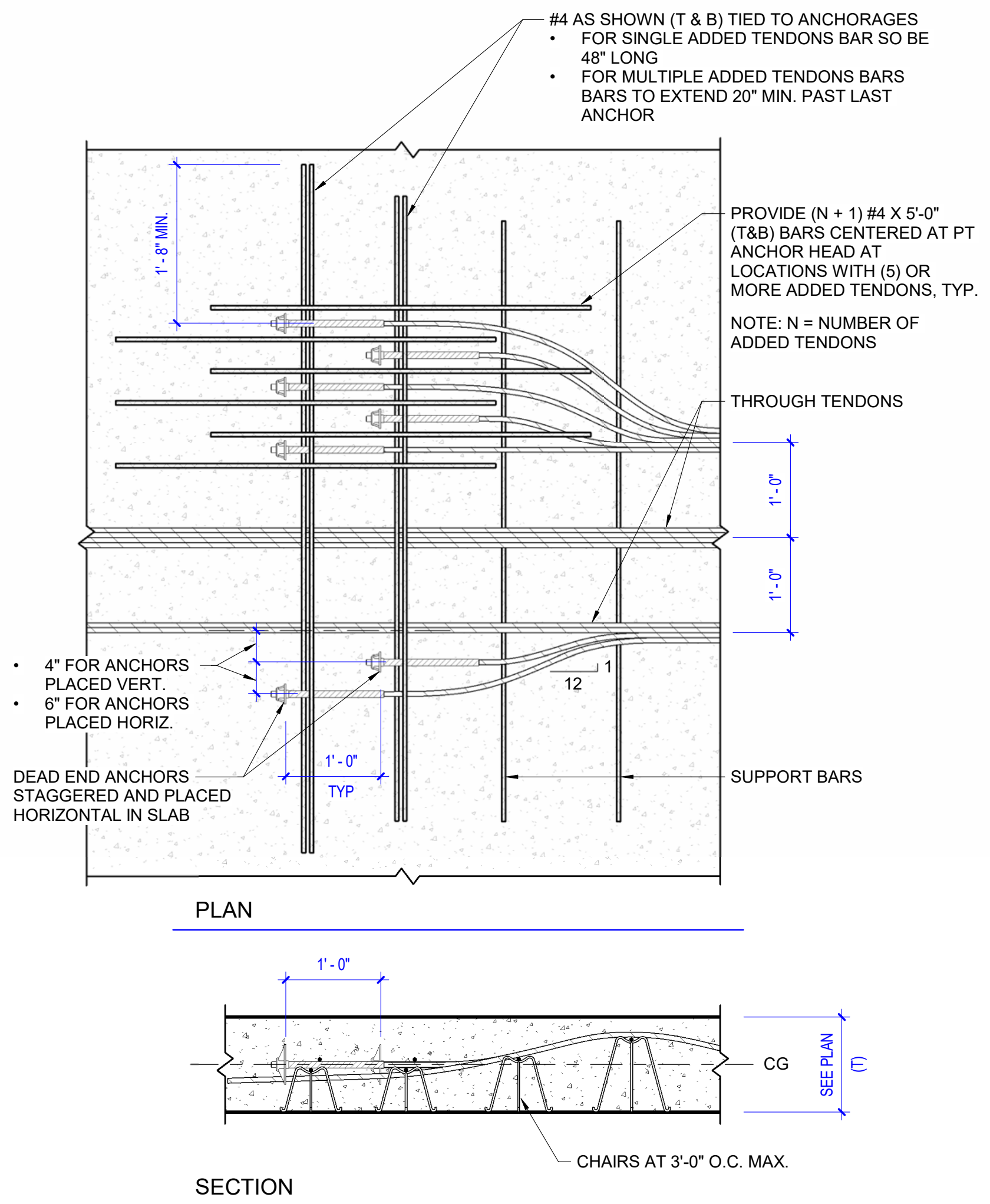
3/4" = 1'-0"


8" MAX.
8" MAX.
8" MAX.
SLAB EDGE
FORMED KEYWAY
CUT PLASTIC BACK FOR INTERMEDIATE STRESS
1' - 0"
#4 X 5'-0" AT 12" O.C. TOP & BOTT.
T
1' - 4" STRAIGHT
1' - 4" STRAIGHT
EQ
EQ
USE SPLIT POCKET FORMERS AT CONTINUOUS INTERMEDIATE STRESS ANCHORS
SLEEVE

NOTE:
LOCATIONS OF PROPOSED SLAB CONSTRIUCTION JOINTS TO BE TO BE SUBMITED TO VSMsq BY CONTACTOR FOR APPROVAL

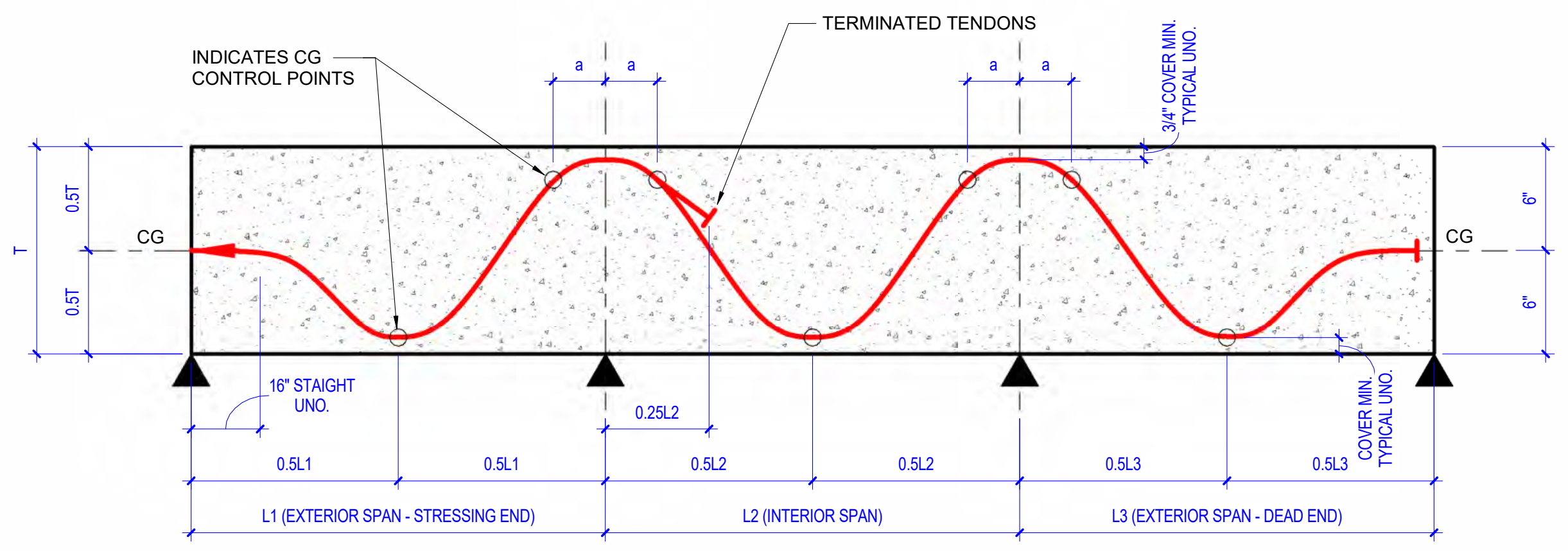
11 Typical Slab Construction Joint

3/4" = 1'-0"


#4 AS SHOWN (T & B) TIED TO ANCHORAGES
• FOR SINGLE ADDED TENDONS BAR SO BE 48" LONG
• FOR MULTIPLE ADDED TENDONS BARS BARS TO EXTEND 20" MIN. PAST LAST ANCHOR
1' - 8" MIN.
PROVIDE (N + 1) #4 X 5'-0" (T&B) BARS CENTERED AT PT ANCHOR HEAD AT LOCATIONS WITH (5) OR MORE ADDED TENDONS, TYP.
NOTE: N = NUMBER OF ADDED TENDONS
THROUGH TENDONS
1' - 0"
1' - 0"
• 4" FOR ANCHORS PLACED VERT.
• 6" FOR ANCHORS PLACED HORIZ.
1
12
DEAD END ANCHORS STAGGERED AND PLACED HORIZONTAL IN SLAB
1' - 0"
TYP
SUPPORT BARS
PLAN
1' - 0"
CG
SEE PLAN (T)
CHAIRS AT 3'-0" O.C. MAX.
SECTION

10 Placement of Added Tendon

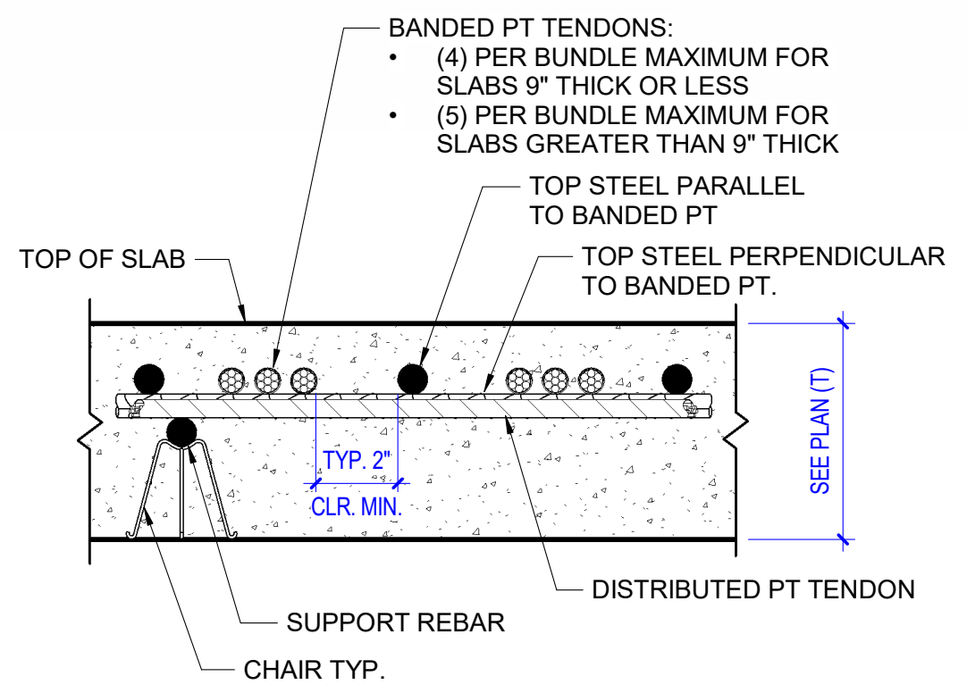
3/4" = 1'-0"



NOTE:
1. CG= CENTER GRAVITY OF PT TENDON - SEE PT PLAN FOR CONTROL POINTS
2. INFLECTION POINT a AT L/10 TYPICAL

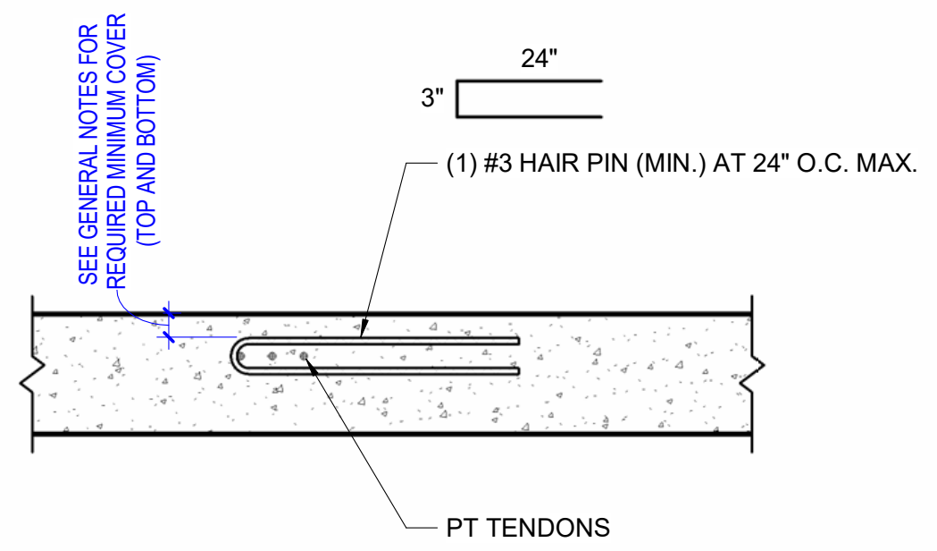
9 Profile for Reversed Parabola Tendons

1 1/2" = 1'-0"


BANDED PT TENDONS:
• (4) PER BUNDLE MAXIMUM FOR SLABS 9" THICK OR LESS
• (5) PER BUNDLE MAXIMUM FOR SLABS GREATER THAN 9" THICK
TOP STEEL PARALLEL TO BANDED PT
TOP STEEL PERPENDICULAR TO BANDED PT.
TOP OF SLAB
TYP. 2" CLR. MIN.
SEE PLAN (T)
SUPPORT REBAR
CHAIR TYP.
DISTRIBUTED PT TENDON

8 Section Through Banded Tendon at Column

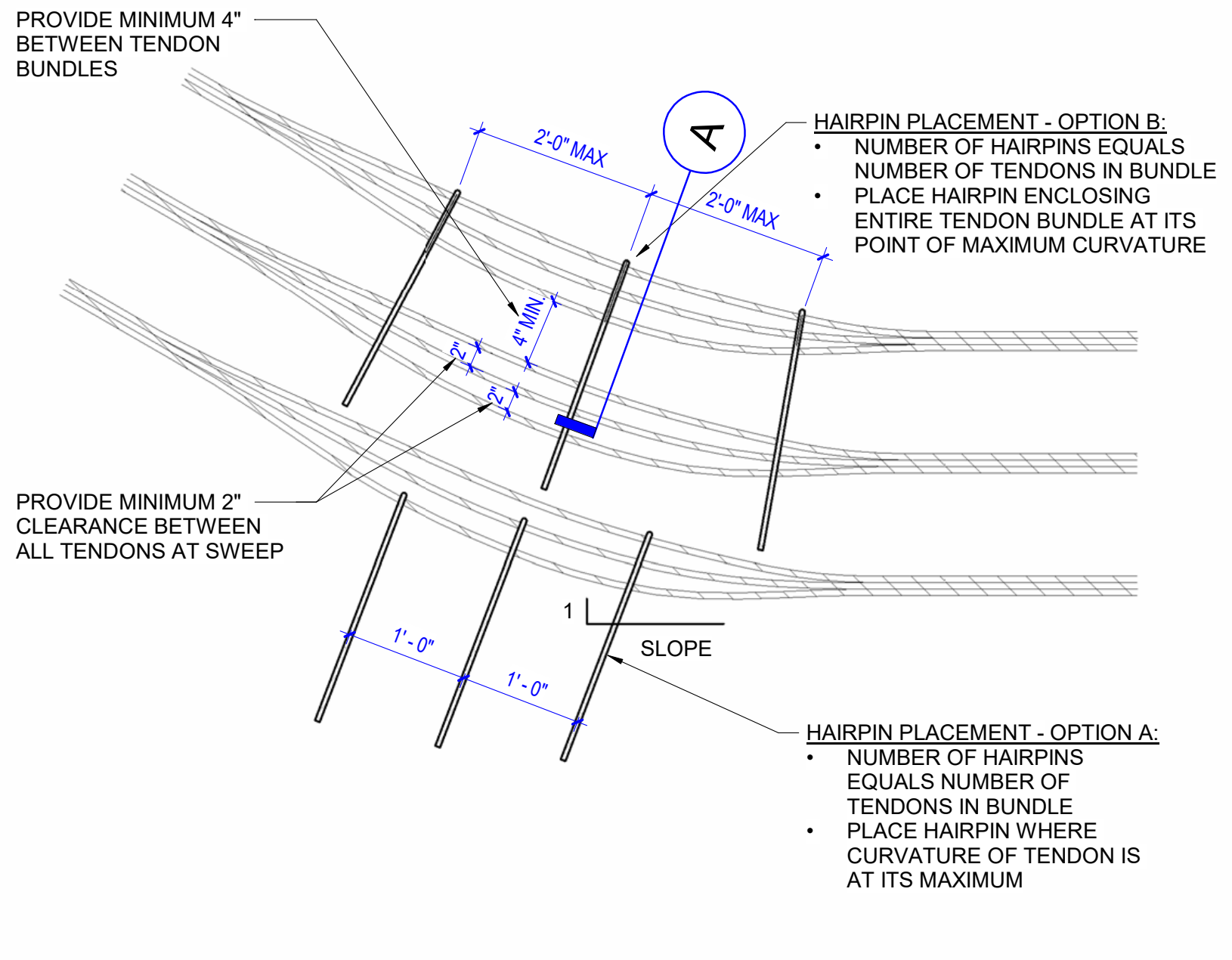
3/4" = 1'-0"


SEE GENERAL NOTES FOR REQUIRED MINIMUM COVER (TOP AND BOTTOM)
24"
3"
(1) #3 HAIR PIN (MIN.) AT 24" O.C. MAX.
PT TENDONS

A Detail at Hairpin

HAIRPIN PLACEMENT CHART			
TENDON SLOPE	PLACEMENT		
	PROCEDURE	OPTION	
LESS THAN 1:12	SPLAY TENDONS NOT REQUIRED	NO HAIRPINS REQUIRED	
1:12 - 1:6	SPLAY TENDONS	OPTION A OR OPTION B	

NOTE: FOR SLOPES GREATER THAN 1:6 REFERENCE PT SUPPLIER APPROVED SUBMITTAL

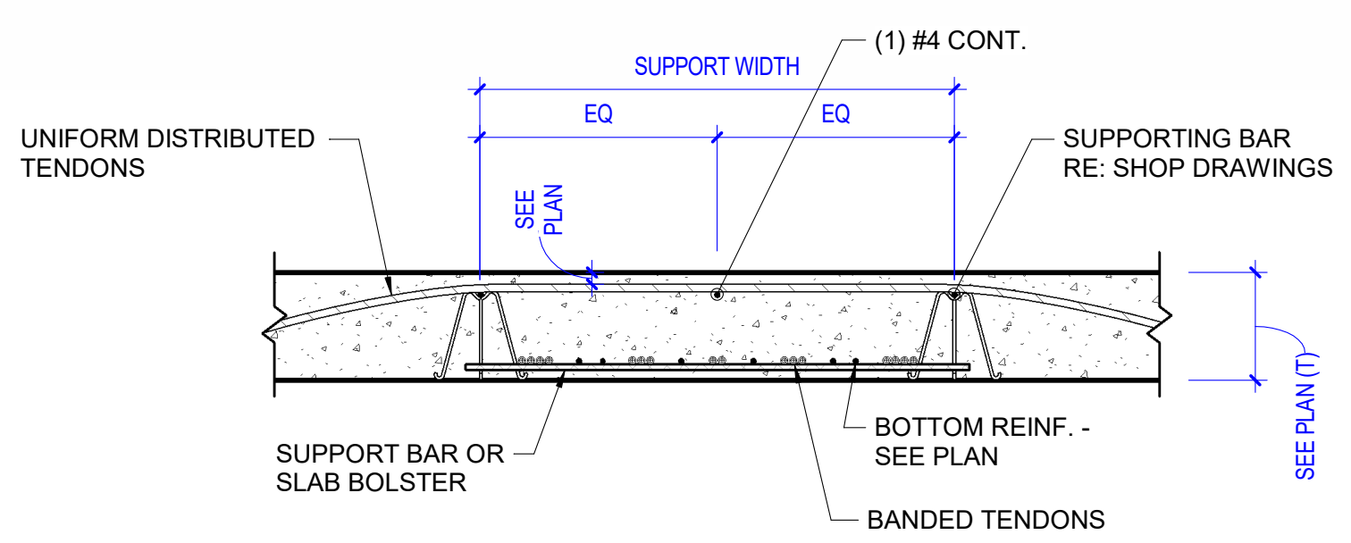

PROVIDE MINIMUM 4" BETWEEN TENDON BUNDLES
2'-0" MAX
2'-0" MAX
A
4" MIN.
2"
2"
PROVIDE MINIMUM 2" CLEARANCE BETWEEN ALL TENDONS AT SWEEP
1' - 0"
1' - 0"
1
SLOPE

HAIRPIN PLACEMENT - OPTION B:
- NUMBER OF HAIRPINS EQUALS NUMBER OF TENDONS IN BUNDLE
- PLACE HAIRPIN ENCLOSING ENTIRE TENDON BUNDLE AT ITS POINT OF MAXIMUM CURVATURE

HAIRPIN PLACEMENT - OPTION A:
- NUMBER OF HAIRPINS EQUALS NUMBER OF TENDONS IN BUNDLE
- PLACE HAIRPIN WHERE CURVATURE OF TENDON IS AT ITS MAXIMUM

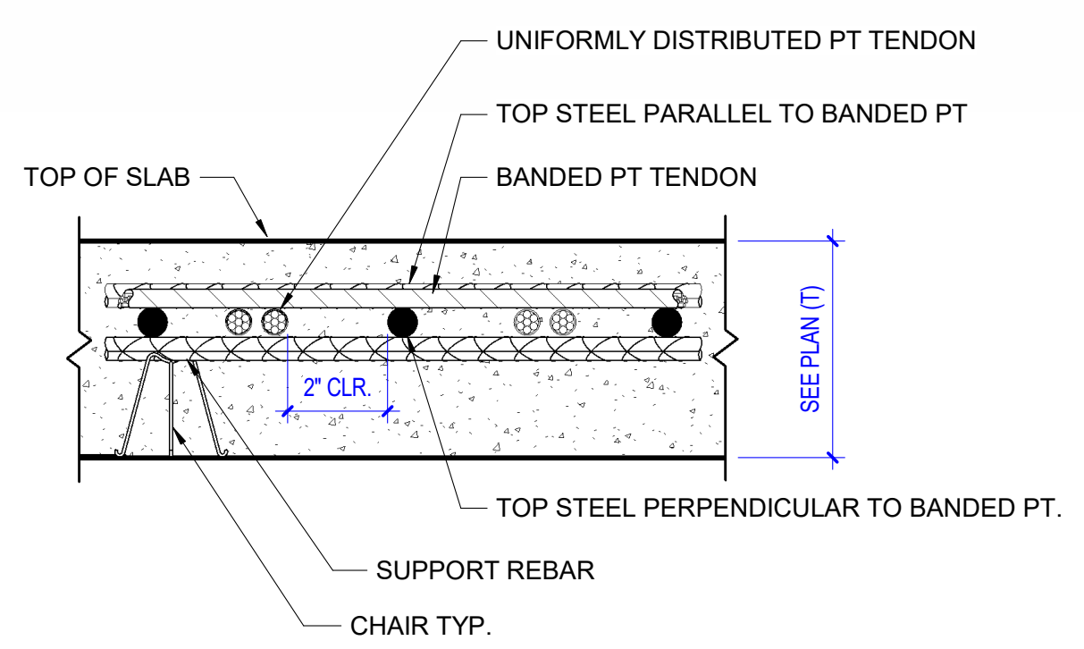
7 Tendon Curvature

3/4" = 1'-0"



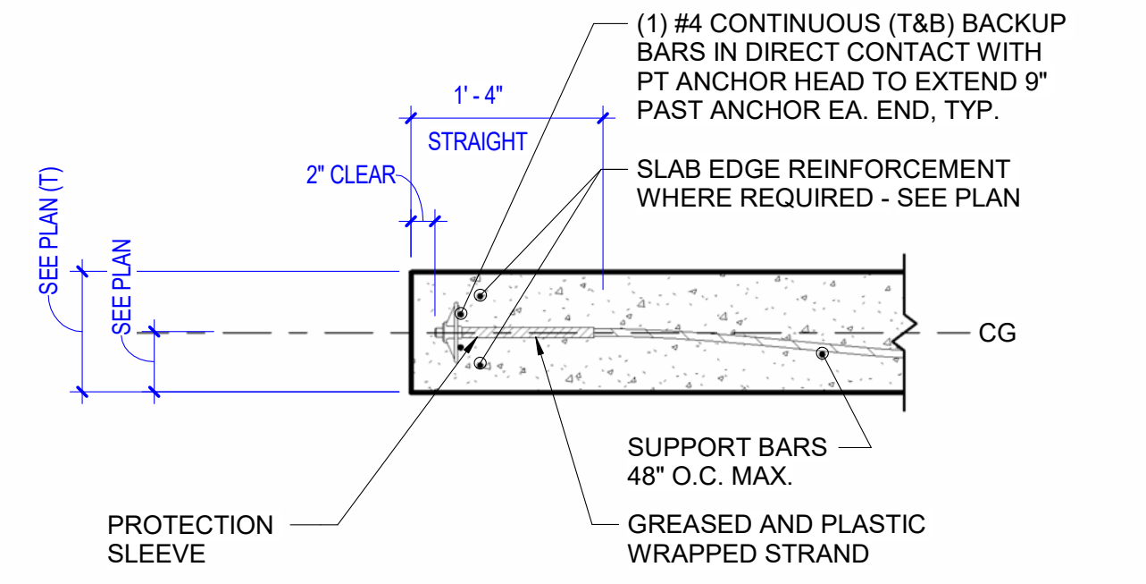
6 Banded Tendons at Mid-Span

3/4" = 1'-0"


UNIFORMLY DISTRIBUTED PT TENDON
TOP STEEL PARALLEL TO BANDED PT
TOP OF SLAB
BANDED PT TENDON
2" CLR.
SEE PLAN (T)
TOP STEEL PERPENDICULAR TO BANDED PT.
SUPPORT REBAR
CHAIR TYP.

5 Section Through Distributed Tendon at Column

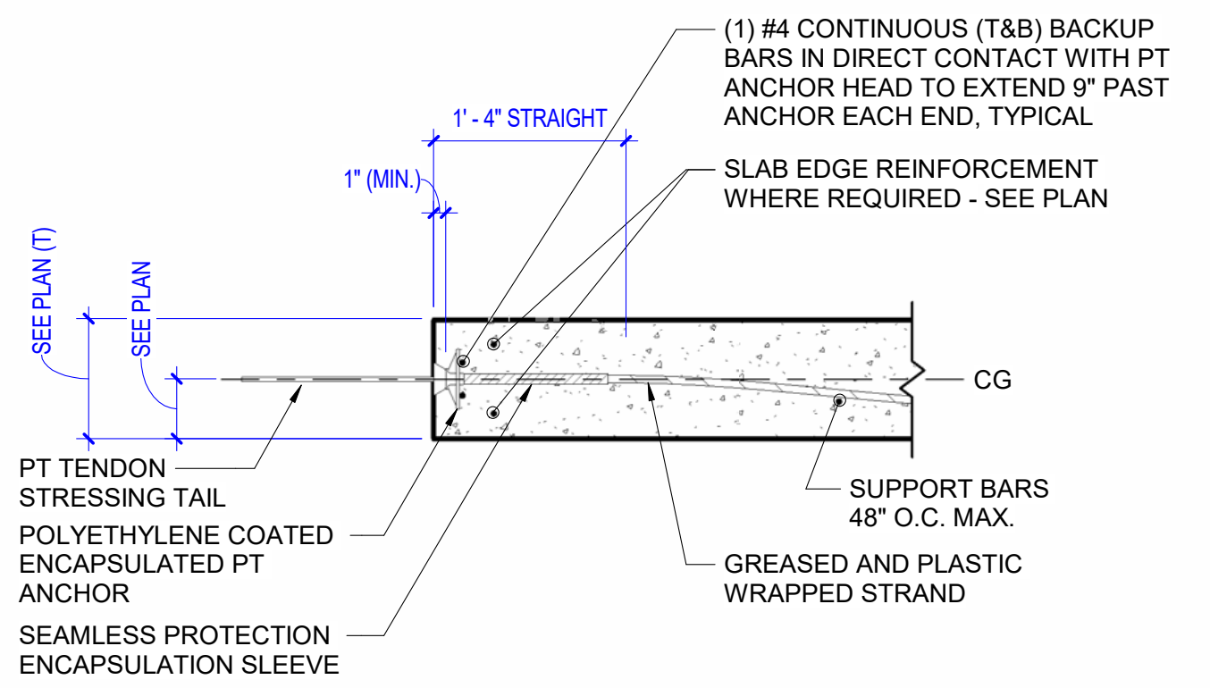
3/4" = 1'-0"


(1) #4 CONTINUOUS (T&B) BACKUP BARS IN DIRECT CONTACT WITH PT ANCHOR HEAD TO EXTEND 9" PAST ANCHOR EA. END, TYP.
1' - 4"
STRAIGHT
2" CLEAR
SLAB EDGE REINFORCEMENT WHERE REQUIRED - SEE PLAN
SEE PLAN (T)
SEE PLAN
CG
SUPPORT BARS 48" O.C. MAX.
PROTECTION SLEEVE
GREASED AND PLASTIC WRAPPED STRAND

CG = CENTER OF GRAVITY OF TENDON

4 Dead End at Distributed Tendons

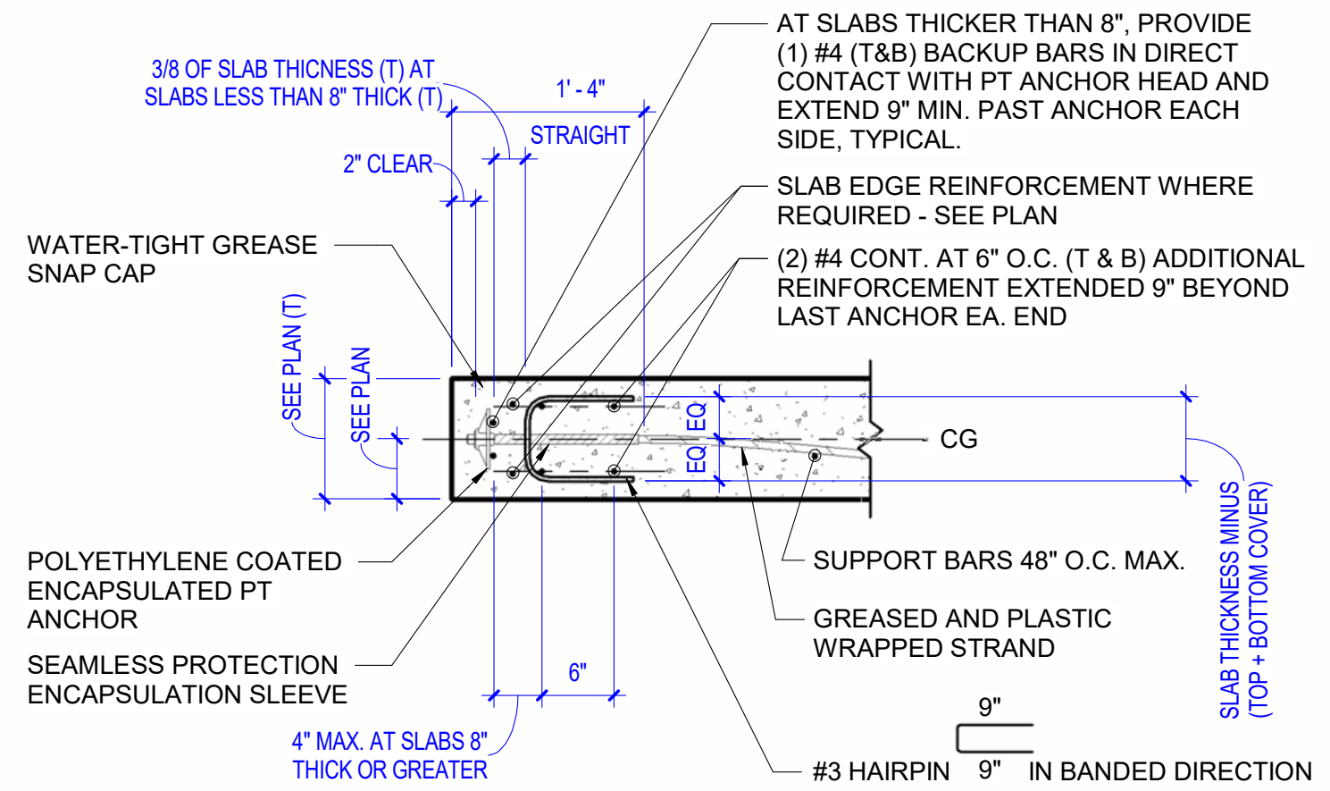
3/4" = 1'-0"


(1) #4 CONTINUOUS (T&B) BACKUP BARS IN DIRECT CONTACT WITH PT ANCHOR HEAD TO EXTEND 9" PAST ANCHOR EACH END, TYPICAL
1' - 4" STRAIGHT
1" (MIN.)
SLAB EDGE REINFORCEMENT WHERE REQUIRED - SEE PLAN
SEE PLAN (T)
SEE PLAN
CG
PT TENDON STRESSING TAIL
POLYETHYLENE COATED ENCAPSULATED PT ANCHOR
SEAMLESS PROTECTION ENCAPSULATION SLEEVE
SUPPORT BARS 48" O.C. MAX.
GREASED AND PLASTIC WRAPPED STRAND

CG = CENTER OF GRAVITY OF TENDON

3 Stressing End at Distributed Tendons

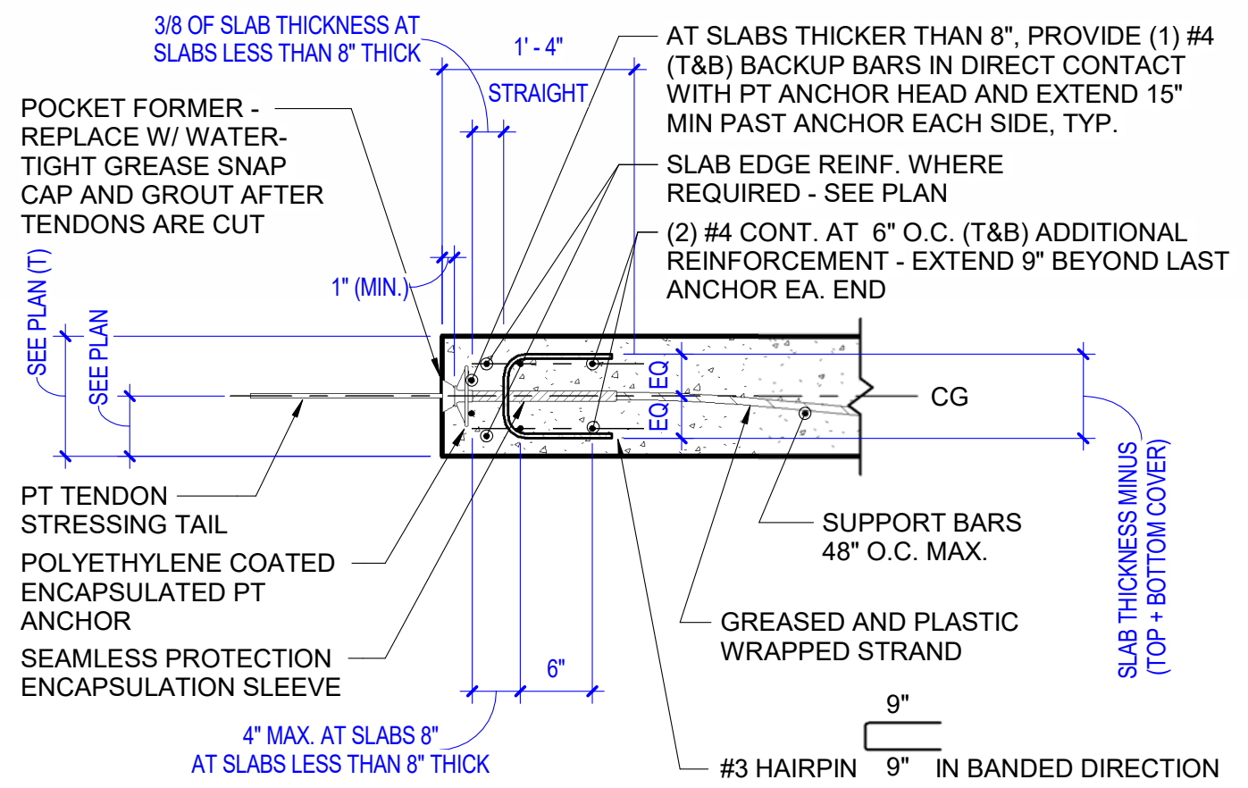
3/4" = 1'-0"


3/8 OF SLAB THICNESS (T) AT SLABS LESS THAN 8" THICK (T)
1' - 4"
STRAIGHT
2" CLEAR
AT SLABS THICKER THAN 8", PROVIDE (1) #4 (T&B) BACKUP BARS IN DIRECT CONTACT WITH PT ANCHOR HEAD AND EXTEND 9" MIN. PAST ANCHOR EACH SIDE, TYPICAL.
SLAB EDGE REINFORCEMENT WHERE REQUIRED - SEE PLAN
(2) #4 CONT. AT 6" O.C. (T & B) ADDITIONAL REINFORCEMENT EXTENDED 9" BEYOND LAST ANCHOR EA. END
WATER-TIGHT GREASE SNAP CAP
SEE PLAN (T)
SEE PLAN
EQ
EQ
CG
POLYETHYLENE COATED ENCAPSULATED PT ANCHOR
SEAMLESS PROTECTION ENCAPSULATION SLEEVE
6"
4" MAX. AT SLABS 8" THICK OR GREATER
SUPPORT BARS 48" O.C. MAX.
GREASED AND PLASTIC WRAPPED STRAND
SLAB THICKNESS MINUS (TOP + BOTTOM COVER)
9"
#3 HAIRPIN 9" IN BANDED DIRECTION ONLY. TOTAL EQUAL TO NUMBER OF ANCHORS PLUS (1)

CG = CENTER OF GRAVITY OF TENDON

2 Dead End at Banded Tendons (Tendon Groups of (5) or More Tendons)

3/4" = 1'-0"


3/8 OF SLAB THICKNESS AT SLABS LESS THAN 8" THICK
1' - 4"
STRAIGHT
AT SLABS THICKER THAN 8", PROVIDE (1) #4 (T&B) BACKUP BARS IN DIRECT CONTACT WITH PT ANCHOR HEAD AND EXTEND 15" MIN PAST ANCHOR EACH SIDE, TYP.
POCKET FORMER - REPLACE W/ WATER-TIGHT GREASE SNAP CAP AND GROUT AFTER TENDONS ARE CUT
SLAB EDGE REINF. WHERE REQUIRED - SEE PLAN
(2) #4 CONT. AT 6" O.C. (T&B) ADDITIONAL REINFORCEMENT - EXTEND 9" BEYOND LAST ANCHOR EA. END
1" (MIN.)
SEE PLAN (T)
SEE PLAN
EQ
EQ
CG
PT TENDON STRESSING TAIL
POLYETHYLENE COATED ENCAPSULATED PT ANCHOR
SEAMLESS PROTECTION ENCAPSULATION SLEEVE
SUPPORT BARS 48" O.C. MAX.
GREASED AND PLASTIC WRAPPED STRAND
SLAB THICKNESS MINUS (TOP + BOTTOM COVER)
6"
4" MAX. AT SLABS 8" AT SLABS LESS THAN 8" THICK
9"
#3 HAIRPIN 9" IN BANDED DIRECTION ONLY. TOTAL EQUAL TO NUMBER OF ANCHORS PLUS (1)

CG = CENTER OF GRAVITY OF TENDON

1 Stressing End at Banded Tendons (Tendon Groups of (5) or More Tendons)

3/4" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMsq Structural Engineers, LLC
1080 Eldridge Parkway
Suite 350
Houston, TX 77077
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www.vsmsq.com

PROJECT # : 26154
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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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Typical PT Details

SHEET NUMBER:
S0.121

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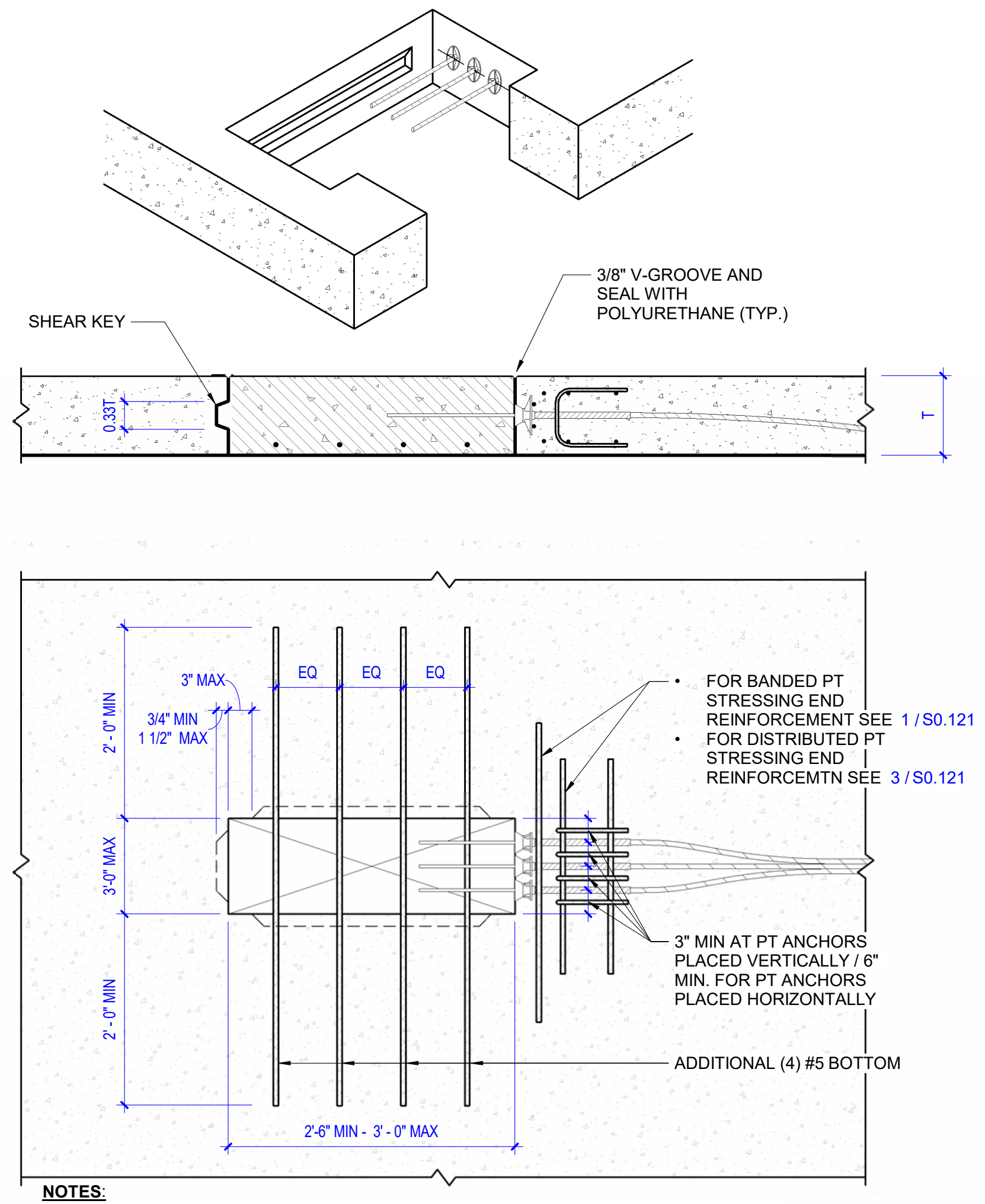
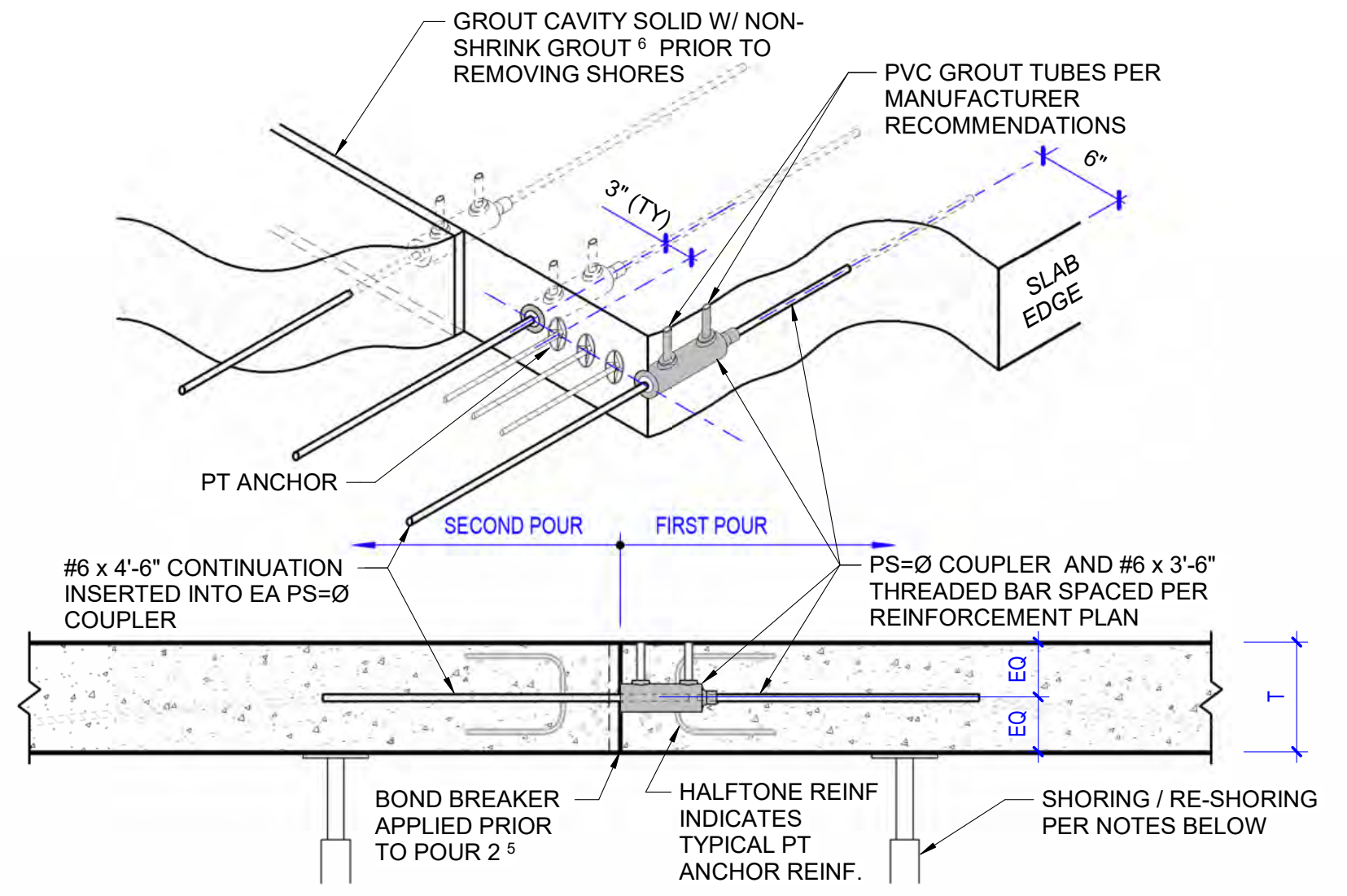

SHEAR KEY
3/8" V-GROOVE AND SEAL WITH POLYURETHANE (TYP.)
0.33T
T
3" MAX.
EQ
EQ
EQ
3/4" MIN
1 1/2" MAX
2' - 0" MIN
3' - 0" MAX
2' - 0" MIN
FOR BANDED PT STRESSING END REINFORCEMENT SEE 1 / S0.121
FOR DISTRIBUTED PT STRESSING END REINFORCEMTN SEE 3 / S0.121
3" MIN AT PT ANCHORS PLACED VERTICALLY / 6" MIN. FOR PT ANCHORS PLACED HORIZONTALLY
ADDITIONAL (4) #5 BOTTOM
2'-6" MIN - 3' - 0" MAX

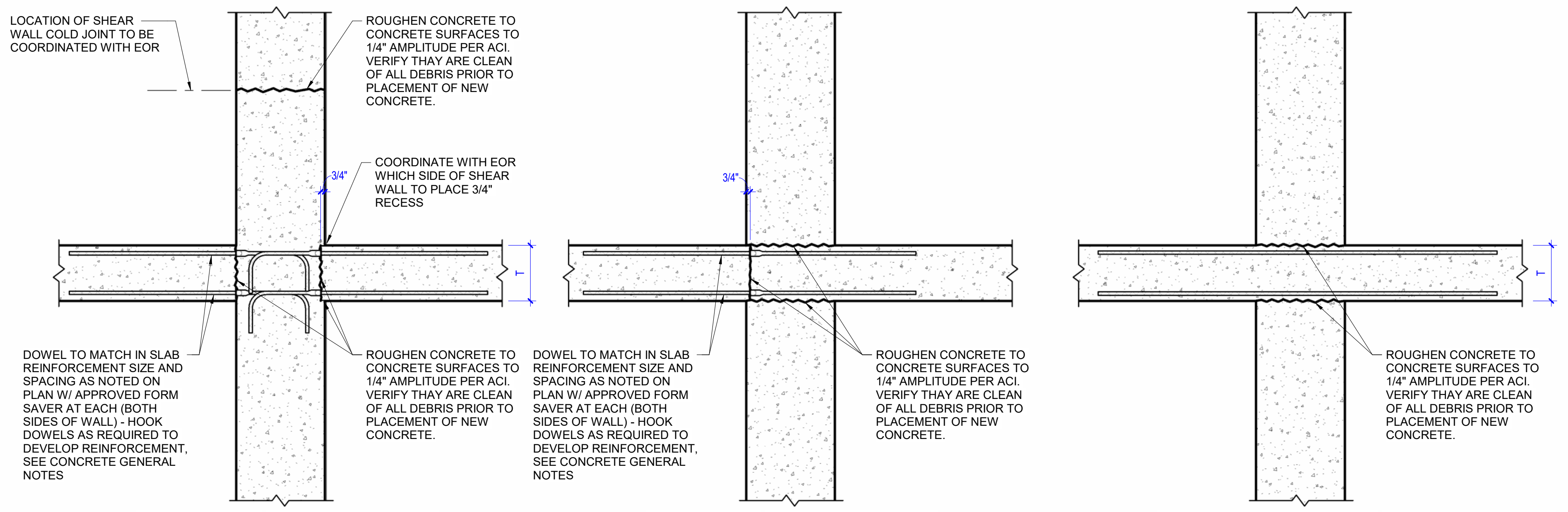
NOTES:
1. FINISH TENDONS AND FILL STRESSING POCKETS PER POST-TENISONED CONCRETE REQUIREMENTS IN GENERAL NOTES
2. CONTINUE ALL REINFORCEMENT SHOWN IN PLANS THROUGH THE BLOCKOUT. IF TOP BARS PREVENT STRESSING RAM ACCESS, CONSULT SEOR.
3. FILL BLOCKOUT W/ CONCRETE SPEIFIED FOR SLAB. BLOCKOUT MAY BE FILLED IMMEDIATELY AFTER STRESSING AND FINISHING TENDONS.

8 PT Tendons at Opening
3/4" = 1'-0"


GROUT CAVITY SOLID W/ NON-SHRINK GROUT 8 PRIOR TO REMOVING SHORES
PVC GROUT TUBES PER MANUFACTURER RECOMMENDATIONS
6"
3" (TYP.)
SLAB EDGE
PT ANCHOR
SECOND POUR
FIRST POUR
#6 x 4'-6" CONTINUATION INSERTED INTO EA PS=Ø COUPLER
PS=Ø COUPLER AND #6 x 3'-6" THREADED BAR SPACED PER REINFORCEMENT PLAN
EQ
EQ
T
BOND BREAKER APPLIED PRIOR TO POUR 2 5
HALFTONE REINF INDICATES TYPICAL PT ANCHOR REINF.
SHORING / RE-SHORING PER NOTES BELOW

NOTES:
1. CONSTRUCTION JOINTS UTILIZING PS = Ø REBAR SPLICES CONSTITUTE A PROPRIETARY SYSTEM BY TECH DEVELOPMENT, LLC. JOINT DOWELS AND COUPLERS SHALL BE DESIGNED AND DETAILED BY THE MANUFACTURER SUCH THAT THEY PROVIDE FOR FREE SHRINKAGE OF PT SLABS ON EACH SIDE OF JOINT WHILE SUPPORTING THE FULL GRAVITY LOAD OF THE LONGEST SLAB SPAN ADJACENT TO JOINT DURING CONSTRUCTION. IN ADDITION, JOINT DOWELS SHALL PROVIDE RIGID CONNECTION WITH FULL DIAPHRAGM FORCE TRANSFER AFTER GROUTING.
2. SHOP DRAWINGS SHOWING LAYOUT AND PROCEDURE OF DOWEL AND COUPLER INSTALLATION, AS WELL AS CORRESPONDING CALCULATIONS, EACH BEARING THE SEAL AND SIGNATURE OF THE DESIGNING ENGINEER LICENSED IN THE JURISDICTION OF THE PROJECT, SHALL BE SUBMITTED FOR REVIEW PRIOR TO CONSTRUCTION.
3. LOCATE AND ATTACH PS = Ø COUPLERS TO BULKHEAD PRIOR TO PLACEMENT OF PT TENDON ANCHORS. CONTRACTOR SHALL FOLLOW ALL PS = Ø MANUFACTURER INSTALLATION RECOMMENDATIONS.
4. DO NOT REMOVE FORMWORK UNTIL BOTH POURS HAVE BEEN PLACED, PT HAS BEEN STRESSED, AND TENDON ELONGATIONS HAVE BEEN APPROVED BY EOR. RESHORE SLAB FOLLOWING FORMWORK REMOVAL AS REQUIRED IN ACCORDANCE WITH APPROVED FORMWORK SHOP DRAWINGS.
5. CONTRACTOR TO NOTE SPECIAL SHORING CONDITIONS ON EITHER SIDE OF CONSTRUCTION JOINT AND PROVIDE ADDITIONAL SHORING AS REQUIRED. NEXT FULL SPANS ADJACENT TO SPAN CONTAINING CONSTRUCTION JOINT SHALL REMAIN SHORED AS REQUIRED TO MINIMIZE RISK OF UPWARD DEFLECTION OF FIRST POUR.
6. CONTRACTOR SHALL ENSURE THAT SECOND POUR CONTINUATION REBAR IS PROPERLY POSITIONED AT THE REAR OPENING OF THE PS= Ø COUPLER (REBAR SHOULD BE IN DIRECT CONTACT WITH REAR COUPLER OPENING) TO MINIMIZE DIFFERENTIAL VERTICAL MOVEMENT BETWEEN POURS 1 & 2.
7. APPLY NOX-CRETE SILCOSEAL CLASSIC BOND BREAKER TO FACE OF POUR 1 SLAB NO LESS THAN 2 HOURS AND NO MORE THAN 4 HOURS PRIOR TO POUR 2 CONCRETE PLACEMENT.
8. 56 DAYS AFTER STRESSING SECOND POUR PT TENDONS, PS = Ø COUPLERS AND JOINT SHALL BE GROUTED USING BASF MASTERFLOW 885 HIGH-PRECISION, NONSHRINK METALLIC-AGGREGATE GROUT. GROUT SUBSTITUTIONS ARE NOT PERMITTED.
9. SEE PS=Ø MANUFACTURER DATA FOR MORE INFORMATION (ICC-ES NO. ESR-4213).

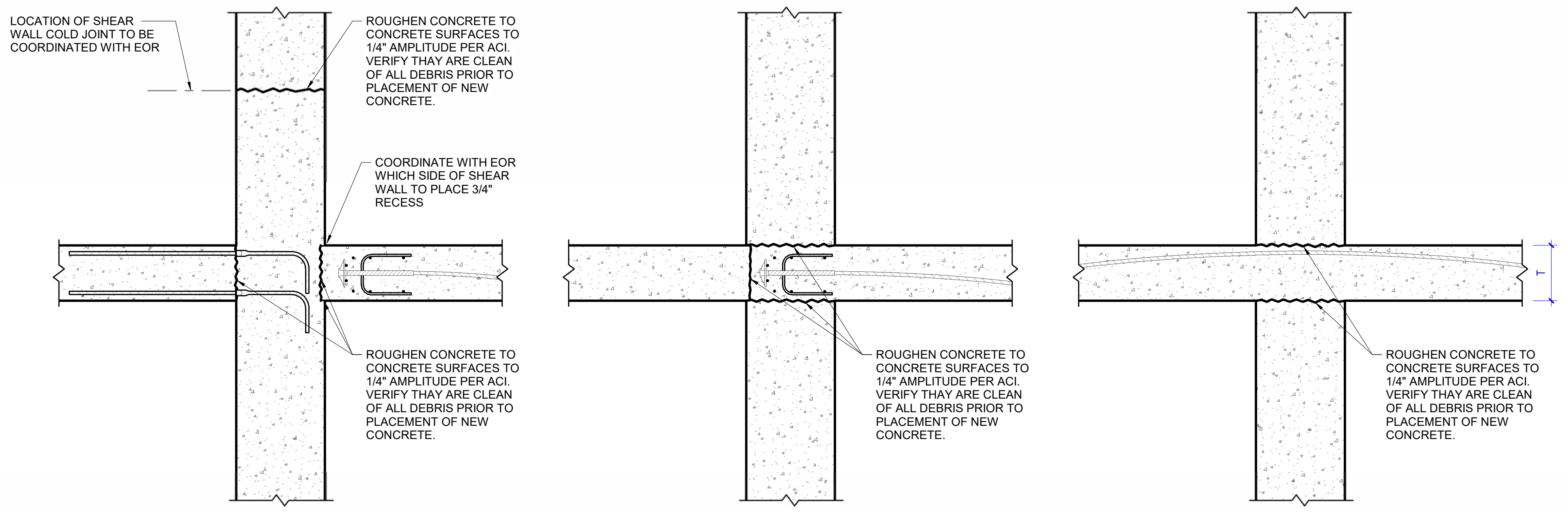
5 PS=Ø Construction Joint W/ Out Stressing Block-Out
3/4" = 1'-0"


LOCATION OF SHEAR WALL COLD JOINT TO BE COORDINATED WITH EOR
ROUGHEN CONCRETE TO CONCRETE SURFACES TO 1/4" AMPLITUDE PER ACI. VERIFY THAY ARE CLEAN OF ALL DEBRIS PRIOR TO PLACEMENT OF NEW CONCRETE.
COORDINATE WITH EOR WHICH SIDE OF SHEAR WALL TO PLACE 3/4" RECESS
3/4"
T
DOWEL TO MATCH IN SLAB REINFORCEMENT SIZE AND SPACING AS NOTED ON PLAN W/ APPROVED FORM SAVER AT EACH (BOTH SIDES OF WALL) - HOOK DOWELS AS REQUIRED TO DEVELOP REINFORCEMENT, SEE CONCRETE GENERAL NOTES
ROUGHEN CONCRETE TO CONCRETE SURFACES TO 1/4" AMPLITUDE PER ACI. VERIFY THAY ARE CLEAN OF ALL DEBRIS PRIOR TO PLACEMENT OF NEW CONCRETE.
3/4"
DOWEL TO MATCH IN SLAB REINFORCEMENT SIZE AND SPACING AS NOTED ON PLAN W/ APPROVED FORM SAVER AT EACH (BOTH SIDES OF WALL) - HOOK DOWELS AS REQUIRED TO DEVELOP REINFORCEMENT, SEE CONCRETE GENERAL NOTES
ROUGHEN CONCRETE TO CONCRETE SURFACES TO 1/4" AMPLITUDE PER ACI. VERIFY THAY ARE CLEAN OF ALL DEBRIS PRIOR TO PLACEMENT OF NEW CONCRETE.
T
ROUGHEN CONCRETE TO CONCRETE SURFACES TO 1/4" AMPLITUDE PER ACI. VERIFY THAY ARE CLEAN OF ALL DEBRIS PRIOR TO PLACEMENT OF NEW CONCRETE.

MAST CLIMBER - BETWEEN PT ANCHOR

SLIP FORMED - BETWEEN PT ANCHOR

CONVENTIONALLY FORMED - BETWEEN PT ANCHOR

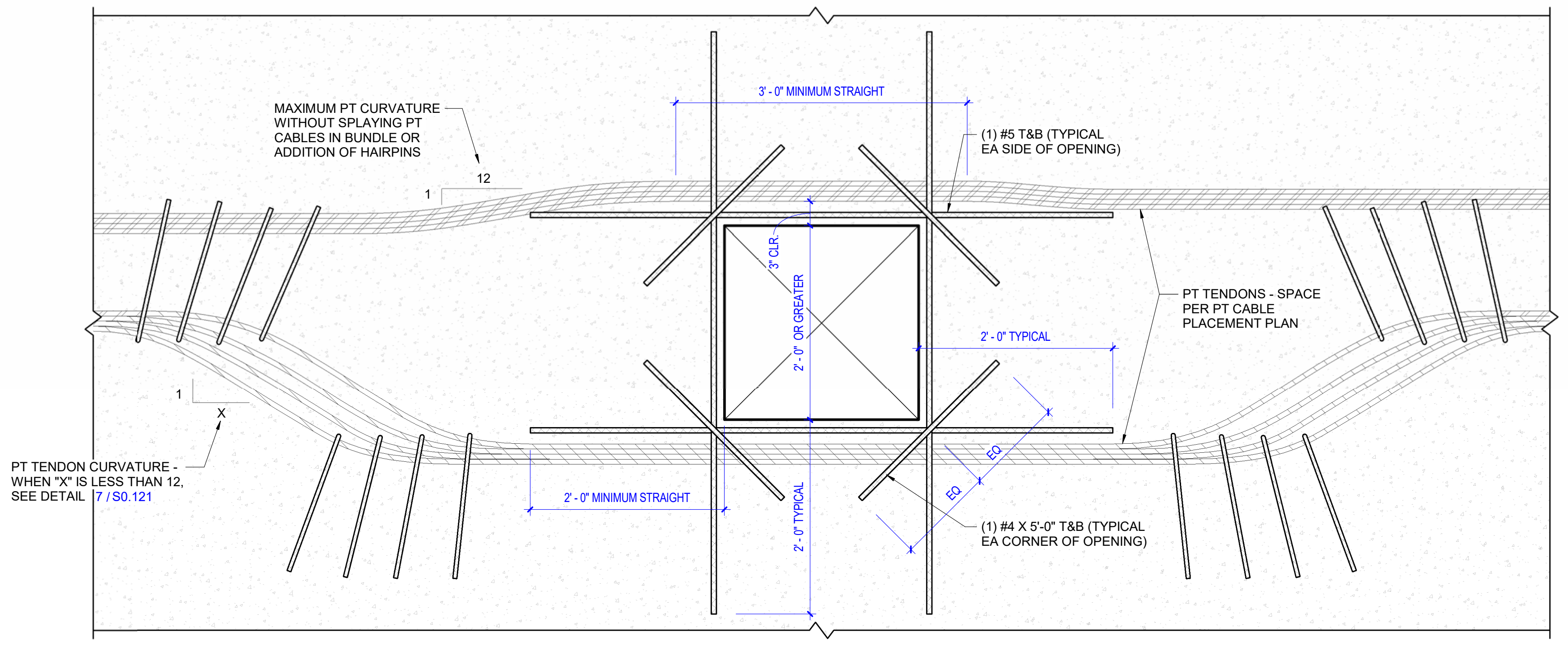

LOCATION OF SHEAR WALL COLD JOINT TO BE COORDINATED WITH EOR
ROUGHEN CONCRETE TO CONCRETE SURFACES TO 1/4" AMPLITUDE PER ACI. VERIFY THAY ARE CLEAN OF ALL DEBRIS PRIOR TO PLACEMENT OF NEW CONCRETE.
COORDINATE WITH EOR WHICH SIDE OF SHEAR WALL TO PLACE 3/4" RECESS
T
ROUGHEN CONCRETE TO CONCRETE SURFACES TO 1/4" AMPLITUDE PER ACI. VERIFY THAY ARE CLEAN OF ALL DEBRIS PRIOR TO PLACEMENT OF NEW CONCRETE.
ROUGHEN CONCRETE TO CONCRETE SURFACES TO 1/4" AMPLITUDE PER ACI. VERIFY THAY ARE CLEAN OF ALL DEBRIS PRIOR TO PLACEMENT OF NEW CONCRETE.
ROUGHEN CONCRETE TO CONCRETE SURFACES TO 1/4" AMPLITUDE PER ACI. VERIFY THAY ARE CLEAN OF ALL DEBRIS PRIOR TO PLACEMENT OF NEW CONCRETE.

MAST CLIMBER - AT PT ANCHOR

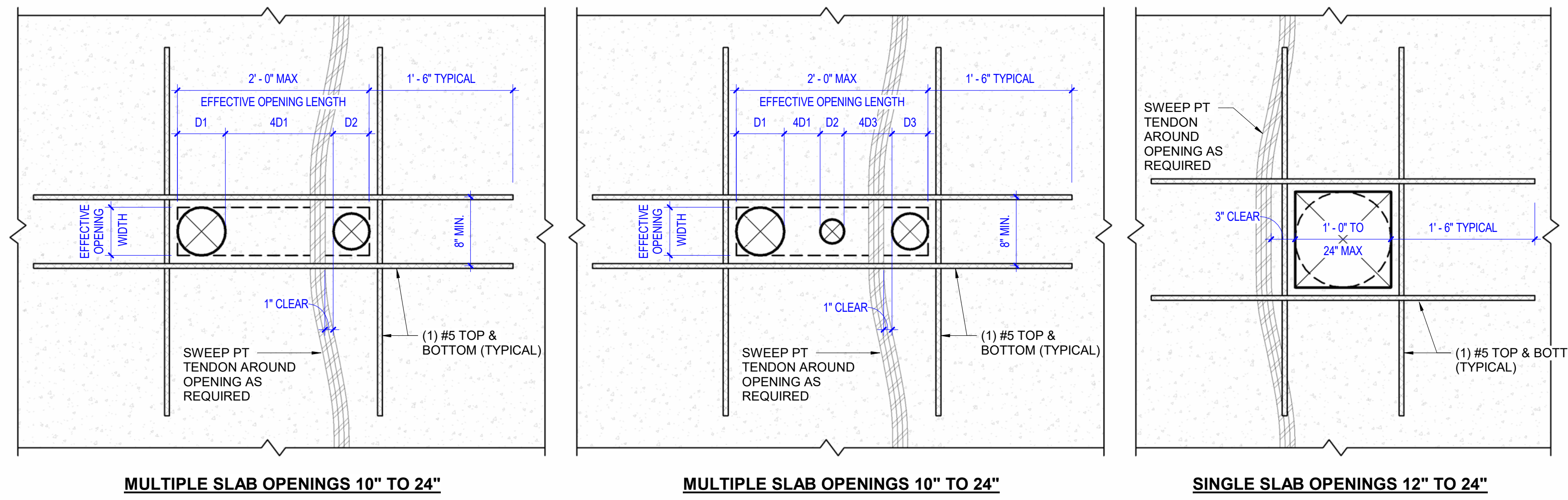
SLIP FORM AT PT ANCHOR

CONVENTIONALLY - AT PT ANCHOR

4 PT Tendons at Shearwall
3/4" = 1'-0"


MAXIMUM PT CURVATURE WITHOUT SPLAYING PT CABLES IN BUNDLE OR ADDITION OF HAIRPINS
12
1
3' - 0" MINIMUM STRAIGHT
(1) #5 T&B (TYPICAL EA SIDE OF OPENING)
3" CLR
2' - 0" OR GREATER
2' - 0" TYPICAL
PT TENDONS - SPACE PER PT CABLE PLACEMENT PLAN
1
X
PT TENDON CURVATURE - WHEN "X" IS LESS THAN 12, SEE DETAIL 7 / S0.121
2' - 0" MINIMUM STRAIGHT
2' - 0" TYPICAL
EQ
EQ
(1) #4 X 5'-0" T&B (TYPICAL EA CORNER OF OPENING)

3 Tendon Placement at Opening 24" or Larger
1" = 1'-0"



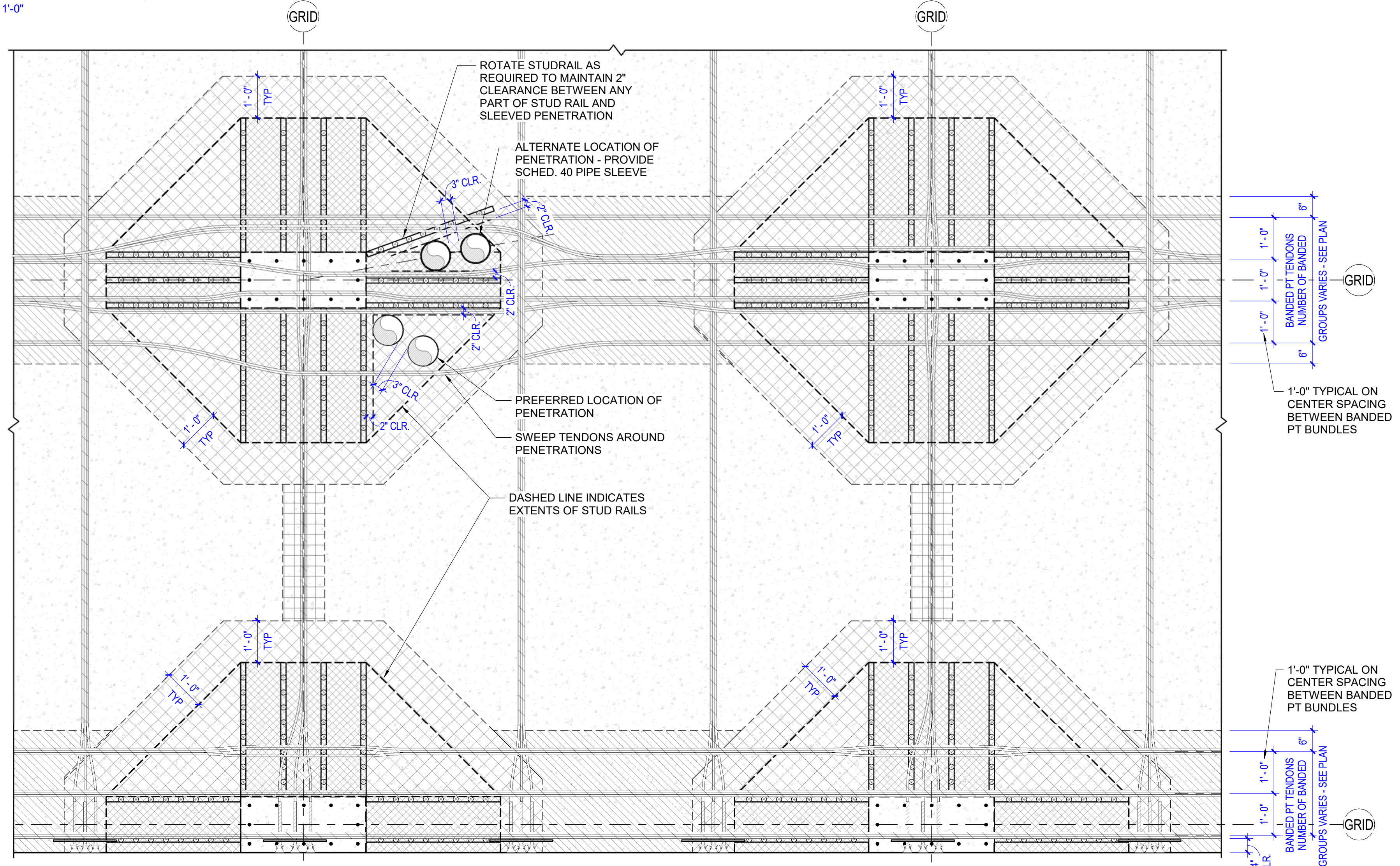
MULTIPLE SLAB OPENINGS 10" TO 24"

MULTIPLE SLAB OPENINGS 10" TO 24"

SINGLE SLAB OPENINGS 12" TO 24"

NOTE:
1. FOR SLAB OPENINGS LESS THAN 12" NO ADDITIONAL REINFOREMENT REQUIRED.
2. FOR OPENINGS LARGER THAN 24" SEE 3 / S0.122

2 Trim Bars at Slab Openings 12" to 24"
1" = 1'-0"


GRID
GRID
ROTATE STUDRAIL AS REQUIRED TO MAINTAIN 2" CLEARANCE BETWEEN ANY PART OF STUD RAIL AND SLEEVED PENETRATION
ALTERNATE LOCATION OF PENETRATION - PROVIDE SCHED. 40 PIPE SLEEVE
PREFERRED LOCATION OF PENETRATION
SWEEP TENDONS AROUND PENETRATIONS
DASHED LINE INDICATES EXTENTS OF STUD RAILS
1' - 0" TYP
2" CLR
1" CLR
1'-0" TYPICAL ON CENTER SPACING BETWEEN BANDED PT BUNDLES
BANDED PT TENDONS NUMBER OF BANDED GROUPS VARIES - SEE PLAN
1' - 0"
6"
4" CLR.
GRID

LEGEND:

▦	PENETRATIONS IN THIS AREA MUST BE LOCATED TO ALLOW DISTRIBUTED PT TENDONS TO PASS OVER COLUMNS
▩	PENETRATIONS IN THIS AREA REQUIRE FURTHER ANALYSIS BY EOR. PENETRATIONS IN THIS MAY REQUIRE THE ADDITION OF STUDRAILS OR STUDRAIL REDESIGN WHEN THEY PRESENTLY OCCUR
▧	AVOID LOCATING CAST IN SLAB ELEMENTS (SUCH AS ELECTRICAL CONDUIT, ETC. AND SLAB PENETRATIONS) IN THIS ZONE. SLAB PENETRATIONS IN THIS SHOULD BE AVOIDED IF POSSIBLE. ALL PENETRATIONS IN THIS ZONE SHALL BE CLEARLY IDENTIFIED IN LOCATION, SIZE, AND GEOMETRY ON PLANS AND PROVIDED TO EOR FOR REVIEW / APPROVAL PRIOR TO CONSTRUCTION.
▒	ALL PENETRATIONS IN THIS AREA SHOUD BE AVOIDED. IN INSTANCES SUCH AS GARAGE DRAIN RISERS THAT MUST OCCUR IN THIS AREA SEE PIPE SLEEVE PENETRATION GUIDELINES SHOWN IN DIAGRAM ABOVE FOR ACCEPTABLE LOCATION AND ADJUSTMENTS TO STUD RAILS REQUIRED. WHEN PENETRATIONS OCCUR IN THIS ZONE THEY MUST BE APPOVED BY EOR ON A CASE BY CASE BASIS.

NOTES:
1. AREAS THAT ARE NOT HATCHED ARE CONSIDERED GENERALLY ACCEPTABLE FOR PENETRATIONS.
2. GC TO SUBMIT COMPLETE COORDINATED MEP / FP PENETRATION PLANS WITH ALL PENETRATIONS / CAST IN SLAB ELEMENT FOR EACH FLOOR CLEARLY IDENTIFIED IN LOCATION, SIZE, AND GEOMETRY **FOR REVIEW / APPROVAL BY THE EOR SIX WEEKS PRIOR TO CONSTRUCTION.**

1 PT Slab Penetration Guide
1/2" = 1'-0"

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BATON ROUGE, LOUISIANA 70808
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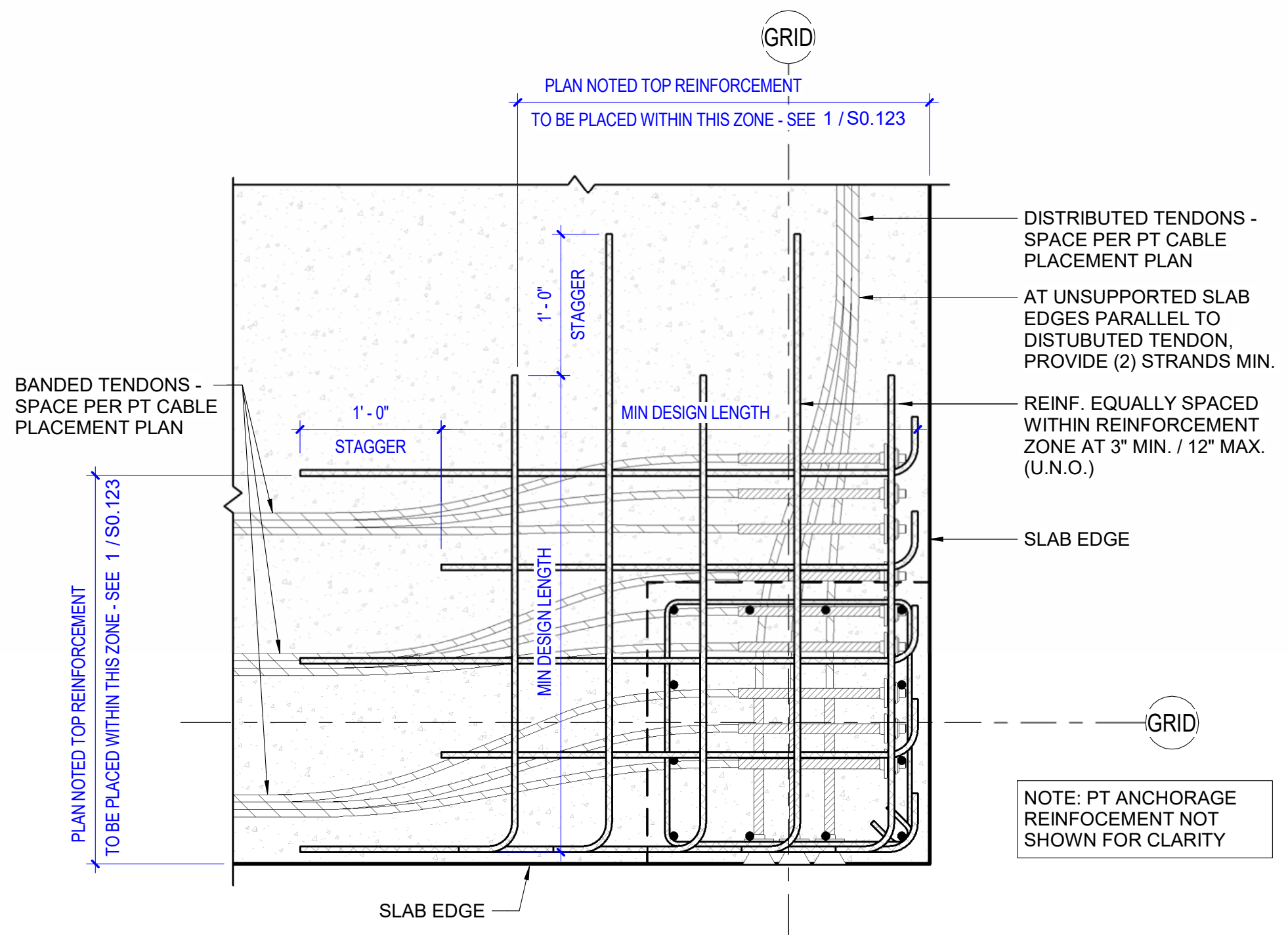
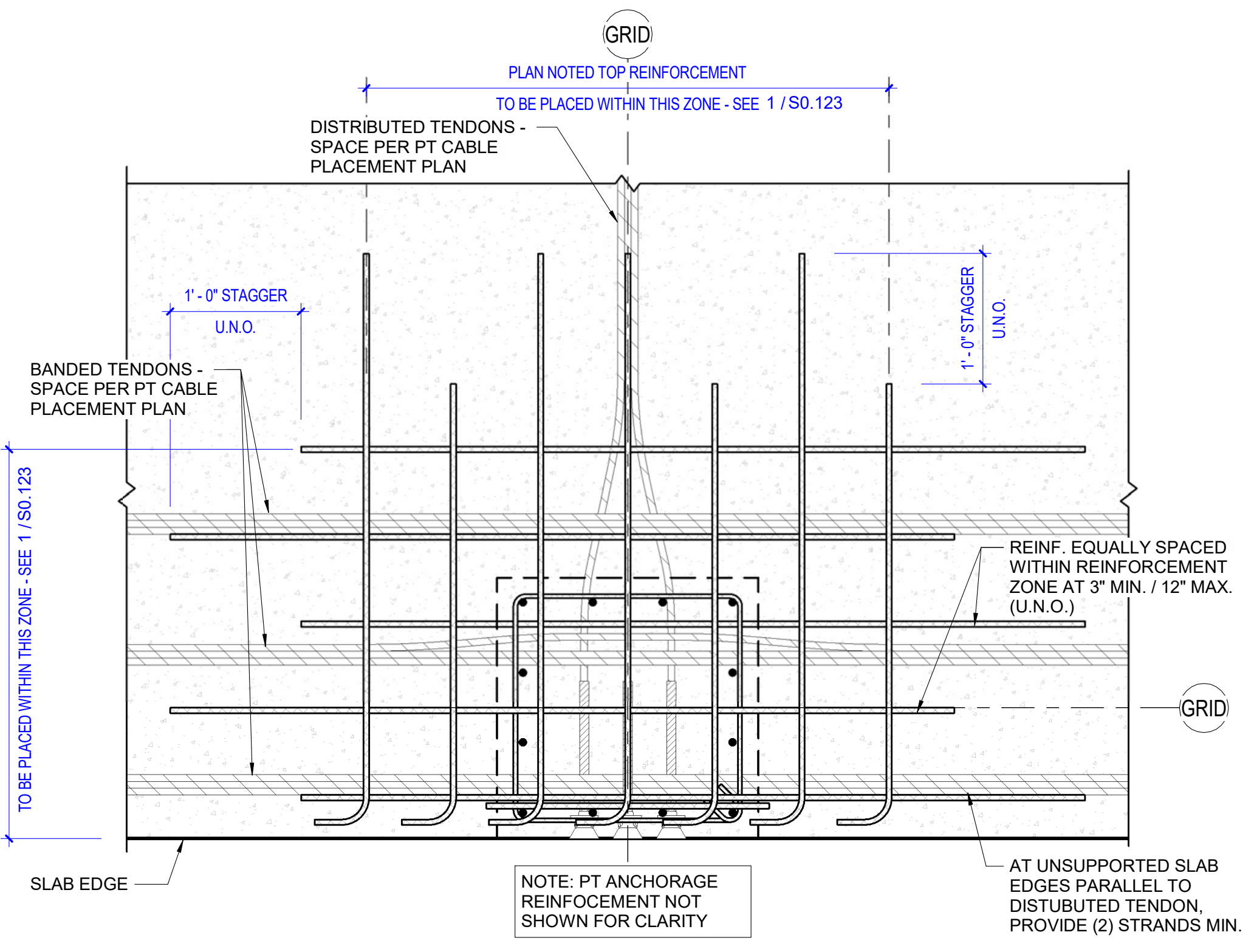
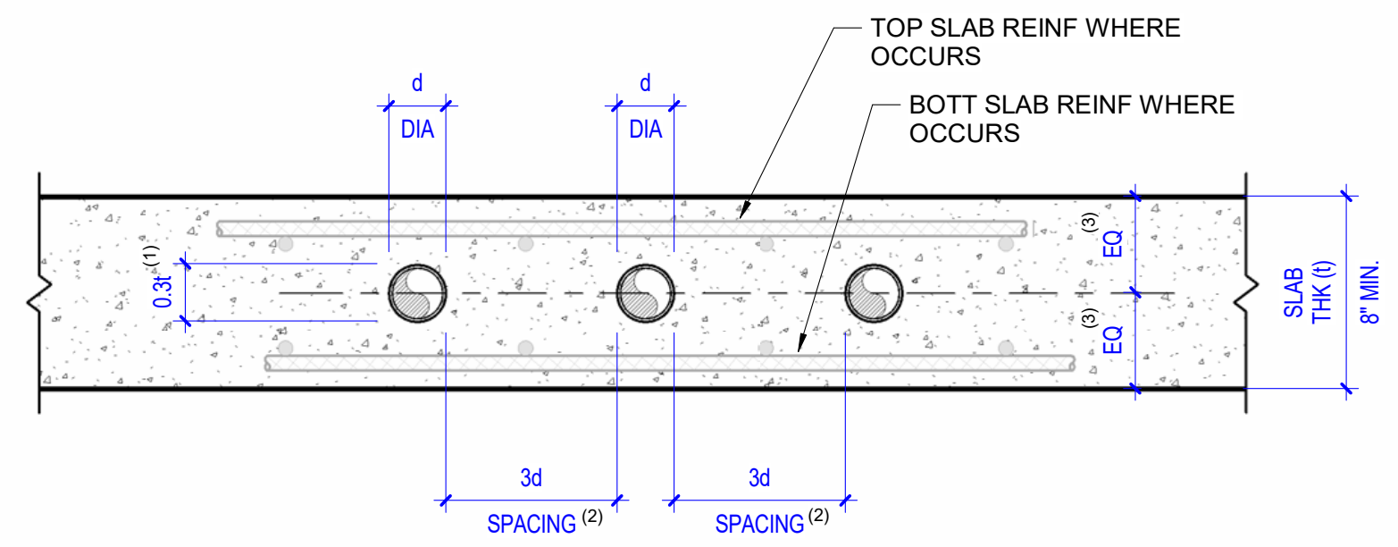

GRID
PLAN NOTED TOP REINFORCEMENT
TO BE PLACED WITHIN THIS ZONE - SEE 1 / S0.123
DISTRIBUTED TENDONS - SPACE PER PT CABLE PLACEMENT PLAN
AT UNSUPPORTED SLAB EDGES PARALLEL TO DISTUBUTED TENDON, PROVIDE (2) STRANDS MIN.
BANDED TENDONS - SPACE PER PT CABLE PLACEMENT PLAN
1' - 0"
STAGGER
MIN DESIGN LENGTH
REINF. EQUALLY SPACED WITHIN REINFORCEMENT ZONE AT 3" MIN. / 12" MAX. (U.N.O.)
SLAB EDGE
PLAN NOTED TOP REINFORCEMENT
TO BE PLACED WITHIN THIS ZONE - SEE 1 / S0.123
MIN DESIGN LENGTH
GRID
NOTE: PT ANCHORAGE REINFOCEMENT NOT SHOWN FOR CLARITY
SLAB EDGE

5 Top Reinforcement at Outside Corner Column
1" = 1'-0"

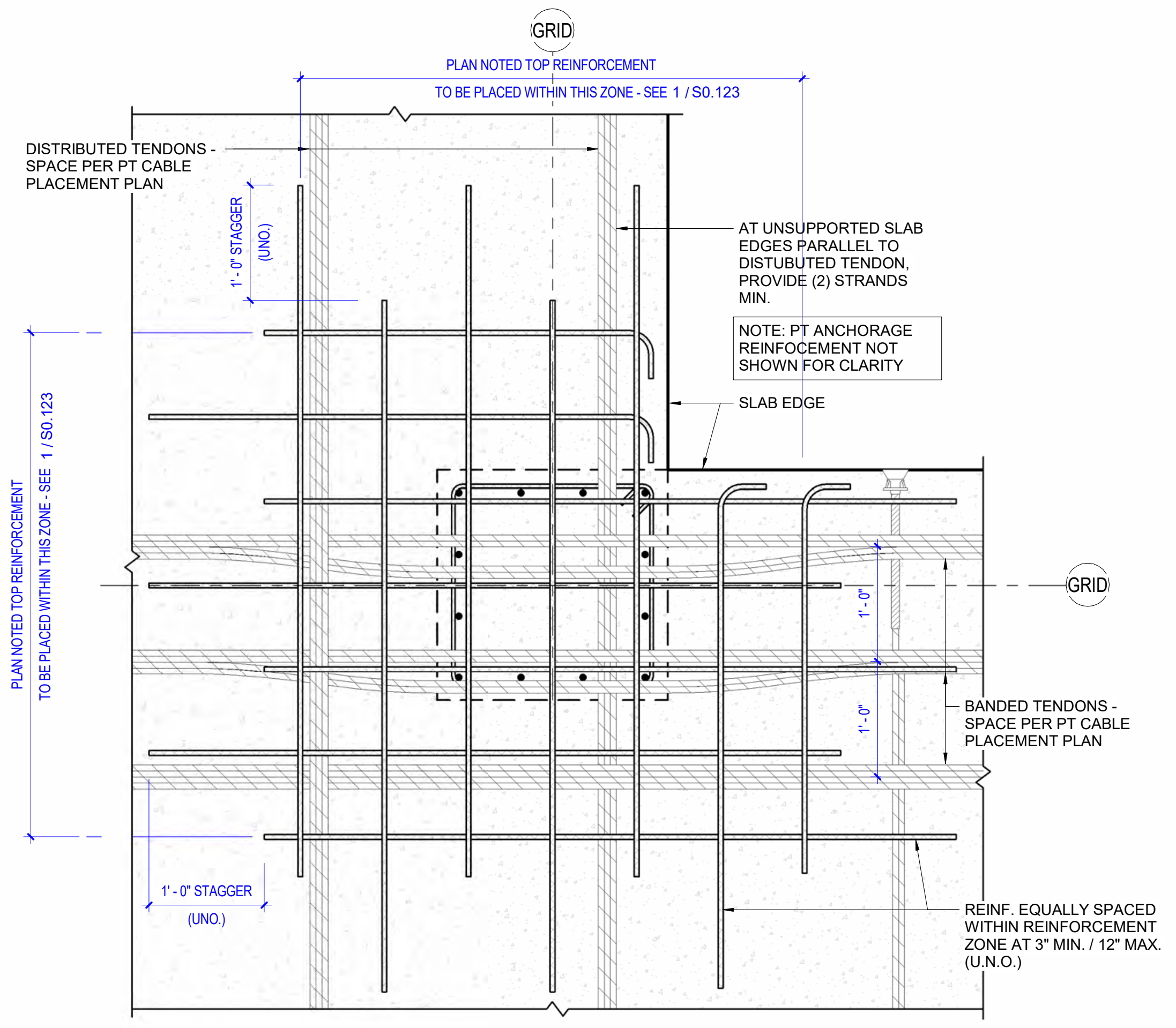

GRID
PLAN NOTED TOP REINFORCEMENT
TO BE PLACED WITHIN THIS ZONE - SEE 1 / S0.123
DISTRIBUTED TENDONS - SPACE PER PT CABLE PLACEMENT PLAN
1' - 0" STAGGER
U.N.O.
1' - 0" STAGGER
U.N.O.
BANDED TENDONS - SPACE PER PT CABLE PLACEMENT PLAN
PLAN NOTED TOP REINFORCEMENT
TO BE PLACED WITHIN THIS ZONE - SEE 1 / S0.123
REINF. EQUALLY SPACED WITHIN REINFORCEMENT ZONE AT 3" MIN. / 12" MAX. (U.N.O.)
GRID
SLAB EDGE
NOTE: PT ANCHORAGE REINFOCEMENT NOT SHOWN FOR CLARITY
AT UNSUPPORTED SLAB EDGES PARALLEL TO DISTUBUTED TENDON, PROVIDE (2) STRANDS MIN.

3 Top Reinforcement at Edge Column
1" = 1'-0"

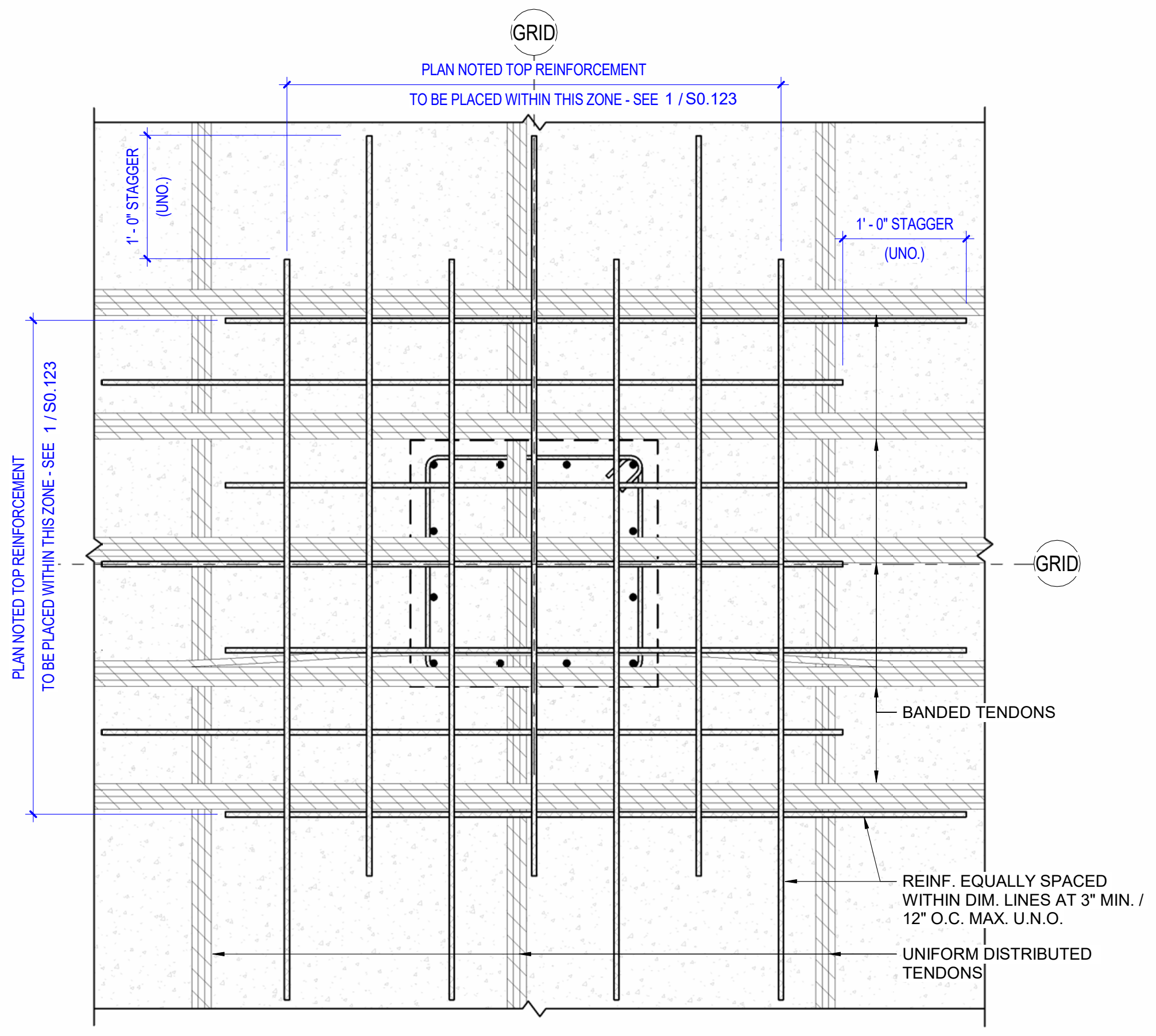


NOTE:
1. MAXIMUM CONDUIT OUTSIDE DIAMETER SHALL BE NO MORE THAN 0.3 X SLAB THICKNESS (t)
2. MINIMUM SPACING BETWEEN CONDUIT SHALL BE NO CLOSER THAN 3 X LARGEST ADJACENT CONDUIT DIAMETER (d)
3. CONDUIT SHALL BE PLACED AS CLOSE TO MID-DEPTH OF SLAB AS POSSIBLE BETWEEN TOP AND BOTTOM SLAB REINFORCEMENT
4. CONDUIT SHALL NOT BE PLACED WITHIN 2'-0" MINIMUM TO FACE OF COLUMNS
5. MINIMUM CONCRETE COVERAGE AROUND CONDUIT SHALL BE THE SAME AS THAT SPECIFIED FOR SLAB REINFORCEMENT
6. CONDUITS / PIPES EMBEDDED IN CONCRETE MAY NOT BE ALUMINUM

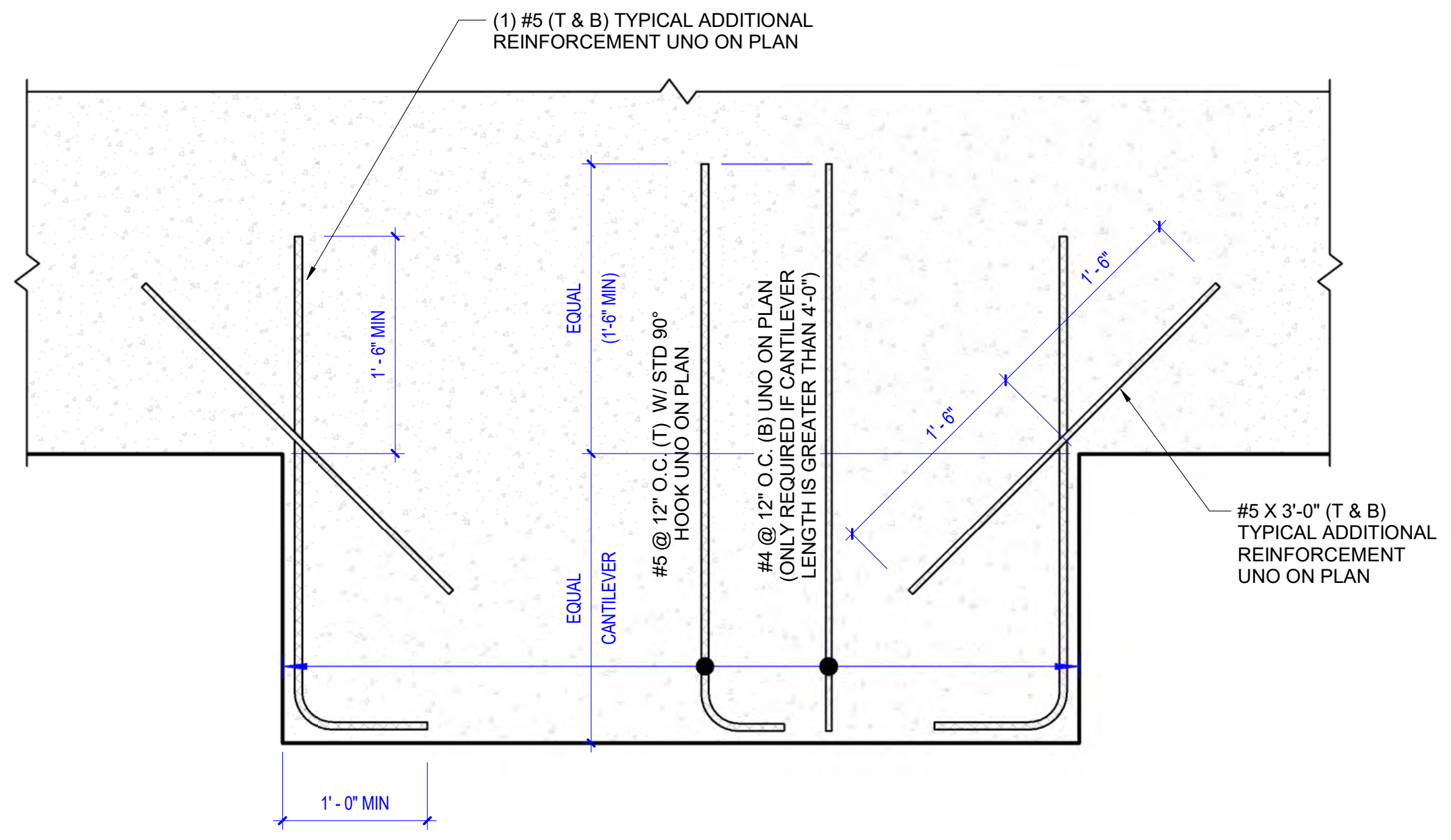
7 Embedded Conduits
1 1/2" = 1'-0"


GRID
PLAN NOTED TOP REINFORCEMENT
TO BE PLACED WITHIN THIS ZONE - SEE 1 / S0.123
DISTRIBUTED TENDONS - SPACE PER PT CABLE PLACEMENT PLAN
1' - 0" STAGGER
(UNO.)
AT UNSUPPORTED SLAB EDGES PARALLEL TO DISTUBUTED TENDON, PROVIDE (2) STRANDS MIN.
NOTE: PT ANCHORAGE REINFOCEMENT NOT SHOWN FOR CLARITY
SLAB EDGE
PLAN NOTED TOP REINFORCEMENT
TO BE PLACED WITHIN THIS ZONE - SEE 1 / S0.123
GRID
1' - 0"
1' - 0"
BANDED TENDONS - SPACE PER PT CABLE PLACEMENT PLAN
1' - 0" STAGGER
(UNO.)
REINF. EQUALLY SPACED WITHIN REINFORCEMENT ZONE AT 3" MIN. / 12" MAX. (U.N.O.)

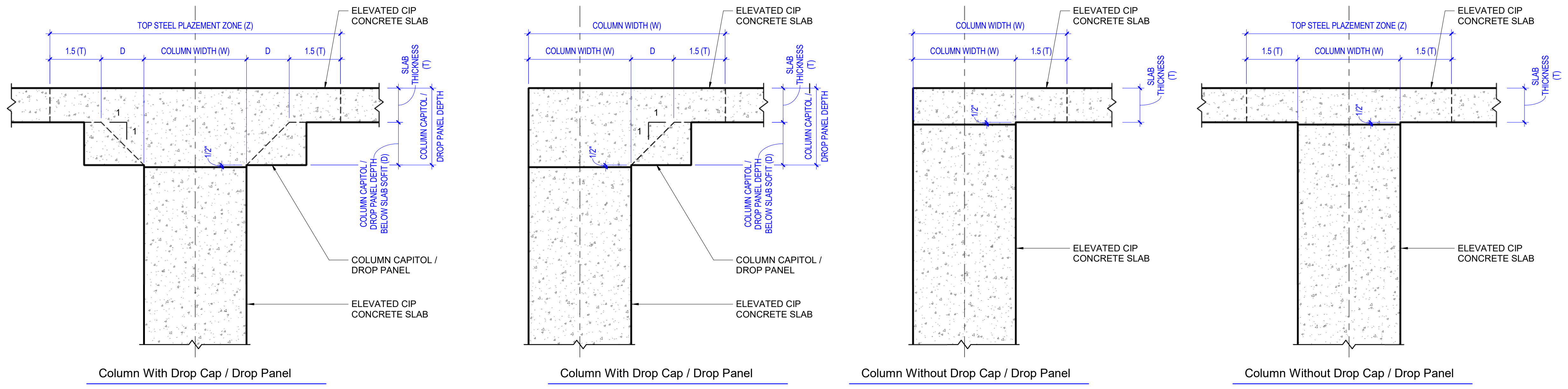
4 Top Reinforcement at Interior Corner Column
1" = 1'-0"


GRID
PLAN NOTED TOP REINFORCEMENT
TO BE PLACED WITHIN THIS ZONE - SEE 1 / S0.123
1' - 0" STAGGER
(UNO.)
1' - 0" STAGGER
(UNO.)
PLAN NOTED TOP REINFORCEMENT
TO BE PLACED WITHIN THIS ZONE - SEE 1 / S0.123
GRID
BANDED TENDONS
REINF. EQUALLY SPACED WITHIN DIM. LINES AT 3" MIN. / 12" O.C. MAX. U.N.O.
UNIFORM DISTRIBUTED TENDONS

2 Top Reinforcement at Interior Column
1" = 1'-0"


(1) #5 (T & B) TYPICAL ADDITIONAL REINFORCEMENT UNO ON PLAN
1' - 6" MIN
EQUAL
(1'-6" MIN)
EQUAL
CANTILEVER
#5 @ 12" O.C. (T) W/ STD 90° HOOK UNO ON PLAN
#4 @ 12" O.C. (B) UNO ON PLAN (ONLY REQUIRED IF CANTILEVER LENGTH IS GREATER THAN 4'-0")
1' - 6"
1' - 6"
#5 X 3'-0" (T & B) TYPICAL ADDITIONAL REINFORCEMENT UNO ON PLAN
1' - 0" MIN

6 Trim Bars at Cantilevered Slab
1" = 1'-0"


TOP STEEL PLAZEMENT ZONE (Z)
1.5 (T)
D
COLUMN WIDTH (W)
D
1.5 (T)
ELEVATED CIP CONCRETE SLAB
SLAB THICKNESS (T)
COLUMN CAPITOL / DROP PANEL DEPTH
COLUMN CAPITOL / DROP PANEL DEPTH BELOW SLAB SOFIT (D)
1
1
1/2"
COLUMN CAPITOL / DROP PANEL
ELEVATED CIP CONCRETE SLAB
COLUMN WIDTH (W)
COLUMN WIDTH (W)
D
1.5 (T)
ELEVATED CIP CONCRETE SLAB
SLAB THICKNESS (T)
COLUMN CAPITOL / DROP PANEL DEPTH
COLUMN CAPITOL / DROP PANEL DEPTH BELOW SLAB SOFIT (D)
1
1
1/2"
COLUMN CAPITOL / DROP PANEL
ELEVATED CIP CONCRETE SLAB
COLUMN WIDTH (W)
COLUMN WIDTH (W)
1.5 (T)
ELEVATED CIP CONCRETE SLAB
SLAB THICKNESS (T)
1/2"
ELEVATED CIP CONCRETE SLAB
TOP STEEL PLAZEMENT ZONE (Z)
1.5 (T)
COLUMN WIDTH (W)
1.5 (T)
ELEVATED CIP CONCRETE SLAB
SLAB THICKNESS (T)
1/2"
ELEVATED CIP CONCRETE SLAB

Column With Drop Cap / Drop Panel

Column With Drop Cap / Drop Panel

Column Without Drop Cap / Drop Panel

Column Without Drop Cap / Drop Panel

1 Top Reinforcement at Interior Corner Column
3/4" = 1'-0"

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Structural Engineers

PROJECT # : 26154
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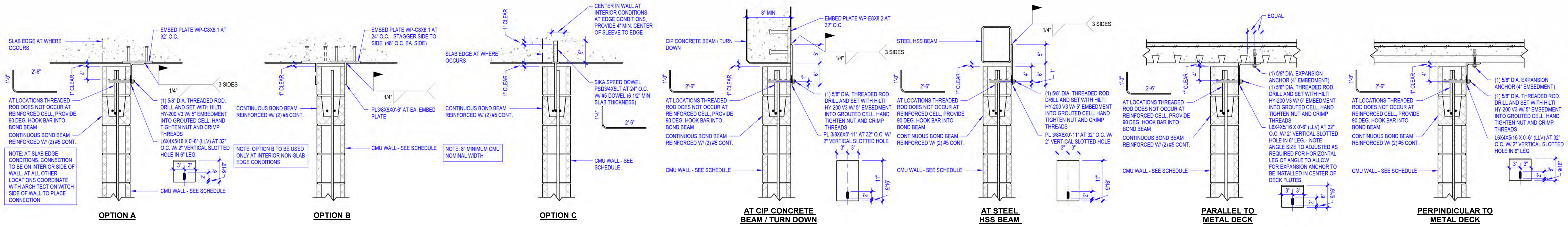
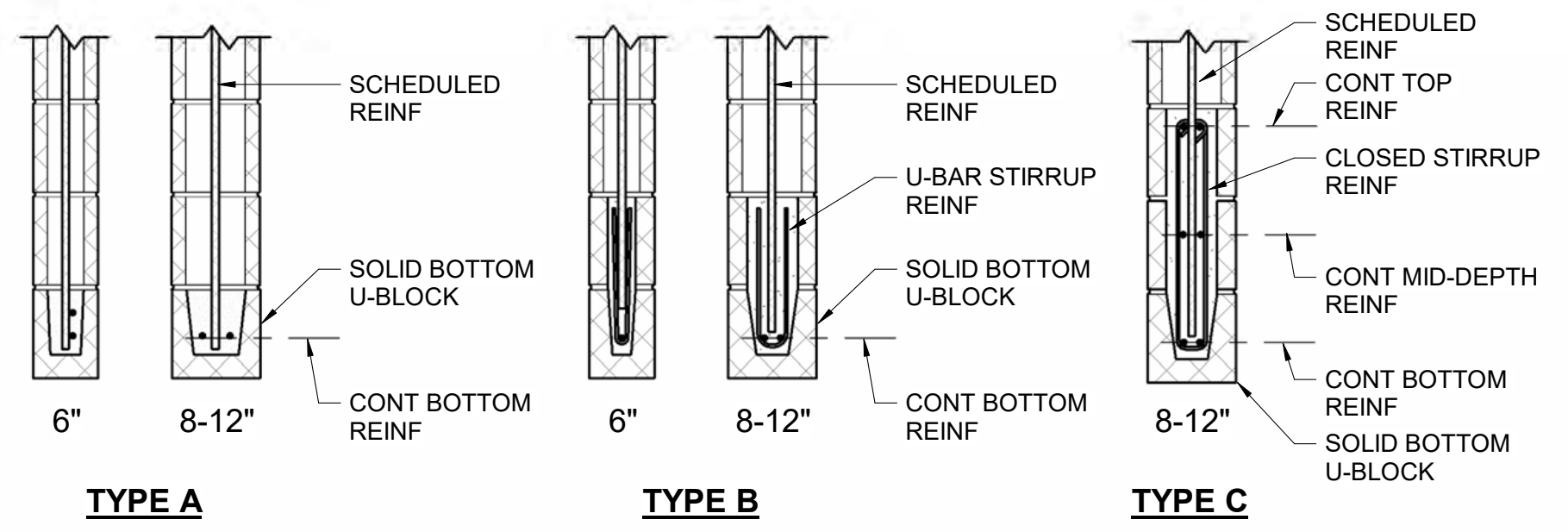

SLAB EDGE AT WHERE OCCURS
EMBED PLATE WP-C8X8.1 AT 32" O.C.
1'-0"
2'-6"
1" CLEAR
4"
1/4"
3 SIDES
AT LOCATIONS THREADED ROD DOES NOT OCCUR AT REINFORCED CELL, PROVIDE 90 DEG. HOOK BAR INTO BOND BEAM
CONTINUOUS BOND BEAM REINFORCED W/ (2) #5 CONT.
(1) 5/8" DIA. THREADED ROD. DRILL AND SET WITH HILTI HY-200 V3 W/ 5" EMBEDMENT INTO GROUTED CELL. HAND TIGHTEN NUT AND CRIMP THREADS
L6X4X5/16 X 0'-6" (LLV) AT 32" O.C. W/ 2" VERTICAL SLOTTED HOLE IN 6" LEG.
NOTE: AT SLAB EDGE CONDITIONS, CONNECTION TO BE ON INTERIOR SIDE OF WALL. AT ALL OTHER LOCATIONS COORDINATE WITH ARCHITECT ON WITCH SIDE OF WALL TO PLACE CONNECTION.
3" 3" 2" 6" 9/16"
CMU WALL - SEE SCHEDULE
OPTION A
EMBED PLATE WP-C8X8.1 AT 24" O.C. - STAGGER SIDE TO SIDE. (48" O.C. EA. SIDE)
CONTINUOUS BOND BEAM REINFORCED W/ (2) #5 CONT.
PL3/8X6X0'-6" AT EA. EMBED PLATE
NOTE: OPTION B TO BE USED ONLY AT INTERIOR NON-SLAB EDGE CONDITIONS
CMU WALL - SEE SCHEDULE
OPTION B
CENTER IN WALL AT INTERIOR CONDITIONS. AT EDGE CONDITIONS, PROVIDE 4" MIN. CENTER OF SLEEVE TO EDGE
SLAB EDGE AT WHERE OCCURS
5"
SIKA SPEED DOWEL PSD3/4X5LT AT 24" O.C. W/ #5 DOWEL (6 1/2" MIN. SLAB THICKNESS)
1'-4"
2'-6"
CONTINUOUS BOND BEAM REINFORCED W/ (2) #5 CONT.
NOTE: 8" MINIMUM CMU NOMINAL WIDTH
CMU WALL - SEE SCHEDULE
OPTION C
8" MIN.
CIP CONCRETE BEAM / TURN DOWN
EMBED PLATE WP-E8X8.2 AT 32" O.C.
5" 6" 1"
(1) 5/8" DIA. THREADED ROD. DRILL AND SET WITH HILTI HY-200 V3 W/ 5" EMBEDMENT INTO GROUTED CELL. HAND TIGHTEN NUT AND CRIMP THREADS
PL 3/8X6X0'-11" AT 32" O.C. W/ 2" VERTICAL SLOTTED HOLE
11" 9/16"
AT CIP CONCRETE BEAM / TURN DOWN
STEEL HSS BEAM
AT STEEL HSS BEAM
EQUAL
(1) 5/8" DIA. EXPANSION ANCHOR (4" EMBEDMENT)
L6X4X5/16 X 0'-6" (LLV) AT 32" O.C. W/ 2" VERTICAL SLOTTED HOLE IN 6" LEG. - NOTE: ANGLE SIZE TO ADJUSTED AS REQUIRED FOR HORIZONTAL LEG OF ANGLE TO ALLOW FOR EXPANSION ANCHOR TO BE INSTALLED IN CENTER OF DECK FLUTES
PARALLEL TO METAL DECK
L6X4X5/16 X 0'-6" (LLV) AT 32" O.C. W/ 2" VERTICAL SLOTTED HOLE IN 6" LEG
PERPINDICULAR TO METAL DECK

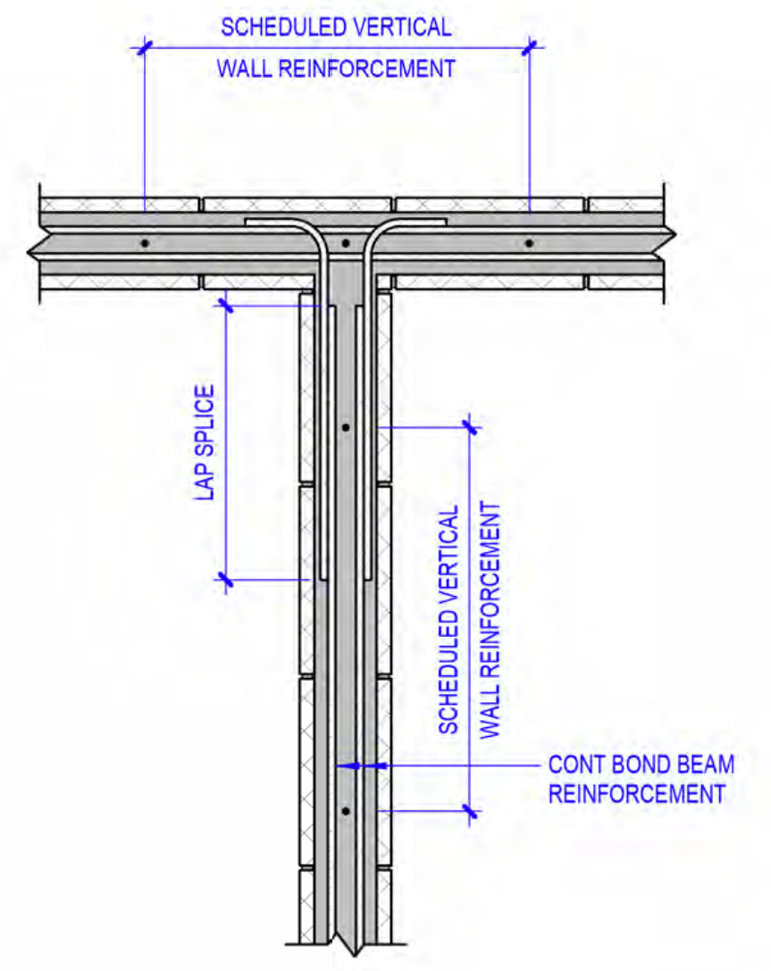
8 Attachment at Top of CMU Wall
1" = 1'-0"



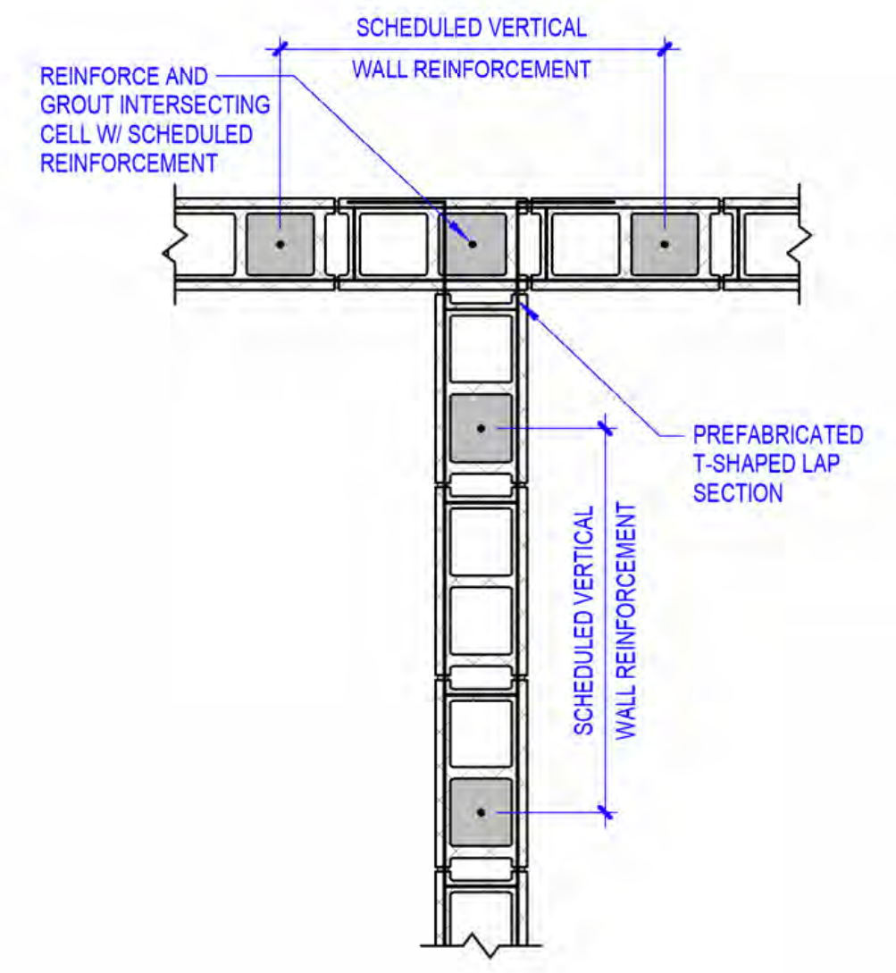
CMU LINTEL SCHEDULE

CMU SIZE	SPAN (LS)	TYPE	MIN. BEARING	CONT BOTT REINF	CONT TOP REINF	CONT MID-DEPTH REINF	STIRRUP REINF
6"	LS ≤ 5'-0"	A	8"	(1) #5	-	-	-
6"	LS > 5'-0" ≤ 8'-0"	B	8"	(1) #5	-	-	#2 AT 12" O.C.
8" - 12"	LS ≤ 5'-0"	A	8"	(2) #5	-	-	-
8" - 12"	LS > 5'-0" ≤ 8'-0"	B	8"	(2) #5	-	-	#2 AT 12" O.C.
8" - 12"	LS > 8'-0" ≤ 12'-0"	C	16"	(2) #5	(2) #5	(2) #5	#2 AT 12" O.C.

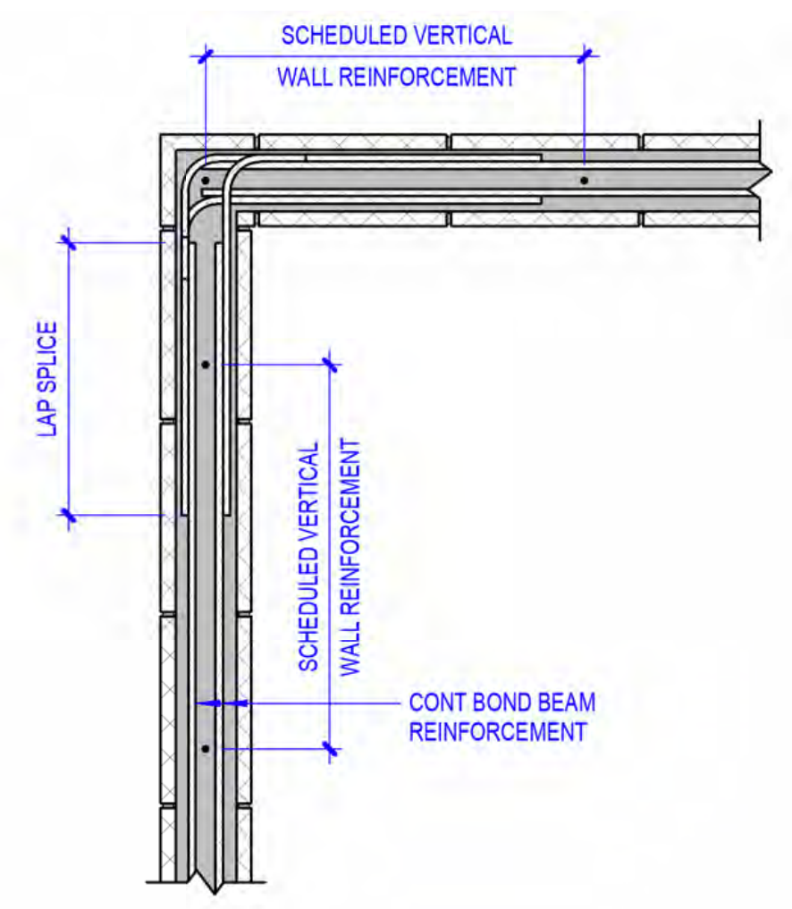
7 Opening in CMU Wall
3/4" = 1'-0"


SCHEDULED VERTICAL WALL REINFORCEMENT
LAP SPLICE
SCHEDULED VERTICAL WALL REINFORCEMENT
CONT BOND BEAM REINFORCEMENT

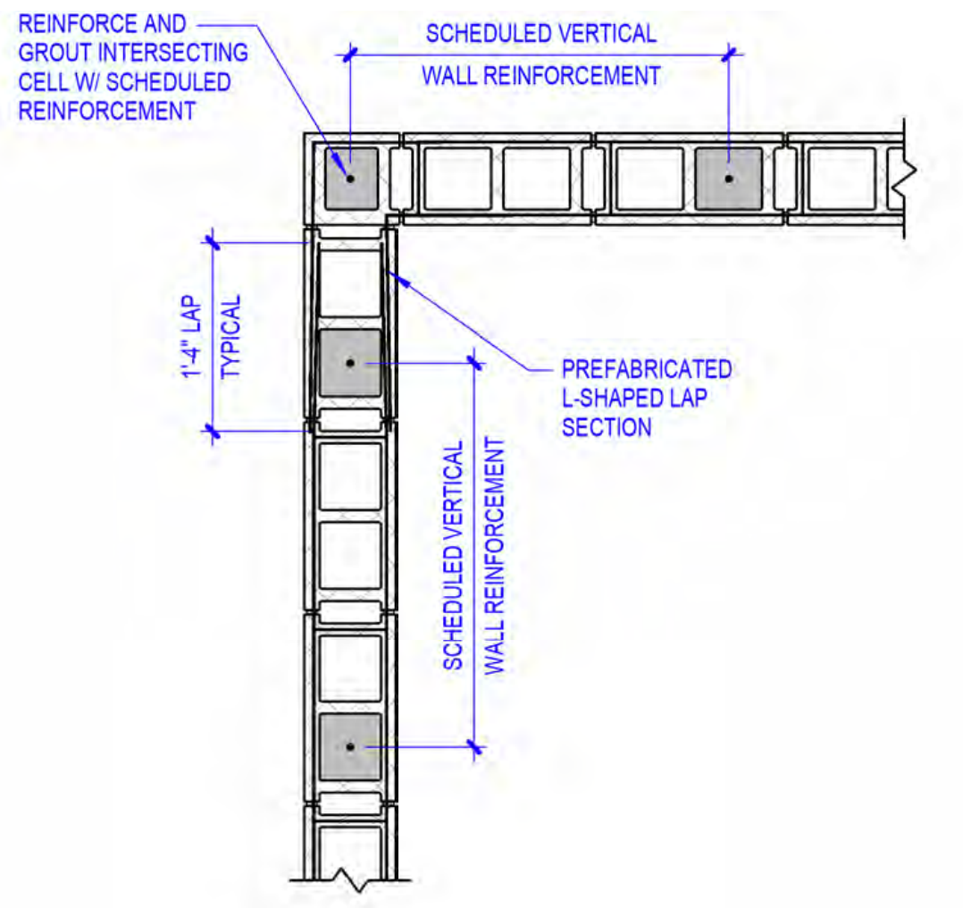
4 CMU Bond Beam Intersection - Plan
3/4" = 1'-0"


REINFORCE AND GROUT INTERSECTING CELL W/ SCHEDULED REINFORCEMENT
SCHEDULED VERTICAL WALL REINFORCEMENT
PREFABRICATED T-SHAPED LAP SECTION
SCHEDULED VERTICAL WALL REINFORCEMENT

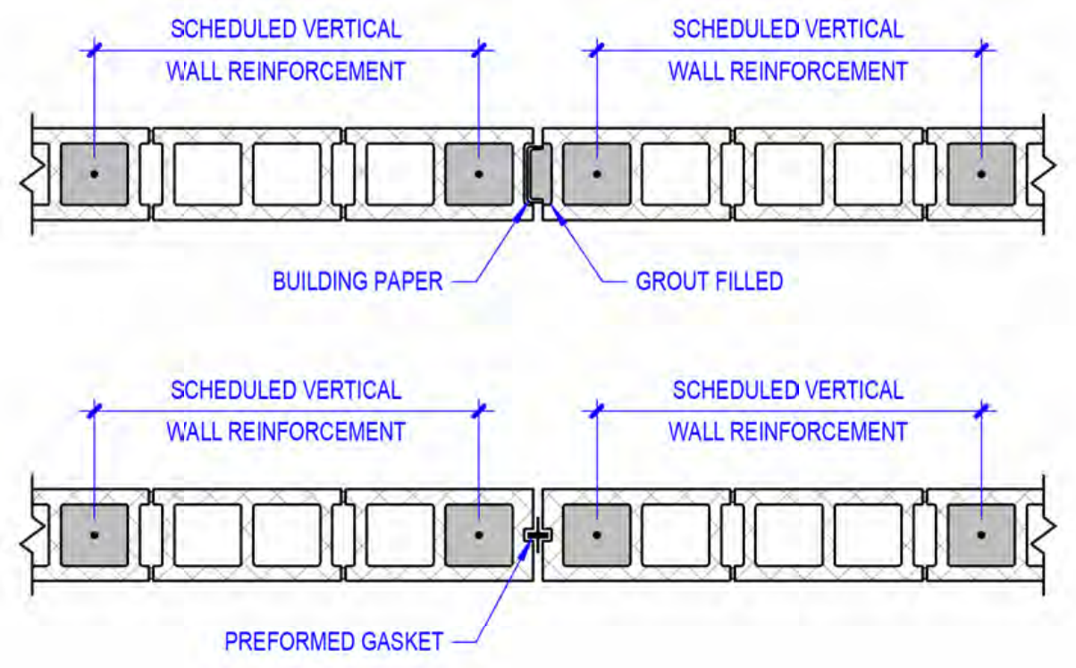
3 CMU Intersection - Plan
3/4" = 1'-0"


SCHEDULED VERTICAL WALL REINFORCEMENT
LAP SPLICE
SCHEDULED VERTICAL WALL REINFORCEMENT
CONT BOND BEAM REINFORCEMENT

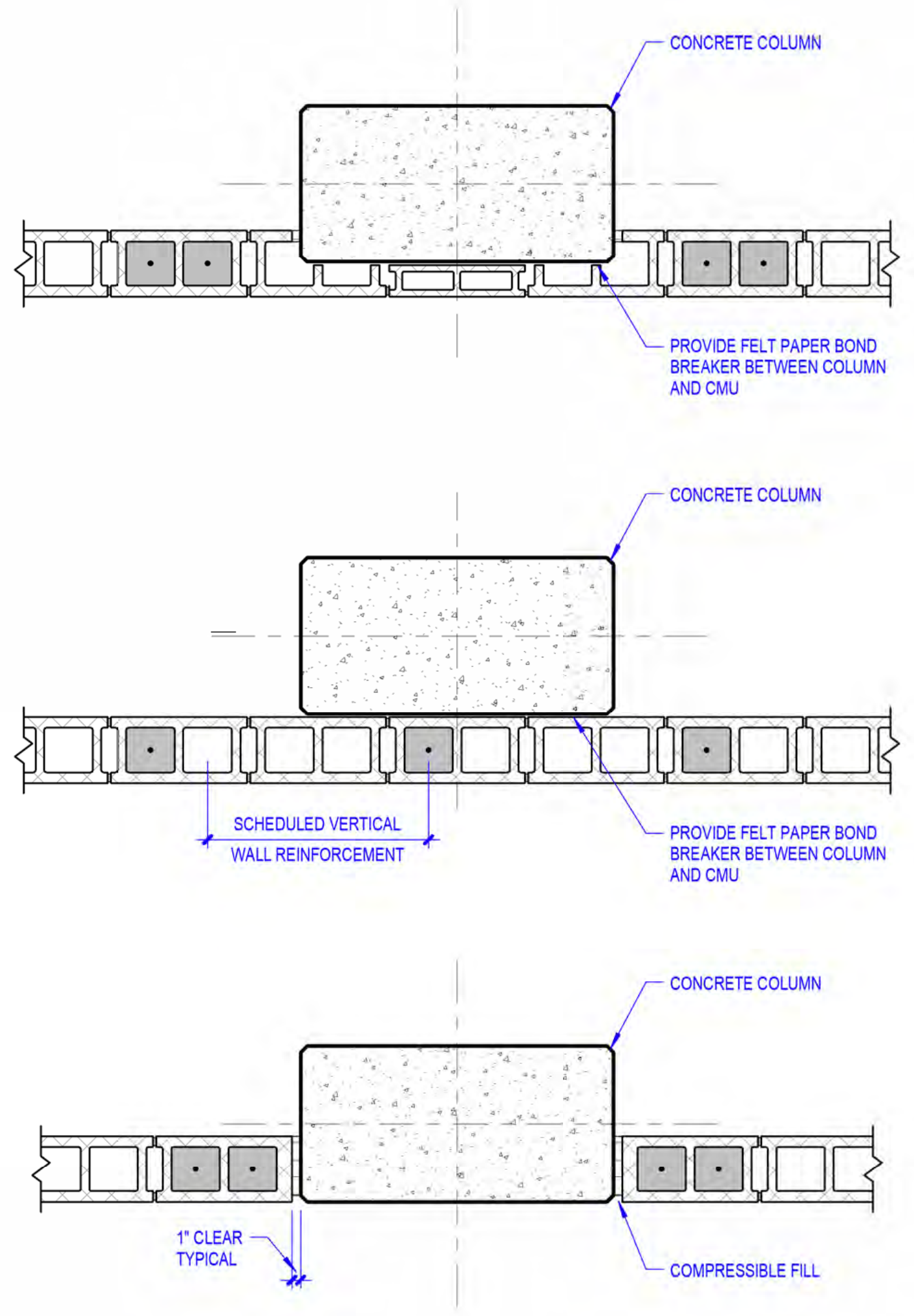
2 CMU Bond Beam Corner - Plan
3/4" = 1'-0"


REINFORCE AND GROUT INTERSECTING CELL W/ SCHEDULED REINFORCEMENT
SCHEDULED VERTICAL WALL REINFORCEMENT
1'-4" LAP TYPICAL
PREFABRICATED L-SHAPED LAP SECTION
SCHEDULED VERTICAL WALL REINFORCEMENT

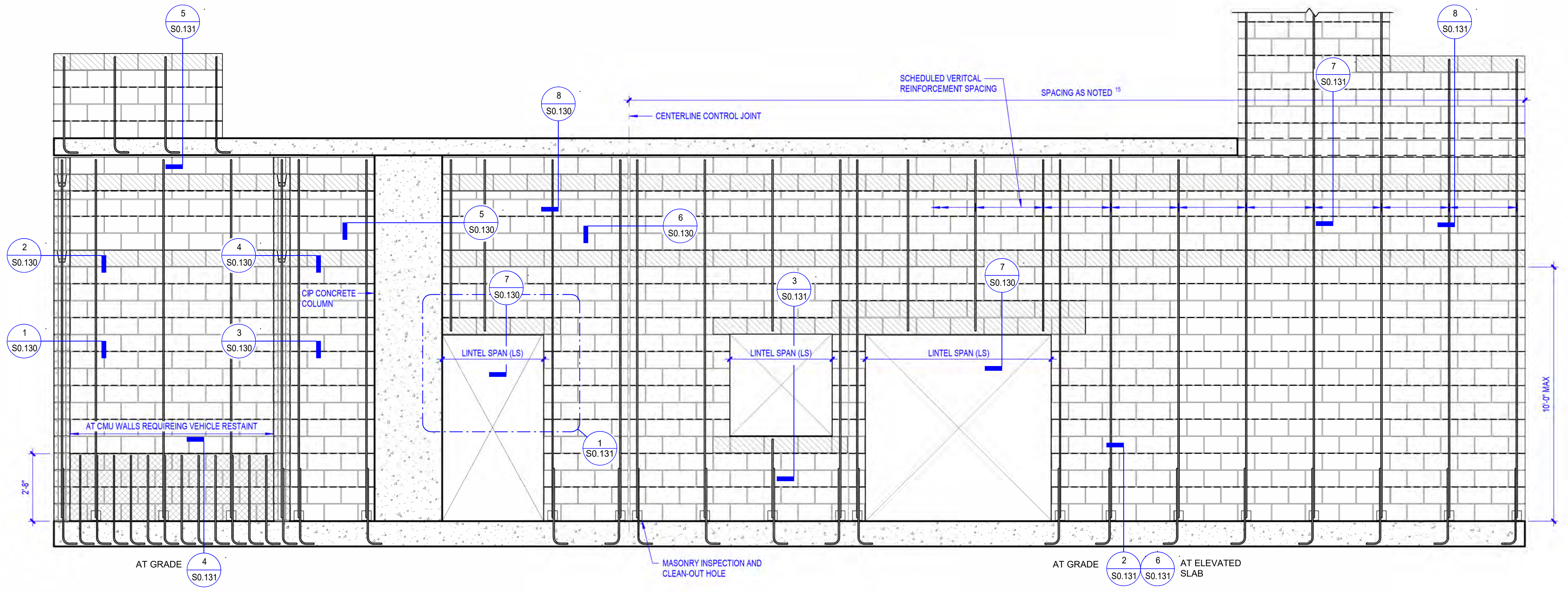
1 CMU Corner - Plan
3/4" = 1'-0"


SCHEDULED VERTICAL WALL REINFORCEMENT
SCHEDULED VERTICAL WALL REINFORCEMENT
BUILDING PAPER
GROUT FILLED
SCHEDULED VERTICAL WALL REINFORCEMENT
SCHEDULED VERTICAL WALL REINFORCEMENT
PREFORMED GASKET

6 CMU Control Joint
3/4" = 1'-0"


CONCRETE COLUMN
PROVIDE FELT PAPER BOND BREAKER BETWEEN COLUMN AND CMU
CONCRETE COLUMN
SCHEDULED VERTICAL WALL REINFORCEMENT
PROVIDE FELT PAPER BOND BREAKER BETWEEN COLUMN AND CMU
CONCRETE COLUMN
1" CLEAR TYPICAL
COMPRESSIBLE FILL

5 CMU at Concrete Column
3/4" = 1'-0"



CMU WALL DIAGRAM

CONCRETE MASONRY UNIT (CMU) NOTES:

1. CONCRETE MASONRY WALL MATERIALS, CONSTRUCTION, AND INSPECTION SHALL CONFORM TO THE REQUIREMENTS PRESCRIBED IN THE AMERICAN CONCRETE INSTITUTE "BUILDING CODE REQUIREMENTS OF MASONRY STRUCTURES" (ACI 530/ASCE 5/TMS 402) AND THE "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602), AS WELL AS REQUIREMENTS PRESCRIBED BY LOCAL BUILDING CODE.
2. REFER TO ARCHITECTURAL DRAWINGS AND DETAILS FOR DIMENSIONAL CONTROL OF MASONRY CONSTRUCTION.
3. CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT CONFORMING TO ASTM C-90 AND HAVE MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1,900 PSI UNLESS NOTED OTHERWISE.
4. MASONRY MORTAR SHALL CONFORM TO ASTM C-270, TYPE M OR S. TYPE M MORTAR SHALL HAVE MINIMUM 28-DAY STRENGTH = 2,500 PSI. TYPE S MORTAR SHALL HAVE MINIMUM 28-DAY STRENGTH = 1,800 PSI.

RECOMMENDED MORTAR TYPE BY USE:

BUILDING SEGMENT	MORTAR TYPE
EXTERIOR, ABOVE GRADE:	
• LOAD BEARING	S or M
• NON-LOAD BEARING & PARAPET	S
EXTERIOR, AT OR BELOW GRADE	S or M
INTERIOR:	
• LOAD BEARING	S or M
• NON-LOAD BEARING	S

5. REINFORCED CELLS AND BOND BEAMS SHALL BE GROUTED SOLID. GROUT SHALL CONFORM TO ASTM C-476 AND HAVE MINIMUM 28-DAY STRENGTH = 2,500 PSI. USE COURSE GROUT FOR SPACES 4" OR LARGER, AND USE FINE GROUT FOR SPACES SMALLER THAN 4". GROUT SHALL BE CONSOLIDATED BY THOROUGH RODDING OR VIBRATING ALL CELLS DURING PLACEMENT. AIR ENTRAINMENT IS NOT PERMITTED.
6. GROUT SOLID ALL CELLS BELOW GRADE.
7. CONCRETE MASONRY SHALL HAVE A MINIMUM PRISM ASSEMBLY STRENGTH OF 1,900 PSI UNLESS NOTED OTHERWISE.
8. CMU SHALL BE CONSTRUCTED IN A RUNNING BOND CONFIGURATION WITH LOW-LIFT GROUT PLACEMENT.
9. REBAR SHALL CONFORM TO ASTM A-615, GRADE 60 UNLESS NOTED OTHERWISE. UTILIZE BAR SUPPORTS AND POSITIONERS AS REQUIRED TO ENSURE PROPER PLACEMENT WITHIN WALLS AS NOTED IN THE DRAWINGS.
10. PROVIDE MINIMUM ONE VERTICAL BAR EACH SIDE OF OPENINGS UP TO 4'-0 WIDE. PROVIDE MINIMUM TWO VERTICAL BARS EACH SIDE OF OPENING GREATER THAN 4'-0. PROVIDE MINIMUM ONE VERTICAL BAR AT WALL CORNERS, INTERSECTIONS AND EACH SIDE OF CONTROL JOINTS. EDGE AND INTERSECTION REINFORCING SHALL MATCH TYPICAL WALL REINFORCING SIZE AND EXTEND FULL HEIGHT OF WALL.
11. PROVIDE DOWELS INTO SUPPORTING STRUCTURE (FOOTINGS, BEAMS, SLABS) TO MATCH SIZE AND SPACING OF WALL VERTICAL REINFORCEMENT. REFER TO TYPICAL DETAILS FOR ADDITIONAL INFORMATION.
12. PROVIDE MINIMUM REBAR LAP LENGTHS AS NOTED BELOW:
    - #4: 20"
    - #5: 28"
    - #6: 45"
    - #7: 62"
    a. LAPS SHALL OCCUR MINIMUM 24" CLEAR OF INTERSECTING BOND BEAMS.
13. PROVIDE CONTINUOUS 9 GAUGE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C.. PROVIDE PREFABRICATED L-SHAPED AND T-SHAPED LAP SECTIONS AT WALL INTERSECTIONS. FULLY EMBED IN MORTAR BED AND EXTEND ENTIRE LENGTH OF BED JOINT.
14. PROVIDE CONTINUOUS HORIZONTAL BOND BEAM AT FLOOR LEVELS, ROOF LEVEL, TOP OF PARAPET, AND AS SHOWN IN TYPICAL DETAILS. WHERE WALL HEIGHT EXCEEDS 12'-0, PROVIDE CONTINUOUS HORIZONTAL BOND BEAM AT 10'-0 O.C. MAX. UTILIZE BOND BEAM BLOCKS OR KNOCK OUT BLOCKS AND REINFORCE WITH (2) #5 CONTINUOUS UNLESS NOTED OTHERWISE. KNOCK OUT BLOCKS IN WALLS SHALL HAVE METAL LATH, MESH, OR WIRE SCREEN IN BED JOINTS TO PREVENT FILLING OF UNGROUTED CELLS. USE OF ROOFING FELT OR MATERIALS THAT BREAK BOND BETWEEN UNITS AND MORTAR ARE NOT ACCEPTABLE.
15. U.N.O. ON ARCHITECTURAL NOTES AND SPECIFICATIONS, CONTROL JOINT SPACING SHALL NOT EXCEED THE SMALLER OF 2.5 TIMES WALL HEIGHT OR 25'-0". REINFORCE CELLS ADJACENT TO CONTROL JOINT WITH CONTINUOUS VERTICAL REINFORCEMENT AND GROUT SOLID. CONTROL JOINT SHALL NOT BE PLACED CLOSER THAN 2'-0" FROM THE VERTICAL FACE OF ANY OPENING. HORIZONTAL BOND BEAM REINFORCEMENT SHALL BE CONTINUOUS THROUGH JOINTS U.N.O..
16. CONCRETE MASONRY WALLS SHOWN IN THE DRAWINGS ARE DETAILED TO RESIST CODE REQUIRED LOADS AFTER REACHING THEIR FINAL BUILT CONFIGURATION ASSUMING TOP, BOTTOM, AND SIDE CONNECTIONS/BRACING AS SHOWN. THE CONTRACTOR SHALL PROVIDE ENGINEERED BRACING FOR MASONRY WALLS DURING CONSTRUCTION, WHICH SHALL NOT BE REMOVED UNTIL WALLS ARE CONNECTED TO SUPPORTING ELEMENTS AT ALL POINTS AS NOTED IN THE CONTRACT DOCUMENTS.
17. SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH "SPECIAL INSPECTIONS" SECTION OF THESE DRAWINGS.

CMU SCHEDULE - INTERIOR WALLS

HEIGHT (H)	SIZE	REINFORCEMENT
H≤10'-0"	8" CMU	#5 AT 32" O.C.
H>10'-0" ≤ 15'-0"	8" CMU	#5 AT 32" O.C.
H>15'-0" ≤ 20'-0"	8" CMU	#5 AT 24" O.C.
H>20'-0" ≤ 25'-0"	10" CMU	#5 AT 16" O.C.
H>25'-0" ≤ 30'-0"	12" CMU	#5 AT 16" O.C.

CMU SCHEDULE - EXTERIOR WALLS

HEIGHT (H)	SIZE	REINFORCEMENT
H≤10'-0"	8" CMU	#5 AT 32" O.C.
H>10'-0" ≤ 15'-0"	8" CMU	#4 AT 16" O.C.
H>15'-0" ≤ 20'-0"	10" CMU	#6 AT 24" O.C.

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- NO. 85617

SHEET TITLE:
CMU General Notes / Typical Details

SHEET NUMBER:
S0.130

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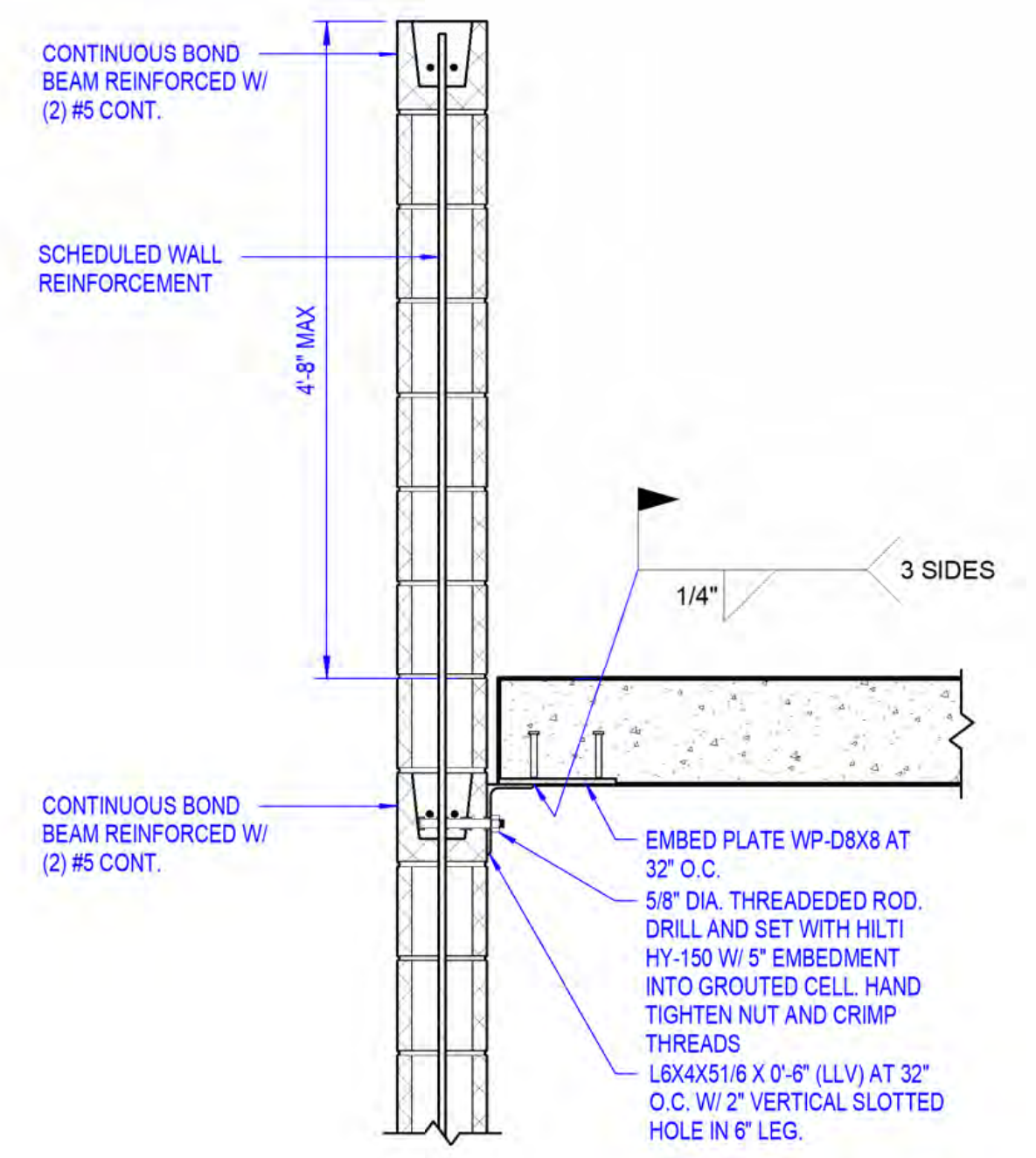
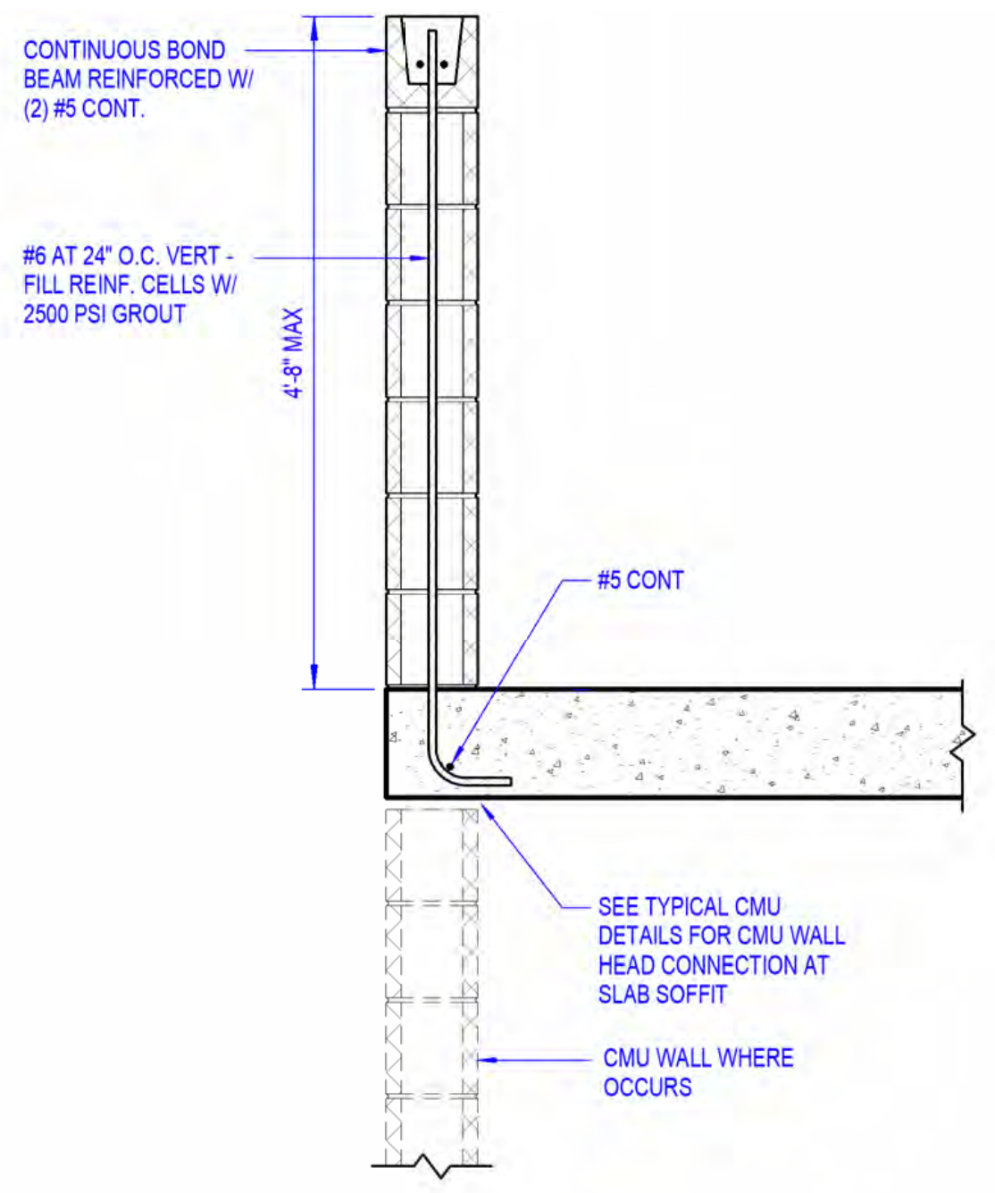
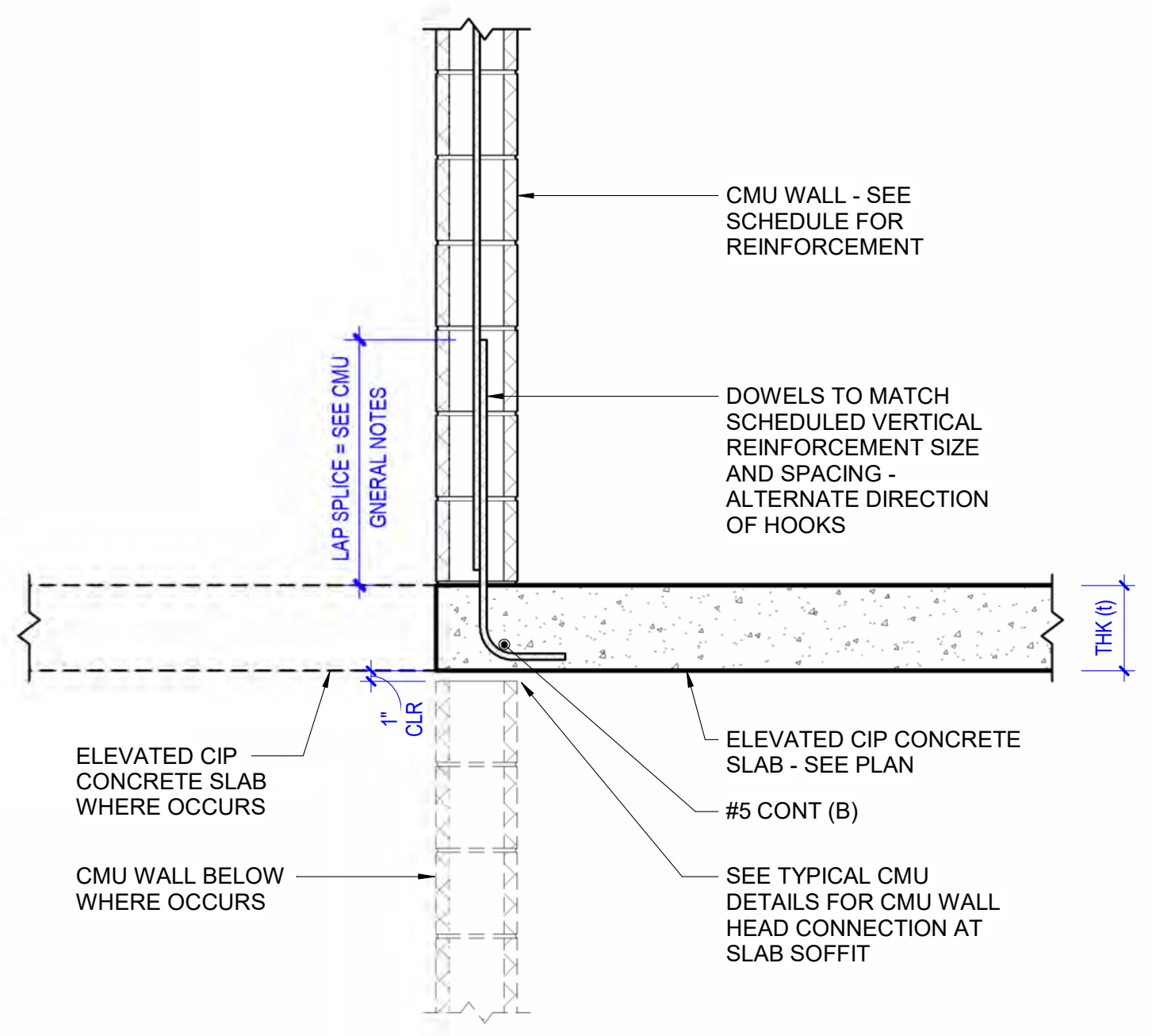

CONTINUOUS BOND BEAM REINFORCED W/ (2) #5 CONT.
SCHEDULED WALL REINFORCEMENT
4'-8" MAX
1/4"
3 SIDES
CONTINUOUS BOND BEAM REINFORCED W/ (2) #5 CONT.
EMBED PLATE WP-D8X8 AT 32" O.C.
5/8" DIA. THREADEDED ROD. DRILL AND SET WITH HILTI HY-150 W/ 5" EMBEDMENT INTO GROUTED CELL. HAND TIGHTEN NUT AND CRIMP THREADS
L6X4X51/6 X 0'-6" (LLV) AT 32" O.C. W/ 2" VERTICAL SLOTTED HOLE IN 6" LEG.

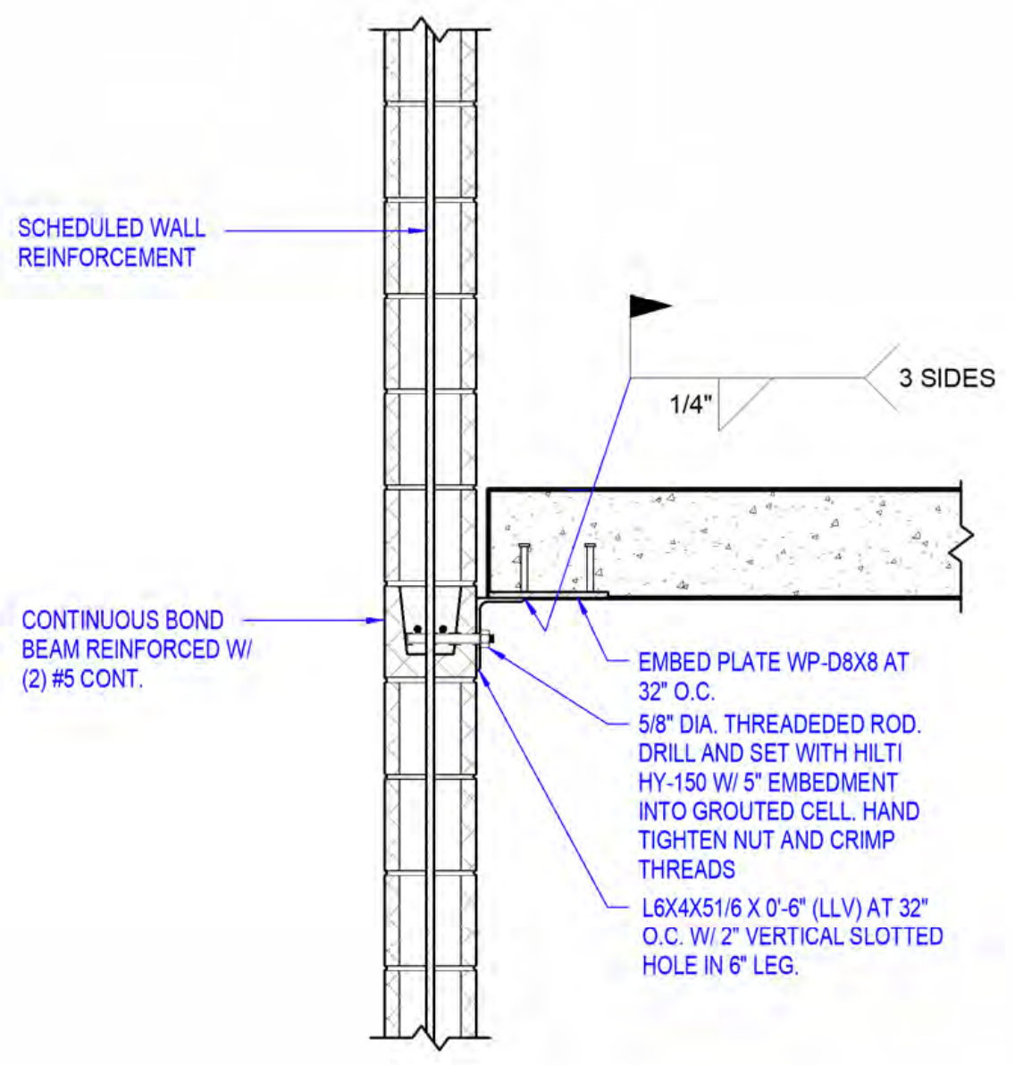
8 CMU at Bypass Cantilevered Wall
3/4" = 1'-0"


CONTINUOUS BOND BEAM REINFORCED W/ (2) #5 CONT.
#6 AT 24" O.C. VERT - FILL REINF. CELLS W/ 2500 PSI GROUT
4'-8" MAX
#5 CONT
SEE TYPICAL CMU DETAILS FOR CMU WALL HEAD CONNECTION AT SLAB SOFFIT
CMU WALL WHERE OCCURS

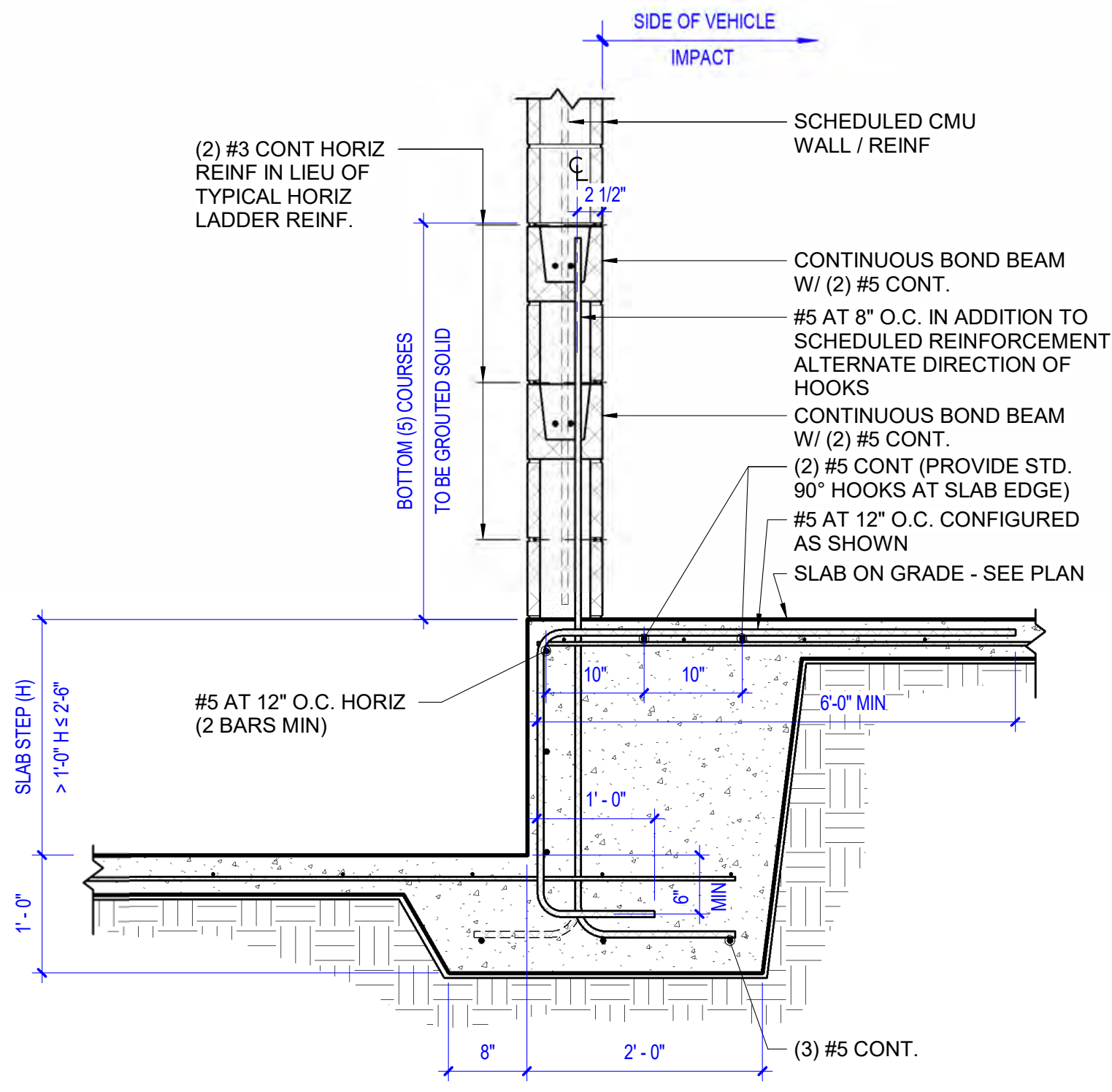
7 CMU Cantilevered Parapet
3/4" = 1'-0"


CMU WALL - SEE SCHEDULE FOR REINFORCEMENT
LAP SPLICE = SEE CMU GNERAL NOTES
DOWELS TO MATCH SCHEDULED VERTICAL REINFORCEMENT SIZE AND SPACING - ALTERNATE DIRECTION OF HOOKS
THK (t)
1" CLR
ELEVATED CIP CONCRETE SLAB WHERE OCCURS
CMU WALL BELOW WHERE OCCURS
ELEVATED CIP CONCRETE SLAB - SEE PLAN
#5 CONT (B)
SEE TYPICAL CMU DETAILS FOR CMU WALL HEAD CONNECTION AT SLAB SOFFIT

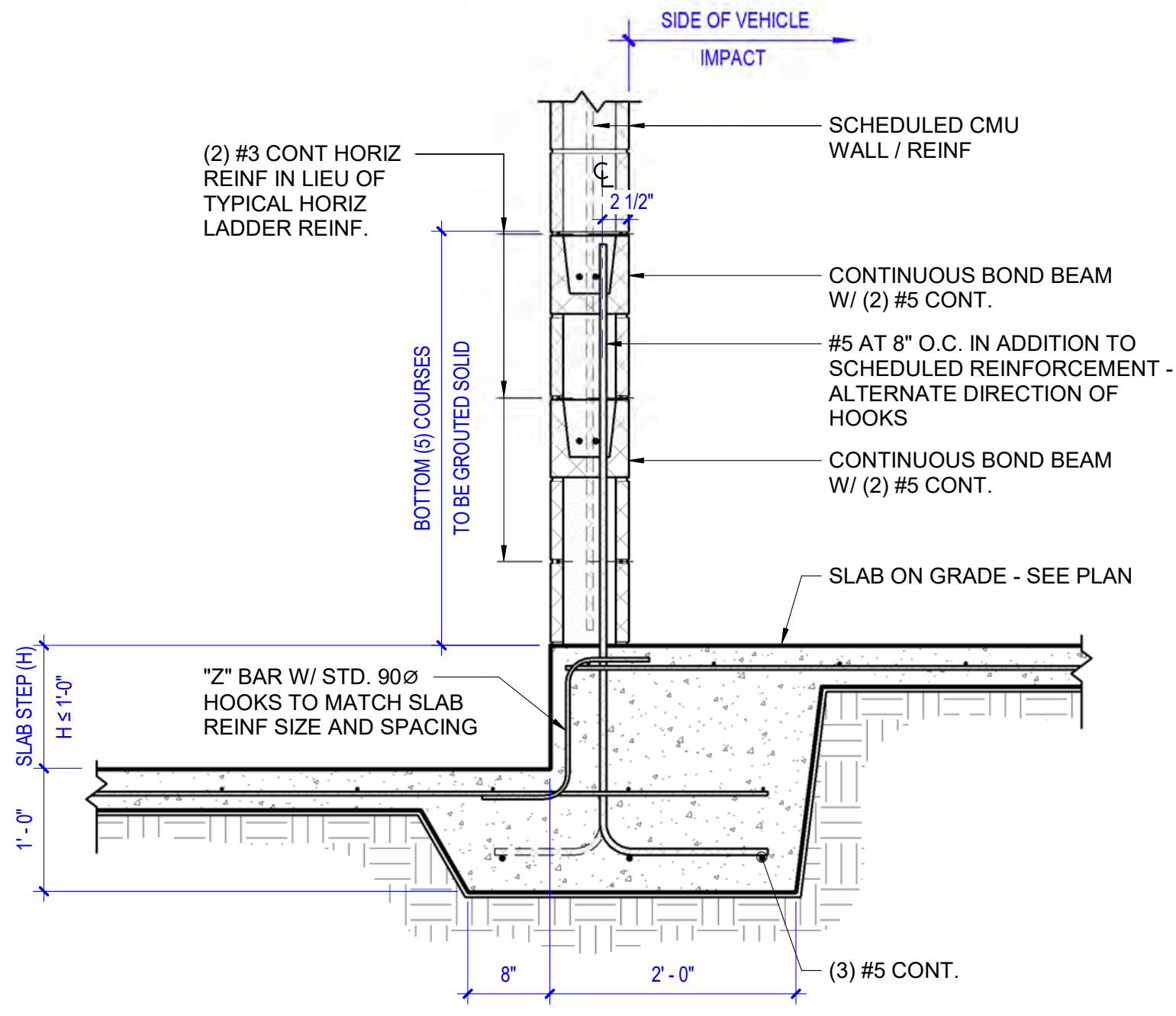
6 CMU Bearing at Elevated Slab
3/4" = 1'-0"


SCHEDULED WALL REINFORCEMENT
1/4"
3 SIDES
CONTINUOUS BOND BEAM REINFORCED W/ (2) #5 CONT.
EMBED PLATE WP-D8X8 AT 32" O.C.
5/8" DIA. THREADEDED ROD. DRILL AND SET WITH HILTI HY-150 W/ 5" EMBEDMENT INTO GROUTED CELL. HAND TIGHTEN NUT AND CRIMP THREADS
L6X4X51/6 X 0'-6" (LLV) AT 32" O.C. W/ 2" VERTICAL SLOTTED HOLE IN 6" LEG.

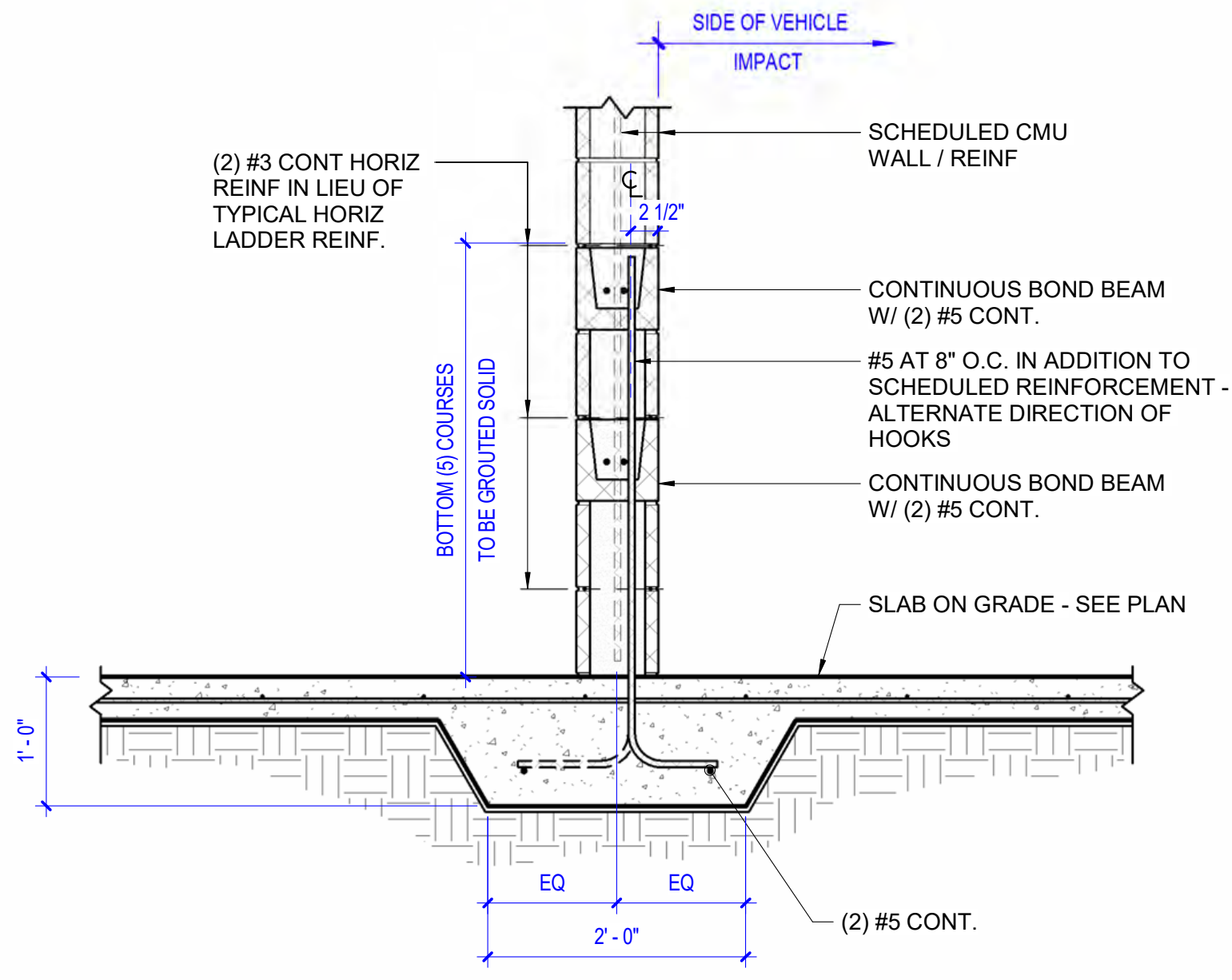
5 CMU at Continuous Bypass Condition
3/4" = 1'-0"


SIDE OF VEHICLE
IMPACT
SCHEDULED CMU WALL / REINF
(2) #3 CONT HORIZ REINF IN LIEU OF TYPICAL HORIZ LADDER REINF.
2 1/2"
CONTINUOUS BOND BEAM W/ (2) #5 CONT.
#5 AT 8" O.C. IN ADDITION TO SCHEDULED REINFORCEMENT - ALTERNATE DIRECTION OF HOOKS
CONTINUOUS BOND BEAM W/ (2) #5 CONT.
(2) #5 CONT (PROVIDE STD. 90° HOOKS AT SLAB EDGE)
#5 AT 12" O.C. CONFIGURED AS SHOWN
SLAB ON GRADE - SEE PLAN
BOTTOM (5) COURSES TO BE GROUTED SOLID
SLAB STEP (H) > 1'-0" H ≤ 2'-6"
#5 AT 12" O.C. HORIZ (2 BARS MIN)
10"
10"
6'-0" MIN
1'-0"
6" MIN
1'-0"
8"
2'-0"
(3) #5 CONT.

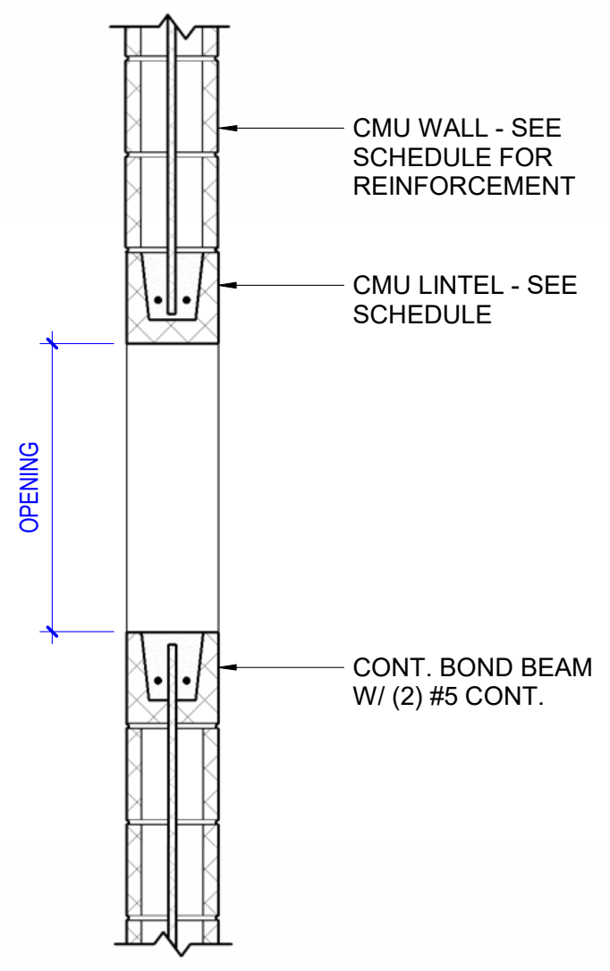
SLAB STEP (> 1'-0" H ≤ 2'-6")


SIDE OF VEHICLE
IMPACT
SCHEDULED CMU WALL / REINF
(2) #3 CONT HORIZ REINF IN LIEU OF TYPICAL HORIZ LADDER REINF.
2 1/2"
CONTINUOUS BOND BEAM W/ (2) #5 CONT.
#5 AT 8" O.C. IN ADDITION TO SCHEDULED REINFORCEMENT - ALTERNATE DIRECTION OF HOOKS
CONTINUOUS BOND BEAM W/ (2) #5 CONT.
SLAB ON GRADE - SEE PLAN
BOTTOM (5) COURSES TO BE GROUTED SOLID
SLAB STEP (H) H ≤ 1'-0"
"Z" BAR W/ STD. 90Ø HOOKS TO MATCH SLAB REINF SIZE AND SPACING
1'-0"
8"
2'-0"
(3) #5 CONT.

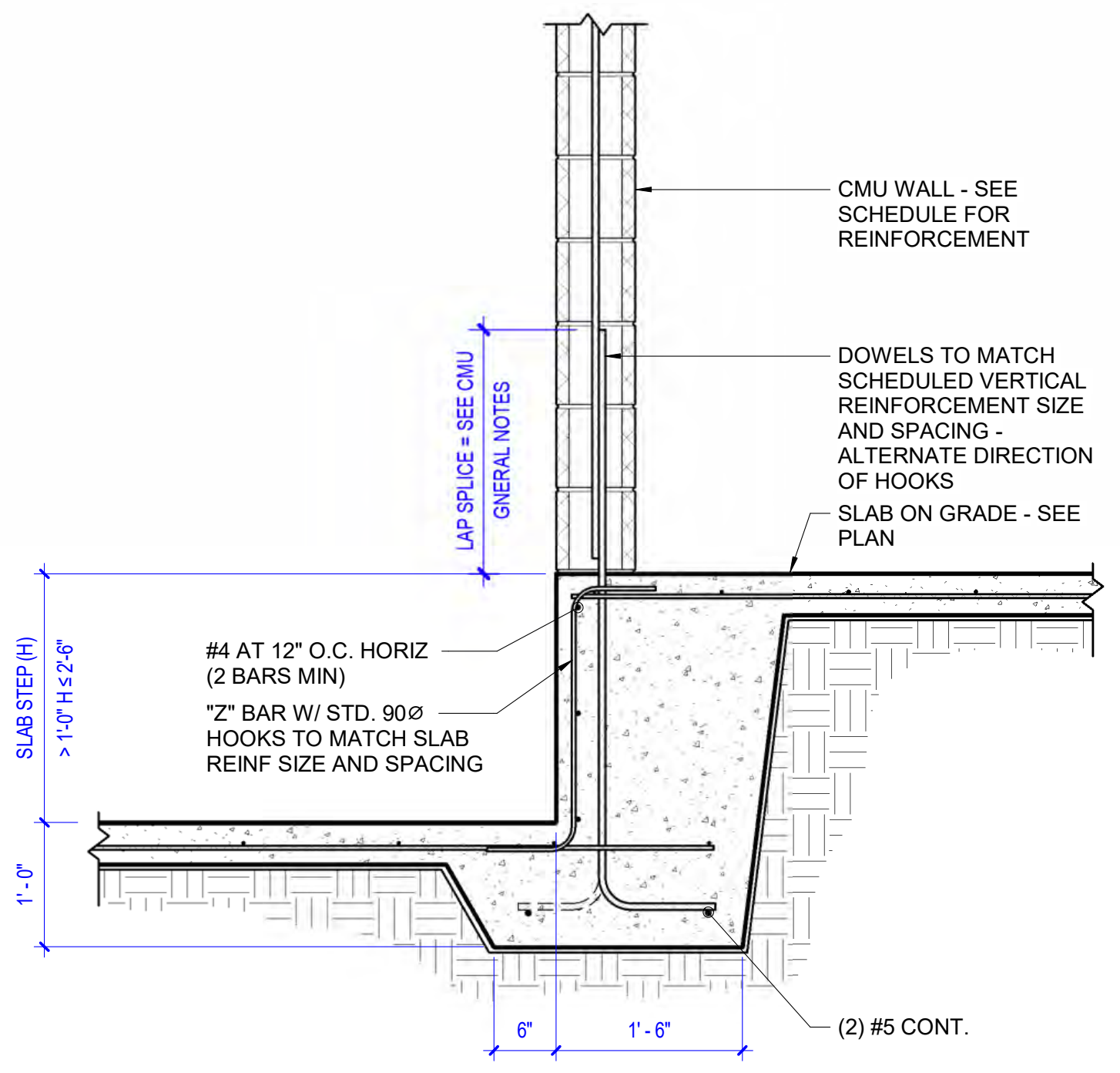
SLAB STEP (H ≤ 1'-0")


SIDE OF VEHICLE
IMPACT
SCHEDULED CMU WALL / REINF
(2) #3 CONT HORIZ REINF IN LIEU OF TYPICAL HORIZ LADDER REINF.
2 1/2"
CONTINUOUS BOND BEAM W/ (2) #5 CONT.
#5 AT 8" O.C. IN ADDITION TO SCHEDULED REINFORCEMENT - ALTERNATE DIRECTION OF HOOKS
CONTINUOUS BOND BEAM W/ (2) #5 CONT.
SLAB ON GRADE - SEE PLAN
BOTTOM (5) COURSES TO BE GROUTED SOLID
1'-0"
EQ
EQ
2'-0"
(2) #5 CONT.

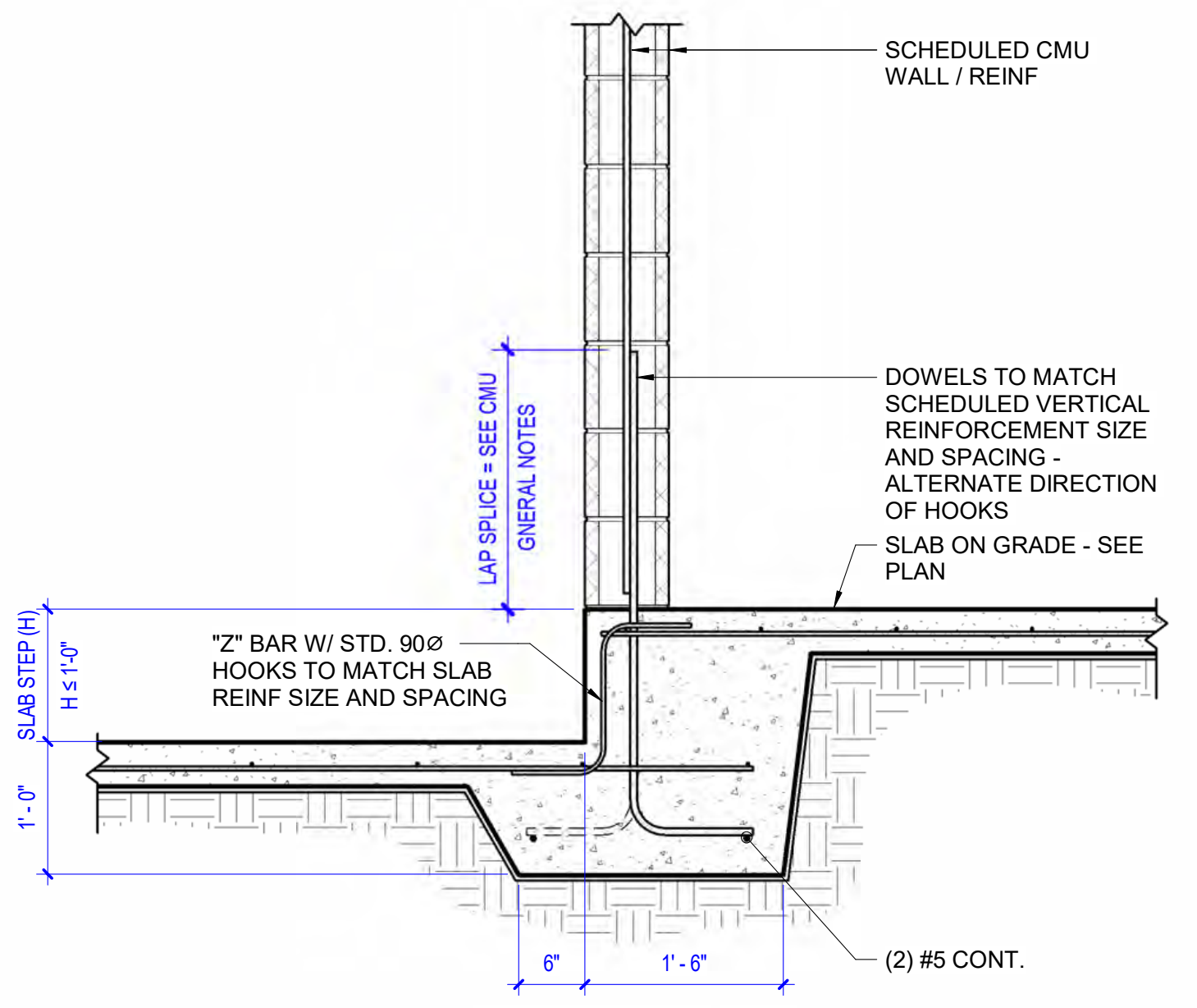
4 CMU Bearing at Grade - Vehicle Barrier
3/4" = 1'-0"


CMU WALL - SEE SCHEDULE FOR REINFORCEMENT
CMU LINTEL - SEE SCHEDULE
OPENING
CONT. BOND BEAM W/ (2) #5 CONT.

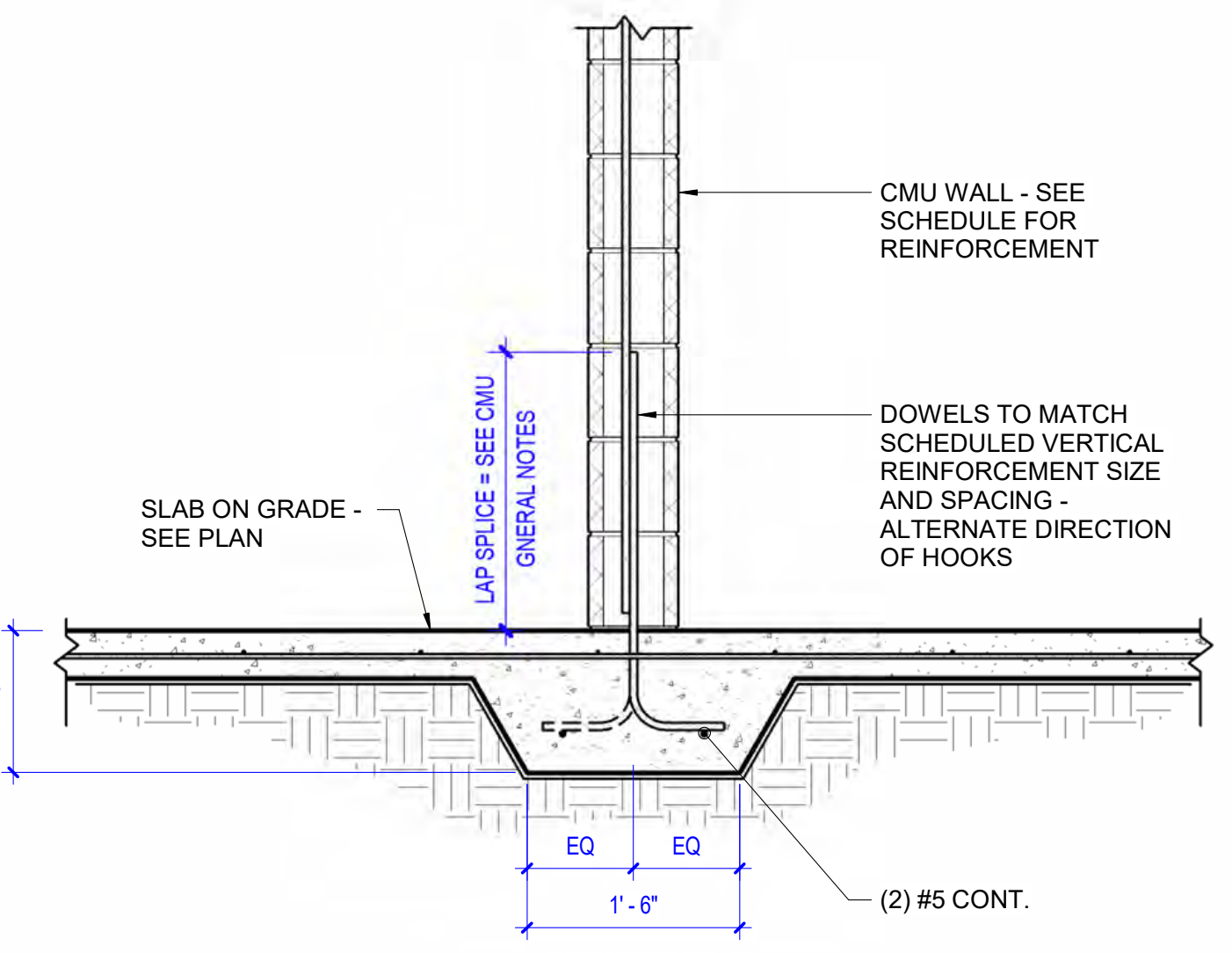
3 Opening in CMU Wall
3/4" = 1'-0"


CMU WALL - SEE SCHEDULE FOR REINFORCEMENT
LAP SPLICE = SEE CMU GNERAL NOTES
DOWELS TO MATCH SCHEDULED VERTICAL REINFORCEMENT SIZE AND SPACING - ALTERNATE DIRECTION OF HOOKS
SLAB ON GRADE - SEE PLAN
SLAB STEP (H) > 1'-0" H ≤ 2'-6"
#4 AT 12" O.C. HORIZ (2 BARS MIN)
"Z" BAR W/ STD. 90Ø HOOKS TO MATCH SLAB REINF SIZE AND SPACING
1'-0"
6"
1'-6"
(2) #5 CONT.

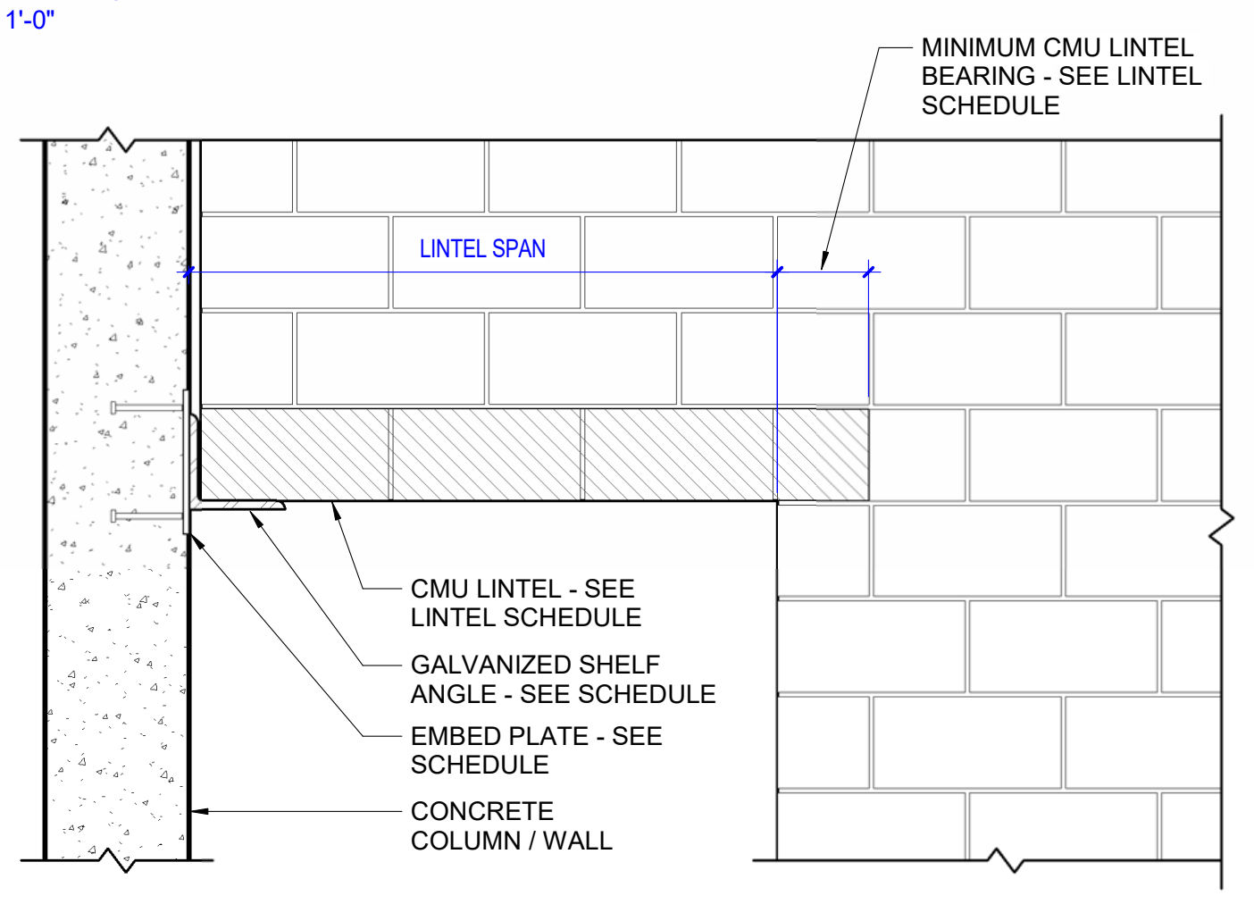
SLAB STEP (> 1'-0" H ≤ 2'-6")


SCHEDULED CMU WALL / REINF
LAP SPLICE = SEE CMU GNERAL NOTES
DOWELS TO MATCH SCHEDULED VERTICAL REINFORCEMENT SIZE AND SPACING - ALTERNATE DIRECTION OF HOOKS
SLAB ON GRADE - SEE PLAN
SLAB STEP (H) H ≤ 1'-0"
"Z" BAR W/ STD. 90Ø HOOKS TO MATCH SLAB REINF SIZE AND SPACING
1'-0"
6"
1'-6"
(2) #5 CONT.

SLAB STEP (H ≤ 1'-0")


CMU WALL - SEE SCHEDULE FOR REINFORCEMENT
LAP SPLICE = SEE CMU GNERAL NOTES
DOWELS TO MATCH SCHEDULED VERTICAL REINFORCEMENT SIZE AND SPACING - ALTERNATE DIRECTION OF HOOKS
SLAB ON GRADE - SEE PLAN
1'-0"
EQ
EQ
1'-6"
(2) #5 CONT.

2 CMU Bearing at Grade
3/4" = 1'-0"


MINIMUM CMU LINTEL BEARING - SEE LINTEL SCHEDULE
LINTEL SPAN
CMU LINTEL - SEE LINTEL SCHEDULE
GALVANIZED SHELF ANGLE - SEE SCHEDULE
EMBED PLATE - SEE SCHEDULE
CONCRETE COLUMN / WALL

SHELF ANGLE SCHEDULE

CMU SIZE	LINTEL SPAN		
	LESS THAN 5'-0"	5'-0" TO 7'-11"	8'-0" TO 12'-0"
6" CMU	L6X6X3/8 X 0'-4"	L6X6X1/2 X 0'-4"	
	EMBED PL 1/2X8X0'-8" W/ (4) 1/2" DIA. X 4" H.S.A	EMBED PL 1/2X8X0'-8" W/ (4) 1/2" DIA. X 4" H.S.A	
8" CMU		L8X8X3/4 X 0'-6"	
10" CMU			
12" CMU			

1 CMU Lintel at CIP Column / Wall
3/4" = 1'-0"

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SHEET NUMBER:
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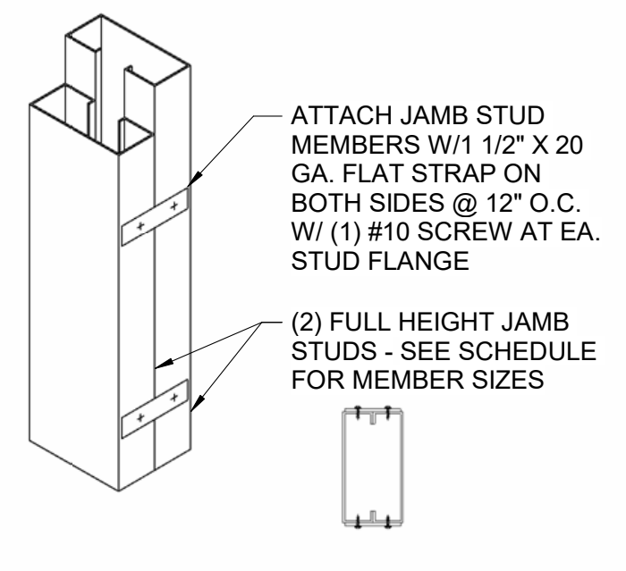
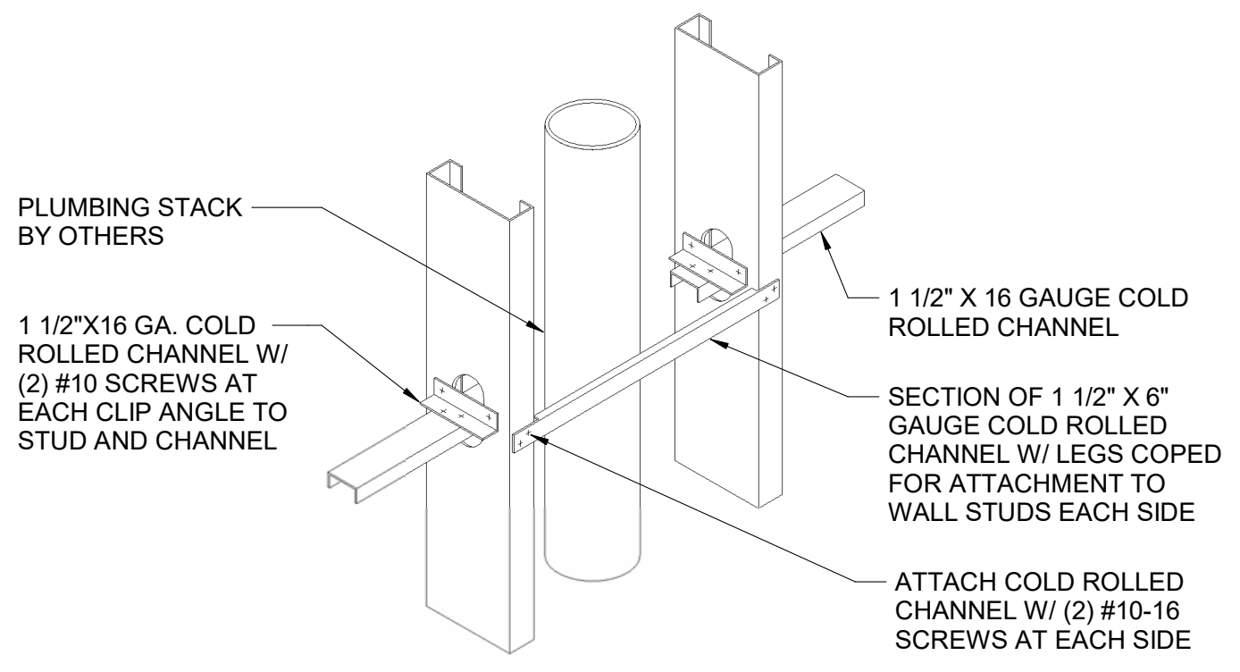

ATTACH JAMB STUD MEMBERS W/1 1/2" X 20 GA. FLAT STRAP ON BOTH SIDES @ 12" O.C. W/ (1) #10 SCREW AT EA. STUD FLANGE
(2) FULL HEIGHT JAMB STUDS - SEE SCHEDULE FOR MEMBER SIZES

(2) MEMBER STUD PACK (SCREWED)

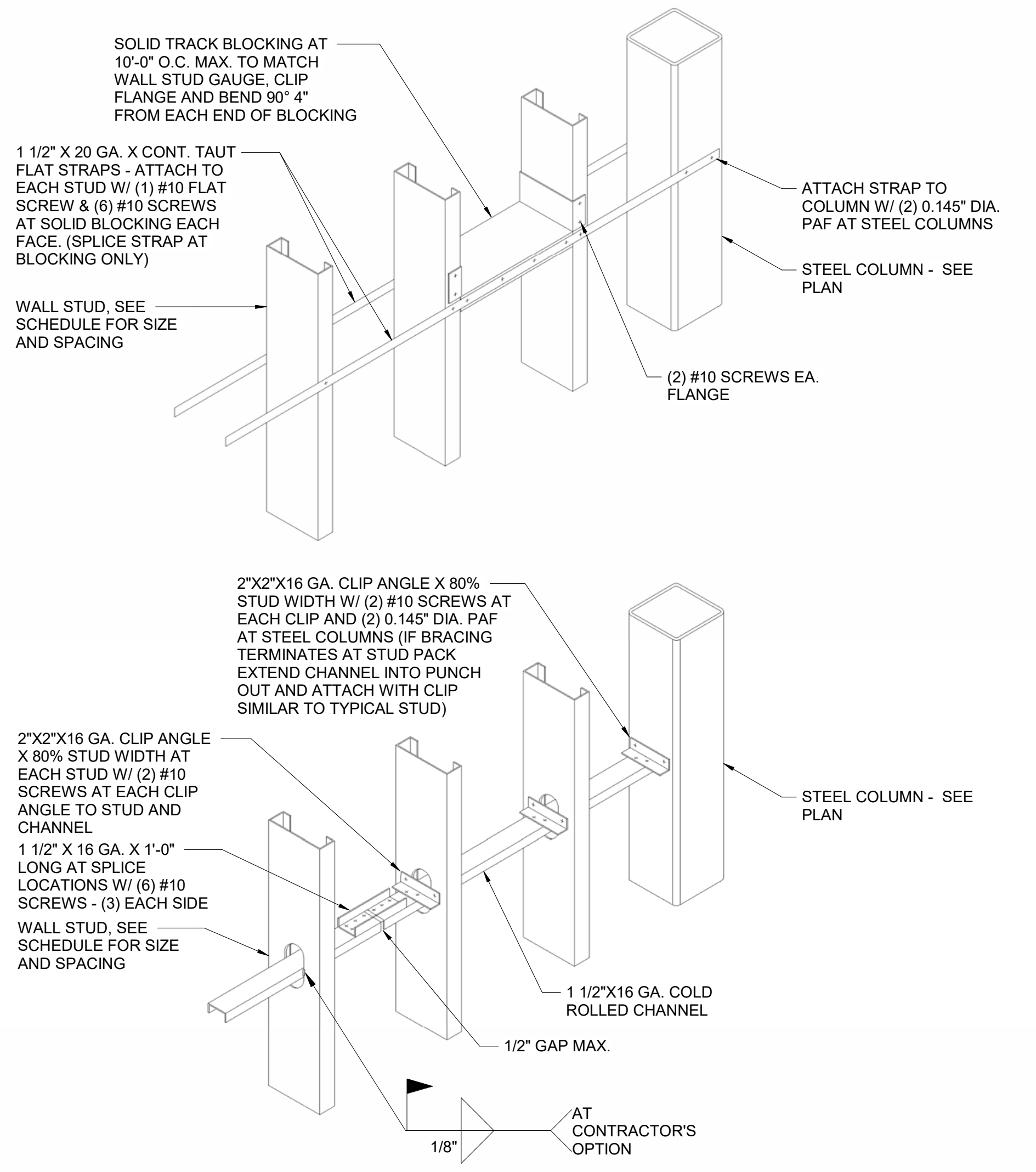
5 Jamb Attachments

3/4" = 1'-0"


PLUMBING STACK BY OTHERS
1 1/2"X16 GA. COLD ROLLED CHANNEL W/ (2) #10 SCREWS AT EACH CLIP ANGLE TO STUD AND CHANNEL
1 1/2" X 16 GAUGE COLD ROLLED CHANNEL
SECTION OF 1 1/2" X 6" GAUGE COLD ROLLED CHANNEL W/ LEGS COPED FOR ATTACHMENT TO WALL STUDS EACH SIDE
ATTACH COLD ROLLED CHANNEL W/ (2) #10-16 SCREWS AT EACH SIDE

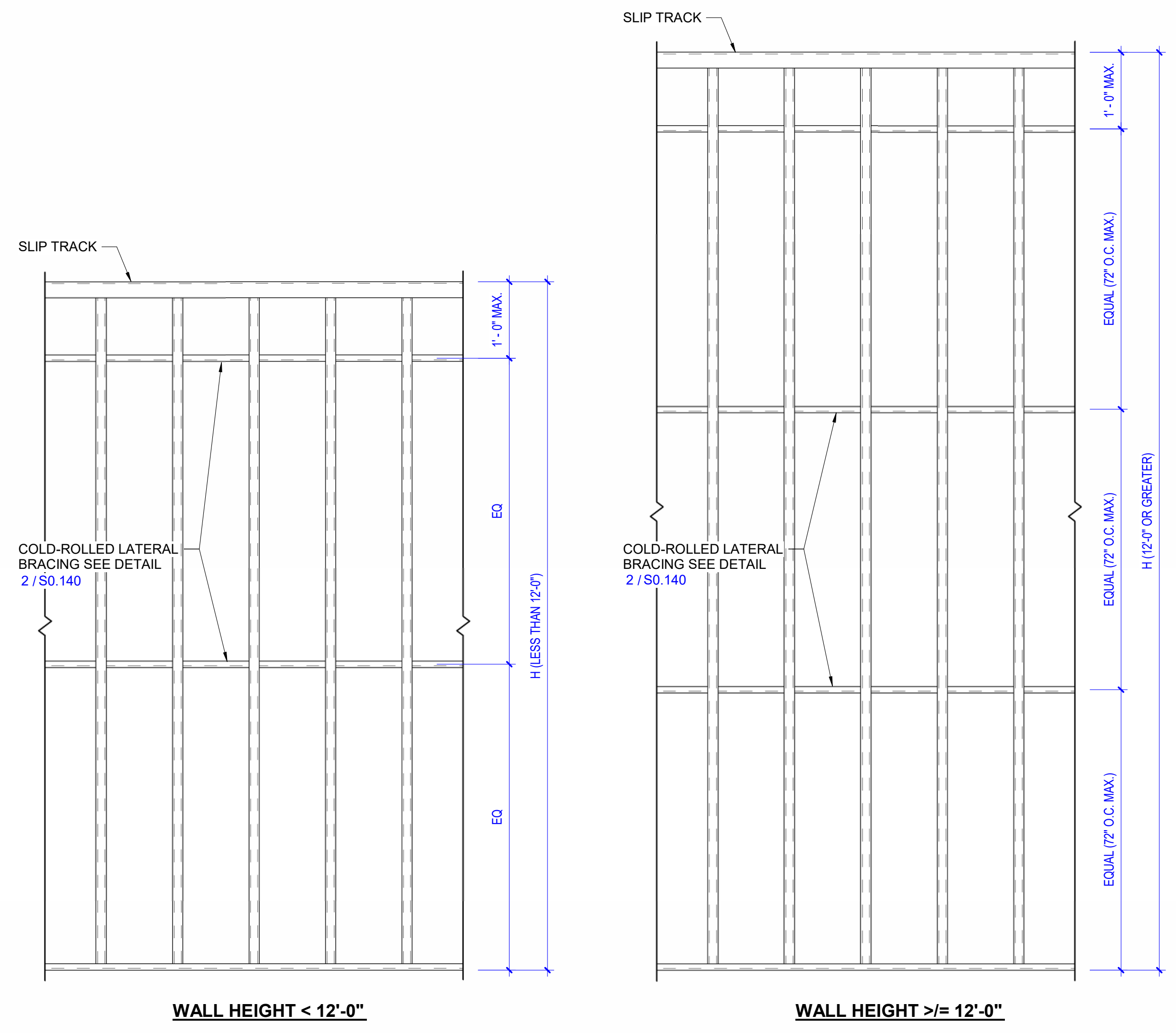
4 Bridging Attachment at Pipe Penetration

3/4" = 1'-0"


SOLID TRACK BLOCKING AT 10'-0" O.C. MAX. TO MATCH WALL STUD GAUGE, CLIP FLANGE AND BEND 90° 4" FROM EACH END OF BLOCKING
1 1/2" X 20 GA. X CONT. TAUT FLAT STRAPS - ATTACH TO EACH STUD W/ (1) #10 FLAT SCREW & (6) #10 SCREWS AT SOLID BLOCKING EACH FACE. (SPLICE STRAP AT BLOCKING ONLY)
WALL STUD, SEE SCHEDULE FOR SIZE AND SPACING
ATTACH STRAP TO COLUMN W/ (2) 0.145" DIA. PAF AT STEEL COLUMNS
STEEL COLUMN - SEE PLAN
(2) #10 SCREWS EA. FLANGE
2"X2"X16 GA. CLIP ANGLE X 80% STUD WIDTH W/ (2) #10 SCREWS AT EACH CLIP AND (2) 0.145" DIA. PAF AT STEEL COLUMNS (IF BRACING TERMINATES AT STUD PACK EXTEND CHANNEL INTO PUNCH OUT AND ATTACH WITH CLIP SIMILAR TO TYPICAL STUD)
2"X2"X16 GA. CLIP ANGLE X 80% STUD WIDTH AT EACH STUD W/ (2) #10 SCREWS AT EACH CLIP ANGLE TO STUD AND CHANNEL
1 1/2" X 16 GA. X 1'-0" LONG AT SPLICE LOCATIONS W/ (6) #10 SCREWS - (3) EACH SIDE
WALL STUD, SEE SCHEDULE FOR SIZE AND SPACING
STEEL COLUMN - SEE PLAN
1 1/2"X16 GA. COLD ROLLED CHANNEL
1/2" GAP MAX.
1/8"
AT CONTRACTOR'S OPTION

2 Cold-Rolled Lateral Bracing Options

3/4" = 1'-0"

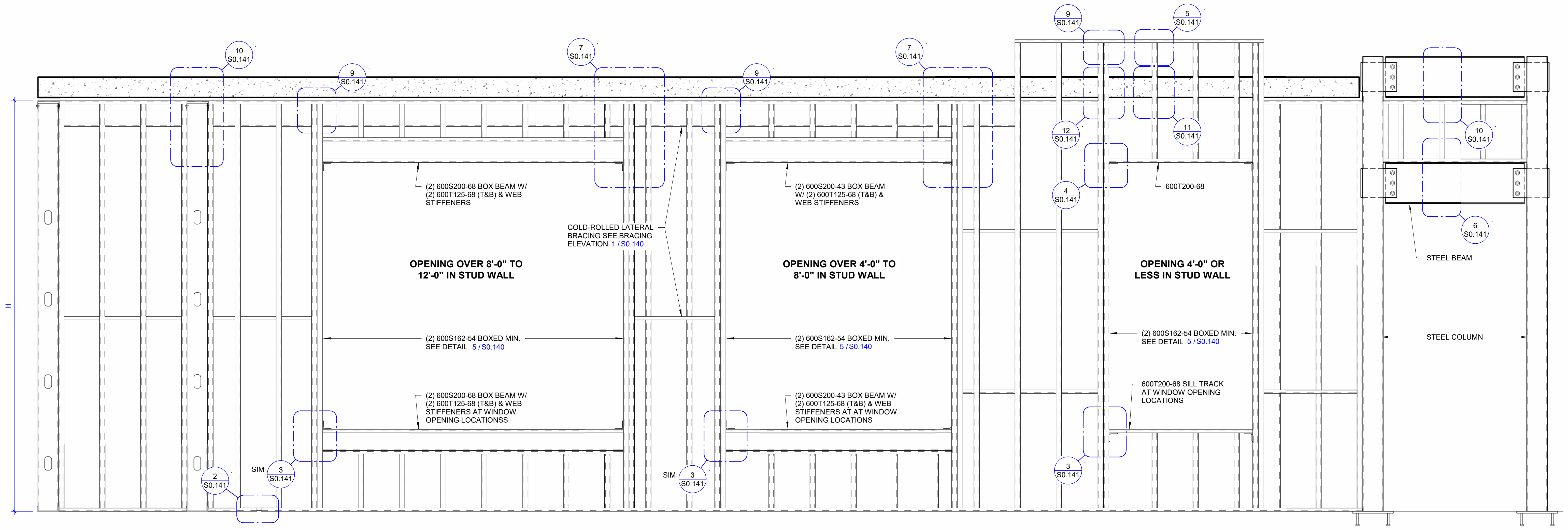


WALL HEIGHT < 12'-0"

WALL HEIGHT >/= 12'-0"

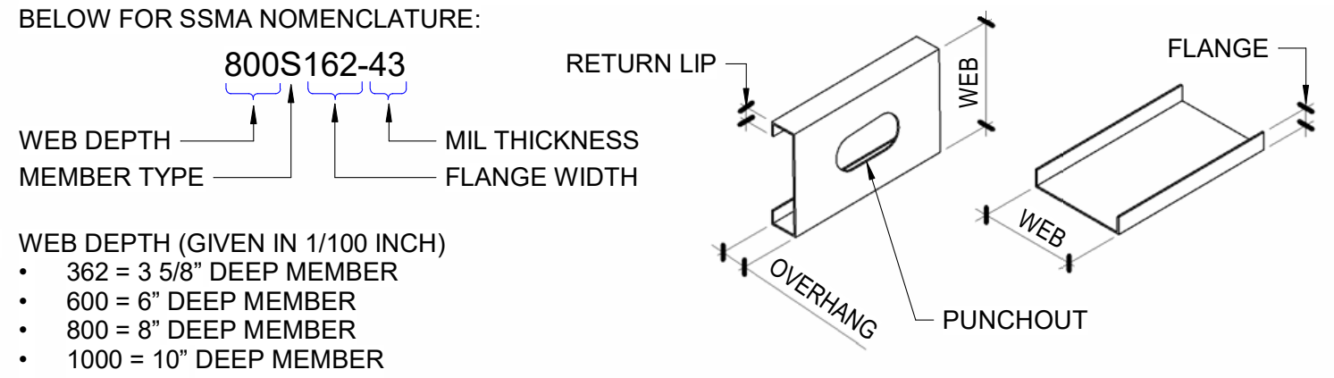
1 Lateral Bracing Locations By Wall Height - Non-Load Bearing

3/4" = 1'-0"


10 S0.141
9 S0.141
7 S0.141
9 S0.141
7 S0.141
9 S0.141
5 S0.141
12 S0.141
11 S0.141
4 S0.141
10 S0.141
6 S0.141
H
(2) 600S200-68 BOX BEAM W/ (2) 600T125-68 (T&B) & WEB STIFFENERS
COLD-ROLLED LATERAL BRACING SEE BRACING ELEVATION 1 / S0.140
OPENING OVER 8'-0" TO 12'-0" IN STUD WALL
(2) 600S162-54 BOXED MIN. SEE DETAIL 5 / S0.140
(2) 600S200-68 BOX BEAM W/ (2) 600T125-68 (T&B) & WEB STIFFENERS AT WINDOW OPENING LOCATIONSS
(2) 600S200-43 BOX BEAM W/ (2) 600T125-68 (T&B) & WEB STIFFENERS
OPENING OVER 4'-0" TO 8'-0" IN STUD WALL
(2) 600S162-54 BOXED MIN. SEE DETAIL 5 / S0.140
(2) 600S200-43 BOX BEAM W/ (2) 600T125-68 (T&B) & WEB STIFFENERS AT AT WINDOW OPENING LOCATIONS
600T200-68
OPENING 4'-0" OR LESS IN STUD WALL
(2) 600S162-54 BOXED MIN. SEE DETAIL 5 / S0.140
600T200-68 SILL TRACK AT WINDOW OPENING LOCATIONS
STEEL BEAM
STEEL COLUMN
2 S0.141
SIM 3 S0.141
SIM 3 S0.141
3 S0.141

LG1.0 COLD FORM METAL FRAMING:

1. PRODUCT IDENTIFICATION:
   A. THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) STANDARD PRODUCT TECHNICAL INFORMATION HAS BEEN USED FOR SELECTION OF MEMBERS THROUGH OUT THE STRUCTURAL DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE SELECTED MANUFACTURER'S PRODUCTS MEET OR EXCEED ALL MINIMUM GEOMETRIES AS OUTLINED BY THE SSMA STANDARDS. SEE BELOW FOR SSMA NOMENCLATURE:



   A. WEB DEPTH (GIVEN IN 1/100 INCH)
      - 362 = 3 5/8" DEEP MEMBER
      - 600 = 6" DEEP MEMBER
      - 800 = 8" DEEP MEMBER
      - 1000 = 10" DEEP MEMBER
      - 1200 = 12" DEEP MEMBER
      - 1400 = 14" DEEP MEMBER
   B. MEMBER TYPE
      - S = STUD OR JOIST SECTION
      - T = TRACK SECTION
      - U = CHANNEL SECTIONS
   C. FLANGE WIDTH (GIVEN IN 1/100 INCH)
      - 125 = 1 1/4" WIDE
      - 162 = 1 5/8" WIDE
      - 200 = 2" WIDE
      - 250 = 2 1/2" WIDE
   D. MIL THICKNESS (BASE STEEL THICKNESS, 1 MIL = 1/1000 INCH)
      - 33 MIL (20 GA.)
      - 43 MIL (18 GA.)
      - 54 MIL (16 GA.)
      - 68 MIL (14 GA.)
      - 97 MIL (12 GA.)
2. ALL FRAMING MEMBERS SHALL BE FORMED FROM GALVANIZED STEEL (G60 COATING MINIMUM) CONFORMING TO ASTM C955:
   - 10, 12, 14, AND 16 GA MEMBERS – FY = 50 KSI
   - 18 AND 20 GA MEMBERS, FLAT STRAP BRACING – FY = 33 KSI
3. ALL COLD FORM MEMBERS AND ACCESSORIES SHALL CONFORM TO MINIMUM DESIGN THICKNESSES AND SECTION GEOMETRIES AS OUTLINED BY SSMA.
4. ALL STRUCUTURAL MEMBER AND CONNECTION DESIGN SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
5. DIMENSIONS SHOWN ARE FOR REFERENCE ONLY. ACTUAL LOCATION AND EXACT MEMBER LENGTHS, JOINT PLACEMENT, RELATIONSHIP TO OTHER ELEMENTS, ETC. MUST BE COORDINATED FROM THE ARCHITECT DRAWINGS AND VERIFIED PRIOR TO INSTALLATION.
6. ANY DEVIATIONS IN THE ACTUAL FIELD CONDITIONS THAT CHANGE THE SPAN, SPACING, EDGE DISTANCES OR BEARING CONDITIONS MUST BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL.
7. PRECAUTIONS MUST BE TAKEN TO AVOID CONSTRUCTION MATERIAL LOADS OR CONSTRUCTION OPERATION LOADS WHICH EXCEED SPECIFIED DESIGN LIVE LOADS AS CONSTRUCTION LOADS HAVE NOT BEEN CONSIDERED IN DESIGN. CONTRACTOR IS RESPONSIBLE FOR PROVIDING ADEQUATE TEMPORARY BRACING UNTIL PERMANENT BRIDGING IS INSTALLED PROVIDING STABILITY TO BOTH FLANGES. ALL BRIDGING SHALL BE TERMINATED AT JAMBS, CORNER STUDS, OR COLUMNS. BRIDGING END **SHALL NOT** HANG LOOSE.
8. FRAMING MEMBERS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS, OR AS REQUIRED, FOR AN ANGULAR FIT AGAINST ABUTTING MEMBERS. MEMBERS TO BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.
9. ALL FIELD CUTS MUST BE BY SHEARING OR SAWING. TORCH CUTTING OF COLD FORMED MEMBERS IS PROHIBITED.
10. PREFABRICATED WALL OR FLOOR PANELS SHALL FASTENED AND SQAURE TO PREVENT RACKING. LIFTING AND HANDLING OF PANELS SHALL BE COMPLETED IN A MANNER WHICH DOES NOT CAUSE DISTORTION OF PANELS.
11. CONTRACTOR SHALL SUBMIT **SIGNED AND SEALED** DRAWINGS AND CALCULATIONS PREPARED BY FABRICATORS STRUCTURAL ENGINEER LICENSED IN PROJECT JURISDICTION FOR REVIEW AND APPROVAL BY EOR. STRUCTURAL DRAWINGS SHALL BE USED AS A MINIMUM BASIS OF DESIGN. IN ADDITION, CONTRACTOR SHALL SUBMIT CURRENT PRODUCT LITERATURE FROM THE SELECTED MANUFACTURER FOR APPROVAL PRIOR TO INSTALLATION.
12. CONNECTIONS:
    A. ALL FASTENING OF COLD FORM MEMBERS SHALL BE WITH SELF-DRILLING SCREWS OR WELD. ALL WELDS SHALL BE PERFORMED BY CERTIFIED WELDERS IN STRICT ACCORDANCE WITH PROVISIONS OF AWS D1.3 SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURES, LATEST EDITION.
    B. ALL FASTENERS FOR CONNECTIONS MUST MEET OR EXCEED THE STRUCTURAL PROPERTIES OUTLINED IN THESE DRAWINGS AND BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
    C. FOR SCREWS AND PAF'S: 3/4" MINIMUM CLEARANCE MUST BE MAINTAINED FROM ALL EDGES OF STEEL MEMBERS AND 3/4" MINIMUM SPACING BETWEEN ADJACENT SCREWS OR PAF'S.
    D. SCREW PENETRATION THROUGH JOINTED MATERIALS SHALL NOT BE LESS THAN THREE EXPOSED THREADS. SELECT SCREWS WITH AN ADEQUATE CUTTING TIP TO ACCOMMODATE THE TOTAL THICKNESS TO BE DRILLED.
    E. MINIMUM WELD THROAT THICKNESS (T) MUST MATCH OR EXCEED THE BASE STEEL THICKNESS OF THE THINNEST CONNECTED PART (1/8" MINIMUM).
    F. WHEN WELDING MEMBERS, THE ZINC COATING WILL BE BURNED AWAY. TOUCH UP ALL WELDS WITH ZINC-RICH PAINT ON ALL SIDES (ASTM A780) FOLLOWING MANUFACTURER'S RECOMMENDATIONS.
    G. FRAMING DESIGN ASSUMES ALL CLADDING IS UNIFORMLY ATTACHED TO EACH FRAMING MEMBER RESULTING IN A UNIFORM DISTRIBUTION OF LOAD TO THE FRAMING MEMBER.
    H. PROVIDE A MINIMUM OF (3) #12 SCREWS FOR ALL STUD-TO-STUD CONNECTIONS.
    I. SECURE TRACKS AS REQUIRED IN THE DRAWINGS TO THE SUPPORTING STRUCTURE. PROVIDE FASTENERS AT CORNERS AND AT ENDS OF TRACKS. ABUTTING LENGTHS OF TRACK SHALL BE SECURED BY ANCHOR TO A COMMON STRUCTURAL MEMBER OR SPLICED PER DETAILS.
13. HEADERS:
    A. SPLICING OF HEADERS IS NOT PERMITTED.
    B. MEMBERS SHALL CONSIST OF UNPUNCHED SECTIONS UNLESS APPROVED BY EOR.
14. STUDS AND JOISTS:
    A. STUDS SHALL NOT DEVIATE FROM PLUMB, LEVEL, AND TRUE BY MORE THAN 1/8" IN 10'-0". ATTACHMENT OF BOTH FLANGES TO TOP AND BOTTOM TRACK IS CRITICAL.
    B. CONTACTOR SHALL VERIFY THAT STUD MEETS MINIMUM GAUGE REQUIRED FOR ALL EXTERIOR FINISHES (BRICKS, STUCCO, EFIS, ETC.). WEIGHT OF ANY MASONRY VENEER IS ASSUMED TO BE SUPPORTED INDEPENDENT OF THE COLD FORM METAL FRAMING U.N.O. IN DOCUMENTS.
    C. SPLICING OF WALL STUDS OR JOISTS IS NOT PERMITTED WITHOUT PRIOR ENGINEERING REVIEW AND APPROVAL. SPECIFIC DETAILS FOR SUCH SPLICES CAN BE PROVIDED ON A CASE-BY-CASE BASIS.

A. NO NOTCHING OR COPING OF STUDS, JOISTS, OR ANY OTHER LOAD CARRYING MEMBER IS ALLOWED UNLESS STATED WITHIN STRUCTURAL DRAWINGS. IF ADDITIONAL HOLES ARE REQUIRED IN A MEMBER, CONTACT EOR PRIOR TO CUTTING HOLES.
B. NO NOTCHING OR COPING OF STUDS, JOISTS, OR ANY OTHER LOAD CARRYING MEMBER IS ALLOWED UNLESS STATED WITHIN STRUCTURAL DRAWINGS. IF ADDITIONAL HOLES ARE REQUIRED IN A MEMBER, CONTACT EOR PRIOR TO CUTTING HOLES.FRAMING CONTRACTOR TO ENSURE PUNCH OUT ALIGNMENT IN STUDS WHEN ASSEMBLING LATERAL BRACING.
C. THE MAXIMUM ALLOWABLE GAP FOR A STUD SEATED IN A TRACK IN A NON-AXIAL LOAD BEARING CONDITION IS 1/8" U.N.O. IT IS RECOMMENDED THAT THIS GAP BE ELIMINATED IN AN AXIAL LOAD BEARING CONDITION BY PRE-LOADING THE STUDS UNTIL FULLY SEATED IN THE TRACK. FAILURE TO DO SO COULD RESULT IN LONGTERM SERVICIBILITY ISSUES OF THE STRUCTURE.
D. AXIAL LOADED STUDS SHALL BEAR FULLY ON STRUCTURE. NON-AXIAL LOAD BEARING STUDS MAY OVERHANGING SUPPORTING STRUCTURE AS NOTED IN DRAWINGS.
E. A MINIMUM OF THREE STUDS SHALL BE USED AT THE CORNER AND INTERSECTION OF ALL WALLS.
F. STUDS FROM FLOOR ABOVE MUST BEAR DIRECTLY OVER JOISTS OR A STRUCTURAL MEMBER OF ADEQUATE STRENGTH AND STIFFNESS SHALL BE PROVIDED ON TOP OF JOIST FOR PROPER BEARING AND LOAD DISTRIBUTION.
G. JOISTS/RAFTER MEMBERS SHALL BEAR DIRECTLY OVER STUDS BELOW OR A STRUCTURAL MEMBER OF ADEQUATE STRENGTH AND STIFFNESS SHALL BE PROVIDED FOR DISTRIBUTION OF JOIST/RAFTER LOADS TO STUDS.
H. BRIDGING SHALL BE PLACED IMMEDIATELY AFTER INSTALLATION OF JOIST/RAFTERS AND PRIOR TO CONSTRCUTION LOAD APPLICATION TO ENSURE FLANGE ROTATION IS RESISTED AND COMPRESSION FLANGES ARE LATERALLY SUPPORTED.
   - BRIDGING SHALL BE INSTALLED AT MID SPAN FOR SPANS 16'-0" OR LESS AND AT 8'-0" MAX FOR SPANS OF GREATER LENGTH. INSTALL BLOCKING BETWEEN OUTER TWO BAYS OF JOISTS, OVER ALL INTERIOR SUPPORTS, AND ADJACENT TO OPENINGS AT 10'-0" O.C. MAX.
   - BRIDGING SHALL CONSIST OF SOLID BLOCKING (TRACK OR JOIST SECTION) PLUS STRAP BRACING OR 1 ½" COLD-ROLLED CHANNELS SCREWED OR WELDED TO BOTTOM FLANGE.

METAL STUD SCHEDULE
EXTERIOR NON-LOAD BEARING

HEIGHT	STUD	TRACK
0' TO 10'-0"	600S200-43 @ 16" O.C.	600T125-54
10'-1" TO 18'-0"	800S200-54 @ 16" O.C.	800T125-54
18'-1" TO 20'-0"	800S200-68 @ 16" O.C.	800T125-68
20'-1" TO 22'-0"	(2) 800S200-54 @ 16" O.C.	800T125-68

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EAST BATON ROUGE PARISH

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SHEET TITLE:
Typical Metal Stud Framing General Notes / Details

SHEET NUMBER:
S0.140

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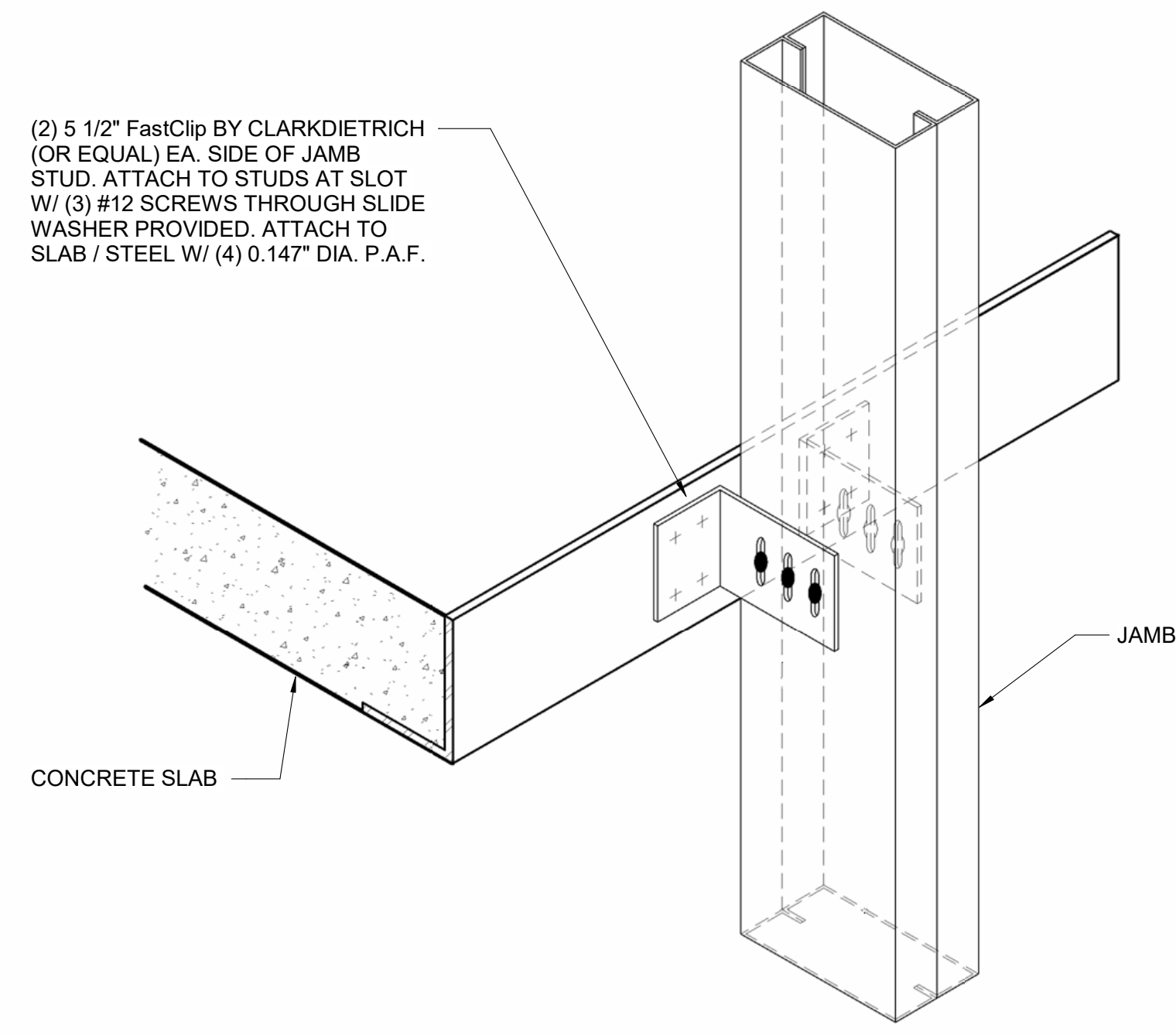
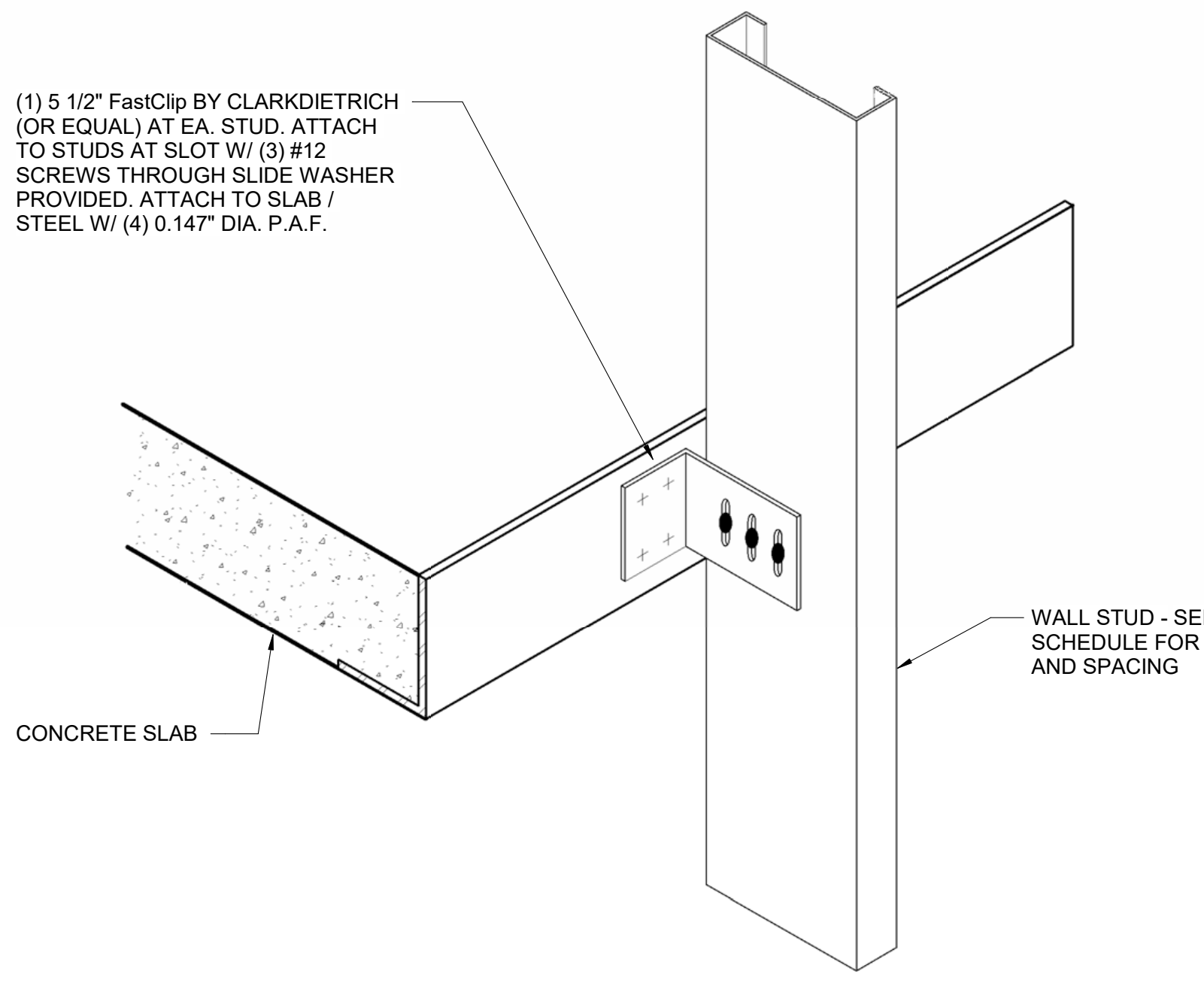
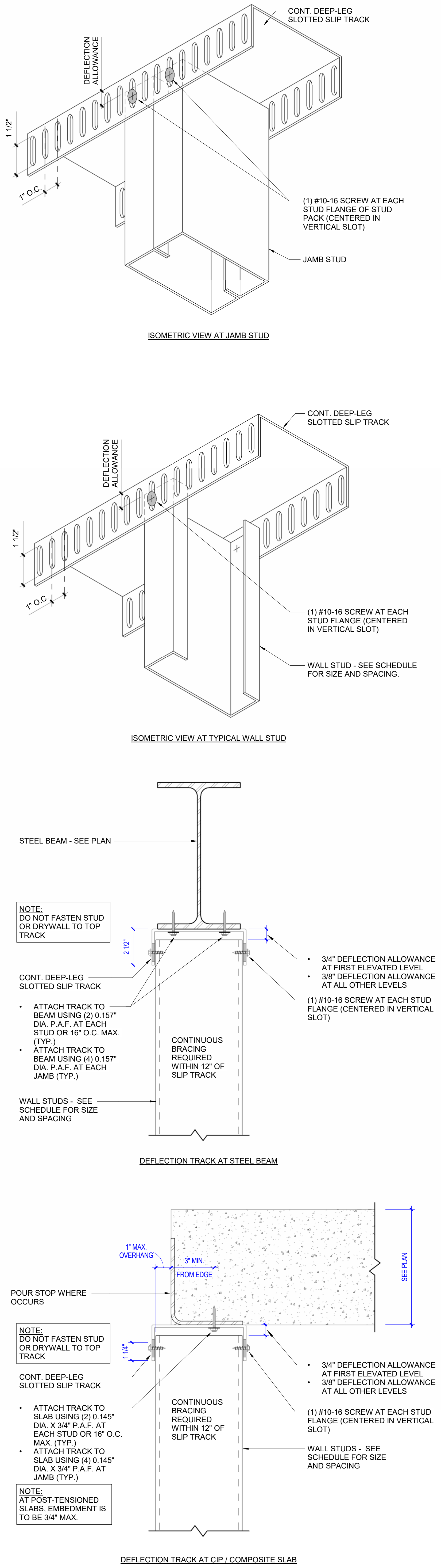

(2) 5 1/2" FastClip BY CLARKDIETRICH (OR EQUAL) EA. SIDE OF JAMB STUD. ATTACH TO STUDS AT SLOT W/ (3) #12 SCREWS THROUGH SLIDE WASHER PROVIDED. ATTACH TO SLAB / STEEL W/ (4) 0.147" DIA. P.A.F.
JAMB STUDS
CONCRETE SLAB

12 Jamb Stud Deflection at Bypass Slab
1 1/2" = 1'-0"


(1) 5 1/2" FastClip BY CLARKDIETRICH (OR EQUAL) AT EA. STUD. ATTACH TO STUDS AT SLOT W/ (3) #12 SCREWS THROUGH SLIDE WASHER PROVIDED. ATTACH TO SLAB / STEEL W/ (4) 0.147" DIA. P.A.F.
WALL STUD - SEE SCHEDULE FOR SIZE AND SPACING
CONCRETE SLAB

11 Stud Deflection at Bypass Slab
1 1/2" = 1'-0"


CONT. DEEP-LEG SLOTTED SLIP TRACK
DEFLECTION ALLOWANCE
1 1/2"
1" O.C.
(1) #10-16 SCREW AT EACH STUD FLANGE OF STUD PACK (CENTERED IN VERTICAL SLOT)
JAMB STUD
CONT. DEEP-LEG SLOTTED SLIP TRACK
DEFLECTION ALLOWANCE
1 1/2"
1" O.C.
(1) #10-16 SCREW AT EACH STUD FLANGE (CENTERED IN VERTICAL SLOT)
WALL STUD - SEE SCHEDULE FOR SIZE AND SPACING.
STEEL BEAM - SEE PLAN
2 1/2"
CONT. DEEP-LEG SLOTTED SLIP TRACK
CONTINUOUS BRACING REQUIRED WITHIN 12" OF SLIP TRACK
WALL STUDS - SEE SCHEDULE FOR SIZE AND SPACING
1" MAX. OVERHANG
3" MIN. FROM EDGE
SEE PLAN
POUR STOP WHERE OCCURS
1 1/4"
CONT. DEEP-LEG SLOTTED SLIP TRACK
CONTINUOUS BRACING REQUIRED WITHIN 12" OF SLIP TRACK
WALL STUDS - SEE SCHEDULE FOR SIZE AND SPACING

ISOMETRIC VIEW AT JAMB STUD

ISOMETRIC VIEW AT TYPICAL WALL STUD

NOTE:
DO NOT FASTEN STUD OR DRYWALL TO TOP TRACK

- ATTACH TRACK TO BEAM USING (2) 0.157" DIA. P.A.F. AT EACH STUD OR 16" O.C. MAX. (TYP.)
- ATTACH TRACK TO BEAM USING (4) 0.157" DIA. P.A.F. AT EACH JAMB (TYP.)

- 3/4" DEFLECTION ALLOWANCE AT FIRST ELEVATED LEVEL
- 3/8" DEFLECTION ALLOWANCE AT ALL OTHER LEVELS

(1) #10-16 SCREW AT EACH STUD FLANGE (CENTERED IN VERTICAL SLOT)

DEFLECTION TRACK AT STEEL BEAM

NOTE:
DO NOT FASTEN STUD OR DRYWALL TO TOP TRACK

- ATTACH TRACK TO SLAB USING (2) 0.145" DIA. X 3/4" P.A.F. AT EACH STUD OR 16" O.C. MAX. (TYP.)
- ATTACH TRACK TO SLAB USING (4) 0.145" DIA. X 3/4" P.A.F. AT JAMB (TYP.)

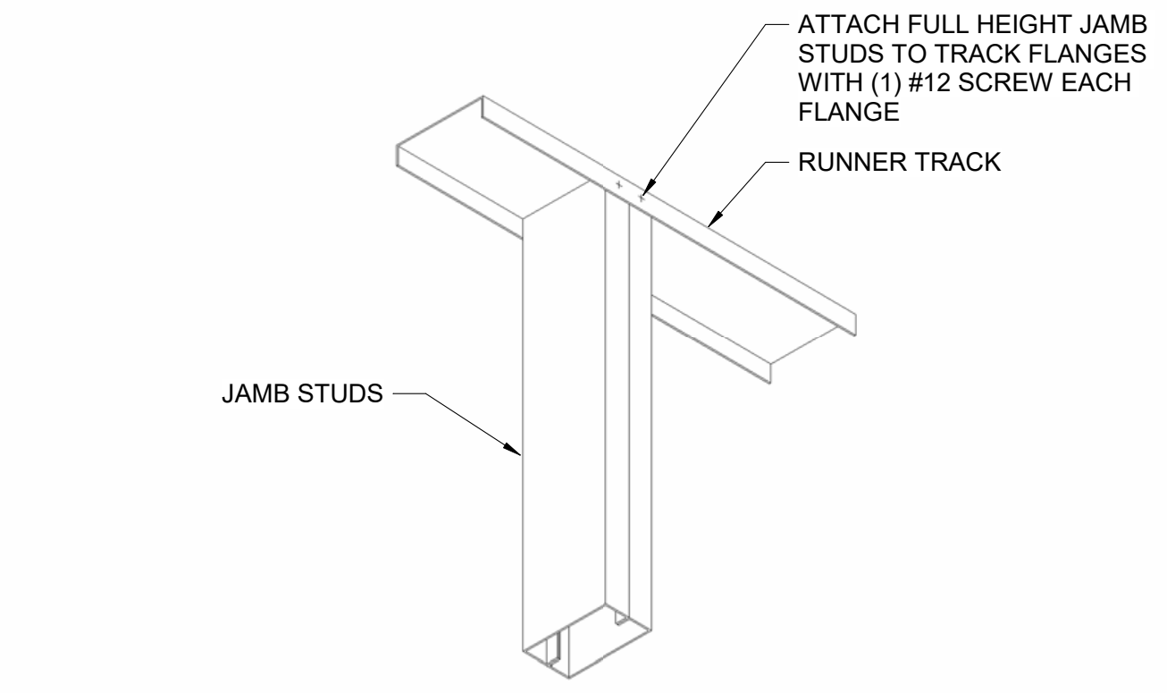
NOTE:
AT POST-TENSIONED SLABS, EMBEDMENT IS TO BE 3/4" MAX.

- 3/4" DEFLECTION ALLOWANCE AT FIRST ELEVATED LEVEL
- 3/8" DEFLECTION ALLOWANCE AT ALL OTHER LEVELS

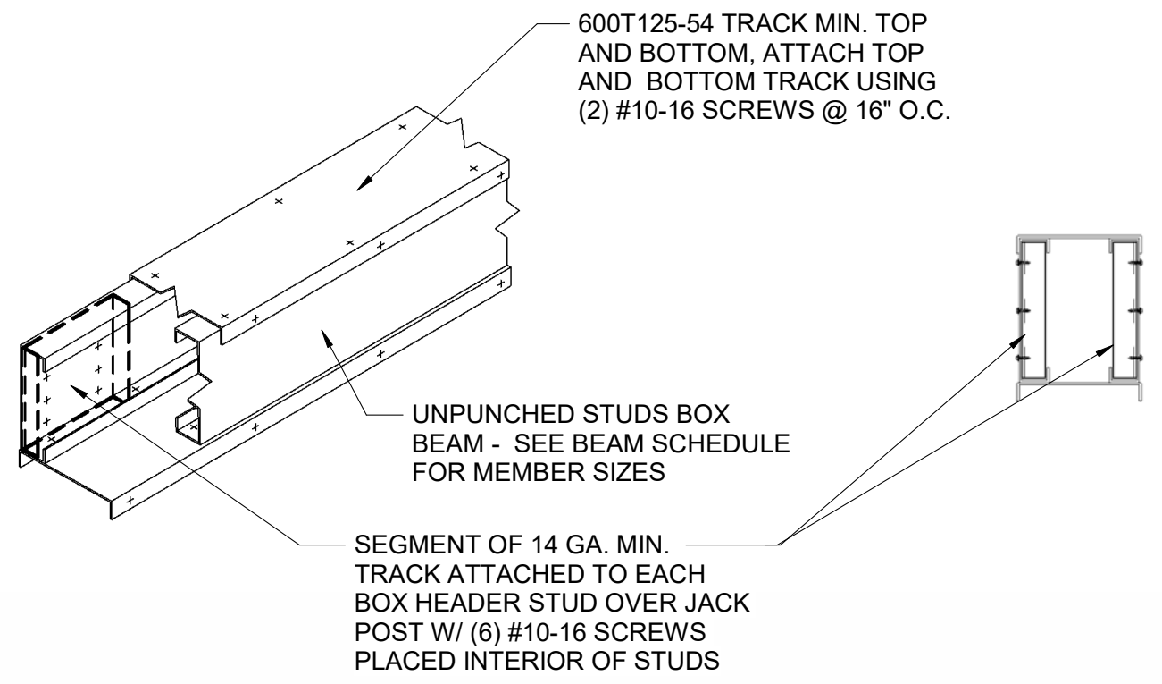
(1) #10-16 SCREW AT EACH STUD FLANGE (CENTERED IN VERTICAL SLOT)

DEFLECTION TRACK AT CIP / COMPOSITE SLAB

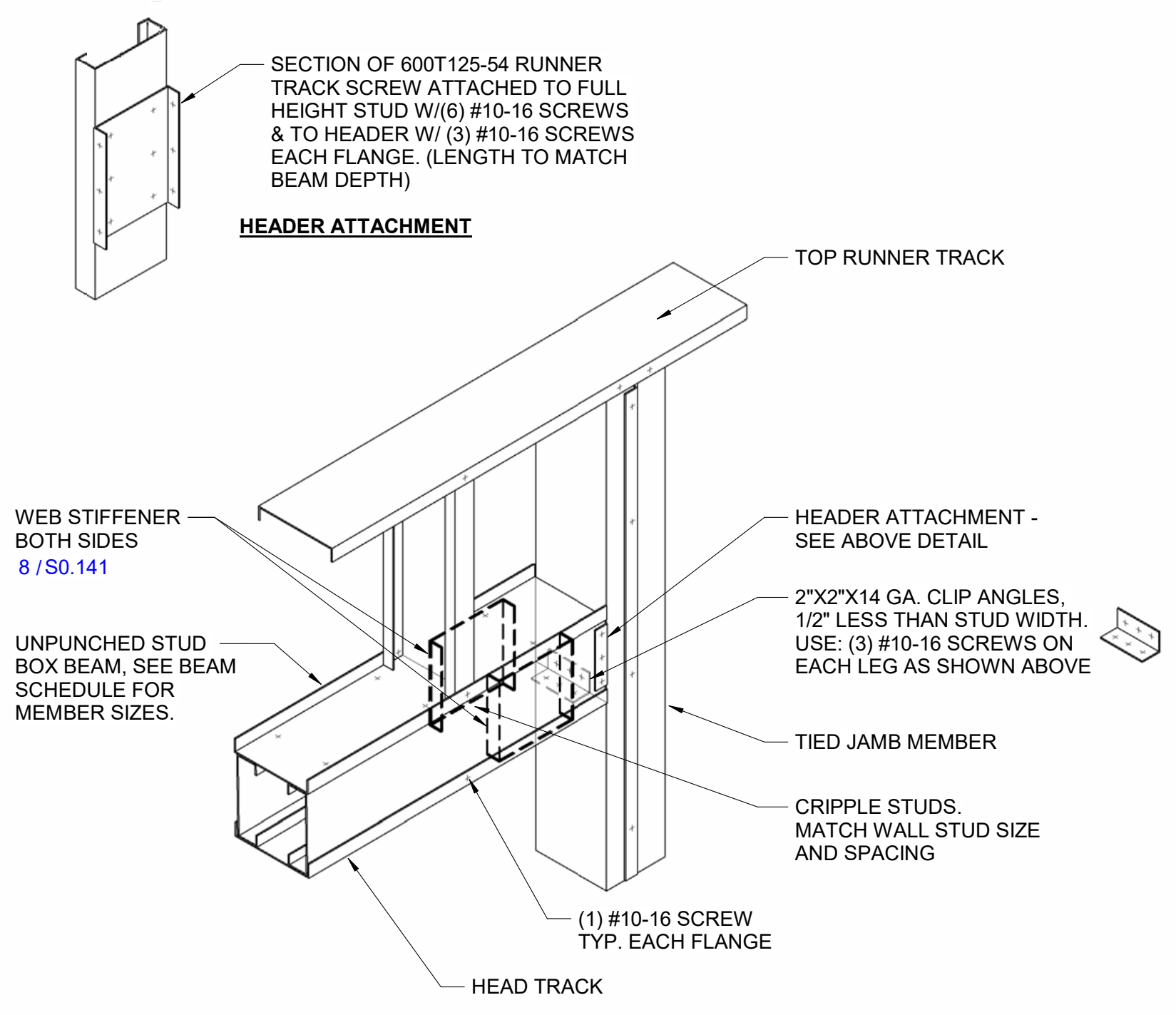
10 Deflection Track Attachment
3" = 1'-0"


ATTACH FULL HEIGHT JAMB STUDS TO TRACK FLANGES WITH (1) #12 SCREW EACH FLANGE
RUNNER TRACK
JAMB STUDS

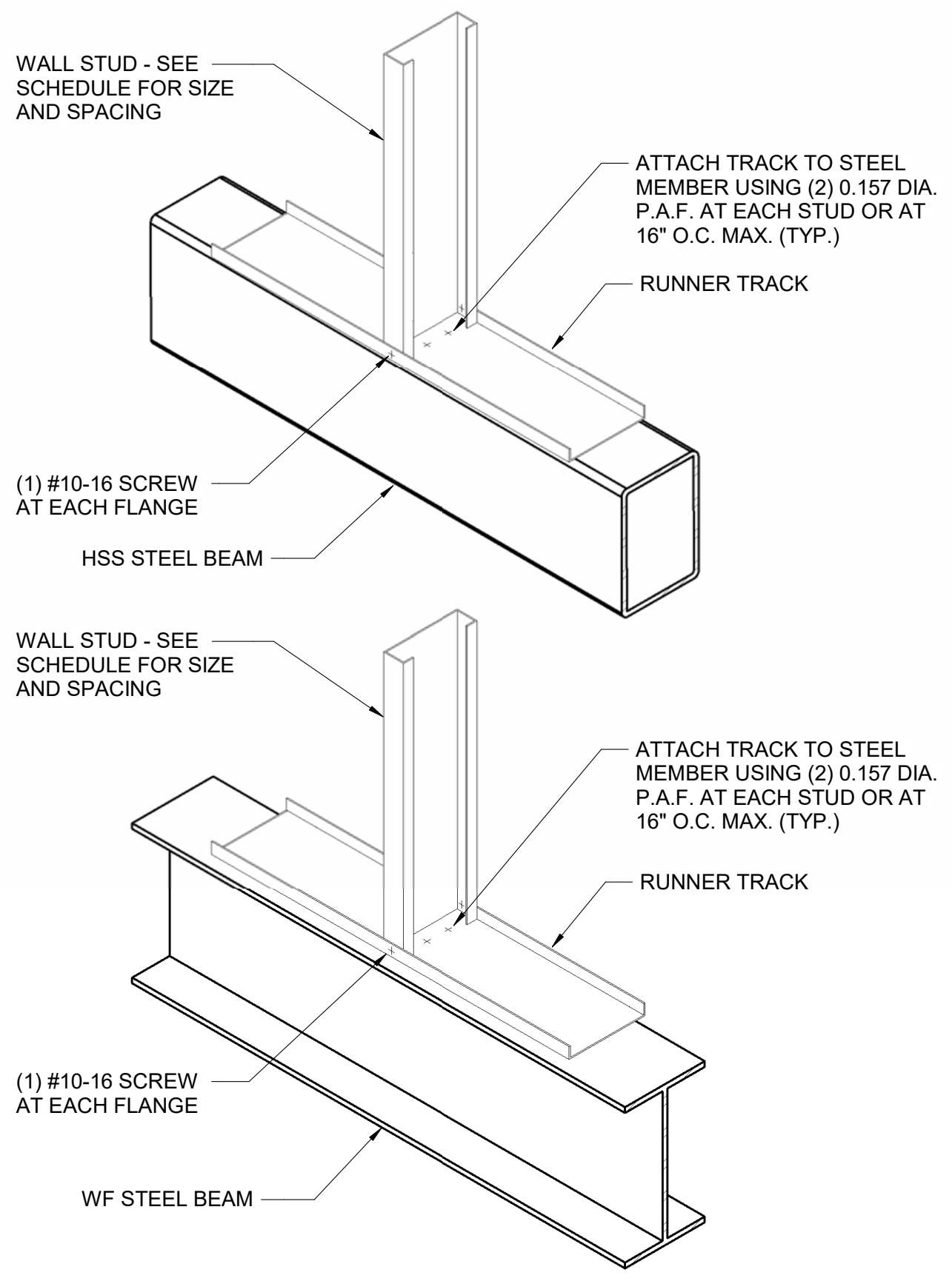
9 Jamb Stud to Top Track Attachment
3/4" = 1'-0"


600T125-54 TRACK MIN. TOP AND BOTTOM, ATTACH TOP AND BOTTOM TRACK USING (2) #10-16 SCREWS @ 16" O.C.
UNPUNCHED STUDS BOX BEAM - SEE BEAM SCHEDULE FOR MEMBER SIZES
SEGMENT OF 14 GA. MIN. TRACK ATTACHED TO EACH BOX HEADER STUD OVER JACK POST W/ (6) #10-16 SCREWS PLACED INTERIOR OF STUDS

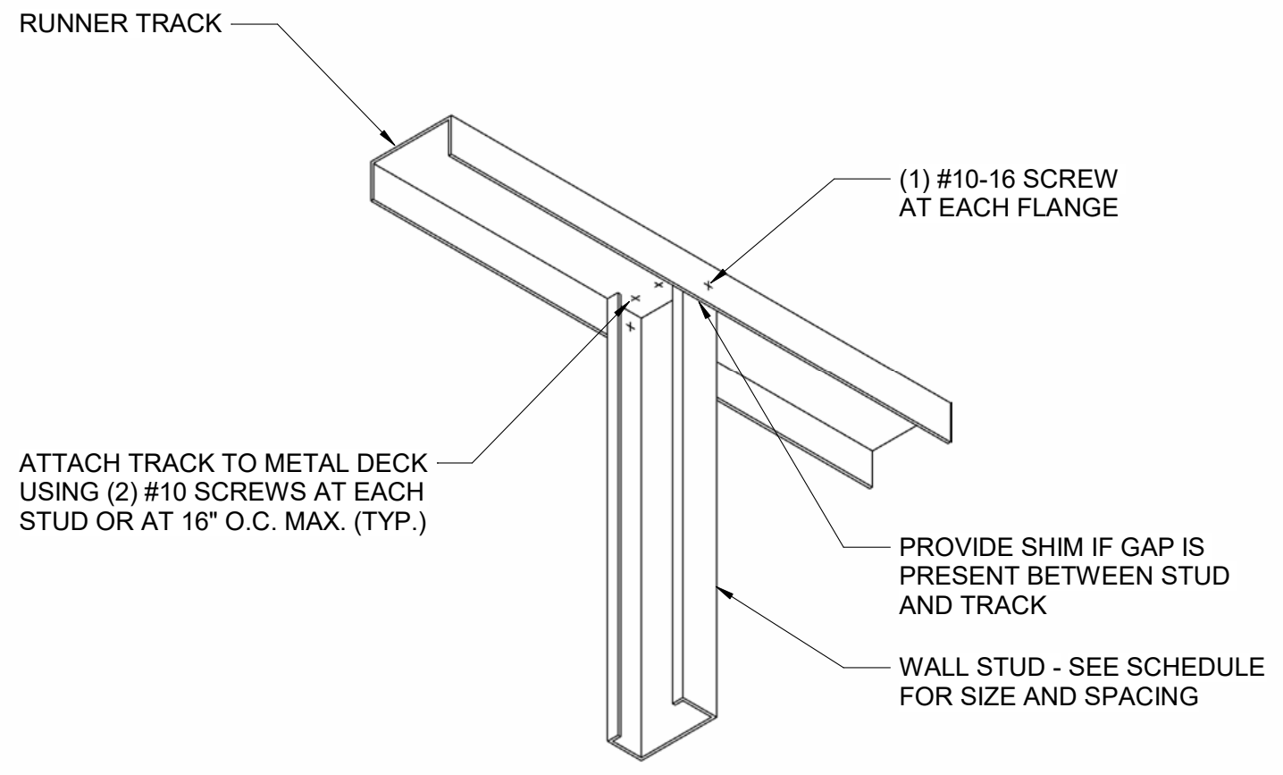
8 Web Stiffener Detail
3/4" = 1'-0"


SECTION OF 600T125-54 RUNNER TRACK SCREW ATTACHED TO FULL HEIGHT STUD W/(6) #10-16 SCREWS & TO HEADER W/ (3) #10-16 SCREWS EACH FLANGE. (LENGTH TO MATCH BEAM DEPTH)
HEADER ATTACHMENT
TOP RUNNER TRACK
WEB STIFFENER BOTH SIDES
8 / S0.141
UNPUNCHED STUD BOX BEAM, SEE BEAM SCHEDULE FOR MEMBER SIZES.
HEADER ATTACHMENT - SEE ABOVE DETAIL
2"X2"X14 GA. CLIP ANGLES, 1/2" LESS THAN STUD WIDTH. USE: (3) #10-16 SCREWS ON EACH LEG AS SHOWN ABOVE
TIED JAMB MEMBER
CRIPPLE STUDS. MATCH WALL STUD SIZE AND SPACING
(1) #10-16 SCREW TYP. EACH FLANGE
HEAD TRACK

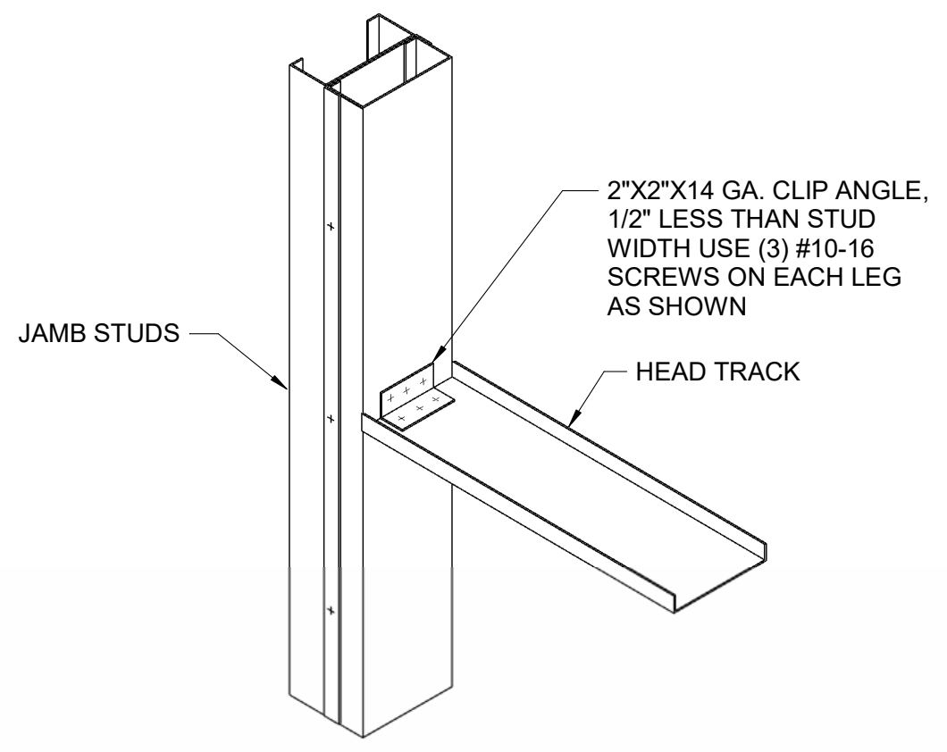
7 Boxed Header Connection
3/4" = 1'-0"


WALL STUD - SEE SCHEDULE FOR SIZE AND SPACING
ATTACH TRACK TO STEEL MEMBER USING (2) 0.157 DIA. P.A.F. AT EACH STUD OR AT 16" O.C. MAX. (TYP.)
RUNNER TRACK
(1) #10-16 SCREW AT EACH FLANGE
HSS STEEL BEAM
WALL STUD - SEE SCHEDULE FOR SIZE AND SPACING
ATTACH TRACK TO STEEL MEMBER USING (2) 0.157 DIA. P.A.F. AT EACH STUD OR AT 16" O.C. MAX. (TYP.)
RUNNER TRACK
(1) #10-16 SCREW AT EACH FLANGE
WF STEEL BEAM

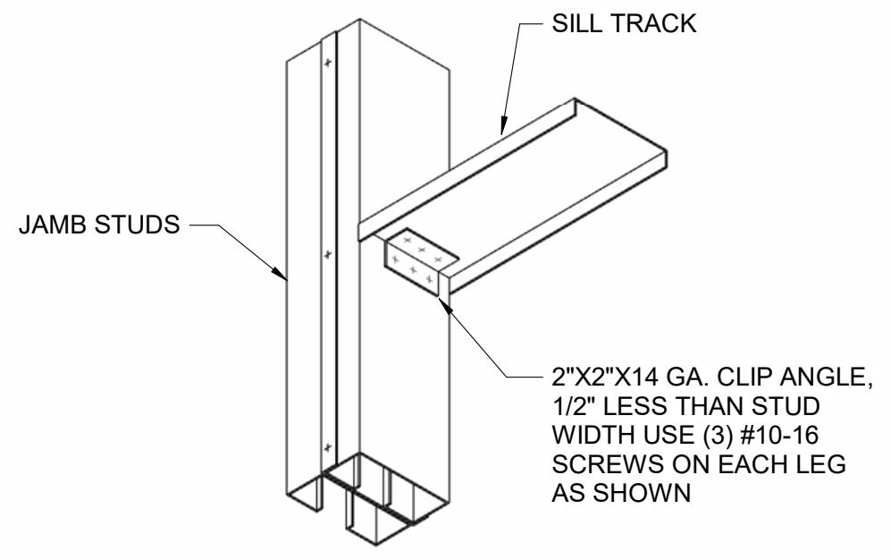
6 Stud to Bottom Track Detail at Steel Beam
3/4" = 1'-0"


RUNNER TRACK
(1) #10-16 SCREW AT EACH FLANGE
ATTACH TRACK TO METAL DECK USING (2) #10 SCREWS AT EACH STUD OR AT 16" O.C. MAX. (TYP.)
PROVIDE SHIM IF GAP IS PRESENT BETWEEN STUD AND TRACK
WALL STUD - SEE SCHEDULE FOR SIZE AND SPACING

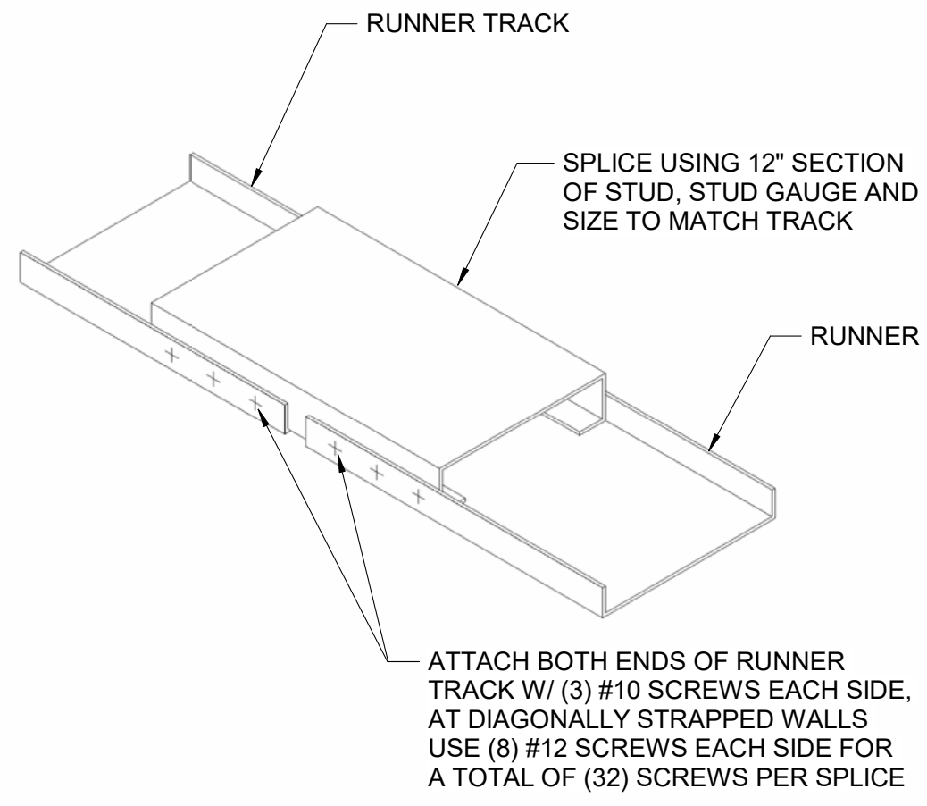
5 Load Bearing Stud to Top Track Detail
3/4" = 1'-0"


JAMB STUDS
2"X2"X14 GA. CLIP ANGLE, 1/2" LESS THAN STUD WIDTH USE (3) #10-16 SCREWS ON EACH LEG AS SHOWN
HEAD TRACK

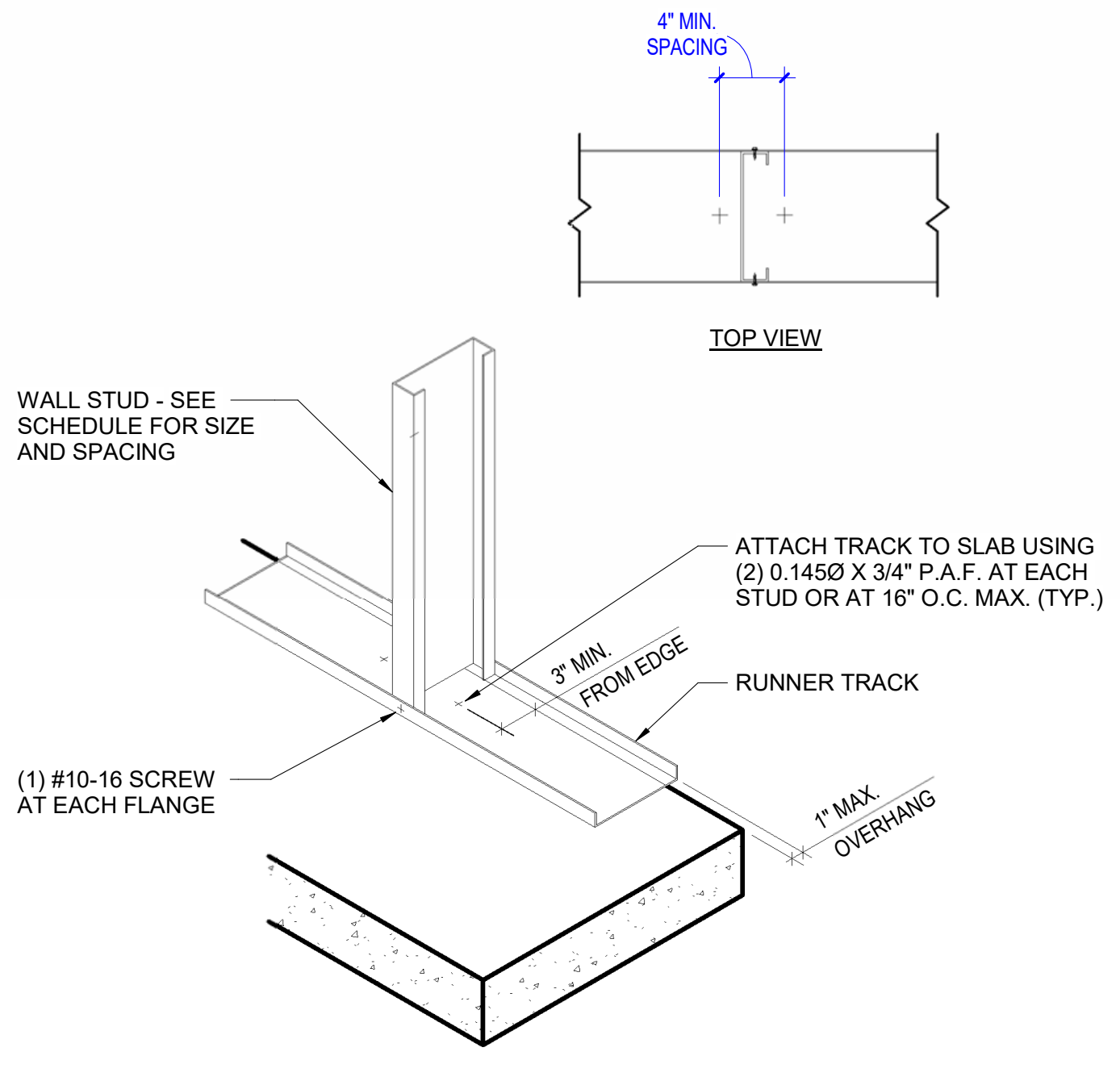
4 Head Track Connection
3/4" = 1'-0"


SILL TRACK
JAMB STUDS
2"X2"X14 GA. CLIP ANGLE, 1/2" LESS THAN STUD WIDTH USE (3) #10-16 SCREWS ON EACH LEG AS SHOWN

3 Sill Track Connection
3/4" = 1'-0"


RUNNER TRACK
SPLICE USING 12" SECTION OF STUD, STUD GAUGE AND SIZE TO MATCH TRACK
RUNNER TRACK
ATTACH BOTH ENDS OF RUNNER TRACK W/ (3) #10 SCREWS EACH SIDE, AT DIAGONALLY STRAPPED WALLS USE (8) #12 SCREWS EACH SIDE FOR A TOTAL OF (32) SCREWS PER SPLICE

2 Track Splice
1 1/2" = 1'-0"


4" MIN. SPACING
WALL STUD - SEE SCHEDULE FOR SIZE AND SPACING
ATTACH TRACK TO SLAB USING (2) 0.145Ø X 3/4" P.A.F. AT EACH STUD OR AT 16" O.C. MAX. (TYP.)
3" MIN. FROM EDGE
RUNNER TRACK
(1) #10-16 SCREW AT EACH FLANGE
1" MAX. OVERHANG

TOP VIEW

1 Non-Load Bearing Stud to Bottom Track Detail
3/4" = 1'-0"

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EAST BATON ROUGE PARISH

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SHEET TITLE:
Typical Metal Stud Framing Details

SHEET NUMBER:
S0.141

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WD1.0 WOOD CONSTRUCTION GENERAL NOTES:

A. ALL WOOD HORIZONTAL AND VERTICAL FRAMING, FURRING, ANCHORAGE AND CONNECTIONS NOT EXPLICITLY SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE PER MINIMUM REQUIRED BY THE LOCALLY ADOPTED BUILDING CODE.

B. PROVIDE STRUCTURAL LUMBER AND ENGINEERED WOOD OF THE FOLLOWING MINIMUM GRADES, SPECIES AND MINIMUM ALLOWABLE STRESSES UNLESS NOTED OTHERWISE ON PLAN:

a. STUDS	SEE STUD SCHEDULE
b. PLATES	SYP OR DFL NO. 3
c. JOISTS	SYP OR DFL NO. 3
d. BEAMS UP TO 4” THK	SYP OR DFL NO. 3
e. POSTS	SYP OR DFL NO. 3
f. BLOCKING	SYP OR DFL NO. 3
g. GLULAM MEMBERS	24F-V4 (DF) OR V3 (SYP) FOR SIMPLY SUPPORTED BEAMS 24F-V8 (DF) OR V5 (SYP) FOR CANTILEVERED AND CONTINUOUS BEAMS
a. PSL BEAMS	MIN. Fb = 2,900 PSI, Fv = 285 PSI, E = 2,000 KSI
b. PSL POSTS	MIN. Fb = 2,400 PSI, E = 1,800 KSI
c. LSL MEMBERS	MIN. Fb = 1,700 PSI, Fv = 400 PSI, E = 1,300 KSI

ADDITIONAL NOTES:

- a. ALL LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED TESTING AGENCY.
- b. LVL BEAMS/POSTS WITH EQUAL OR GREATER SIZE THAN PSL BEAMS/POSTS AND MEET PSL MINIMUM STRENGTH AND STIFFNESS REQUIREMENTS MAY BE SUBSTITUTED FOR PSL BEAMS/POSTS.
- c. FLOOR BRIDGING TO BE PER LOCALLY ADOPTED BUILDING CODE.
- d. PROVIDE LUMBER WITH 19% MAXIMUM MOISTURE CONTENT. MOISTURE MAY BE MEASURED JUST PRIOR TO PLACEMENT.

C. WOOD STRUCTURAL PANEL FLOOR SHEATHING: 23/32” THICK TONGUE AND GROOVE (TG) DOC PS-1 (PLY) OR PS-2 (OSB) EXPOSURE 1 WITH A PANEL INDEX OF 48/24 AND STAMPED WITH APA GRADE TRADEMARK. ALL WOOD STRUCTURAL FLOOR PANELS SHALL BE NAILED TO SUPPORTS WITH 0.148”Ø NAILS X 2.25” MIN. LONG WITH NAIL SPACING OF 6” OC AT PANEL EDGES / BOUNDARIES AND 12” OC AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.

D. WOOD STRUCTURAL PANEL ROOF SHEATHING: DOC PS-1 (PLY) OR DOC PS-2 (OSB) EXPOSURE 1 STAMPED WITH APA GRADE TRADEMARK WITH THE FOLLOWING MINIMUM THICKNESS, PANEL INDEX AND FASTENING REQUIREMENTS:
- a. ROOF SLOPE OF 3:12 OR GREATER: 15/32” THICKNESS WITH A PANEL INDEX 32/16. FASTEN TO SUPPORTS WITH 0.131”Ø NAILS x 2.5” LONG WITH NAIL SPACING PER STANDARD FASTENING SCHEDULE OR AS INDICATED ON ROOF DIAPHRAGM BLOCKING AND NAILING PLANS, WHICHEVER IS SMALLER.
- b. ROOF SLOPE LESS THAN 3:12 TO FLAT: 19/32” THICKNESS WITH A PANEL INDEX OF 40/20. FASTEN TO SUPPORTS WITH 0.148”Ø NAILS x 2.25” LONG WITH NAIL SPACING PER STANDARD FASTENING SCHEDULE OR AS INDICATED ON ROOF DIAPHRAGM BLOCKING AND NAILING PLANS, WHICHEVER IS SMALLER.

E. WOOD STRUCTURAL PANEL SHEAR WALL SHEATHING: EXPOSURE 1. SEE SHEAR WALL SCHEDULE FOR OTHER INFORMATION INCLUDING MINIMUM THICKNESS, GRADE AND FASTENING REQUIREMENTS.

F. ALL LUMBER IN CONTACT WITH CONCRETE, CMU OR EXPOSED TO WEATHER SHALL BE PRESERVATIVE TREATED.

G. PROVIDE FIRE RETARDANT TREATED (FRT) WOOD WHERE INDICATED ON THE STRUCTURAL OR ARCHITECTURAL DRAWINGS.

H. UNLESS NOTED OTHERWISE, ALL NAILS ARE COMMON NAILS WITH DIMENSONAL PROPERTIES COMPLYING WITH AFPA NDS TABLE L4 AND ASTM F1667. INSTALL ALL NAILS PER FASTENING SCHEDULE OR AS INDICATED ON THE STRUCTURAL DRAWINGS.

I. ALL FASTENERS AND HARDWARE CONNECTORS, INCLUDING NUTS AND WASHERS INSTALLED IN OR ATTACHED TO FRT WOOD USED IN *EXTERIOR* APPLICATIONS OR WET OR DAMP LOCATIONS SHALL BE HOT DIP GALVANIZED, STAINLESS STEEL, SILICON BRONZE OR COPPER. FASTENERS OTHER THAN NAILS, TIMBER RIVETS, WOOD LAG SCREWS AND LAG SCREWS SHALL BE PERMITTED TO BE OF MECHANICALY DEPOSITED ZINC COATED STEEL WITH COATING WEIGHTS IN ACCORDANCE WITH ASTM B695, CLASS 55 MIN.

J. ALL FASTENERS AND HARDWARE CONNECTORS, INCLUDING NUTS AND WASHERS INSTALLED IN OR ATTACHED TO FRT AND PRESERVATIVE TREATED WOOD USED IN *INTERIOR* APPLICATIONS SHALL BE IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS. IN INSTANCES WHERE SUCH RECOMMENDATIONS DO NOT EXIST, CONFORM TO THE REQUIREMENTS FOR EXTERIOR APPLICATIONS.

K. ALL FASTENERS AND HARDWARE CONNECTORS, INCLUDING NUTS AND WASHERS INSTALLED IN OR ATTACHED TO PRESERVATIVE TREATED WOOD SHALL BE HOT DIP GALVANIZED, STAINLESS STEEL, SILICON BRONZE OR COPPER. FASTENERS OTHER THAN NAILS, TIMBER RIVETS, WOOD LAG SCREWS AND LAG SCREWS SHALL BE PERMITTED TO BE OF MECHANICALY DEPOSITED ZINC COATED STEEL WITH COATING WEIGHTS IN ACCORDANCE WITH ASTM B695, CLASS 55 MIN. CONNECTORS THAT ARE USED IN *EXTERIOR* APPLICATIONS AND IN CONTACT WITH PRESERVATIVE TREATED WOOD SHALL HAVE COATING TYPES AND WEIGHTS IN ACCORDANCE WITH THE TREATED WOOD OR CONNECTOR MANUFACTURER RECOMMENDATIONS. IN THE ABSENCE OF MANUFACTURER RECOMMENDATIONS, PROVIDE MINIMUM OF ASTM A653, TYPE G185 ZINC COATED GALVANIZED STEEL OR EQUIVALENT. **EXCEPTION**: PLAIN CARBON STEEL FASTENERS, INCUDING NUTS AND WASHERS IN SBX / DOT AND ZINC BORATE PRESERVATIVE TREATED WOOD IN AN INTERIOR, DRY ENVIRONMENT MAY BE USED.

L. FASTENING SCHEDULE SHALL BE IN CONFORMANCE WITH IBC / CBC TABLE 2304.10.1.

M. CONTRACTOR MAY FURR OUT WOOD HEADERS A ND BEAMS USING NON-STRUCTURAL WOOD MEMBERS OR FURRING STRIPS AS REQUIRED TO ALIGN WITH ARCHITECTURAL WALL FINISHES. FURRING SHALL NOT REDUCE THE STRUCTURAL SECTION OF ANY BEAM OR INTERFERE WITH MECHANICAL, ELECTRICAL, OR PLUMBING SYSTEMS. ALL FURRING MATERIALS AND FASTENING METHODS SHALL COMPLY WITH APPLICABLE CODE REQUIREMENTS AND BE COMPARABLE WITH FINISH MATERIALS. COORDINATE WITH ARCHITECTURAL DRAWINGS.

WD2.0 WOOD FRAMING GENERAL NOTES:

A. FLOOR AND ROOF FRAMING LAYOUTS ARE PROVIDED TO ILLUSTRATE DESIGN INTENT AND CONDITIONS OF CONSTRUCTION AND DO NOT NECESSARILY INDICATE SPECIFIC QUANTITIES OF MATERIALS NOR COMPONENTS REQUIRED FOR CONSTRUCTION. CONSTRUCTION BRACING SHALL BE PROVIDED BY THE CONTRACTOR TO MAINTAIN THE BUILDING PLUMB AND TRUE WITH BRACING TO REMAIN IN PLACE UNTIL THE SPECIFIED SHEAR WALLS ARE FULLY INSTALLED.

B. BEARING WALL ENDS OF ALL BEAMS, HEADERS (DROP BEAMS), FLUSH BEAMS, AND GIRDER TRUSSES SHALL MEET THE FOLLOWING MINIMUMS AT EACH BEARING END / POINT:

FRAMING MEMBER MINIMUM STUDS/STUDPACKS (UNLESS NOTED OTHERWISE ON PLAN) [h]

LOCATION / MEMBER	MINIMUM WIDTH		
	4'-0" OR LESS	4'-1" TO 8'-0"	8'-1" TO 12"-0"
INTERIOR AND EXTERIOR LOAD BEARING HEADER (DROP BEAM)	SEE NOTE a	SEE NOTE a	SEE NOTE b
INTERIOR AND EXTERIOR FLUSH BEAM	SEE NOTE c	SEE NOTE d	SEE NOTE d
EXTERIOR NON-LOAD BEARING HEADER (DROP BEAM)	SEE NOTE e	SEE NOTE e	SEE NOTE f
GIRDER TRUSS	SEE NOTE g		

- a. PROVIDE SAME NUMBER OF TRIMMER STUDS AS NUMBER OF BEAM PLIES PLUS (1) KINGSTUD (PROVIDE MINIMUM OF 3 TOTAL STUDS)
- b. PROVIDE (1) MORE TRIMMER STUDS AS NUMBER OF BEAM PLIES PLUS (2) KINGSTUDS (PROVIDE MINIMUM OF 4 TOTAL STUDS)
- c. PROVIDE (1) MORE STUD THAN NUMBER OF PLIES OF BEAM
- d. PROVIDE (2) MORE STUDS THAN NUMBER OF PLIES OF BEAM
- e. PROVIDE (1)2x6 TRIMMER AND (1)2x6 KINGSTUD
- f. PROVIDE (2)2x6 TRIMMERS AND (2)2x6 KINGSTUDS
- g. PROVIDE (1) MORE STUD THAN NUMBER OF TRUSS PLIES (PROVIDE MINIMUM 4 STUDS)
- h. FOR SOLID FRAMING MEMBERS, "NUMBER OF PLIES OF BEAM" EQUALS 2, 3, AND 4 PLIES FOR NOMINAL BEAM WIDTHS OF 4 INCHES, 6 INCHES, AND 8 INCHES, RESPECTIVELY

C. THE CENTERLINE OF THE GIRDER TRUSS OR SOLID WOOD BEAM SHALL BE THE CENTERLINE OF THE SUPPORTING BEARING WALL STUD PACK OR POST.

D. THE SUPPORTING BEARING WALL STUD PACK OR POST SHALL BE CONTINUOUS LEVEL BY LEVEL DOWN T SUPPORTING SLAB WITH SQUASH BLOCKS INSTALLED AT ALL FLOOR / ROOF CAVITIES COMPOSED OF EQUAL SIZE AND MATERIALS TO ENSURE LOAD PATH CONTINUITY.

E. PROVIDE 2x4 (MIN.) FLATWISE @ 24” OC MAX. BETWEEN FLOOR FRAMING UNDER PARTITIONS ABOVE WHICH ARE PARALLEL TO FRAMING BUT NOT DIRECTLY OVER THE FRAMING AND FASTEN TO FLOOR FRAMING WITH (2) 0.148” DIA X 3” LONG TOENAILS EA. END UNO ON PLAN OR DETAILS.

F. STRUCTURAL MEMBERS SHALL NOT BE CUT, TRIMMED OR BORED FOR PIPES OR OTHER BUILDING COMPONENTS UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS. NOTCHING AND BORING OF STUDS AND TOP/BOTTOM PLATES SHALL CONFORM TO SECTIONS 2308.5.9 AND 2308.5.10 OF THE IBC/CBC.WHERE TOP/BOTTOM PLATES ARE CUT FOR PIPES, PROVIDE 1.5 INCH WIDE X 16 GA STEEL STRAP TIE FASTENED TO EACH PLATE ACROSS AND TO EACH SIDE OF THE CUT-OUT FASTENED ACROSS EACH PLATE ON EACH SIDE OF THE CUT OUT WITH NO LESS THAN (6) 16d NAILS EACH SIDE PER CHAPTER 23 OF THE IBC/CBC.

G. ALL JOIST / FLUSH BEAMS SHALL BE SUPPORTED BY A HANGER WITH AN APPROVAL FROM AN AGENCY DEEMED ACCEPTABLE TO THE LOCAL BUILDING OFFICIAL.

H. PROVIDE CONTINUOUS END TO END STRONGBACKS ON ALL JOIST SPANS OVER 7’-6” AND (2) STRONGBACKS AT ALL JOIST SPANS OVER 15’-0”. STRONGBACKS SHALL BE INSTALLED PERPENDICULAR TO AND NAILED DIRECTLY TO JOIST FRAMING.

I. AT CONVENTIONAL FRAMING, PROVIDE THE FOLLOWING WHERE APPLICABLE:
- a. COLLAR TIES OF 1X6 BOARDS INSTALLED AT THE UPPER THIRD OF THE RIDGE BEAM DEPTH SPACED 4’-0” OC MAX;
- b. SOLID 2X, METAL CROSS OR WOOD CROSS BRIDGING AT 8’-0” OC MAX FOR JOIST SPANS OVER 10’-0”;
- c. PURLINS OF SAME SIZE AS RAFTERS AT ALL RAFTER SPANS OVER 7’-6” FOR 2X6, 10’-0” FOR 2X8, 12’-6” FOR 2X10 AND 15’-0” FOR 2X12

J. PROVIDE DOUBLE JOISTS AT LOCATIONS WHERE NON LOAD BEARING PARTITIONS ARE PLACED PARALLEL TO JOIST SPAN. IF WALL ABOVE IS A PLUMBING WALL, APPROPRIATELY SPACE DOUBLE JOISTS TO ALLOW PLUMBING LINES TO STACK.

K. EXTERIOR WALLS AT VAULTED CEILINGS SHALL BE BALLOON FRAMED.

L. WHERE BUILT-UP OVERFRAMING IS USED TO CREATE SLOPES AT EXPOSED ROOF DECKS AND SKYDECKS, OVERFRAMING PERPENDICULAR TO FLOOR FRAMING SHALL BE RIPPED 2X MEMBERS AT 24” OC MAX. WITH PERPENDICULAR BLOCKING AT 48” OC MAX. TYPICAL UNO ON PLAN OR DETAILS.

M. 2X4 CHIMNEY CONSTRUCTION SHALL BE SHEATHED WITH 15/32” WOOD STRUCTURAL PANEL.

WD3.0 WOOD CONNECTOR GENERAL NOTES:

A. ALL SPECIFIED FASTENERS MUST BE INSTALLED ACCORDING TO MANUFACTURER INSTRUCTIONS. INCORRECT FASTENER QUANTITY, SIZE, PLACEMENT, TYPE, MATERIAL, OR FINISH MAY CAUSE THE CONNECTION TO FAIL. PRIOR TO USING A PARTICULAR FASTENER, REFER TO MANUFACTURER SPECIFICATIONS.

B. LARGER DIAMETER FASTENERS MAY BE SUBSTITUTED FOR SMALLER DIAMETER FASTENERS IN CONNECTORS PROVIDED THE LARGER FASTENER DOES NOT CAUSE SPLITTING IN THE WOOD MEMBER AND THE CONNECTOR HOLES ARE NOT ENLARGED.

C. FILL ALL FASTENER HOLES AS SPECIFIED IN THE INSTALLATION INSTRUCTIONS FOR THAT PRODUCT. REFER TO MANUFACTURER FOR THE REQUIREMENTS OF VARIOUS SHAPED FASTENER HOLES.

D. DO NOT OVERDRIVE NAILS. OVERDRIVEN NAILS REDUCE SHEAR CAPACITY.

E. PRODUCTS SHALL BE INSTALLED FOR THE USE SPECIFIED. USE ONLY THE MATERIALS SPECIFIED IN THE INSTALLATION INSTRUCTIONS. SUBSTITUTION OF OR FAILURE TO USE SPECIFIED MATERIALS MAY CAUSE THE CONNECTION TO FAIL. DO NOT ALTER INSTALLATION PROCEDURES FROM THOSE SET FORTH IN MANUFACTURER SPECIFICATIONS.

F. DO NOT ADD FASTENER HOLES OR OTHERWISE MODIFY CONNECTOR PRODUCTS. THE PERFORMANCE OF MODIFIED PRODUCTS MAY BE SUBSTANTIALLY WEAKENED. BE ADVISED MANUFACTURER WILL NOT WARRANT OR GUARANTEE THE PERFORMANCE OF SUCH MODIFIED PRODUCTS.

G. THE PROPER USE OF CERTAIN PRODUCTS REQUIRES THAT THE PRODUCT BE BENT. FOR THOSE PRODUCTS, INSTALLERS MUST NOT BEND THE PRODUCT MORE THAN ONE TIME (ONE FULL CYCLE).

H. BOLT HOLES SHALL BE AT LEAST A MINIMUM OF 1/32" AND NO MORE THAN A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER (PER THE 2015 / 2018 NDS, SECTION 12.1.3.2 AND AISI S100, TABLE E3A IF APPLICABLE).

I. INSTALL ALL SPECIFIED FASTENERS BEFORE LOADING THE CONNECTION.

J. SOME HARDENED FASTENERS MAY HAVE PREMATURE FAILURE IF EXPOSED TO MOISTURE. THESE FASTENERS ARE RECOMMENDED TO BE USED IN DRY INTERIOR APPLICATIONS.

K. USE PROPER SAFETY EQUIPMENT.

L. WELDING GALVANIZED STEEL MAY PRODUCE HARMFUL FUMES; FOLLOW PROPER WELDING PROCEDURES AND SAFETY PRECAUTIONS. WELDING SHOULD BE IN ACCORDANCE WITH A.W.S. (AMERICAN WELDING SOCIETY) STANDARDS. UNLESS OTHERWISE NOTED, CONNECTORS CANNOT BE WELDED.

M. PNEUMATIC OR POWER-ACTUATED FASTENERS MAY DEFLECT AND INJURE THE OPERATOR OR OTHERS. PNEUMATIC NAIL TOOLS MAY BE USED TO INSTALL CONNECTORS, PROVIDED THE CORRECT QUANTITY AND TYPE OF NAILS (LENGTH AND DIAMETER) ARE PROPERLY INSTALLED IN THE NAIL HOLES. CONNECTORS WITH TOOL EMBOSSMENTS OR TOOLS WITH NAIL HOLE-LOCATING MECHANISMS SHOULD BE USED. FOLLOW THE MANUFACTURER’S INSTRUCTIONS AND USE THE APPROPRIATE SAFETY EQUIPMENT. POWER-ACTUATED FASTENERS SHOULD NOT BE USED TO INSTALL CONNECTORS, UNLESS SPECIFICALLY NOTED BY MANUFACTURER.

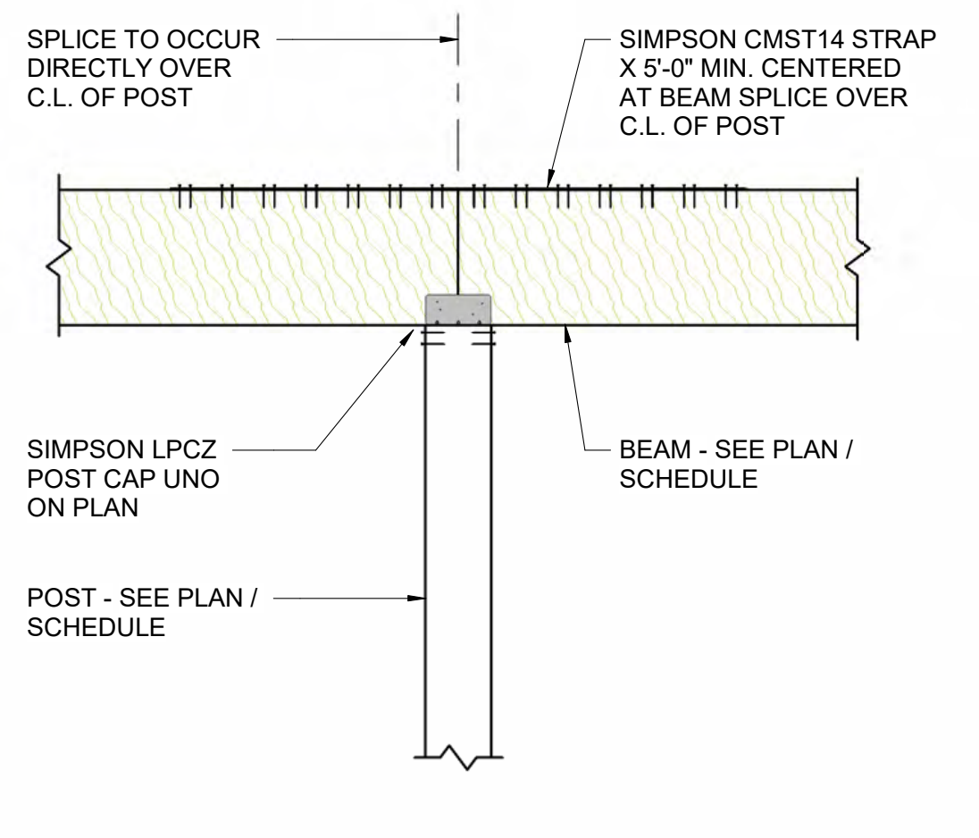
N. JOIST SHALL BEAR COMPLETELY ON THE CONNECTOR SEAT, AND THE GAP BETWEEN THE JOIST END AND THE HEADER SHALL NOT EXCEED 1/8" PER ICC-ES AC13, ASTM D1761 AND ASTM D7147 TEST STANDARDS (UNLESS SPECIFICALLY NOTED OTHERWISE).

O. FASTENERS ARE PERMITTED TO BE INSTALLED THROUGH METAL TRUSS PLATES WHEN APPROVED BY THE TRUSS DESIGNER IN ACCORDANCE WITH ANSI/TPI 1-2014, SECTION 7.5.3.4 AND 8.9.2. DO NOT DRIVE NAILS THROUGH THE TRUSS PLATE ON THE OPPOSITE SIDE OF SINGLE-PLY TRUSSES WHICH COULD FORCE THE PLATE OFF THE TRUSS.

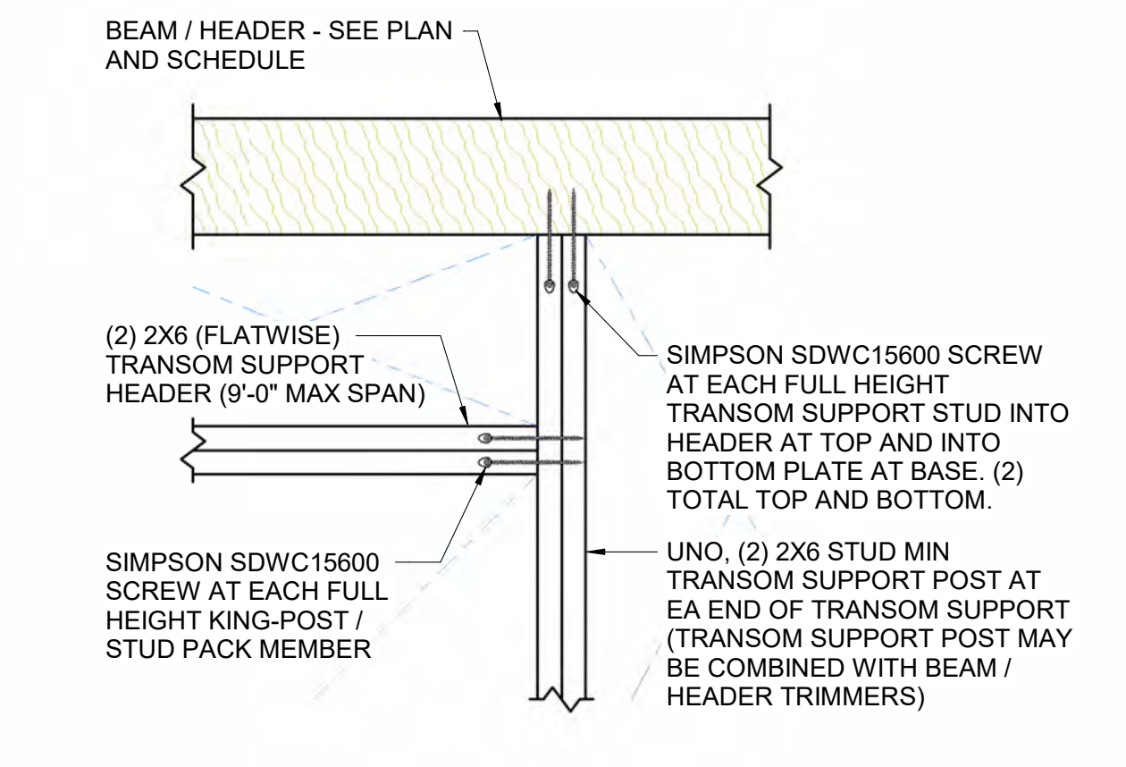
P. NUTS SHALL BE INSTALLED SUCH THAT THE END OF THE THREADED ROD OR BOLT IS AT LEAST FLUSH WITH THE TOP OF THE NUT.

Q. WHEN INSTALLING HURRICANE TIES ON THE INSIDE OF THE WALL, SPECIAL CONSIDERATIONS MUST BE TAKEN TO PREVENT CONDENSATION ON THE INSIDE OF THE COMPLETED STRUCTURE IN COLD CLIMATES.

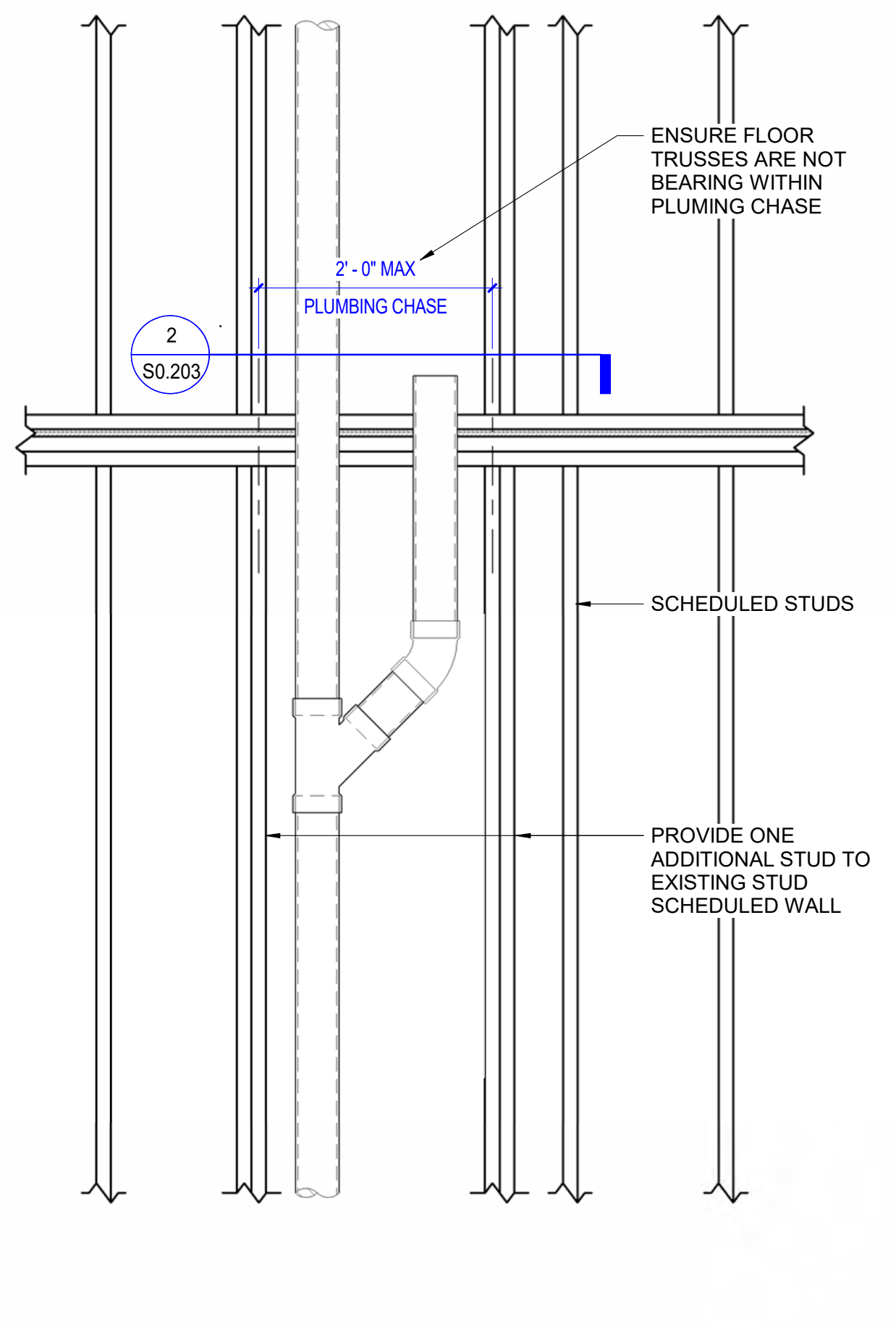
R. UNLESS OTHERWISE NOTED, CONNECTORS HAVE BEEN DESIGNED TO BE INSTALLED AT THE TIME THE FRAMING MEMBERS ARE INSTALLED.


SPLICE TO OCCUR DIRECTLY OVER C.L. OF POST
SIMPSON CMST14 STRAP X 5'-0" MIN. CENTERED AT BEAM SPLICE OVER C.L. OF POST
SIMPSON LPCZ POST CAP UNO ON PLAN
BEAM - SEE PLAN / SCHEDULE
POST - SEE PLAN / SCHEDULE

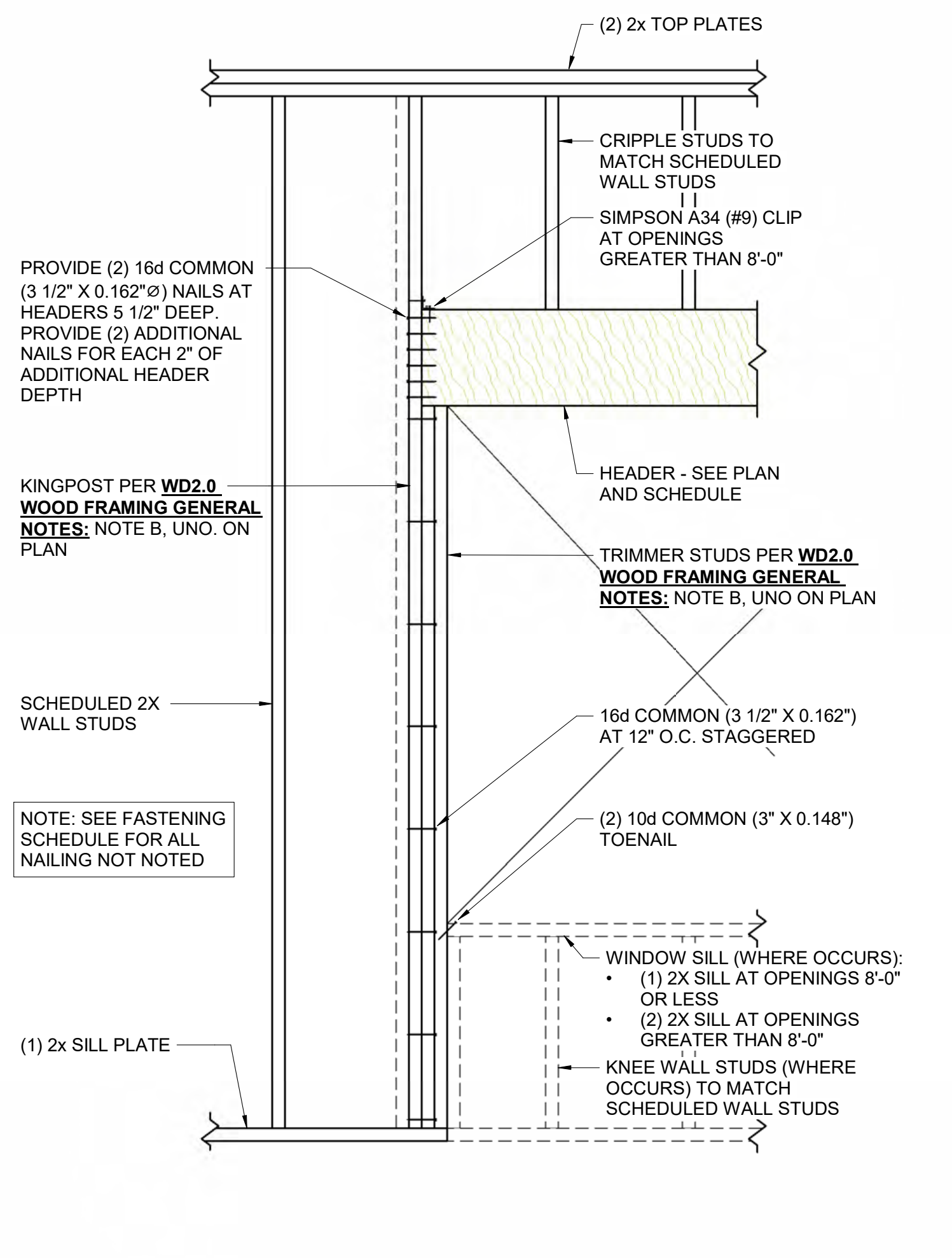
6 Wood Beam to Post Interior Connection
3/4" = 1'-0"


BEAM / HEADER - SEE PLAN AND SCHEDULE
(2) 2X6 (FLATWISE) TRANSOM SUPPORT HEADER (9'-0" MAX SPAN)
SIMPSON SDWC15600 SCREW AT EACH FULL HEIGHT TRANSOM SUPPORT STUD INTO HEADER AT TOP AND INTO BOTTOM PLATE AT BASE. (2) TOTAL TOP AND BOTTOM.
SIMPSON SDWC15600 SCREW AT EACH FULL HEIGHT KING-POST / STUD PACK MEMBER
UNO, (2) 2X6 STUD MIN TRANSOM SUPPORT POST AT EA END OF TRANSOM SUPPORT (TRANSOM SUPPORT POST MAY BE COMBINED WITH BEAM / HEADER TRIMMERS)

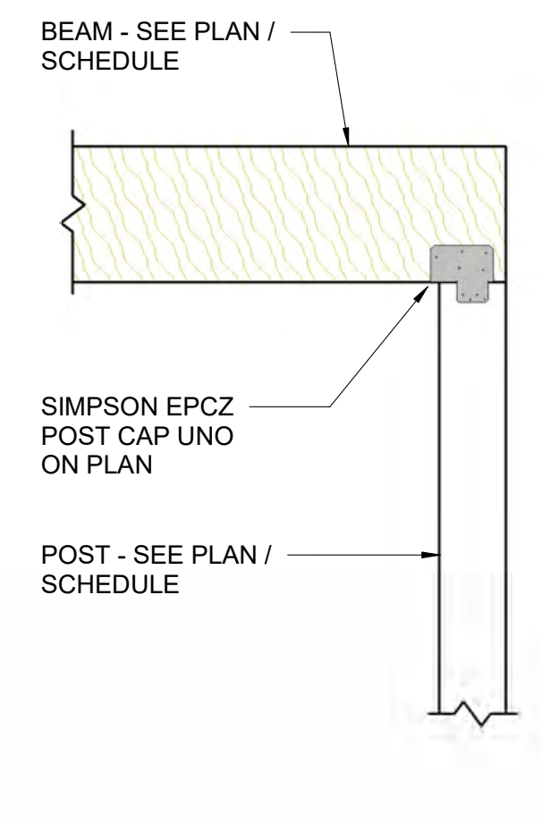
4 Transom Header / Support Post
1" = 1'-0"


ENSURE FLOOR TRUSSES ARE NOT BEARING WITHIN PLUMBING CHASE
2'-0" MAX
PLUMBING CHASE
2
S0.203
SCHEDULED STUDS
PROVIDE ONE ADDITIONAL STUD TO EXISTING STUD SCHEDULED WALL

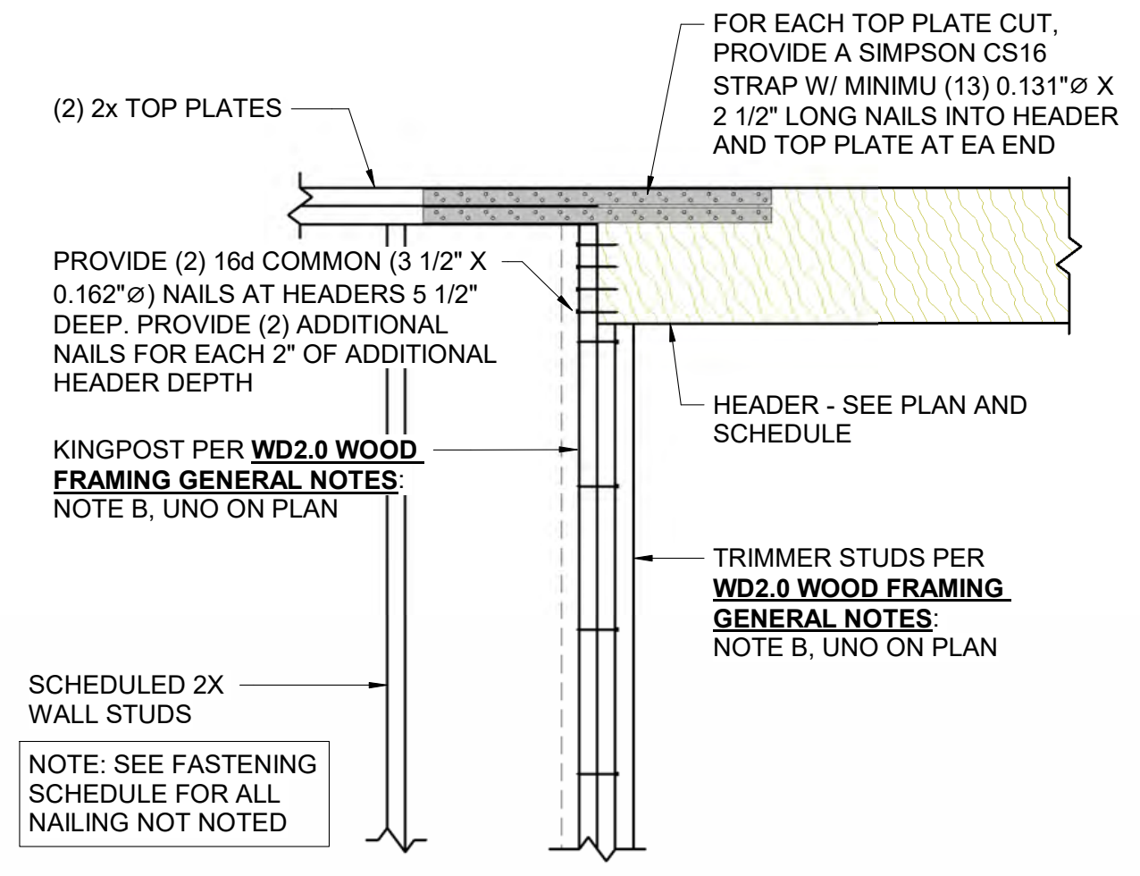
2 Plumbing Riser in Exterior / Load Bearing Stud Wall
3/4" = 1'-0"


(2) 2x TOP PLATES
CRIPPLE STUDS TO MATCH SCHEDULED WALL STUDS
SIMPSON A34 (#9) CLIP AT OPENINGS GREATER THAN 8'-0"
PROVIDE (2) 16d COMMON (3 1/2" X 0.162"Ø) NAILS AT HEADERS 5 1/2" DEEP. PROVIDE (2) ADDITIONAL NAILS FOR EACH 2" OF ADDITIONAL HEADER DEPTH
HEADER - SEE PLAN AND SCHEDULE
KINGPOST PER WD2.0 WOOD FRAMING GENERAL NOTES: NOTE B, UNO ON PLAN
TRIMMER STUDS PER WD2.0 WOOD FRAMING GENERAL NOTES: NOTE B, UNO ON PLAN
SCHEDULED 2X WALL STUDS
16d COMMON (3 1/2" X 0.162") AT 12" O.C. STAGGERED
NOTE: SEE FASTENING SCHEDULE FOR ALL NAILING NOT NOTED
(2) 10d COMMON (3" X 0.148") TOENAIL
WINDOW SILL (WHERE OCCURS):
• (1) 2X SILL AT OPENINGS 8'-0" OR LESS
• (2) 2X SILL AT OPENINGS GREATER THAN 8'-0"
KNEE WALL STUDS (WHERE OCCURS) TO MATCH SCHEDULED WALL STUDS
(1) 2x SILL PLATE

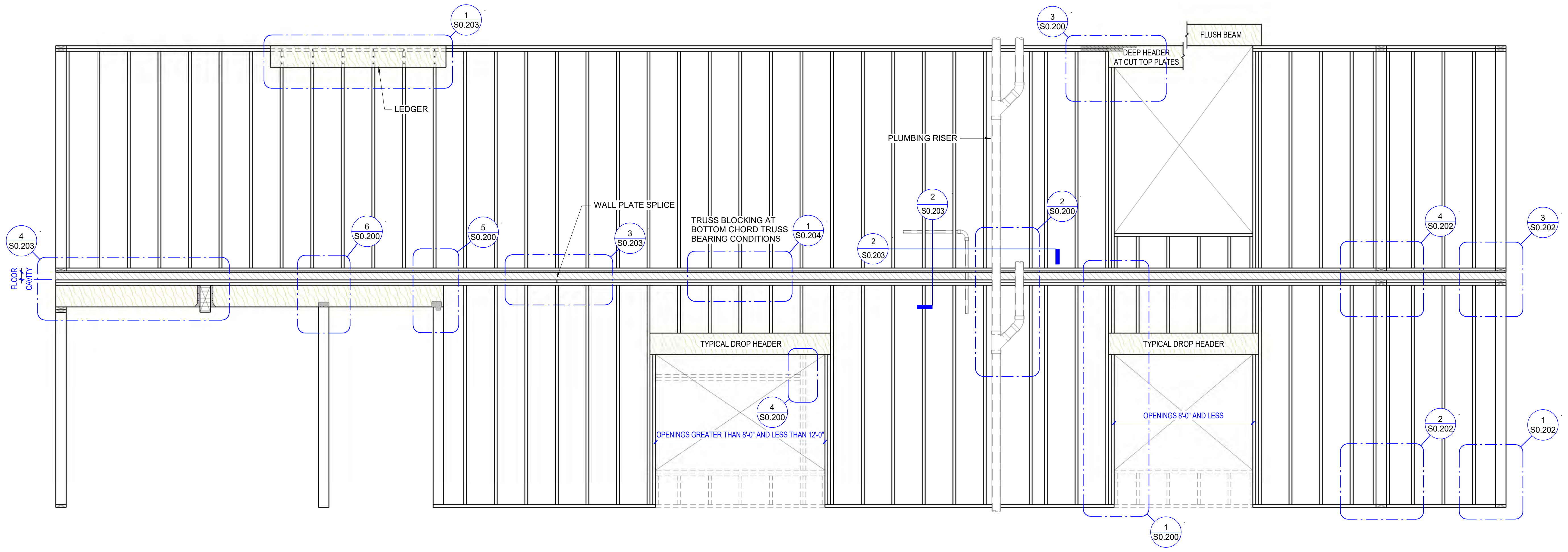
1 Framed Opening in Stud Wall
3/4" = 1'-0"


BEAM - SEE PLAN / SCHEDULE
SIMPSON EPCZ POST CAP UNO ON PLAN
POST - SEE PLAN / SCHEDULE

5 Wood Beam to Post End Connection
3/4" = 1'-0"


(2) 2x TOP PLATES
FOR EACH TOP PLATE CUT, PROVIDE A SIMPSON CS16 STRAP W/ MINIMU (13) 0.131"Ø X 2 1/2" LONG NAILS INTO HEADER AND TOP PLATE AT EA END
PROVIDE (2) 16d COMMON (3 1/2" X 0.162"Ø) NAILS AT HEADERS 5 1/2" DEEP. PROVIDE (2) ADDITIONAL NAILS FOR EACH 2" OF ADDITIONAL HEADER DEPTH
HEADER - SEE PLAN AND SCHEDULE
KINGPOST PER WD2.0 WOOD FRAMING GENERAL NOTES: NOTE B, UNO ON PLAN
TRIMMER STUDS PER WD2.0 WOOD FRAMING GENERAL NOTES: NOTE B, UNO ON PLAN
SCHEDULED 2X WALL STUDS
NOTE: SEE FASTENING SCHEDULE FOR ALL NAILING NOT NOTED

3 Deep Header at Cut Top Plates
3/4" = 1'-0"



WD4.0 PRE-MANUFACTURED WOOD TRUSS SPECIFICATIONS:

1. DESIGN
   - A. DESIGN AND DETAILING OF PRE-MANUFACTURED WOOD TRUSSES ARE DELEGATED TO THE GENERAL CONTRACTOR IN ACCORDANCE WITH SECTION "GN1.5 - DEFERRED SUBMITTALS".
   - B. TRUSS MANUFACTURER SHALL REFER TO ARCHITECTURAL DRAWINGS FOR TRUSS END BEARING TYPE AND/OR END SUPPORT CONDITION. ALL TRUSSES SHALL BE DESIGNED TO MEET ARCHITECTURAL INTENT AT EACH LEVEL AND EACH END SUPPORT CONDITION AS REQUIRED.
   - C. DESIGN TRUSSES FOR LOADING CRITERIA PROVIDED IN SECTIONS GN1.1 THROUGH GN1.4 OF THE GENERAL NOTES AS A MINIMUM.
     - a. INCREASE DEAD LOAD AS NEEDED TO ACCOUNT FOR TRUSS SELF WEIGHT.
     - b. DESIGN TRUSS BOTTOM CHORD FOR 5 PSF DEAD LOAD AS A MINIMUM.
     - c. INCREASE DEAD LOAD AS NEEDED TO ACCOUNT FOR LIGHTWEIGHT / GYPCRETE TOPPING WHERE APPLICABLE.
     - d. REFER TO PLUMBING DRAWINGS AND INCREASE LOADING AS NEEDED TO ACCOUNT FOR SPRINKLER AND WATER SUPPLY LINES
   - D. TRUSS MANUFACTURER SHALL FURNISH TRUSS DESIGN DRAWINGS PREPARED IN ACCORDANCE WITH ALL APPLICABLE LEGAL REQUIREMENTS.
   - E. THE TRUSS MANUFACTURER SHALL FURNISH A TRUSS PLACEMENT DIAGRAM WHICH SHALL PROVIDE AT A MINIMUM THE LOCATION ASSUMED FOR EACH TRUSS BASED ON THE TRUSS MANUFACTURER'S INTERPRETATION OF THE CONSTRUCTION DOCUMENTS.
   - F. THE TRUSS MANUFACTURER SHALL SUBMIT THE TRUSS SUBMITTAL PACKAGE TO THE ARCHITECT, SEOR, AND GENERAL CONTRACTOR FOR REVIEW AND APPROVAL PRIOR TO THE MANUFACTURING OF THE TRUSSES.
   - G. TRUSS DESIGN DOCUMENTS SUBMITTED FOR REVIEW SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE JURISDICTION OF THE PROJECT AND INCLUDE THE FOLLOWING ITEMS:
     - a. BUILDING CODE USED FOR DESIGN CALCULATIONS CORRESPONDING TO THE DESIGN SHOWN.
     - b. SLOPE OR DEPTH, SPAN, AND SPACING.
     - c. LOCATION OF ALL JOINTS AND SUPPORT LOCATIONS.
     - d. NUMBER OF PLIES IF GREATER THAN ONE.
     - e. REQUIRED BEARING WIDTHS.
     - f. DESIGN LOADS AS APPLICABLE, INCLUDING (REFER TO GENERAL NOTES FOR APPLICABLE DESIGN LOADS. TRUSS MANUF TO PROVIDE A MINIMUM OF 5PSF BOTTOM CHORD DEAD LOAD. TRUSS MANUFACTURER):
       - TOP CHORD LIVE LOAD (FOR ROOF TRUSSES THIS SHALL BE THE CONTROLLING CASE OF LIVE, RAIN OR SNOW LOAD); XX
       - TOP CHORD DEAD LOAD; XX
       - BOTTOM CHORD LIVE LOAD; XX
       - BOTTOM CHORD DEAD LOAD; XX
       - ADDITIONAL LOADS AND LOCATIONS. (INCLUDING ROOF LEVEL MECHANICAL LOADS AS APPLICABLE)
       - ENVIRONMENTAL LOAD DESIGN CRITERIA (WIND SPEED, SNOW, SEISMIC, AND ALL APPLICABLE FACTORS AS REQUIRED TO CALCULATE THE TRUSS LOADS); AND
       - OTHER LATERAL LOADS, INCLUDING DRAG STRUT LOADS.
     - g. ADJUSTMENTS TO WOOD MEMBER AND METAL CONNECTOR PLATE DESIGN VALUES FOR CONDITIONS OF USE.
     - h. MAXIMUM REACTION FORCE AND DIRECTION, INCLUDING MAXIMUM UPLIFT REACTION FORCES WHERE APPLICABLE.
     - i. METAL CONNECTOR PLATE TYPE, MANUFACTURER, SIZE, THICKNESS OR GAUGE, AND THE DIMENSIONED LOCATION OF EACH METAL CONNECTOR PLATE EXCEPT WHERE SYMMETRICALLY LOCATED RELATIVE TO THE JOINT INTERFACE.
     - j. SIZE, SPECIES, AND GRADE FOR EACH WOOD MEMBER;
     - k. TRUSS-TO-TRUSS CONNECTION AND TRUSS FIELD ASSEMBLY REQUIREMENTS.
     - l. CALCULATED SPAN TO DEFLECTION RATIO AND/OR MAXIMUM VERTICAL AND HORIZONTAL DEFLECTION FOR LIVE AND FOR LIVE PLUS DEAD LOAD AND K_{CR} (CREEP FACTOR) AS APPLICABLE.
     - m. MAXIMUM AXIAL COMPRESSION AND TENSION FORCES IN THE TRUSS MEMBERS.
     - n. FABRICATION TOLERANCE PER ANSI/TPI 1.
     - o. REQUIRED PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT LOCATION.
     - p. TRUSS DESIGNER

WD4.0 PREMANUFACTURED WOOD TRUSS SPECIFICATIONS (CONT):

   - H. PROVIDE DOUBLE TOP CHORD FOR ALL APPLICABLE TRUSSES AS NOTED ON WOOD DIAPHRAGM BLOCKING/STRAPPING PLANS.
   - I. TRUSSES SHALL BE DESIGNED FOR WIND UPLIFT WHERE APPLICABLE AND MECHANICALLY FASTENED AT EACH BEARING LOCATION WITH APPROPRIATE UPLIFT CONNECTORS. SEE SECTION "GN3.0 - COMPONENT AND CLADDING WIND PRESSURE" FOR LOADING AND TYPICAL FRAMING DETAILS FOR LOAD PATH INTENT.
   - J. LIMIT DEFLECTIONS FOR FLOOR TRUSSES TO L/480 FOR LIVE LOADS AND L/360 FOR TOTAL LOADS, BUT NOT TO EXCEED 1 INCH. LIMIT DEFLECTIONS FOR ROOF TRUSSES TO L/240 FOR LIVE LOADS AND L/180 FOR TOTAL LOADS, BUT NOT TO EXCEED 1.25 INCH.
   - K. PROVIDE TRUSS BLOCKS OR ALIGN DRAG TRUSSES ABOVE SHEARWALLS AS REQUIRED. SEE TYPICAL DETAILS. DESIGN FOR FORCES SHOWN ON PLANS OR SCHEDULED SHEARWALL CAPACITY.
   - L. TRUSS BLOCKS. TRUSS SUPPLIER TO PROVIDE TRUSS BLOCKS AT INTERMEDIATE TRUSS BEARING POINTS (ALL TRUSS TYPES) AND AT ENDS (BOTTOM CHORD BEARING TRUSSES ONLY). TRUSSES MAY BE SITE BUILT OR PREFABRICATED. ALL TRUSS BLOCKS SHALL BE DESIGNED TO RESIST A MINIMUM ALLOWABLE LATERAL LOAD OF 1.5 KIPS AT THE ROOF LEVEL AND 2.5 KIPS AT ALL FLOORS, TYPICAL. IN ADDITION, THE TOP CHORD OF THE TRUSS BLOCK MUST BE VERTICAL WHEN INSTALLED UNDER LOAD BEARING WALLS.
2. MATERIALS
   - A. LUMBER
     - a. LUMBER USED SHALL BE IDENTIFIED BY GRADE MARK OF A LUMBER INSPECTION BUREAU OR AGENCY APPROVED BY THE AMERICAN LUMBER STANDARDS COMMITTEE, AND SHALL BE THE SIZE, SPECIES, AND GRADE AS SHOWN ON THE TRUSS DESIGN DRAWINGS, OR EQUIVALENT AS APPROVED BY THE TRUSS DESIGNER.
     - b. ADJUSTMENT OF VALUE FOR DURATION OF LOAD OR CONDITIONS OF USE SHALL BE IN ACCORDANCE WITH ANSI/TPI 1.
     - c. FIRE RETARDANT TREATED (FRT) LUMBER, WHERE APPLICABLE, SHALL MEET THE SPECIFICATIONS OF THE FIRE RETARDANT CHEMICAL MANUFACTURER, THE TRUSS DESIGN AND ANSI/TPI 1 AND SHALL BE RE-DRIED AFTER TREATMENT TO 19 PERCENT MAXIMUM MOISTURE CONTENT AT TEMPERATURES NOT TO EXCEED 160°F (71°C) IN ACCORDANCE WITH AWPA STANDARDS C20. FRT LUMBER DESIGN VALUES SHALL BE DEVELOPED FROM APPROVED TEST METHODS AND PROCEDURES THAT CONSIDER POTENTIAL STRENGTH-REDUCTION CHARACTERISTICS, INCLUDING THE EFFECTS OF ELEVATED TEMPERATURE AND MOISTURE. DESIGN VALUES SHALL BE APPROVED BY THE AUTHORITIES HAVING JURISDICTION. LUMBER TREATER SHALL SUPPLY CERTIFICATE OF COMPLIANCE.
   - B. METAL CONNECTOR PLATES
     - a. METAL CONNECTOR PLATES SHALL BE MANUFACTURED BY A SBCA MEMBER PLATE MANUFACTURER AND SHALL BE OF GALVANIZED STEEL, ALUMINUM-ZINC ALLOY COATED STEEL OR STAINLESS STEEL CONFORMING TO THE REQUIREMENTS OF ANSI/TPI 1. MINIMUM THICKNESS IN INCHES (OR MM FOR METRIC UNITS), INCLUDING BOTH UNCOATED AND COATED THICKNESSES, IF GALVANIZED AND ALUMINUM-ZINC ALLOY COATED, SHALL BE SPECIFIED FOR EACH TYPE OF METAL CONNECTOR PLATE. WORKING STRESSES IN STEEL ARE TO BE APPLIED TO EFFECTIVENESS RATIOS FOR PLATES AS DETERMINED BY TEST AND IN ACCORDANCE WITH ANSI/TPI 1.
     - b. IN HIGHLY CORROSIVE ENVIRONMENTS, SPECIAL APPLIED COATINGS OR STAINLESS STEEL MAY BE REQUIRED, AS SPECIFIED IN THE CONTRACT DOCUMENTS.
3. MANUFACTURING
   - A. TRUSSES SHALL BE MANUFACTURED TO MEET THE QUALITY REQUIREMENTS OF ANSI/TPI 1 AND IN ACCORDANCE WITH THE INFORMATION PROVIDED IN THE FINAL APPROVED TRUSS DESIGN DRAWINGS.
4. HANDLING, INSTALLING, AND BRACING
   - A. TRUSSES SHALL BE HANDLED DURING MANUFACTURING, DELIVERY AND BY THE CONTRACTOR AT THE JOB SITE SO AS NOT TO BE SUBJECTED TO EXCESSIVE BENDING.
   - B. TRUSSES SHALL BE UNLOADED IN A MANNER SO AS TO MINIMIZE LATERAL STRAIN. TRUSSES SHALL BE PROTECTED FROM DAMAGE THAT MIGHT RESULT FROM ON-SITE ACTIVITIES AND ENVIRONMENTAL CONDITIONS. TRUSSES SHALL BE HANDLED IN SUCH A WAY SO AS TO PREVENT TOPPLING WHEN BANDING IS REMOVED.

WD4.0 PREMANUFACTURED WOOD TRUSS SPECIFICATIONS (CONT):

   - C. CONTRACTOR SHALL BE RESPONSIBLE FOR THE HANDLING, INSTALLATION, AND TEMPORARY RESTRAINT/BRACING OF THE TRUSSES IN A GOOD WORKMANLIKE MANNER AND IN ACCORDANCE WITH THE RECOMMENDATIONS SET FORTH IN SBCA/TPI’S *BUILDING COMPONENT SAFETY INFORMATION (BCSI): GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES*.
   - D. APPARENT DAMAGE TO TRUSSES, IF ANY, SHALL BE REPORTED TO TRUSS MANUFACTURER PRIOR TO ERECTION.
   - E. TRUSSES SHALL BE SET AND SECURED LEVEL AND PLUMB, AND IN CORRECT LOCATION. EACH TRUSS SHALL BE HELD IN CORRECT ALIGNMENT UNTIL SPECIFIED PERMANENT RESTRAINT AND BRACING IS INSTALLED.
   - F. CUTTING AND ALTERING OF TRUSSES IS NOT PERMITTED. IF ANY TRUSS SHOULD BECOME BROKEN, DAMAGED, OR ALTERED, WRITTEN CONCURRENCE AND APPROVAL BY A REGISTERED DESIGN PROFESSIONAL IS REQUIRED.
   - G. CONCENTRATED LOADS SHALL NOT BE PLACED ON TOP OF TRUSSES UNTIL ALL SPECIFIED RESTRAINT AND BRACING HAS BEEN INSTALLED AND STRUCTURAL SHEATHING IS PERMANENTLY NAILED IN PLACE. SPECIFICALLY AVOID STACKING FULL BUNDLES OF CONSTRUCTION MATERIALS OR OTHER CONCENTRATED LOADS ON TOP OF TRUSSES.
   - H. THE TRUSS SUBMITTAL PACKAGE AND ANY SUPPLEMENTARY INFORMATION PROVIDED BY THE TRUSS MANUFACTURER SHALL BE PROVIDED BY THE CONTRACTOR TO THE INDIVIDUAL OR ORGANIZATION RESPONSIBLE FOR THE INSTALLATION OF THE TRUSSES.
   - I. TRUSSES SHALL BE PERMANENTLY RESTRAINED AND BRACED IN A MANNER CONSISTENT WITH GOOD BUILDING PRACTICES AS OUTLINED IN BCSI AND IN ACCORDANCE WITH THE REQUIREMENTS OF THE CONSTRUCTION DOCUMENTS. TRUSSES SHALL FURTHERMORE BE ANCHORED OR RESTRAINED TO PREVENT OUT-OF-PLANE MOVEMENT SO AS TO KEEP ALL TRUSS MEMBERS FROM SIMULTANEOUSLY BUCKLING TOGETHER IN THE SAME DIRECTION. SUCH PERMANENT LATERAL RESTRAINT SHALL BE ACCOMPLISHED BY: (A) ANCHORAGE TO SOLID END WALLS; (B) PERMANENT DIAGONAL BRACING IN THE PLANE OF THE WEB MEMBERS; OR (C) OTHER SUITABLE MEANS.
   - J. MATERIALS USED IN TEMPORARY AND PERMANENT RESTRAINT AND BRACING SHALL BE FURNISHED BY CONTRACTOR.
5. MISCELLANEOUS
   - A. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED OR OTHERWISE ALTERED IN ANY WAY WITHOUT WRITTEN APPROVAL FROM THE TRUSS ENGINEER. WHERE FIELD REPAIRS OF DAMAGED TRUSSES ARE REQUIRED, THE TRUSS ENGINEER SHALL SUBMIT A SIGNED/SEALED TRUSS REPAIR PROFILE TO THE GENERAL CONTRACTOR PRIOR TO REPAIR INSTALLATION. THE GENERAL CONTRACTOR SHALL SUBMIT ALL SIGNED/SEALED TRUSS REPAIRS TO THE STRUCTURAL ENGINEER OF RECORD ON A MONTHLY BASIS FOR PROJECT RECORD (INFORMATIONAL PURPOSES ONLY).
   - B. TRUSS MANUFACTURER SHALL COORDINATE TRUSS LAYOUT WITH MECH / PLUMBING DRAWINGS TO ALLOW ALL PIPES AND DUCTS ADEQUATE SPACE FOR PROPER INSTALLATION. THE MANUFACTURER SHALL COORDINATE THE WEB MEMBER CONFIGURATION WITH THE MECHANICAL DRAWINGS AND ARCHITECTURAL DRAWINGS SUCH THAT ADEQUATE OPENING IS PROVIDED FOR ANY MECHANICAL UNITS AND DUCTS AND ACCESS CATWALKS. HARD DUCT LOCATIONS, AS INDICATED ON THE MECHANICAL DRAWINGS, SHALL BE SHOWN ON THE SUBMITTED TRUSS LAYOUT SHOP DRAWINGS FOR DESIGN TEAM REVIEW.
   - C. TRUSSES SHALL BE SPACED AS SHOWN ON THE PLANS; LESSER SPACING MAY BE USED IF REQUIRED BY THE TRUSS DESIGNER. HOWEVER, THE USE OF LESS TRUSS SPACING THAN INDICATED IN THE DESIGN DOCUMENTS SHALL NOT BE JUSTIFICATION FOR AN OWNER CHANGE ORDER. IT IS THE RESPONSIBILITY OF THE FRAMER AND THE TRUSS DESIGNER TO REVIEW PROJECT LOADING, REQUIRED SPANS AND TRUSS CONFIGURATIONS DURING THE BIDDING PROCESS.
   - D. IN ADDITION TO THE TYPICAL TRUSS LOADING PROVIDED HEREIN, TRUSS MANUFACTURER TO COORDINATE WITH MEP DRAWINGS FOR ADDITIONAL LOADS DUE TO WATER-FILLED PIPE SUPPORTS AND SPRINKLER PIPE SUPPORTS. TRUSS DESIGNER SHALL APPLY A MINIMUM 250 POUND CONCENTRATED LIVE LOAD TO ANY SINGLE FIRE SPRINKLER SUPPORT POINT TO ACCOMODATE INSTALLATION PERSONNEL PER NFPA 13 (NATIONAL FIRE PROTECTION ASSOCIATION) STANDARD FOR INSTALLATION. THE 250 POUND CONCENTRATED LOAD IS NOT REQUIRED TO BE APPLIED CONCURRENTLY TO OTHER CONCENTRATED SUPPORT POINTS AND NON-CURRENT WITH OTHER LIVE LOADS SUCH AS ROOF LIVE, FLOOR LIVE, SNOW, WIND, ETC.

VSM²
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WD8.0 WOOD FASTENING SCHEDULE:

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENERS	SPACING AND LOCATION
ROOF		
1. BLOCKING BETWEEN CEILING JOIST, RAFTERS, OR TRUSSES TO TOP PLATE OF OTHER FRAMING BELOW	(3) 8d COMMON (2 1/2" X 0.131"); OR (3) 10d BOX (3" X 0.128"); OR (3) 3" X 0.131" NAILS; OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	EACH END, TOENAIL
• BLOCKING BETWEEN RAFTERS OR TRUSS NOT AT THE WALL TOP PLATE, TO RAFTER OR TRUSS	(2) 8d COMMON (2 1/2" X 0.131"); OR (2) 3" X 0.131" NAILS; OR (3) 3" 14 GAGE STAPLES	EACH END, TOENAIL
	(2) 16d COMMON (3 1/2" X 0.162"); OR (2) 3" X 0.131" NAILS: OR (2) 3" 14 GAGE STAPLES	END NAIL
• FLAT BLOCKING TO TRUSS AND WEB FILLER	16d COMMON (3 1/2" X 0.162") @ 6"o.c. 3" X 0.131" NAILS @ 6"o.c. 3" 14 GAGE STAPLES @ 6"o.c.	FACE NAIL
2. CEILING JOIST TO TOP PLATE	(3) 8d COMMON (2 1/2" X 0.131"); OR (3) 10d BOX (3" X 0.128"); OR (3) 3" X 0.131" NAILS: OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	EACH JOIST, TOENAIL
3. CEILING JOIST ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS (NO THRUST)	(3) 16d COMMON (3 1/2" X 0.162"); OR (4) 10d BOX (3" X 0.128"); OR (4) 3" X 0.131" NAILS: OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	FACE NAIL
4. CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT)	PER TABLE	FACE NAIL
5. COLLAR TIE TO RAFTER	(3) 10d COMMON (3" X 0.148"); OR (4) 10d BOX (3" X 0.128"); OR (4) 3" X 0.131" NAILS: OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	FACE NAIL
6. RAFTER OR TRUSS TO TOP PLATE	(3) 10d COMMON (3" X 0.148"); OR (3) 16d BOX (3 1/2" X 0.135"); OR (4) 10d BOX (3" X 0.128" NAILS; OR (4) 3" X 0.131" NAILS; OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	TOENAIL[C]
7. ROOF RAFTER TO RIDGE VALLEY OR HIP RAFTERS; OR ROOF RAFTER TO 2 INCH RIDGE BEAM	(2) 16d COMMON (3 1/2" X 0.162"); OR (3) 10d BOX (3" X 0.128"); OR (3) 3" X 0.131" NAILS: OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	END NAIL
	(3) 10d COMMON (3" X 0.148"); OR (4) 16d BOX (3 1/2" X 0.135"); OR (4) 10d BOX (3" X 0.128" NAILS; OR (4) 3" X 0.131" NAILS; OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	TOENAIL
WALL		
8. STUD TO STUD (NOT AT BRACED WALL PANELS)	16d COMMON (3 1/2" X 0.162")	24" O.C. FACE NAIL
	10d BOX (3" X 0.128"); OR 3" X 0.131" NAILS: OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	16" O.C. FACE NAIL
9. STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	16d COMMON (3 1/2" X 0.162"); OR	16" O.C. FACE NAIL
	16d BOX (3 1/2" X 0.162"); OR	12" O.C. FACE NAIL
	3" X 0.131" NAILS: OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	12" O.C. FACE NAIL
10. BUILT-UP HEADER (2" TO 2" HEADER)	16d COMMON (3 1/2" X 0.162"); OR	16" O.C. EACH EDGE, FACE NAIL
	16d BOX (3 1/2" X 0.162"); OR	12" O.C. EACH EDGE, FACE NAIL
11. CONTINUOUS HEADER TO STUD	(4) 8d COMMON (2 1/2" X 0.131"); OR (4) 10d BOX (3" X 0.128"); OR	TOENAIL
12. TOP PLATE TO TOP PLATE	16d COMMON (3 1/2" X 0.162"); OR	16" O.C. FACE NAIL
	(4) 10d BOX (3" X 0.128" NAILS; OR (4) 3" X 0.131" NAILS; OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	12" O.C. FACE NAIL
13. TOP PLATE TO TOP PLATE, AT END JOINTS	(8) 16d BOX (3 1/2" X 0.135"); OR (12) 10d BOX (3" X 0.128" NAILS; OR (12) 3" X 0.131" NAILS; OR (12) 3" 14 GAGE STAPLES, 7/16" CROWN	EACH SIDE OF END JOIST, FACE NAIL (MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOIST)
14. BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (NOT AT BRACED WALL PANELS)	16d COMMON (3 1/2" X 0.162"); OR	16" O.C. FACE NAIL
	16d BOX (3 1/2" X 0.135"); OR 3" X 0.131" NAILS; OR 3" 14 GAGE STAPLES, 7/16" CROWN	12" O.C. FACE NAIL
15. BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING AT BRACED WALL PANELS	(2) 16d COMMON (3 1/2" X 0.162"); OR (3) 16d BOX (3 1/2" X 0.135"); OR (4) 3" X 0.131" NAILS; OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	16" O.C. FACE NAIL
16. STUD TO TOP OR BOTTOM PLATE	(4) 8d COMMON (2 1/2" X 0.131"); OR (4) 10d BOX (3" X 0.128"); OR (4) 3" X 0.131" NAILS; OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	TOENAIL
	(2) 16d COMMON (3 1/2" X 0.162"); OR (3) 10d BOX (3" X 0.128"); OR (3) 3" X 0.131" NAILS; OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	END NAIL
17. TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	(2) 16d COMMON (3 1/2" X 0.162"); OR (3) 10d BOX (3" X 0.128"); OR (3) 3" X 0.131" NAILS; OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	FACE NAIL
18. 1" BRACE TO EACH STUD AND PLATE	(2) 8d COMMON (2 1/2" X 0.131"); OR (2) 10d BOX (3" X 0.128"); OR (2) 3" X 0.131" NAILS; OR (2) 3" 14 GAGE STAPLES, 7/16" CROWN	FACE NAIL
19. 1" X 6" SHEATHING TO EACH BEARING	(2) 8d COMMON (2 1/2" X 0.131"); OR (2) 10d BOX (3" X 0.128")	FACE NAIL
20. 1" X 8" AND WIDER SHEATHING TO EACH BEARING	(2) 8d COMMON (2 1/2" X 0.131"); OR (2) 10d BOX (3" X 0.128")	FACE NAIL
21. JOIST TO SILL, TOP PLATE, OR GIRDER	(3) 8d COMMON (2 1/2" X 0.131"); OR FLR (3) 10d BOX (3" X 0.128"); OR (3) 3" X 0.131" NAILS: OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	TOENAIL
22. RIM JOIST, BAND JOIST, OR BLOCKING TO TOP PLATE, SILL, OR OTHER FRAMING BELOW	8d COMMON (2 1/2" X 0.131"); OR 10d BOX (3" X 0.128"); OR 3" X 0.131" NAILS: OR 3" 14 GAGE STAPLES, 7/16" CROWN	6" O.C. TOE NAIL

WD8.0 WOOD FASTENING SCHEDULE (CONT):

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENERS	SPACING AND LOCATION
FLOOR		
23. 1" X 6" SUBFLOOR OR LESS TO EACH JOIST	(2) 8d COMMON (2 1/2" X 0.131"); OR (2) 10d BOX (3" X 0.128")	FACE NAIL
24. 2" SUBFLOOR TO JOIST OR GIRDER	(2) 16d COMMON (3 1/2" X 0.162")	FACE NAIL
25. 2" PLANK (PLANK AND BEAM - FLOOR AND ROOF)	(2) 16d COMMON (3 1/2" X 0.162")	EACH BEARING, FACE NAIL
26. BUILT-UP GIRDERS AND BEAMS, 2" LUMBER LAYERS	20d COMMON (4" X 0.192")	32" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
	10d BOX (3" X 0.128"); OR 3" X 0.131" NAILS: OR 3" 14 GAGE STAPLES, 7/16" CROWN	24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
	AND: (2) 20d COMMON (4" X 0.192"); OR (3) 10d BOX (3" X 0.128"); OR (3) 3" X 0.131" NAILS: OR (3) 3" 14 GAGE STAPLES, 7/16" CROWN	ENDS AND AT EACH SPLICE, FACE NAIL
27. LEDGER STRIP SUPPORTING JOIST OR RAFTERS	(3) 16d COMMON (3 1/2" X 0.161"); OR (4) 10d BOX (3" X 0.128"); OR (4) 3" X 0.131" NAILS: OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	EACH JOIST OR RAFTER, FACE NAIL
28. JOIST TO BAND OR RIM JOIST	(3) 16d COMMON (3 1/2" X 0.161"); OR (4) 10d BOX (3" X 0.128"); OR (4) 3" X 0.131" NAILS: OR (4) 3" 14 GAGE STAPLES, 7/16" CROWN	END NAIL
29. BRIDGING OR BLOCKIN TO JOIST, RAFTER OR TRUSS	(2) 8d COMMON (2 1/2" X 0.131"); OR (2) 10d BOX (3" X 0.128"); OR (2) 3" X 0.131" NAILS: OR (2) 3" 14 GAGE STAPLES, 7/16" CROWN	EACH END, TOENAIL

WOOD STRUCTURAL PANELS (WSP), SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICALBOARD WALL SHEATHING TO FRAMING[a]

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENERS	EDGES (INCHES)	INTERMEDIATE SUPPORTS (INCHES)
30. 3/8" - 1/2" [h]	6d COMMON DEFORMED (2" X 0.113") (SUBFLOOR AND WALL)	6"	12"
	8d COMMON DEFORMED (2 1/2" X 0.113") (ROOF); OR RSRS-01 (2 3/8" X 0.113") NAIL (ROOF)[d]	6" [e]	6" [e]
	2 3/8" X 0.266" HEAD NAIL (ROOF)	4"	8"
	1 3/4" 16 GAGE STAPLE W/ 7/16" CROWN (SUBFLOOR AND WALL)	4"	8"
	2 3/8" X 0.113 NAIL (ROOF)	3" [f]	3" [f]
	1 3/4" 16 GAGE STAPLE W/ 7/16" CROWN (ROOF)	3" [f]	3" [f]
31. 19/32" - 3/4" [h]	8d COMMON (2 1/2" X 0.131"); OR 6d DEFORMED (2" X 0.113") (SUBFLOOR AND WALL)	6"	12"
	8d COMMON OR DEFORMED (2 1/2" X 0.131") (ROOF); OR RSRS-01 (2 3/8" X 0.113") NAIL (ROOF)[d]	6" [e]	6" [e]
	2 3/8" X 0.113 NAIL; OR 2" 16 GAGE STAPLE W/ 7/16" CROWN	4"	8"
32. 7/8" - 1 1/4" [h]	10d COMMON (3" X 0.148"); OR 8d DEFORMED (2 1/2" X 0.131")	6"	12"
OTHER EXTERIOR WALL SHEATHING			
33. 1/2" FIBERBOARD SHEATHING [b]	1 1/2" GALVANIZED ROOFING NAIL (7/16" HEAD DIAMETER); OR 1 1/4" 16 GAGE STAPLE WITH 7/16" OR 1" CROWN	3"	6"
34. 25/32" FIBERBOARD SHEATHING [b]	1 3/4" GALVANIZED ROOFING NAIL (7/16" HEAD DIAMETER); OR 1 1/2" 16 GAGE STAPLE WITH 7/16" OR 1" CROWN	3"	6"
WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING			
35. 3/4" AND LESS [h]	8d COMMON (2 1/2" X 0.131"); OR 6d DEFORMED (2" X 0.113")	6"	12"
36. 7/8" - 1" [h]	8d COMMON (2 1/2" X 0.131"); OR 8d DEFORMED (2 1/2" X 0.131")	6"	12"
37. 1 1/8" - 1 1/4" [h]	10d COMMON (3" X 0.148"); OR 8d DEFORMED (2 1/2" X 0.131")	6"	12"
PANEL SIDING TO FRAMING			
38. 1/2" OR LESS [h]	6d CORROSION-RESISTANT SIDING (1 7/8" X 0.106"); OR 6d CORROSION-RESISTANT CASING (2" X 0.099")	6"	12"
39. 5/8" [h]	8d CORROSION-RESISTANT SIDING (2 3/8" X 0.128"); OR 8d CORROSION-RESISTANT CASING (2 1/2" X 0.113")	6"	12"
INTERIOR PANELING			
40. 1/4"	4d CASING (1 1/2" X 0.080"); OR 4d FINISH (1 1/2" X 0.072")	6"	12"
41. 3/8"	6d CASING (2" X 0.099"); OR 6d FINISH (PANEL SUPPORTS AT 24")	6"	12"

NOTES:

a. NAILS SPACED AT 6 INCHES AT INTERMEDIATE SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO WOOD SHEAR WALL SCHEDULE AND NOTES. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE COMMON, BOX, OR CASING

b. SPACING SHALL BE 6 INCHES ON CENTER ON THE EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS FOR NON-STRUCTURAL APPLICATIONS. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS OTHERWISE MARKED)

c. WHERE A RAFTER IS FASTENED TO AN ADJACENT PARALLEL CEILING JOIST IN ACCORDANCE WITH THIS SCHEDULE, THE NUMBER OF TOENAILS IN THE RAFTER SHALL BE PERMITTED TO BE REDUCED BY ONE NAIL.

d. RSRS-01 IS A SHEATHING RING SHANK NAIL MEETING THE SPECIFICATIONS IN ASTM F1667.

e. TABULATED FASTENER REQUIREMENTS APPLY WHERE THE ULTIMATE DESIGN WIND SPEED IS LESS THAN 140 MPH. FOR WOOD STRUCTURAL PANEL ROOF SHEATHING ATTACHED TO GABLE END ROOF FRAMING AND TO INTERMEDIATE SUPPORTS WITHIN 48 INCHES OF ROOF EDGES AND RIDGES, NAILS SHALL BE SPACED AT 4 INCHES ON CENTER WHERE THE ULTIMATE WIND SPEED IS GREATER THAN 130 MPH IN EXPOSURE B OR GREATER THAN 110 MPH IN EXPOSURE C. SPACING EXCEEDING 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS SHALL BE PERMITTED WHERE THE FASTENING IS DESIGNED PER THE AWS NDS.

f. FASTENING IS ONLY PERMITTED WHERE THE ULTIMATE DESIGN WIND SPEED IS LESS THAN OR EQUAL TO 110 MPH

g. NAILS AND STAPLES ARE CARBON STEEL MEETING THE SPECIFICATIONS OF ASTM F1667. CONNECTIONS USING NAILS AND STAPLES OF OTHER MATERIALS, SUCH AS STAINLESS STEEL, SHALL BE DESIGNED BY ACCEPTABLE ENGINEERING PRACTICE APPROVED UNDER SECTION 104.11.

h. SEE WOOD CONSTRUCTION GENERAL NOTES SECTIONS C, D, AND E FOR MINIMUM NAIL SIZES TO BE USED AT FLOOR SHEATHING (TO FRAMING), ROOF SHEATHING (TO FRAMING), AND SHEAR WALL SHEATHING (TO FRAMING), RESPECTIVELY. SEE PLANS FOR HIGH LOAD DIAPHRAGM NAILING AND CORRESPONDING BLOCKING WHERE OCCURS.

WD5.0 CONTINUOUS ROD TIE-DOWN SYSTEM (HOLDOWN) GENERAL NOTES:

A. THE CONTINUOUS ROD TIEDOWN SYSTEM SPECIFIED AND SCHEDULED FOR THIS PROJECT IS BASED ON THE SIMPSON STRONG-TIE STRONG-ROD ANCHOR TIEDOWN SYSTEM (ATS) OR APPROVED EQUAL FOR SHEAR WALL OVERTURNING RESTRAINT.

B. THE TOTAL VERTICAL DISPLACEMENT BETWEEN RESTRAINT LOCATIONS, INCLUDING STEEL ROD ELONGATION AND SHRINKAGE COMPENSATING DEVICE DEFLECTION, IS CALCULATED TO BE LESS THAN 0.20 INCHES USING ALLOWABLE STRESS DESIGN (ASD).

C. THE ATS IS BE RESTRAINED BY A BEARING PLATE AND TAKE-UP DEVICE ASSEMBLY AT EACH STORY OF THE MULTI-STORY SHEAR WALLS.

D. DO NOT WELD PRODUCTS UNLESS THE ATS INSTALLATION DETAILS SPECIFICALLY IDENTIFY A PRODUCT AS ACCEPTABLE FOR WELDING. RODS, NUTS, AND COUPLER NUTS SHALL NOT BE WELDED.

E. IN THE EVENT OF A DISCREPANCY BETWEEN THESE STRUCTURAL DRAWINGS AND THE ATS INSTALLATION DETAILS, THESE STRUCTURAL DRAWINGS SHALL GOVERN.

F. THE STRUCTURAL DRAWINGS ARE SPECIFIC TO THE SIMPSON STRONG-TIE STRONG-ROD ANCHOR TIEDOWN SYSTEM (ATS) AND ARE NOT APPLICABLE TO OTHER MANUFACTURERS' CONTINUOUS ROD TIEDOWN SYSTEMS. PROPOSED SUBSTITUTIONS OF OTHER MANUFACTURERS' CONTINUOUS ROD TIEDOWN SYSTEMS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL. SUBMITTAL SHALL INCLUDE EVALUATION REPORTS INDICATING COMPLIANCE WITH GOVERNING BUILDING CODES AND TEST DATA DEEMED ACCEPTABLE TO THE BUILDING OFFICIAL. IN ADDITION, SUBMITTAL SHALL INCLUDE INSTALLATION DETAILS AND INSTRUCTIONS, CALCULATIONS IN ACCORDANCE WITH THE GOVERNING BUILDING CODES, AND CERTIFICATION BY THE MANUFACTURER OF COMPLIANCE WITH THESE ATS SPECIFICATIONS AND THE STRUCTURAL DRAWINGS.

G. ATS RUN START / TERMINATIONS SHALL BE AS SET FORTH ON THESE STRUCTURAL DRAWINGS. ALTERNATE RUN START / TERMINATIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO CONCRETE PLACEMENT. SUBMITTAL SHALL INCLUDE CALCULATIONS IN COMPLIANCE WITH THE GOVERNING BUILDING CODE, INCLUDING CONCRETE ANCHORAGE IN ACCORDANCE WITH ACI 318 PROVISIONS FOR STRENGTH DESIGN AND CONVERSION TO ASD LOAD LEVELS.

H. THE ATS IS DESIGNED TO BE INSTALLED FLOOR BY FLOOR AS THE STRUCTURE IS BUILT. INSTALLATION IN THIS MANNER, WITH SHEAR WALLS, WILL PROVIDE A LATERAL-FORCE-RESISTING SYSTEM DURING CONSTRUCTION. THE DESIGN AND EXPENSE OF ALTERNATIVE METHODS OF TEMPORARY LATERAL-FORCE-RESISTING SYSTEMS ARE THE RESPONSIBILITY OF THE CONTRACTOR.

I. A PRE-CONSTRUCTION MEETING IS RECOMMENDED WITH SIMPSON STRONG-TIE PRIOR TO PLACEMENT OF THE CONCRETE. THE PURPOSE OF THIS MEETING IS TO ASSIST IN VERIFYING QUANTITIES AND UNDERSTANDING THE INSTALLATION PROCESS. TO COORDINATE THIS MEETING, CALL SIMPSON STRONG-TIE AT (800) 999-5099.

J. RTUD AND ATUD BEARING PLATES SHALL BE PROVIDED BY SIMPSON STRONG-TIE BASED ON SCHEDULED WALL WIDTH AND MAXIMUM TAKE UP DEVICE TENSION CAPACITY AT EACH LEVEL.

WD5.1 CONTINUOUS ROD ROOF UPLIFT RESTRAINT SYSTEM GENERAL NOTES:

A. THE CONTINUOUS ROD ROOF UPLIFT RESTRAINT SYSTEM SPECIFIED AND SCHEDULED FOR THIS PROJECT IS BASED ON THE SIMPSON STRONG-TIE "STRONG-ROD UPLIFT RESTRAINT SYSTEM FOR ROOFS" (URS) OR APPROVED EQUAL.

B. THE URS IS BE RESTRAINED BY A BEARING PLATE AND TAKE-UP DEVICE ASSEMBLY AT THE ROOF.

C. DO NOT WELD PRODUCTS UNLESS THE URS INSTALLATION DETAILS SPECIFICALLY IDENTIFY A PRODUCT AS ACCEPTABLE FOR WELDING. RODS, NUTS, AND COUPLER NUTS SHALL NOT BE WELDED.

D. IN THE EVENT OF A DISCREPANCY BETWEEN THESE STRUCTURAL DRAWINGS AND THE URS INSTALLATION DETAILS, THESE STRUCTURAL DRAWINGS SHALL GOVERN.

E. THE STRUCTURAL DRAWINGS ARE SPECIFIC TO THE SIMPSON STRONG-TIE STRONG-ROD UPLIFT RESTRAINT SYSTEM FOR ROOFS (URS) AND ARE NOT APPLICABLE TO OTHER MANUFACTURERS' CONTINUOUS ROD UPLIFT RESTRAINT SYSTEMS. PROPOSED SUBSTITUTIONS OF OTHER MANUFACTURERS' CONTINUOUS ROD UPLIFT RESTRAINT SYSTEMS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL. SUBMITTAL SHALL INCLUDE EVALUATION REPORTS INDICATING COMPLIANCE WITH GOVERNING BUILDING CODES AND TEST DATA DEEMED ACCEPTABLE TO THE BUILDING OFFICIAL. IN ADDITION, SUBMITTAL SHALL INCLUDE INSTALLATION DETAILS AND INSTRUCTIONS, CALCULATIONS IN ACCORDANCE WITH THE GOVERNING BUILDING CODES, AND CERTIFICATION BY THE MANUFACTURER OF COMPLIANCE WITH THESE URS SPECIFICATIONS AND THE STRUCTURAL DRAWINGS.

F. URS RUN START/TERMINATIONS SHALL BE AS SET FORTH ON THESE STRUCTURAL DRAWINGS. ALTERNATE RUN START/TERMINATIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO CONCRETE PLACEMENT. SUBMITTAL SHALL INCLUDE CALCULATIONS IN COMPLIANCE WITH THE GOVERNING BUILDING CODE, INCLUDING CONCRETE ANCHORAGE IN ACCORDANCE WITH ACI 318 PROVISIONS FOR STRENGTH DESIGN AND CONVERSION TO ASD LOAD LEVELS.

G. A PRE-CONSTRUCTION MEETING IS RECOMMENDED WITH SIMPSON STRONG-TIE PRIOR TO PLACEMENT OF THE CONCRETE. THE PURPOSE OF THIS MEETING IS TO ASSIST IN VERIFYING QUANTITIES AND UNDERSTANDING THE INSTALLATION PROCESS. TO COORDINATE THIS MEETING, CALL SIMPSON STRONG-TIE AT (800) 999-5099.

WD6.0 TYPE IIIA WOOD CONSTRUCTION GENERAL NOTES:

A. PER CHAPTER 6 OF THE REFERENCE BUILDING CODE, ALL EXTERIOR WALLS SHALL BE CONSTRUCTED OF NON-COMBUSTIBLE MATERIALS. ALL EXTERIOR WALLS, AND ALL MEMBERS THAT PENETRATE EXTERIOR WALLS, BOTH NON-LOAD-BEARING AND LOAD-BEARING, SHALL BE FRAMED WITH FIRE-RETARDANT-TREATED (FRT) WOOD INCLUDING, BUT NOT LIMITED TO STUD MEMBERS, POSTS, HEADERS, FLUSH BEAMS, AND TRUSS ENDS.

B. ACCEPTABLE FIRE-RETARDANT-TREATED (FRT) WOOD PRODUCTS ARE AS FOLLOWS:
   a. 'DRICON' BY EXTERIOR WOOD, INC. (ICC ESR-1626)
   b. 'PYROGUARD' BY HOOVER TREATED WOOD PRODUCTS, INC. (ICC ESR-1791)
   c. 'D-BLAZE' BY VIANCE TREATED WOOD SOLUTIONS (ICC ESR-2645)

IF THE CONTRACTOR SUBSTITUTES A FIRE-RETARDANT PRODUCT OTHER THAN ONE OF THE OPTIONS LISTED ABOVE, CONTRACTOR SHALL SUBMIT MANUFACTURER PRODUCT INFORMATION INCLUDING ICC REPORT AND LUMBER DESIGN VALUE MODIFICATIONS WHICH IF LESS THAN LISTED IN TABLE BELOW, PROPOSED PRODUCT SUBSTITUTION MAY BE REJECTED.

FRT LUMBER REDUCTIONS

DESIGN PARAMETER	MODIFIER
COMPRESSION PARALLEL TO GRAIN	0.91
HORIZONTAL SHEAR	0.93
TENSION PARALLEL TO GRAIN	0.83
MODULUS OF ELASTICITY	0.94
EXTREME FIBER STRESS (BENDING)	0.88
COMPRESSION PERPENDICULAR TO GRAIN	0.95

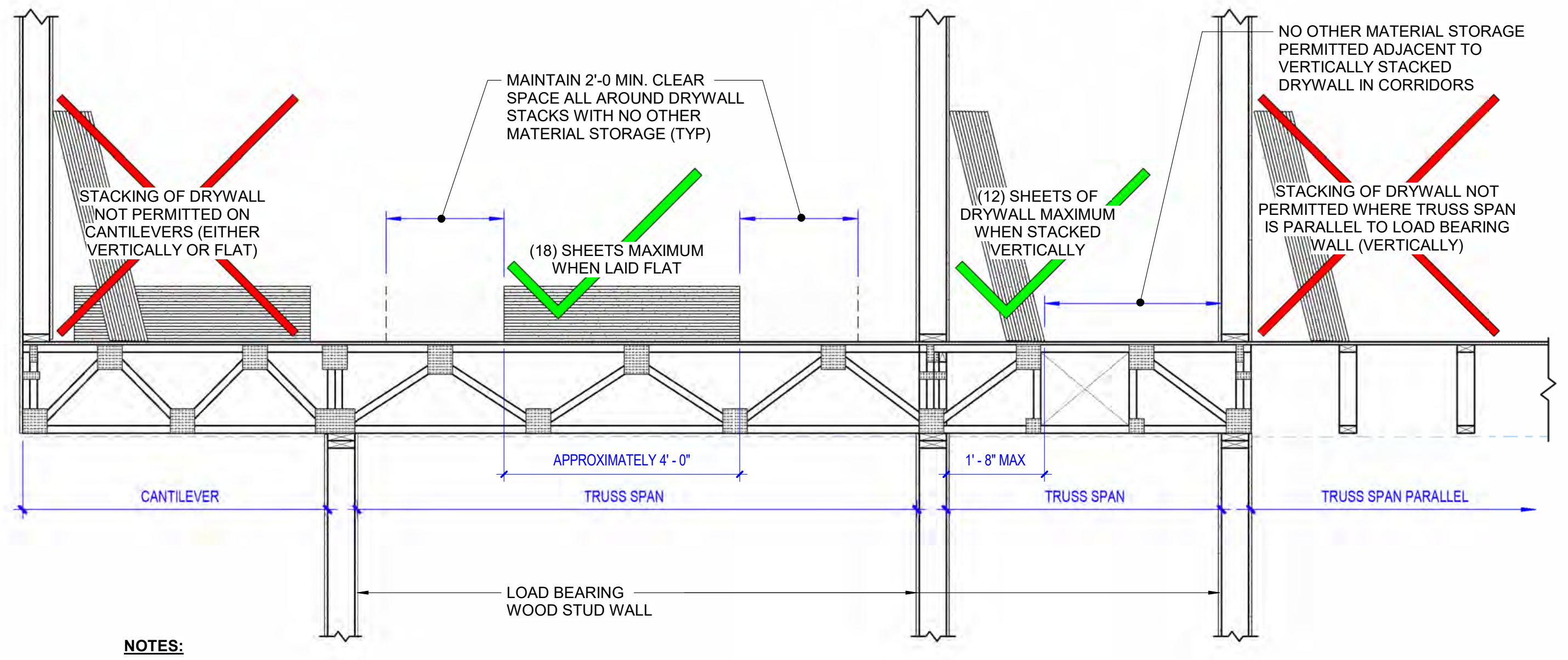
C. EXTERIOR FIRE-RETARDANT WALL SHEATHING AT EXTERIOR RATED WALLS SHALL BE AN APPROVED FIRE-TREATED PLYWOOD OR OSB PRODUCT IN CONFORMANCE WITH THE WALL ASSEMBLY SPECIFIED BY THE ARCHITECT.

D. AT EXTERIOR UN-RATED WALL LOCATIONS, THE EXTERIOR FIRE-RETARDANT WALL SHEATHING SHALL MEET CHAPTER 6 REQUIREMENTS OF THE REFERENCE BUILDING WITH RESPECT TO FIRE-RETARDANT-TREATED WOOD FRAMING.

E. STRUCTURAL DESIGN SHOWN IN THESE STRUCTURAL DRAWINGS HAS ASSUMED THE USE OF LP FLAMEBLOCK, BLAZEGUARD, OR EQUIVALENT, 2-SIDE PYROTITE-COATED STRUCTURAL SHEATHING PANELS AS THE BASIS OF DESIGN. THE GENERAL CONTRACTOR SHALL SUBMIT THE PROPOSED EXTERIOR FIRE-RETARDANT SHEATHING PRODUCT(S) TO THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL.

F. WHERE FIRE-RETARDANT TREATED (FRT) LUMBER IS EXPOSED TO WEATHER OR TO CONDITIONS THAT WILL RESULT IN THE MEMBER POTENTIALLY BEING DAMP OR WET (I.E. BALCONY FRAMING), THE LUMBER MATERIALS SHALL BE IDENTIFIED AS "EXTERIOR" TO INDICATE NO INCREASE IN LISTED FLAME SPREAD INDEX WHEN SUBJECTED TO TESTING PER ASTM D2898 AS DEFINED IN SECTION 2302.2 0F THE REFERENCE BUILDING CODE.

G. WHERE STRUCTURAL STEEL TRANSFER FRAMING IS EMBEDDED WITHIN THE WOOD-FRAMED BUILDING SUPERSTRUCTURE, THE GENERAL CONTRACTOR SHALL SUBMIT A DETAILED FINISH CONTROL JOINT LAYOUT PLAN TO THE ARCHITECT FOR REVIEW TO ENSURE DIFFERENTIAL VERTICAL MOVEMENT BETWEEN THE STEEL FRAMING AND WOOD-FRAMED BUILDING COMPONENTS HAS BEEN APPROPRIATELY ACCOUNTED FOR.


MAINTAIN 2'-0 MIN. CLEAR SPACE ALL AROUND DRYWALL STACKS WITH NO OTHER MATERIAL STORAGE (TYP)
NO OTHER MATERIAL STORAGE PERMITTED ADJACENT TO VERTICALLY STACKED DRYWALL IN CORRIDORS
STACKING OF DRYWALL NOT PERMITTED ON CANTILEVERS (EITHER VERTICALLY OR FLAT)
(18) SHEETS MAXIMUM WHEN LAID FLAT
(12) SHEETS OF DRYWALL MAXIMUM WHEN STACKED VERTICALLY
STACKING OF DRYWALL NOT PERMITTED WHERE TRUSS SPAN IS PARALLEL TO LOAD BEARING WALL (VERTICALLY)
APPROXIMATELY 4' - 0"
1' - 8" MAX
CANTILEVER
TRUSS SPAN
TRUSS SPAN
TRUSS SPAN PARALLEL
LOAD BEARING WOOD STUD WALL

NOTES:

1. DRYWALL SUPPLIER SHALL SUBMIT MATERIAL STOCKING PLAN TO GENERAL CONTRACTOR FOR REVIEW AND APPROVAL PRIOR TO DELIVERY.
2. DRYWALL STACKING DURING CONSTRUCTION IS PERMITTED ONLY AS SHOWN IN ABOVE DETAIL.
3. WHEN STACKED VERTICALLY, NO MORE THAN 12 SHEETS MAY BE STACKED TOGETHER AND MAY BEAR ON THE DECK NO MORE THAN 1'-8 FROM LOAD BEARING WALL. STACK SHEETS ON THEIR LONG END (SHORT SIDE UP).
4. WHEN LAID FLAT, NO MORE THAN 18 SHEETS MAY BE STACKED TOGETHER.
5. DRYWALL STACKS SHALL NOT BE DROPPED ONTO DECK. DECK IS NOT RATED FOR HEAVY IMPACT LOADS, AND STRUCTURAL DAMAGE MAY OCCUR FROM DROPPING EVEN PARTIAL DRYWALL STACKS.
6. SLEEPERS ARE NOT PERMITTED UNDER DRYWALL STACKS. STACKS SHALL BEAR DIRECTLY ON DECK SUCH THAT THEIR WEIGHT IS DISTRIBUTED EQUALLY ALONG THEIR LENGTH.
7. DRYWALL STACKS ARE NOT PERMITTED ON CANTILEVERS AND SHALL NOT OVERHANG THE BEARING WALL.
8. MAINTAIN A MINIMUM OF 2'-0 CLEAR SPACE AROUND DRYWALL STACKS WITH NO OTHER MATERIAL STORAGE. NO OTHER MATERIAL STORAGE IS PERMITTED ADJACENT TO VERTICALLY STACKED DRYWALL IN CORRIDORS.

① Drywall Stacking Over Load Bearing Wood Stud Walls
1/2" = 1'-0"

WD7.0 WOOD SHRINKAGE REQUIREMENTS:

THE FOLLOWING ARE REQUIRED (GENERAL CONTRACTOR TO COORDINATE WITH ALL TRADES AS REQUIRED):

A. THE VERITCAL WOOD FRAMING SHOWN IN THE DRAWINGS IS ANTICIPATED TO SHRINK OVER TIME. LIKEWISE, EXTERIOR MASONRY FACADE (WHEN APPLICABLE) MAY EXPAND DUE TO SEASONAL TEMPERATURE EFFECTS. THESE DIFFERENTIAL MOVEMENTS MUST BE ACCOUNTED FOR TO PREVENT DAMAGE TO RIGID FINISHES AND NON-STRUCTURAL ITEMS.

B. DETAILING AND CONSTRUCTION OF MECHANICAL SYSTEMS AND ARCHITECTURAL FINISHES (INCLUDING BUT NOT LIMITED TO MASONRY FACADE, STONE FACADE, VENEERS, DRYWALL SHEATHING, PLYWOOD SHEATHING, WINDOWS, DOOR THRESHHOLDS, OTHER BRITTLE FINISHES, AND MECHANICAL/ELECTRICAL. PLUMBING LINES AND ROOF DRAINAGE SYSTEMS) SHALL ACCOUNT FOR ESTIMATED DIFFERENTIAL MOVEMENT (WOOD SHRINKAGE + BRICK EXPANSION) AS INDICATED IN THE FOLLOWING TABLES WHERE '1ST FLOOR' REPRESENTS LOWEST WOOD FRAMED LEVEL:

ESTIMATED MAXIMUM WOOD SHRINKAGE [DOWNWARD]	
5TH FLOOR	1 1/4"
4TH FLOOR	1"
3RD FLOOR	3/4"
2ND FLOOR	1/2"
1ST FLOOR	1/4"

ESTIMATED MAXIMUM BRICK EXPANSION [UPWARD] (WHEN APPLICABLE)	
5TH FLOOR	5/8"
4TH FLOOR	1/2"
3RD FLOOR	3/8"
2ND FLOOR	1/4"
1ST FLOOR	1/8"

C. MASONRY LATERAL TIES SHALL ALLOW FOR FULL FREE DIFFERENTIAL MOVEMENTS DEFINED ABOVE.

D. ROUGH OPENINGS IN WOOD FRAMINGS SHALL BE OVERSIZED VERTICALLY TO PROVIDE A MINIMUM 1" ADDITIONAL CLEARANCE BETWEEN TOP OF NON-STRUCTURAL PASS THROUGH ELEMENT AND TOP OF HOLE.

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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No.	Description	Date
	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA
BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH
LV COLLECTIVE

DOCUMENTS ARE NOT TO BE USED FOR PRICING OR CONSTRUCTION PURPOSES. DOCUMENTS ARE ISSUED FOR THE PURPOSES OF:
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DOCUMENTS ARE ISSUED UNDER THE AUTHORITY OF:
- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
General Notes - Wood

SHEET NUMBER:
S0.201

NOT RELEASED FOR CONSTRUCTION

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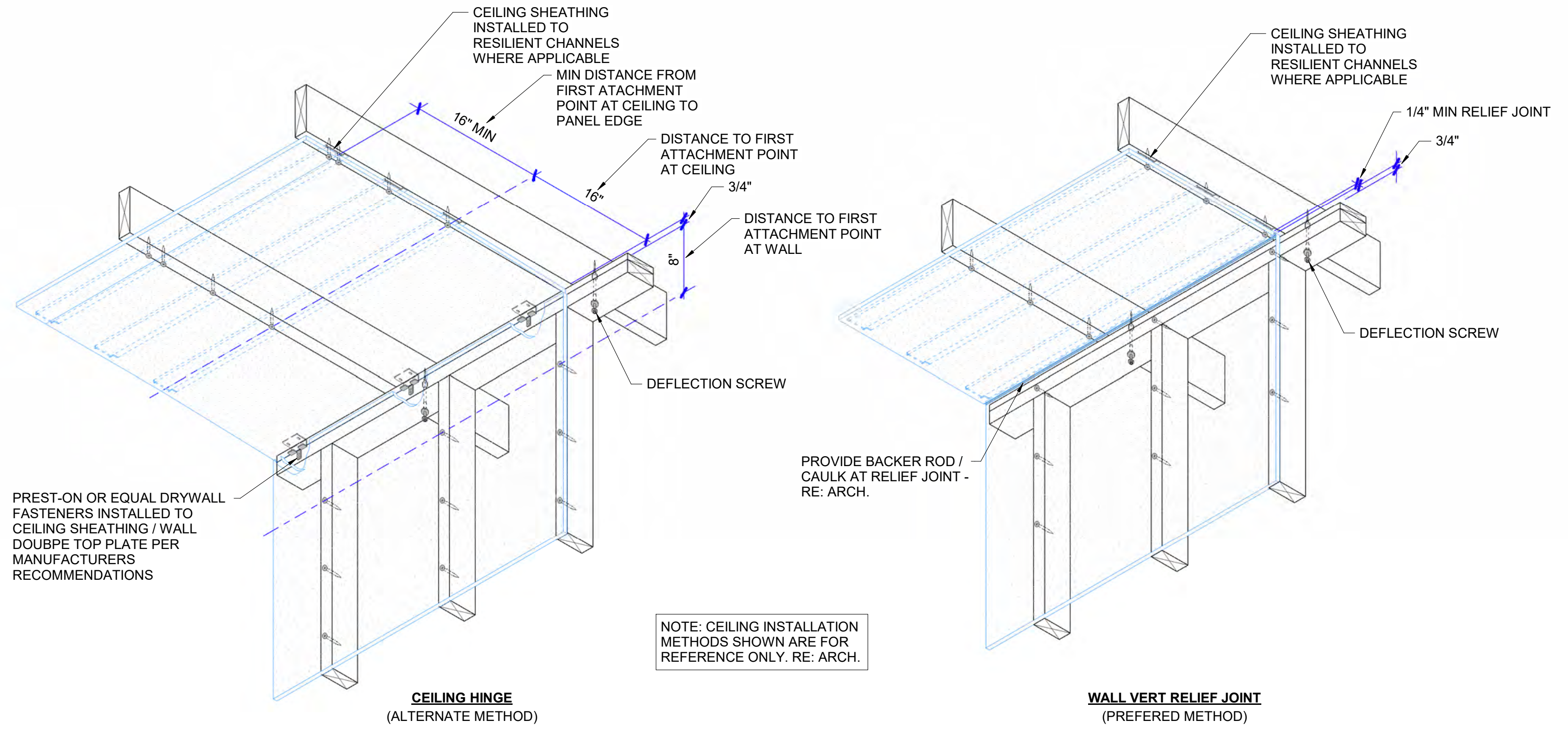
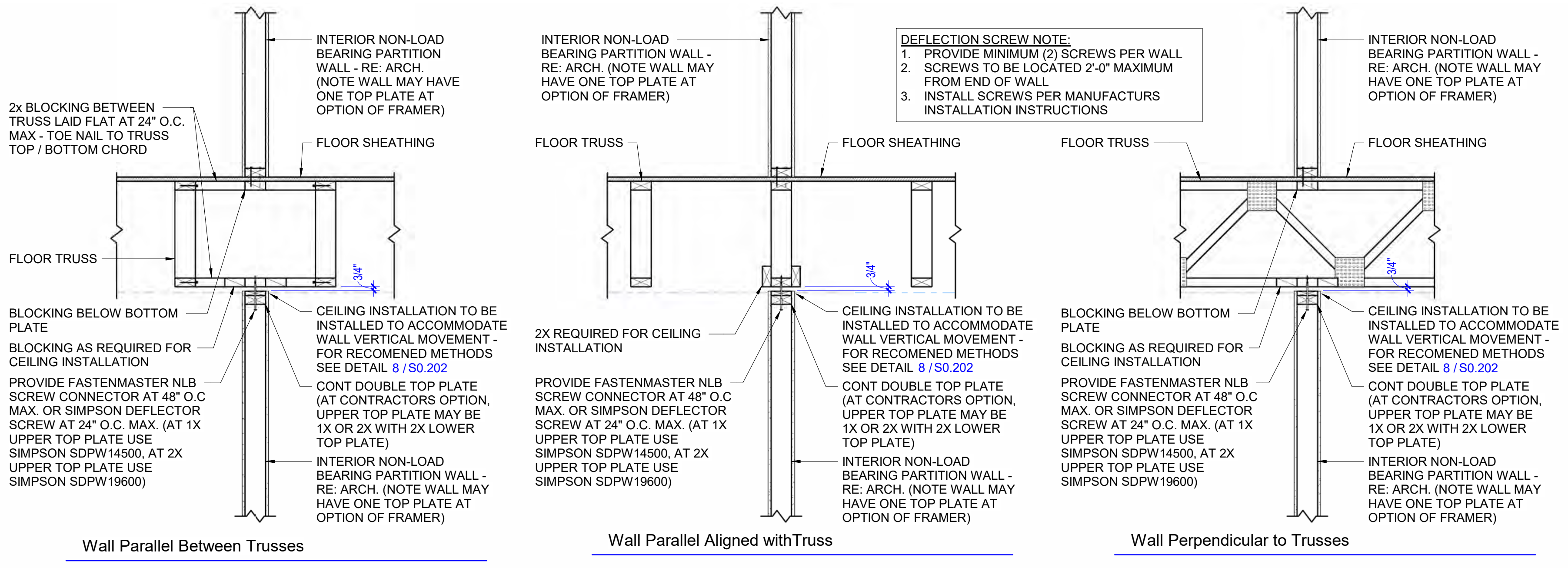

CEILING SHEATHING INSTALLED TO RESILIENT CHANNELS WHERE APPLICABLE
MIN DISTANCE FROM FIRST ATACHMENT POINT AT CEILING TO PANEL EDGE
16" MIN
DISTANCE TO FIRST ATTACHMENT POINT AT CEILING
16"
3/4"
DISTANCE TO FIRST ATTACHMENT POINT AT WALL
8"
DEFLECTION SCREW
PREST-ON OR EQUAL DRYWALL FASTENERS INSTALLED TO CEILING SHEATHING / WALL DOUBPE TOP PLATE PER MANUFACTURERS RECOMMENDATIONS
CEILING SHEATHING INSTALLED TO RESILIENT CHANNELS WHERE APPLICABLE
1/4" MIN RELIEF JOINT
3/4"
DEFLECTION SCREW
PROVIDE BACKER ROD / CAULK AT RELIEF JOINT - RE: ARCH.

NOTE: CEILING INSTALLATION METHODS SHOWN ARE FOR REFERENCE ONLY. RE: ARCH.

CEILING HINGE
(ALTERNATE METHOD)

WALL VERT RELIEF JOINT
(PREFERED METHOD)

8 Ceiling Sheathing at Non-Load Bearing Wall
1" = 1'-0"


INTERIOR NON-LOAD BEARING PARTITION WALL - RE: ARCH. (NOTE WALL MAY HAVE ONE TOP PLATE AT OPTION OF FRAMER)
2x BLOCKING BETWEEN TRUSS LAID FLAT AT 24" O.C. MAX - TOE NAIL TO TRUSS TOP / BOTTOM CHORD
FLOOR SHEATHING
FLOOR TRUSS
3/4"
BLOCKING BELOW BOTTOM PLATE
BLOCKING AS REQUIRED FOR CEILING INSTALLATION
PROVIDE FASTENMASTER NLB SCREW CONNECTOR AT 48" O.C MAX. OR SIMPSON DEFLECTOR SCREW AT 24" O.C. MAX. (AT 1X UPPER TOP PLATE USE SIMPSON SDPW14500, AT 2X UPPER TOP PLATE USE SIMPSON SDPW19600)
CEILING INSTALLATION TO BE INSTALLED TO ACCOMMODATE WALL VERTICAL MOVEMENT - FOR RECOMENED METHODS SEE DETAIL 8 / S0.202
CONT DOUBLE TOP PLATE (AT CONTRACTORS OPTION, UPPER TOP PLATE MAY BE 1X OR 2X WITH 2X LOWER TOP PLATE)
INTERIOR NON-LOAD BEARING PARTITION WALL - RE: ARCH. (NOTE WALL MAY HAVE ONE TOP PLATE AT OPTION OF FRAMER)
INTERIOR NON-LOAD BEARING PARTITION WALL - RE: ARCH. (NOTE WALL MAY HAVE ONE TOP PLATE AT OPTION OF FRAMER)
FLOOR TRUSS
FLOOR SHEATHING
3/4"
2X REQUIRED FOR CEILING INSTALLATION
PROVIDE FASTENMASTER NLB SCREW CONNECTOR AT 48" O.C MAX. OR SIMPSON DEFLECTOR SCREW AT 24" O.C. MAX. (AT 1X UPPER TOP PLATE USE SIMPSON SDPW14500, AT 2X UPPER TOP PLATE USE SIMPSON SDPW19600)
CEILING INSTALLATION TO BE INSTALLED TO ACCOMMODATE WALL VERTICAL MOVEMENT - FOR RECOMENED METHODS SEE DETAIL 8 / S0.202
CONT DOUBLE TOP PLATE (AT CONTRACTORS OPTION, UPPER TOP PLATE MAY BE 1X OR 2X WITH 2X LOWER TOP PLATE)
INTERIOR NON-LOAD BEARING PARTITION WALL - RE: ARCH. (NOTE WALL MAY HAVE ONE TOP PLATE AT OPTION OF FRAMER)
INTERIOR NON-LOAD BEARING PARTITION WALL - RE: ARCH. (NOTE WALL MAY HAVE ONE TOP PLATE AT OPTION OF FRAMER)
FLOOR TRUSS
FLOOR SHEATHING
3/4"
BLOCKING BELOW BOTTOM PLATE
BLOCKING AS REQUIRED FOR CEILING INSTALLATION
PROVIDE FASTENMASTER NLB SCREW CONNECTOR AT 48" O.C MAX. OR SIMPSON DEFLECTOR SCREW AT 24" O.C. MAX. (AT 1X UPPER TOP PLATE USE SIMPSON SDPW14500, AT 2X UPPER TOP PLATE USE SIMPSON SDPW19600)
CEILING INSTALLATION TO BE INSTALLED TO ACCOMMODATE WALL VERTICAL MOVEMENT - FOR RECOMENED METHODS SEE DETAIL 8 / S0.202
CONT DOUBLE TOP PLATE (AT CONTRACTORS OPTION, UPPER TOP PLATE MAY BE 1X OR 2X WITH 2X LOWER TOP PLATE)
INTERIOR NON-LOAD BEARING PARTITION WALL - RE: ARCH. (NOTE WALL MAY HAVE ONE TOP PLATE AT OPTION OF FRAMER)

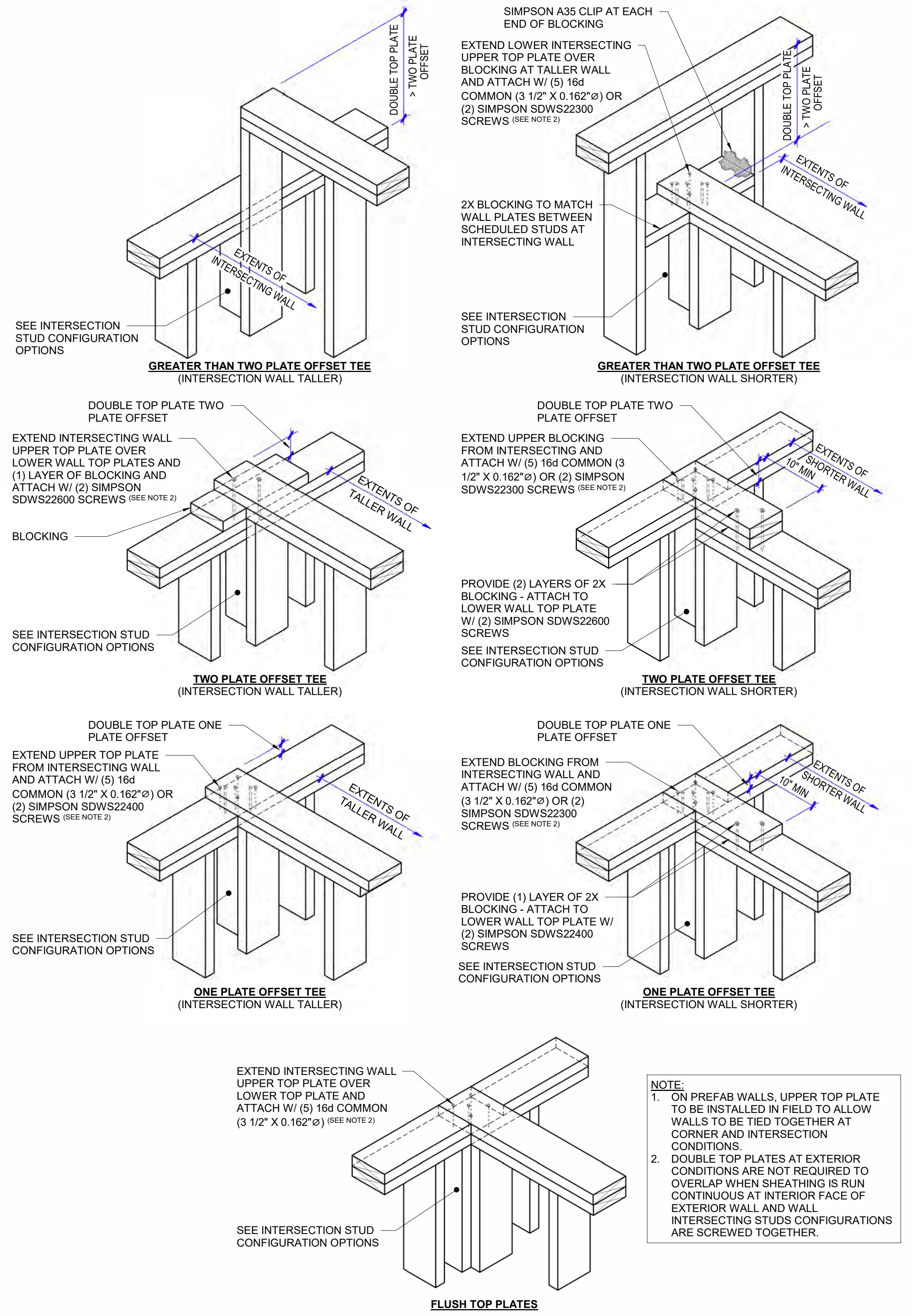
DEFLECTION SCREW NOTE:
1. PROVIDE MINIMUM (2) SCREWS PER WALL
2. SCREWS TO BE LOCATED 2'-0" MAXIMUM FROM END OF WALL
3. INSTALL SCREWS PER MANUFACTURS INSTALLATION INSTRUCTIONS

Wall Parallel Between Trusses

Wall Parallel Aligned withTruss

Wall Perpendicular to Trusses

7 Non Load Bearing Wall Connection
3/4" = 1'-0"


DOUBLE TOP PLATE
> TWO PLATE OFFSET
EXTENTS OF INTERSECTING WALL
SEE INTERSECTION STUD CONFIGURATION OPTIONS
SIMPSON A35 CLIP AT EACH END OF BLOCKING
EXTEND LOWER INTERSECTING UPPER TOP PLATE OVER BLOCKING AT TALLER WALL AND ATTACH W/ (5) 16d COMMON (3 1/2" X 0.162"Ø) OR (2) SIMPSON SDWS22300 SCREWS (SEE NOTE 2)
DOUBLE TOP PLATE
> TWO PLATE OFFSET
EXTENTS OF INTERSECTING WALL
2X BLOCKING TO MATCH WALL PLATES BETWEEN SCHEDULED STUDS AT INTERSECTING WALL
SEE INTERSECTION STUD CONFIGURATION OPTIONS
DOUBLE TOP PLATE TWO PLATE OFFSET
EXTEND INTERSECTING WALL UPPER TOP PLATE OVER LOWER WALL TOP PLATES AND (1) LAYER OF BLOCKING AND ATTACH W/ (2) SIMPSON SDWS22600 SCREWS (SEE NOTE 2)
EXTENTS OF TALLER WALL
BLOCKING
SEE INTERSECTION STUD CONFIGURATION OPTIONS
DOUBLE TOP PLATE TWO PLATE OFFSET
EXTEND UPPER BLOCKING FROM INTERSECTING AND ATTACH W/ (5) 16d COMMON (3 1/2" X 0.162"Ø) OR (2) SIMPSON SDWS22300 SCREWS (SEE NOTE 2)
EXTENTS OF SHORTER WALL
10" MIN
PROVIDE (2) LAYERS OF 2X BLOCKING - ATTACH TO LOWER WALL TOP PLATE W/ (2) SIMPSON SDWS22600 SCREWS
SEE INTERSECTION STUD CONFIGURATION OPTIONS
DOUBLE TOP PLATE ONE PLATE OFFSET
EXTEND UPPER TOP PLATE FROM INTERSECTING WALL AND ATTACH W/ (5) 16d COMMON (3 1/2" X 0.162"Ø) OR (2) SIMPSON SDWS22400 SCREWS (SEE NOTE 2)
EXTENTS OF TALLER WALL
SEE INTERSECTION STUD CONFIGURATION OPTIONS
DOUBLE TOP PLATE ONE PLATE OFFSET
EXTEND BLOCKING FROM INTERSECTING WALL AND ATTACH W/ (5) 16d COMMON (3 1/2" X 0.162"Ø) OR (2) SIMPSON SDWS22300 SCREWS (SEE NOTE 2)
EXTENTS OF SHORTER WALL
10" MIN
PROVIDE (1) LAYER OF 2X BLOCKING - ATTACH TO LOWER WALL TOP PLATE W/ (2) SIMPSON SDWS22400 SCREWS
SEE INTERSECTION STUD CONFIGURATION OPTIONS
EXTEND INTERSECTING WALL UPPER TOP PLATE OVER LOWER TOP PLATE AND ATTACH W/ (5) 16d COMMON (3 1/2" X 0.162"Ø) (SEE NOTE 2)
SEE INTERSECTION STUD CONFIGURATION OPTIONS

GREATER THAN TWO PLATE OFFSET TEE
(INTERSECTION WALL TALLER)

GREATER THAN TWO PLATE OFFSET TEE
(INTERSECTION WALL SHORTER)

TWO PLATE OFFSET TEE
(INTERSECTION WALL TALLER)

TWO PLATE OFFSET TEE
(INTERSECTION WALL SHORTER)

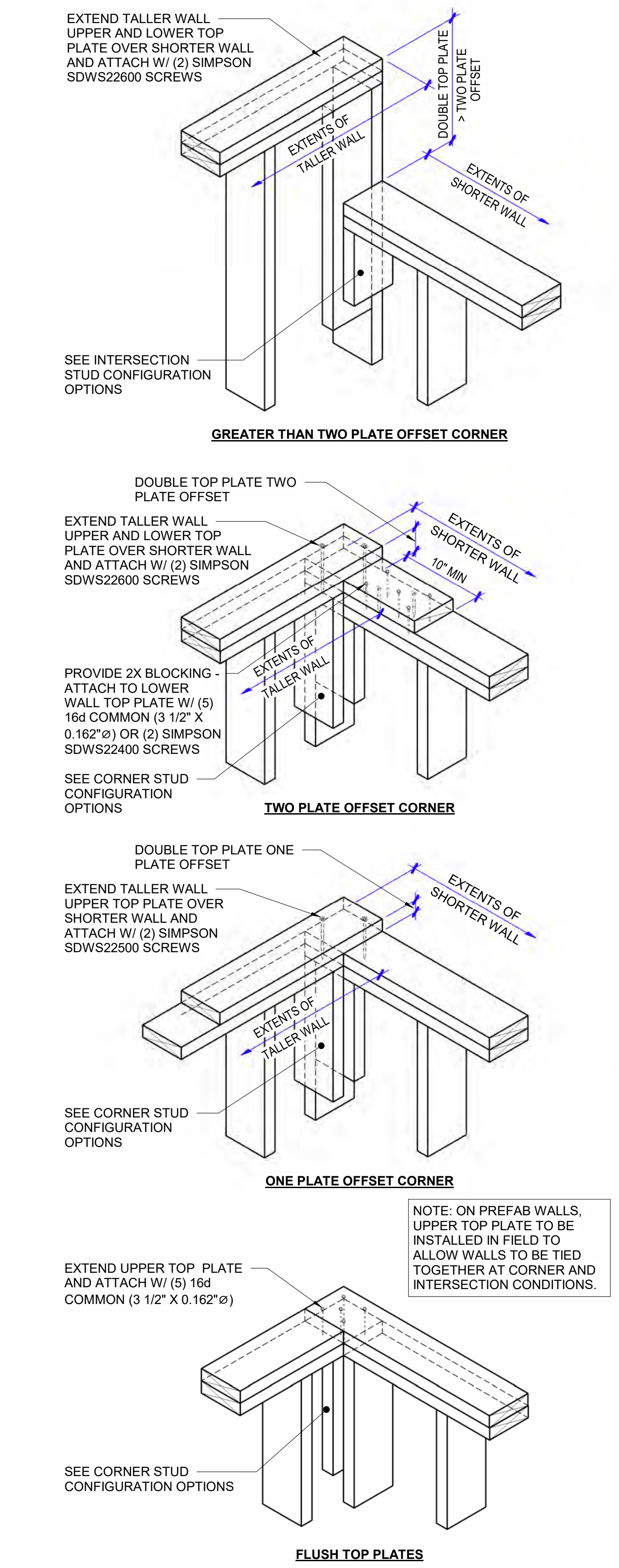
ONE PLATE OFFSET TEE
(INTERSECTION WALL TALLER)

ONE PLATE OFFSET TEE
(INTERSECTION WALL SHORTER)

NOTE:
1. ON PREFAB WALLS, UPPER TOP PLATE TO BE INSTALLED IN FIELD TO ALLOW WALLS TO BE TIED TOGETHER AT CORNER AND INTERSECTION CONDITIONS.
2. DOUBLE TOP PLATES AT EXTERIOR CONDITIONS ARE NOT REQUIRED TO OVERLAP WHEN SHEATHING IS RUN CONTINUOUS AT INTERIOR FACE OF EXTERIOR WALL AND WALL INTERSECTING STUDS CONFIGURATIONS ARE SCREWED TOGETHER.

FLUSH TOP PLATES

4 Wall Intersection Top Plate Connections
1" = 1'-0"


EXTEND TALLER WALL UPPER AND LOWER TOP PLATE OVER SHORTER WALL AND ATTACH W/ (2) SIMPSON SDWS22600 SCREWS
DOUBLE TOP PLATE
> TWO PLATE OFFSET
EXTENTS OF TALLER WALL
EXTENTS OF SHORTER WALL
SEE INTERSECTION STUD CONFIGURATION OPTIONS
DOUBLE TOP PLATE TWO PLATE OFFSET
EXTEND TALLER WALL UPPER AND LOWER TOP PLATE OVER SHORTER WALL AND ATTACH W/ (2) SIMPSON SDWS22600 SCREWS
EXTENTS OF SHORTER WALL
10" MIN
EXTENTS OF TALLER WALL
PROVIDE 2X BLOCKING - ATTACH TO LOWER WALL TOP PLATE W/ (5) 16d COMMON (3 1/2" X 0.162"Ø) OR (2) SIMPSON SDWS22400 SCREWS
SEE CORNER STUD CONFIGURATION OPTIONS
DOUBLE TOP PLATE ONE PLATE OFFSET
EXTEND TALLER WALL UPPER TOP PLATE OVER SHORTER WALL AND ATTACH W/ (2) SIMPSON SDWS22500 SCREWS
EXTENTS OF SHORTER WALL
EXTENTS OF TALLER WALL
SEE CORNER STUD CONFIGURATION OPTIONS
EXTEND UPPER TOP PLATE AND ATTACH W/ (5) 16d COMMON (3 1/2" X 0.162"Ø)
SEE CORNER STUD CONFIGURATION OPTIONS

GREATER THAN TWO PLATE OFFSET CORNER

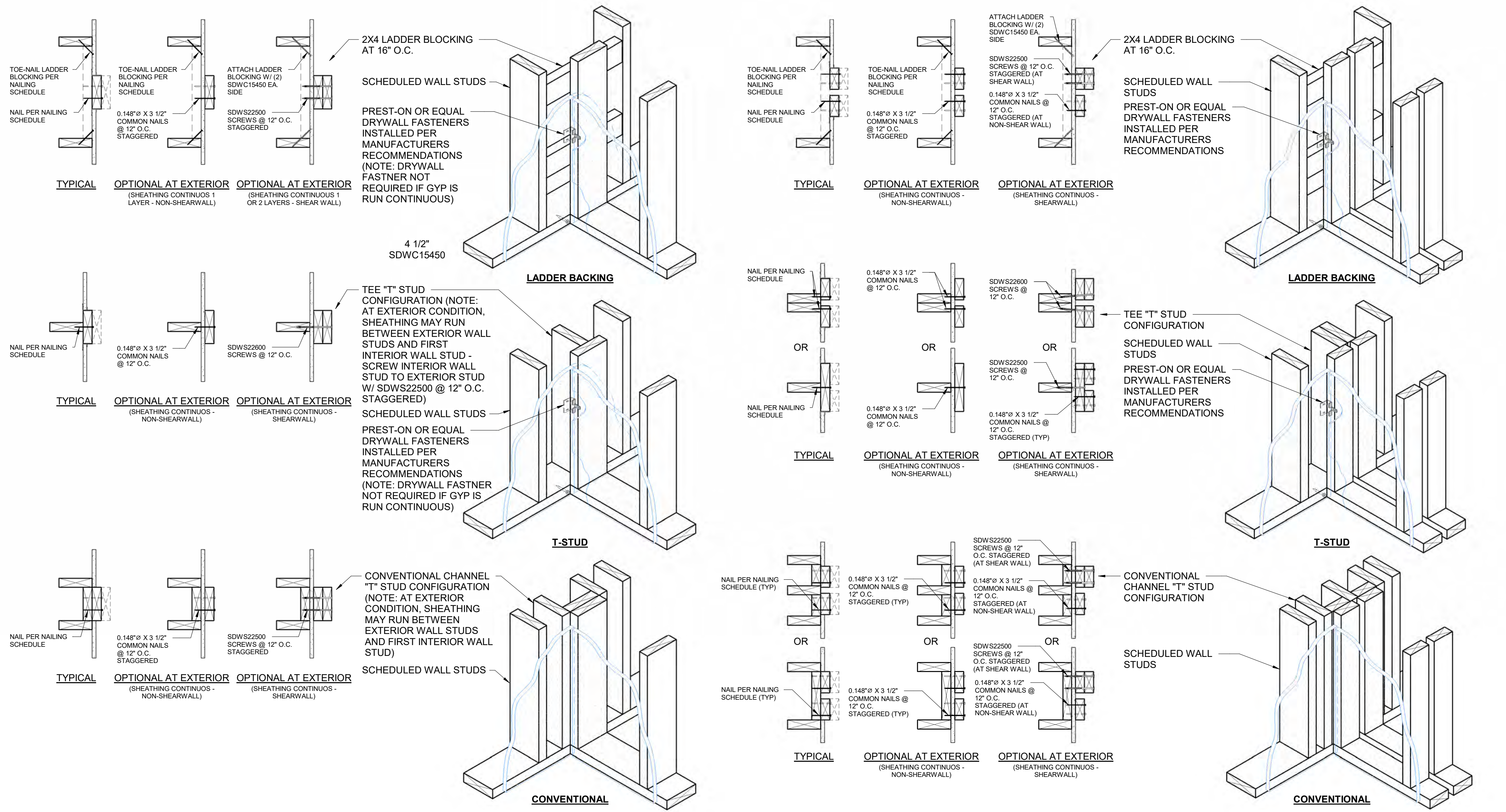
TWO PLATE OFFSET CORNER

ONE PLATE OFFSET CORNER

NOTE: ON PREFAB WALLS, UPPER TOP PLATE TO BE INSTALLED IN FIELD TO ALLOW WALLS TO BE TIED TOGETHER AT CORNER AND INTERSECTION CONDITIONS.

FLUSH TOP PLATES

3 Wall Corner Top Plate Connections
1" = 1'-0"


TOE-NAIL LADDER BLOCKING PER NAILING SCHEDULE
NAIL PER NAILING SCHEDULE
TOE-NAIL LADDER BLOCKING PER NAILING SCHEDULE
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED
ATTACH LADDER BLOCKING W/ (2) SDWC15450 EA. SIDE
SDWS22500 SCREWS @ 12" O.C. STAGGERED
2X4 LADDER BLOCKING AT 16" O.C.
SCHEDULED WALL STUDS
PREST-ON OR EQUAL DRYWALL FASTENERS INSTALLED PER MANUFACTURERS RECOMMENDATIONS (NOTE: DRYWALL FASTNER NOT REQUIRED IF GYP IS RUN CONTINUOUS)
4 1/2" SDWC15450
NAIL PER NAILING SCHEDULE
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C.
SDWS22600 SCREWS @ 12" O.C.
TEE "T" STUD CONFIGURATION (NOTE: AT EXTERIOR CONDITION, SHEATHING MAY RUN BETWEEN EXTERIOR WALL STUDS AND FIRST INTERIOR WALL STUD - SCREW INTERIOR WALL STUD TO EXTERIOR STUD W/ SDWS22500 @ 12" O.C. STAGGERED)
SCHEDULED WALL STUDS
PREST-ON OR EQUAL DRYWALL FASTENERS INSTALLED PER MANUFACTURERS RECOMMENDATIONS (NOTE: DRYWALL FASTNER NOT REQUIRED IF GYP IS RUN CONTINUOUS)
NAIL PER NAILING SCHEDULE
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED
SDWS22500 SCREWS @ 12" O.C. STAGGERED
CONVENTIONAL CHANNEL "T" STUD CONFIGURATION (NOTE: AT EXTERIOR CONDITION, SHEATHING MAY RUN BETWEEN EXTERIOR WALL STUDS AND FIRST INTERIOR WALL STUD)
SCHEDULED WALL STUDS
TOE-NAIL LADDER BLOCKING PER NAILING SCHEDULE
NAIL PER NAILING SCHEDULE
TOE-NAIL LADDER BLOCKING PER NAILING SCHEDULE
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED
ATTACH LADDER BLOCKING W/ (2) SDWC15450 EA. SIDE
SDWS22500 SCREWS @ 12" O.C. STAGGERED (AT SHEAR WALL)
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED (AT NON-SHEAR WALL)
2X4 LADDER BLOCKING AT 16" O.C.
SCHEDULED WALL STUDS
PREST-ON OR EQUAL DRYWALL FASTENERS INSTALLED PER MANUFACTURERS RECOMMENDATIONS
NAIL PER NAILING SCHEDULE
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C.
SDWS22600 SCREWS @ 12" O.C.
OR
OR
OR
NAIL PER NAILING SCHEDULE
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C.
SDWS22500 SCREWS @ 12" O.C.
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED (TYP)
TEE "T" STUD CONFIGURATION
SCHEDULED WALL STUDS
PREST-ON OR EQUAL DRYWALL FASTENERS INSTALLED PER MANUFACTURERS RECOMMENDATIONS
NAIL PER NAILING SCHEDULE (TYP)
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED (TYP)
SDWS22500 SCREWS @ 12" O.C. STAGGERED (AT SHEAR WALL)
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED (AT NON-SHEAR WALL)
OR
OR
OR
NAIL PER NAILING SCHEDULE (TYP)
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED (TYP)
SDWS22500 SCREWS @ 12" O.C. STAGGERED (AT SHEAR WALL)
0.148"Ø X 3 1/2" COMMON NAILS @ 12" O.C. STAGGERED (AT NON-SHEAR WALL)
CONVENTIONAL CHANNEL "T" STUD CONFIGURATION
SCHEDULED WALL STUDS

TYPICAL

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS 1 LAYER - NON-SHEARWALL)

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOUS 1 OR 2 LAYERS - SHEAR WALL)

LADDER BACKING

TYPICAL

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - NON-SHEARWALL)

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - SHEARWALL)

T-STUD

TYPICAL

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - NON-SHEARWALL)

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - SHEARWALL)

CONVENTIONAL

STANDARED INTERSECTION

TYPICAL

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - NON-SHEARWALL)

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - SHEARWALL)

LADDER BACKING

TYPICAL

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - NON-SHEARWALL)

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - SHEARWALL)

T-STUD

TYPICAL

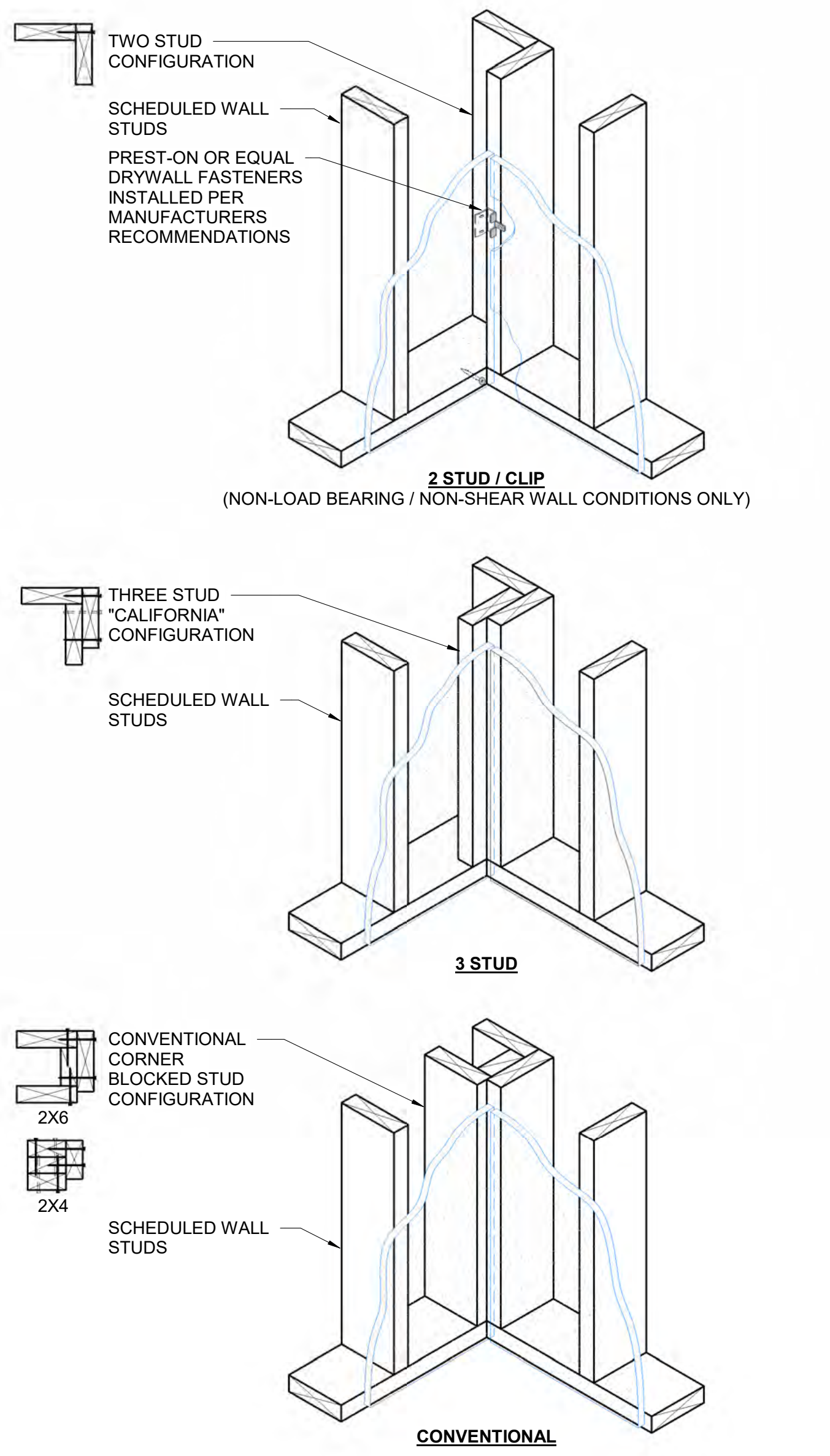
OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - NON-SHEARWALL)

OPTIONAL AT EXTERIOR
(SHEATHING CONTINUOS - SHEARWALL)

CONVENTIONAL

DEMISING WALL INTERSECTION

2 Wall Intersection Stud Configurations
1" = 1'-0"


TWO STUD CONFIGURATION
SCHEDULED WALL STUDS
PREST-ON OR EQUAL DRYWALL FASTENERS INSTALLED PER MANUFACTURERS RECOMMENDATIONS
THREE STUD "CALIFORNIA" CONFIGURATION
SCHEDULED WALL STUDS
CONVENTIONAL CORNER BLOCKED STUD CONFIGURATION
2X6
2X4
SCHEDULED WALL STUDS

2 STUD / CLIP
(NON-LOAD BEARING / NON-SHEAR WALL CONDITIONS ONLY)

3 STUD

CONVENTIONAL

1 Wall Corner Stud Configurations
1" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
Typical Framing Details - Wood

SHEET NUMBER:
S0.202

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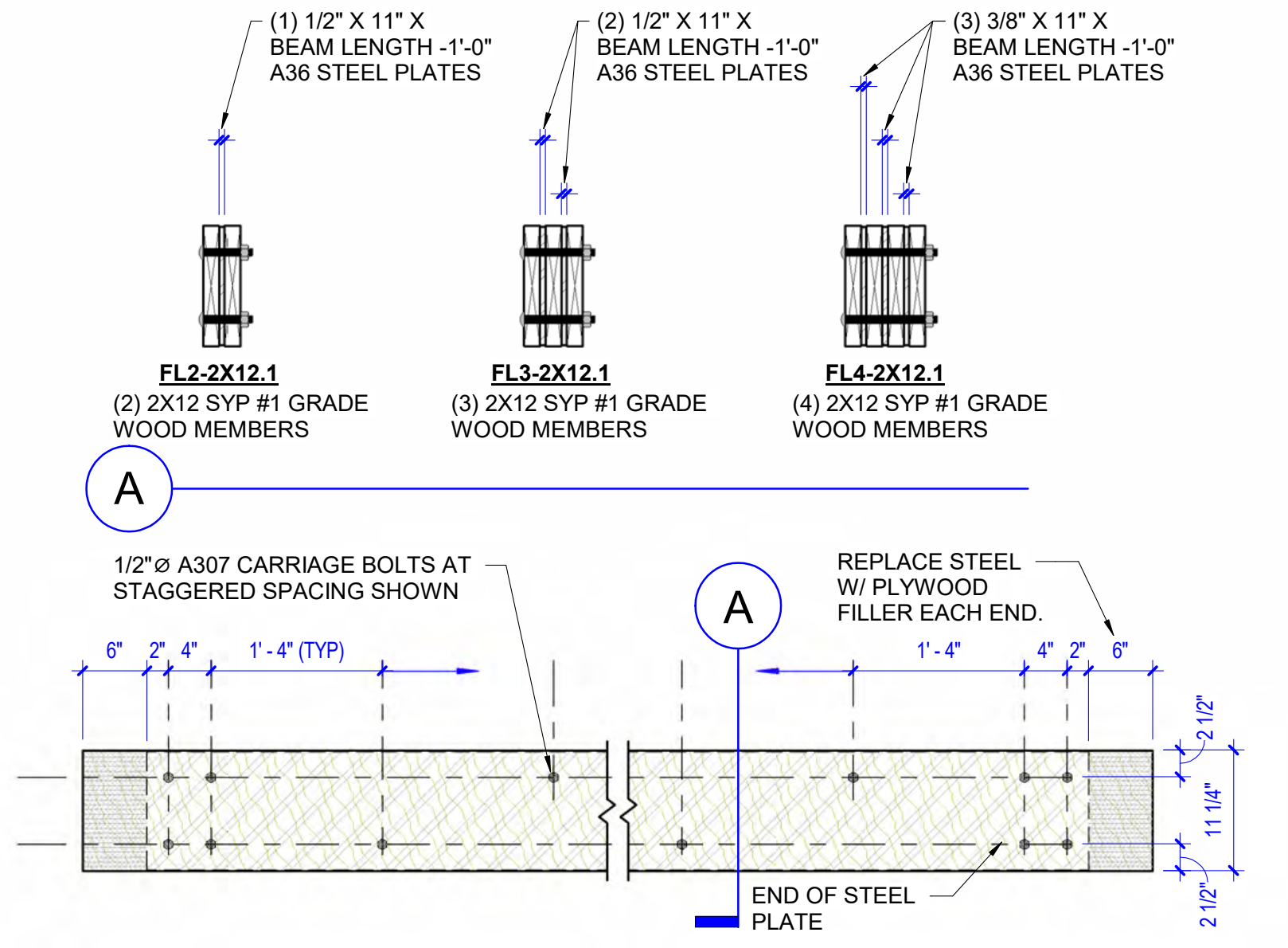
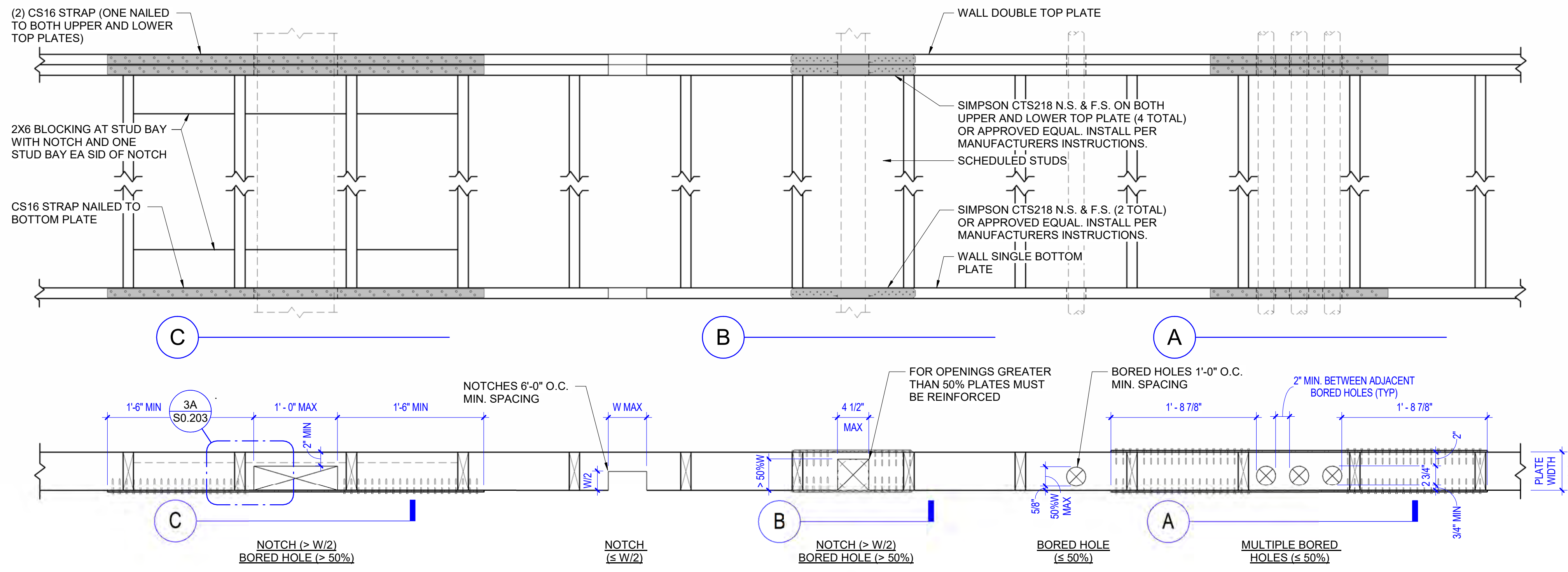


NOTE: AT 3A CONSTRUCTION, PROVIDE FIRE-RETARDANT-TREATED (FRT) LUMBER FOR ALL 2X WOOD MEMBERS LOCATED WITHIN OR PENETRATING EXTERIOR WALL ASSEMBLIES.

5 Typical Flitch Beam Assembly
3/4" = 1'-0"


(2) CS16 STRAP (ONE NAILED TO BOTH UPPER AND LOWER TOP PLATES)
2X6 BLOCKING AT STUD BAY WITH NOTCH AND ONE STUD BAY EA SID OF NOTCH
CS16 STRAP NAILED TO BOTTOM PLATE
WALL DOUBLE TOP PLATE
SIMPSON CTS218 N.S. & F.S. ON BOTH UPPER AND LOWER TOP PLATE (4 TOTAL) OR APPROVED EQUAL. INSTALL PER MANUFACTURERS INSTRUCTIONS.
SCHEDULED STUDS
SIMPSON CTS218 N.S. & F.S. (2 TOTAL) OR APPROVED EQUAL. INSTALL PER MANUFACTURERS INSTRUCTIONS.
WALL SINGLE BOTTOM PLATE
C
B
A
1'-6" MIN
3A
S0.203
1' - 0" MAX
1'-6" MIN
2" MIN
NOTCHES 6'-0" O.C. MIN. SPACING
W MAX
W/2
FOR OPENINGS GREATER THAN 50% PLATES MUST BE REINFORCED
4 1/2" MAX
> 50%W
BORED HOLES 1'-0" O.C. MIN. SPACING
5/8"
50%W MAX
2" MIN. BETWEEN ADJACENT BORED HOLES (TYP)
1' - 8 7/8"
1' - 8 7/8"
2"
2 3/4"
3/4" MIN
PLATE WIDTH (W)
NOTCH (> W/2) BORED HOLE (> 50%)
NOTCH (≤ W/2)
NOTCH (> W/2) BORED HOLE (> 50%)
BORED HOLE (≤ 50%)
MULTIPLE BORED HOLES (≤ 50%)

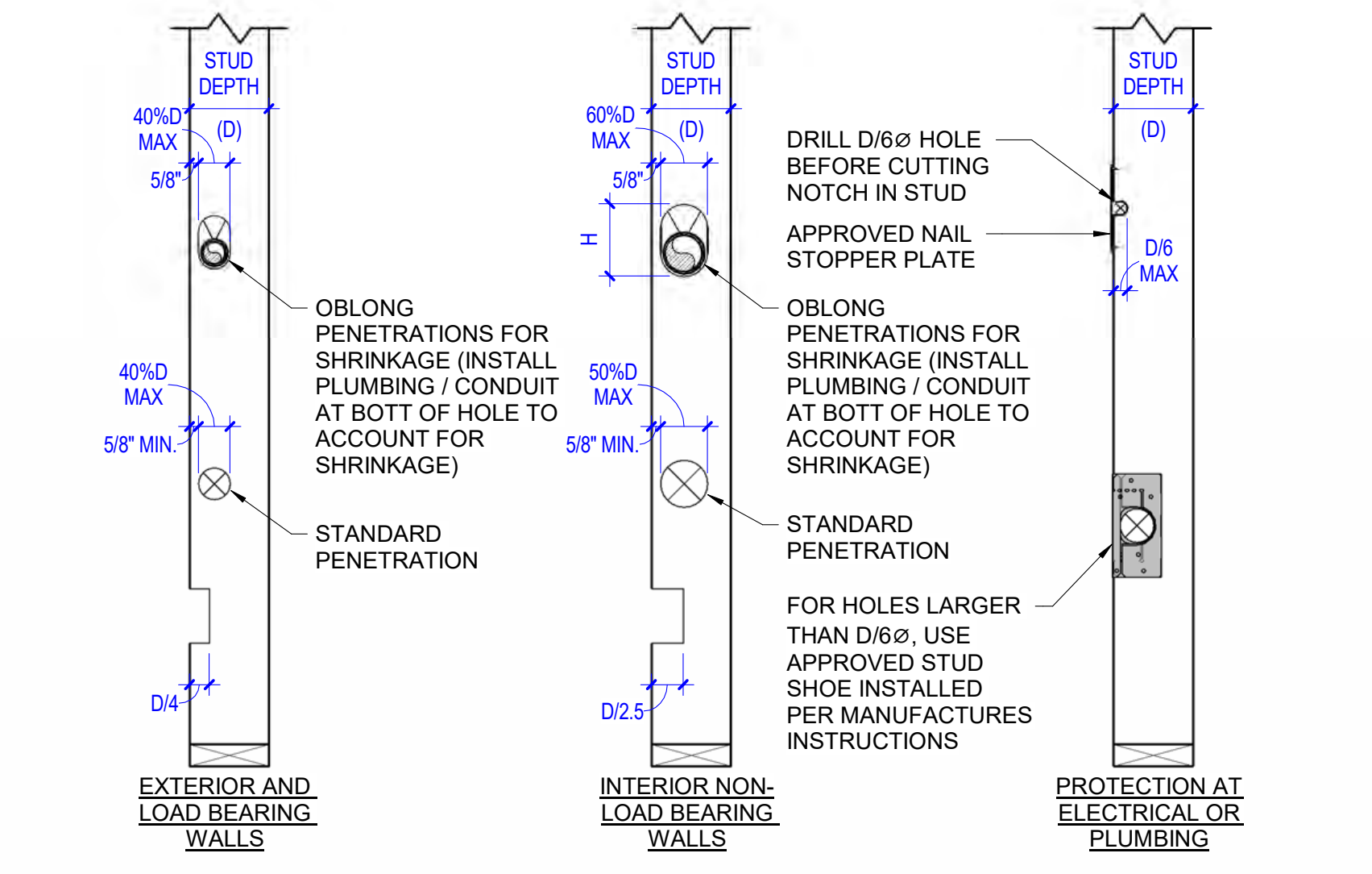
EXTERIOR AND LOAD BEARING WALL TOP AND BOTTOM PLATE

HOLE / NOTCHSIZE CHART (PLATES)		
PLATE WIDTH	50% HOLE DIAMETER	W/2 NOTCH
2X4 (3 1/2")	1 3/4"⌀	1 3/4"
2X6 (5 1/2")	2 3/4"⌀	2 3/4"
2X8 (7 1/4")	3 5/8"⌀	3 5/8"
2X10 (9 1/4")	4 5/8"⌀	4 5/8"

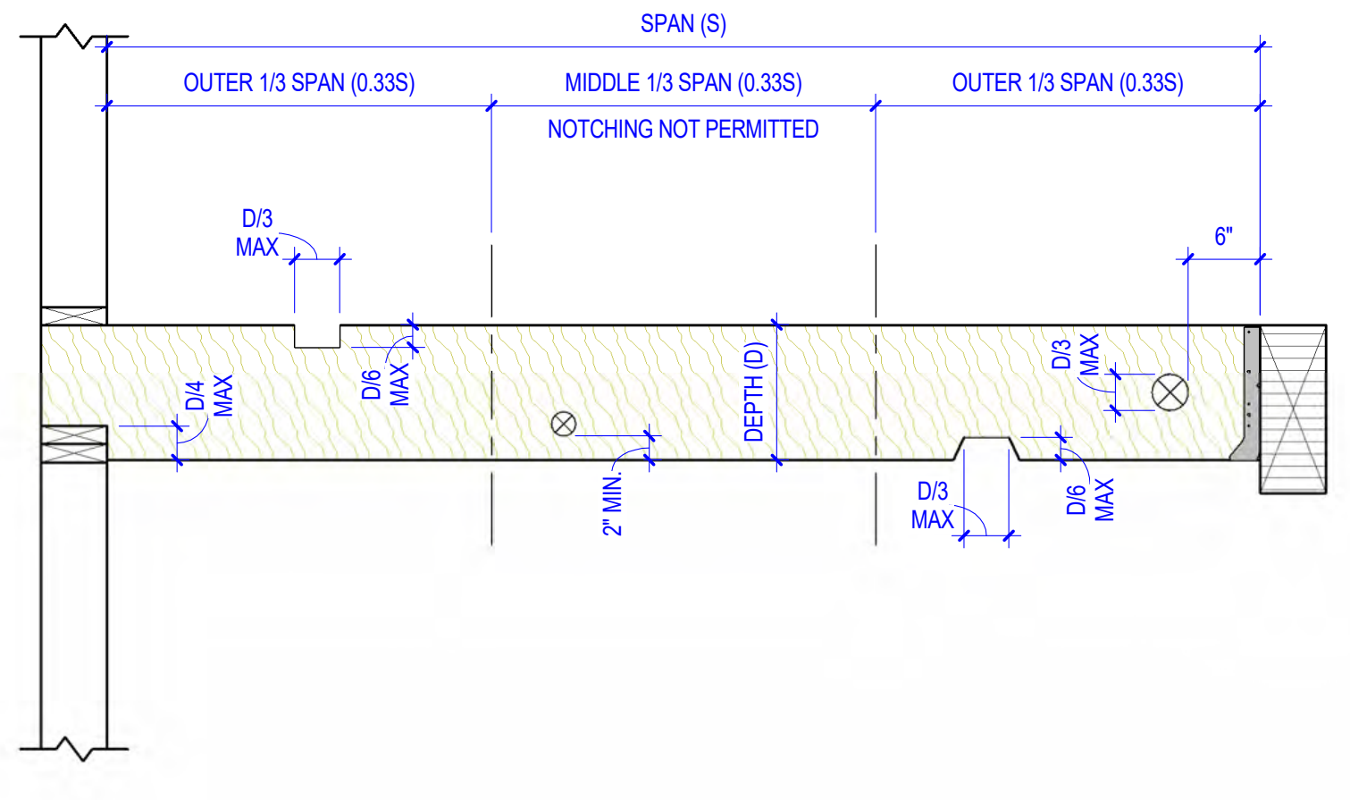
NOTCH SIZE CHART (STUDS)		
STUD DEPTH	D/4	D/2.5
2X4 (3 1/2")	7/8"	1 3/8"
2X6 (5 1/2")	1 3/8"	2 3/16"
2X8 (7 1/4")	1 13/16"	2 7/8"
2X10 (9 1/4")	2 5/16"	3 11/16"

HOLE SIZE CHART (STUDS)		
STUD DEPTH	40% HOLE DIAMETER	60% HOLE DIAMETER
2X4 (3 1/2")	1 3/8"⌀	2 1/8"⌀
2X6 (5 1/2")	2 3/16"⌀	3 5/16"⌀
2X8 (7 1/4")	2 7/8"⌀	4 3/8"⌀
2X10 (9 1/4")	3 3/4"⌀	5 9/16"⌀

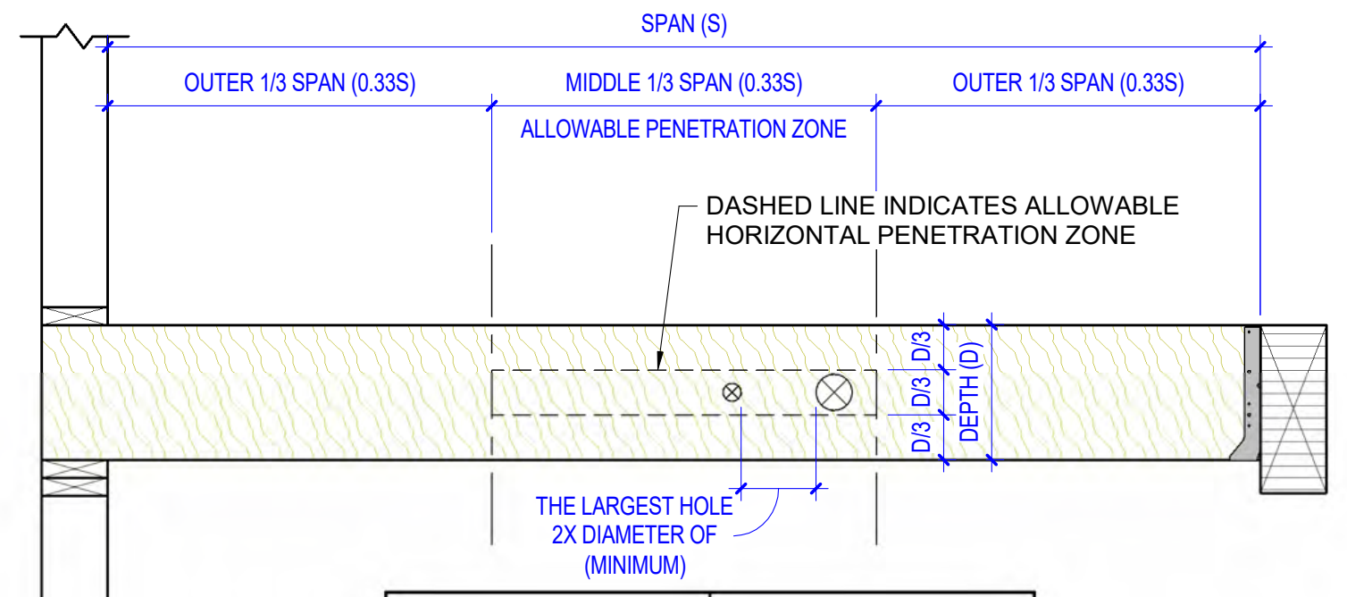
OBLONG HOLE HEIGHT CHART (STUDS)		
WOOD FRAMED LEVEL	40% HOLE HEIGHT (H)	60% HOLE HEIGHT (H)
1ST	1 3/4"	2 1/2"
2ND	2 15/16"	4 1/16"
3RD	4"	5 1/8"
4TH	5 1/4"	7 1/16"⌀


STUD DEPTH
(D)
40%D MAX
5/8"
40%D MAX
5/8" MIN.
D/4
OBLONG PENETRATIONS FOR SHRINKAGE (INSTALL PLUMBING / CONDUIT AT BOTT OF HOLE TO ACCOUNT FOR SHRINKAGE)
STANDARD PENETRATION
EXTERIOR AND LOAD BEARING WALLS
STUD DEPTH
(D)
60%D MAX
5/8"
H
50%D MAX
5/8" MIN.
D/2.5
OBLONG PENETRATIONS FOR SHRINKAGE (INSTALL PLUMBING / CONDUIT AT BOTT OF HOLE TO ACCOUNT FOR SHRINKAGE)
STANDARD PENETRATION
INTERIOR NON-LOAD BEARING WALLS
STUD DEPTH
(D)
DRILL D/6⌀ HOLE BEFORE CUTTING NOTCH IN STUD
APPROVED NAIL STOPPER PLATE
D/6 MAX
FOR HOLES LARGER THAN D/6⌀, USE APPROVED STUD SHOE INSTALLED PER MANUFACTURES INSTRUCTIONS
PROTECTION AT ELECTRICAL OR PLUMBING

2 Allowable Notches and Penetrations in Wall Studs and Plates
1" = 1'-0"



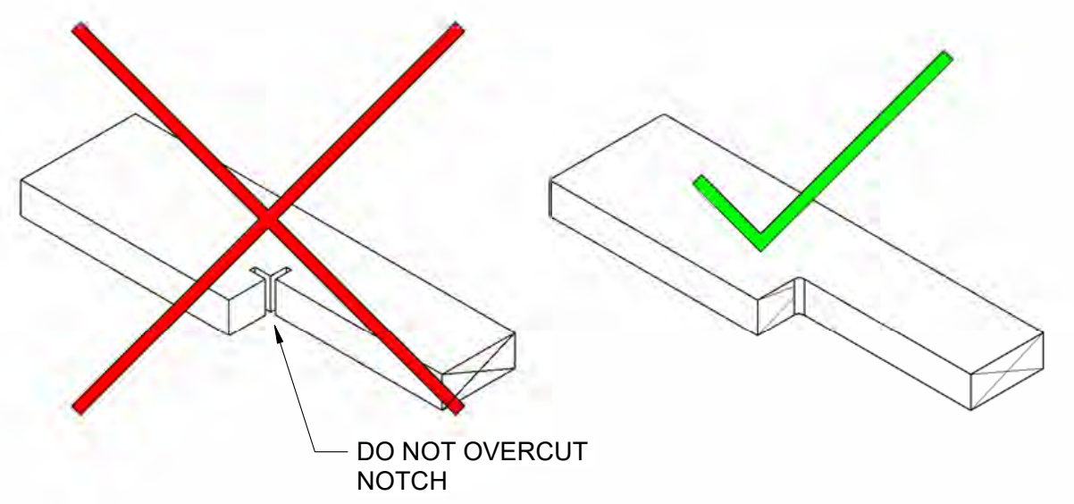
2X NOMINAL CUT BEAMS AND PURLINS


SPAN (S)
OUTER 1/3 SPAN (0.33S)
MIDDLE 1/3 SPAN (0.33S)
OUTER 1/3 SPAN (0.33S)
ALLOWABLE PENETRATION ZONE
DASHED LINE INDICATES ALLOWABLE HORIZONTAL PENETRATION ZONE
D/3
D/3
D/3
DEPTH (D)
THE LARGEST HOLE 2X DIAMETER OF (MINIMUM)

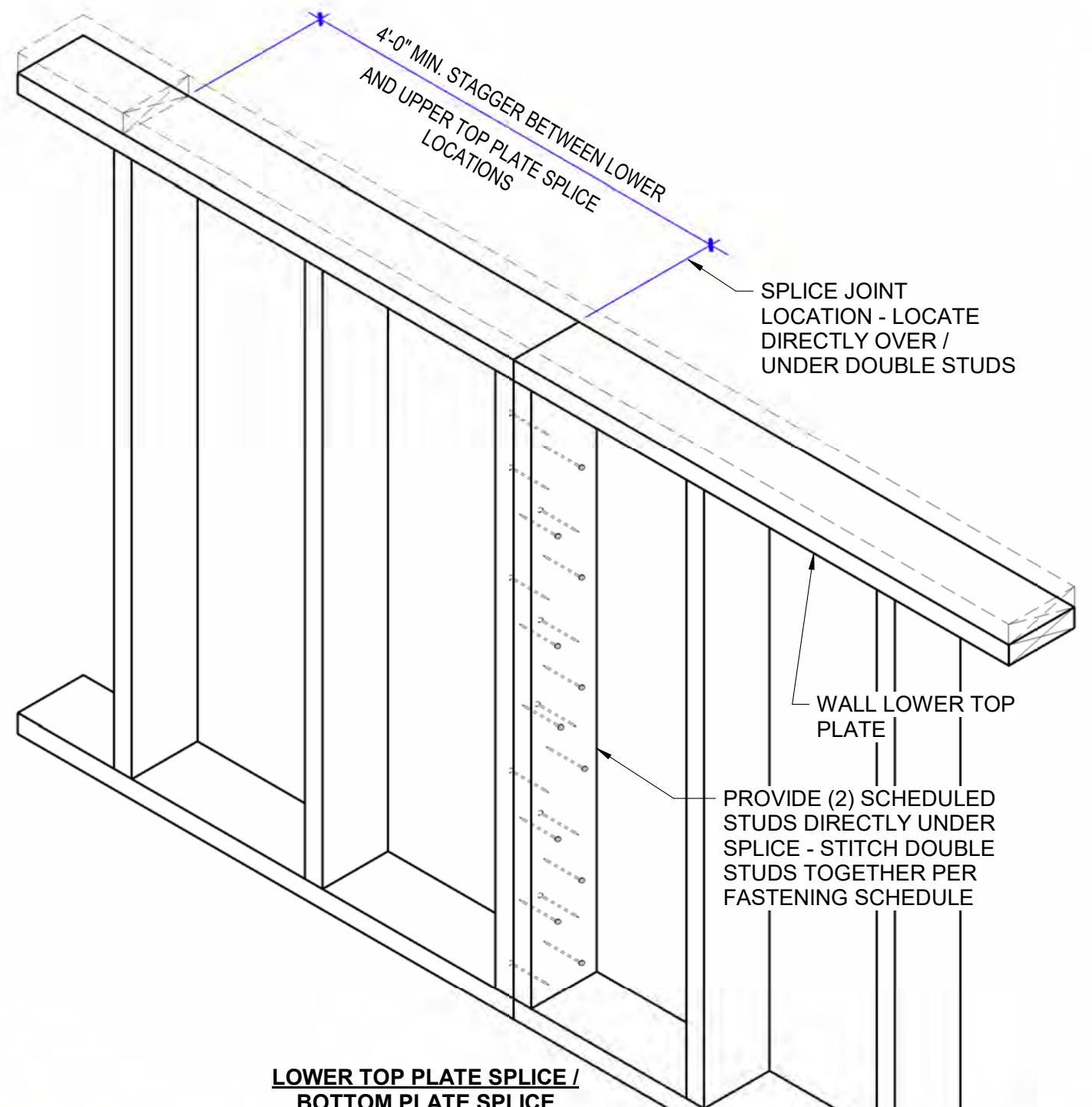
HEADER / BEAM DEPTH	MAX ROUND HOLE SIZE
4 3/8"	1"⌀
5 1/2"	1 3/4"⌀
7 1/4" - 20"	2"⌀

ENGINEERED BEAMS (GLULAM / LVL / PSL)

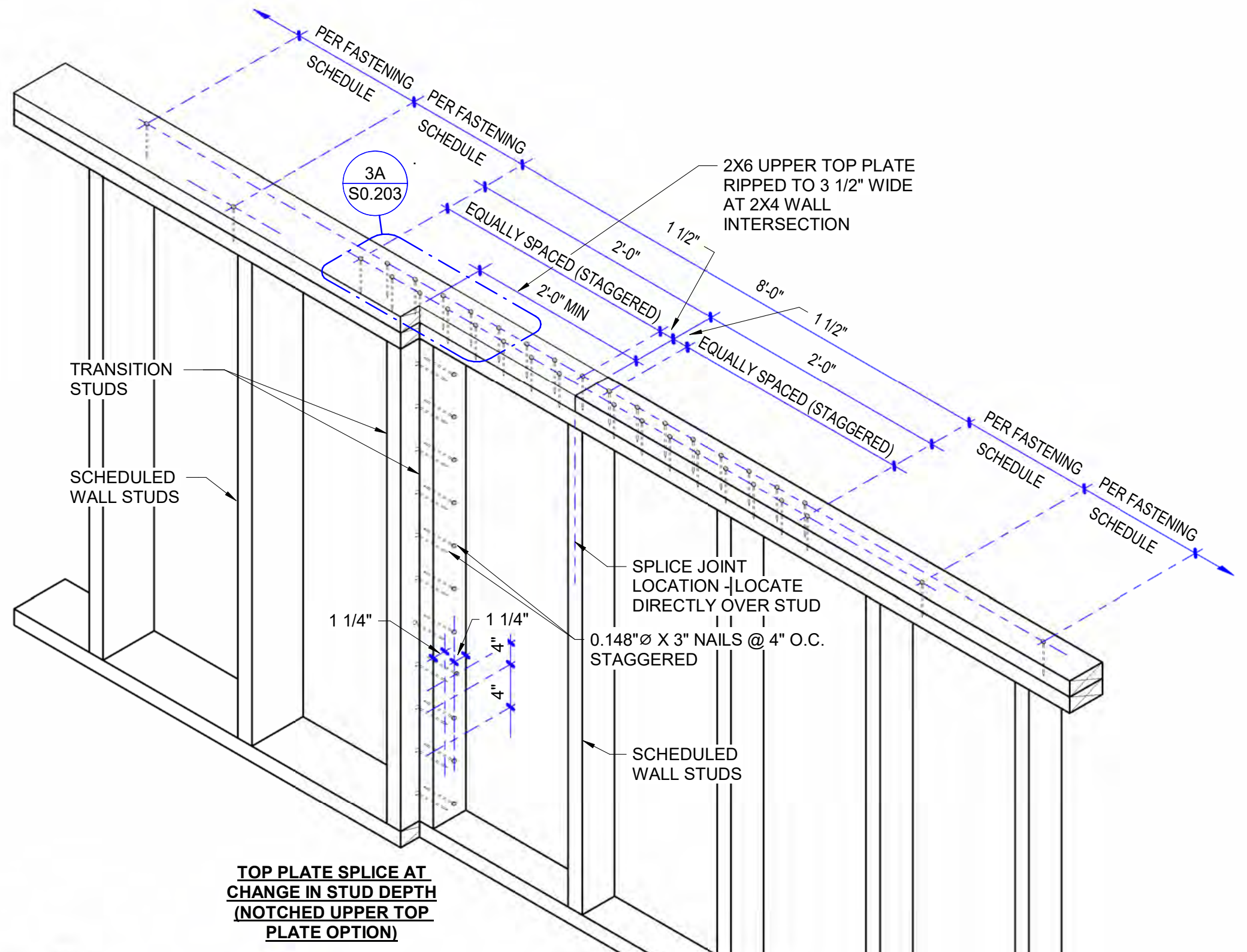
4 Allowable Penetrations in Wood Beams
3/4" = 1'-0"



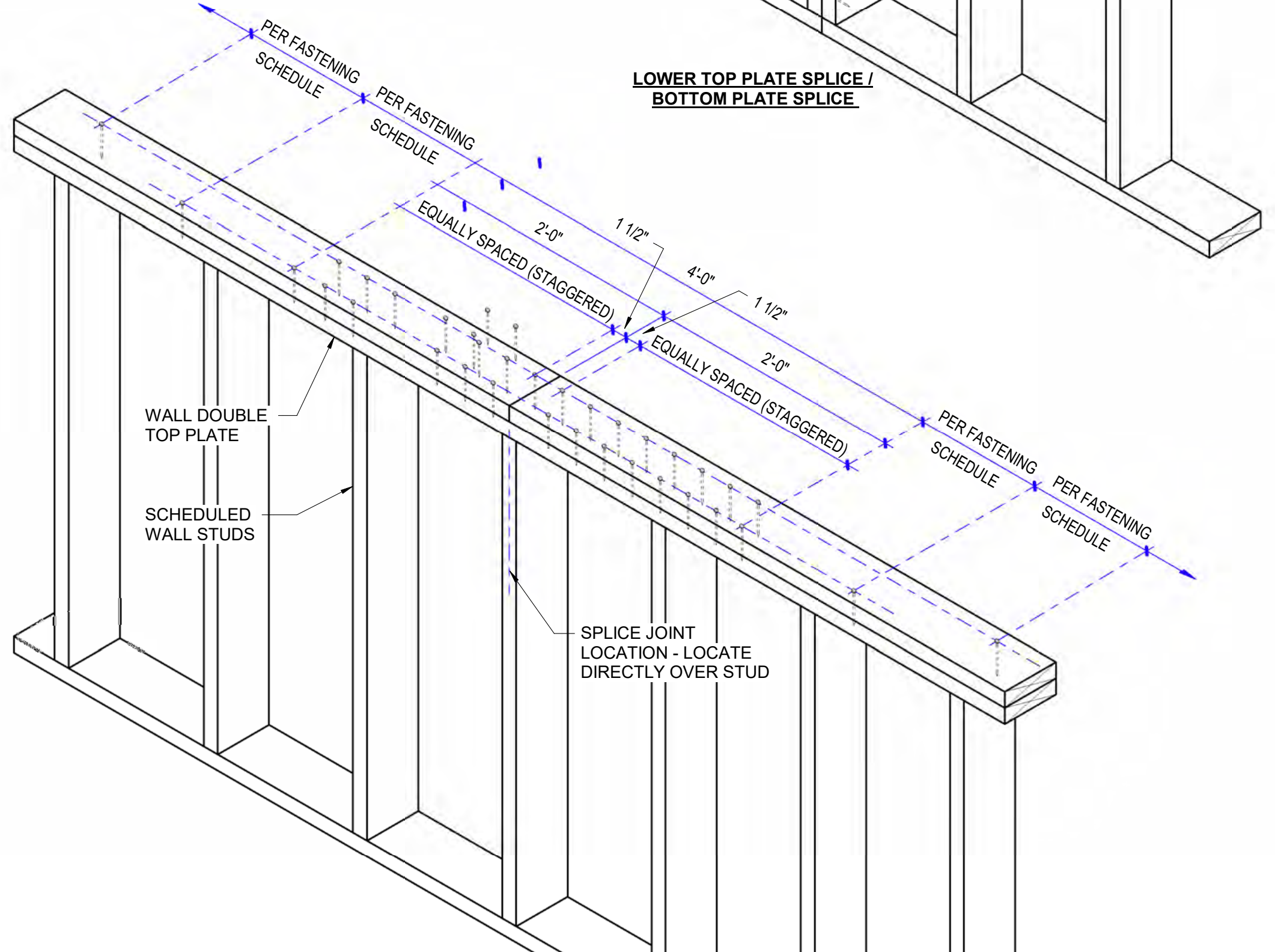
3A Over-Cut Detail
1 1/2" = 1'-0"


4'-0" MIN. STAGGER BETWEEN LOWER AND UPPER TOP PLATE SPLICE LOCATIONS
SPLICE JOINT LOCATION - LOCATE DIRECTLY OVER / UNDER DOUBLE STUDS
WALL LOWER TOP PLATE
PROVIDE (2) SCHEDULED STUDS DIRECTLY UNDER SPLICE - STITCH DOUBLE STUDS TOGETHER PER FASTENING SCHEDULE

LOWER TOP PLATE SPLICE / BOTTOM PLATE SPLICE


PER FASTENING SCHEDULE
PER FASTENING SCHEDULE
3A
S0.203
2X6 UPPER TOP PLATE RIPPED TO 3 1/2" WIDE AT 2X4 WALL INTERSECTION
EQUALLY SPACED (STAGGERED)
2'-0"
2'-0" MIN
1 1/2"
8'-0"
1 1/2"
2'-0"
EQUALLY SPACED (STAGGERED)
PER FASTENING SCHEDULE
PER FASTENING SCHEDULE
TRANSITION STUDS
SCHEDULED WALL STUDS
SPLICE JOINT LOCATION - LOCATE DIRECTLY OVER STUD
1 1/4"
1 1/4"
4"
4"
0.148"⌀ X 3" NAILS @ 4" O.C. STAGGERED
SCHEDULED WALL STUDS

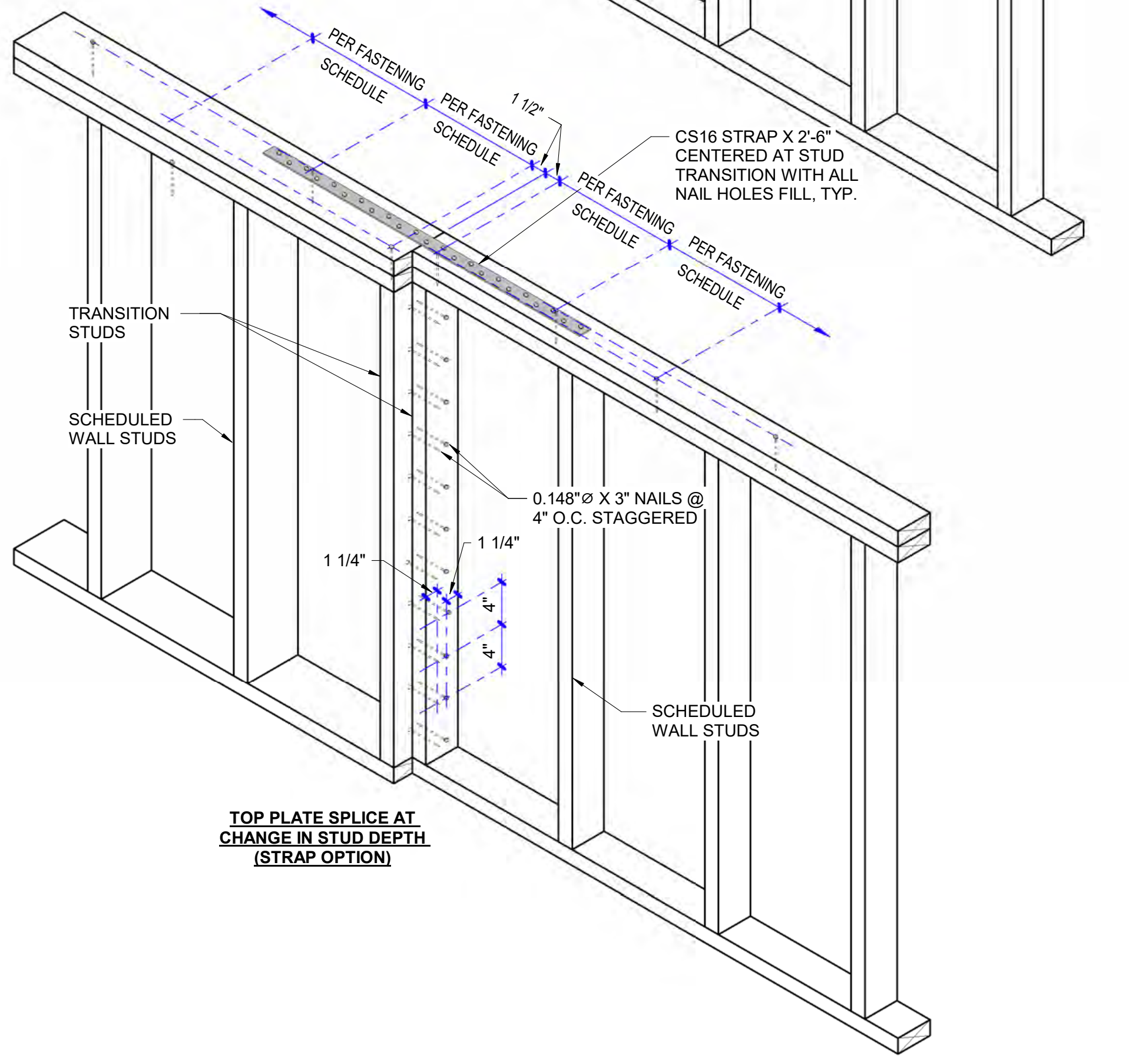
TOP PLATE SPLICE AT CHANGE IN STUD DEPTH (NOTCHED UPPER TOP PLATE OPTION)



UPPER TOP PLATE SPLICE

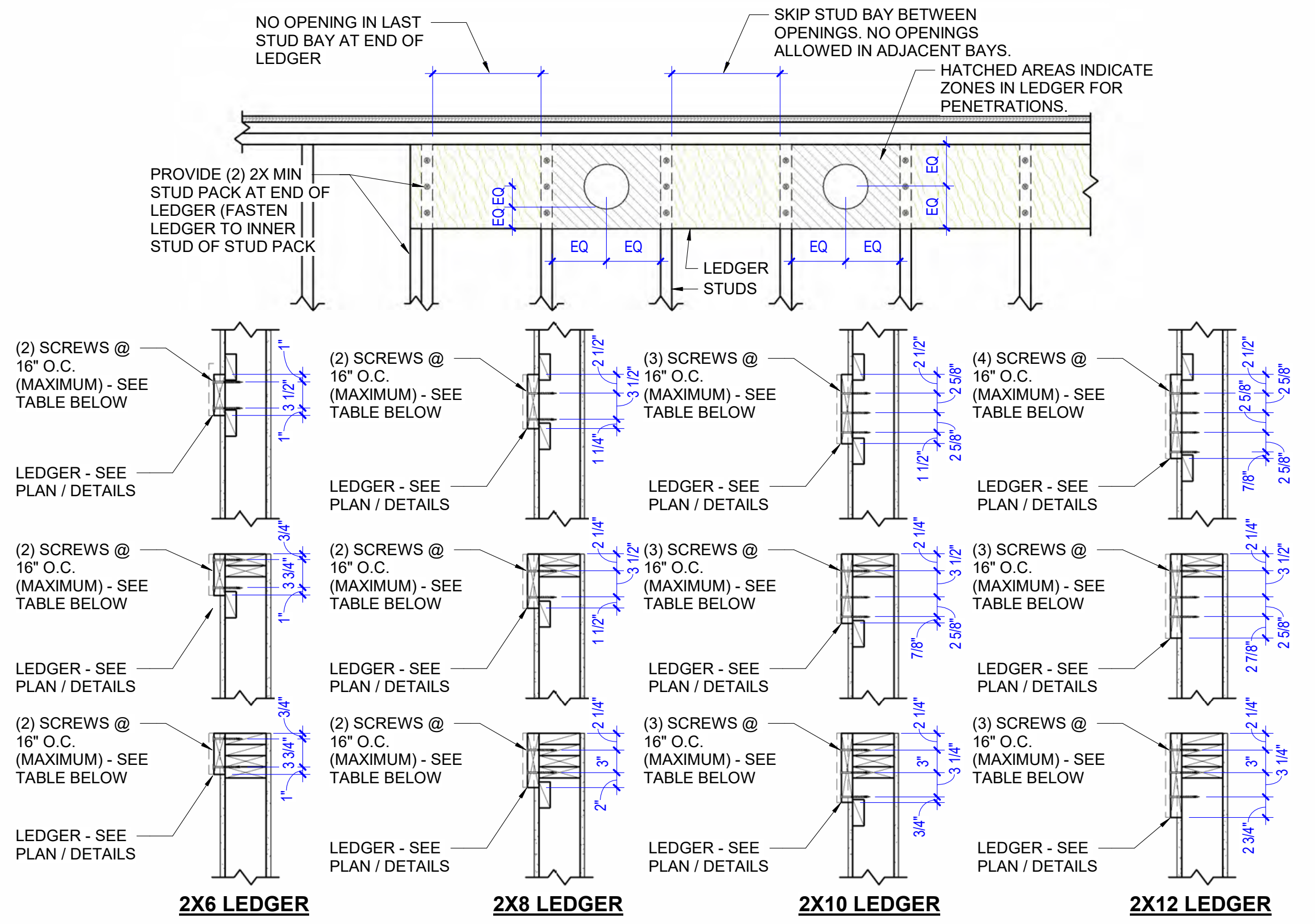
UPPER TOP PLATE SPLICE NOTES:

1. 2X4 PLATES: (14) 0.148" DIA. X 3" LONG STAGGERED NAILS EA. SIDE OF SPLICE JOINT
2. 2X6 PLATES: (22) 0.148" DIA. X 3" LONG STAGGERED NAILS EA. SIDE OF SPLICE JOINT
3. TYPICAL FACE NAILING: PER FASTENING SCHEDULE
4. SPLICE JOINT LOCATION SHALL OCCUR OVER WALL STUDS. SEE REQUIRED WALL PLATE SPLICE REPAIR DETAIL ABOVE FOR OTHER CONDITIONS (TYPICAL)



TOP PLATE SPLICE AT CHANGE IN STUD DEPTH (STRAP OPTION)

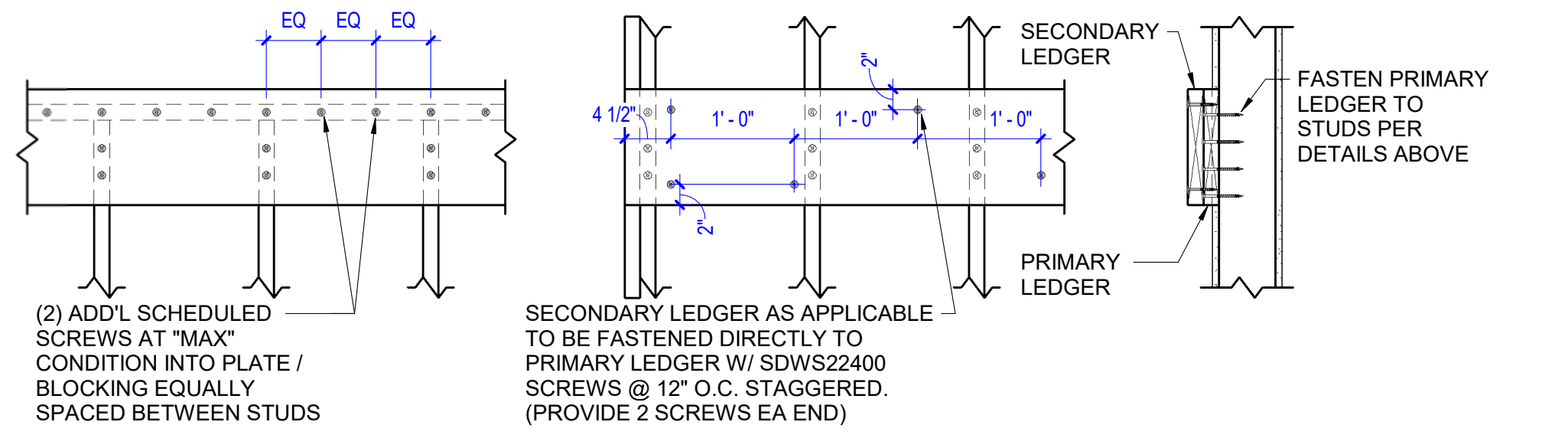
3 Wall Plate Splice Detail
1" = 1'-0"


NO OPENING IN LAST STUD BAY AT END OF LEDGER
SKIP STUD BAY BETWEEN OPENINGS. NO OPENINGS ALLOWED IN ADJACENT BAYS.
HATCHED AREAS INDICATE ZONES IN LEDGER FOR PENETRATIONS.
PROVIDE (2) 2X MIN STUD PACK AT END OF LEDGER (FASTEN LEDGER TO INNER STUD OF STUD PACK
EQ
EQ
LEDGER STUDS
(2) SCREWS @ 16" O.C. (MAXIMUM) - SEE TABLE BELOW
LEDGER - SEE PLAN / DETAILS
(3) SCREWS @ 16" O.C. (MAXIMUM) - SEE TABLE BELOW
(4) SCREWS @ 16" O.C. (MAXIMUM) - SEE TABLE BELOW
3 3/4"
1"
2 1/4"
3"
2"
3 1/4"
3/4"
2 3/4"
2X6 LEDGER
2X8 LEDGER
2X10 LEDGER
2X12 LEDGER

LEDGER PENETRATION / FASTENING SCHEDULE				
LEDGER SIZE	PENETRATION (MAX DIA)	FASTENERS		
		DIRECTLY APPLIED TO STUD 6	APPLIED OVER (1) LAYER OF SHEATHING	APPLIED OVER (2) LAYER OF SHEATHING
2 X 6	3" ⌀	SDWS22400 SCREWS	SDWS22600 SCREWS	SDWS22600 SCREWS
2 X 8	4" ⌀	SDWS22400 SCREWS	SDWS22600 SCREWS	SDWS22600 SCREWS
2 X10	6" ⌀	SDWS22600 SCREWS	SDWS22600 SCREWS	SDWS22600 SCREWS
2 X 12	8" ⌀	SDWS22400 SCREWS	SDWS22600 SCREWS	SDWS22600 SCREWS

NOTE:

1. SCREWS SHALL PENETRATE 2½" MINIMUM INTO CENTER OF 2X WALL MEMBERS BEHIND LEDGER. SCREWS SHALL MEET SPACING AND EDGE DIMENSIONS SHOWN IN DETAIL BASED ON LEDGER SIZE AND INSTALLATION LOCATION.
2. INSTALLATION BASED ON TESTED CAPACITIES PER ICC-ESAC2
3. INSTALLATION BASED ON TESTED CAPACITIES PER ICC-ESAC233 AS PROVIDED BY SIMPSON STRONG-TIE.
4. LEDGER MAY BE INSTALLED OVER 1 OR 2 LAYERS OF SHEATING AS APPLICABLE.
5. SHEATHING MAY BE WOOD STRUCTURAL PANEL (OSB OR PLYWOOD) OR GYPSUM BOARD.
6. PROVIDE 2X BLOCK AS NEEDED FOR EDGE NAILING (WHEN LEDGER IS NOT INSTALLED OVER SHEATHING)
7. "MAX" CONDITION TO BE USED WHEN SPECIFIED ON PLAN, SEE DETAIL BELOW.


EQ
EQ
EQ
(2) ADD'L SCHEDULED SCREWS AT "MAX" CONDITION INTO PLATE / BLOCKING EQUALLY SPACED BETWEEN STUDS
4 1/2"
1' - 0"
1' - 0"
1' - 0"
2"
2"
SECONDARY LEDGER
PRIMARY LEDGER
SECONDARY LEDGER AS APPLICABLE TO BE FASTENED DIRECTLY TO PRIMARY LEDGER W/ SDWS22400 SCREWS @ 12" O.C. STAGGERED. (PROVIDE 2 SCREWS EA END)
FASTEN PRIMARY LEDGER TO STUDS PER DETAILS ABOVE

"MAX" CONDITION

SECONDARY LEDGER ATTACHMENT

1 Ledger Penetrations / Attachment
3/4" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMeg Structural Engineers, LLC
1080 Eldridge Parkway
Suite 350
Houston, TX 77077
281-407-5734
www.vsmeg.com

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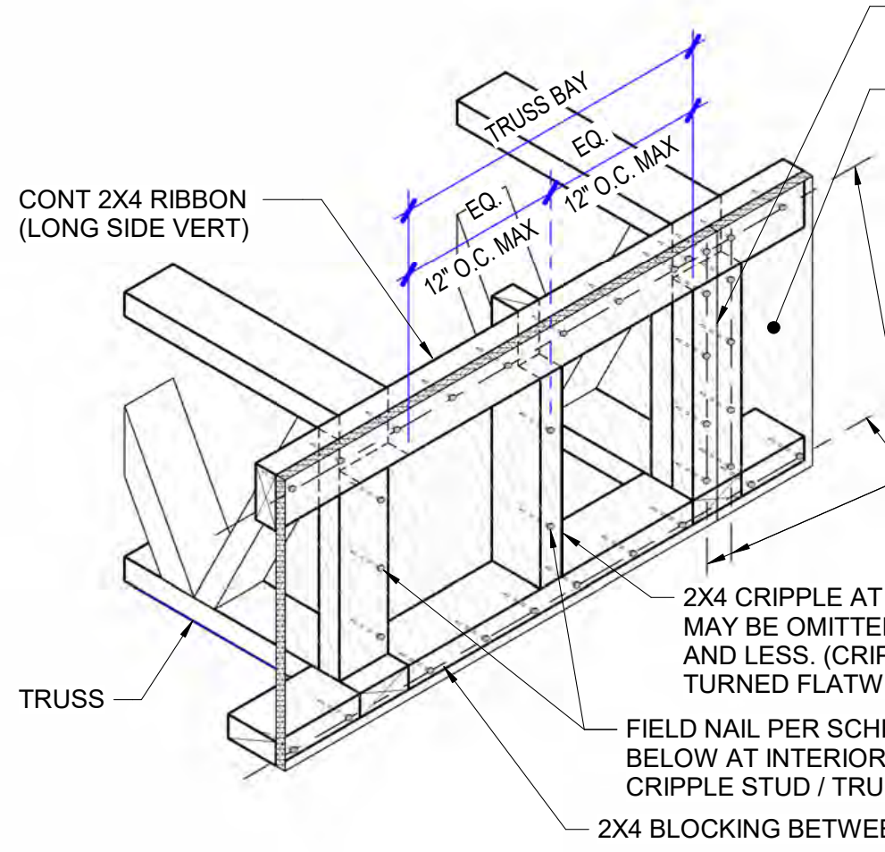


CONT 2X4 RIBBON (LONG SIDE VERT)

PANEL JOINT

- AT WSP SHEAR WALL LOCATIONS, PROVIDE W.S.P. SHEATHING TO MATCH SHEATHING THICKNESS AND NAILING OF SHEARWALL BELOW.
- AT GYP SHEAR WALL AND NON-SHEAR WALL LOCATIONS, PROVIDE 7/16" WSP

BOUNDARY EDGE NAIL PER SCHEDULED SHEAR WALL BELOW

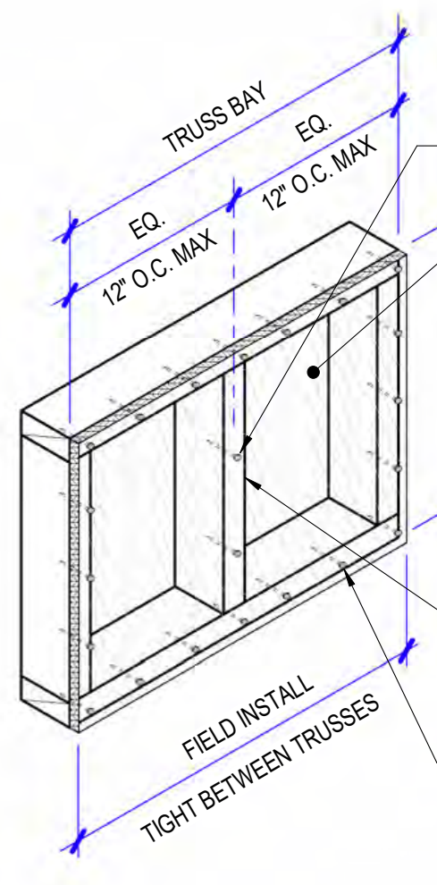
2X4 CRIPPLE AT 12" O.C MAX. CRIPPLE MAY BE OMITTED FOR TRUSS BAYS 12" AND LESS. (CRIPPLE MAY BE LET IN OR TURNED FLATWISE AT FRAMERS OPTION)

FIELD NAIL PER SCHEDULED SHEAR WALL BELOW AT INTERIOR PANEL CONDITIONS AT CRIPPLE STUD / TRUSSE ENDS

2X4 BLOCKING BETWEEN TRUSSES

TRUSS

1D Cont 2x4 Ribbon Truss Brace
1" = 1'-0"



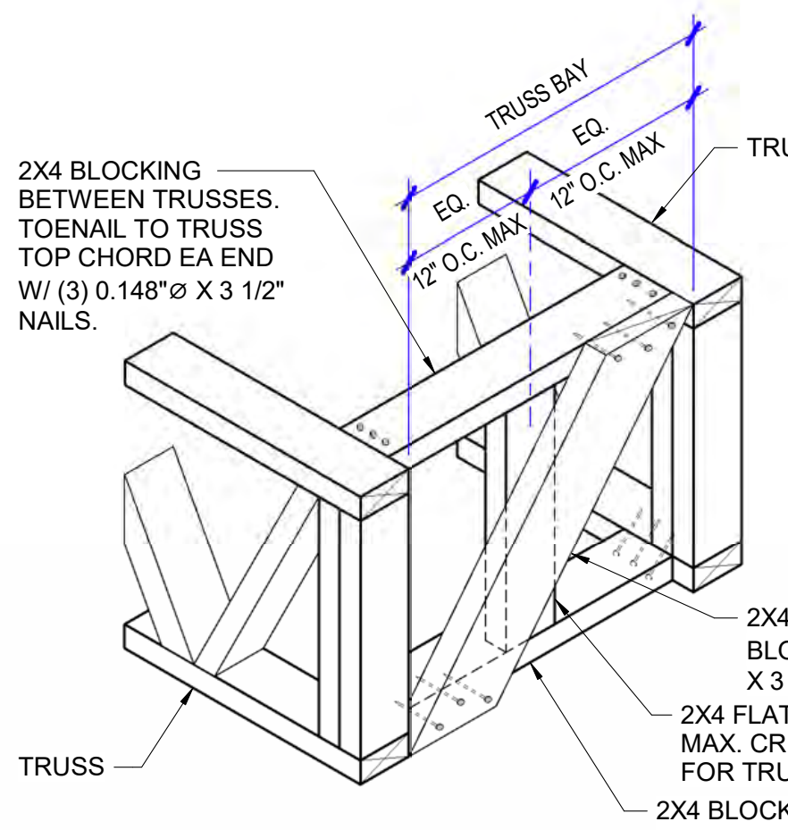
FIELD NAILING

- AT WSP SHEAR WALL LOCATIONS, PROVIDE W.S.P. SHEATHING TO MATCH SHEATHING THICKNESS AND NAILING OF SHEAR WALL BELOW.
- AT GYP SHEAR WALL AND NON-SHEAR WALL LOCATIONS, PROVIDE 7/16" WSP NAILED PER FASTENING SCHEDULE.

2X4 CRIPPLE AT 12" O.C. MAX. CRIPPLE MAY BE OMITTED FOR TRUSS BAYS 12" AND LESS.

BOUNDARY EDGE NAILING AT PERIMETER

1B Field Built Truss Block Detail
1" = 1'-0"



2X4 BLOCKING BETWEEN TRUSSES. TOENAIL TO TRUSS TOP CHORD EA END W/ (3) 0.148"⌀ X 3 1/2" NAILS.

TRUSS

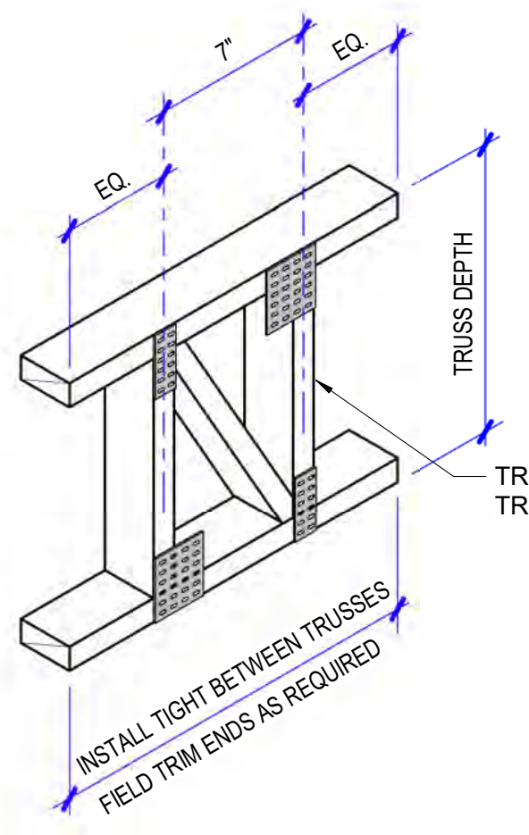
2X4 BRACE ATTACH TO BLOCKING W/ (3) 0.148"⌀ X 3 1/2" NAILS EA END.

2X4 FLATWISE CRIPPLE AT 12" O.C MAX. CRIPPLE MAY BE OMITTED FOR TRUSS BAYS 12" AND LESS.

2X4 BLOCKING BETWEEN TRUSSES. TOENAIL TO TRUSS BOTT CHORD EA END W/ (3) 0.148"⌀ X 3 1/2" NAILS.

TRUSS

1C 2x4 Diagonal Truss Brace
1" = 1'-0"

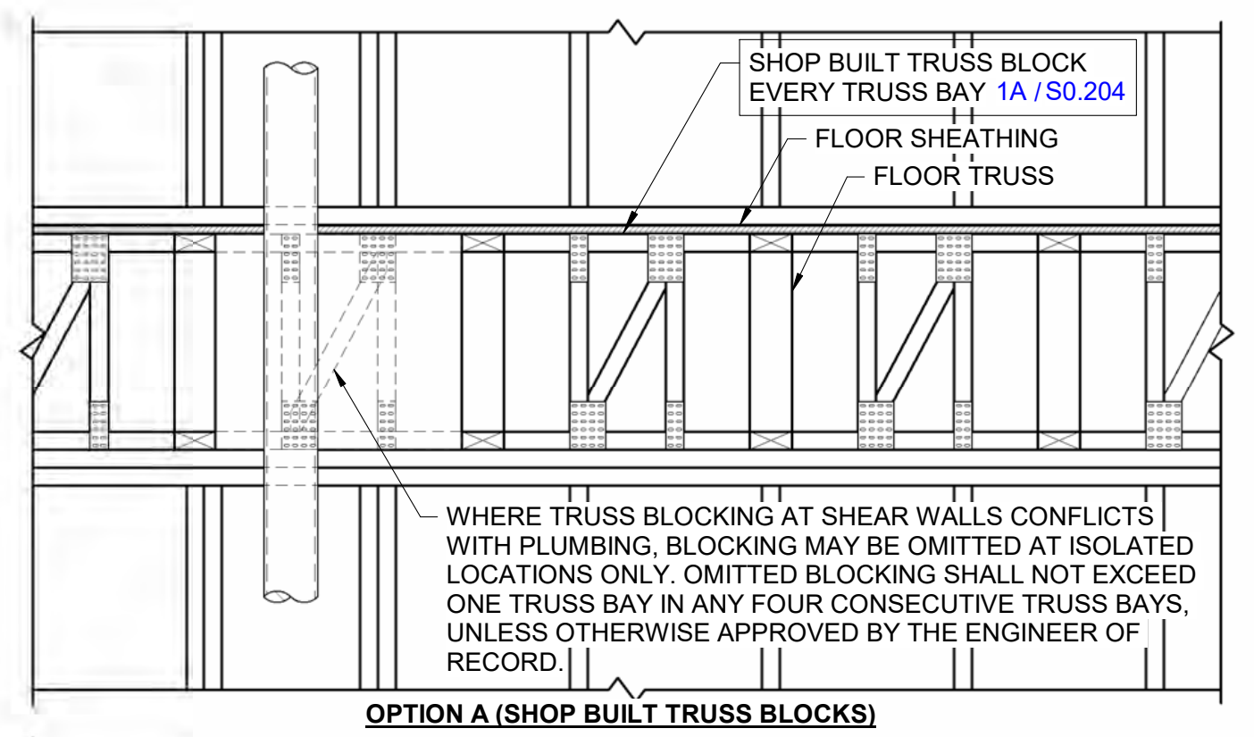


TRUSS BLOCK BY TRUSS MANUFACTURER

1A Shop Built Truss Block Detail
1" = 1'-0"

1 Bottom Chord Bearing Blocking
3/4" = 1'-0"

TRUSS BLOCKING AT SHEAR WALL LOCATIONS

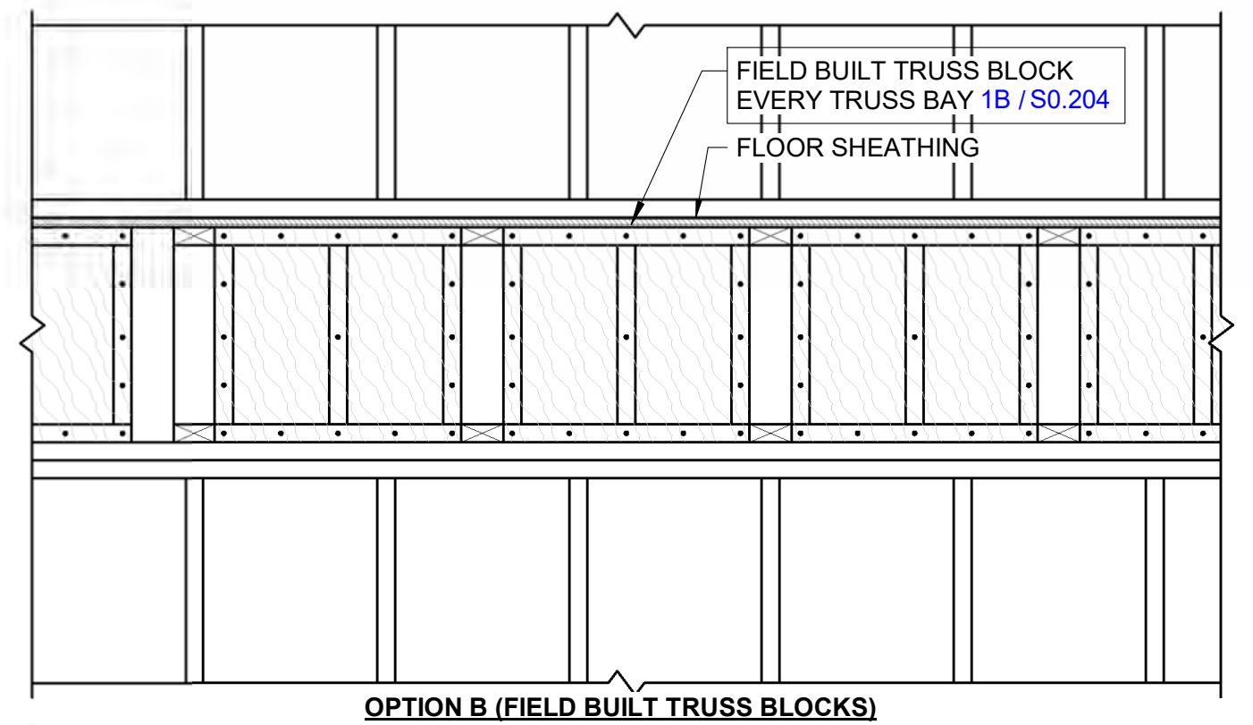

SHOP BUILT TRUSS BLOCK EVERY TRUSS BAY 1A / S0.204

FLOOR SHEATHING

FLOOR TRUSS

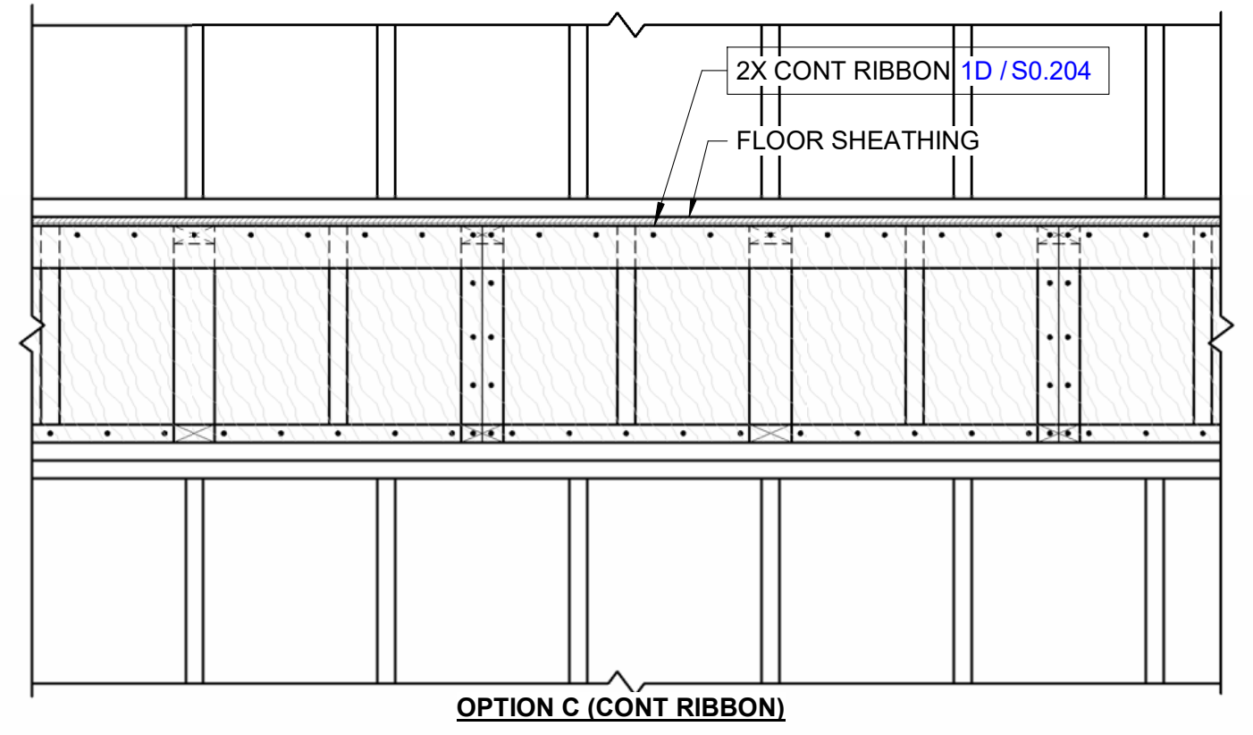
WHERE TRUSS BLOCKING AT SHEAR WALLS CONFLICTS WITH PLUMBING, BLOCKING MAY BE OMITTED AT ISOLATED LOCATIONS ONLY. OMITTED BLOCKING SHALL NOT EXCEED ONE TRUSS BAY IN ANY FOUR CONSECUTIVE TRUSS BAYS, UNLESS OTHERWISE APPROVED BY THE ENGINEER OF RECORD.

OPTION A (SHOP BUILT TRUSS BLOCKS)


FIELD BUILT TRUSS BLOCK EVERY TRUSS BAY 1B / S0.204

FLOOR SHEATHING

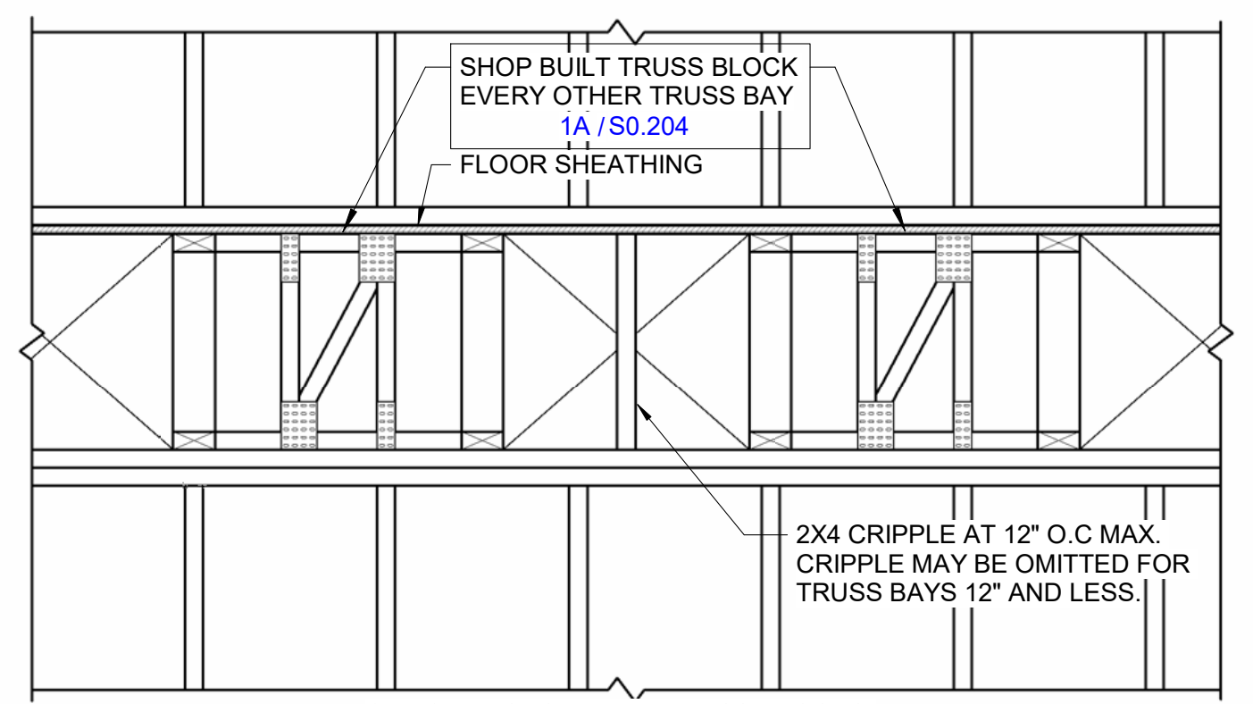
OPTION B (FIELD BUILT TRUSS BLOCKS)


2X CONT RIBBON 1D / S0.204

FLOOR SHEATHING

OPTION C (CONT RIBBON)

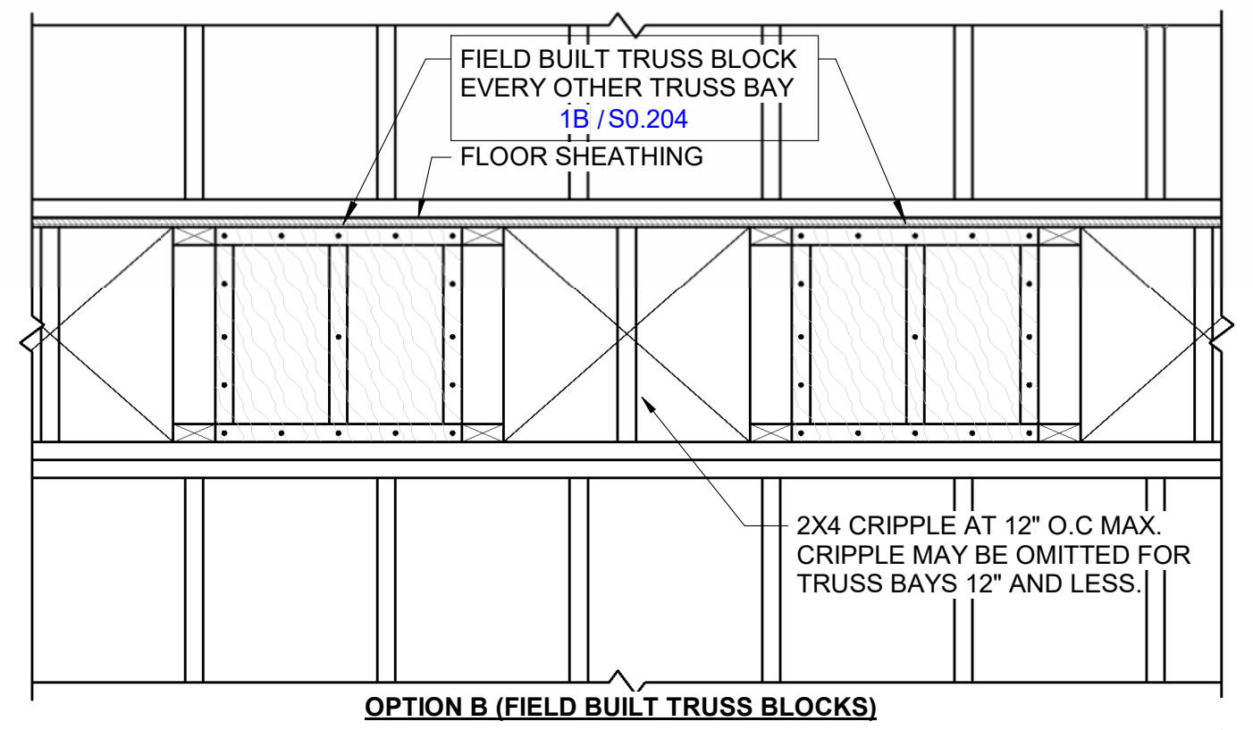
TRUSS BLOCKING AT NON-SHEAR WALL LOCATIONS


SHOP BUILT TRUSS BLOCK EVERY OTHER TRUSS BAY 1A / S0.204

FLOOR SHEATHING

2X4 CRIPPLE AT 12" O.C MAX. CRIPPLE MAY BE OMITTED FOR TRUSS BAYS 12" AND LESS.

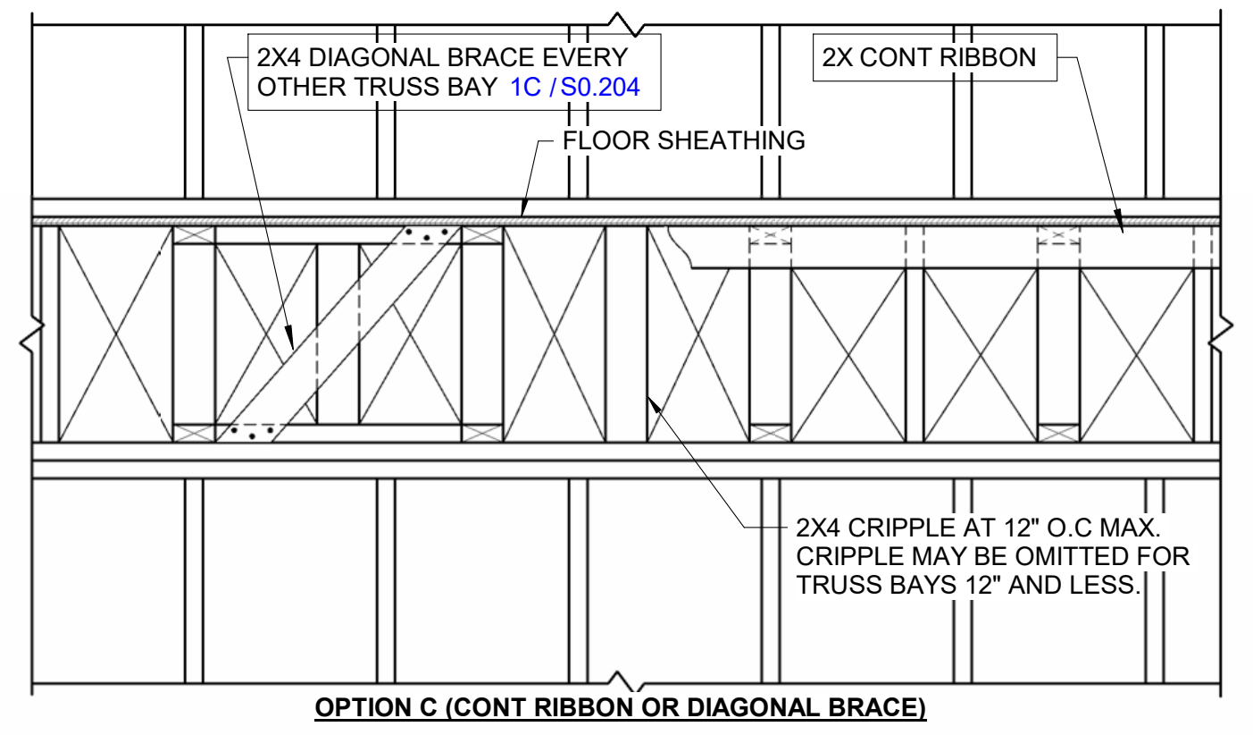
OPTION A (SHOP BUILT TRUSS BLOCKS)


FIELD BUILT TRUSS BLOCK EVERY OTHER TRUSS BAY 1B / S0.204

FLOOR SHEATHING

2X4 CRIPPLE AT 12" O.C MAX. CRIPPLE MAY BE OMITTED FOR TRUSS BAYS 12" AND LESS.

OPTION B (FIELD BUILT TRUSS BLOCKS)


2X4 DIAGONAL BRACE EVERY OTHER TRUSS BAY 1C / S0.204

2X CONT RIBBON

FLOOR SHEATHING

2X4 CRIPPLE AT 12" O.C MAX. CRIPPLE MAY BE OMITTED FOR TRUSS BAYS 12" AND LESS.

OPTION C (CONT RIBBON OR DIAGONAL BRACE)

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Typical Framing Details - Wood

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STUD SCHEDULE (SPF No. 2)

	CORR w/UNIT BEARING		ALL OTHER CORRIDOR WALLS		UNIT DEMISING @ S TYPE UNITS		ALL OTHER DEMISING WALLS		UNIT INTERIOR		EXTERIOR LB	EXTERIOR NON-LB
	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x6	2x6
Level 5	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.
Level 4	1@12"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.
Level 3	2@16"o.c.	1@16"o.c.	1@12"o.c.	1@16"o.c.	1@12"o.c.	1@16"o.c.	2@16"o.c.	1@16"o.c.	2@16"o.c.	1@16"o.c.	1@16"o.c.	1@16"o.c.
Level 2	2@12"o.c.	1@16"o.c.	1@12"o.c.	1@16"o.c.	2@16"o.c.	1@16"o.c.	2@12"o.c.	1@16"o.c.	2@12"o.c.	1@12"o.c.	1@12"o.c.	1@16"o.c.
Level 1	3@12"o.c.	1@12"o.c.	2@16"o.c.	1@16"o.c.	2@12"o.c.	1@16"o.c.	3@12"o.c.	1@12"o.c.	4@12"o.c.	2@16"o.c.	2@16"o.c.	1@16"o.c.

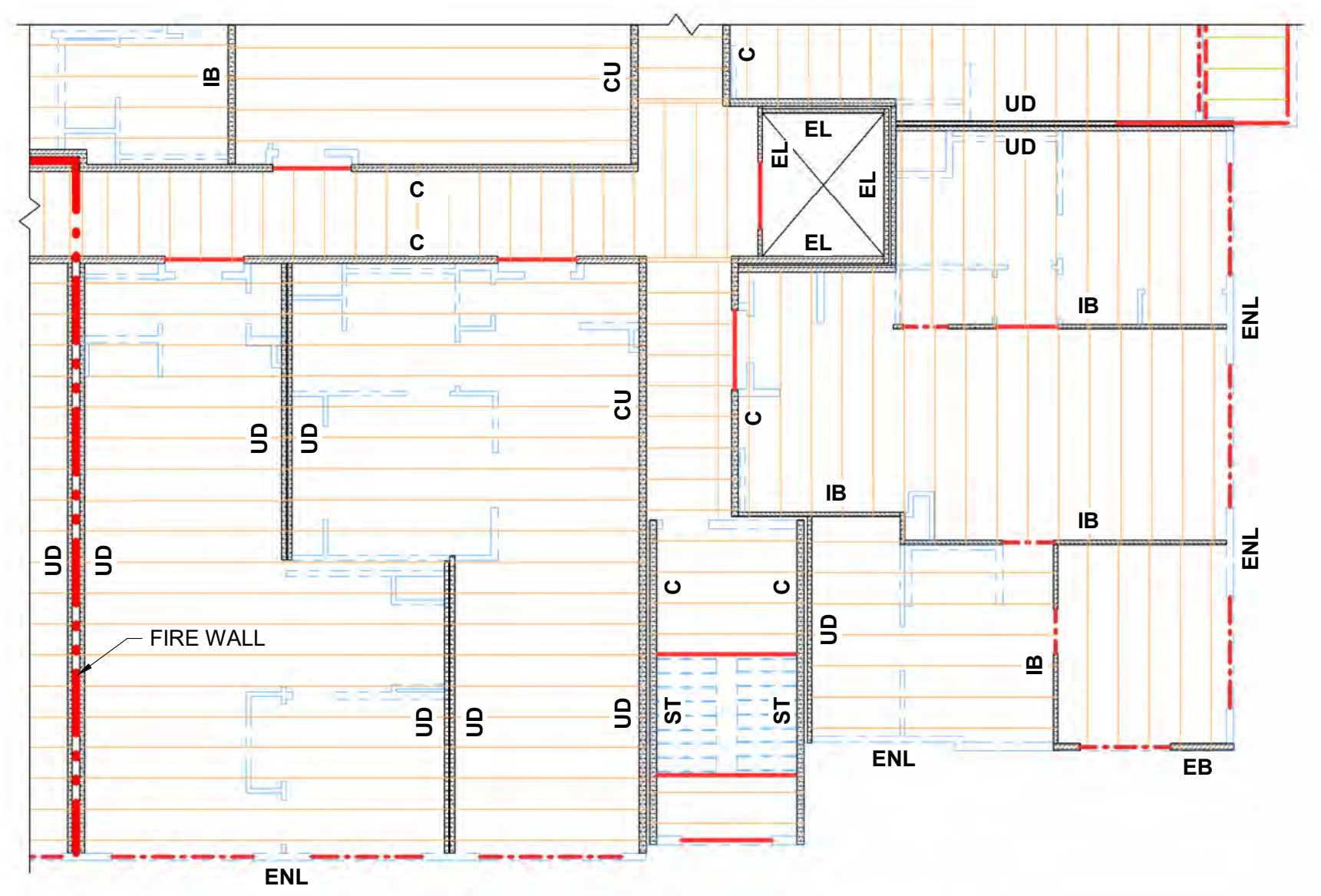
WD9.0 WOOD WALL STUD SCHEDULE NOTES:

A. STUD SIZES INDICATED ARE THE MINIMUM REQUIRED STRUCTURAL SIZE. CONTRACTOR TO VERIFY STUD WALL WIDTH WITH SIZES INDICATED ON ARCHITECTURAL DRAWINGS PRIOR TO WALL PANEL FABRICATION AND / OR INSTALLATION.
B. LOAD BEARING WALLS ARE SHOWN AS SHADED ON PLAN.
C. EXTERIOR LOAD BEARING WALLS ARE THOSE WITH FLOOR / ROOF TRUSSES ORIENTED PERPENDICULAR TO EXTERIOR WALL.
D. EXTERIOR NON-LOAD BEARING (NLB) WALLS ARE THOSE WITH FLOOR / ROOF TRUSSES ORIENTED PARALLEL TO EXTERIOR WALL.
E. STUDS MAY BE SOLID SAWN OR FINGER JOINTED LUMBER OF THE GRADE AND SPECIES SPECIFIED HEREIN.
F. ALL SHEAR WALLS SHALL HAVE A MAXIMUM STUD SPACING OF 16" O.C.. REFER TO BRACING PLANS FOR SHEAR WALL LOCATIONS / LENGTHS AS NOT ALL SHEAR WALLS ARE LOAD BEARING WALLS.
G. REFER TO SHEAR WALL END ANCHORAGE PLANS FOR ADDITIONAL FRAMING MEMBERS REQUIRED AT SHEAR WALL ENDS.
H. SEE FRAMING PLANS AND GENERAL NOTES FOR ADDITIONAL FRAMING MEMBERS NOT SHOWN OR REFERENCED HEREIN.
I. STAIRWELL WALL FRAMING: PROVIDE (2)2X4 @ 12" OC OR (1)2X6 @ 16" O.C. (UNO) FOR WALL HEIGHTS UP TO 13'-6".
J. * INDICATES 2X4 STAGGERED STUDS ON A 2X6 PLATE
K. IT IS STRUCTURALLY ACCEPTABLE TO LOCATE RESIDENTIAL UNIT ELECTRICAL PANELS WITHIN 14 1/2" CLEAR STUD BAY. CONTRACTOR TO COORDINATE ELECTRICAL PANEL LOCATIONS PRIOR TO WALL PANEL FABRICATION AND OR INSTALLATION.
L. ALL INTERIOR NON-LOAD BEARING WALLS SHALL BE (1)2X4 @ 24" O.C. MAX UNO ON PLAN (TYPICAL)
M. AT LOCATIONS WHERE THE CORRIDOR WIDTH EXCEEDS 7'-0" WIDTH OR WITHOUT UNIT BEARING, PROVIDE "CORRIDOR WITH UNIT BEARING" WALL FRAMING (TYPICAL)
N. AT 2X6 AND 2X8 INTERIOR BEARING WALLS, THE FOLLOWING SCHEDULE APPLIES WITH STUDS SPACED AT 16" O.C. MAXIMUM. STUDS IDENTIFIED WITH * SHALL BE SPACED AT 12" O.C.

INTERIOR BEARING WALL SCHEDULE

LEVEL	6X WALL	8X WALL
5	(1) 2X6	(1) 2X8
4	(1) 2X6	(1) 2X8
3	(1) 2X6	(1) 2X8
2	(2) 2X6	(1) 2X8
1	(2) 2X6	(1) 2X8*

WD9.1 WOOD WALL STUD LAYOUT KEY PLAN:



STUD KEY ABBREVIATIONS

MARK	DESCRIPTION
C	CORRIDOR WALL
CU	CORRIDOR WALL W/ UNIT BEARING
UD	UNIT DEMSING WALL
IB	INTERIOR BEARING WALL
EB	EXTERIOR BEARING WALL
ENL	EXTERIOR NON-LOAD BEARING WALL
ST	STAIR WALL
EL	ELEVATOR WALL

WD10.0 WOOD 2X BOTTOM / SILL PLATE ANCHORAGE TO CONCRETE

BOTTOM / SILL PLATE ANCHORAGE TO CONCRETE

ANCHORAGE OPTIONS	SHEAR WALLS	NON-SHEAR WALLS		
		EXTERIOR WALLS **(EB / ENL)**	ALL INTERIOR LOAD BEARING WALLS **(IB, C, CU, UD, ST, EL)**	INTERIOR NON-LOAD BEARING WALLS
5/8"Ø x 8" EMBEDMENT J-BOLTS	SEE SHEAR WALL SCHEDULE	48" O.C.	48" O.C.	72" O.C.
5/8"Ø FLUSH TOP SILL BOLTS BY CHUBBYSMACK	SEE SHEAR WALL SCHEDULE	48" O.C.	48" O.C.	72" O.C.
5/8"Ø x 5" TITEN HD ANCHORS	SEE SHEAR WALL SCHEDULE	48" O.C.	48" O.C.	72" O.C.
MASA / MASAP MUDSILL ANCHORS	SEE SHEAR WALL SCHEDULE	24" O.C.	36"O.C.	54" O.C.
PDPAWL-287MG (0.157" x 2 7/8") P.A.F. W/ WASHER	SEE SHEAR WALL SCHEDULE	10" O.C.	18" O.C.	48" O.C.

SILL PLATE ANCHORAGE TO CONCRETE NOTES:

1. FASTENERS FOR PRESERVATIVE-TREATED WOOD SHALL BE AS FOLLOWS:
   A. UNCOATED IF TREATED WITH SBS / DOT & ZINC BORATE.
   B. GALVANIZED (ASTM A153) IF TREATED WITH ACQ-C, ACQ-D (CARBONATE), CA-B & CB-A; WITH OR WITHOUT AMMONIA
   C. STAINLESS STEEL (TYPE 303, 304, 305, OR 316) IF WOODS HAVE ACTUAL RETENTION LEVELS GREATER THAN 0.40 PCF FOR ACQ, 0.41 PCF FOR CBA-A 0.21 PCF FOR CA-B (GROUND CONTACT LEVEL) OR ACZA
2. ALL PLATES TO HAVE MINIMUM (2) ANCHORS PER BOARD AND AN ANCHOR NO FURTHER THAN 12" FROM THE ENDS OF EACH BOARD.
3. AT ALL SHEAR WALLS WITH J-BOLTS, FLUSH TOP SILL BOLTS, OR TITEN HD ANCHORS PROVIDE 3"x3"x1/4" PLATE WASHERS. PROVIDE STANDARD CUT WASHERS AT ALL OTHER LOCATIONS.
4. MASA / MASAP ANCHORS SHALL BE INSTALLED WITH STANDARD INSTALLATION AND 20% MAX OF ONE LEG UP INSTALLATION ALONG WALL LINE (1 OF EVERY 5 MAX) WITH ALL NAIL HOLES FILLED (TYPICAL).
5. AT 2x10 AND WIDER SILL PLATE / SLEEPER PLATES, MASA / MASAP ANCHOR SHALL BE STAGGERED ON EACH SIDE OF 2x PLATE.
6. CONTRACTOR SHALL APPROPRIATELY SURVEY STRUCTURAL SLABS PRIOR TO POST-INSTALLED ANCHOR INSTALLATION TO ENSURE POST-TENSIONING TENDONS ARE NOT DAMAGED DURING ANCHOR INSTALLATION (TYPICAL). IN NO INSTANCE SHALL EXISTING POST-TENSIONING TENDONS BE DAMAGED.
7. CONTRACTOR SHALL FOLLOW ALL MANUFACTURER POST-INSTALLED ANCHOR, P.A.F. AND MASA / MASAP ANCHOR INSTALLATION INSTRUCTIONS, RECOMMENDATIONS, AND PROTOCOLS IN ADDITION TO BUILDING CODE SPECIAL INSPECTION REQUIREMENTS.
8. NO SUBSTITUTIONS OF ANCHOR TYPE, SIZE, MANUFACTURER, OR INSTALLATION METHOD SHALL BE PERMITTED WITHOUT WRITTEN APPROVAL FROM THE ENGINEER OF RECORD.

WD11.0 WOOD POST SCHEDULE NOTES:

A. WHERE POSTS OR STUDPACKS ARE NOT SPECIFIED ON PLAN AT ANY BEARING SUPPORT OF BEAMS, HEADERS (DROP BEAMS), FLUSH BEAMS, AND GIRDER TRUSSES, SEE WOOD FRAMING GENERAL NOTE B THROUGH D FOR MINIMUM SUPPORT MEMBERS REQUIRED.
B. WHERE STUDPACK WIDTHS ARE NOT SPECIFIED, PROVIDE STUD SIZE TO FIT WITHIN ENTIRE WALL CAVITY (IE: PROVIDE 2X6 STUDS AT 6" NOMINAL WALL WIDTHS).
C. POST CAPS AND BASES SHALL BE SIMPSON OR APPROVED EQUAL WITH STRAPS ROTATED AS NEEDED BASED ON POST ORIENTATION.
D. ALL POSTS THAT ARE NOT WITHIN WALL ASSEMBLY SHALL HAVE APPROPRIATE POST CAP AND BASE, TYPICAL. UNO PROVIDE ABU TYPE BASE AT SUPPORT SLAB AND BC TYPE CAP AND BASE AT FRAMING LEVELS.
E. POSTS AND STUDPACKS SHALL BE CONTINUOUS LEVEL BY LEVEL DOWN TO SUPPORTING SLAB WITH SQUASH BLOCKS INSTALLED AT ALL FLOOR/ROOF CAVITIES COMPOSED OF EQUAL SIZE AND MATERIALS TO ENSURE LOAD PATH CONTINUITY.
F. REFER TO ARCHITECTURAL DRAWINGS FOR ALL FIRE PROTECTION REQUIREMENTS, TYPICAL.

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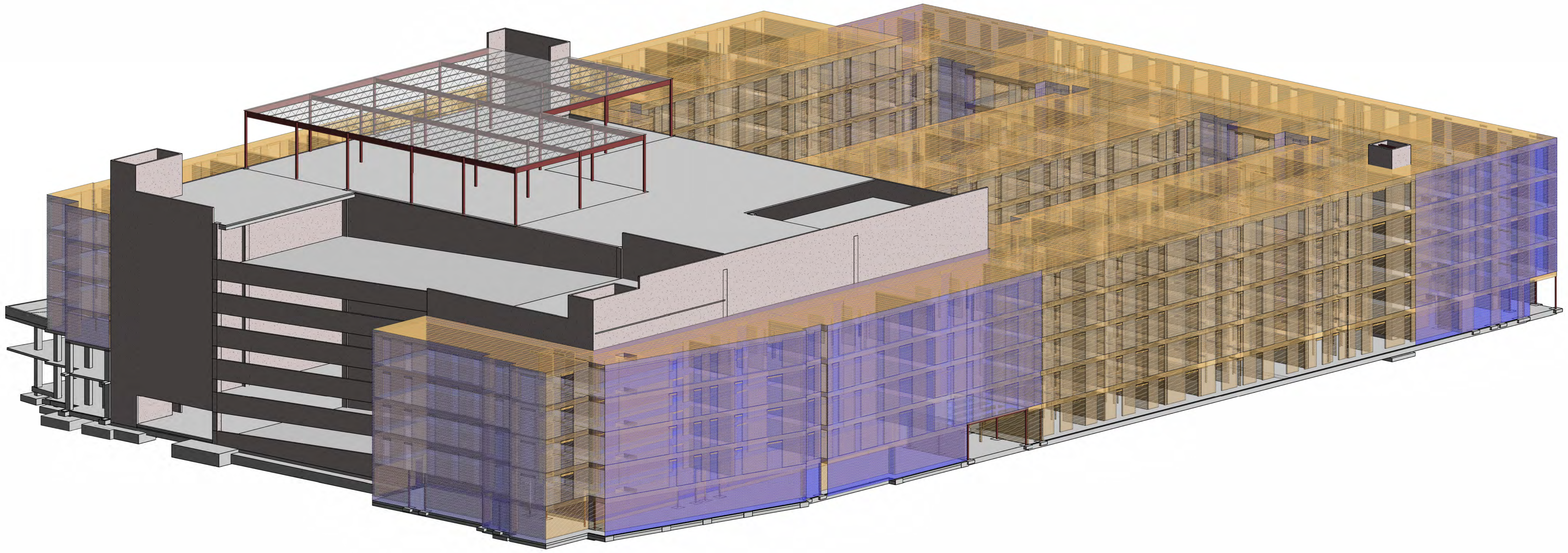
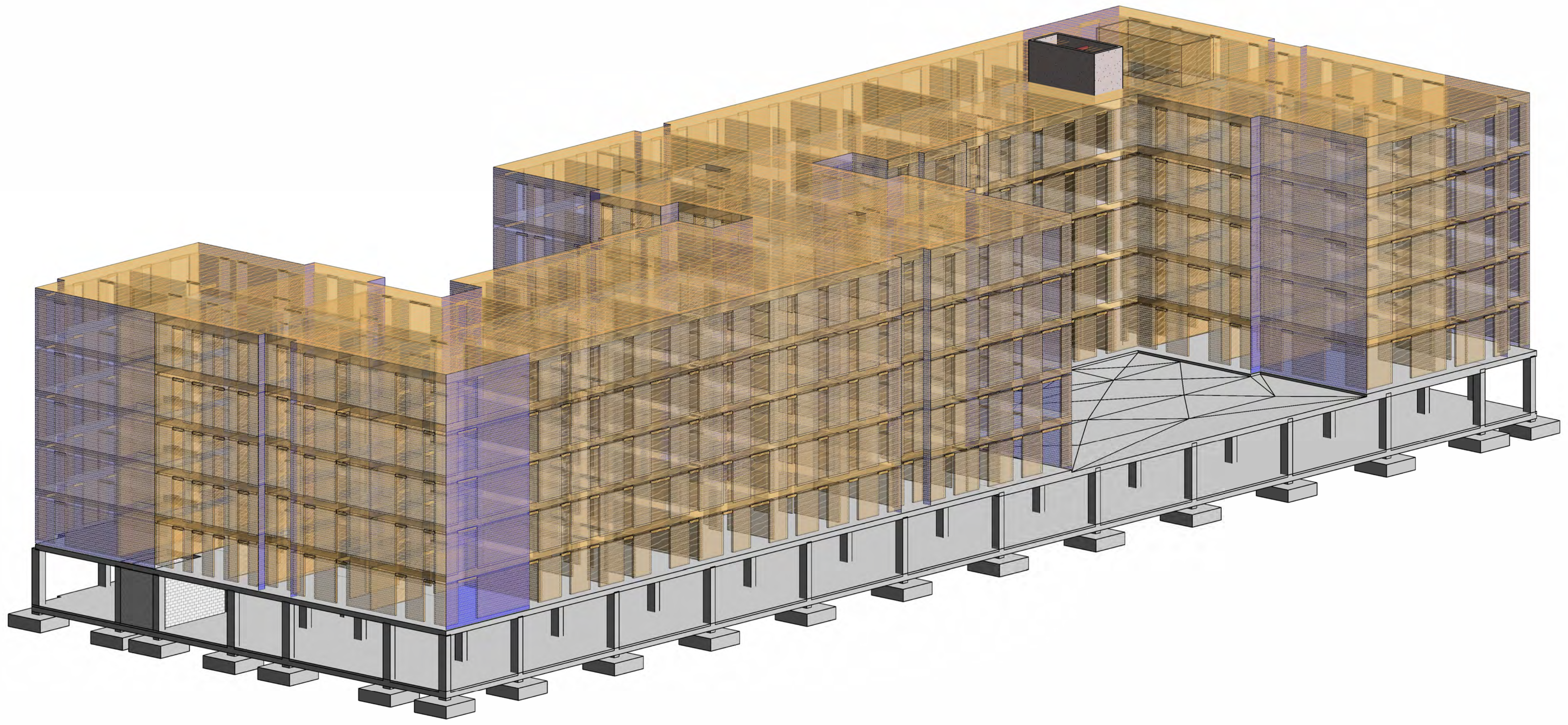


① 3D View - NE Corner - West Building



② 3D View - NE Corner - East Building

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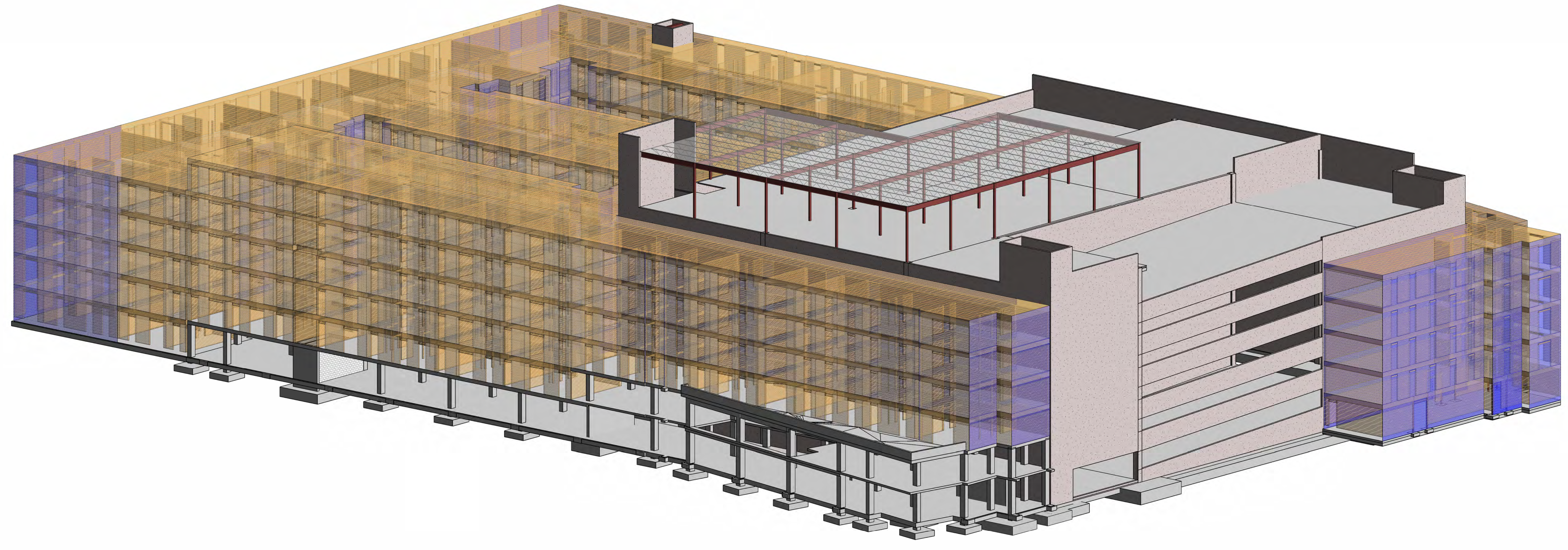
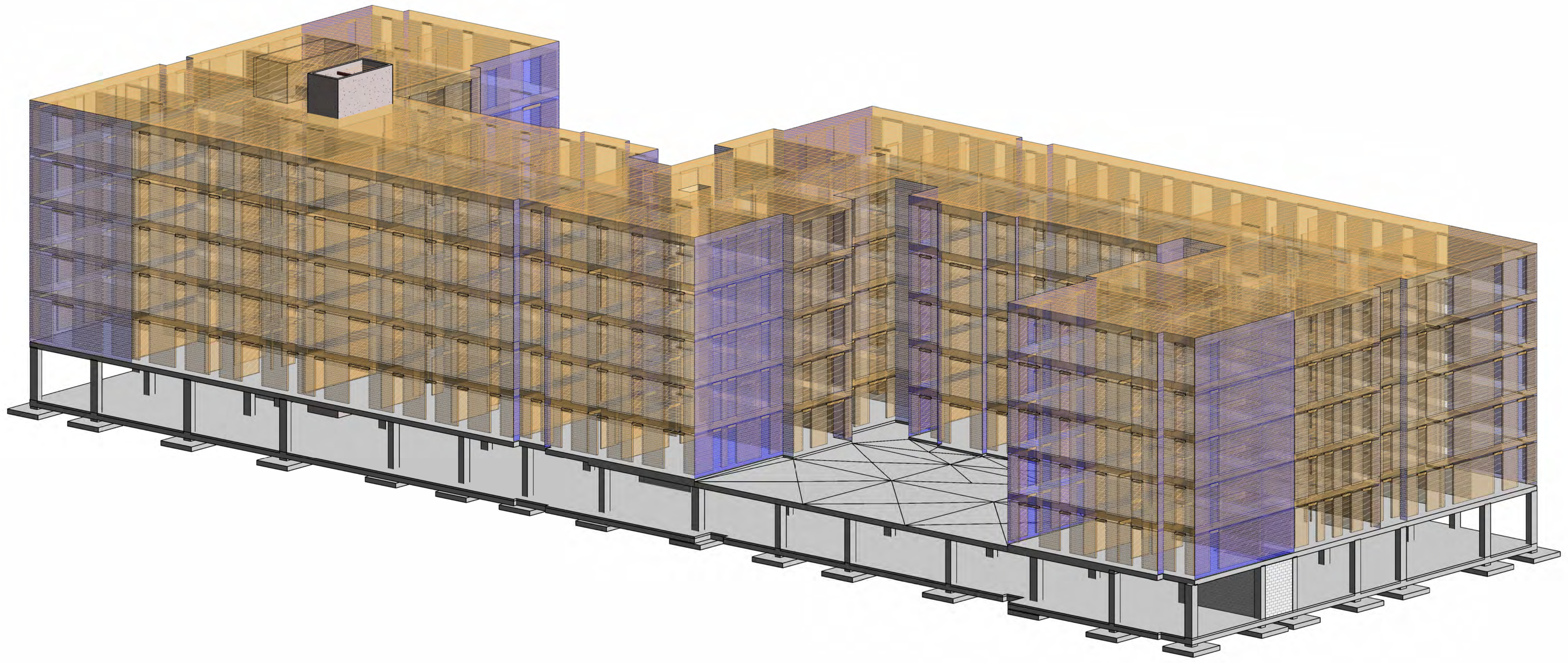


① 3D View - SE Corner - West Building



② 3D View - SE Corner - East Building

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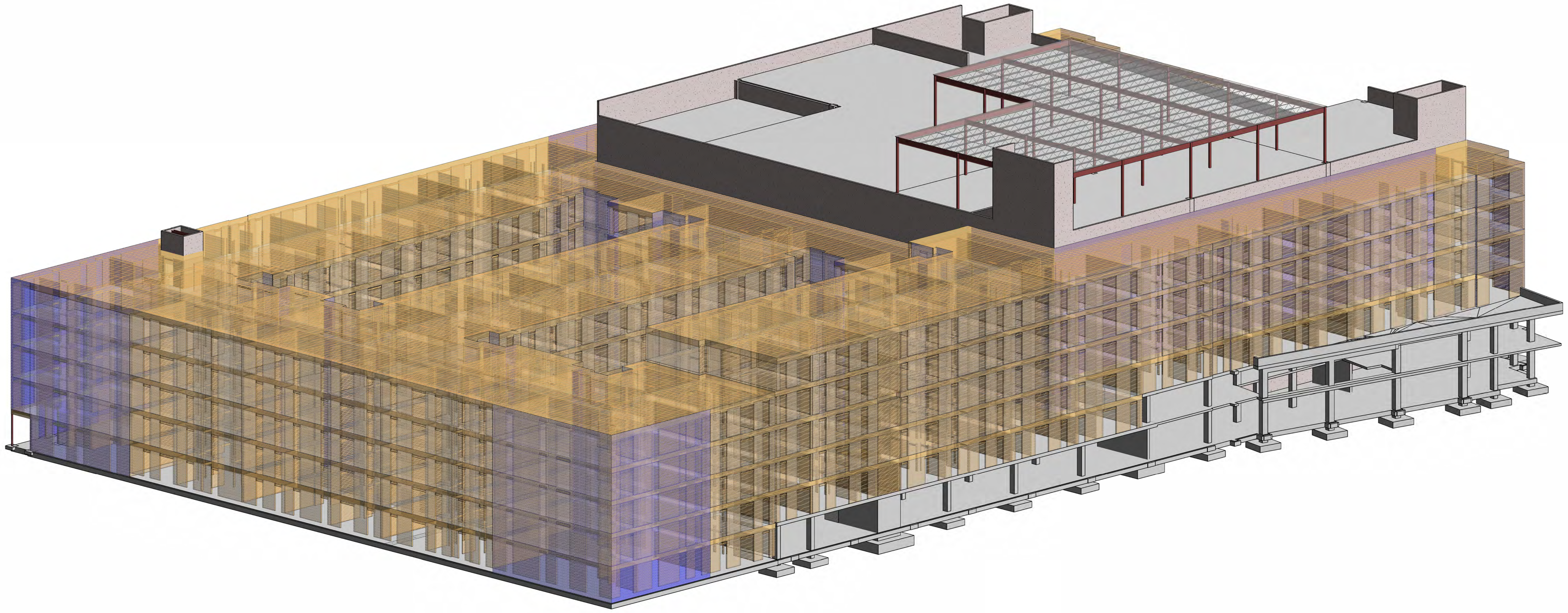
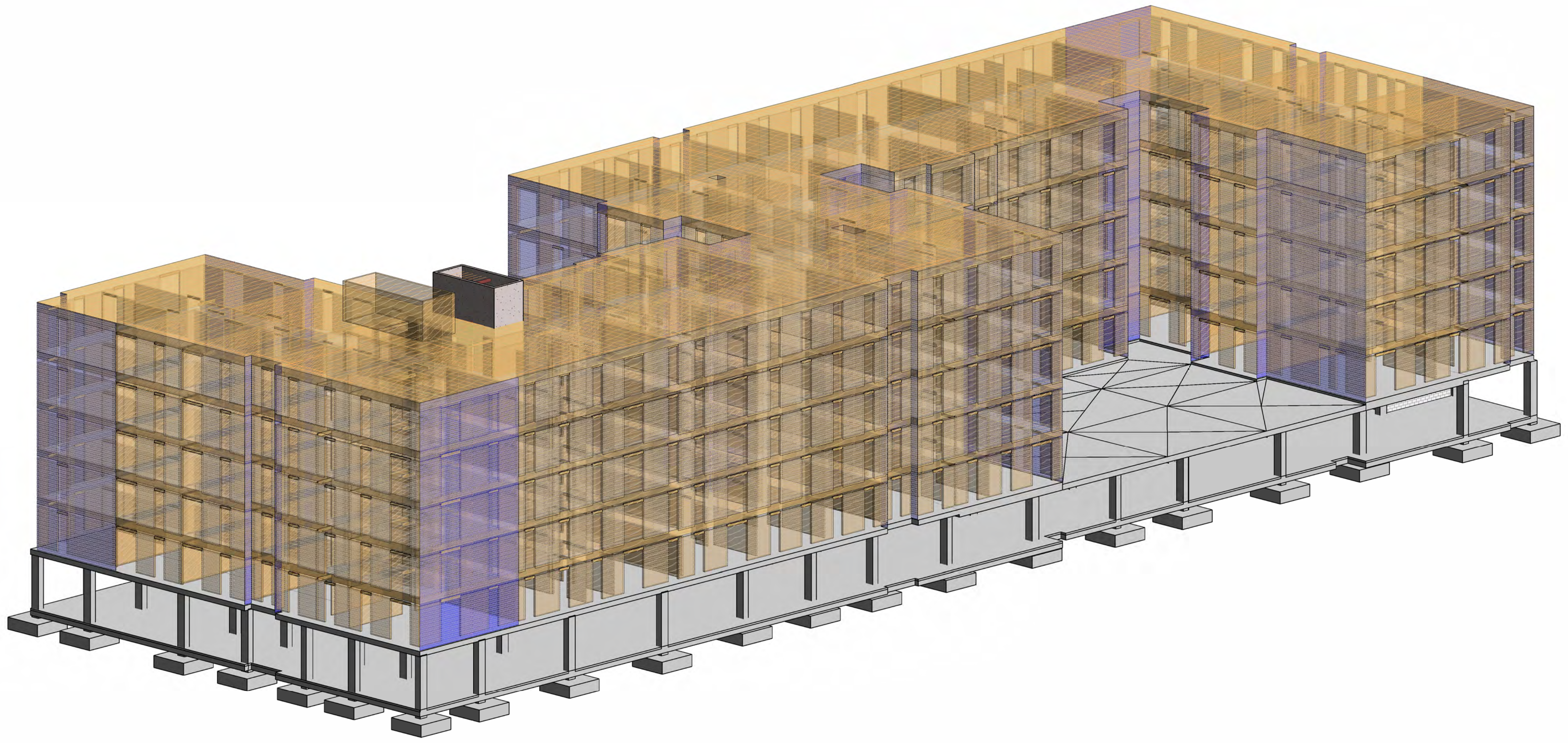


① 3D View - SW Corner - West Building



② 3D View - SW Corner - East Building

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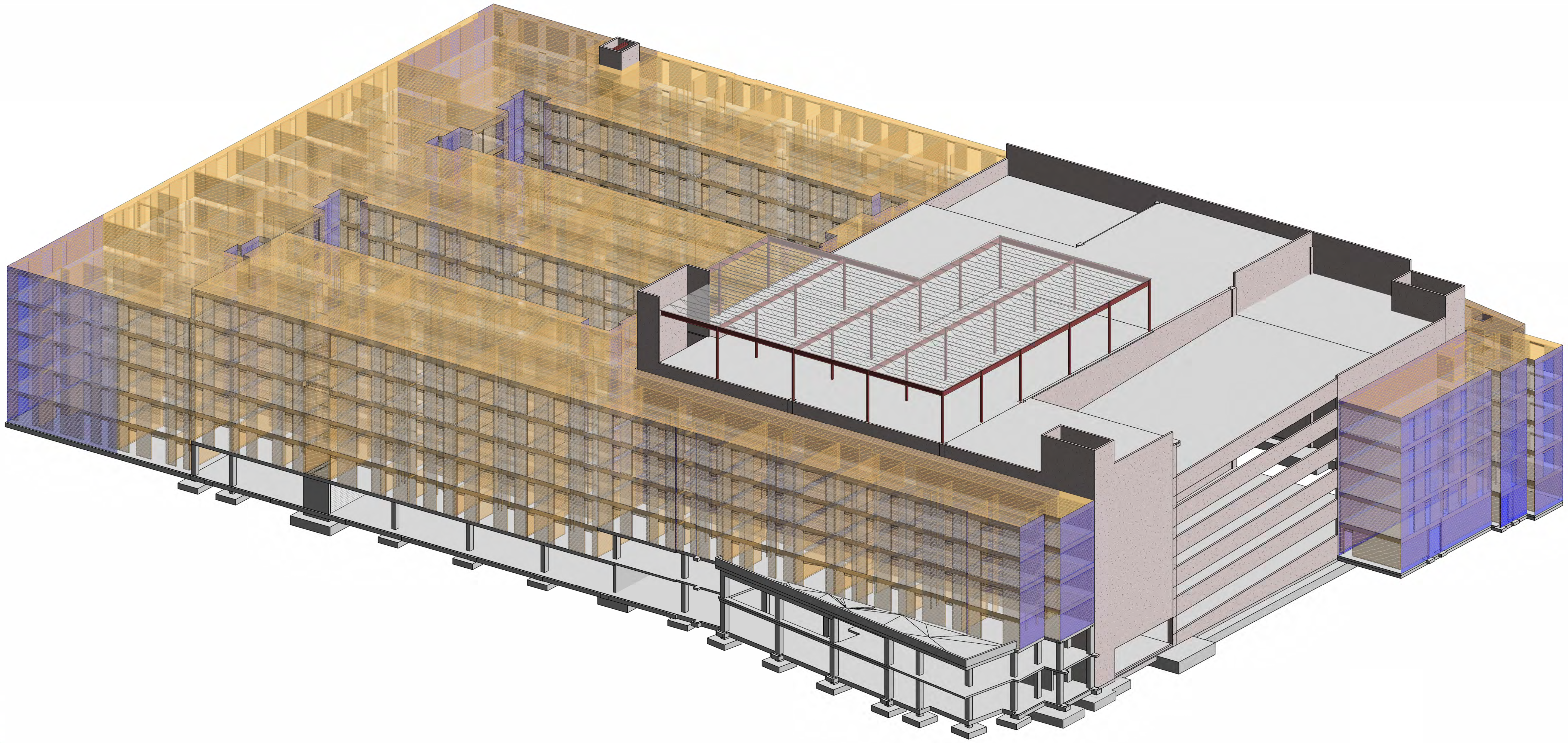
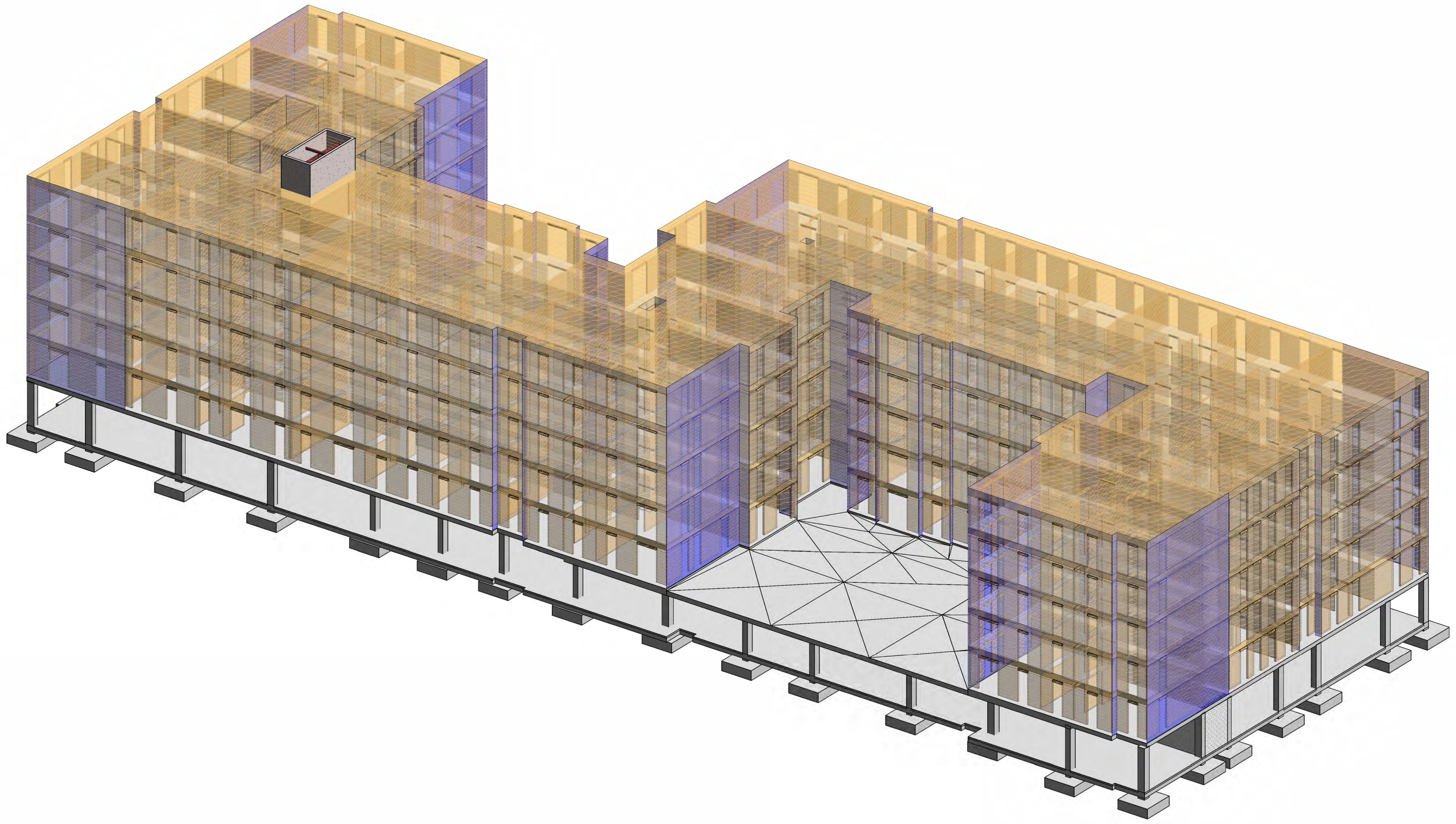


① 3D View - NW Corner - West Building



② 3D View - NW Corner - East Building

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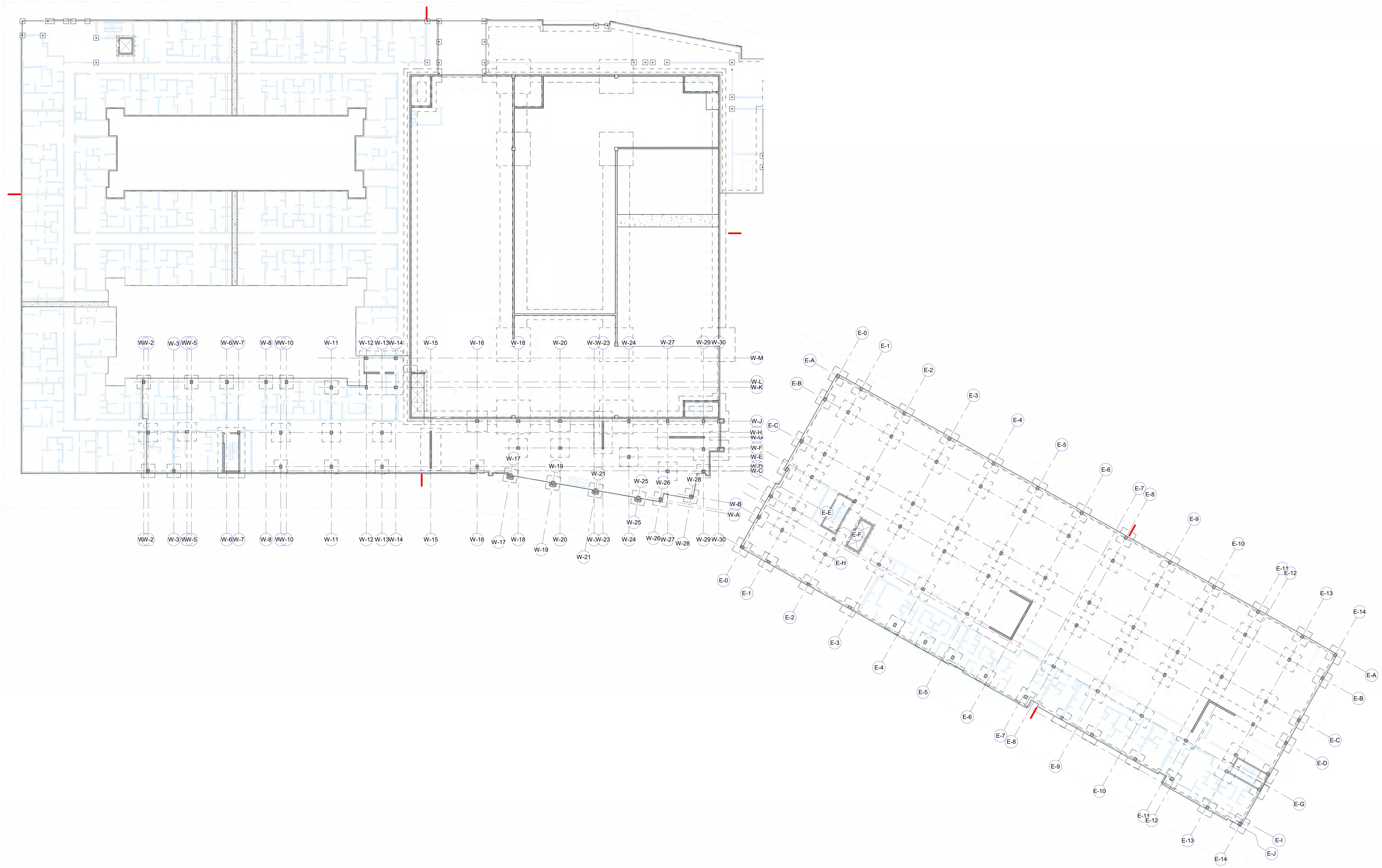

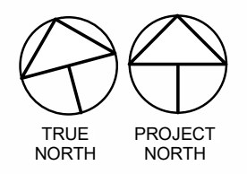

1 Full Site - Foundation Plan - Level 01
1" = 20'-0"

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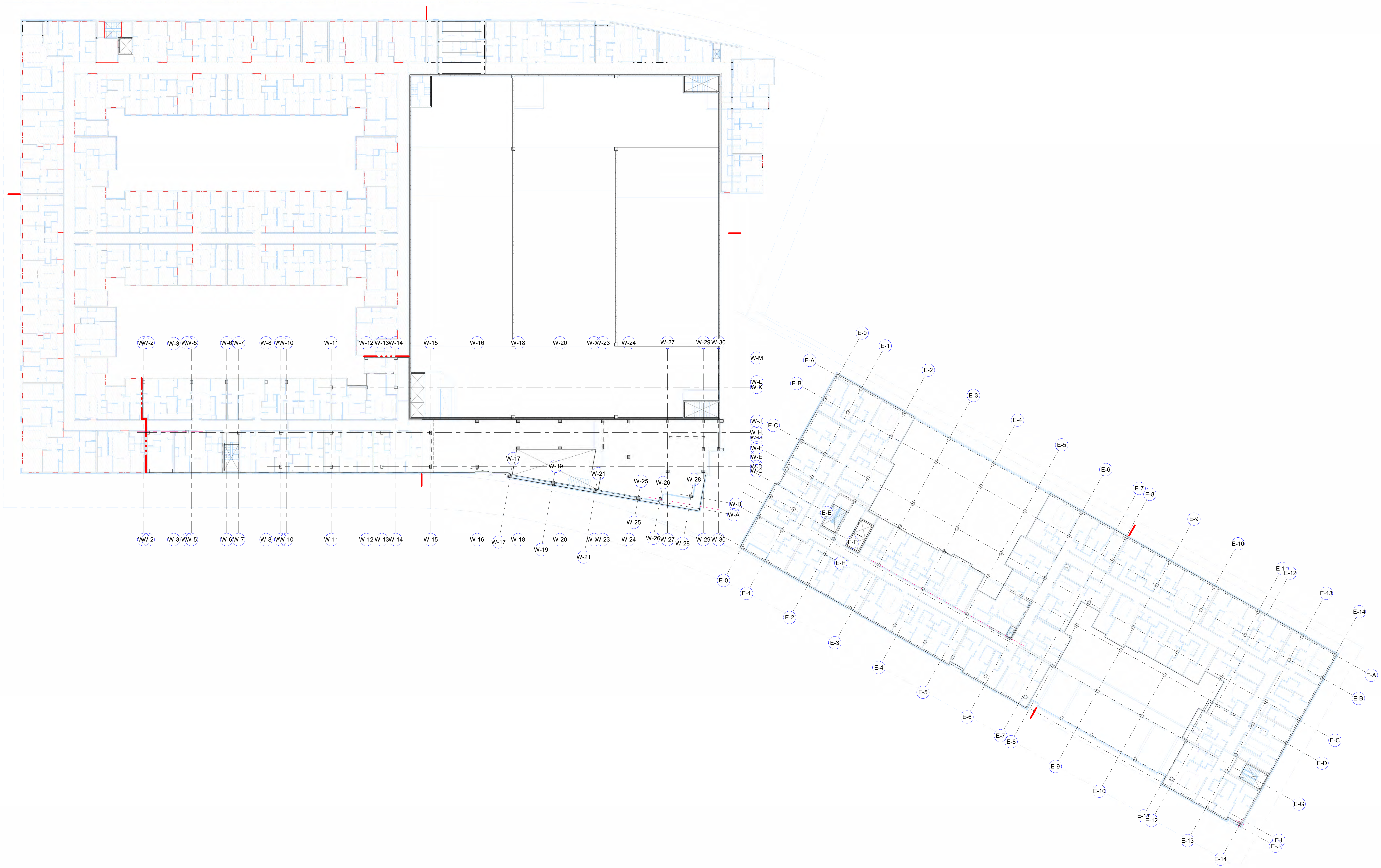

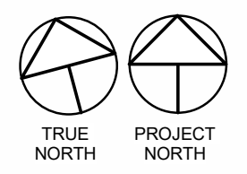

① Full Site - Forming Plan - Level 02
1" = 20'-0"

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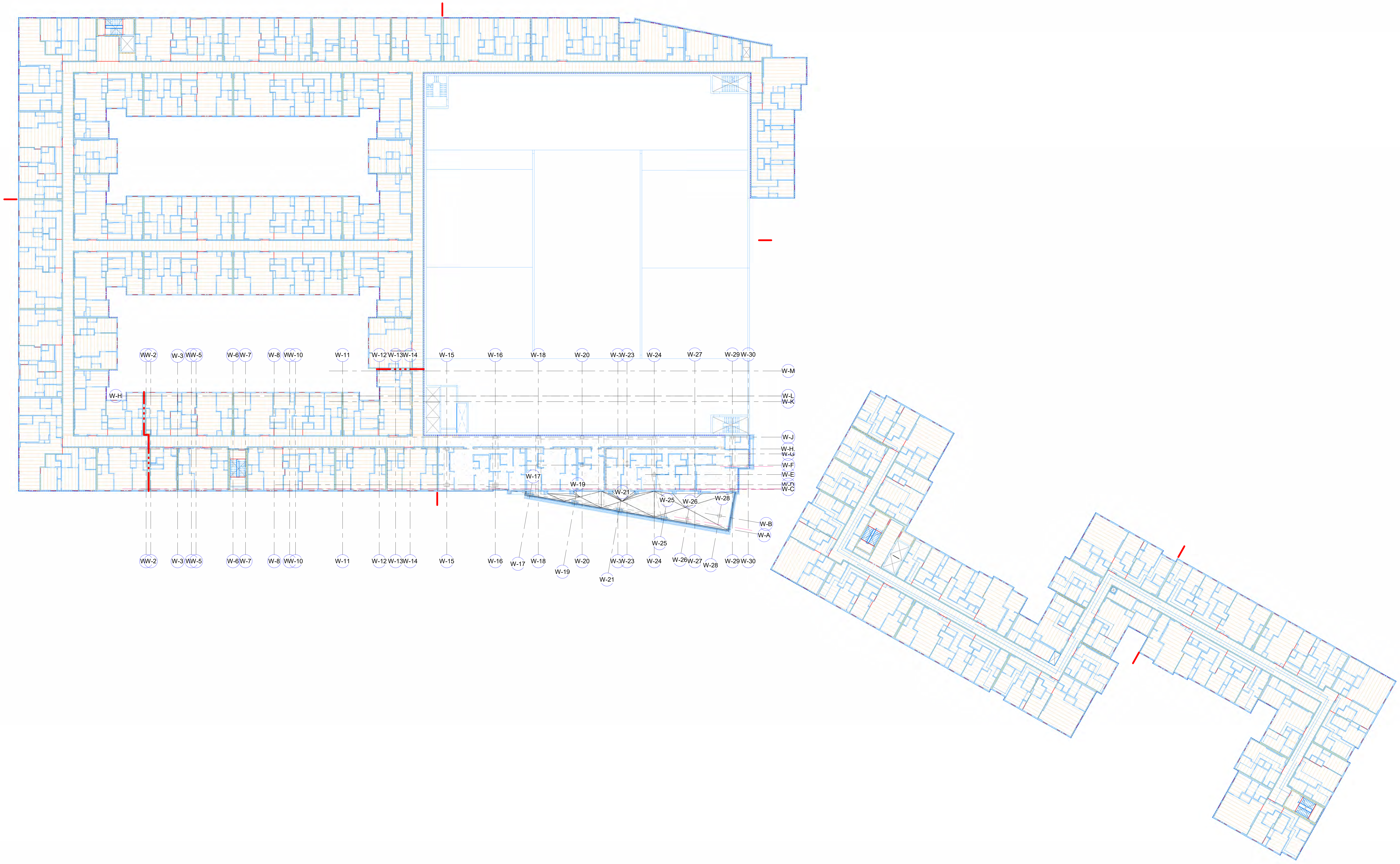

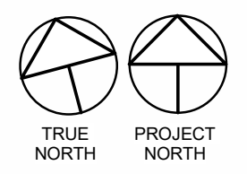

① Full Site - Wood Framing Plan - Level 03
1" = 20'-0"

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Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
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NILES BOLTON ASSOCIATES

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T 404 365 7600

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SHEET TITLE:
Full Site - Wood Framing Plan - Level 03

SHEET NUMBER:
S1.103

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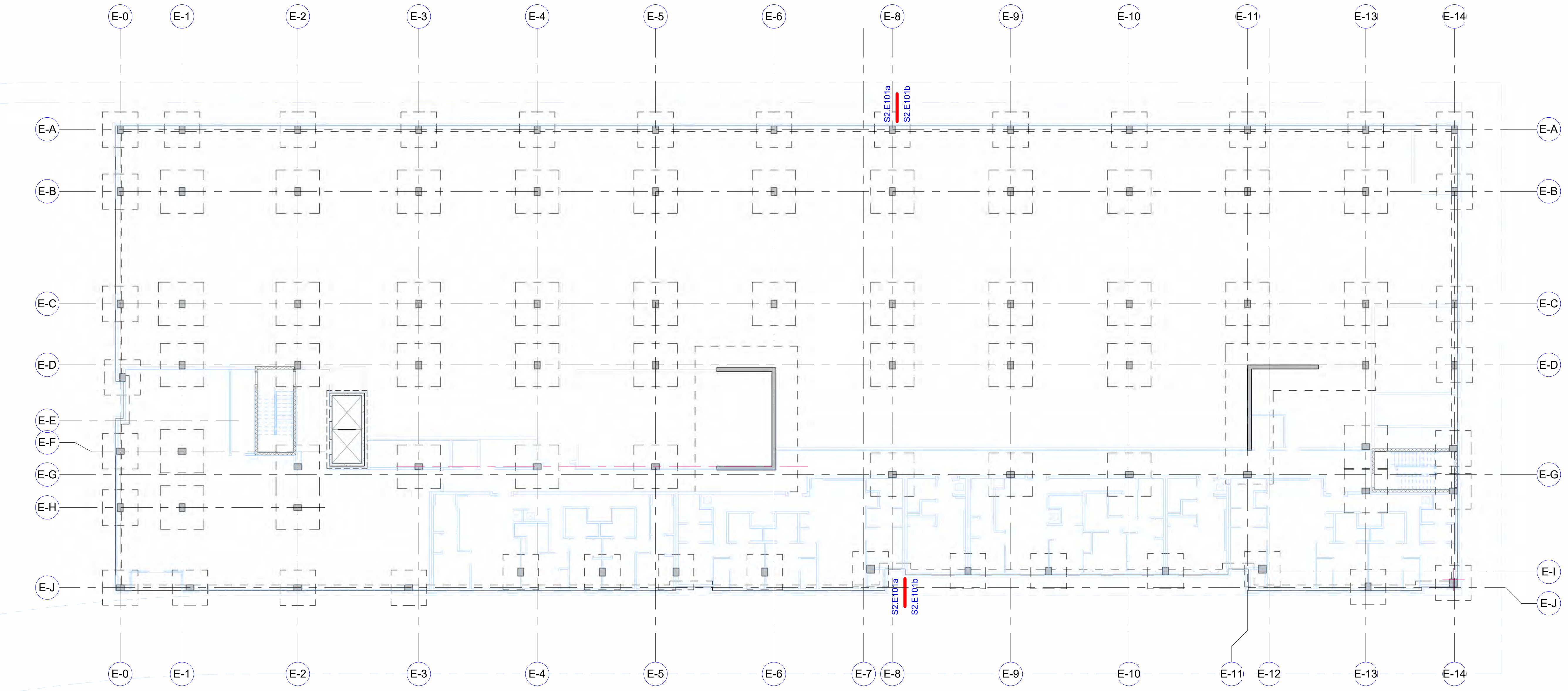

TRUE NORTH
PROJECT NORTH

① Foundation Plan - East Building - Level 01 - OA
1/16" = 1'-0"

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Structural Engineers
VSMeg Structural Engineers, LLC
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Houston, TX 77077
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www.vsmeg.com

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SHEET TITLE:
Foundation Plan - East Building - Level 01 - OA

SHEET NUMBER:
S1.E101

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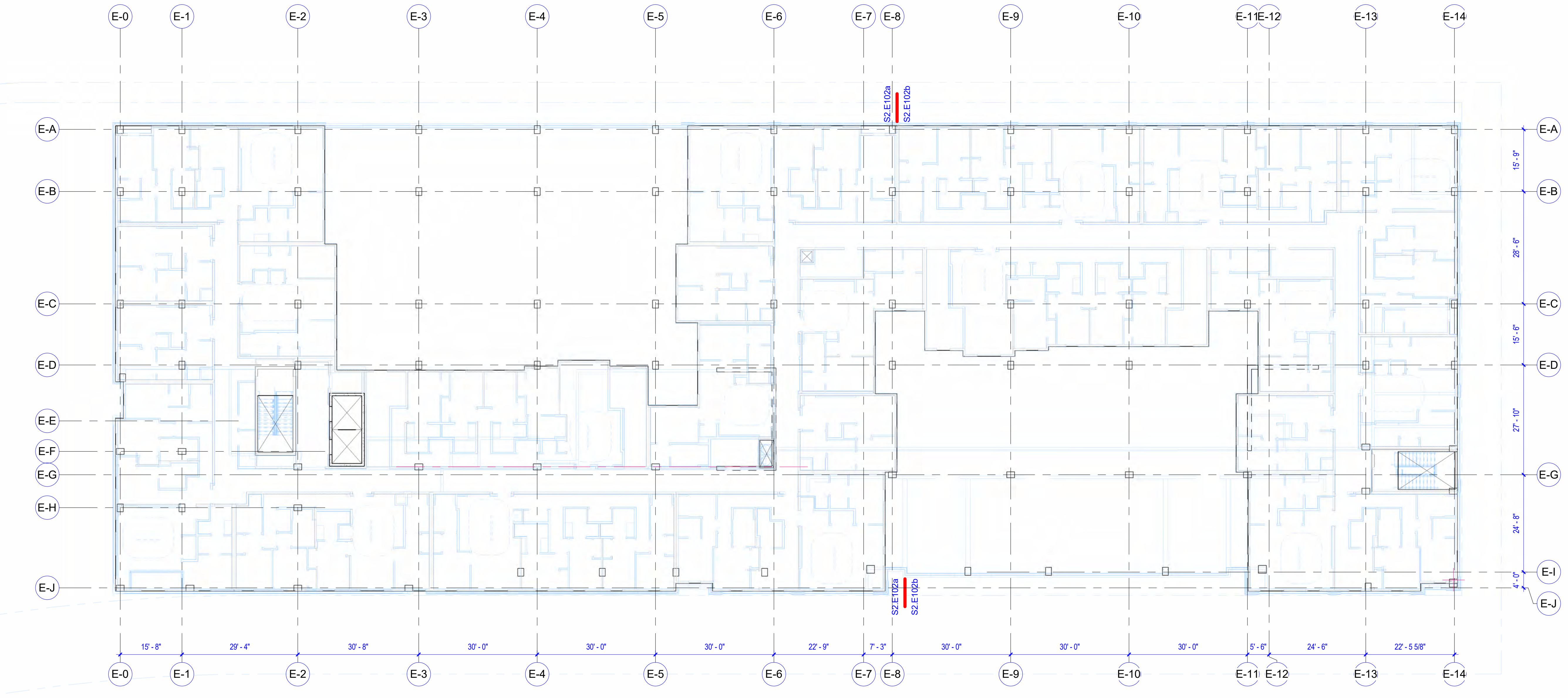

TRUE NORTH
PROJECT NORTH

① Forming Plan - East Building - Level 02 - OA
1/16" = 1'-0"

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- NO. 85617

SHEET TITLE:
Forming Plan - East Building - Level 02 - OA

SHEET NUMBER:
S1.E102

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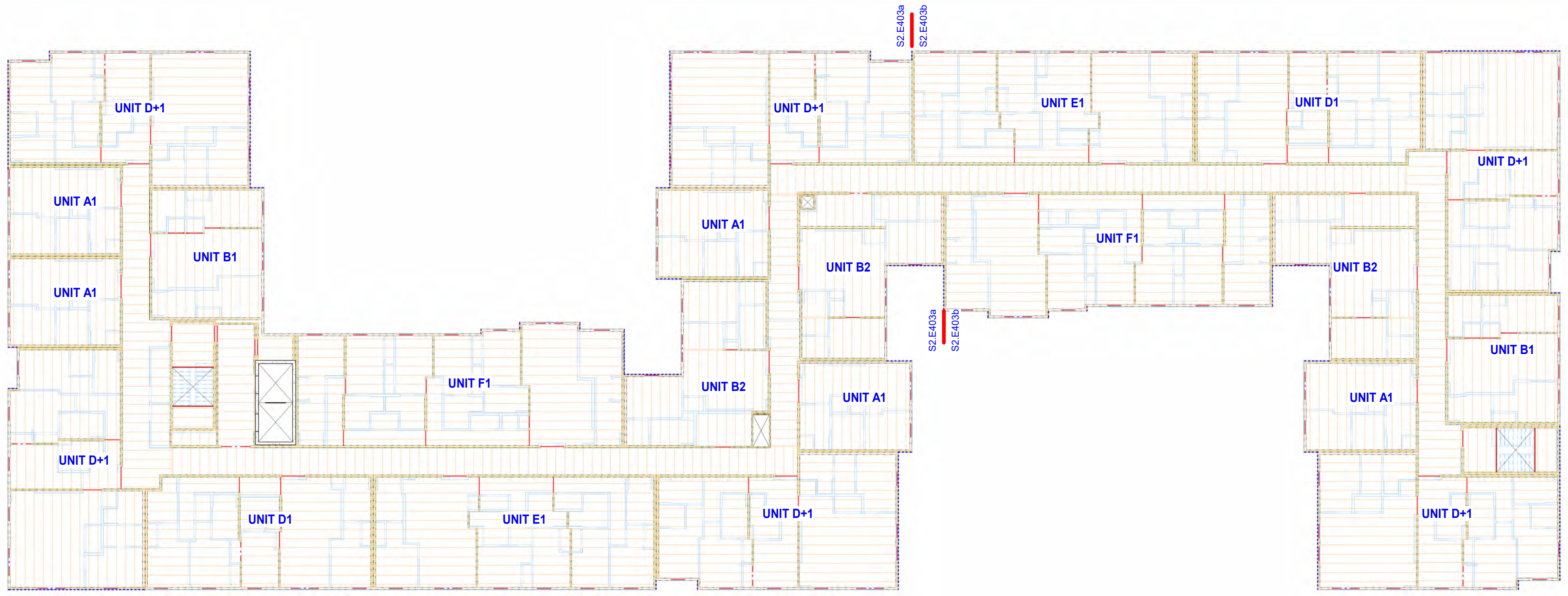

TRUE NORTH
PROJECT NORTH

1 Wood Framing Plan - East Building - Level 03 - OA
1/16" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - East Building - Level 03 - OA

SHEET NUMBER:
S1.E103

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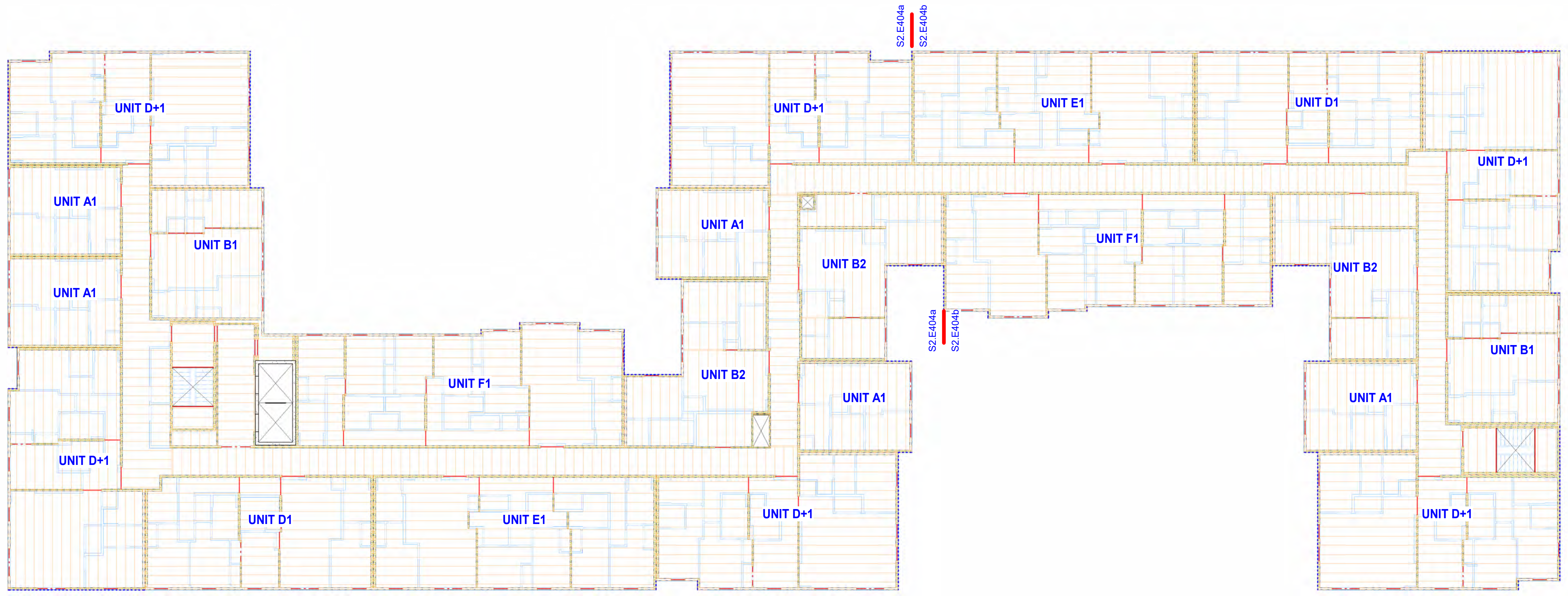

TRUE NORTH
PROJECT NORTH

① Wood Framing Plan - East Building - Level 04 - OA
1/16" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - East Building - Level 04 - OA

SHEET NUMBER:
S1.E104

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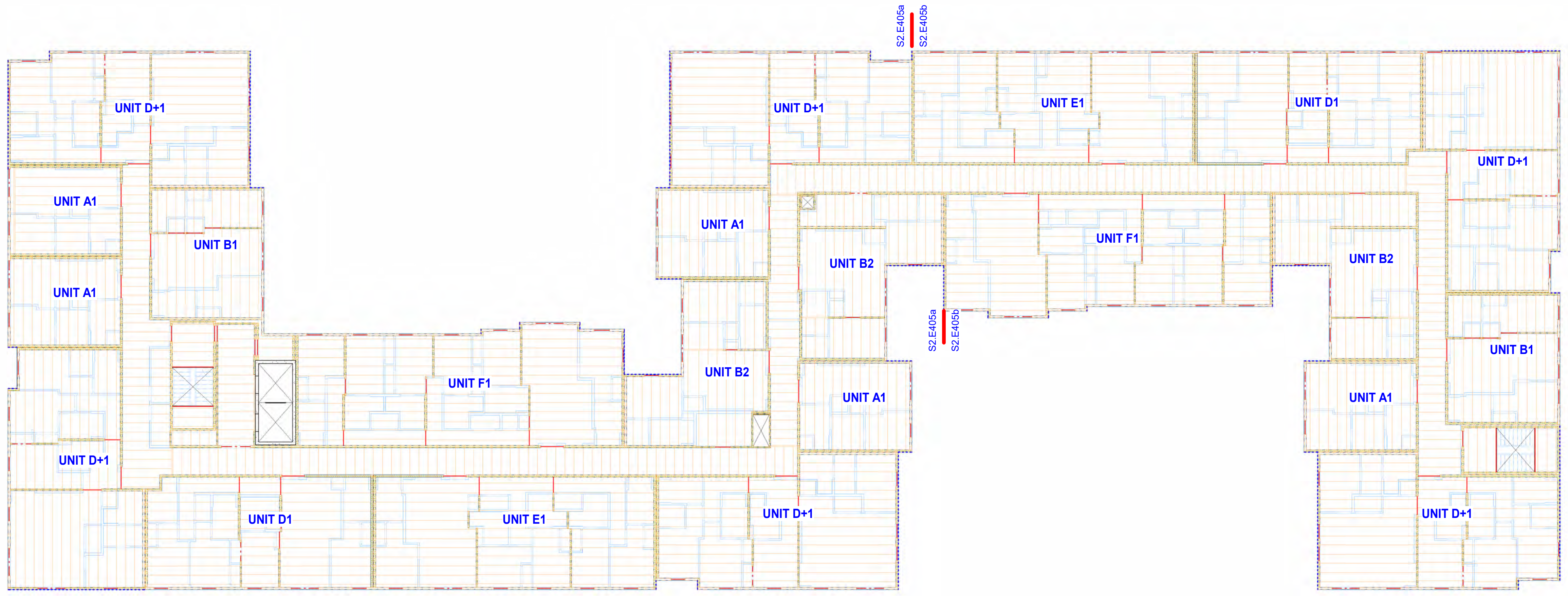

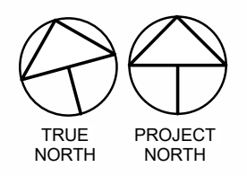

① Wood Framing Plan - East Building - Level 05 - OA
1/16" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - East Building - Level 05 - OA

SHEET NUMBER:
S1.E105

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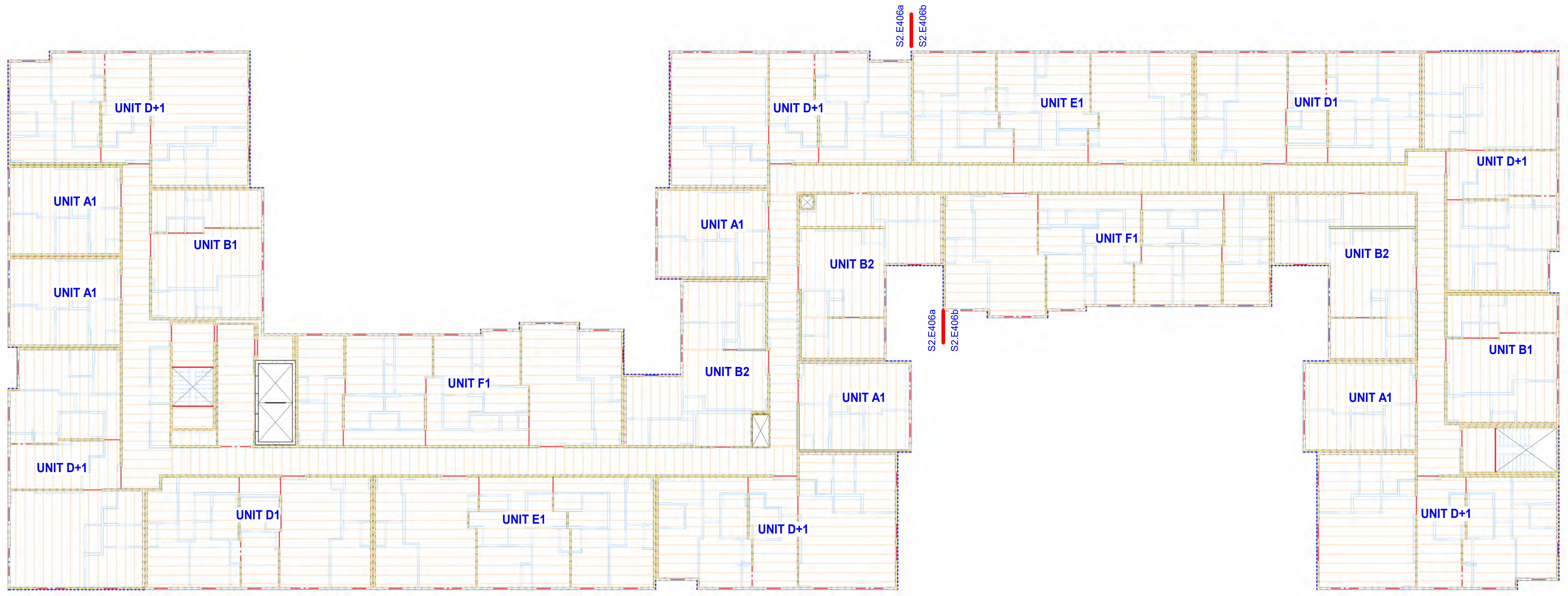

TRUE NORTH
PROJECT NORTH

1 Wood Framing Plan - East Building - Level 06 - OA
1/16" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - East Building - Level 06 - OA

SHEET NUMBER:
S1.E106

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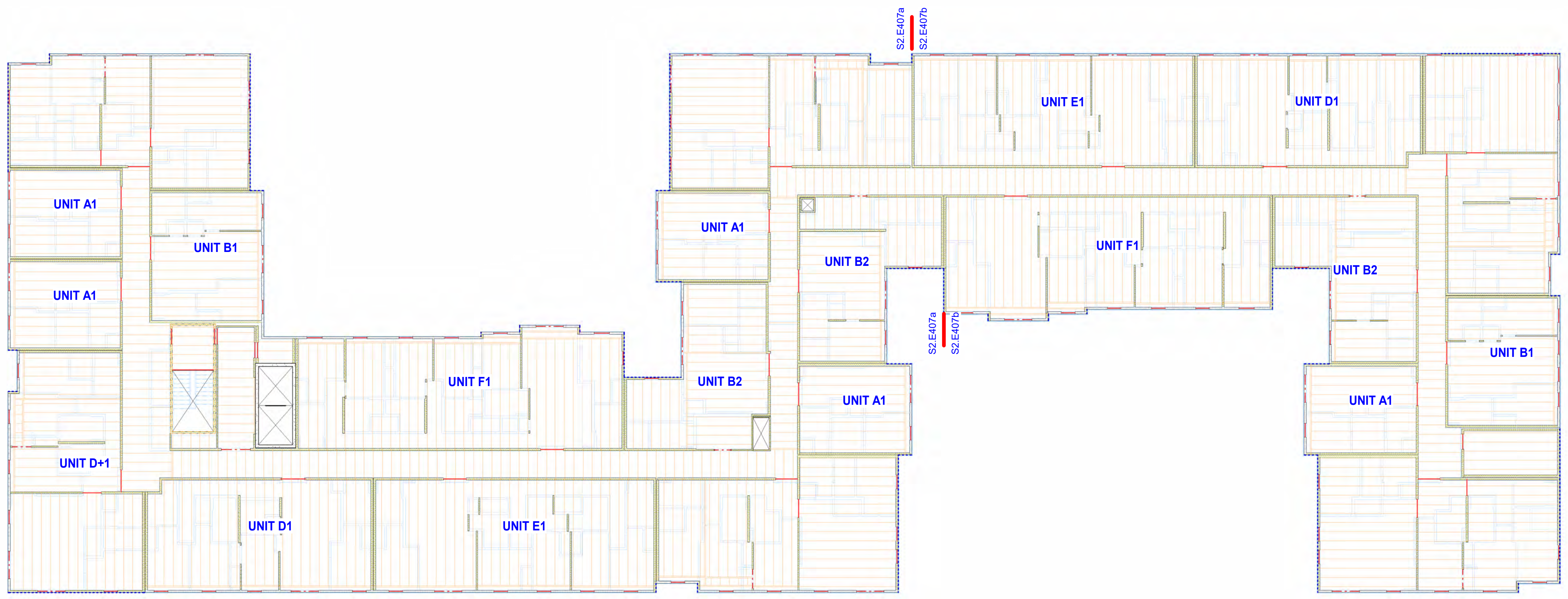

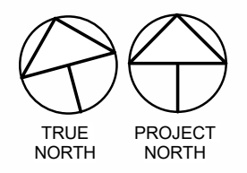

① Roof Framing Plan - East Building - Level 07 - OA
1/16" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - East Building - Roof - OA

SHEET NUMBER:
S1.E107

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WOOD SHEAR WALL NOTES:

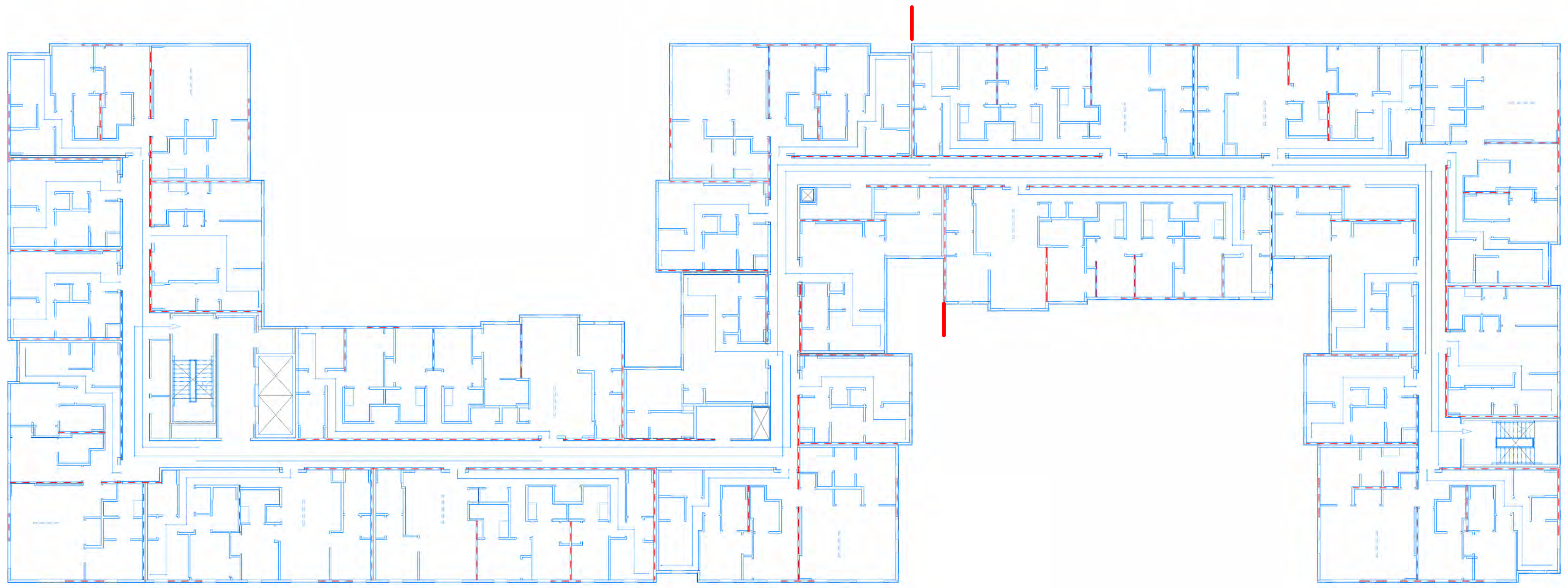
1. SHEAR WALLS SHALL BE SHEATHED AND FASTENED PER THE SHEAR WALL SCHEDULE SWL = TOTAL SHEAR WALL LENGTH MEASURED OUTSIDE FACE OF END POST TO OUTSIDE FACE OF END POST. REFER TO END POST SCHEDULE FOR SIZE AND CONFIGURATION OF END POST MEMBERS. CONTRACTOR TO NOTIFY SEOR IF THE GIVEN LENGTH IS NOT ACHIEVABLE BASED ON REVIEW OF THE ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION.
2. WHERE STAGGERED 4X STUDS OCCUR IN A 6X WALL, SHEAR WALL END POSTS SHALL BE 6X MEMBERS.
3. SHEAR WALL SHEATHING SHALL BE INSTALLED BOTTOM PLATE TO TOP PLATE OF WALL.
4. WHERE SHEAR WALL SHEATHING IS SHOWN TO OCCUR AT UNIT DEMISING WALLS, SHEATHING MAY BE INSTALLED IN AIR GAP PROVIDED THAT AN AIR GAP IS MATAINED BETWEEN SHEATHING AND ADJACENT STUD WALL.
5. SHEAR WALL SHEATHING MAY BE INSTALLED ON EITHER FACE OF A GIVEN WALL WHEN FASTENED DIRECTLY TO STUDS. SHEATHING SHALL BE INSTALLED ON THE OPPOSING SIDE OF WALL AS RESILIENT CHANNELS WHERE OCCURS.
6. PROVIDE TRUSS BLOCKS ABOVE ALL SHEAR WALLS THAT ARE PERPENDICULAR TO BOTTOM CHORD BEARING TRUSSES (TYPICAL)
7. CONTINUOUS TIE-DOWN SYSTEM ACTIVATION PINS SHALL BE PULLED AS EACH FLOOR IS FRAMED
8. DO NOT FIRE CAULK TOP OR BOTTOM BEARING PLATES AT END ANCHOR ROD LOCATIONS.

BRACING PRICING NOTES:

1. SHEAR WALLS WHERE INDICATED ON PLAN DENOTED AS ▬ ▬ ▬ ▬ ASSUME THE FOLLOWING SHEAR WALL TYPE DESIGNATIONS

WOOD SHEAR WALL SCHEDULE				
LEVEL	CORRIDOR WALLS	DEMISING WALLS	UNIT INTERIOR WALLS	UNIT INTERIOR WALLS
LEVEL 5	TYPE 'H'	TYPE 'H'	TYPE 'HH'	TYPE 'C'
LEVEL 4	TYPE 'H'	TYPE 'H'	TYPE 'HH'	TYPE 'C'
LEVEL 3	TYPE 'C'	TYPE 'C'	TYPE 'C'	TYPE 'C'
LEVEL 2	TYPE 'B'	TYPE 'B'	TYPE 'B'	TYPE 'B'
LEVEL 1	TYPE 'A'	TYPE 'A'	TYPE 'A'	TYPE 'A'

2. ASSUME ANCHOR ROD TYPE ATR1-1/8 EACH END OF EACH SHEAR WALL.
3. SEE SHEETS S5.410 THROUGH S5.421 FOR SHEAR WALL SHEDULE AND ANCHOR ROD DETAILS AND ALL OTHER INFORMATION.



TRUE NORTH
PROJECT NORTH

1 Wood Shearwall Plan - East Building Level 4 - OA
1/16" = 1'-0"

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DRAWN BY: Author
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SHEET TITLE:
Wood Shearwall Plan - East Building - OA

SHEET NUMBER:
S1.E108

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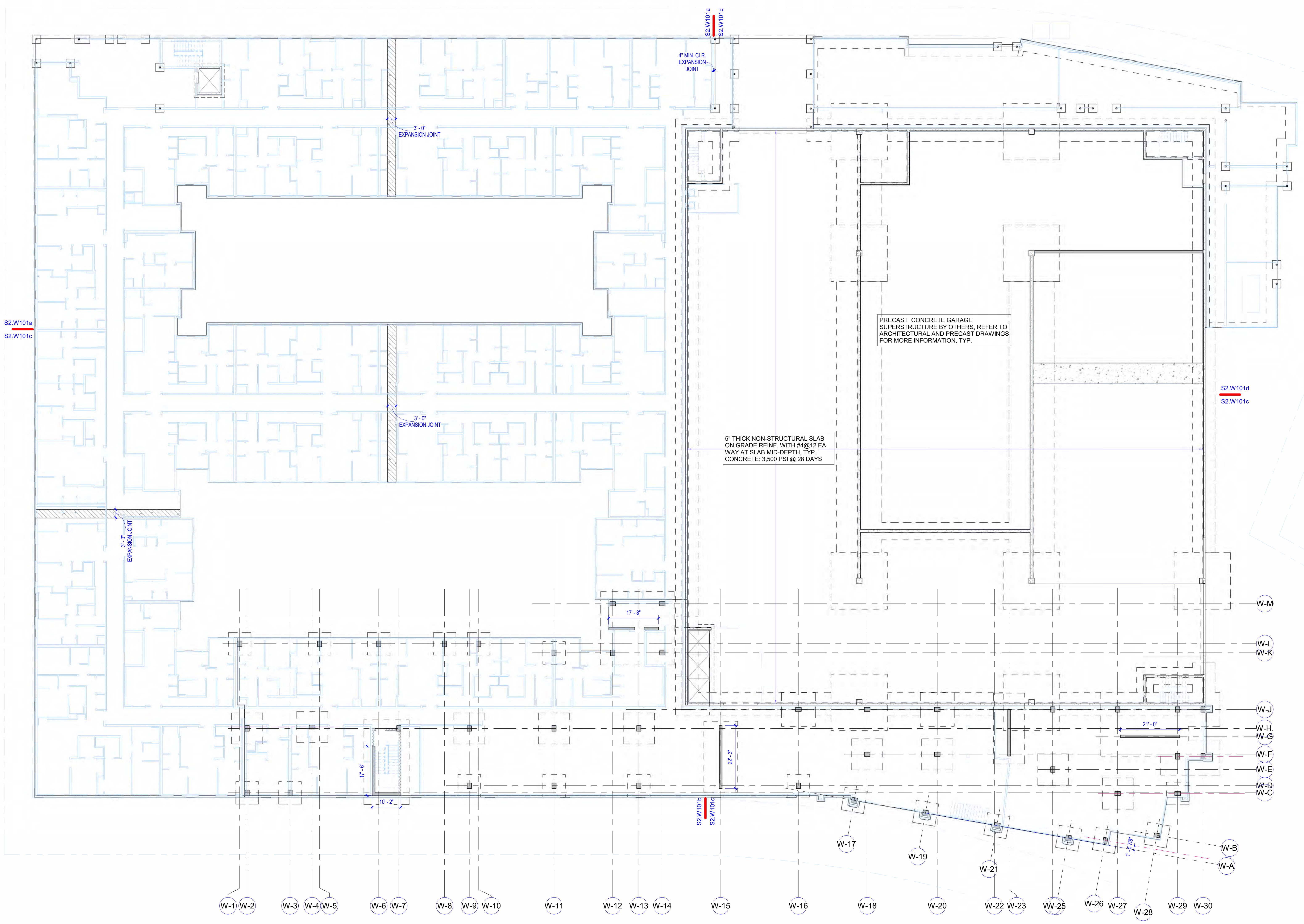

PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.

5" THICK NON-STRUCTURAL SLAB ON GRADE REINF. WITH #4@12 EA. WAY AT SLAB MID-DEPTH, TYP. CONCRETE: 3,500 PSI @ 28 DAYS

TRUE NORTH

PROJECT NORTH

1 Foundation Plan - West Building - Level 01 - OA
1/16" = 1'-0"

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Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
Foundation Plan - West Building - Level 01 - OA

SHEET NUMBER:
S1.W101

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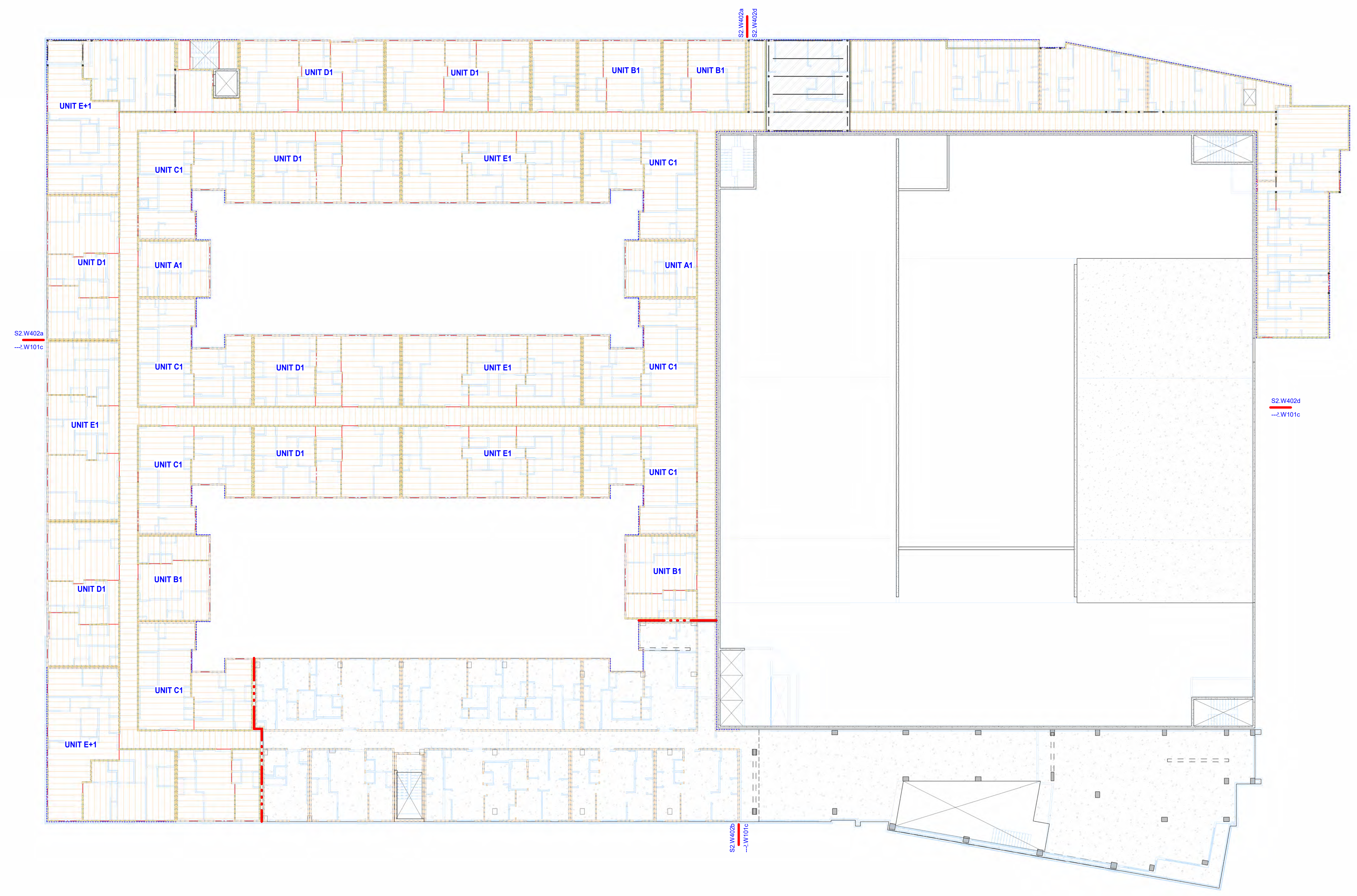

TRUE NORTH
PROJECT NORTH

① Wood Framing Plan - West Building - Level 02 - OA
1/16" = 1'-0"

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Structural Engineers
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PROJECT # : 26154
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SHEET TITLE:
Wood Framing Plan - West Building - Level 02 - OA

SHEET NUMBER:
S1.W102

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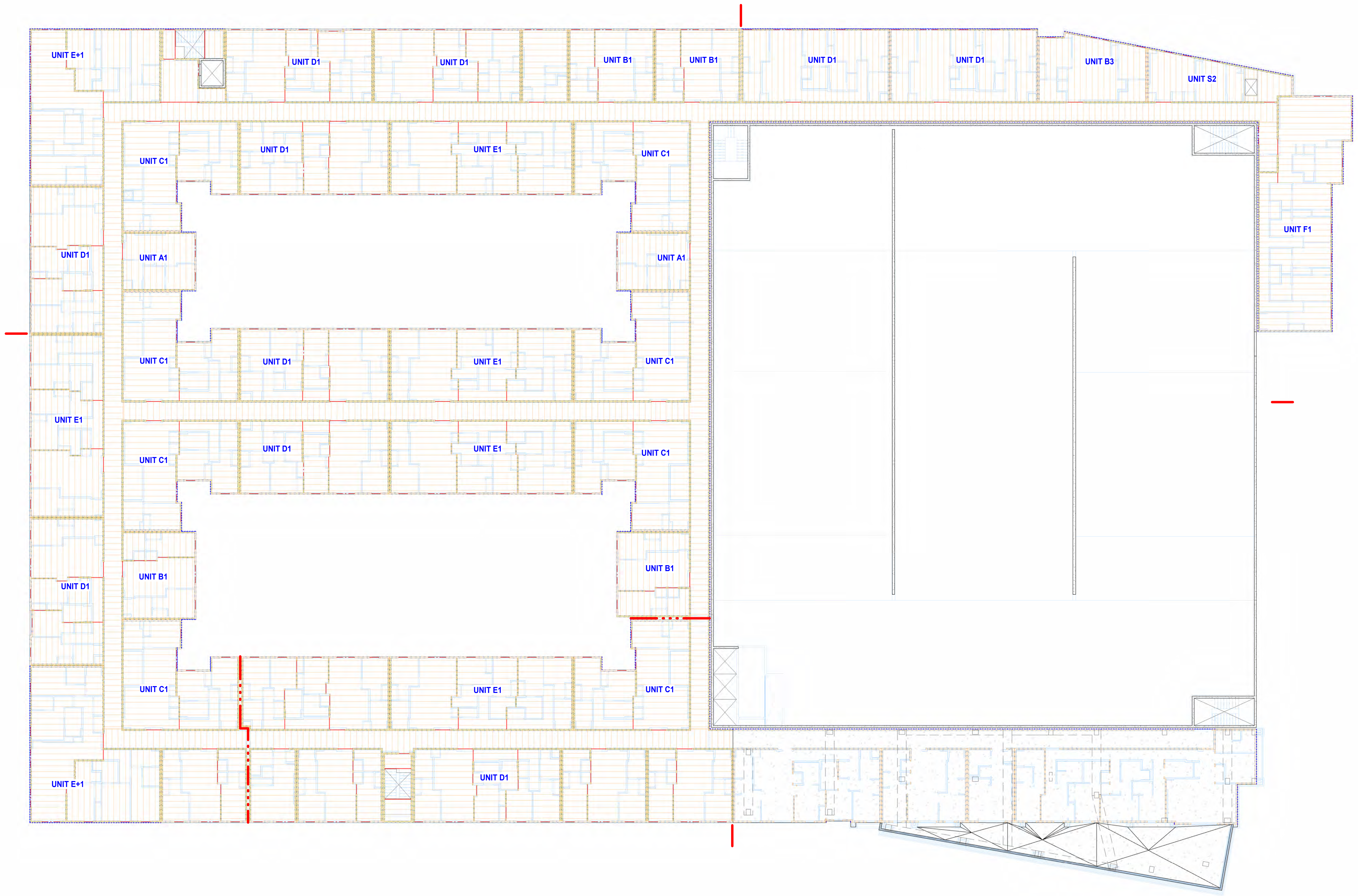

TRUE NORTH
PROJECT NORTH

① Wood Framing Plan - West Building - Level 03 - OA
1/16" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - West Building - Level 03 - OA

SHEET NUMBER:
S1.W103

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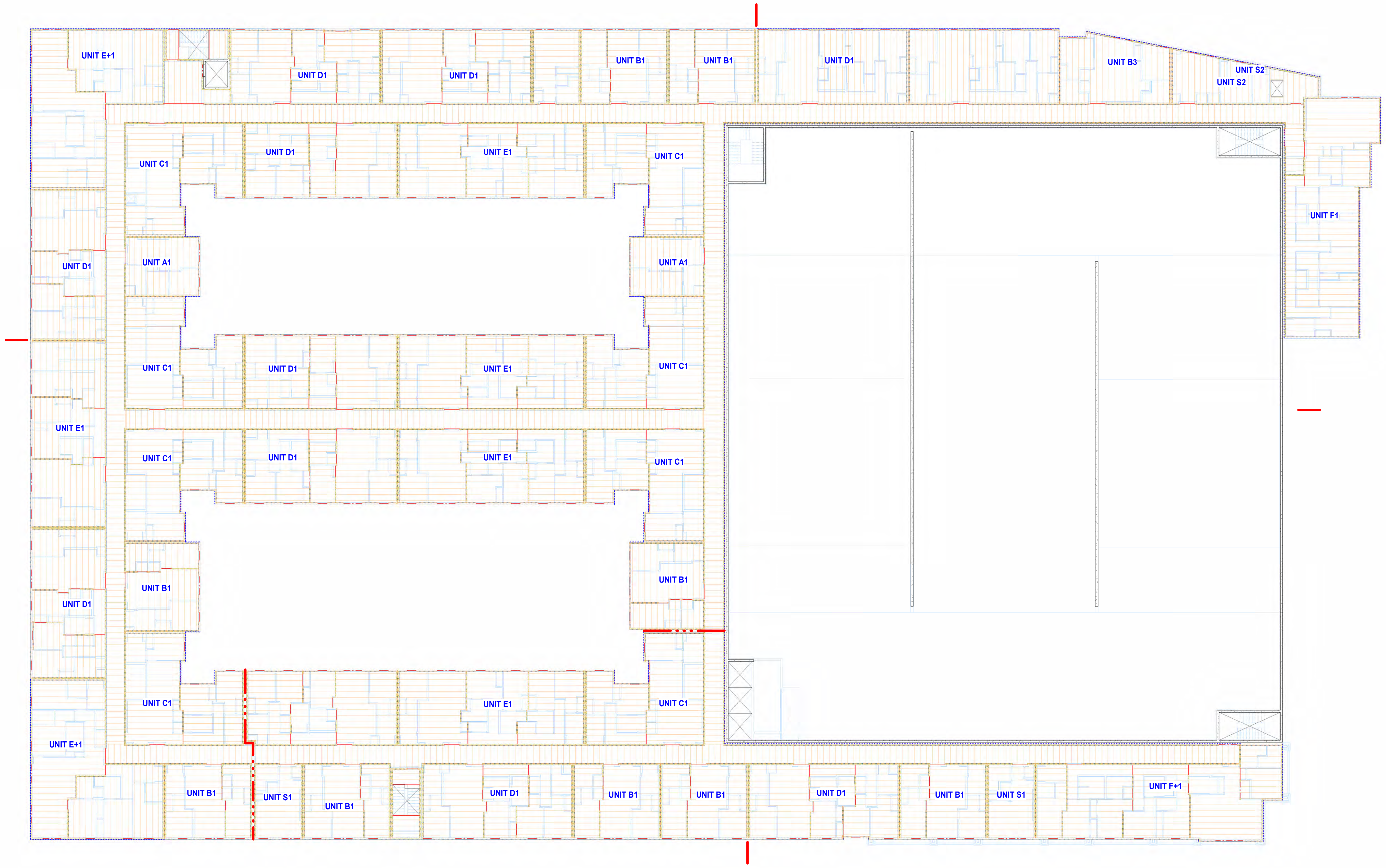

TRUE NORTH
PROJECT NORTH

① Wood Framing plan - West Building - Level 04 - OA
1/16" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - West Building - Level 04 - OA

SHEET NUMBER:
S1.W104

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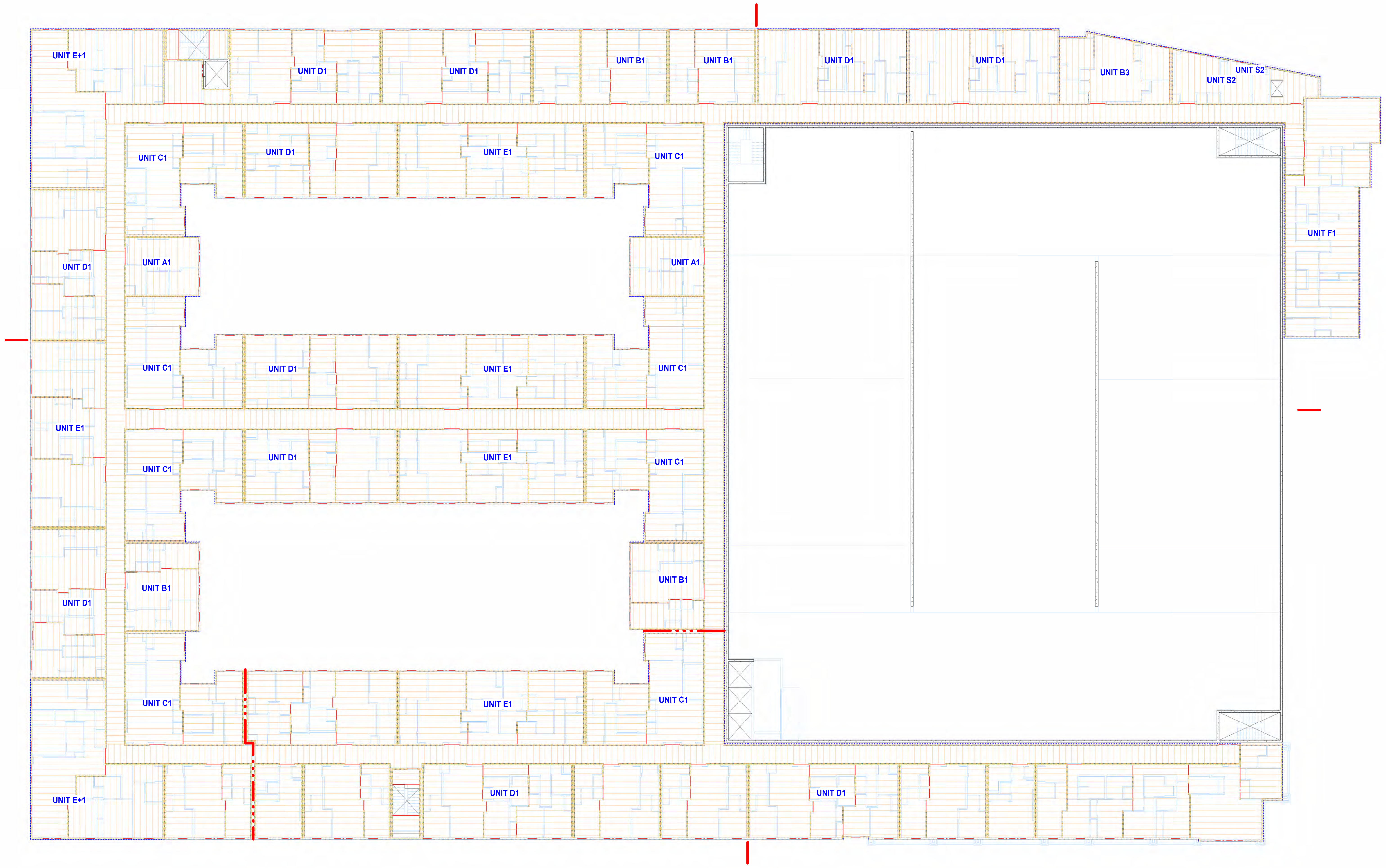

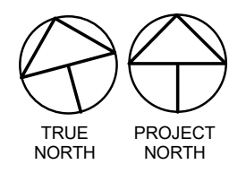

1 Wood Framing Plan - Level 05 West Building OA

1/16" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - West Building - Level 05 - OA

SHEET NUMBER:
S1.W105

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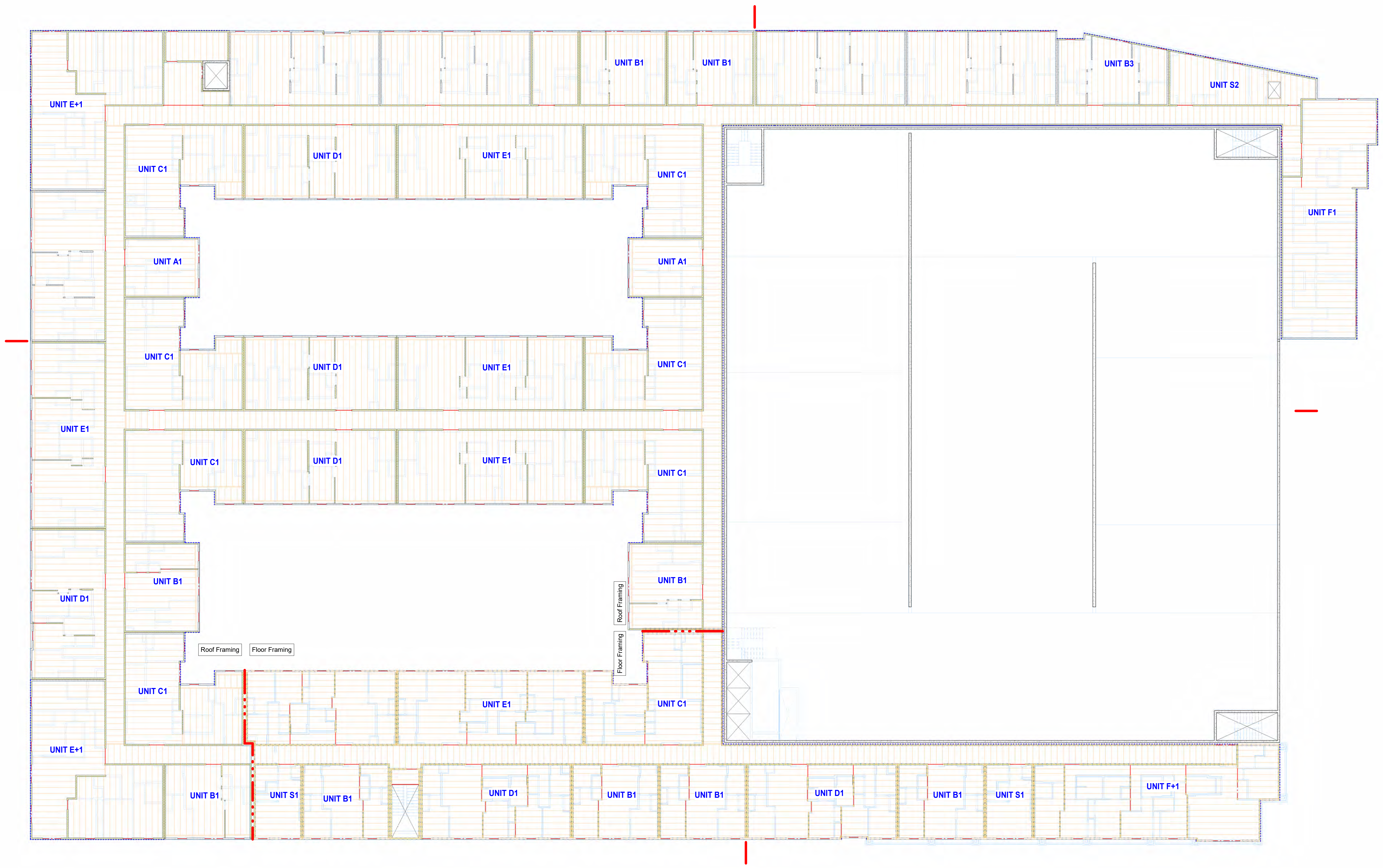

TRUE NORTH
PROJECT NORTH

① Roof Framing Plan - Level 06 - West Building OA
1/16" = 1'-0"

9/30/2026 7:32:46 PM

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Structural Engineers

PROJECT # : 26154
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EAST BATON ROUGE PARISH
LV COLLECTIVE

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- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
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SHEET TITLE:
Wood Framing Plan - West Building - Roof - OA

SHEET NUMBER:
S1.W106

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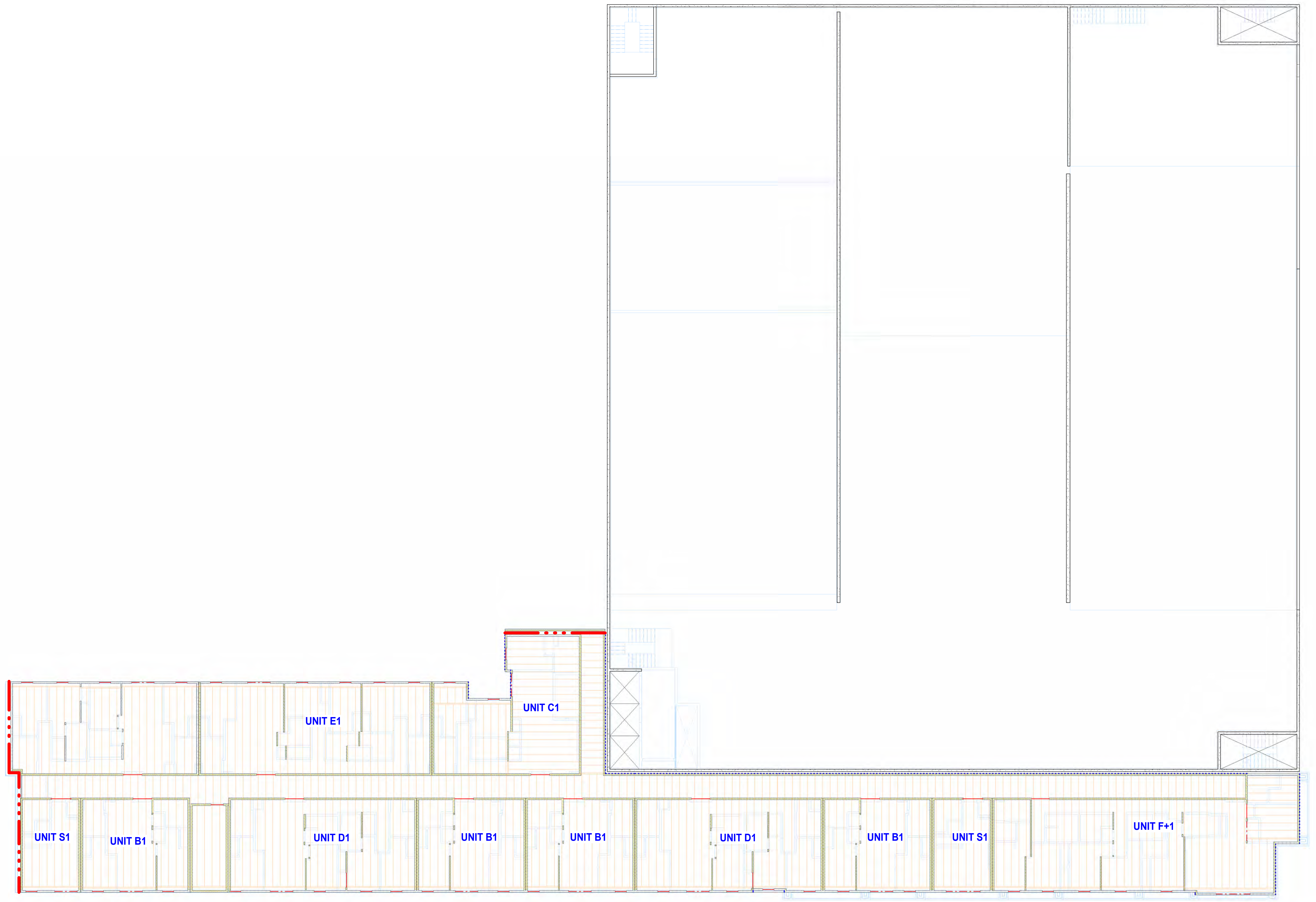

TRUE NORTH

PROJECT NORTH

1 Roof Framing Plan - West Building - Level 07 - OA
1/16" = 1'-0"

9/30/2026 7:32:48 PM

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SHEET TITLE:
Wood Framing Plan - West Building - Roof - OA

SHEET NUMBER:
S1.W107

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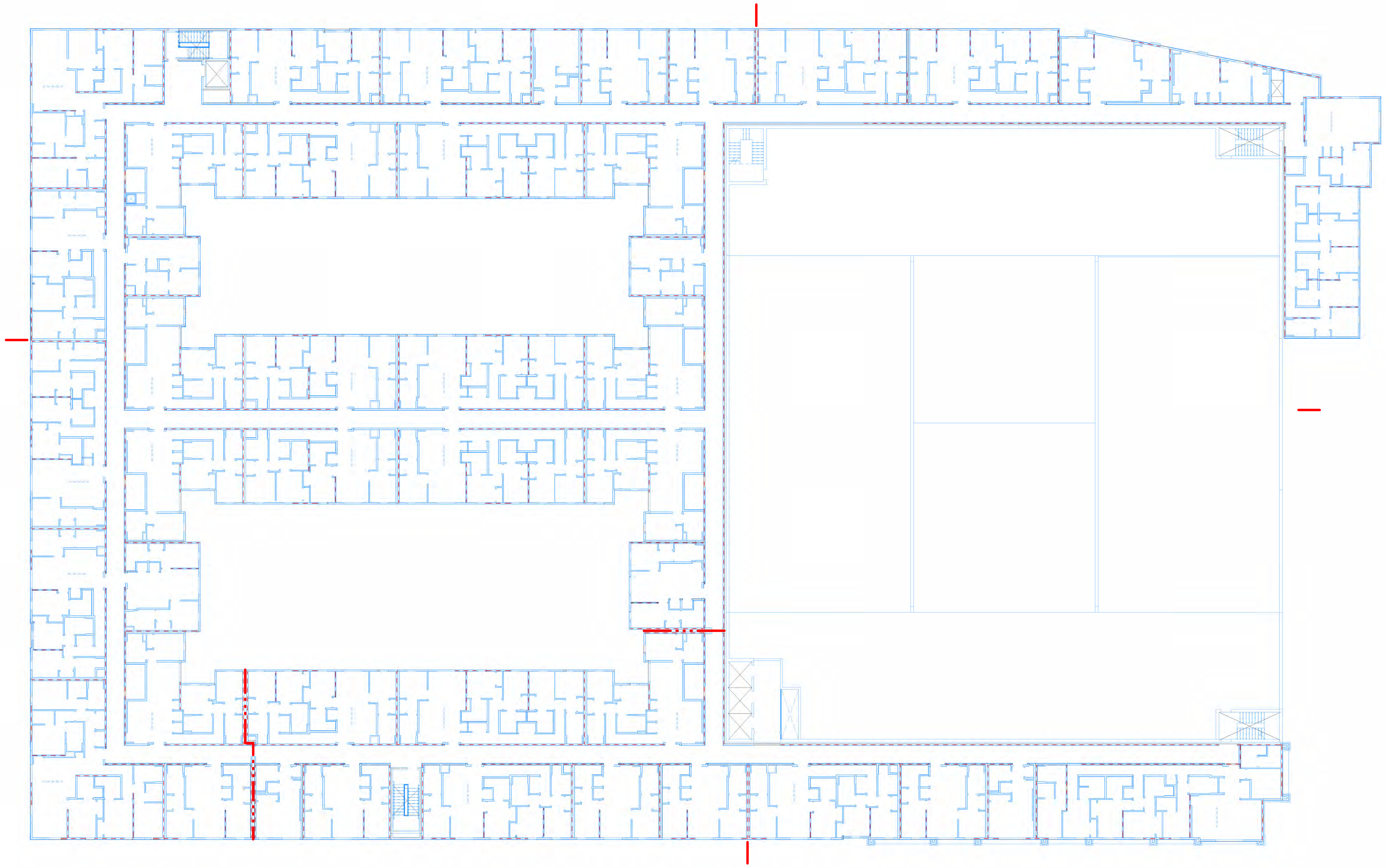


WOOD SHEAR WALL NOTES:

1. SHEAR WALLS SHALL BE SHEATHED AND FASTENED PER THE SHEAR WALL SCHEDULE SWL = TOTAL SHEAR WALL LENGTH MEASURED OUTSIDE FACE OF END POST TO OUTSIDE FACE OF END POST. REFER TO END POST SCHEDULE FOR SIZE AND CONFIGURATION OF END POST MEMBERS. CONTRACTOR TO NOTIFY SEOR IF THE GIVEN LENGTH IS NOT ACHIEVABLE BASED ON REVIEW OF THE ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION.
2. WHERE STAGGERED 4X STUDS OCCUR IN A 6X WALL, SHEAR WALL END POSTS SHALL BE 6X MEMBERS.
3. SHEAR WALL SHEATHING SHALL BE INSTALLED BOTTOM PLATE TO TOP PLATE OF WALL.
4. WHERE SHEAR WALL SHEATHING IS SHOWN TO OCCUR AT UNIT DEMISING WALLS, SHEATHING MAY BE INSTALLED IN AIR GAP PROVIDED THAT AN AIR GAP IS MATAINED BETWEEN SHEATHING AND ADJACENT STUD WALL.
5. SHEAR WALL SHEATHING MAY BE INSTALLED ON EITHER FACE OF A GIVEN WALL WHEN FASTENED DIRECTLY TO STUDS. SHEATHING SHALL BE INSTALLED ON THE OPPOSING SIDE OF WALL AS RESILIENT CHANNELS WHERE OCCURS.
6. PROVIDE TRUSS BLOCKS ABOVE ALL SHEAR WALLS THAT ARE PERPENDICULAR TO BOTTOM CHORD BEARING TRUSSES (TYPICAL)
7. CONTINUOUS TIE-DOWN SYSTEM ACTIVATION PINS SHALL BE PULLED AS EACH FLOOR IS FRAMED
8. DO NOT FIRE CAULK TOP OR BOTTOM BEARING PLATES AT END ANCHOR ROD LOCATIONS.

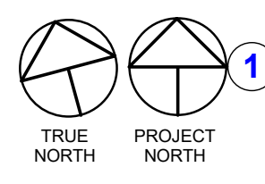
BRACING PRICING NOTES:

1. SHEAR WALLS WHERE INDICATED ON PLAN DENOTED AS ▬ ▬ ▬ ▬ ▬ ASSUME THE FOLLOWING SHEAR WALL TYPE DESIGNATIONS

WOOD SHEAR WALL SCHEDULE

LEVEL	CORRIDOR WALLS	DEMISING WALLS	UNIT INTERIOR WALLS	UNIT INTERIOR WALLS
LEVEL 5	TYPE 'H'	TYPE 'H'	TYPE 'HH'	TYPE 'C'
LEVEL 4	TYPE 'H'	TYPE 'H'	TYPE 'HH'	TYPE 'C'
LEVEL 3	TYPE 'C'	TYPE 'C'	TYPE 'C'	TYPE 'C'
LEVEL 2	TYPE 'B'	TYPE 'B'	TYPE 'B'	TYPE 'B'
LEVEL 1	TYPE 'A'	TYPE 'A'	TYPE 'A'	TYPE 'A'

2. ASSUME ANCHOR ROD TYPE ATR1-1/8 EACH END OF EACH SHEAR WALL.
3. SEE SHEETS S5.410 THROUGH S5.421 FOR SHEAR WALL SHEDULE AND ANCHOR ROD DETAILS AND ALL OTHER INFORMATION.



1 Wood Shearwall Plan - West Building Level 4 - OA
1/16" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMeg Structural Engineers, LLC
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PROJECT # : 26154
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- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Wood Shearwall Plan - West Building - OA

SHEET NUMBER:
S1.W108

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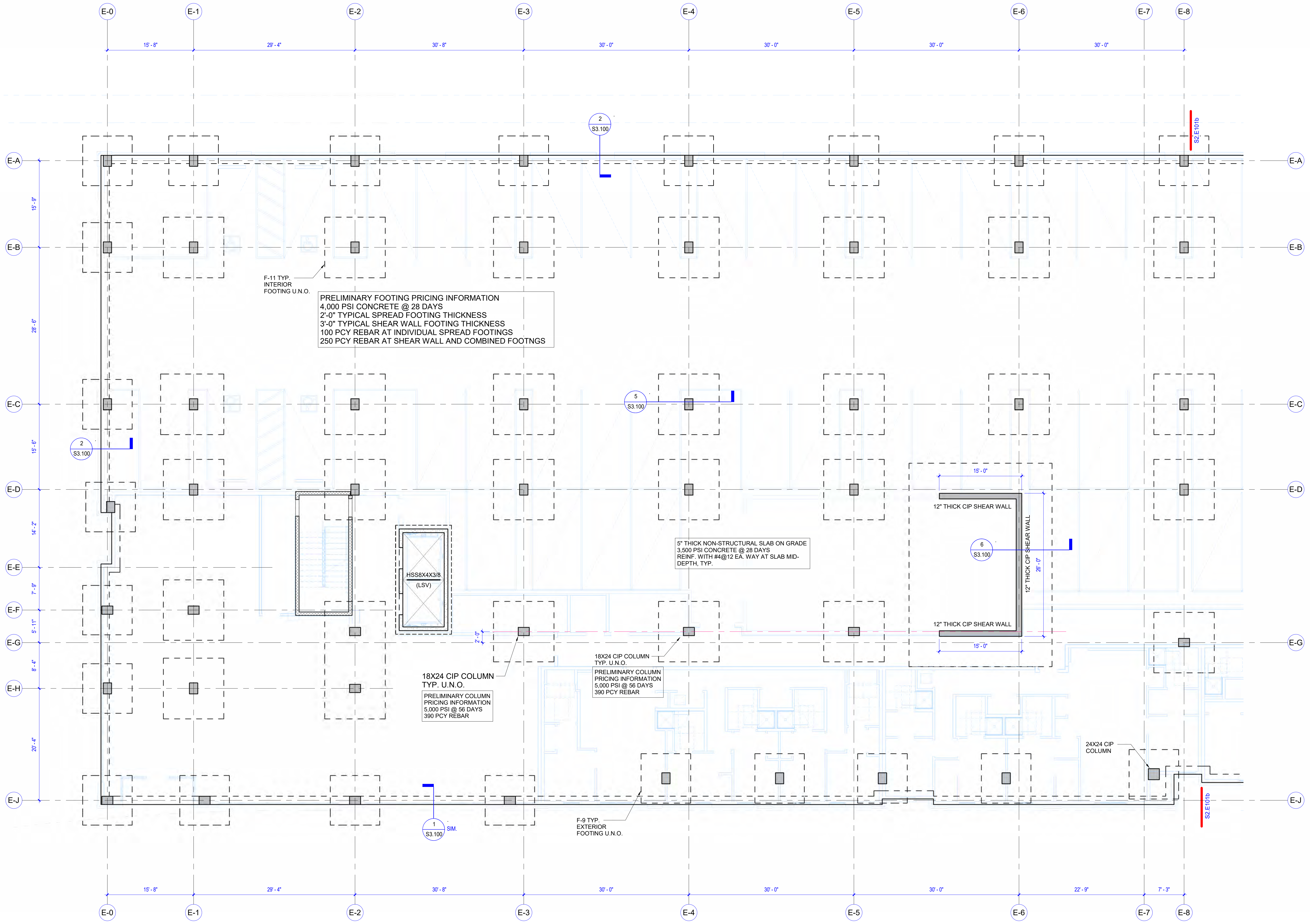
9/30/2026 8:38:51 PM

FOUNDATION NOTES:

1. CONTRACTOR TO CROSS REFERENCE AND/OR VERIFY ALL DIMENSIONS, ELEVATIONS, RECESSES DEPRESSIONS, CURB SIZES/LOCATIONS, WALL ELEVATIONS, GUARDRAILS, FINISHES, OPENINGS AND SLAB PENETRATION SIZES/LOCATIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ACHITECT AND ENGINEER OF ANY DISCREPANCIES PIOR TO CONSTRUCTION.
2. REFER TO ARCHITECTURAL SLAB DIMENSIONAL CONTROL PLAN FOR ALL SLAB EDGE AND OPENING DIMENSIONS NOT SHOWN, TYPICAL.
3. TOP OF CONCRETE (TOC) = REF PLAN
4. SLAB THICKNESS = SEE PLAN
5. CONCRETE STRENGTH = REFER TO GENERAL NOTES
6. **C-X** DENOTES CIP CONCRETE COLUMN. FOR COLUMN SCHEDULE SEE SHEET SERIES S5.100
7. **SW-X** DENOTES CIP CONCRETE SHEAR WALL. FOR SHEAR WALL ELEVATIONS SEE SHEET SERIES S5.200
8. **SR-X** DENOTES STUD RAILS. FOR STUD RAIL SCHEDULE AND DETAILS SEE SHEET S5.130



PRELIMINARY FOOTING PRICING INFORMATION
4,000 PSI CONCRETE @ 28 DAYS
2'-0" TYPICAL SPREAD FOOTING THICKNESS
3'-0" TYPICAL SHEAR WALL FOOTING THICKNESS
100 PCY REBAR AT INDIVIDUAL SPREAD FOOTINGS
250 PCY REBAR AT SHEAR WALL AND COMBINED FOOTNGS

5" THICK NON-STRUCTURAL SLAB ON GRADE
3,500 PSI CONCRETE @ 28 DAYS
REINF. WITH #4@12 EA. WAY AT SLAB MID-DEPTH, TYP.

PRELIMINARY COLUMN
PRICING INFORMATION
5,000 PSI @ 56 DAYS
390 PCY REBAR

PRELIMINARY COLUMN
PRICING INFORMATION
5,000 PSI @ 56 DAYS
390 PCY REBAR

TRUE NORTH
PROJECT NORTH

1 Foundation Plan - East Building - Level 01 - Area A
1/8" = 1'-0"

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Structural Engineers

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SHEET TITLE:
Foundation Plan - East Building - Level 01 - Area A

SHEET NUMBER:
S2.E101a

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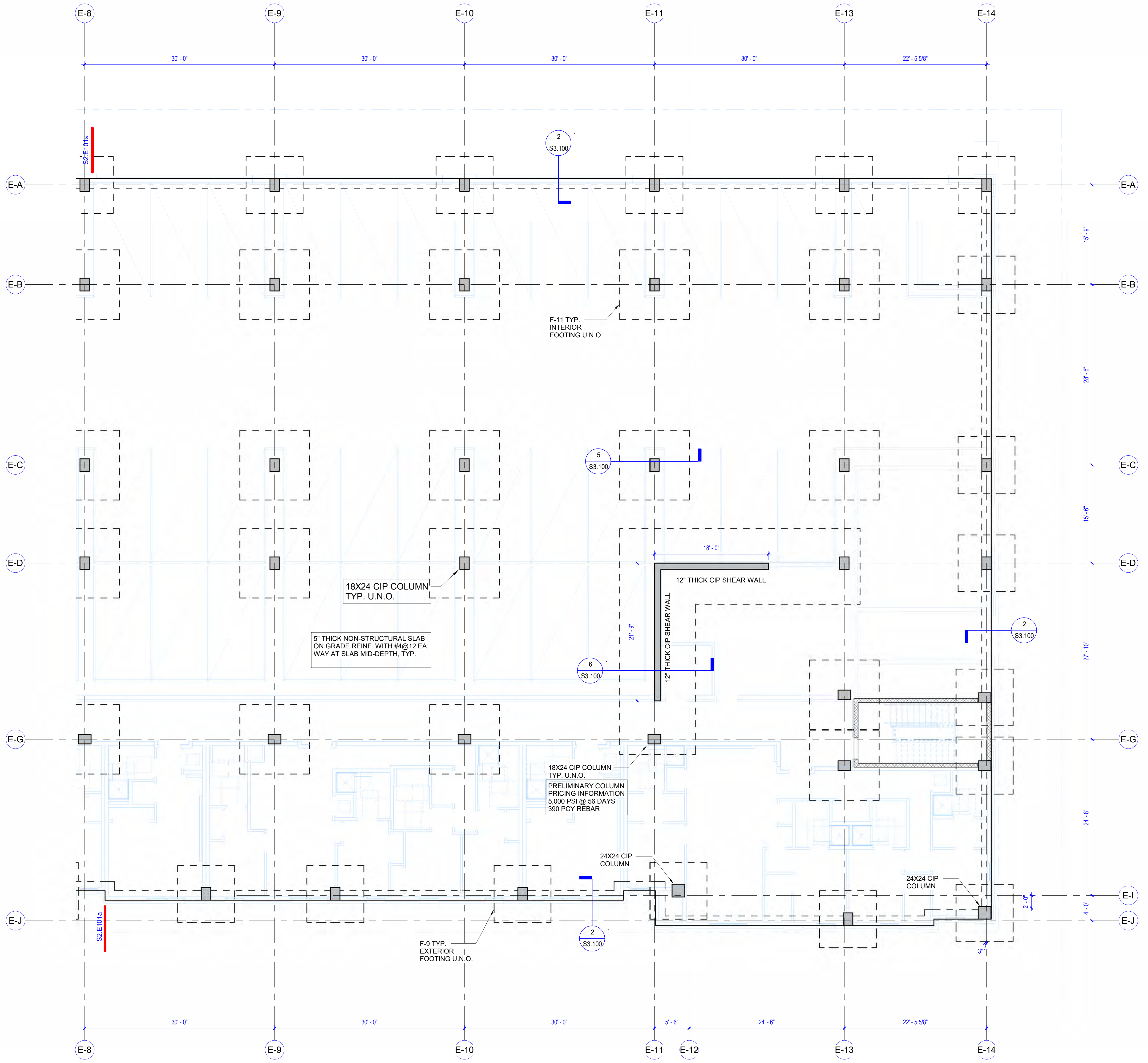
9/30/2026 8:21:12 PM

FOUNDATION NOTES:

1. CONTRACTOR TO CROSS REFERENCE AND/OR VERIFY ALL DIMENSIONS, ELEVATIONS, RECESSES DEPRESSIONS, CURB SIZES/LOCATIONS, WALL ELEVATIONS, GUARDRAILS, FINISHES, OPENINGS AND SLAB PENETRATION SIZES/LOCATIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ACHITECT AND ENGINEER OF ANY DISCREPANCIES PIOR TO CONSTRUCTION.
2. REFER TO ARCHITECTURAL SLAB DIMENSIONAL CONTROL PLAN FOR ALL SLAB EDGE AND OPENING DIMENSIONS NOT SHOWN, TYPICAL.
3. TOP OF CONCRETE (TOC) = REF PLAN
4. SLAB THICKNESS = SEE PLAN
5. CONCRETE STRENGTH = REFER TO GENERAL NOTES
6. **C-X** DENOTES CIP CONCRETE COLUMN. FOR COLUMN SCHEDULE SEE SHEET SERIES S5.100
7. **SW-X** DENOTES CIP CONCRETE SHEAR WALL. FOR SHEAR WALL ELEVATIONS SEE SHEET SERIES S5.200
8. **SR-X** DENOTES STUD RAILS. FOR STUD RAIL SCHEDULE AND DETAILS SEE SHEET S5.130



TRUE NORTH

PROJECT NORTH

1 Foundation Plan - East Building - Level 01 - Area B

1/8" = 1'-0"

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SHEET TITLE:
Foundation Plan - East Building - Level 01 - Area B

SHEET NUMBER:
S2.E101b

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FORMING PLAN NOTES:

1. CONTRACTOR TO CROSS REFERENCE AND / OR VERIFY ALL DIMENSIONS, ELEVATIONS, RECESSES DEPRESSIONS, CURB SIZES / LOCATIONS, WALL ELEVATIONS, GUARDRAILS, FINISHES, OPENINGS AND SLAB PENETRATION SIZES / LOCATIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ARCHITECT AND ENGINEER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.
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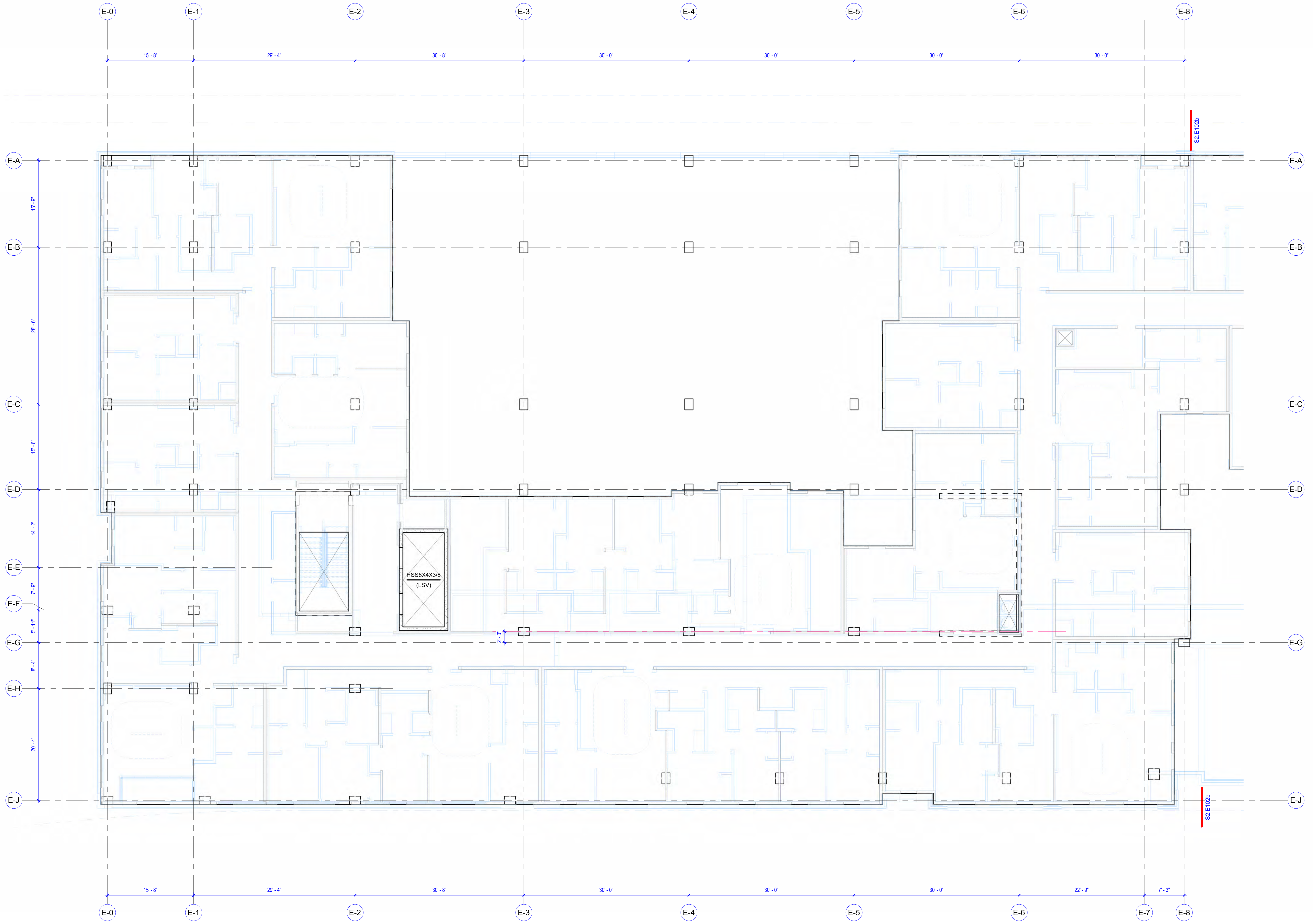
    - **C-X.E** INDICATES SCHEDULED **C-X** COLUMN HAS ECCENTRIC STEP WITHIN HEIGHT OF COLUMN
    - **C-X.S** INDICATES SCHEDULED **C-X** COLUMN SLOPES WITHIN HEIGHT OF COLUMN
7. **SW-X** DENOTES CIP CONCRETE SHEAR WALL. FOR SHEAR WALL ELEVATIONS SEE SHEET SERIES S5.200
8. **SR-X** DENOTES STUD RAILS. FOR STUD RAIL SCHEDULE AND DETAILS SEE SHEET S5.130
9. **DP-X** DENOTES DROP PANEL. FOR DROP PANEL SCHEDULE AND DETAILS SEE SHEET S5.110
10. **PTB-X** DENOTES POST-TENSION CONCRETE BEAM.. FOR BEAM SCHEDULE AND DETAILS SEE SHEET /
11. **RCB-X** DENOTES REINFORCED CONCRETE BEAM. FOR BEAM SCHEDULE AND DETAILS SEE SHEET /
12. 0'-0" = xxx.xx' (VERIFY W/ CIVIL)



TRUE NORTH

PROJECT NORTH

① Forming Plan - East Building - Level 02 - Area A

1/8" = 1'-0"

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SHEET TITLE:
Forming Plan - East Building - Level 02 - Area A

SHEET NUMBER:
S2.E102a

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11. **RCB-X** DENOTES REINFORCED CONCRETE BEAM. FOR BEAM SCHEDULE AND DETAILS SEE SHEET /
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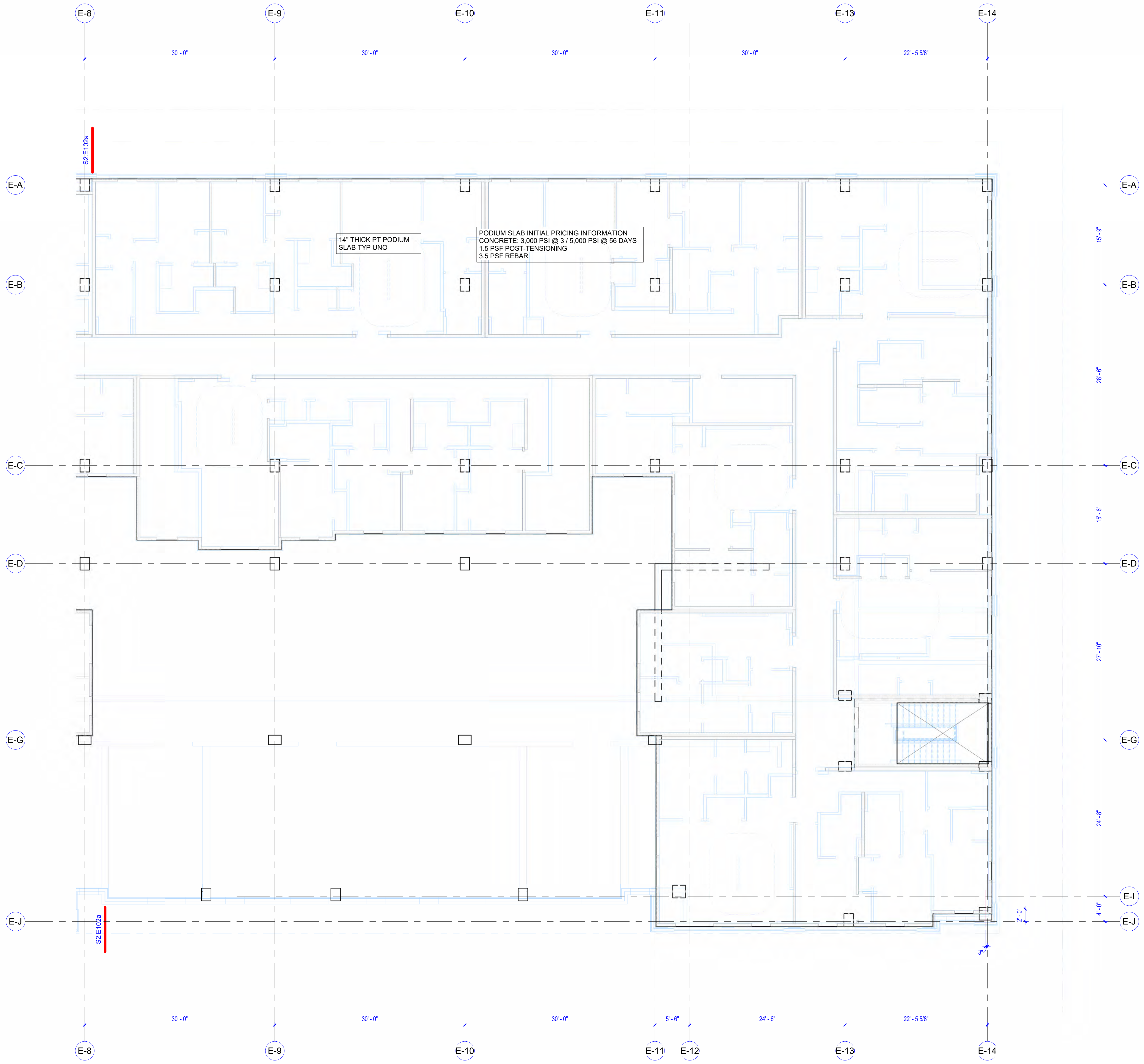

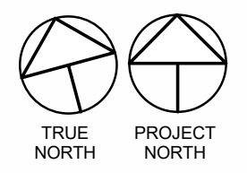

① Forming Plan - East Building - Level 02 - Area B
1/8" = 1'-0"

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Structural Engineers
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PROJECT # : 26154
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CHECKED BY: Checker

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No.	Description	Date
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LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Forming Plan - East Building - Level 02 - Area B

SHEET NUMBER:
S2.E102b

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WOOD FLOOR FRAMING NOTES:

1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS LAYOUT SHALL BE COORDINATED TO AVOID CONFLICTS WITH MEP SYSTEMS. TRUSSES SHALL NOT BE PLACED DIRECTLY ABOVE OR BELOW ANY PARALLEL 2X WALLS TO PREVENT CONFLICTS WITH MEP ROUTING.
3. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
4. HEADERS SHALL BE AT TOP OF WINDOW OPENINGS AND DOOR OPENINGS. FLUSH BEAMS SHALL BE IN FLOOR CAVITY (TYP. U.N.O.)
5. SEE GENERAL NOTE WD2.0 FOR MINIMUM QUANTITY OF STUDS AT BEARING ENDS OF ALL BEAMS, HEADERS (DROP BEAMS), FLUSH BEAMS, AND GIRDER TRUSSES. SEE POST SCHEDULE FOR STUD PACKS AND POST SPECIFIED ON PLAN.
6. FLOOR AND ROOF SHEATHING SHALL BE OSB TYPICAL EXCEPT AT CORRIDORS AND EXPOSED / EXTERIOR AREAS WHICH SHALL BE C-D EXPOSURE 1 PLYWOOD.
7. PROVIDE 3/8" MIN HORIZONTAL GAP BETWEEN EXTERIOR WALL SHEATHING PANELS TO ALLOW FOR SHRINKAGE. GAP TO BE PLACED AT THE LEVEL OF THE FLOOR SHEATHING AT EACH LEVEL (TYP)
8. TRUSSES SHALL RECEIVE DOUBLE TOP CHORD AT SPECIFIC DIAPHRAGM STRAPPING LOCATIONS PER PLAN AND NOTES. REFER TO WOOD DIAPHRAGM REINFORCING PLANS FOR MORE INFO.

WOOD FRAMING SCHEDULE

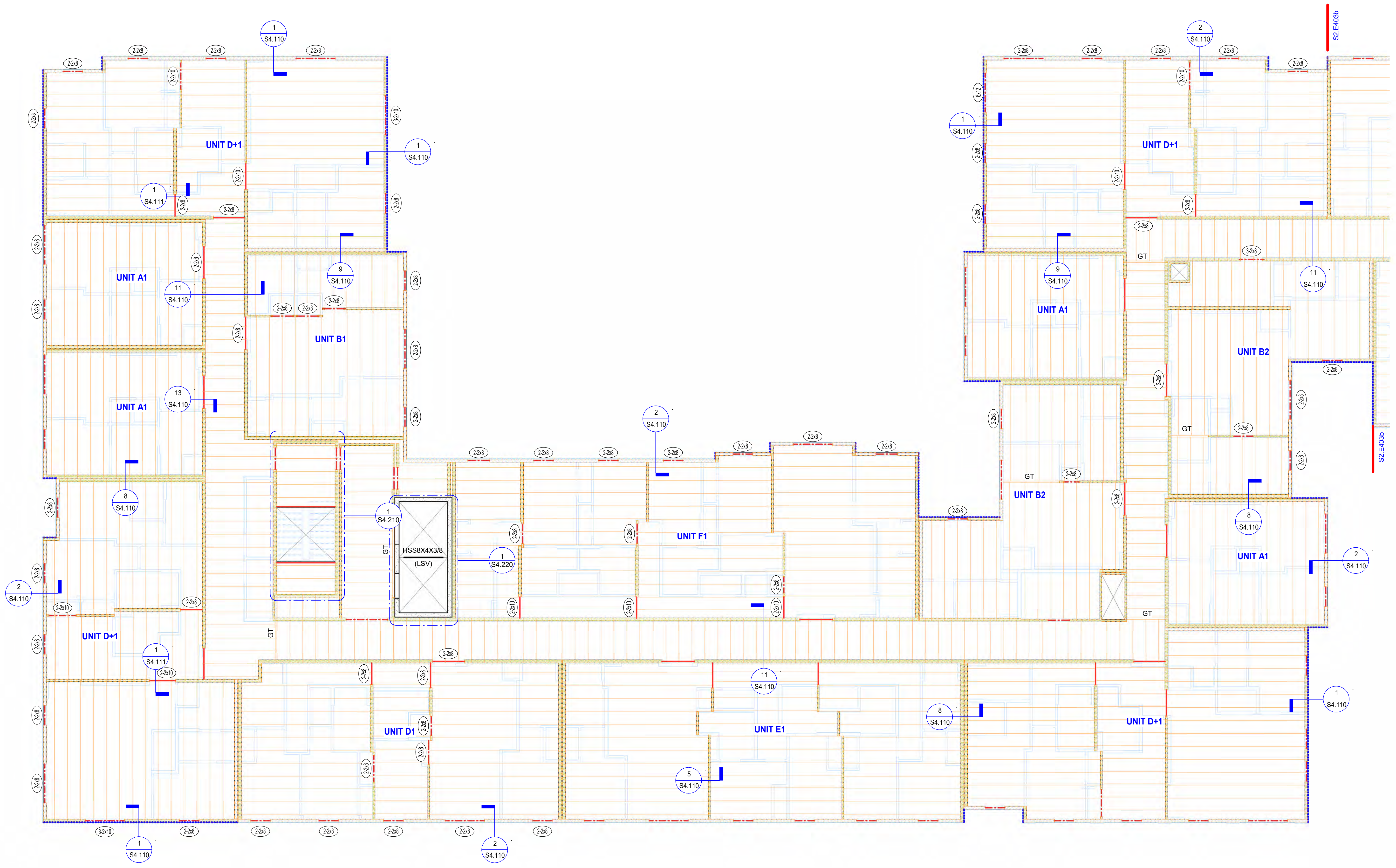
LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:

1. ALL FRAMING EXPOSED TO EXTERIOR ELEMENTS TO BE PRESSURE TREATED (EXCLUDING FRT AND ENGINEERED LUMBER).

LEGEND:

▭ = EXTERIOR NON-LOAD BEARING WOOD FRAMED STUD WALL
▭ = LOAD BEARING WOOD FRAMED STUD WALL
▭ = EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
— • • — = INDICATES 3HR FIRE SEPARATION WALL. REFER TO ARCHITECTURAL DETAILS FOR MORE INFORMATION.
—·—·— = HEADER (DROP BEAM)
—— = FLUSH BEAM
– – – = LEDGER
—— = TRUSS / JOIST
═ = GIRDER TRUSS
HSS / W —— = STEEL BEAM



TRUE NORTH

PROJECT NORTH

① Wood Framing Plan - East Building - Level 03 - Area F
1/8" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Wood Framing Plan - East Building - Level 03 - Area A

SHEET NUMBER:
S2.E403a

NOT RELEASED FOR CONSTRUCTION

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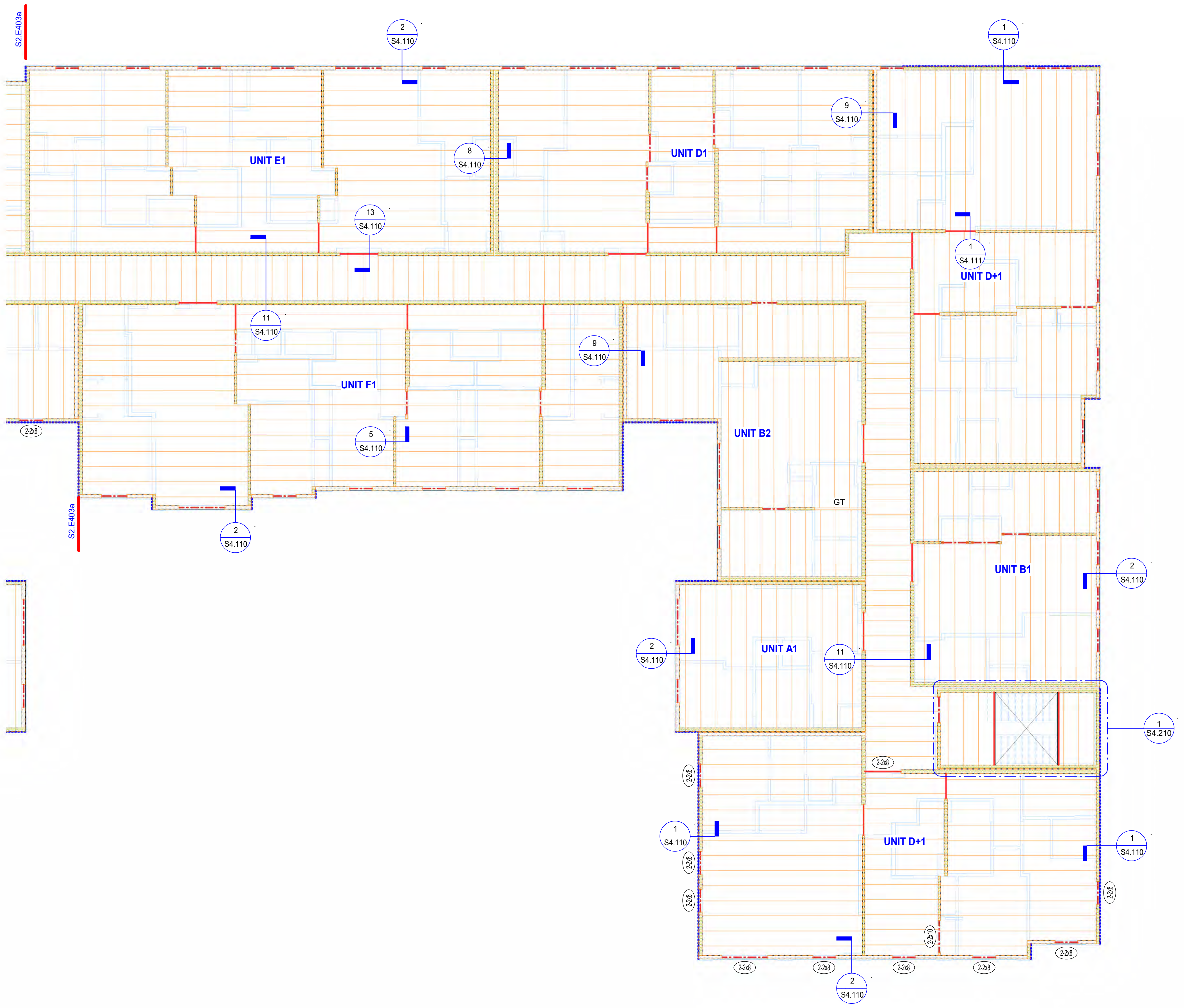

TRUE NORTH
PROJECT NORTH

① Wood Framing Plan - East Building - Level 03 - Area G
1/8" = 1'-0"

WOOD FLOOR FRAMING NOTES:

1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS LAYOUT SHALL BE COORDINATED TO AVOID CONFLICTS WITH MEP SYSTEMS. TRUSSES SHALL NOT BE PLACED DIRECTLY ABOVE OR BELOW ANY PARALLEL 2X WALLS TO PREVENT CONFLICTS WITH MEP ROUTING.
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WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
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NOTES:
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—— = TRUSS / JOIST
═ = GIRDER TRUSS
HSS / W —— = STEEL BEAM

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMeg Structural Engineers, LLC
1080 Eldridge Parkway
Suite 350
Houston, TX 77077
281-407-5734
www.vsmeg.com

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CHECKED BY: Checker

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Atlanta, GA 30305

T 404 365 7600

www.nilesbolton.com

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SHEET TITLE:
Wood Framing Plan - East Building - Level 03 - Area B

SHEET NUMBER:
S2.E403b

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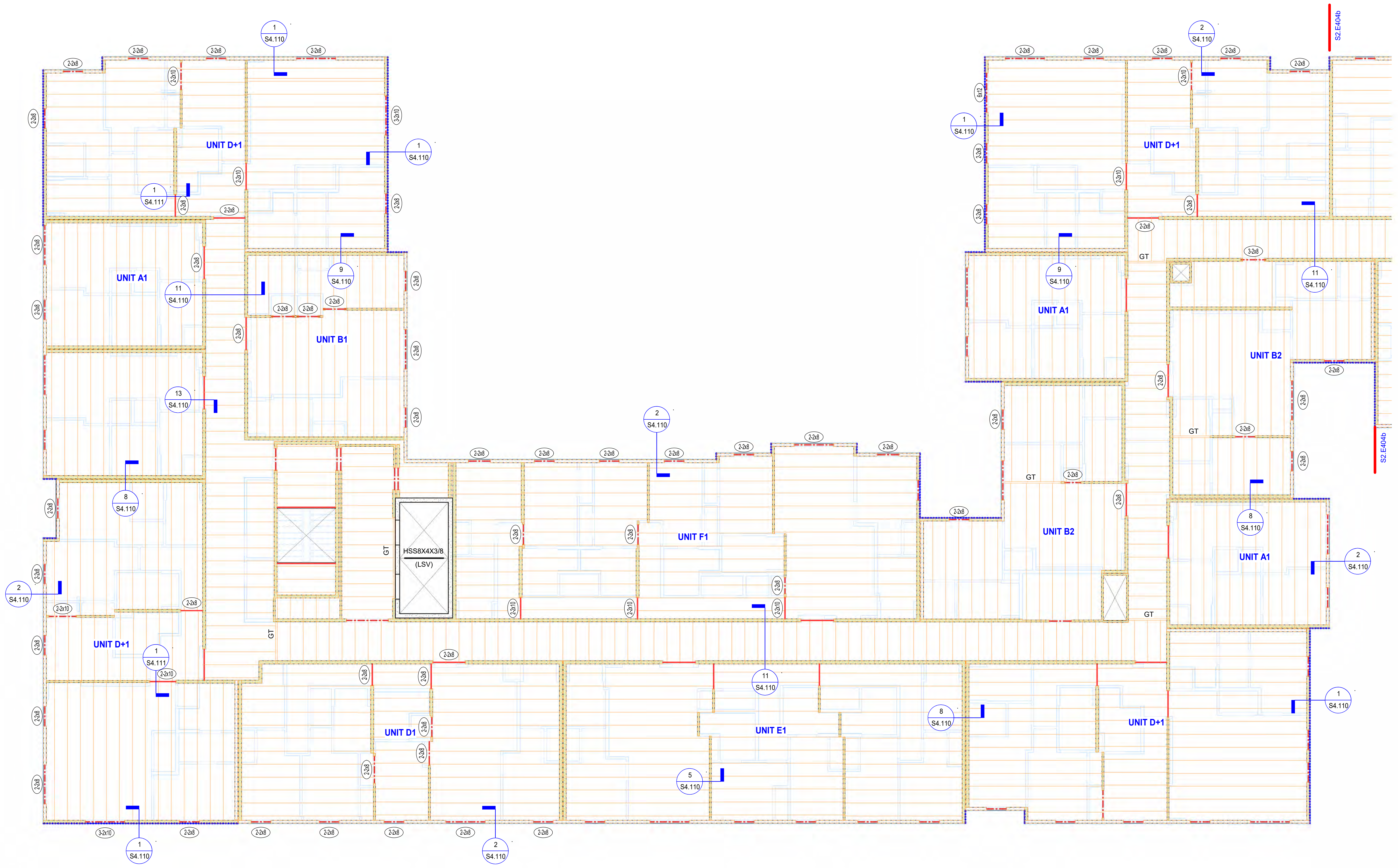

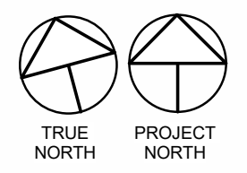

1 Wood Framing Plan - East Building - Level 04 - Area F
1/8" = 1'-0"

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Wood Framing Plan - East Building - Level 04 - Area A

SHEET NUMBER:
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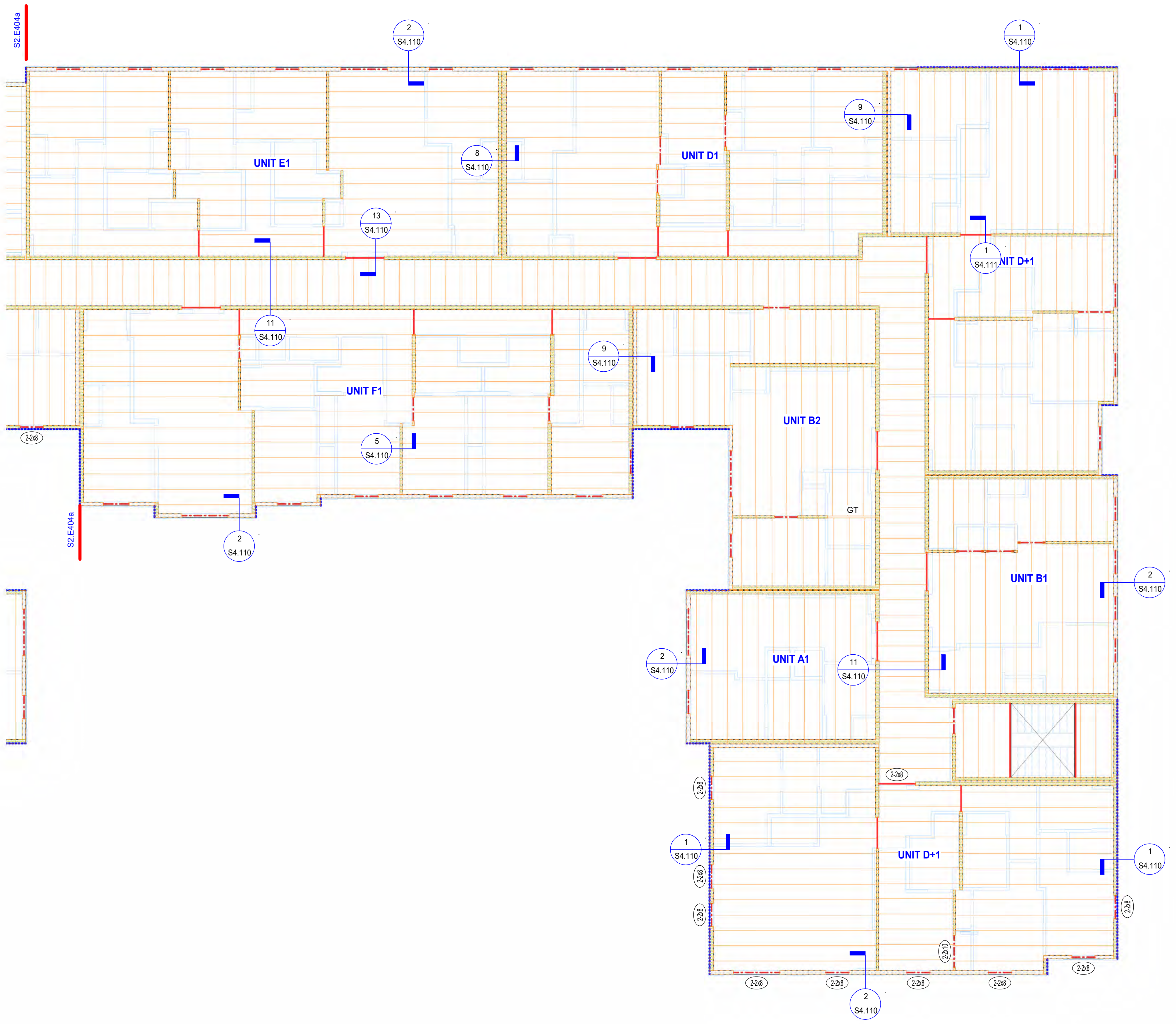

1 Wood Framing Plan - East Building - Level 04 - Area G
1/8" = 1'-0"

TRUE NORTH
PROJECT NORTH

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Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

NILES BOLTON ASSOCIATES

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Atlanta, GA 30305

T 404 365 7600

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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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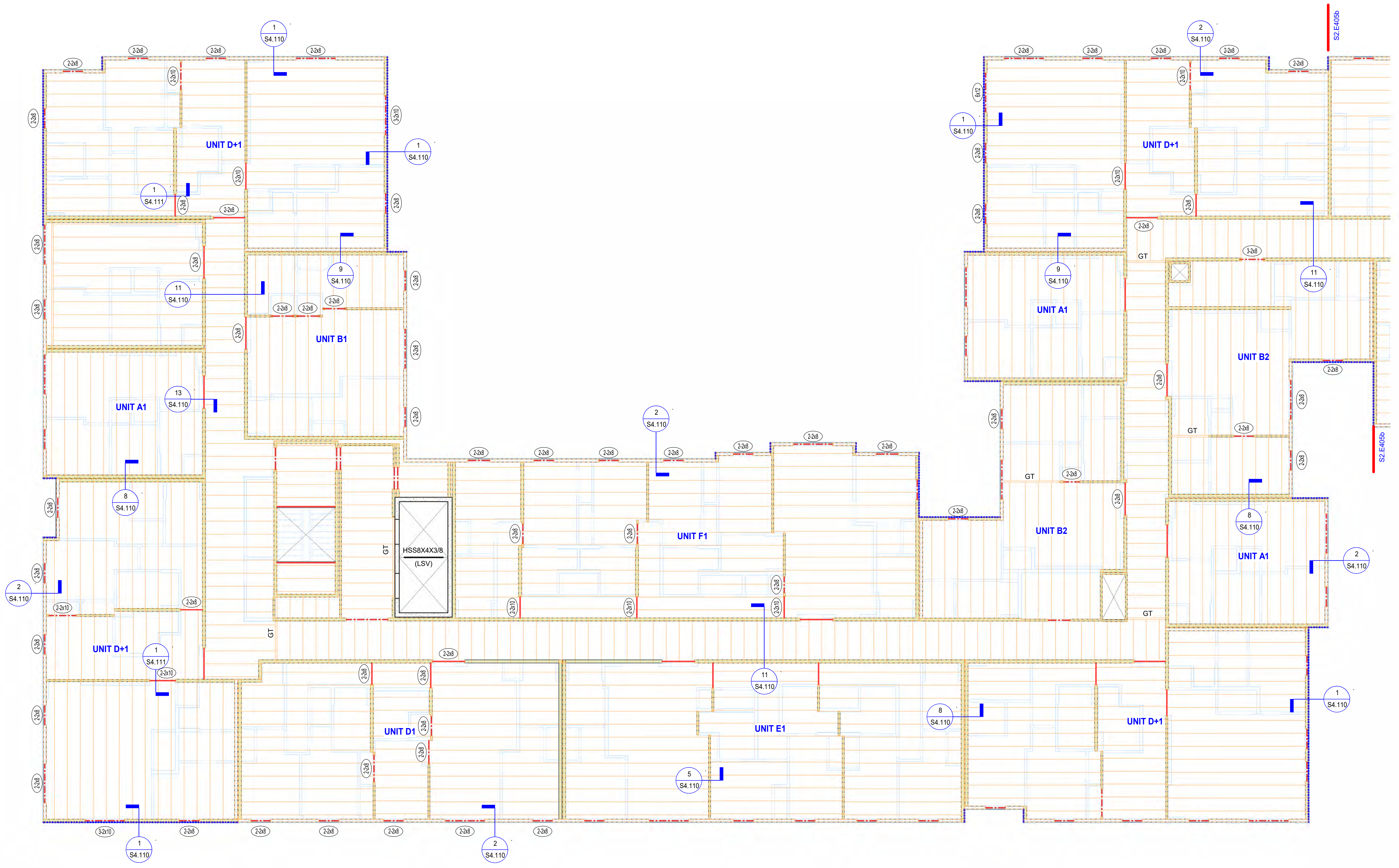

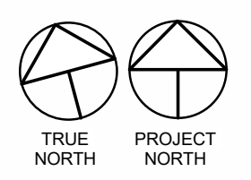

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PROJECT # : 26154
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SHEET NUMBER:
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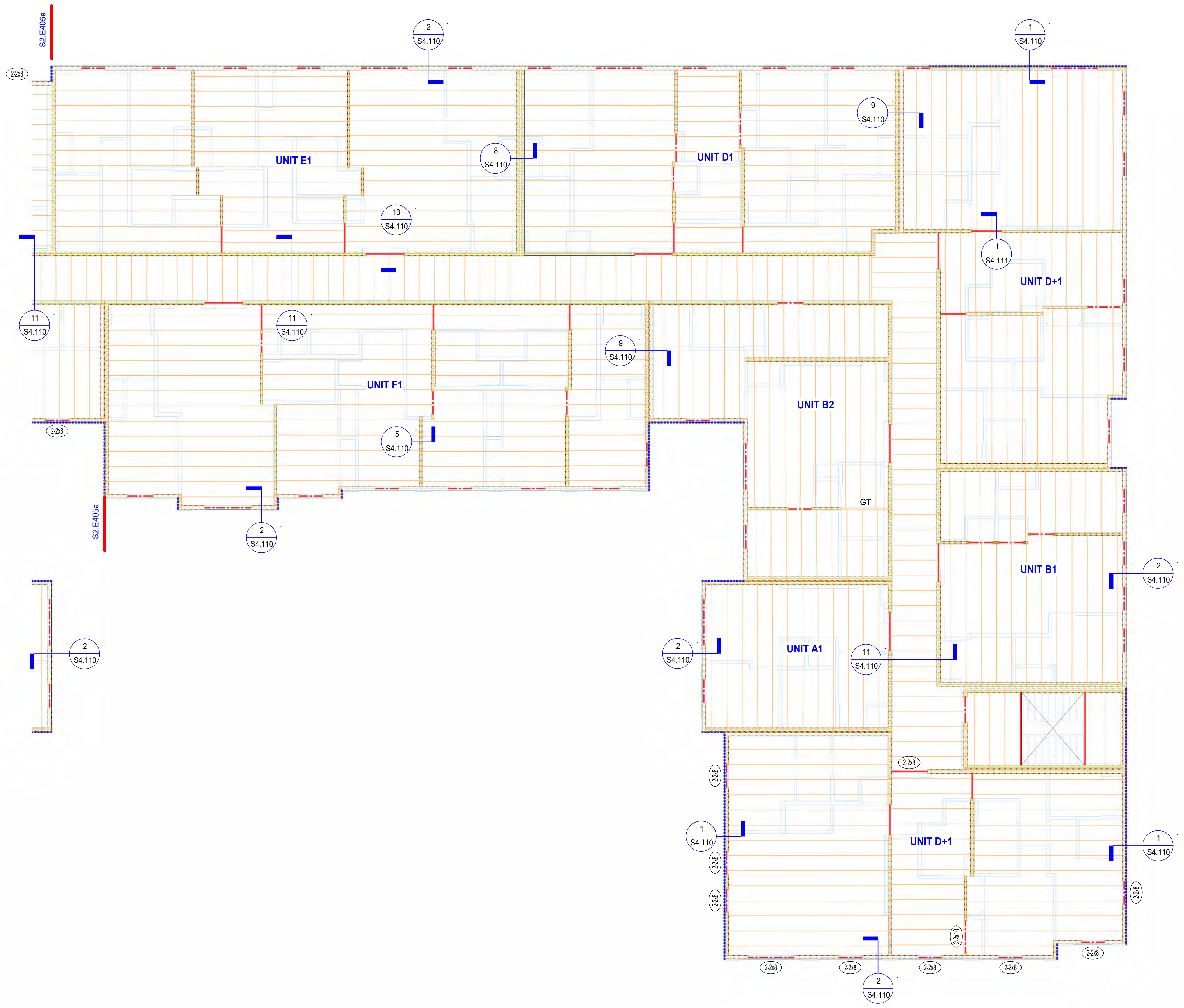

TRUE NORTH
PROJECT NORTH

1 Wood Framing Plan - East Building - Level 05 - Area G
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Wood Framing Plan - East Building - Level 05 - Area B

SHEET NUMBER:
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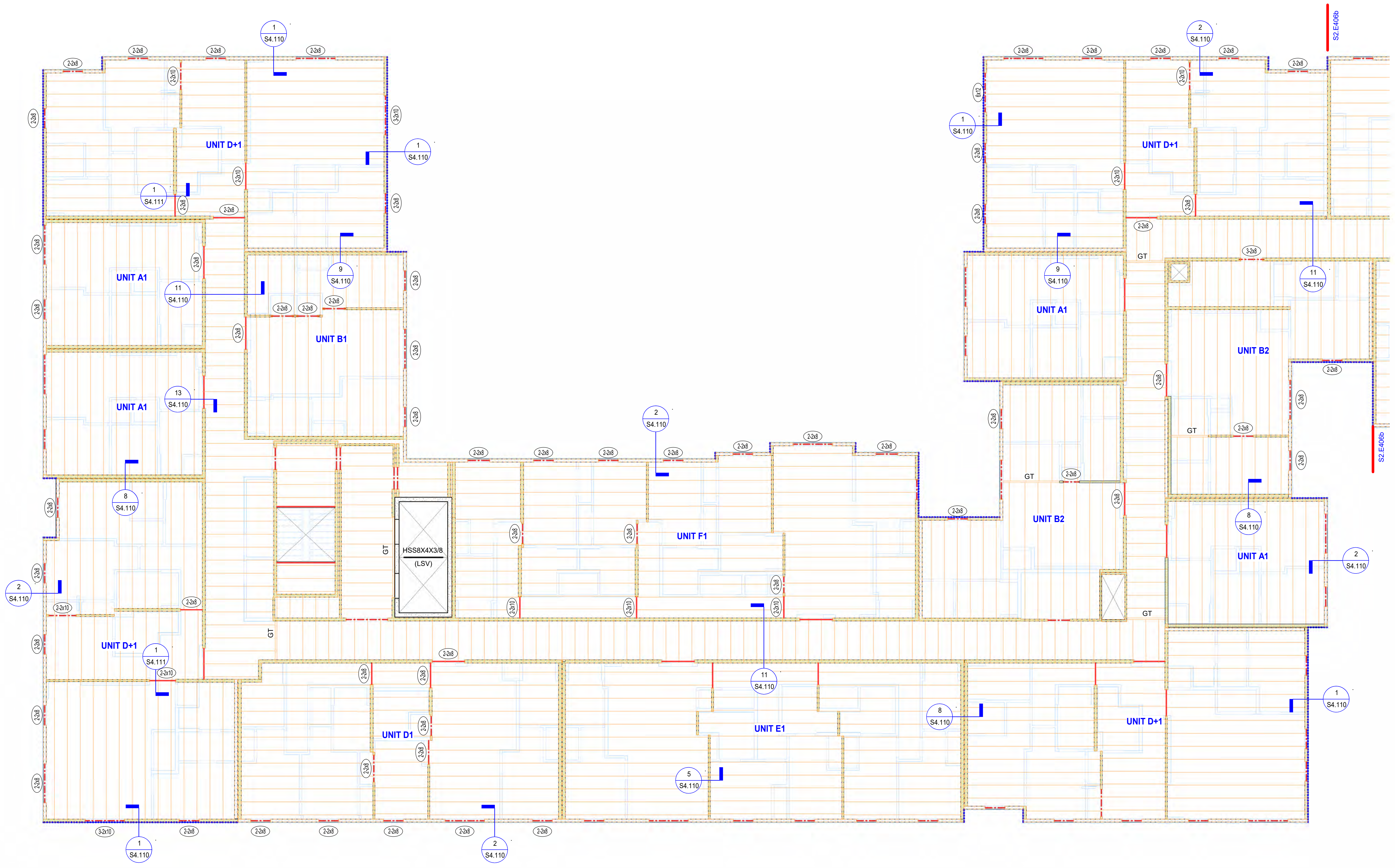
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:

1. ALL FRAMING EXPOSED TO EXTERIOR ELEMENTS TO BE PRESSURE TREATED (EXCLUDING FRT AND ENGINEERED LUMBER).

LEGEND:

- ▭ = EXTERIOR NON-LOAD BEARING WOOD FRAMED STUD WALL
- ▭ = LOAD BEARING WOOD FRAMED STUD WALL
- ▭ = EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
- ━ • • ━ = INDICATES 3HR FIRE SEPARATION WALL. REFER TO ARCHITECTURAL DETAILS FOR MORE INFORMATION.
- ─·─·─ = HEADER (DROP BEAM)
- ─── = FLUSH BEAM
- – – – = LEDGER
- ─── = TRUSS / JOIST
- ═══ = GIRDER TRUSS
- HSS / W ━━━ = STEEL BEAM



TRUE NORTH
PROJECT NORTH

1 Wood Framing Plan - East Building - Level 06 - Area A
1/8" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMeg Structural Engineers, LLC
1080 Eldridge Parkway
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www.vsmeg.com

PROJECT # : 26154
DRAWN BY: Author
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NILES BOLTON ASSOCIATES

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No.	Description	Date
	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

LV COLLECTIVE

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- 100% DD

DOCUMENTS ARE ISSUED UNDER THE AUTHORITY OF:
- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Wood Framing Plan - East Building - Level 06 - Area A

SHEET NUMBER:
S2.E406a

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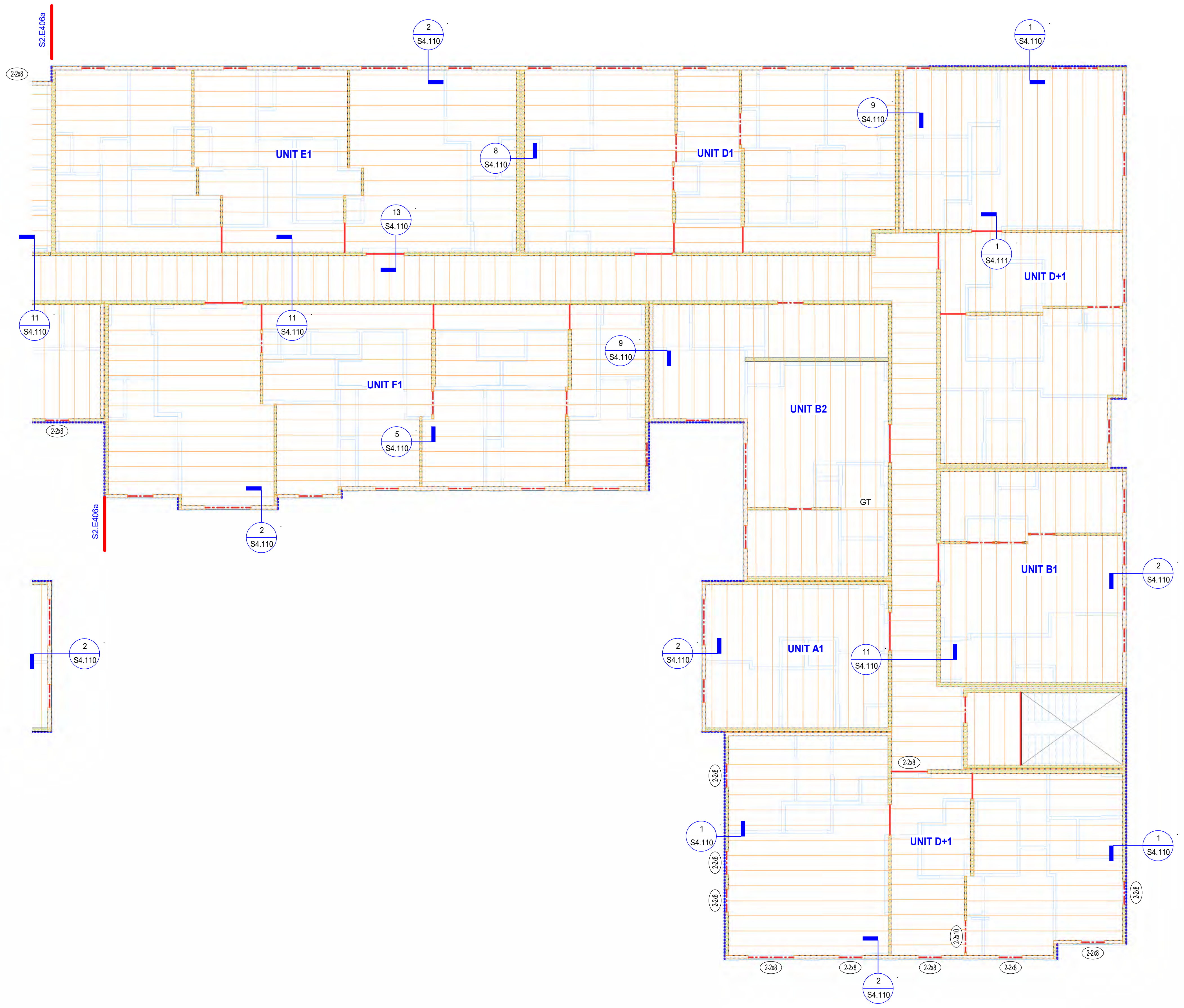

TRUE NORTH
PROJECT NORTH

1 Wood Framing Plan - East Building - Level 06 - Area B
1/8" = 1'-0"

WOOD FLOOR FRAMING NOTES:

1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS LAYOUT SHALL BE COORDINATED TO AVOID CONFLICTS WITH MEP SYSTEMS. TRUSSES SHALL NOT BE PLACED DIRECTLY ABOVE OR BELOW ANY PARALLEL 2X WALLS TO PREVENT CONFLICTS WITH MEP ROUTING.
3. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
4. HEADERS SHALL BE AT TOP OF WINDOW OPENINGS AND DOOR OPENINGS. FLUSH BEAMS SHALL BE IN FLOOR CAVITY (TYP. U.N.O.)
5. SEE GENERAL NOTE WD2.0 FOR MINIMUM QUANTITY OF STUDS AT BEARING ENDS OF ALL BEAMS, HEADERS (DROP BEAMS), FLUSH BEAMS, AND GIRDER TRUSSES. SEE POST SCHEDULE FOR STUD PACKS AND POST SPECIFIED ON PLAN.
6. FLOOR AND ROOF SHEATHING SHALL BE OSB TYPICAL EXCEPT AT CORRIDORS AND EXPOSED / EXTERIOR AREAS WHICH SHALL BE C-D EXPOSURE 1 PLYWOOD.
7. PROVIDE 3/8" MIN HORIZONTAL GAP BETWEEN EXTERIOR WALL SHEATHING PANELS TO ALLOW FOR SHRINKAGE. GAP TO BE PLACED AT THE LEVEL OF THE FLOOR SHEATHING AT EACH LEVEL (TYP)
8. TRUSSES SHALL RECEIVE DOUBLE TOP CHORD AT SPECIFIC DIAPHRAGM STRAPPING LOCATIONS PER PLAN AND NOTES. REFER TO WOOD DIAPHRAGM REINFORCING PLANS FOR MORE INFO.

WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:
1. ALL FRAMING EXPOSED TO EXTERIOR ELEMENTS TO BE PRESSURE TREATED (EXCLUDING FRT AND ENGINEERED LUMBER).

LEGEND:

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- = LOAD BEARING WOOD FRAMED STUD WALL
- = EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
- = INDICATES 3HR FIRE SEPARATION WALL. REFER TO ARCHITECTURAL DETAILS FOR MORE INFORMATION.
- = HEADER (DROP BEAM)
- = FLUSH BEAM
- = LEDGER
- = TRUSS / JOIST
- = GIRDER TRUSS
- HSS / W = STEEL BEAM

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LICENSE NO STATE OF LOUISIANA:
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SHEET TITLE:
Wood Framing Plan - East Building - Level 06 - Area B

SHEET NUMBER:
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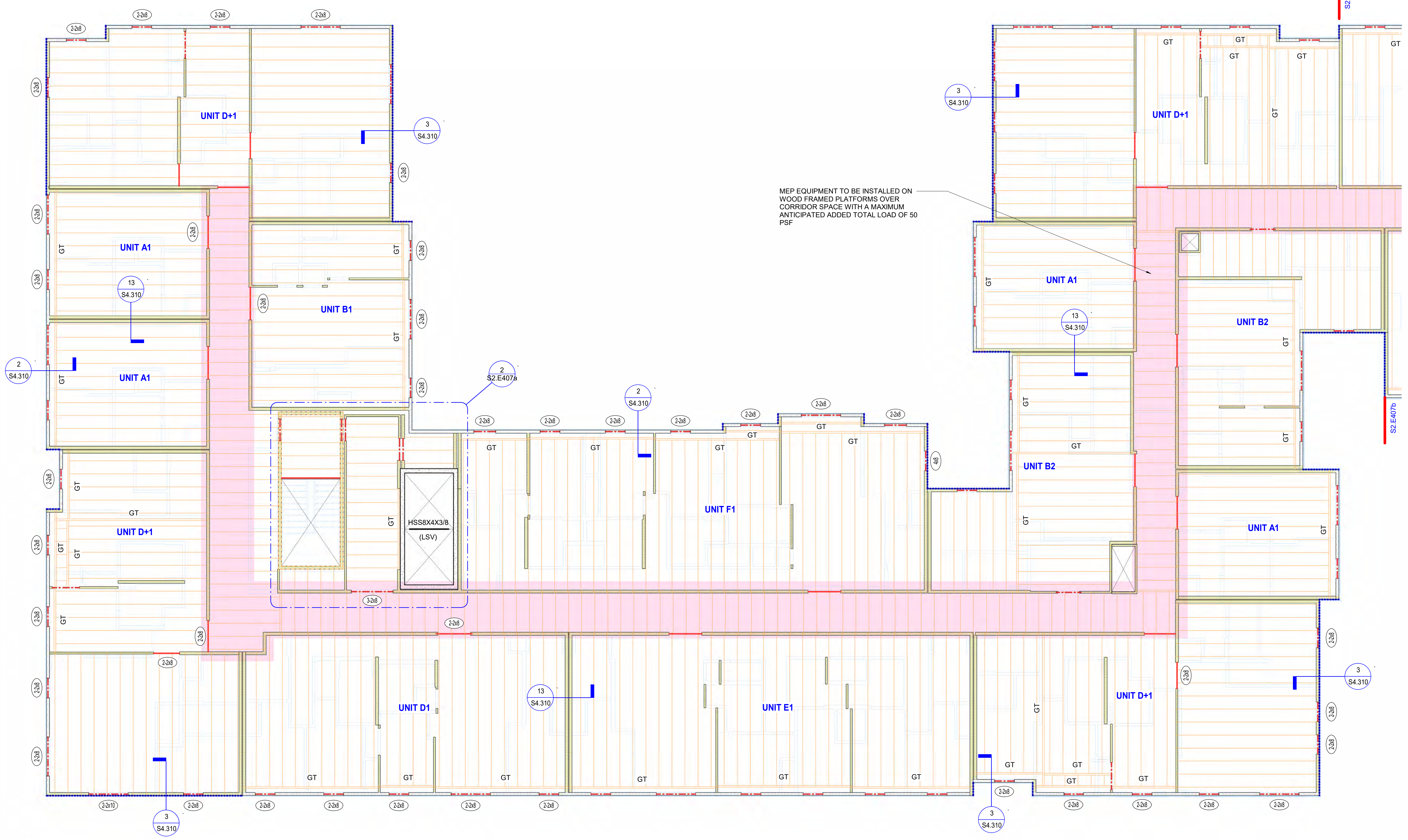
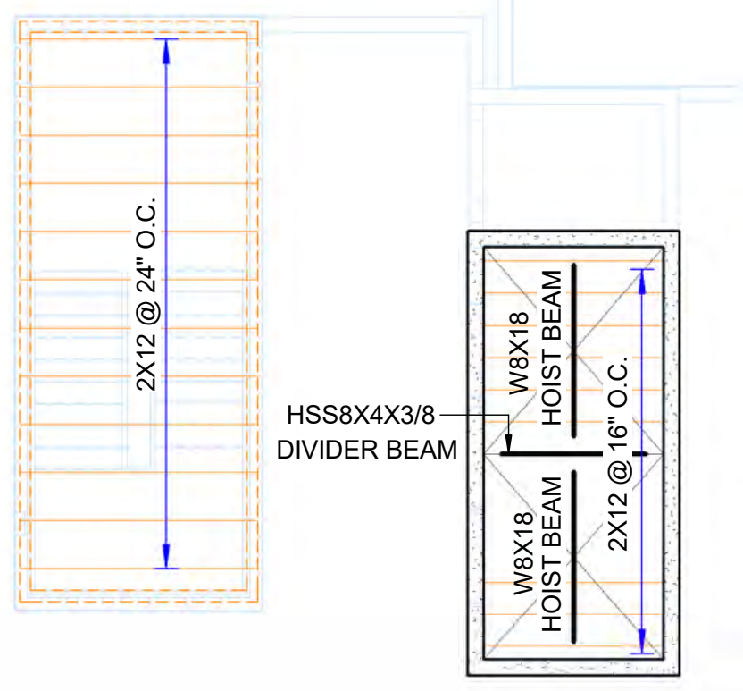


MEP EQUIPMENT TO BE INSTALLED ON WOOD FRAMED PLATFORMS OVER CORRIDOR SPACE WITH A MAXIMUM ANTICIPATED ADDED TOTAL LOAD OF 50 PSF

TRUE NORTH
PROJECT NORTH

1 Roof Framing Plan - East Building - Level 07 - Area F
1/8" = 1'-0"



2 Wood Framing Plan - Stair Roof - East Building
1/8" = 1'-0"

WOOD ROOF FRAMING NOTES:

1. ROOF FRAMING TO BE 24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES @ 24" OC MAX UNO. TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5 PSF OF THE TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS DESIGNER TO REF MEP, ARCH, EBM (EXTERIOR BUILDING MAINTENANCE) EQUIPMENT FOR ADDITIONAL ROOF LOADS AND EXACT LOCATION. IN THE ABSENSE OF SPECIFIC LOADING CRITERIA, TRUSS DESIGNER SHALL INCLUDE A DESIGN ALLOWANCE AS FOLLOWS:
   - AHU'S, BOILERS AND WATER TANKS = 100PSF DL
   - PHOTO-VOLTAIC (SOLAR) ARRAYS = 30PSF DL
   - UNSCHEDULED MAINTENANCE TIE-BACKS = 3,100 LBS ASD IMPACT LOAD APPLIED IN ANY DIRECTION
3. CONTRACTOR TO VERIFY ALL DIMENSIONS, ELEVATIONS, DEPRESSIONS, STEPS, FINISHES, PLATE HEIGHTS AND SPECIFIC/OVERALL BUILDING GEOMETRY WITH THE ARCHITECTURAL DRAWINGS AND NOTIFY ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION START
4. SEE WOOD FRAMING SPECIFICATIONS FOR FLOOR / ROOF SHEATHING AND MINIMUM LUMBER GRADE / SPECIES REQUIREMENTS.
5. SECTIONS CUT ON PLAN SHALL BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS. REFER TO TYPICAL DETAILS/SECTIONS FOR DETAILS / SECTIONS NOT SPECIFICALLY SHOWN OR CUT ON PLAN.
6. SEE ROOF UPLIFT SECTIONS FOR FASTENING OF ROOF LEVEL HEADERS, BEAMS AND GIRDER TRUSSES.
7. AT ROOF UPLIFT ROD LOCATIONS, NUTS SHALL BE RE-TIGHTENED AS REQUIRED PRIOR TO INTERIOR WALL SHEATHING INSTALLATION TO ACCOMMODATE WOOD SHRINKAGE
8. AT LOCATIONS WHERE ROOF UPLIFT RODS COINCIDE WITH SHEAR WALL END ANCHORAGE RODS, ONLY THE SHEAR WALL ANCHORAGE ROD NEED BE INSTALLED. ALL OTHER FASTNENING HARDWARE SHOWN ON THE ROOF UPLIFT DETAILS SHALL BE INSTALLED AS INDICATED.
9. TRUSSES SHALL RECEIVE DOUBLE TOP CHORD AT SPECIFIC DIAPHRAGM STRAPPING LOCATIONS PER PLAN AND NOTES. REFER TO WOOD DIAPHRAGM REINFORCING PLANS FOR MORE INFO.
10. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
11. SEE THIS SHEET FOR POST/HANGER SCHEDULE. ALL BEAM/HEADER BEARING POINTS NOT NOTED ON PLAN SHALL BE SUPPORTED PER NOTE 4 OF THE POST SCHEDULE.
12. ALL HEADERS BEARING ROOF TRUSSES SHALL BE LOCATED DIRECTLY BELOW THE DOUBLE TOP PLATE UNO. SEE HEADER UPLIFT DETAILS.
13. PROVIDE 3/8" MIN HORIZONTAL GAP BETWEEN EXTERIOR WALL SHEATHING PANELS TO ALLOW FOR SHRINKAGE. GAP TO BE PLACED AT THE LEVEL OF THE FLOOR SHEATHING AT EACH LEVEL, TYP.
14. ROOF TIE-BACK AND EBM ANCHORS, WHERE APPLICABLE, SHALL BE ANCHORD TO ROOF TRUSSES PER TYPICAL DETAILS. TRUSS SUPPLIER SHALL LOCATE ALL ANCHORS PER THE EBM/FACADE ACCESS CONSULTANT DRAWINGS IN COORDINATION WITH THE ARCHITECTURAL DRAWINGS. TRUSS SUPPLIER SHALL DESIGN THE SUPPORTING TRUSSES TO MEET THE LOAD DEMAND(S) SHOWN ON THE TYPICAL DETAILS.
15. AT LOCATIONS WITH PARAPET FRAMING INTENDED TO SUPPORT EBM LOADS SHALL BE REINFORCED PER TYPICAL DETAILS. ALL OTHER LOCATIONS HAVE BEEN DESIGNED TO MEET WIND/SEISMIC LATERAL FORCES ONLY.
16. SEE TYPICAL DETAILS FOR MECHANICAL AND HVAC PLATFORM FRAMING.
17. REFER TO WOOD SHRINKAGE NOTES IN GENERAL NOTES FOR ESTIMATED BUILDING MOVEMENT DUE TO WOOD SHRINKAGE. CONTRACTOR SHALL COORDINATE ESTIMATED VERTICAL MOVEMENT WITH OTHER TRADES AS NEEDED. IN THE ABSENCE OF ESTIMATED BUILDING VERTICAL MOVEMENT, ASSUME 1/4" PER FLOOR.
18. ROOF TRUSSES SHALL BE DESIGNED FOR DRAG LOADS SHOWN ON PLAN. IN THE ABSENSE OF D.F. (DRAG FORCES), ALL TRUSSES PARALLEL TO SHEARWALLS SHALL BE DESIGNED TO RESIST MINIMUM 400 PLF DRAG FORCE (TYPICAL).

WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
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= TRUSS / JOIST
= GIRDER TRUSS
HSS / W = STEEL BEAM

VSM²
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Structural Engineers

PROJECT # : 26154
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NILES BOLTON ASSOCIATES

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LICENSE NO STATE OF LOUISIANA:
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SHEET TITLE:
Wood Framing Plan - East Building - Roof - Area A

SHEET NUMBER:
S2.E407a

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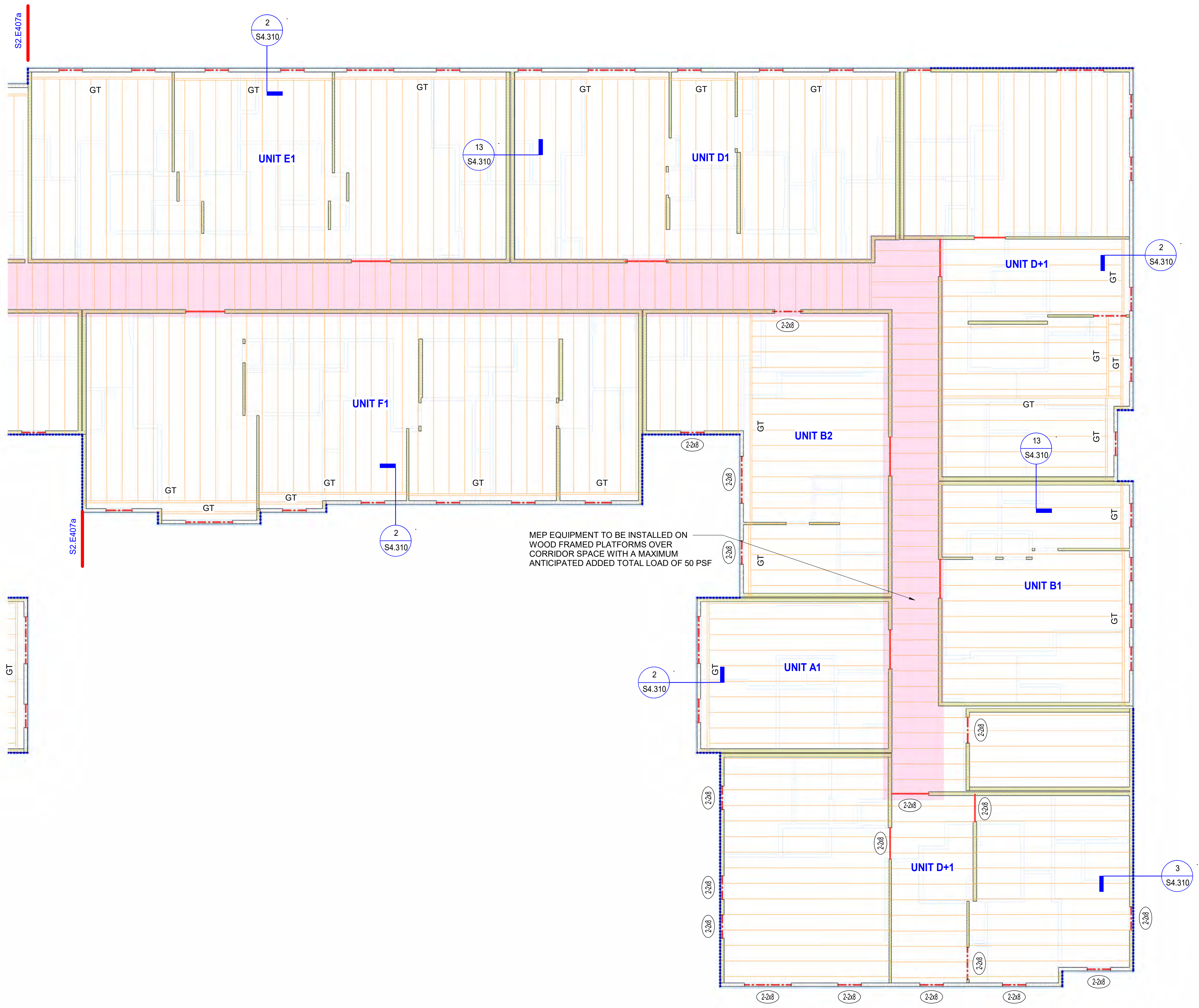

MEP EQUIPMENT TO BE INSTALLED ON WOOD FRAMED PLATFORMS OVER CORRIDOR SPACE WITH A MAXIMUM ANTICIPATED ADDED TOTAL LOAD OF 50 PSF

TRUE NORTH
PROJECT NORTH

1 Roof Framing Plan - East Building - Level 07 - Area G
1/8" = 1'-0"

WOOD ROOF FRAMING NOTES:

1. ROOF FRAMING TO BE 24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES @ 24" OC MAX UNO. TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5 PSF OF THE TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
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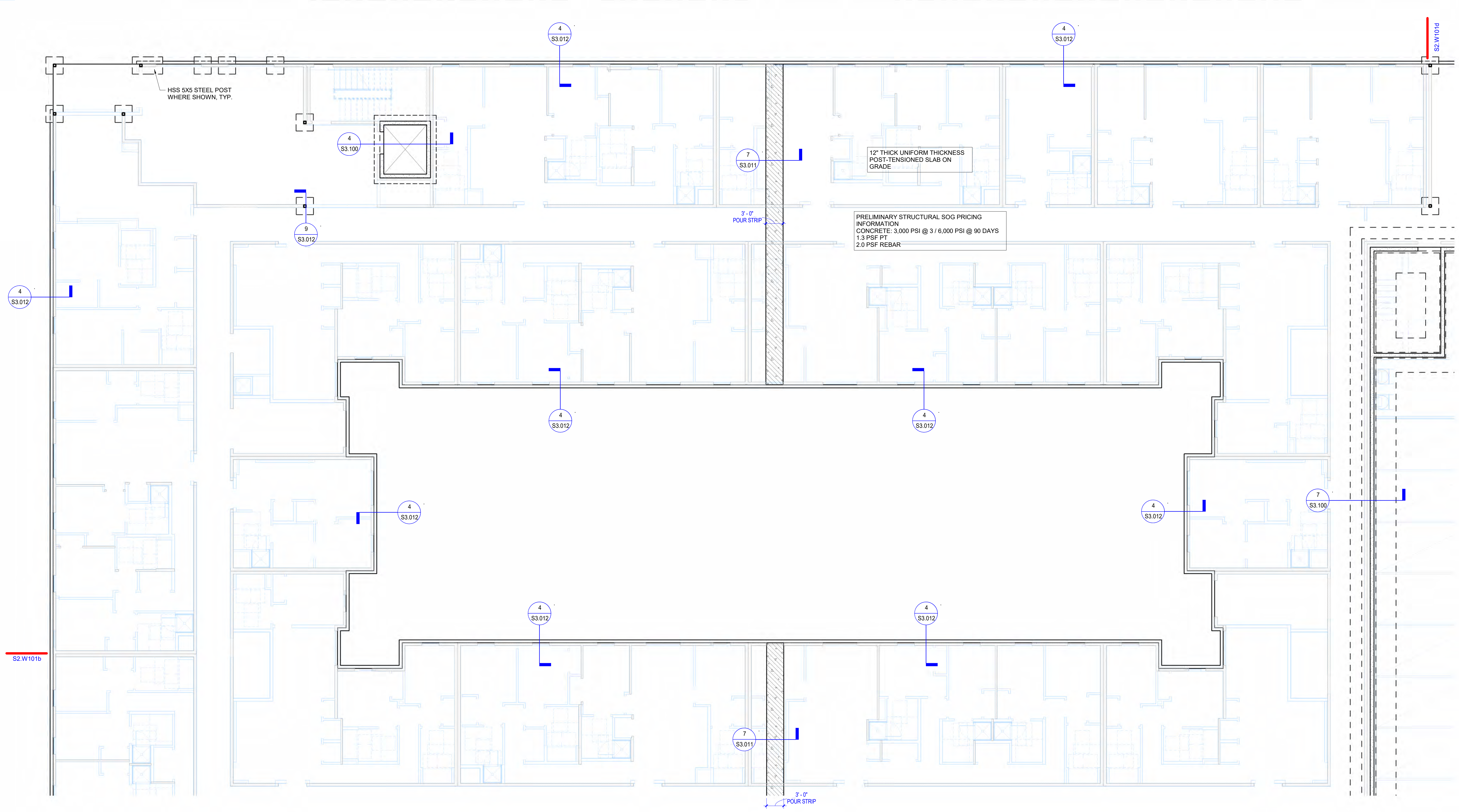
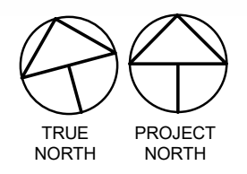


12" THICK UNIFORM THICKNESS POST-TENSIONED SLAB ON GRADE

PRELIMINARY STRUCTURAL SOG PRICING INFORMATION
CONCRETE: 3,000 PSI @ 3 / 6,000 PSI @ 90 DAYS
1.3 PSF PT
2.0 PSF REBAR



① Foundation Plan - West Building - Level 01 - Area A
1/8" = 1'-0"

FOUNDATION NOTES:

1. 100'-0" = XXX.XX' RE: CIVIL
2. CONTRACTOR TO CROSS REFERENCE AND/OR VERIFY ALL DIMENSIONS, ELEVATIONS, RECESSES DEPRESSIONS, CURB SIZES/LOCATIONS, WALL ELEVATIONS, GUARDRAILS, FINISHES, OPENINGS AND SLAB PENETRATION SIZES/LOCATIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ACHITECT AND ENGINEER OF ANY DISCREPANCIES PIOR TO CONSTRUCTION.
3. REFER TO ARCHITECTURAL SLAB DIMENSIONAL CONTROL PLAN FOR ALL SLAB EDGE AND OPENING DIMENSIONS NOT SHOWN, TYPICAL.
4. TOP OF CONCRETE (TOC) = REF PLAN
5. SLAB THICKNESS = SEE PLAN
6. CONCRETE STRENGTH = REFER TO GENERAL NOTES
7. ALL BUILDING EXTERIOR AND LOAD BEARING WALL SOLE PLATES SHALL BE ANCHORED TO THE CONCRETE FOUNDATION WITH 5/8"Ø X 7" EMBEDMENT ANCHOR BOLTS W/ 3"X3"X1/4" PLATE WASHERS @ 48" O.C. (U.N.O.) WITH (2) BOLTS MINIMUM PER BOARD. ALL NON LOAD BEARING INTER'IOR WALLS SHALL HAVE SILL PLATES ATTACHED TO THE FOUNDATION W/ 0.177Ø X 3 1/4" P.A.F. @ 24" O.C. MAXIMUM. IN ADDITION, FASTENERS FOR PRESERVTIVE-TREATED WOOD SHALL BE AS FOLLOWS:

   A. UNCOATED IF TREATED WITH SBS/DOT & ZINC BORATE.
   B. GALVANIZED (ASTM A153) IF TREATED WITH ACQ-C, ACQ-D (CARBONATE), CA-B & CB-A; WITH OR WITHOUT AMMONIA.
   C. STAINLESS STEEL (TYPE 303, 304, 305 OR 316). IF WOODS HAVE ACTUAL RETENTION LEVLES GREATER THAN 0.40" PSF FOR ACQ, 0.41 PSF FOR CBA-A 0.21 PSF FOR CA-B (GROUND CONTACT LEVEL). OR ACZA.
   D. ALTERNATIEVELY, ALL NON-SHEAR WALL ANCHORS BOLTS AND P.A.F.S MAY BE REPLACED WITH SIMPSON MASA/MAMSAP MUDSILL ANCHORS @ 24" O.C. (4" MIN. DISTANCE) SEE WOOD SHEAR WALL SCHEDULE FOR ANCHORAGE AT SHEAR WALLS.

9. REFER TO SHEETS S2.202.A AND S2.202.B FOR WOOD SHEARWALLS. AND SHEETS S5.411 THROUGH S5.417 FOR SHEARWALL SCHEDULE AND END ANCHORAGE DETAILS.
10. PERIMETER FOUNDATION DRAIN SHALL BE INSTALLED ON 0.5% MIN. GRADIENT AS SHOWN ON SHEET REFER TO CIVIL AND GEOTECHNICAL FOR ALL OTHER REQUIREMENTS, TYP.

STRUCTURAL SLAB ON GRADE REINFORCING PLAN NOTES

1. ALL REINFORCING SHOWN SHALL BE PLACED WITHIN SLAB THICKNESS AS FOLLOWS:

   (T) = TOP OF SLAB
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   REFER TO GENERAL NOTES FOR MINIMUM CONCRETE COVER AT TOP AND BOTTOM REINFORCEMENT, TYP.

2. AT STRUCTURAL SLAB ON GRADE, PROVIDE (1) #5 CONTINUOUS (TB) AT ALL SLAB EDGES AND LAP SPLICE AS REQUIRED. AT RE-ENTRANT CORNERS, EXTEND 2'-6 MIN. PAST RE-ENTRANT CORNER IN EACH DIRECTION. AT OUTSIDE CORNERS, PROVIDE 2'-6" x 2'-6" CORNER BARS (TB).

3. BARS WITH LENGTHS GREATER THAN 60'-0" MAY BE SPLICED AT ANY POINT WITH A MIN. SPLICE LENGTH OF Ld. IN ADDITION, STAGGER SPLICES SO THAT NO MORE THAN 50% OF BARS ARE SPLICED AT ANY ONE LOCATION.

4. AT NON-STRUCTURAL SLAB ON GRADE, PROVIDE (1) #4 CONTINUOUS (TB) AT ALL SLAB EDGES AND LAP SPLICE AS REQUIRED. AT RE-ENTRANT CORNERS, EXTEND 2'-0" MIN. PAST RE-ENTRANT CORNER IN EACH DIRECTION. AT OUTSIDE CORNERS, PROVIDE 2'-0" x 2'-0" CORNER BARS (TB).

5. AT CONSTRUCTION JOINTS, ALL SCHEDULED TB REBAR SHALL EXTEND A MIN OF 3'-0" PAST CONSTRUCTION JOINT EACH SIDE, TYP.

6. SEE TYPICAL DETAILS FOR SLAB REINFORCEMENT NOT SHOWN OR SPECIFICALLY NOTED ON PLAN AT ALL SLAB STEPS, CHANGES IN ELEVATION, EDGES, CORNERS, RE-ENTRANT CORNERS, RECESSES , DEPRESSIONS, RAMPS, ETC. ON SHEETS THRU , TYPICAL.

   Fe = 21.6 KLF

7. INDICATES 1/2" ⌀ FULLY ENCAPSULATED POST TENSION TENDONS PER PTI "SPECIFICATIONS FOR UNBONDED SINGLE STRAND TENDONS " AT SLAB MID-DEPTH. (TYP.) FOR SINGLE ENDED STRESSING LIVE AND DEAD ENDS MAY BE SWITCHED AT CONTRACTOR'S OPTION. ALL POST MENTIONED TENDONS ARE TO BE CHAIRED TO MAINTAIN CGS AT MID-DEPTH OF SLAB ALONG FULL LENGTH OF TENDON UNO ON PLAN, AND REFER TO FOR ADDITIONAL REQUIREMENTS, TYP.

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8. POST-TENSIONING SUPPLIER SHALL PROVIDE MINIMUM NUMBER OF TENDONS EQUIVALENT TO THE EFFECTIVE FORCE PER NOTE 7 CONSIDERING A MAXIMUM TENDON GROUP SIZE OF (4) TENDONS AND MAXIMUM HORIZONTAL SPACING OF 5'-0" BETWEEN CENTERLINE OF TENDON GROUPS, TYP.

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10. TABLE BELOW REPRESENTS MINIMUM **ADDITIONAL** PRESTRESSING FORCE REQUIRED TO OVERCOME SUBGRADE FRICTION LOSSES BASED ON TOTAL LENGTH OF TENDON(S) MEASURED ANCHOR TO ANCHOR IN A GIVEN DIRECTION WITH A COEFFICIENT OF FRiCTION OF 0.75..

TOTAL PT TENDON LENGTH	ADDITIONAL PRESTRESS FORCE
(FT)	(KLF)
25	1.4
50	2.8
75	4.2
100	5.6
125	7.0
150	8.4
175	9.8
200	11.3
225	12.7
250	14.1

NOTE: THE ADDITIONAL PRESTRESS FORCE OF THIS TABLE SHALL BE USED IN COMBINATION WITH THE FINAL EFFECTIVE FORCE OF NOTE 7. FOR INSTANCE, A 100' PT TENDON WILL REQUIRE A TOTAL FORCE OF 21.6 KLF + 5.6 KLF = 27.2 KLF

11. STRUCTURAL SLAB ON GRADE SHALL BE UNDERLAIN BY (2) LAYERS OF 6 MIL MIN. POLYTHYELENE SHEETING, TYPICAL. REF. ARCH FOR MORE INFO.
12. PROVIDE #5 BARS TOP & BOTTOM AT 12" O.C. MAX. IN AREAS WHERE PT TENDONS CANNOT BE PLACED AND STRESSED (TYPICALLY SPANS LESS THAN 15'-0").
    G.C. TO COORDINATE WITH PT SUPPLIER.

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PROJECT # : 26154
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No.	Description	Date
	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

LV COLLECTIVE

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- NO. 85617

SHEET TITLE:
Foundation Plan - West Building - Level 01 - Area A

SHEET NUMBER:
S2.W101a

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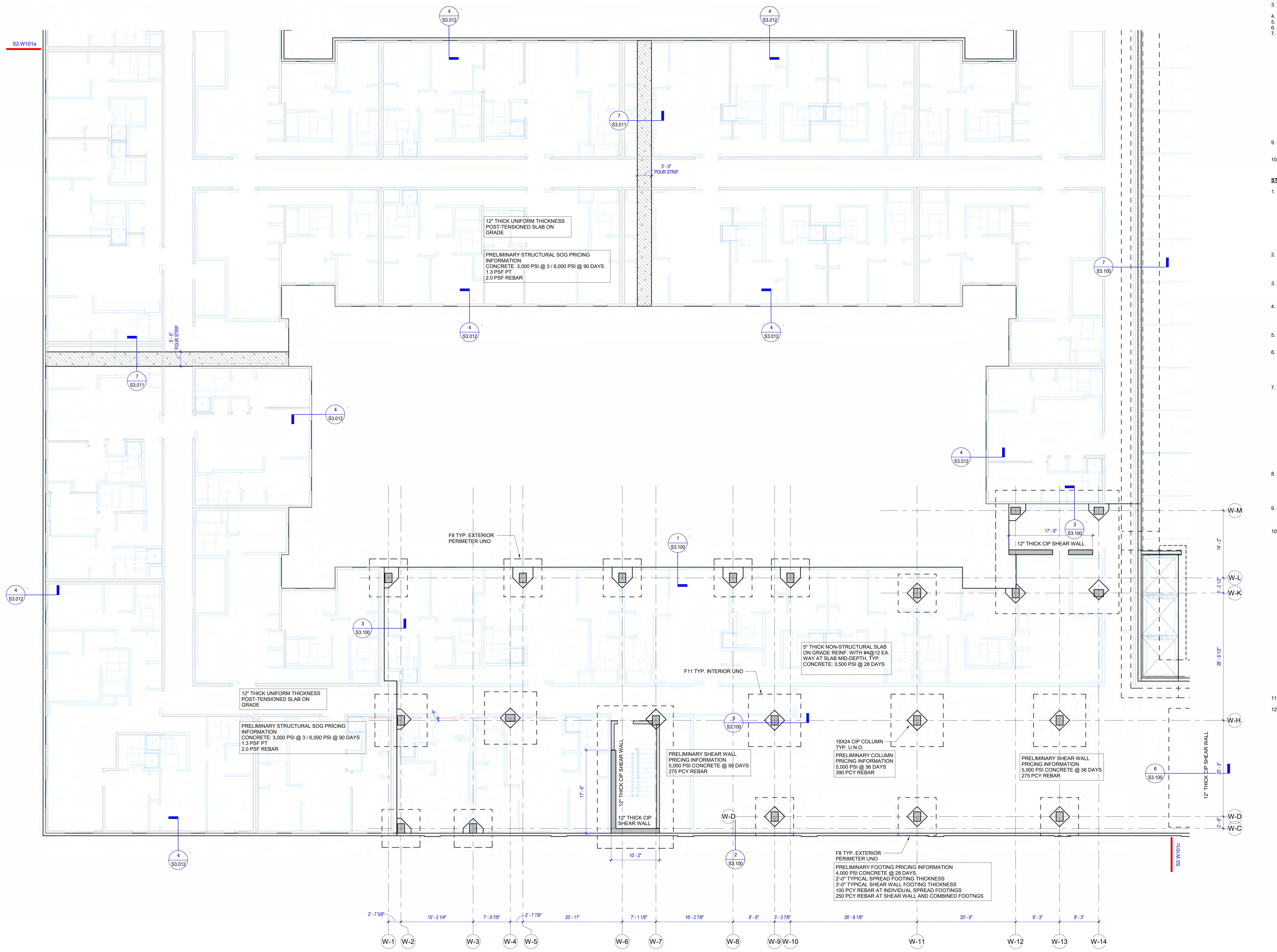


12" THICK UNIFORM THICKNESS POST-TENSIONED SLAB ON GRADE

PRELIMINARY STRUCTURAL SOG PRICING INFORMATION
CONCRETE: 3,000 PSI @ 3 / 6,000 PSI @ 90 DAYS
1.3 PSF PT
2.0 PSF REBAR

12" THICK UNIFORM THICKNESS POST-TENSIONED SLAB ON GRADE

PRELIMINARY STRUCTURAL SOG PRICING INFORMATION
CONCRETE: 3,000 PSI @ 3 / 6,000 PSI @ 90 DAYS
1.3 PSF PT
2.0 PSF REBAR

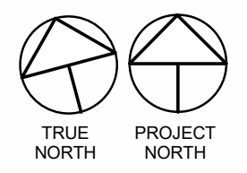
5" THICK NON-STRUCTURAL SLAB ON GRADE REINF. WITH #4@12 EA. WAY AT SLAB MID-DEPTH, TYP.
CONCRETE: 3,500 PSI @ 28 DAYS

PRELIMINARY SHEAR WALL PRICING INFORMATION
5,000 PSI CONCRETE @ 56 DAYS
275 PCY REBAR

PRELIMINARY COLUMN PRICING INFORMATION
5,000 PSI @ 56 DAYS
390 PCY REBAR

PRELIMINARY SHEAR WALL PRICING INFORMATION
5,000 PSI CONCRETE @ 56 DAYS
275 PCY REBAR

PRELIMINARY FOOTING PRICING INFORMATION
4,000 PSI CONCRETE @ 28 DAYS
2'-0" TYPICAL SPREAD FOOTING THICKNESS
3'-0" TYPICAL SHEAR WALL FOOTING THICKNESS
100 PCY REBAR AT INDIVIDUAL SPREAD FOOTINGS
250 PCY REBAR AT SHEAR WALL AND COMBINED FOOTNGS



1 Foundation Plan - West Building - Level 01 - Area B
1/8" = 1'-0"

FOUNDATION NOTES:

1. 100'-0" = XXX.XX' RE: CIVIL
2. CONTRACTOR TO CROSS REFERENCE AND/OR VERIFY ALL DIMENSIONS, ELEVATIONS, RECESSES DEPRESSIONS, CURB SIZES/LOCATIONS, WALL ELEVATIONS, GUARDRAILS, FINISHES, OPENINGS AND SLAB PENETRATION SIZES/LOCATIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ACHITECT AND ENGINEER OF ANY DISCREPANCIES PIOR TO CONSTRUCTION.
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4. TOP OF CONCRETE (TOC) = REF PLAN
5. SLAB THICKNESS = SEE PLAN
6. CONCRETE STRENGTH = REFER TO GENERAL NOTES
7. ALL BUILDING EXTERIOR AND LOAD BEARING WALL SOLE PLATES SHALL BE ANCHORED TO THE CONCRETE FOUNDATION WITH 5/8"Ø X 7" EMBEDMENT ANCHOR BOLTS W/ 3"X3"X1/4" PLATE WASHERS @ 48" O.C. (U.N.O.) WITH (2) BOLTS MINIMUM PER BOARD. ALL NON LOAD BEARING INTER'IOR WALLS SHALL HAVE SILL PLATES ATTACHED TO THE FOUNDATION W/ 0.177Ø X 3 1/4" P.A.F. @ 24" O.C. MAXIMUM. IN ADDITION, FASTENERS FOR PRESERVTIVE-TREATED WOOD SHALL BE AS FOLLOWS:

   A. UNCOATED IF TREATED WITH SBS/DOT & ZINC BORATE.
   B. GALVANIZED (ASTM A153) IF TREATED WITH ACQ-C, ACQ-D (CARBONATE), CA-B & CB-A; WITH OR WITHOUT AMMONIA.
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3. BARS WITH LENGTHS GREATER THAN 60'-0" MAY BE SPLICED AT ANY POINT WITH A MIN. SPLICE LENGTH OF Ld. IN ADDITION, STAGGER SPLICES SO THAT NO MORE THAN 50% OF BARS ARE SPLICED AT ANY ONE LOCATION.

4. AT NON-STRUCTURAL SLAB ON GRADE, PROVIDE (1) #4 CONTINUOUS (TB) AT ALL SLAB EDGES AND LAP SPLICE AS REQUIRED. AT RE-ENTRANT CORNERS, EXTEND 2'-0" MIN. PAST RE-ENTRANT CORNER IN EACH DIRECTION. AT OUTSIDE CORNERS, PROVIDE 2'-0" x 2'-0" CORNER BARS (TB).

5. AT CONSTRUCTION JOINTS, ALL SCHEDULED TB REBAR SHALL EXTEND A MIN OF 3'-0" PAST CONSTRUCTION JOINT EACH SIDE, TYP.

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7. INDICATES 1/2" ⌀ FULLY ENCAPSULATED POST TENSION TENDONS PER PTI "SPECIFICATIONS FOR UNBONDED SINGLE STRAND TENDONS " AT SLAB MID-DEPTH. (TYP.) FOR SINGLE ENDED STRESSING LIVE AND DEAD ENDS MAY BE SWITCHED AT CONTRACTOR'S OPTION. ALL POST MENTIONED TENDONS ARE TO BE CHAIRED TO MAINTAIN CGS AT MID-DEPTH OF SLAB ALONG FULL LENGTH OF TENDON UNO ON PLAN, AND REFER TO FOR ADDITIONAL REQUIREMENTS, TYP.

   Fe=TYPICAL EFFECTIVE DESIGN FORCE IN EACH DIRECTION UNO

8. POST-TENSIONING SUPPLIER SHALL PROVIDE MINIMUM NUMBER OF TENDONS EQUIVALENT TO THE EFFECTIVE FORCE PER NOTE 7 CONSIDERING A MAXIMUM TENDON GROUP SIZE OF (4) TENDONS AND MAXIMUM HORIZONTAL SPACING OF 5'-0" BETWEEN CENTERLINE OF TENDON GROUPS, TYP.

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TOTAL PT TENDON LENGTH	ADDITIONAL PRESTRESS FORCE
(FT)	(KLF)
25	1.4
50	2.8
75	4.2
100	5.6
125	7.0
150	8.4
175	9.8
200	11.3
225	12.7
250	14.1

NOTE: THE ADDITIONAL PRESTRESS FORCE OF THIS TABLE SHALL BE USED IN COMBINATION WITH THE FINAL EFFECTIVE FORCE OF NOTE 7. FOR INSTANCE, A 100' PT TENDON WILL REQUIRE A TOTAL FORCE OF 21.6 KLF + 5.6 KLF = 27.2 KLF

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12. PROVIDE #5 BARS TOP & BOTTOM AT 12" O.C. MAX. IN AREAS WHERE PT TENDONS CANNOT BE PLACED AND STRESSED (TYPICALLY SPANS LESS THAN 15'-0").
    G.C. TO COORDINATE WITH PT SUPPLIER.

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PROJECT # : 26154
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No.	Description	Date
	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Foundation Plan - West Building - Level 01 - Area B

SHEET NUMBER:
S2.W101b

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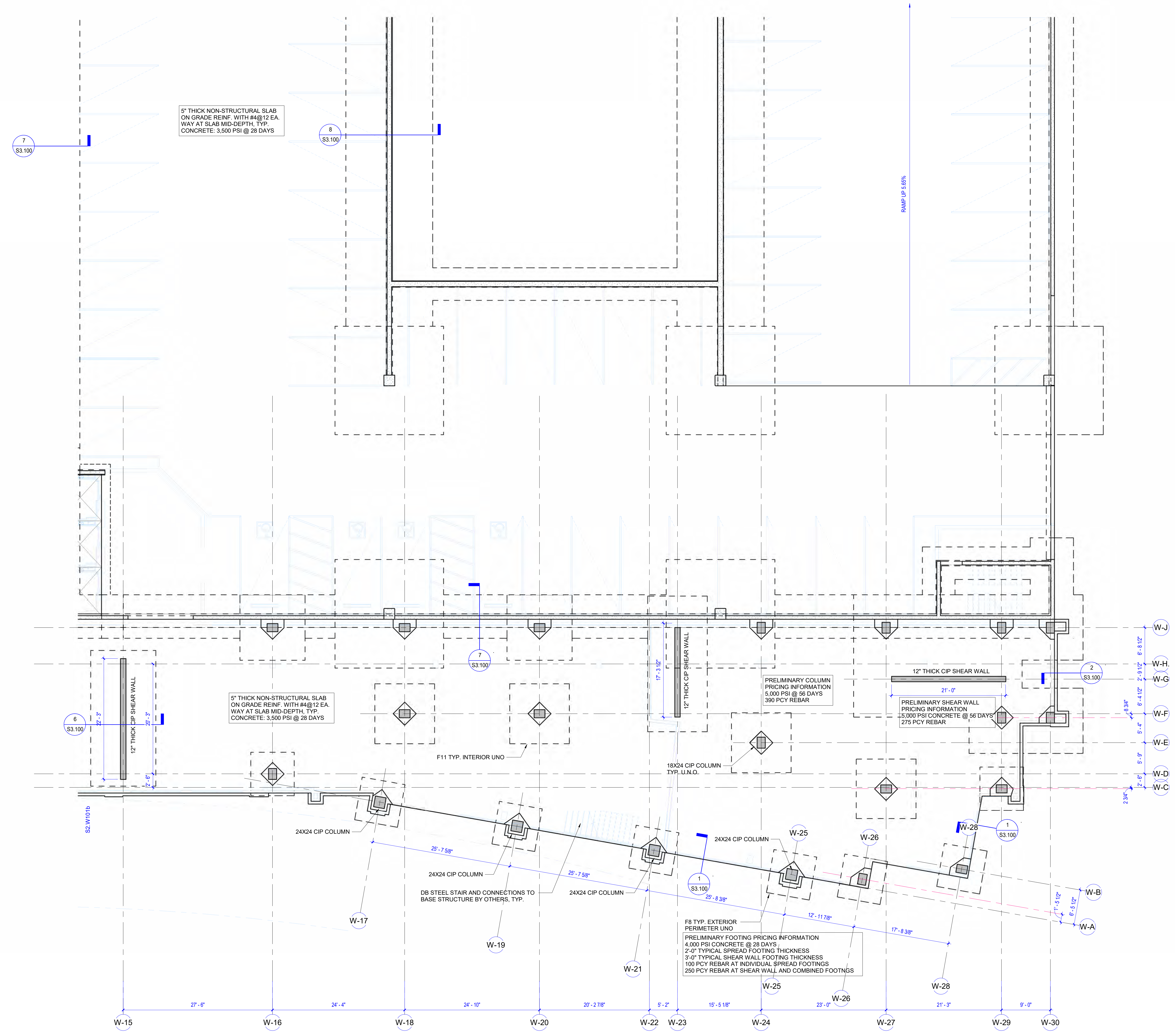
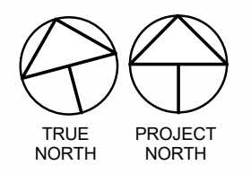

5" THICK NON-STRUCTURAL SLAB ON GRADE REINF. WITH #4@12 EA. WAY AT SLAB MID-DEPTH, TYP. CONCRETE: 3,500 PSI @ 28 DAYS
7
S3.100
8
S3.100
RAMP UP 5.65%
7
S3.100
6
S3.100
22' - 3"
12" THICK CIP SHEAR WALL
20' - 3"
2' - 6"
5" THICK NON-STRUCTURAL SLAB ON GRADE REINF. WITH #4@12 EA. WAY AT SLAB MID-DEPTH, TYP. CONCRETE: 3,500 PSI @ 28 DAYS
F11 TYP. INTERIOR UNO
S2.W101b
24X24 CIP COLUMN
25' - 7 5/8"
24X24 CIP COLUMN
25' - 7 5/8"
DB STEEL STAIR AND CONNECTIONS TO BASE STRUCTURE BY OTHERS, TYP.
24X24 CIP COLUMN
17' - 3 1/2"
12" THICK CIP SHEAR WALL
18X24 CIP COLUMN TYP. U.N.O.
PRELIMINARY COLUMN PRICING INFORMATION 5,000 PSI @ 56 DAYS 390 PCY REBAR
12" THICK CIP SHEAR WALL
21' - 0"
PRELIMINARY SHEAR WALL PRICING INFORMATION 5,000 PSI CONCRETE @ 56 DAYS 275 PCY REBAR
2
S3.100
24X24 CIP COLUMN
1
S3.100
1
S3.100
25' - 8 3/8"
12' - 11 7/8"
17' - 8 3/8"
F8 TYP. EXTERIOR PERIMETER UNO
PRELIMINARY FOOTING PRICING INFORMATION 4,000 PSI CONCRETE @ 28 DAYS 2'-0" TYPICAL SPREAD FOOTING THICKNESS 3'-0" TYPICAL SHEAR WALL FOOTING THICKNESS 100 PCY REBAR AT INDIVIDUAL SPREAD FOOTINGS 250 PCY REBAR AT SHEAR WALL AND COMBINED FOOTNGS
6' - 8 1/2"
2' - 9 1/2"
6' - 4 1/2"
8 3/4"
5' - 4"
5' - 9"
2' - 6"
2 3/4"
1' - 5 1/2"
6' - 5 1/2"
W-J
W-H.
W-G
W-F
W-E
W-D
W-C
W-B
W-A
W-17
W-19
W-21
W-25
W-26
W-28
W-25
W-26
W-28
27' - 6"
24' - 4"
24' - 10"
20' - 2 7/8"
5' - 2"
15' - 5 1/8"
23' - 0"
21' - 3"
9' - 0"
W-15
W-16
W-18
W-20
W-22
W-23
W-24
W-27
W-29
W-30



① Foundation Plan - West Building - Level 01 - Area C
1/8" = 1'-0"

FOUNDATION NOTES:

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   REFER TO GENERAL NOTES FOR MINIMUM CONCRETE COVER AT TOP AND BOTTOM REINFORCEMENT, TYP.

2. AT STRUCTURAL SLAB ON GRADE, PROVIDE (1) #5 CONTINUOUS (TB) AT ALL SLAB EDGES AND LAP SPLICE AS REQUIRED. AT RE-ENTRANT CORNERS, EXTEND 2'-6 MIN. PAST RE-ENTRANT CORNER IN EACH DIRECTION. AT OUTSIDE CORNERS, PROVIDE 2'-6" x 2'-6" CORNER BARS (TB).

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250	14.1

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11. STRUCTURAL SLAB ON GRADE SHALL BE UNDERLAIN BY (2) LAYERS OF 6 MIL MIN. POLYTHYELENE SHEETING, TYPICAL. REF. ARCH FOR MORE INFO.
12. PROVIDE #5 BARS TOP & BOTTOM AT 12" O.C. MAX. IN AREAS WHERE PT TENDONS CANNOT BE PLACED AND STRESSED (TYPICALLY SPANS LESS THAN 15'-0").
    G.C. TO COORDINATE WITH PT SUPPLIER.

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Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
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BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

LV COLLECTIVE

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SHEET TITLE:
Foundation Plan - West Building - Level 01 - Area C

SHEET NUMBER:
S2.W101c

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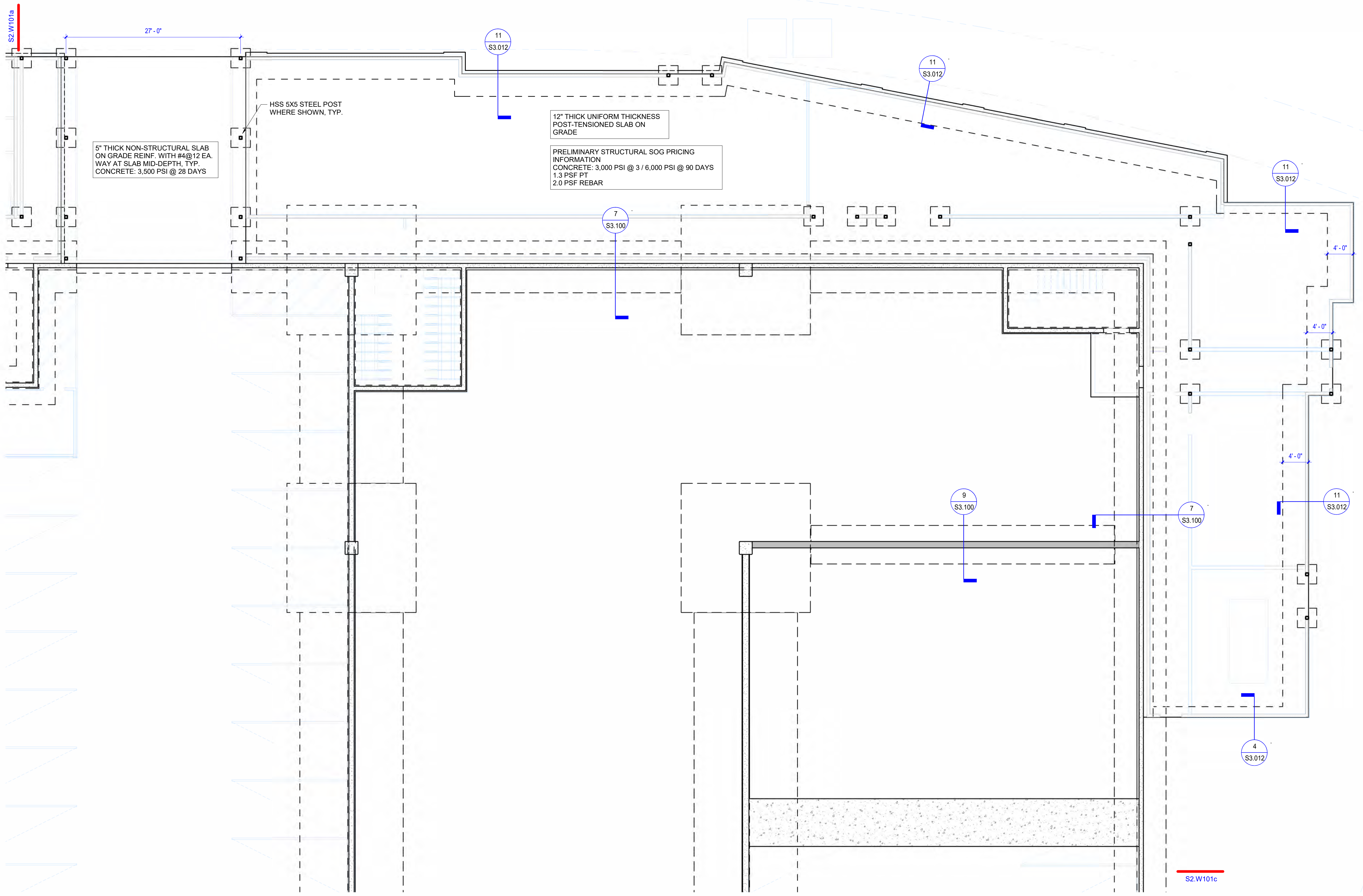
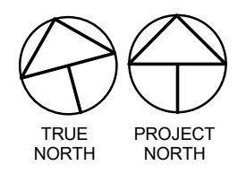

S2.W101a
27' - 0"
11
S3.012
HSS 5X5 STEEL POST WHERE SHOWN, TYP.
5" THICK NON-STRUCTURAL SLAB ON GRADE REINF. WITH #4@12 EA. WAY AT SLAB MID-DEPTH, TYP. CONCRETE: 3,500 PSI @ 28 DAYS
12" THICK UNIFORM THICKNESS POST-TENSIONED SLAB ON GRADE
PRELIMINARY STRUCTURAL SOG PRICING INFORMATION CONCRETE: 3,000 PSI @ 3 / 6,000 PSI @ 90 DAYS 1.3 PSF PT 2.0 PSF REBAR
11
S3.012
11
S3.012
7
S3.100
4' - 0"
4' - 0"
4' - 0"
9
S3.100
7
S3.100
11
S3.012
4
S3.012
S2.W101c



1 Foundation Plan - West Building - Level 01 - Area D
1/8" = 1'-0"

FOUNDATION NOTES:

1. 100'-0" = XXX.XX' RE: CIVIL
2. CONTRACTOR TO CROSS REFERENCE AND/OR VERIFY ALL DIMENSIONS, ELEVATIONS, RECESSES DEPRESSIONS, CURB SIZES/LOCATIONS, WALL ELEVATIONS, GUARDRAILS, FINISHES, OPENINGS AND SLAB PENETRATION SIZES/LOCATIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ACHITECT AND ENGINEER OF ANY DISCREPANCIES PIOR TO CONSTRUCTION.
3. REFER TO ARCHITECTURAL SLAB DIMENSIONAL CONTROL PLAN FOR ALL SLAB EDGE AND OPENING DIMENSIONS NOT SHOWN, TYPICAL.
4. TOP OF CONCRETE (TOC) = REF PLAN
5. SLAB THICKNESS = SEE PLAN
6. CONCRETE STRENGTH = REFER TO GENERAL NOTES
7. ALL BUILDING EXTERIOR AND LOAD BEARING WALL SOLE PLATES SHALL BE ANCHORED TO THE CONCRETE FOUNDATION WITH 5/8"Ø X 7" EMBEDMENT ANCHOR BOLTS W/ 3"X3"X1/4" PLATE WASHERS @ 48" O.C. (U.N.O.) WITH (2) BOLTS MINIMUM PER BOARD. ALL NON LOAD BEARING INTER'IOR WALLS SHALL HAVE SILL PLATES ATTACHED TO THE FOUNDATION W/ 0.177Ø X 3 1/4" P.A.F. @ 24" O.C. MAXIMUM. IN ADDITION, FASTENERS FOR PRESERVTIVE-TREATED WOOD SHALL BE AS FOLLOWS:

   A. UNCOATED IF TREATED WITH SBS/DOT & ZINC BORATE.
   B. GALVANIZED (ASTM A153) IF TREATED WITH ACQ-C, ACQ-D (CARBONATE), CA-B & CB-A; WITH OR WITHOUT AMMONIA.
   C. STAINLESS STEEL (TYPE 303, 304, 305 OR 316). IF WOODS HAVE ACTUAL RETENTION LEVLES GREATER THAN 0.40" PSF FOR ACQ, 0.41 PSF FOR CBA-A 0.21 PSF FOR CA-B (GROUND CONTACT LEVEL). OR ACZA.
   D. ALTERNATIEVELY, ALL NON-SHEAR WALL ANCHORS BOLTS AND P.A.F.S MAY BE REPLACED WITH SIMPSON MASA/MAMSAP MUDSILL ANCHORS @ 24" O.C. (4" MIN. DISTANCE) SEE WOOD SHEAR WALL SCHEDULE FOR ANCHORAGE AT SHEAR WALLS.

9. REFER TO SHEETS S2.202.A AND S2.202.B FOR WOOD SHEARWALLS. AND SHEETS S5.411 THROUGH S5.417 FOR SHEARWALL SCHEDULE AND END ANCHORAGE DETAILS.
10. PERIMETER FOUNDATION DRAIN SHALL BE INSTALLED ON 0.5% MIN. GRADIENT AS SHOWN ON SHEET REFER TO CIVIL AND GEOTECHNICAL FOR ALL OTHER REQUIREMENTS, TYP.

STRUCTURAL SLAB ON GRADE REINFORCING PLAN NOTES

1. ALL REINFORCING SHOWN SHALL BE PLACED WITHIN SLAB THICKNESS AS FOLLOWS:

   (T) = TOP OF SLAB
   (B) = BOTTOM OF SLAB
   (M) = MID-DEPTH OF SLAB
   (TB) = TOP AND BOTTOM OF SLAB

   REFER TO GENERAL NOTES FOR MINIMUM CONCRETE COVER AT TOP AND BOTTOM REINFORCEMENT, TYP.

2. AT STRUCTURAL SLAB ON GRADE, PROVIDE (1) #5 CONTINUOUS (TB) AT ALL SLAB EDGES AND LAP SPLICE AS REQUIRED. AT RE-ENTRANT CORNERS, EXTEND 2'-6 MIN. PAST RE-ENTRANT CORNER IN EACH DIRECTION. AT OUTSIDE CORNERS, PROVIDE 2'-6" x 2'-6" CORNER BARS (TB).

3. BARS WITH LENGTHS GREATER THAN 60'-0" MAY BE SPLICED AT ANY POINT WITH A MIN. SPLICE LENGTH OF Ld. IN ADDITION, STAGGER SPLICES SO THAT NO MORE THAN 50% OF BARS ARE SPLICED AT ANY ONE LOCATION.

4. AT NON-STRUCTURAL SLAB ON GRADE, PROVIDE (1) #4 CONTINUOUS (TB) AT ALL SLAB EDGES AND LAP SPLICE AS REQUIRED. AT RE-ENTRANT CORNERS, EXTEND 2'-0" MIN. PAST RE-ENTRANT CORNER IN EACH DIRECTION. AT OUTSIDE CORNERS, PROVIDE 2'-0" x 2'-0" CORNER BARS (TB).

5. AT CONSTRUCTION JOINTS, ALL SCHEDULED TB REBAR SHALL EXTEND A MIN OF 3'-0" PAST CONSTRUCTION JOINT EACH SIDE, TYP.

6. SEE TYPICAL DETAILS FOR SLAB REINFORCEMENT NOT SHOWN OR SPECIFICALLY NOTED ON PLAN AT ALL SLAB STEPS, CHANGES IN ELEVATION, EDGES, CORNERS, RE-ENTRANT CORNERS, RECESSES , DEPRESSIONS, RAMPS, ETC. ON SHEETS THRU , TYPICAL.

   Fe = 21.6 KLF

7. INDICATES 1/2" ⌀ FULLY ENCAPSULATED POST TENSION TENDONS PER PTI "SPECIFICATIONS FOR UNBONDED SINGLE STRAND TENDONS " AT SLAB MID-DEPTH. (TYP.) FOR SINGLE ENDED STRESSING LIVE AND DEAD ENDS MAY BE SWITCHED AT CONTRACTOR'S OPTION. ALL POST MENTIONED TENDONS ARE TO BE CHAIRED TO MAINTAIN CGS AT MID-DEPTH OF SLAB ALONG FULL LENGTH OF TENDON UNO ON PLAN, AND REFER TO FOR ADDITIONAL REQUIREMENTS, TYP.

   Fe=TYPICAL EFFECTIVE DESIGN FORCE IN EACH DIRECTION UNO

8. POST-TENSIONING SUPPLIER SHALL PROVIDE MINIMUM NUMBER OF TENDONS EQUIVALENT TO THE EFFECTIVE FORCE PER NOTE 7 CONSIDERING A MAXIMUM TENDON GROUP SIZE OF (4) TENDONS AND MAXIMUM HORIZONTAL SPACING OF 5'-0" BETWEEN CENTERLINE OF TENDON GROUPS, TYP.

9. CONSTRUCTION JOINTS AND OR POUR/STRIPS NOT SHOWN ON PLAN SHALL BE COORDINATED WITH AND APPROVED BY THE EOR PRIOR TO CONSTRUCTION

10. TABLE BELOW REPRESENTS MINIMUM **ADDITIONAL** PRESTRESSING FORCE REQUIRED TO OVERCOME SUBGRADE FRICTION LOSSES BASED ON TOTAL LENGTH OF TENDON(S) MEASURED ANCHOR TO ANCHOR IN A GIVEN DIRECTION WITH A COEFFICIENT OF FRiCTION OF 0.75..

TOTAL PT TENDON LENGTH	ADDITIONAL PRESTRESS FORCE
(FT)	(KLF)
25	1.4
50	2.8
75	4.2
100	5.6
125	7.0
150	8.4
175	9.8
200	11.3
225	12.7
250	14.1

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SHEET TITLE:
Foundation Plan - West Building - Level 01 - Area D

SHEET NUMBER:
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FORMING PLAN NOTES:

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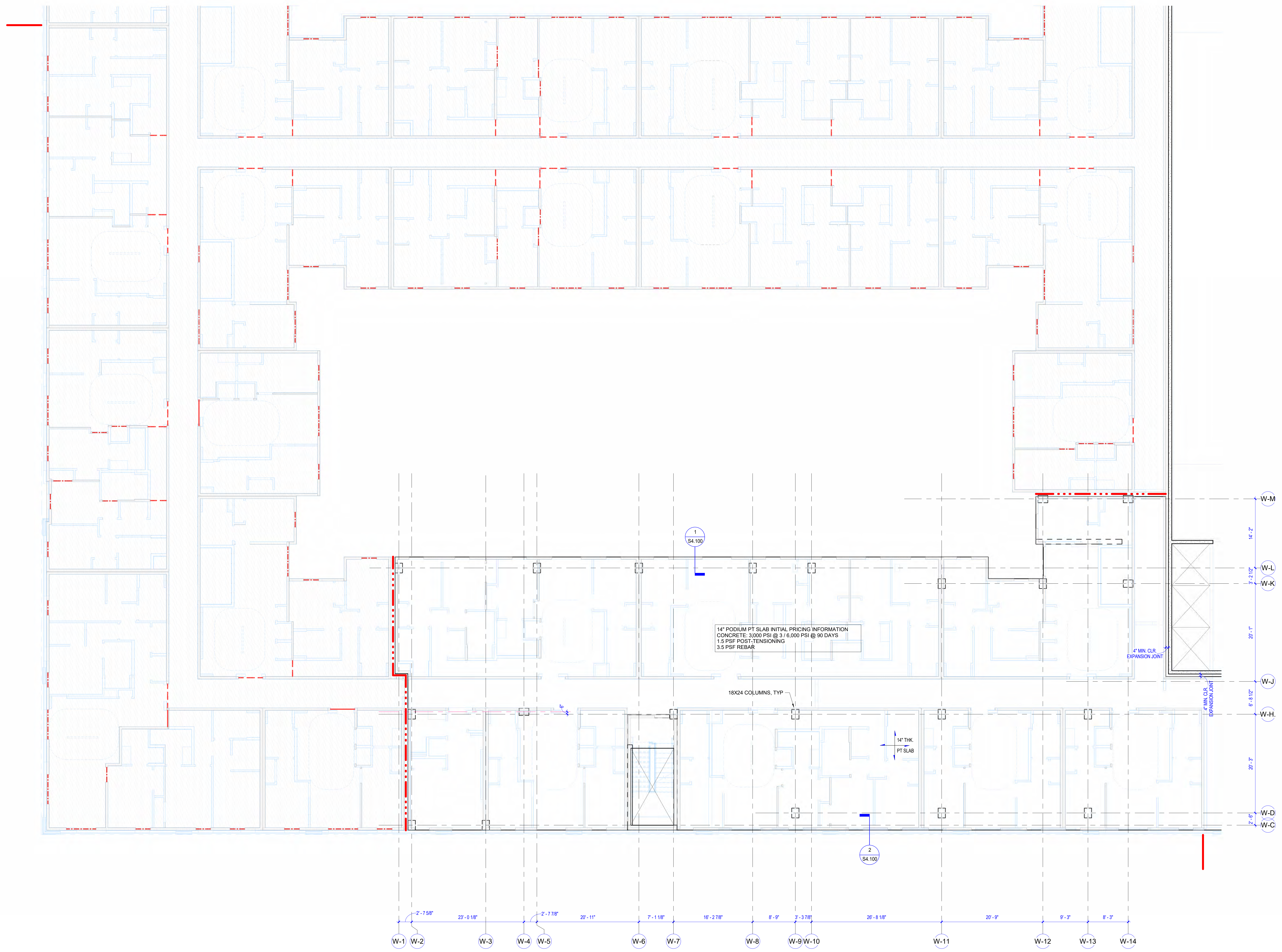
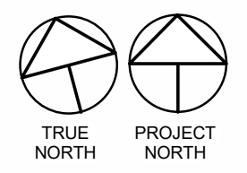
4. SLAB THICKNESS = SEE PLAN
5. CONCRETE STRENGTH = SEE TO GENERAL NOTES
6. **C-X** DENOTES CIP CONCRETE COLUMN. FOR COLUMN SCHEDULE SEE SHEET SERIES S5.100
   - **C-X.E** INDICATES SCHEDULED **C-X** COLUMN HAS ECCENTRIC STEP WITHIN HEIGHT OF COLUMN
   - **C-X.S** INDICATES SCHEDULED **C-X** COLUMN SLOPES WITHIN HEIGHT OF COLUMN
7. **SW-X** DENOTES CIP CONCRETE SHEAR WALL. FOR SHEAR WALL ELEVATIONS SEE SHEET SERIES S5.200
8. **SR-X** DENOTES STUD RAILS. FOR STUD RAIL SCHEDULE AND DETAILS SEE SHEET S5.130
9. **DP-X** DENOTES DROP PANEL. FOR DROP PANEL SCHEDULE AND DETAILS SEE SHEET S5.110
10. **PTB-X** DENOTES POST-TENSION CONCRETE BEAM.. FOR BEAM SCHEDULE AND DETAILS SEE SHEET /
11. **RCB-X** DENOTES REINFORCED CONCRETE BEAM. FOR BEAM SCHEDULE AND DETAILS SEE SHEET /
12. 0'-0" = xxx.xx' (VERIFY W/ CIVIL)





1 Forming Plan - West Building - Level 02 - Area B
1/8" = 1'-0"

VSM²
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VSMeg Structural Engineers, LLC
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Forming Plan - West Building - Level 02 - Area B

SHEET NUMBER:
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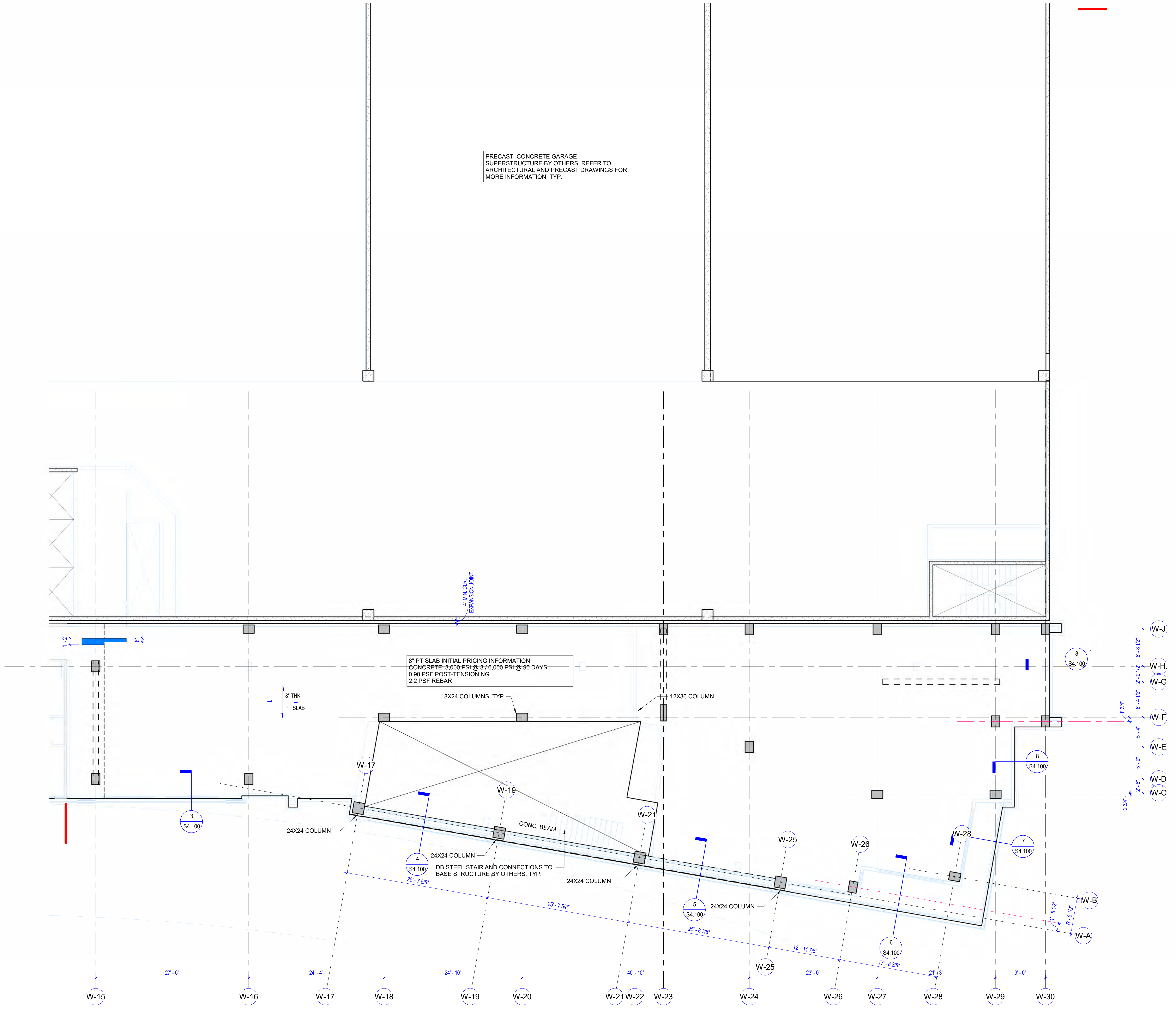

TRUE NORTH

PROJECT NORTH

1 Forming Plan - West Building - Level 02 - Area C
1/8" = 1'-0"

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SHEET TITLE:
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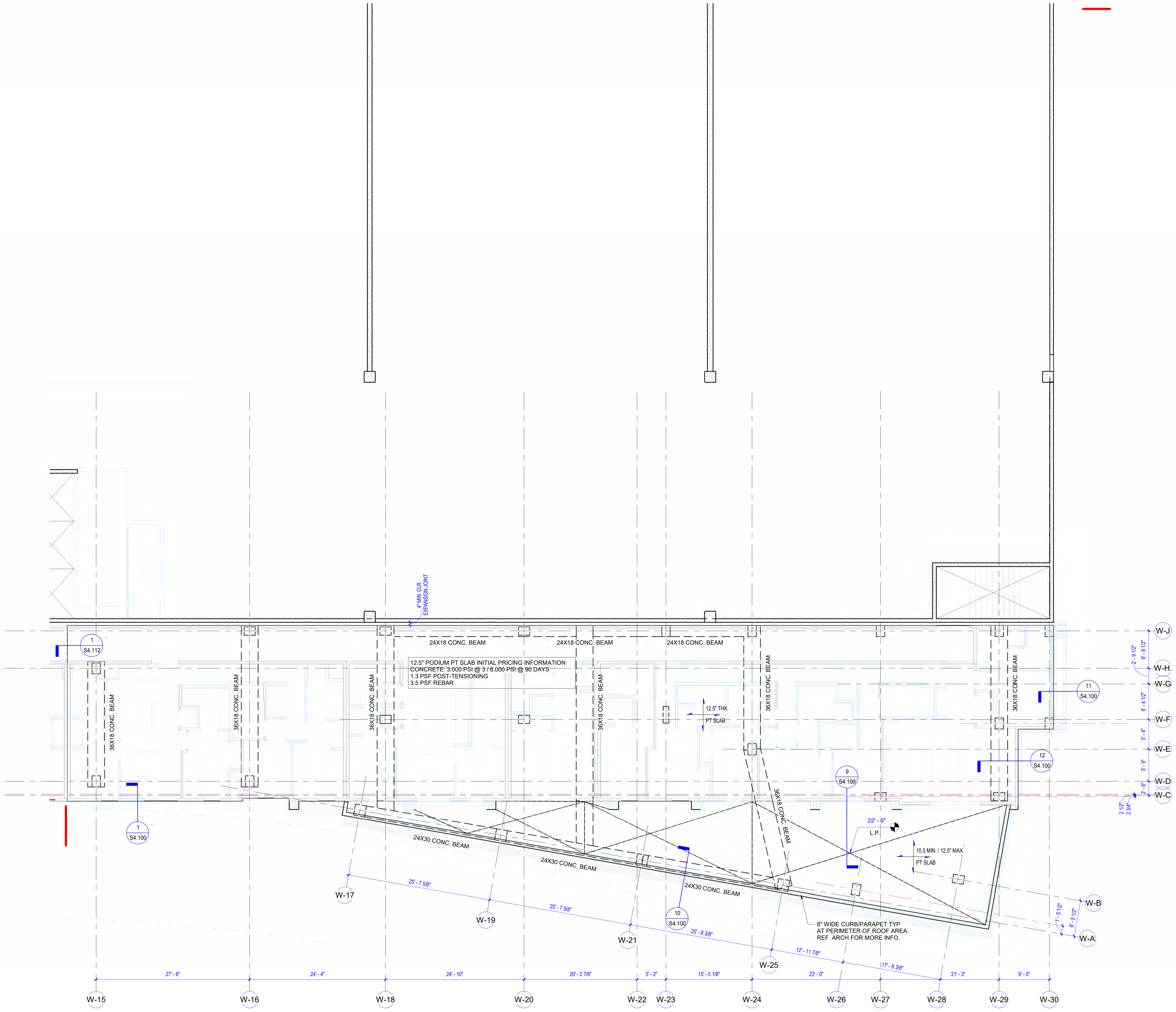

TRUE NORTH
PROJECT NORTH

1 Forming Plan - West Building - Level 3 - Area C
1/8" = 1'-0"

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SHEET TITLE:
Forming Plan - West Building - Level 03 - Area C

SHEET NUMBER:
S2.W103C

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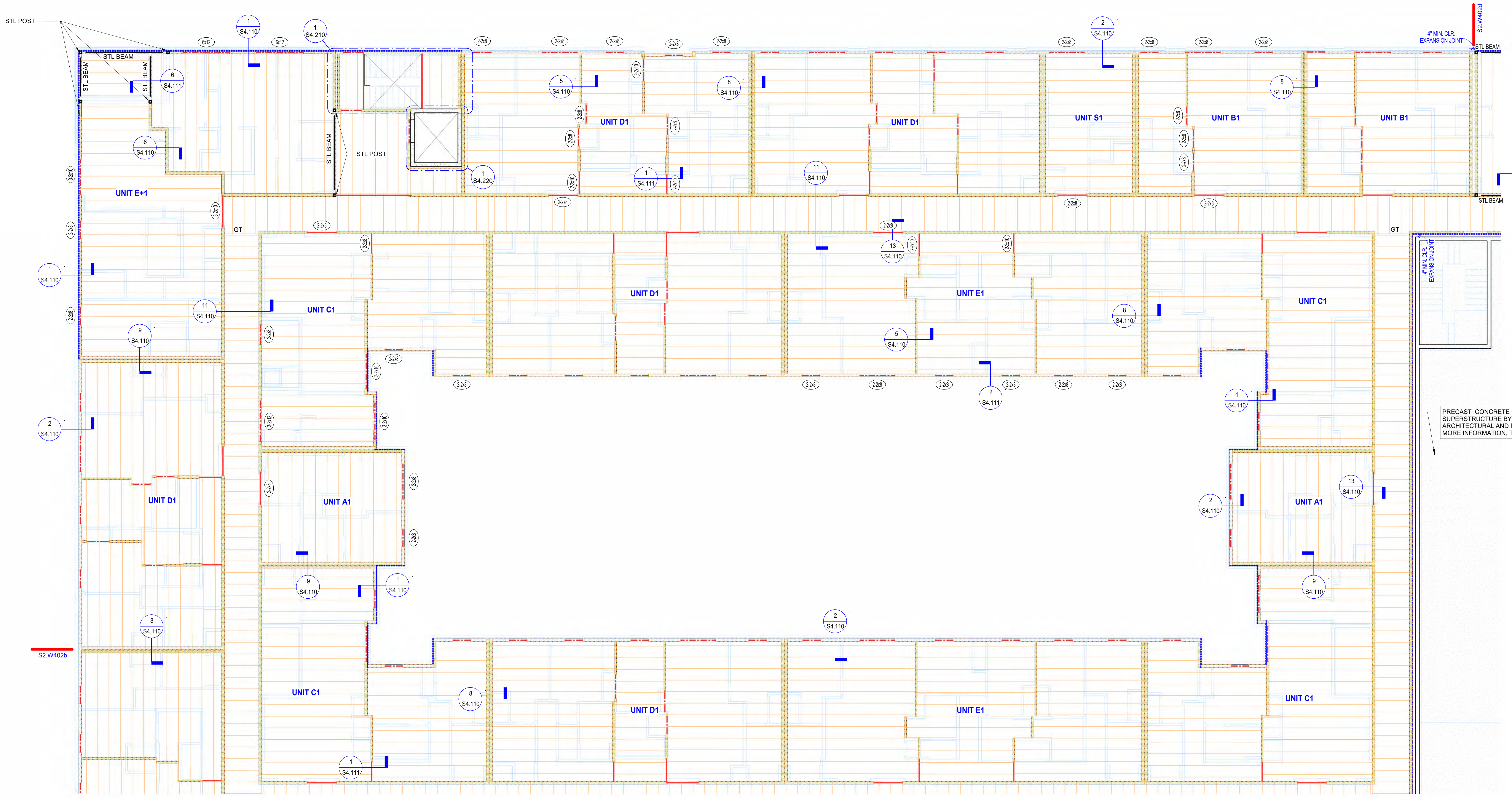

PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.

TRUE NORTH
PROJECT NORTH

1 Wood Framing Plan - West Building - Level 02 - Area A
1/8" = 1'-0"

WOOD FLOOR FRAMING NOTES:

1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS LAYOUT SHALL BE COORDINATED TO AVOID CONFLICTS WITH MEP SYSTEMS. TRUSSES SHALL NOT BE PLACED DIRECTLY ABOVE OR BELOW ANY PARALLEL 2X WALLS TO PREVENT CONFLICTS WITH MEP ROUTING.
3. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
4. HEADERS SHALL BE AT TOP OF WINDOW OPENINGS AND DOOR OPENINGS. FLUSH BEAMS SHALL BE IN FLOOR CAVITY (TYP. U.N.O.)
5. SEE GENERAL NOTE WD2.0 FOR MINIMUM QUANTITY OF STUDS AT BEARING ENDS OF ALL BEAMS, HEADERS (DROP BEAMS), FLUSH BEAMS, AND GIRDER TRUSSES. SEE POST SCHEDULE FOR STUD PACKS AND POST SPECIFIED ON PLAN.
6. FLOOR AND ROOF SHEATHING SHALL BE OSB TYPICAL EXCEPT AT CORRIDORS AND EXPOSED / EXTERIOR AREAS WHICH SHALL BE C-D EXPOSURE 1 PLYWOOD.
7. PROVIDE 3/8" MIN HORIZONTAL GAP BETWEEN EXTERIOR WALL SHEATHING PANELS TO ALLOW FOR SHRINKAGE. GAP TO BE PLACED AT THE LEVEL OF THE FLOOR SHEATHING AT EACH LEVEL (TYP)
8. TRUSSES SHALL RECEIVE DOUBLE TOP CHORD AT SPECIFIC DIAPHRAGM STRAPPING LOCATIONS PER PLAN AND NOTES. REFER TO WOOD DIAPHRAGM REINFORCING PLANS FOR MORE INFO.

WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:
1. ALL FRAMING EXPOSED TO EXTERIOR ELEMENTS TO BE PRESSURE TREATED (EXCLUDING FRT AND ENGINEERED LUMBER).

LEGEND:

= EXTERIOR NON-LOAD BEARING WOOD FRAMED STUD WALL
= LOAD BEARING WOOD FRAMED STUD WALL
= EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
= INDICATES 3HR FIRE SEPARATION WALL. REFER TO ARCHITECTURAL DETAILS FOR MORE INFORMATION.
= HEADER (DROP BEAM)
= FLUSH BEAM
= LEDGER
= TRUSS / JOIST
= GIRDER TRUSS
HSS / W = STEEL BEAM

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SHEET TITLE:
Wood Framing Plan - West Building - Level 02 - Area A

SHEET NUMBER:
S2.W402a

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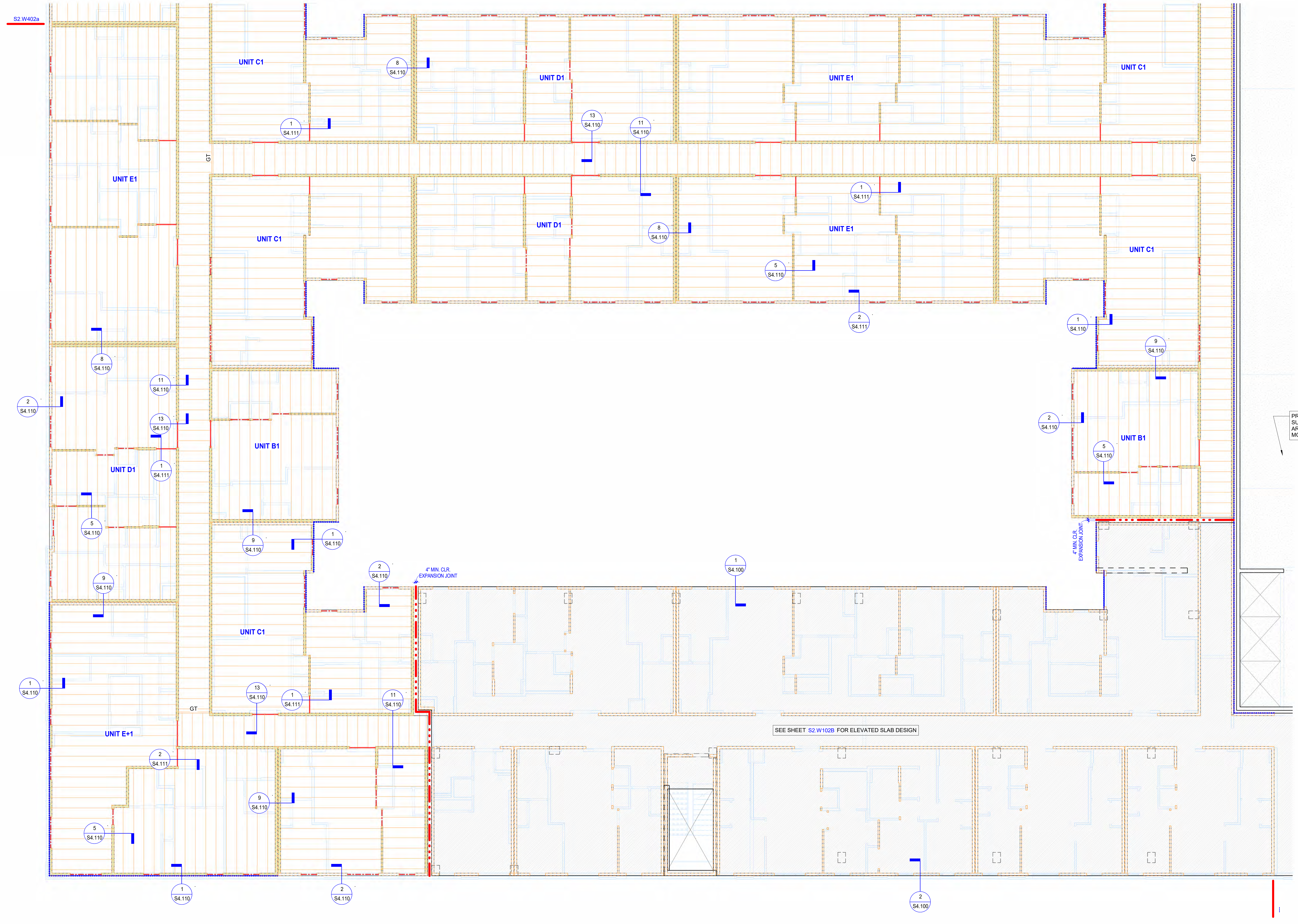

PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.

TRUE NORTH
PROJECT NORTH

1 Wood Framing Plan - West Building - Level 02 - Area B
1/8" = 1'-0"

WOOD FLOOR FRAMING NOTES:

1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS LAYOUT SHALL BE COORDINATED TO AVOID CONFLICTS WITH MEP SYSTEMS. TRUSSES SHALL NOT BE PLACED DIRECTLY ABOVE OR BELOW ANY PARALLEL 2X WALLS TO PREVENT CONFLICTS WITH MEP ROUTING.
3. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
4. HEADERS SHALL BE AT TOP OF WINDOW OPENINGS AND DOOR OPENINGS. FLUSH BEAMS SHALL BE IN FLOOR CAVITY (TYP. U.N.O.)
5. SEE GENERAL NOTE WD2.0 FOR MINIMUM QUANTITY OF STUDS AT BEARING ENDS OF ALL BEAMS, HEADERS (DROP BEAMS), FLUSH BEAMS, AND GIRDER TRUSSES. SEE POST SCHEDULE FOR STUD PACKS AND POST SPECIFIED ON PLAN.
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WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:

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LEGEND:

▭ = EXTERIOR NON-LOAD BEARING WOOD FRAMED STUD WALL
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─── = FLUSH BEAM
- - - = LEDGER
─── = TRUSS / JOIST
═══ = GIRDER TRUSS
HSS / W ━━ = STEEL BEAM

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Structural Engineers
VSMeg Structural Engineers, LLC
1080 Eldridge Parkway
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281-407-5733
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PROJECT # : 26154
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No.	Description	Date
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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Wood Framing Plan - West Building - Level 02 - Area B

SHEET NUMBER:
S2.W402b

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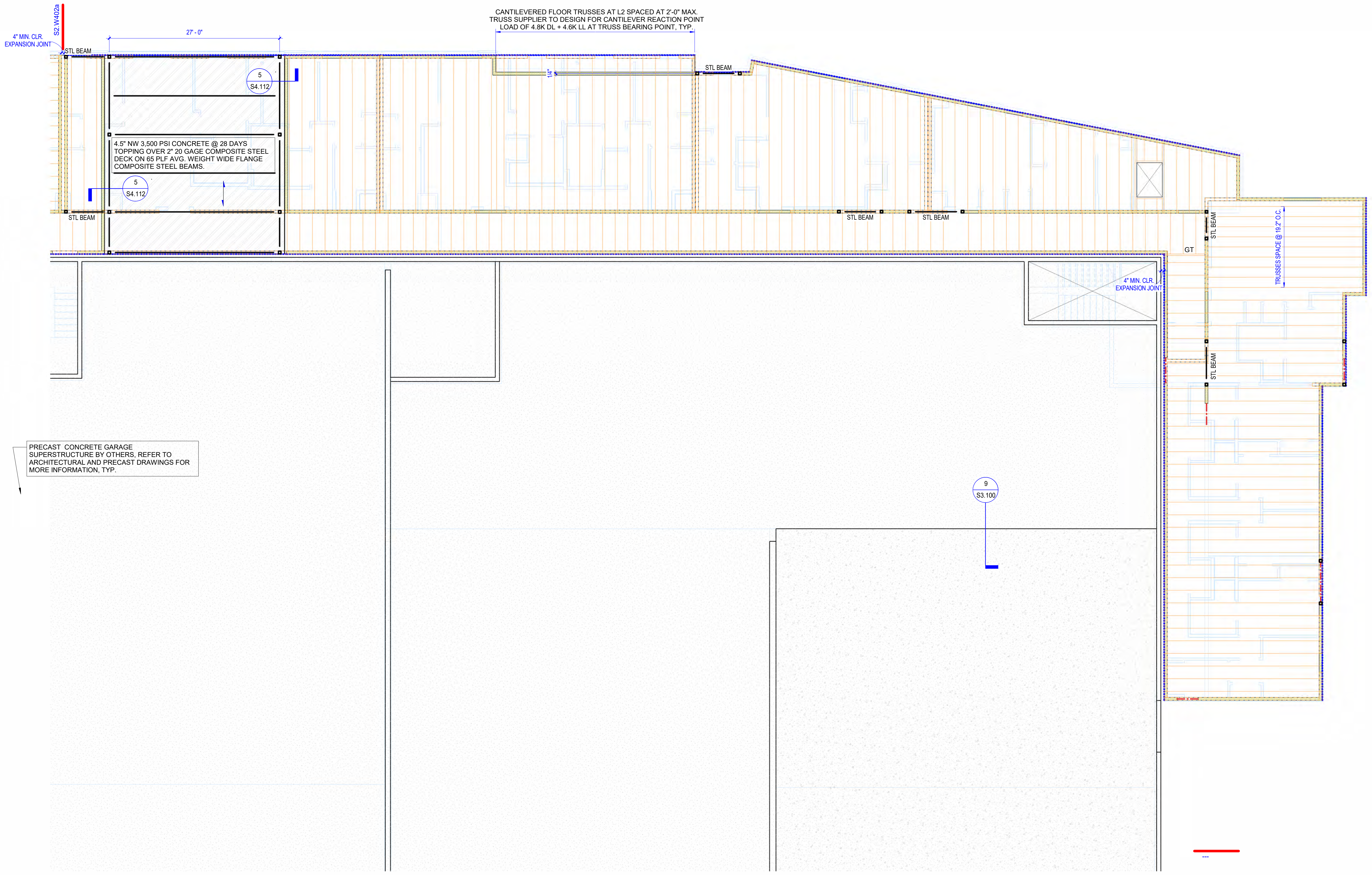
S2.W402a
4" MIN. CLR.
EXPANSION JOINT
STL BEAM
27' - 0"
CANTILEVERED FLOOR TRUSSES AT L2 SPACED AT 2'-0" MAX.
TRUSS SUPPLIER TO DESIGN FOR CANTILEVER REACTION POINT
LOAD OF 4.8K DL + 4.6K LL AT TRUSS BEARING POINT, TYP.
1/4"
STL BEAM
5
S4.112
4.5" NW 3,500 PSI CONCRETE @ 28 DAYS
TOPPING OVER 2" 20 GAGE COMPOSITE STEEL
DECK ON 65 PLF AVG. WEIGHT WIDE FLANGE
COMPOSITE STEEL BEAMS.
5
S4.112
STL BEAM
STL BEAM
STL BEAM
GT
STL BEAM
TRUSSES SPACE @ 19.2" O.C.
4" MIN. CLR.
EXPANSION JOINT
STL BEAM
PRECAST CONCRETE GARAGE
SUPERSTRUCTURE BY OTHERS, REFER TO
ARCHITECTURAL AND PRECAST DRAWINGS FOR
MORE INFORMATION, TYP.
9
S3.100

TRUE NORTH

PROJECT NORTH

1 Wood Framing Plan - West Building - Level 02 - Area D

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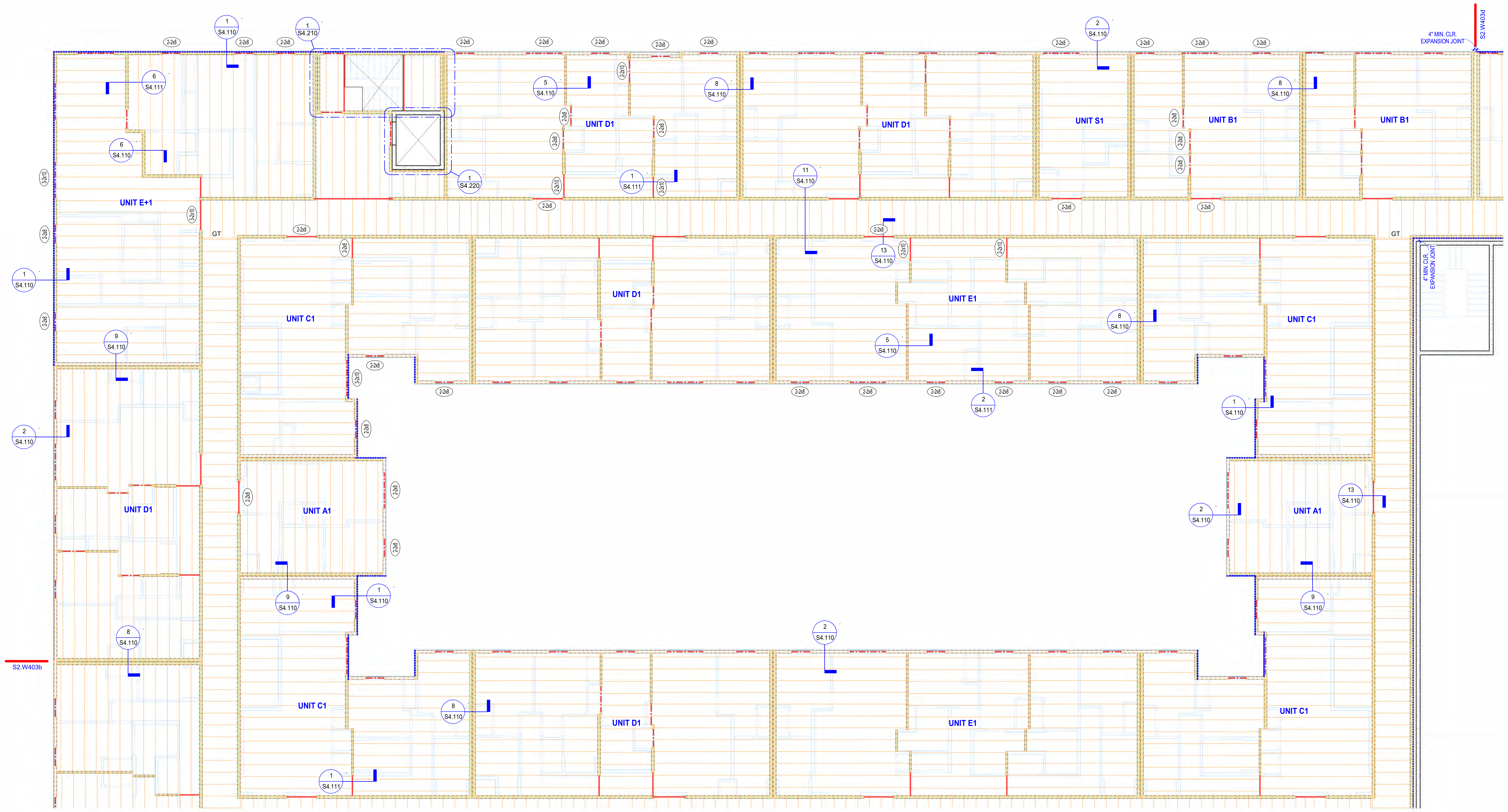

TRUE NORTH

PROJECT NORTH

① Wood Framing Plan - West Building - Level 03 - Area A
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Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
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SHEET TITLE:
Wood Framing Plan - West Building - Level 03 - Area A

SHEET NUMBER:
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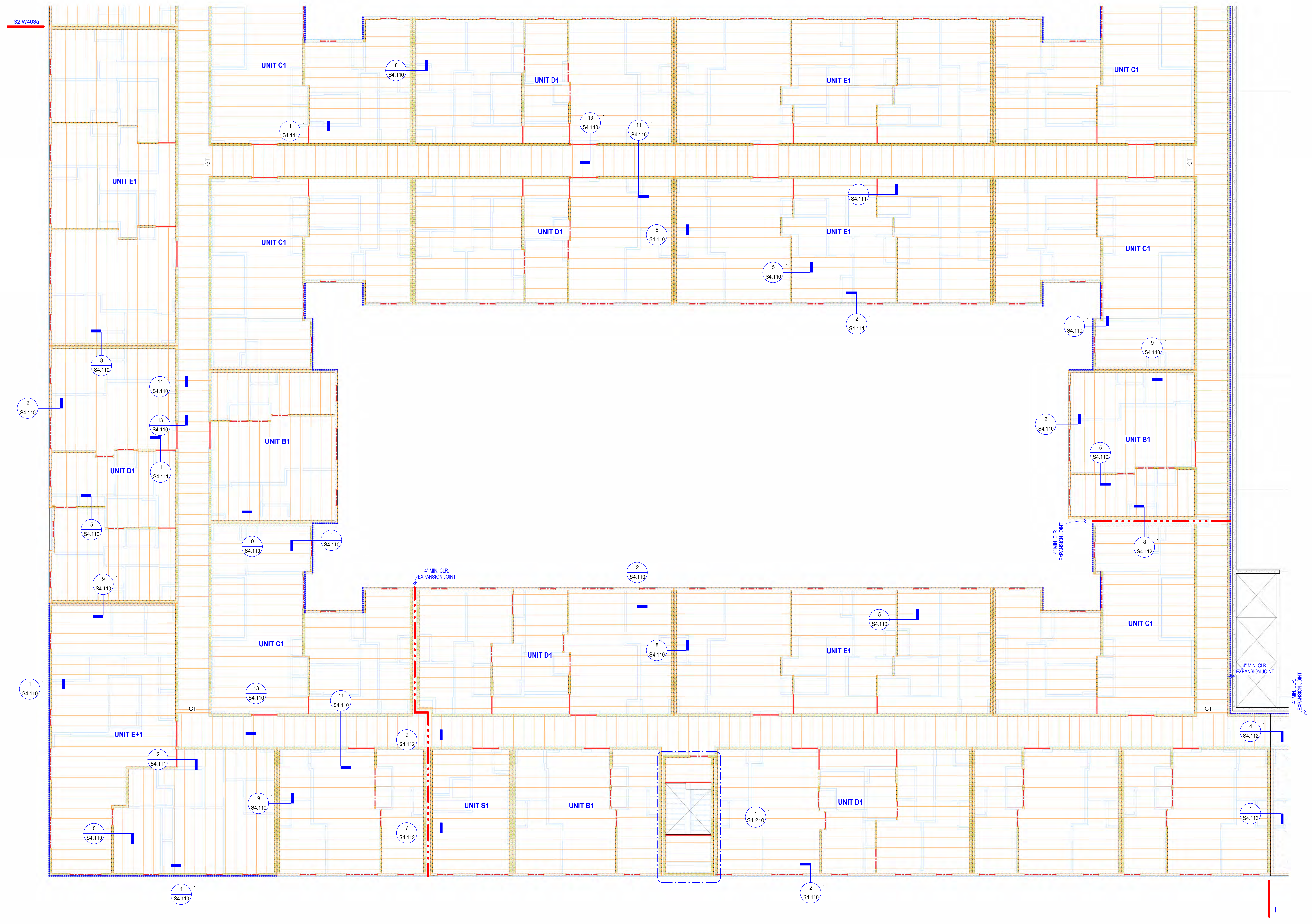

TRUE NORTH PROJECT NORTH

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SHEET TITLE:
Wood Framing Plan - West Building - Level 03 - Area B

SHEET NUMBER:
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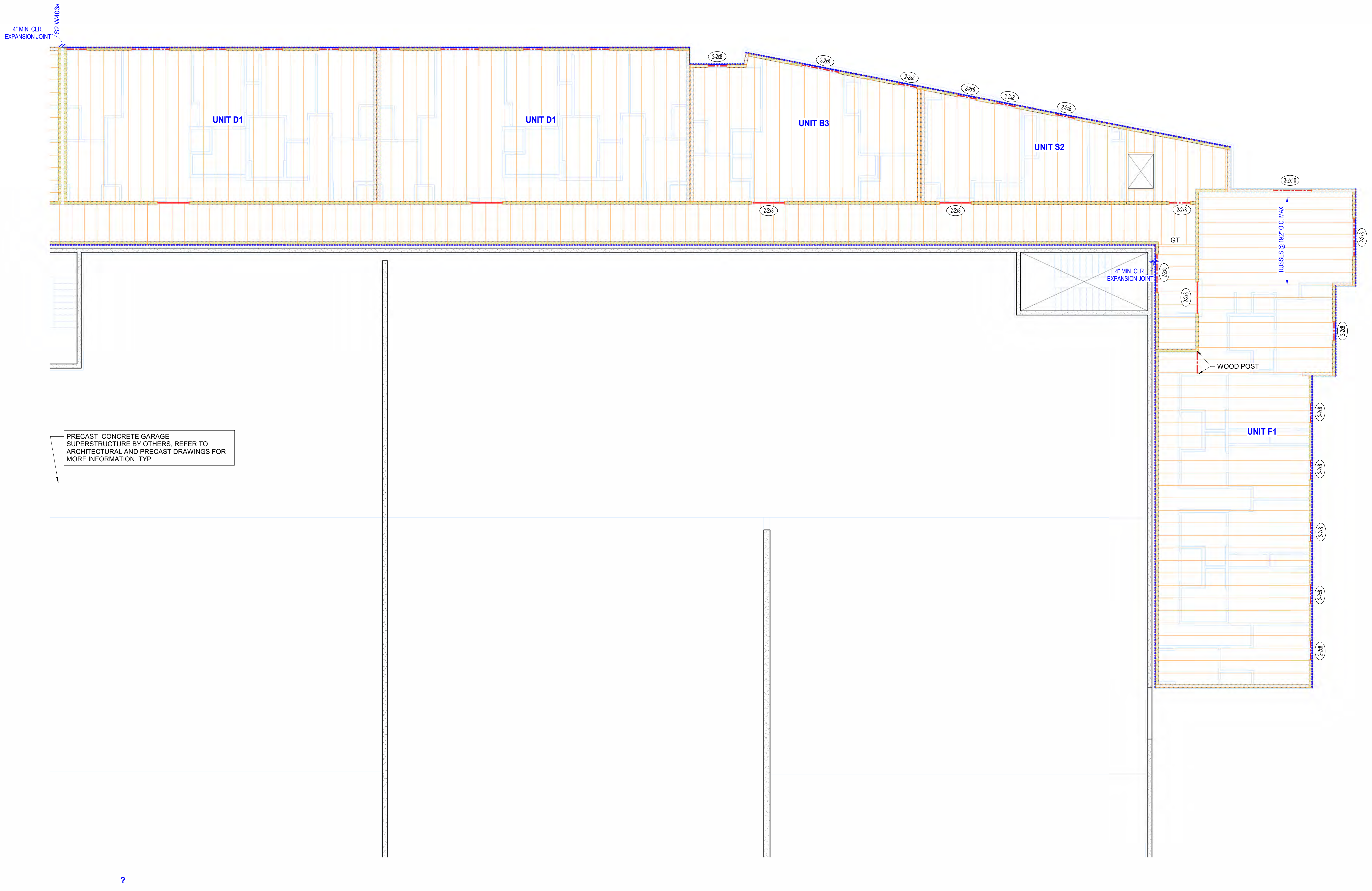

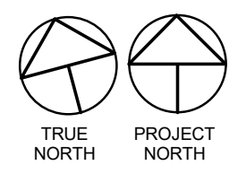

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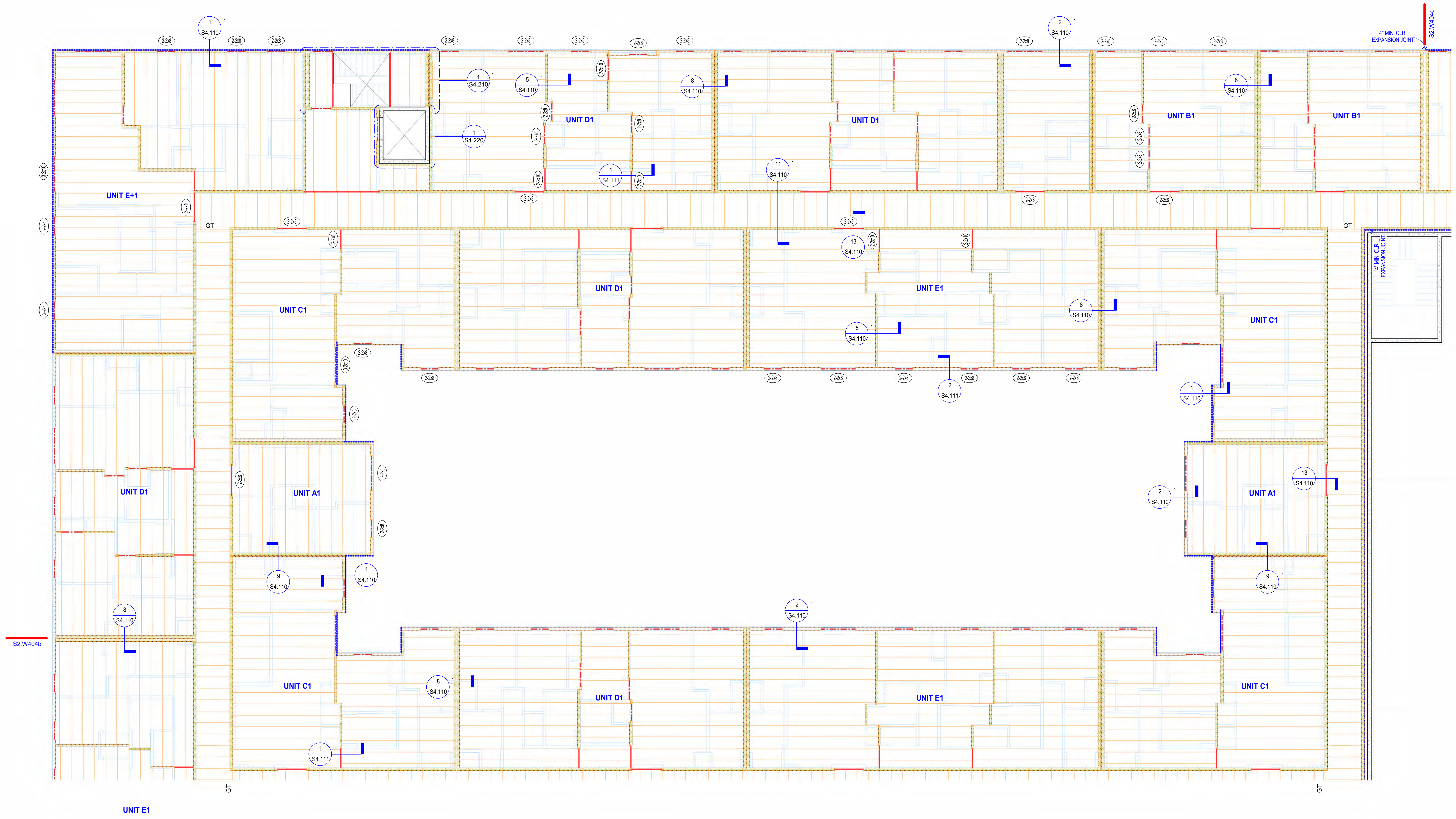

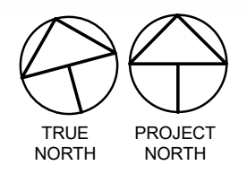

1 Wood Framing plan - West Building - Level 04 - Area A
1/8" = 1'-0"

WOOD FLOOR FRAMING NOTES:

1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS LAYOUT SHALL BE COORDINATED TO AVOID CONFLICTS WITH MEP SYSTEMS. TRUSSES SHALL NOT BE PLACED DIRECTLY ABOVE OR BELOW ANY PARALLEL 2X WALLS TO PREVENT CONFLICTS WITH MEP ROUTING.
3. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
4. HEADERS SHALL BE AT TOP OF WINDOW OPENINGS AND DOOR OPENINGS. FLUSH BEAMS SHALL BE IN FLOOR CAVITY (TYP. U.N.O.)
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WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
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━━━	= FLUSH BEAM
‐ ‐ ‐ ‐	= LEDGER
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═══	= GIRDER TRUSS
HSS / W ━━━	= STEEL BEAM

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Structural Engineers
VSMeg Structural Engineers, LLC
1080 Eldridge Parkway
Suite 350
Houston, TX 77077
713-407-5734
www.vsmeg.com

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

NILES BOLTON ASSOCIATES

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Atlanta, GA 30305

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www.nilesbolton.com

No.	Description	Date
	100% DD	09/30/2026

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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH
LV COLLECTIVE

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DOCUMENTS ARE ISSUED UNDER THE AUTHORITY OF:
- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Wood Framing Plan - West Building - Level 04 - Area A

SHEET NUMBER:
S2.W404a

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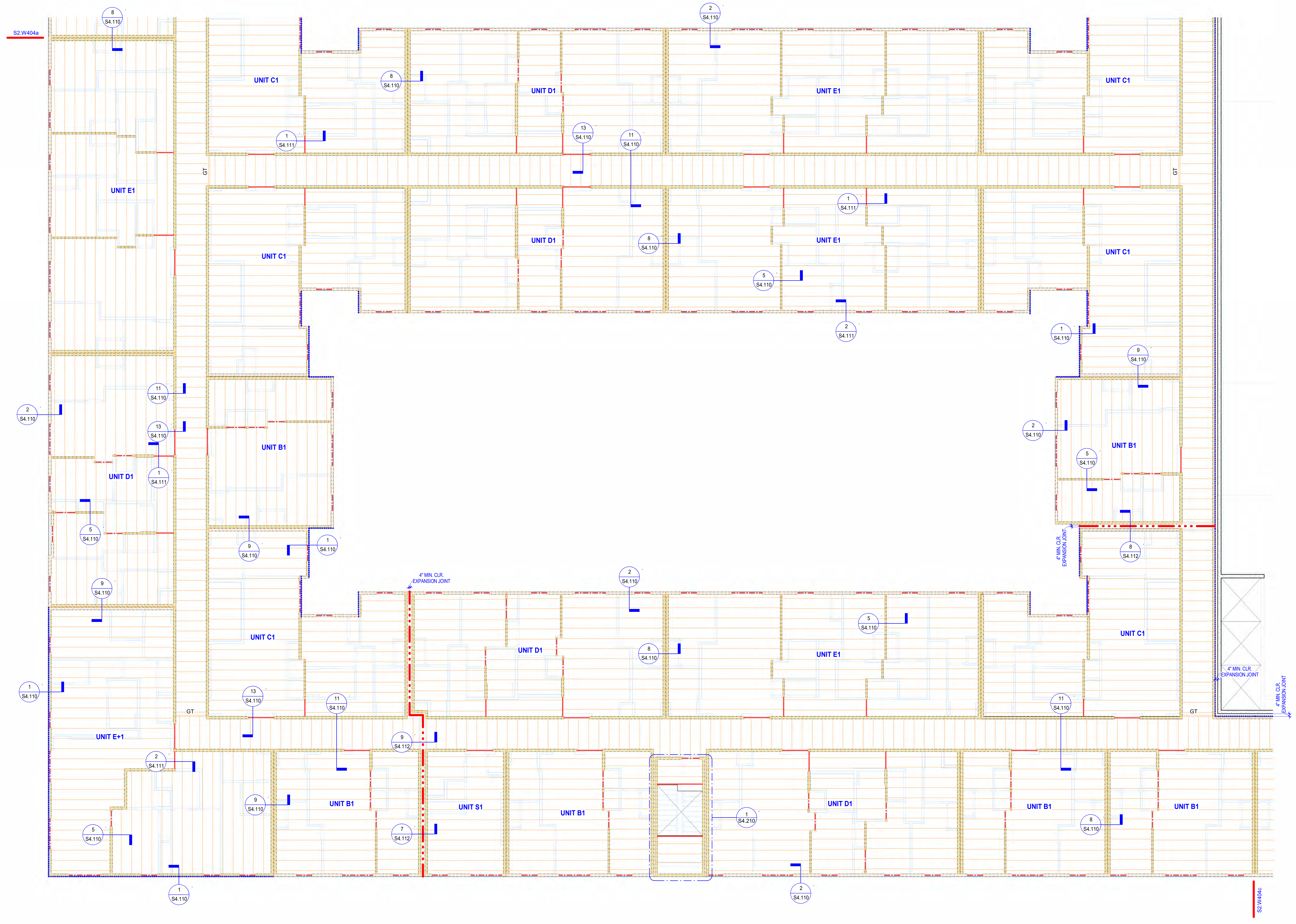

TRUE NORTH PROJECT NORTH

1 Wood Framing plan - West Building - Level 04 - Area B
1/8" = 1'-0"

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SHEET TITLE:
Wood Framing Plan - West Building - Level 04 - Area B

SHEET NUMBER:
S2.W404b

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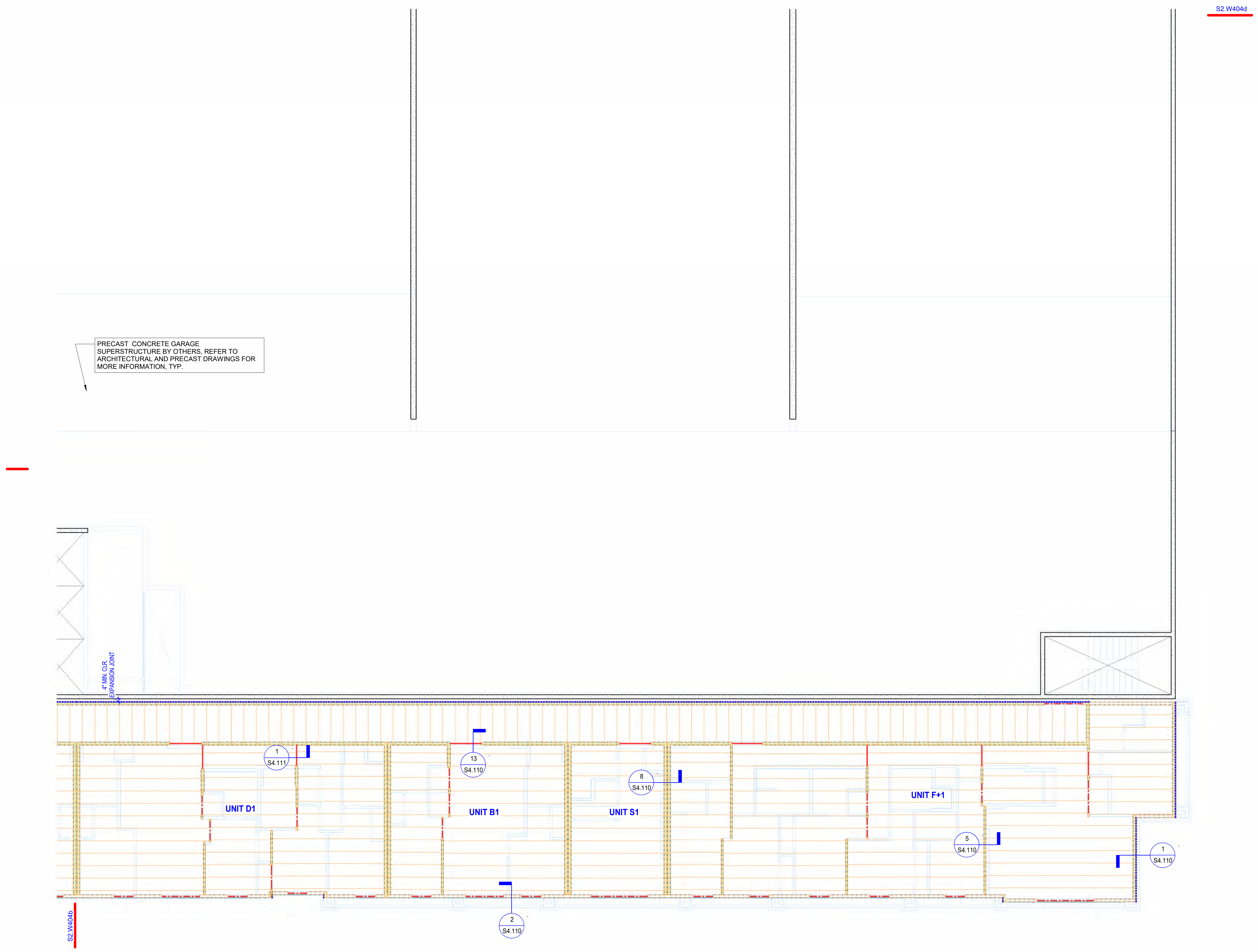
S2.W404d
PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.
4" MIN. CLR. EXPANSION JOINT
1 S4.111
13 S4.110
8 S4.110
UNIT D1
UNIT B1
UNIT S1
UNIT F+1
5 S4.110
1 S4.110
2 S4.110
S2.W404b

TRUE NORTH PROJECT NORTH

1 Wood Framing plan - West Building - Level 04 - Area C
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PROJECT # : 26154
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NILES BOLTON ASSOCIATES

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SHEET NUMBER:
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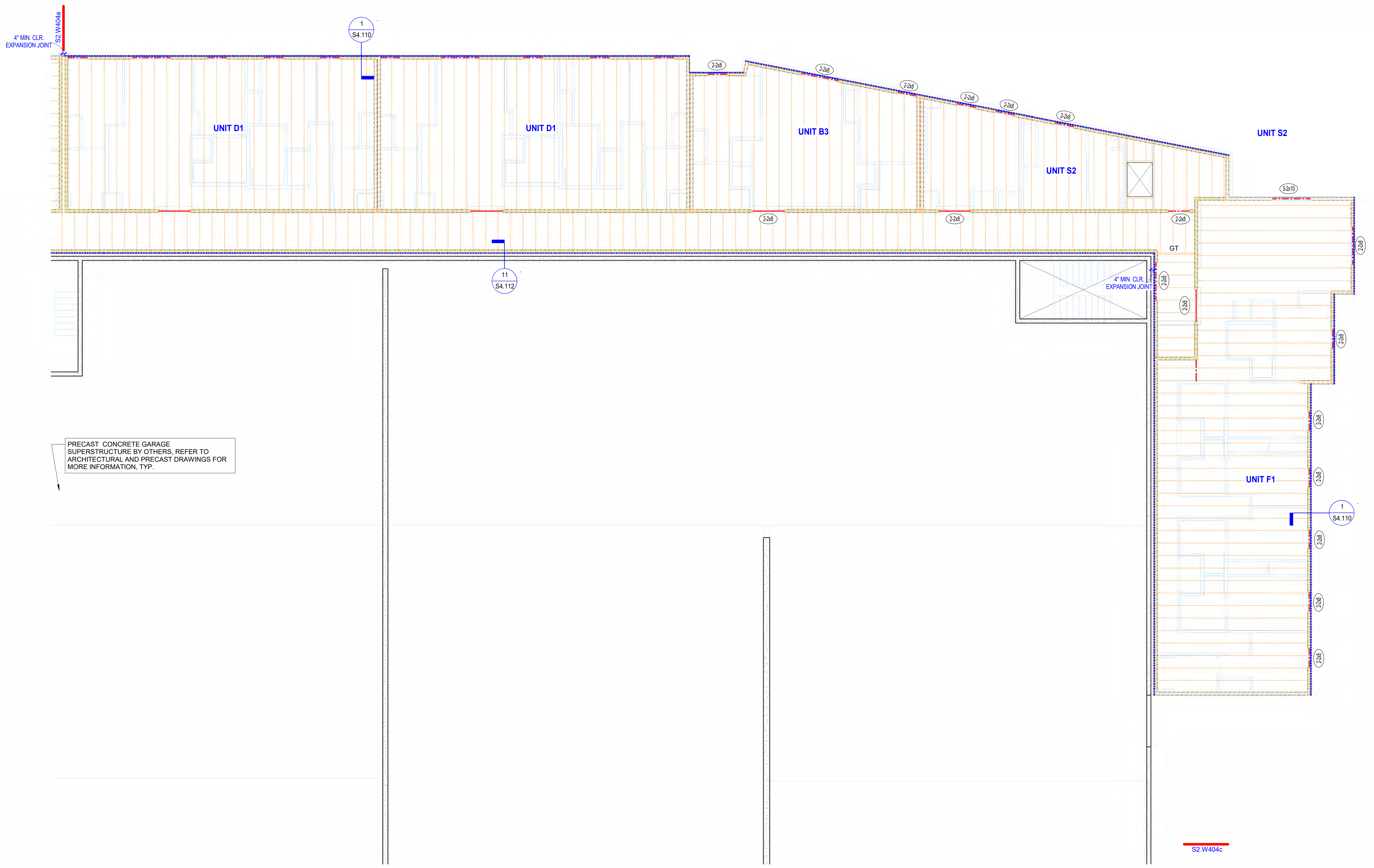

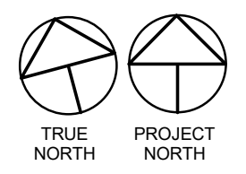

1 Wood Framing plan - West Building - Level 04 - Area D
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PROJECT # : 26154
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SHEET TITLE:
Wood Framing Plan - West Building - Level 04 - Area D

SHEET NUMBER:
S2.W404d

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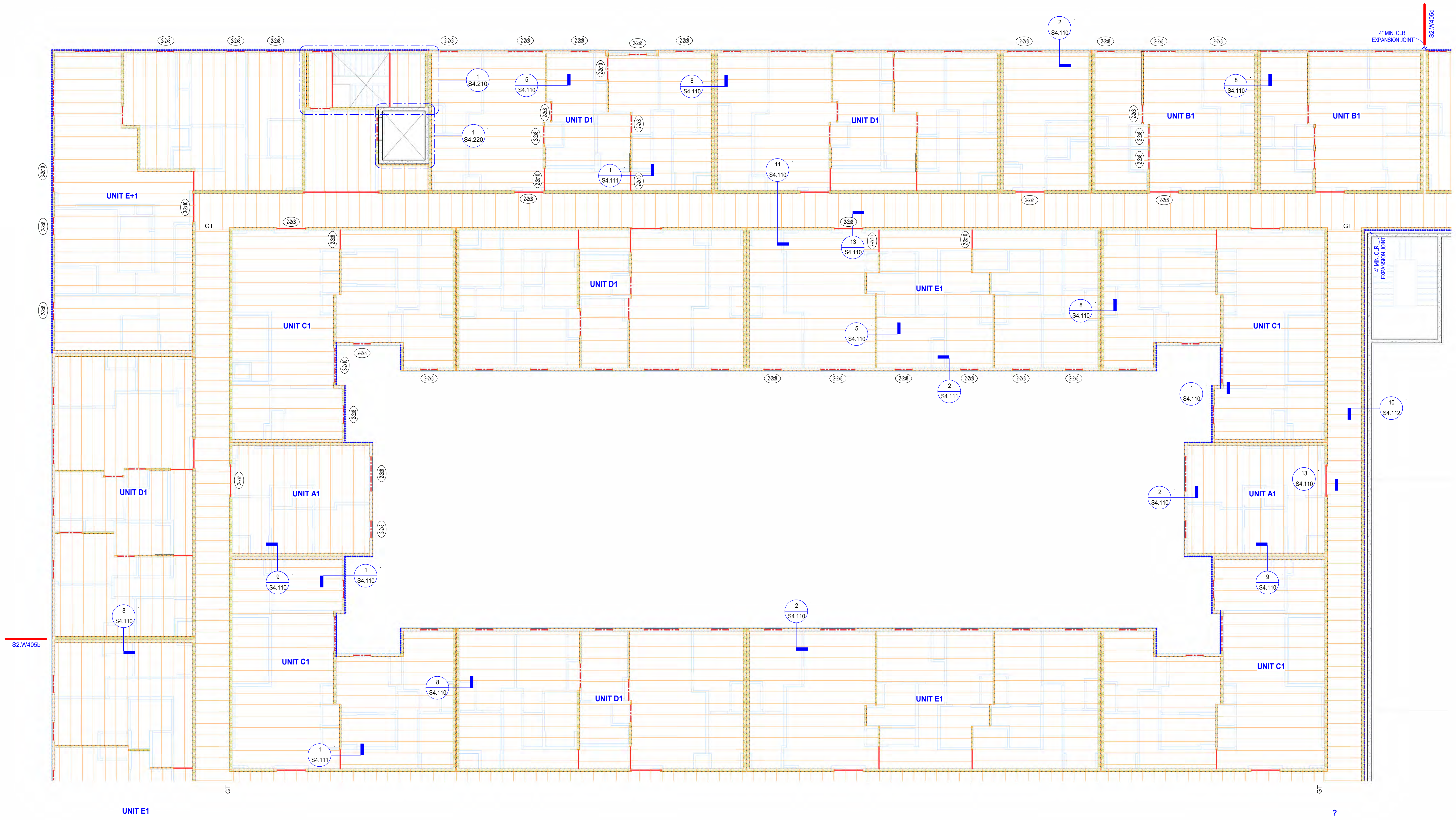

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TRUE NORTH
PROJECT NORTH

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SHEET NUMBER:
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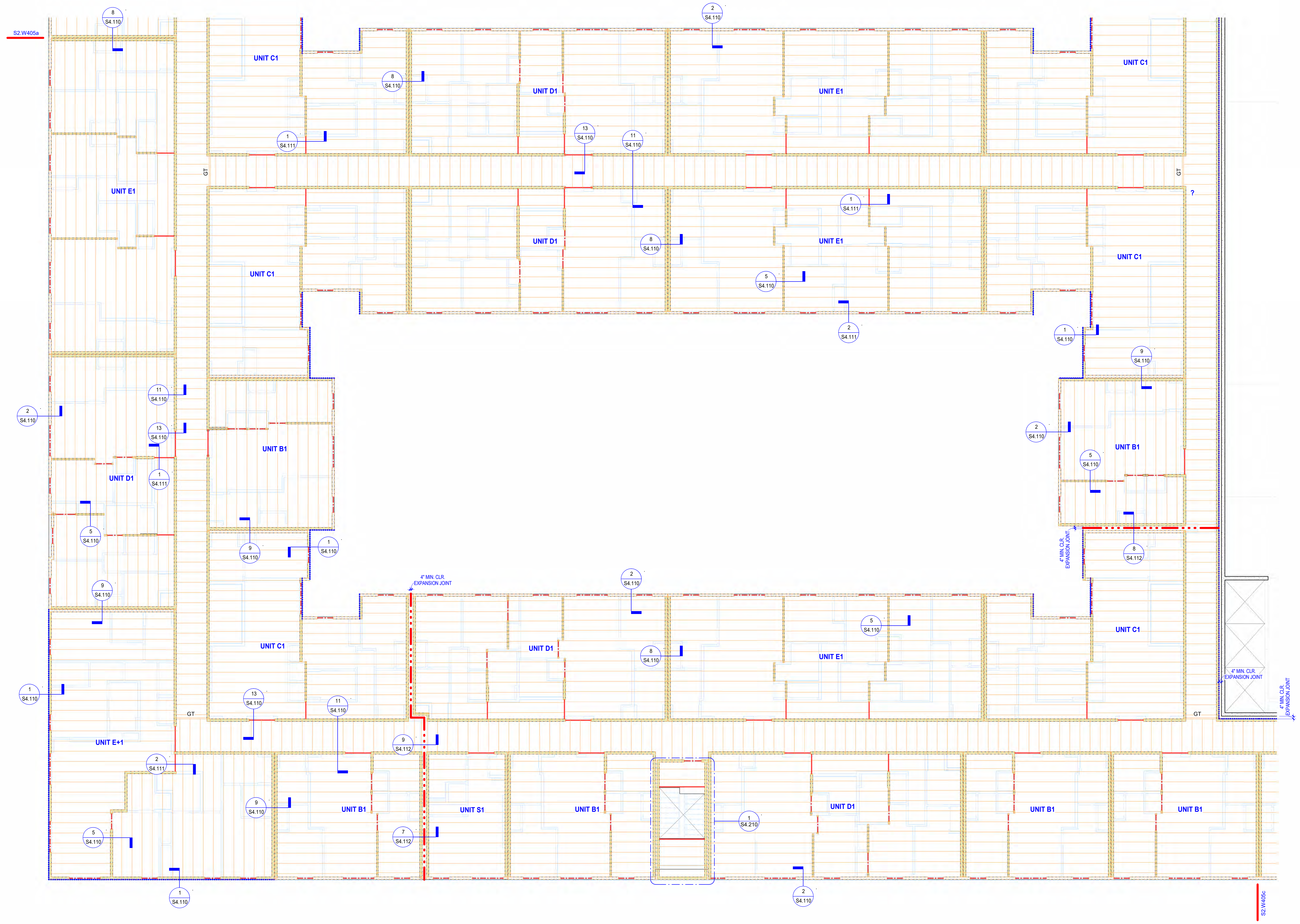

TRUE NORTH

PROJECT NORTH

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1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
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3. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
4. HEADERS SHALL BE AT TOP OF WINDOW OPENINGS AND DOOR OPENINGS. FLUSH BEAMS SHALL BE IN FLOOR CAVITY (TYP. U.N.O.)
5. SEE GENERAL NOTE WD2.0 FOR MINIMUM QUANTITY OF STUDS AT BEARING ENDS OF ALL BEAMS, HEADERS (DROP BEAMS), FLUSH BEAMS, AND GIRDER TRUSSES. SEE POST SCHEDULE FOR STUD PACKS AND POST SPECIFIED ON PLAN.
6. FLOOR AND ROOF SHEATHING SHALL BE OSB TYPICAL EXCEPT AT CORRIDORS AND EXPOSED / EXTERIOR AREAS WHICH SHALL BE C-D EXPOSURE 1 PLYWOOD.
7. PROVIDE 3/8" MIN HORIZONTAL GAP BETWEEN EXTERIOR WALL SHEATHING PANELS TO ALLOW FOR SHRINKAGE. GAP TO BE PLACED AT THE LEVEL OF THE FLOOR SHEATHING AT EACH LEVEL (TYP)
8. TRUSSES SHALL RECEIVE DOUBLE TOP CHORD AT SPECIFIC DIAPHRAGM STRAPPING LOCATIONS PER PLAN AND NOTES. REFER TO WOOD DIAPHRAGM REINFORCING PLANS FOR MORE INFO.

WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:

1. ALL FRAMING EXPOSED TO EXTERIOR ELEMENTS TO BE PRESSURE TREATED (EXCLUDING FRT AND ENGINEERED LUMBER).

LEGEND:

▭ = EXTERIOR NON-LOAD BEARING WOOD FRAMED STUD WALL

▭ = LOAD BEARING WOOD FRAMED STUD WALL

▭ = EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL

— • • — = INDICATES 3HR FIRE SEPARATION WALL. REFER TO ARCHITECTURAL DETAILS FOR MORE INFORMATION.

—·—·— = HEADER (DROP BEAM)

—— = FLUSH BEAM

– – – = LEDGER

—— = TRUSS / JOIST

═ = GIRDER TRUSS

HSS / W —— = STEEL BEAM

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No.	Description	Date
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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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- NO. 85617

SHEET TITLE:
Wood Framing Plan - West Building - Level 05 - Area B

SHEET NUMBER:
S2.W405b

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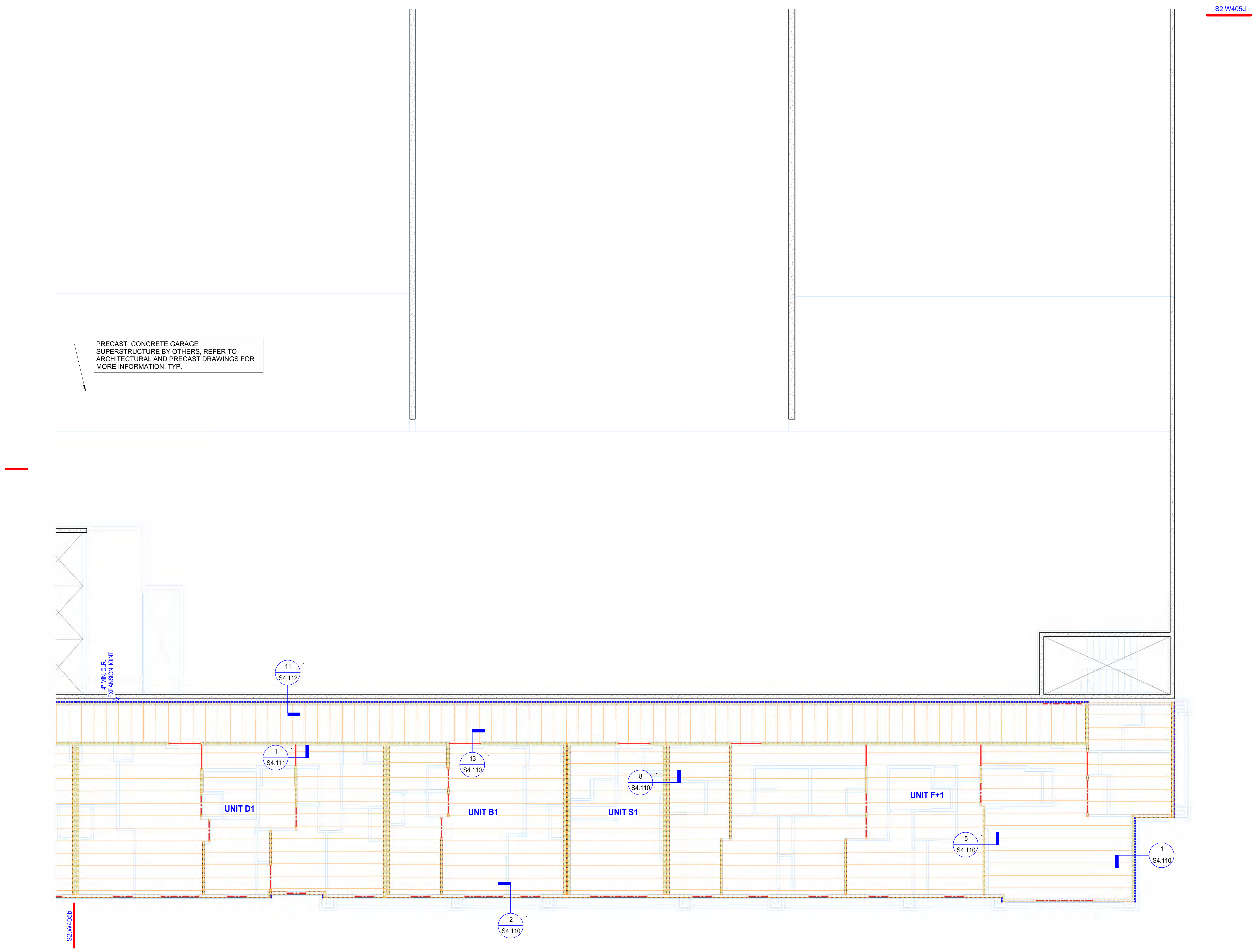
S2.W405d
PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.
4" MIN. CLR. EXPANSION JOINT
11 S4.112
1 S4.111
13 S4.110
8 S4.110
5 S4.110
1 S4.110
2 S4.110
UNIT D1
UNIT B1
UNIT S1
UNIT F+1
S2.W405b

TRUE NORTH
PROJECT NORTH

①Wood Framing Plan - West Building - Level 05 - Area C
1/8" = 1'-0"

WOOD FLOOR FRAMING NOTES:

1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
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5. SEE GENERAL NOTE WD2.0 FOR MINIMUM QUANTITY OF STUDS AT BEARING ENDS OF ALL BEAMS, HEADERS (DROP BEAMS), FLUSH BEAMS, AND GIRDER TRUSSES. SEE POST SCHEDULE FOR STUD PACKS AND POST SPECIFIED ON PLAN.
6. FLOOR AND ROOF SHEATHING SHALL BE OSB TYPICAL EXCEPT AT CORRIDORS AND EXPOSED / EXTERIOR AREAS WHICH SHALL BE C-D EXPOSURE 1 PLYWOOD.
7. PROVIDE 3/8" MIN HORIZONTAL GAP BETWEEN EXTERIOR WALL SHEATHING PANELS TO ALLOW FOR SHRINKAGE. GAP TO BE PLACED AT THE LEVEL OF THE FLOOR SHEATHING AT EACH LEVEL (TYP)
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WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
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ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

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- ▭ = LOAD BEARING WOOD FRAMED STUD WALL
- ▭ = EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
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- — · — · = HEADER (DROP BEAM)
- —— = FLUSH BEAM
- – – – = LEDGER
- —— = TRUSS / JOIST
- ═ = GIRDER TRUSS
- HSS / W —— = STEEL BEAM

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SHEET TITLE:
Wood Framing Plan - West Building - Level 05 - Area C

SHEET NUMBER:
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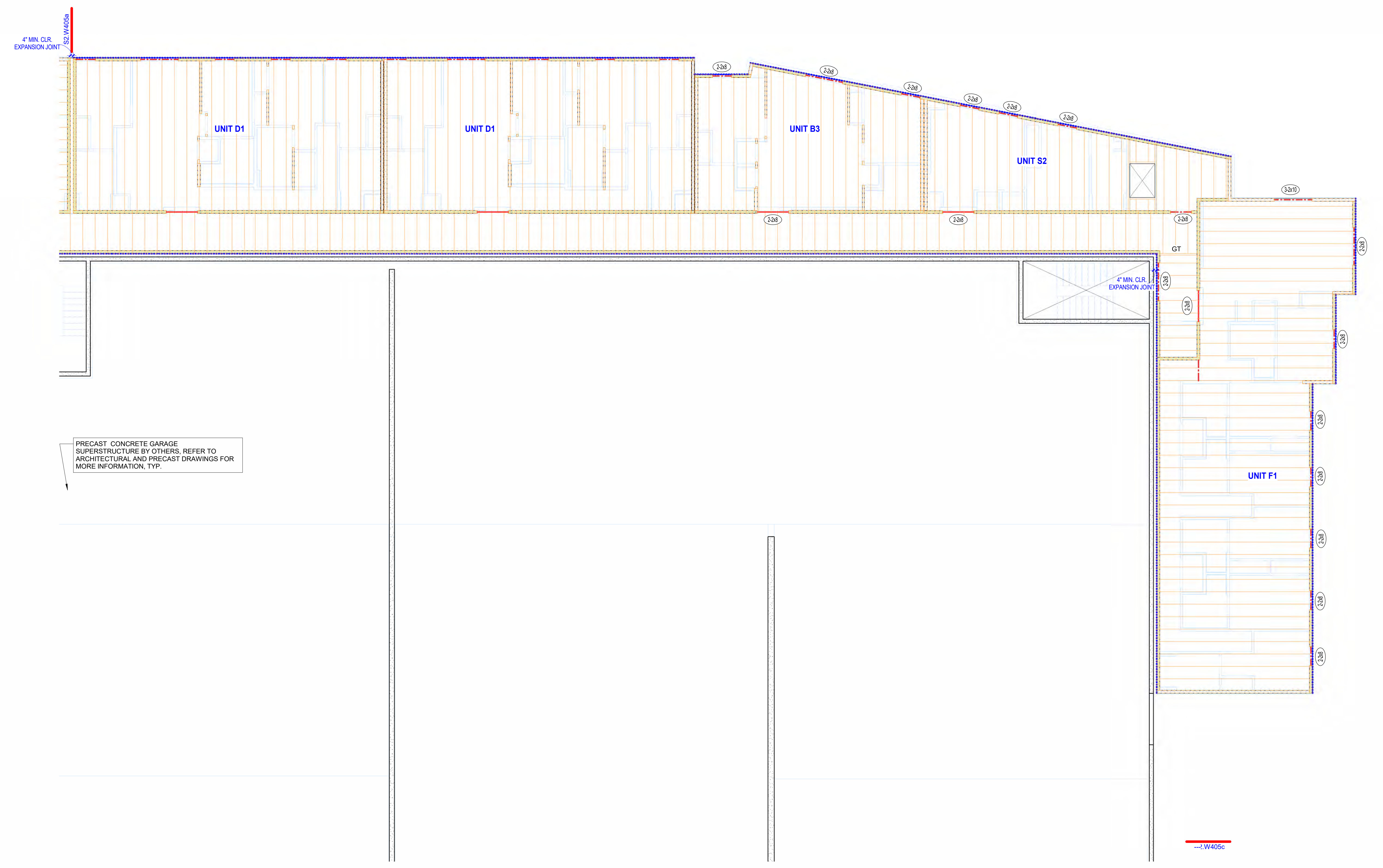
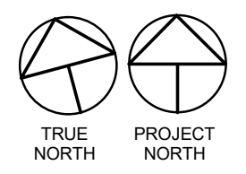

S2.W405a
4" MIN. CLR. EXPANSION JOINT
UNIT D1
UNIT D1
UNIT B3
UNIT S2
UNIT F1
2-2x8
3-2x10
GT
4" MIN. CLR. EXPANSION JOINT
PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.
S2.W405c



① Wood Framing Plan - West Building - Level 05 - Area D
1/8" = 1'-0"

WOOD FLOOR FRAMING NOTES:

1. UNIT FLOOR FRAMING TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5PSF OF TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
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WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
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STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:

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LEGEND:

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= LOAD BEARING WOOD FRAMED STUD WALL
= EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
= INDICATES 3HR FIRE SEPARATION WALL. REFER TO ARCHITECTURAL DETAILS FOR MORE INFORMATION.
= HEADER (DROP BEAM)
= FLUSH BEAM
= LEDGER
= TRUSS / JOIST
= GIRDER TRUSS
HSS / W = STEEL BEAM

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SHEET TITLE:
Wood Framing Plan - West Building - Level 05 - Area D

SHEET NUMBER:
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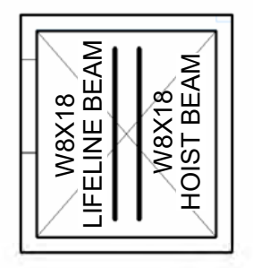
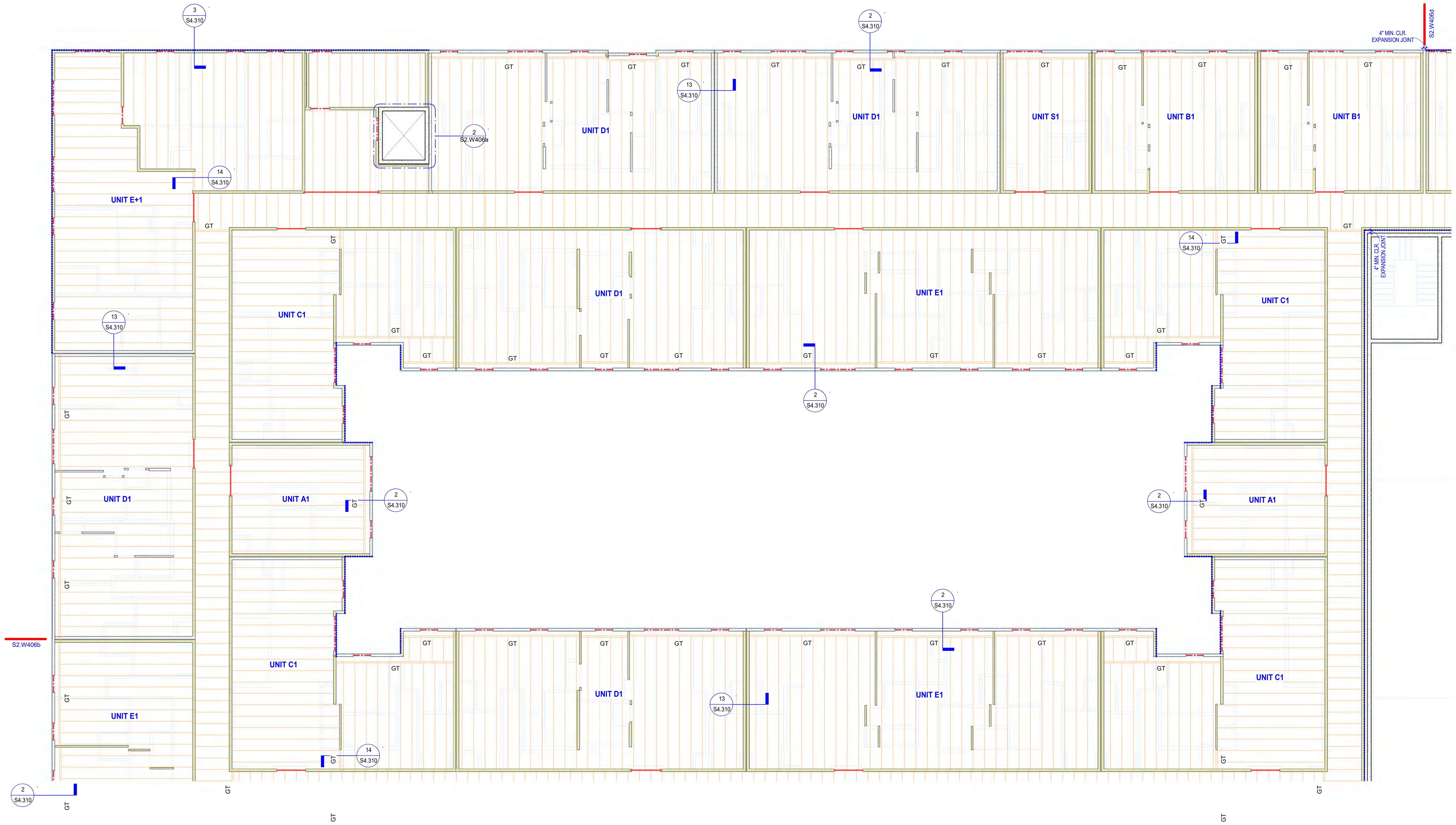
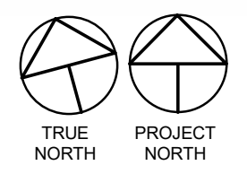


② Wood Framing Plan - Elevator Roof - West Building
1/8" = 1'-0"





① Roof Framing Plan - West Building - Level 06 - Area A
1/8" = 1'-0"

WOOD ROOF FRAMING NOTES:

1. ROOF FRAMING TO BE 24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES @ 24" OC MAX UNO. TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5 PSF OF THE TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS DESIGNER TO REF MEP, ARCH, EBM (EXTERIOR BUILDING MAINTENANCE) EQUIPMENT FOR ADDITIONAL ROOF LOADS AND EXACT LOCATION. IN THE ABSENSE OF SPECIFIC LOADING CRITERIA, TRUSS DESIGNER SHALL INCLUDE A DESIGN ALLOWANCE AS FOLLOWS:
   - AHU'S, BOILERS AND WATER TANKS = 100PSF DL
   - PHOTO-VOLTAIC (SOLAR) ARRAYS = 30PSF DL
   - UNSCHEDULED MAINTENANCE TIE-BACKS = 3,100 LBS ASD IMPACT LOAD APPLIED IN ANY DIRECTION
3. CONTRACTOR TO VERIFY ALL DIMENSIONS, ELEVATIONS, DEPRESSIONS, STEPS, FINISHES, PLATE HEIGHTS AND SPECIFIC/OVERALL BUILDING GEOMETRY WITH THE ARCHITECTURAL DRAWINGS AND NOTIFY ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION START
4. SEE WOOD FRAMING SPECIFICATIONS FOR FLOOR / ROOF SHEATHING AND MINIMUM LUMBER GRADE / SPECIES REQUIREMENTS.
5. SECTIONS CUT ON PLAN SHALL BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS. REFER TO TYPICAL DETAILS/SECTIONS FOR DETAILS / SECTIONS NOT SPECIFICALLY SHOWN OR CUT ON PLAN.
6. SEE ROOF UPLIFT SECTIONS FOR FASTENING OF ROOF LEVEL HEADERS, BEAMS AND GIRDER TRUSSES.
7. AT ROOF UPLIFT ROD LOCATIONS, NUTS SHALL BE RE-TIGHTENED AS REQUIRED PRIOR TO INTERIOR WALL SHEATHING INSTALLATION TO ACCOMMODATE WOOD SHRINKAGE
8. AT LOCATIONS WHERE ROOF UPLIFT RODS COINCIDE WITH SHEAR WALL END ANCHORAGE RODS, ONLY THE SHEAR WALL ANCHORAGE ROD NEED BE INSTALLED. ALL OTHER FASTNENING HARDWARE SHOWN ON THE ROOF UPLIFT DETAILS SHALL BE INSTALLED AS INDICATED.
9. TRUSSES SHALL RECEIVE DOUBLE TOP CHORD AT SPECIFIC DIAPHRAGM STRAPPING LOCATIONS PER PLAN AND NOTES. REFER TO WOOD DIAPHRAGM REINFORCING PLANS FOR MORE INFO.
10. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
11. SEE THIS SHEET FOR POST/HANGER SCHEDULE. ALL BEAM/HEADER BEARING POINTS NOT NOTED ON PLAN SHALL BE SUPPORTED PER NOTE 4 OF THE POST SCHEDULE.
12. ALL HEADERS BEARING ROOF TRUSSES SHALL BE LOCATED DIRECTLY BELOW THE DOUBLE TOP PLATE UNO. SEE HEADER UPLIFT DETAILS.
13. PROVIDE 3/8" MIN HORIZONTAL GAP BETWEEN EXTERIOR WALL SHEATHING PANELS TO ALLOW FOR SHRINKAGE. GAP TO BE PLACED AT THE LEVEL OF THE FLOOR SHEATHING AT EACH LEVEL, TYP.
14. ROOF TIE-BACK AND EBM ANCHORS, WHERE APPLICABLE, SHALL BE ANCHORD TO ROOF TRUSSES PER TYPICAL DETAILS. TRUSS SUPPLIER SHALL LOCATE ALL ANCHORS PER THE EBM/FACADE ACCESS CONSULTANT DRAWINGS IN COORDINATION WITH THE ARCHITECTURAL DRAWINGS. TRUSS SUPPLIER SHALL DESIGN THE SUPPORTING TRUSSES TO MEET THE LOAD DEMAND(S) SHOWN ON THE TYPICAL DETAILS.
15. AT LOCATIONS WITH PARAPET FRAMING INTENDED TO SUPPORT EBM LOADS SHALL BE REINFORCED PER TYPICAL DETAILS. ALL OTHER LOCATIONS HAVE BEEN DESIGNED TO MEET WIND/SEISMIC LATERAL FORCES ONLY.
16. SEE TYPICAL DETAILS FOR MECHANICAL AND HVAC PLATFORM FRAMING.
17. REFER TO WOOD SHRINKAGE NOTES IN GENERAL NOTES FOR ESTIMATED BUILDING MOVEMENT DUE TO WOOD SHRINKAGE. CONTRACTOR SHALL COORDINATE ESTIMATED VERTICAL MOVEMENT WITH OTHER TRADES AS NEEDED. IN THE ABSENCE OF ESTIMATED BUILDING VERTICAL MOVEMENT, ASSUME 1/4" PER FLOOR.
18. ROOF TRUSSES SHALL BE DESIGNED FOR DRAG LOADS SHOWN ON PLAN. IN THE ABSENSE OF D.F. (DRAG FORCES), ALL TRUSSES PARALLEL TO SHEARWALLS SHALL BE DESIGNED TO RESIST MINIMUM 400 PLF DRAG FORCE (TYPICAL).

WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
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SHEET TITLE:
Wood Framing Plan - West Building - Roof - Area A

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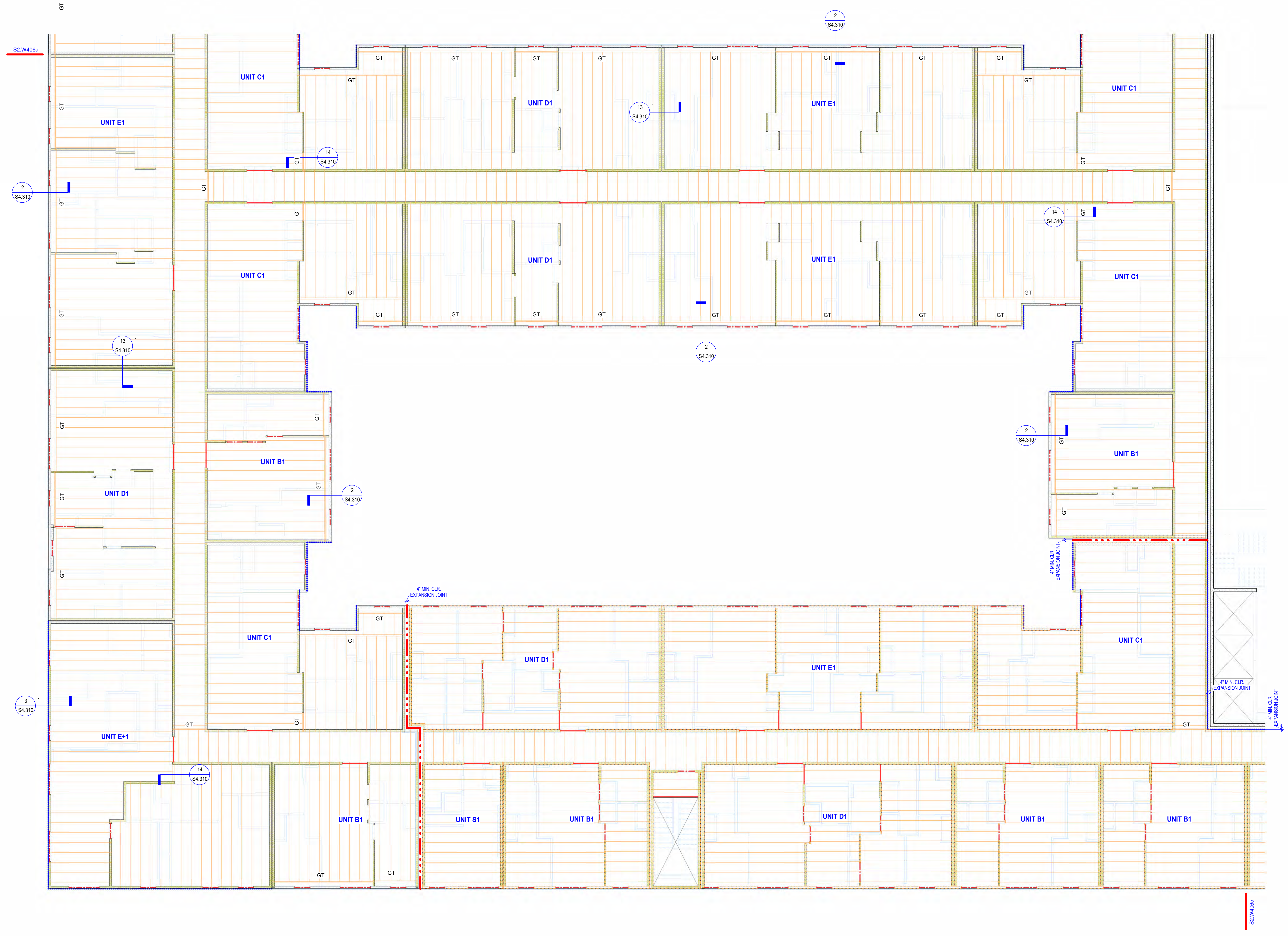

TRUE NORTH
PROJECT NORTH

1 Roof Framing Plan - West Building - Level 06 - Area B
1/8" = 1'-0"

WOOD ROOF FRAMING NOTES:

1. ROOF FRAMING TO BE 24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES @ 24" OC MAX UNO. TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5 PSF OF THE TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
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WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:
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= FLUSH BEAM
= LEDGER
= TRUSS / JOIST
= GIRDER TRUSS
HSS / W = STEEL BEAM

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMeg Structural Engineers, LLC
1080 Eldridge Parkway
Suite 350
Houston, TX 77077
781-407-5734
www.vsmeg.com

PROJECT # : 26154
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No.	Description	Date
	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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Wood Framing Plan - West Building - Level 06/ Roof - Area B

SHEET NUMBER:
S2.W406b

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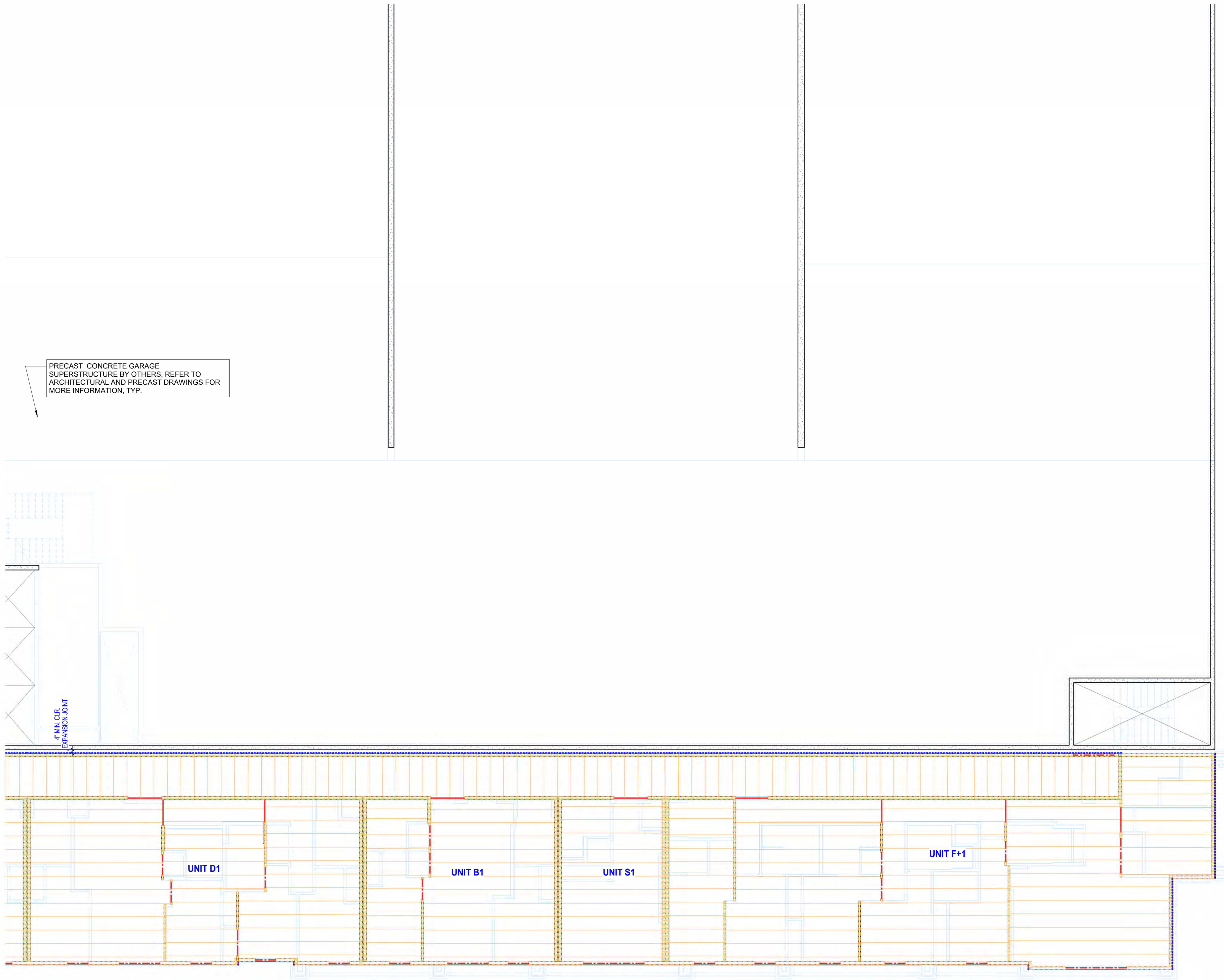
PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.
4" MIN. CLR. EXPANSION JOINT
UNIT D1
UNIT B1
UNIT S1
UNIT F+1

TRUE NORTH
PROJECT NORTH

1 Roof Framing Plan - West Building - Level 06 - Area C
1/8" = 1'-0"

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= LEDGER
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= GIRDER TRUSS
HSS / W = STEEL BEAM

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

NILES BOLTON ASSOCIATES

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Suite 600
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	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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SHEET TITLE:
Wood Framing Plan - West Building - Level 06/ Roof - Area C

SHEET NUMBER:
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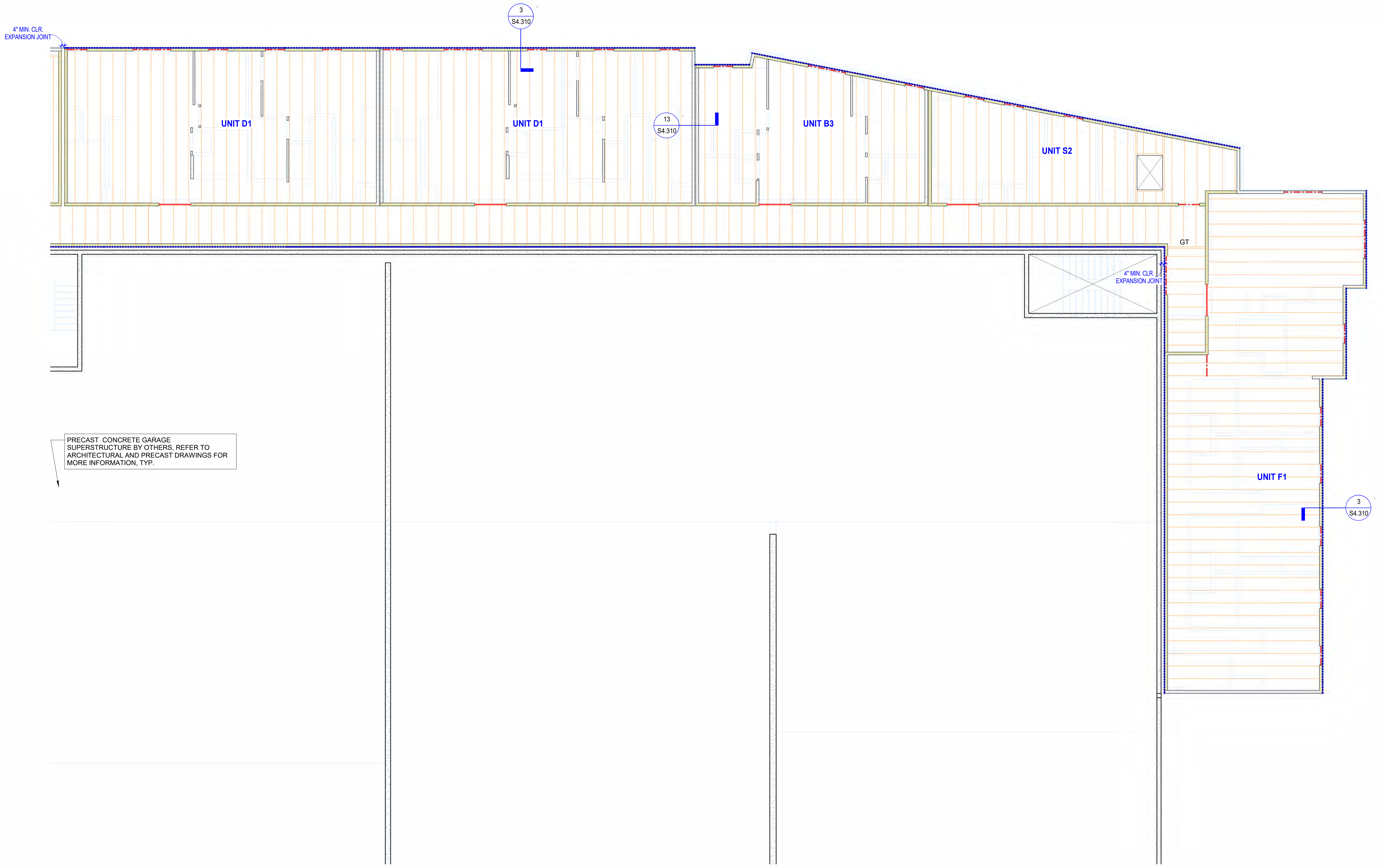
4" MIN. CLR.
EXPANSION JOINT
3
S4.310
UNIT D1
UNIT D1
13
S4.310
UNIT B3
UNIT S2
GT
4" MIN. CLR.
EXPANSION JOINT
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3
S4.310
PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.

TRUE NORTH
PROJECT NORTH

① Roof Framing Plan - West Building - Level 06 - Area D
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VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMsq Structural Engineers, LLC
1080 Eldridge Parkway
Suite 350
Houston, TX 77077
781-407-5734
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PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

NILES BOLTON ASSOCIATES

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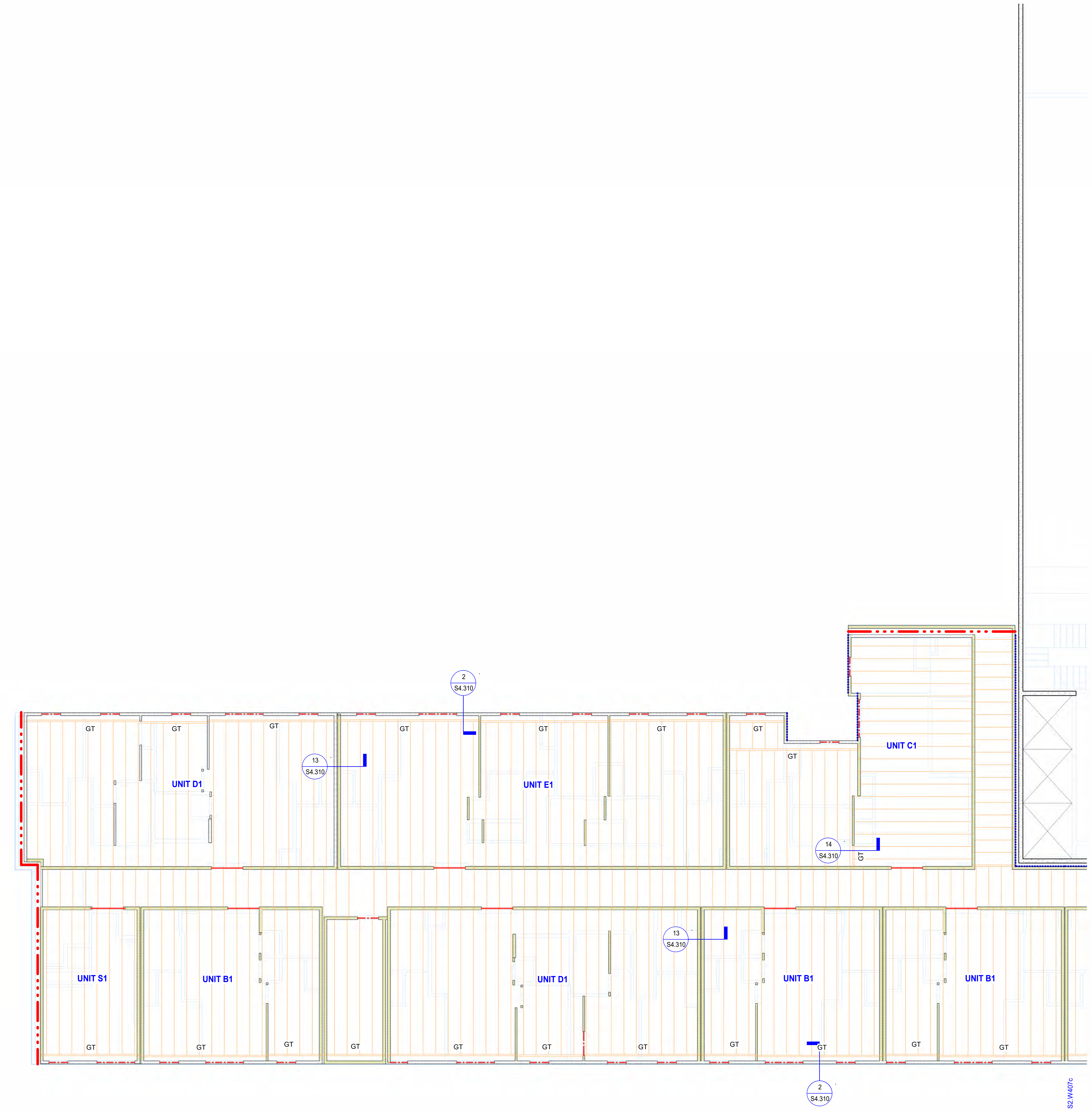

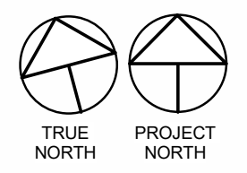

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ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:
1. ALL FRAMING EXPOSED TO EXTERIOR ELEMENTS TO BE PRESSURE TREATED (EXCLUDING FRT AND ENGINEERED LUMBER).

LEGEND:

▭ = EXTERIOR NON-LOAD BEARING WOOD FRAMED STUD WALL
▭ = LOAD BEARING WOOD FRAMED STUD WALL
▭ = EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
━ • • ━ = INDICATES 3HR FIRE SEPARATION WALL. REFER TO ARCHITECTURAL DETAILS FOR MORE INFORMATION.
—·—·— = HEADER (DROP BEAM)
—— = FLUSH BEAM
– – – = LEDGER
— = TRUSS / JOIST
═ = GIRDER TRUSS
HSS / W ━ = STEEL BEAM

VSM²
Voss | Starling | Maingot | Martin
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PROJECT # : 26154
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SHEET TITLE:
Wood Framing Plan - West Building - Roof - Area B

SHEET NUMBER:
S2.W407b

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WOOD ROOF FRAMING NOTES:

1. ROOF FRAMING TO BE 24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES @ 24" OC MAX UNO. TRUSSES TO BE SPACED AS NEEDED TO AVOID PLUMBING AND IN ACCORDANCE W/ TRUSS DESIGNER. TRUSS MANUFACTURER TO REFERENCE GENERAL NOTES FOR TOTAL DESIGN SUPERIMPOSED DEAD AND LIVE LOADS TO BE APPLIED TO FLOOR AND ROOF TRUSSES. IN ADDITION, TRUSS DESIGNER TO ASSUME 5 PSF OF THE TOTAL SUPERIMPOSED DEAD LOAD IS APPLIED TO TRUSS BOTTOM CHORD.
2. TRUSS DESIGNER TO REF MEP, ARCH, EBM (EXTERIOR BUILDING MAINTENANCE) EQUIPMENT FOR ADDITIONAL ROOF LOADS AND EXACT LOCATION. IN THE ABSENSE OF SPECIFIC LOADING CRITERIA, TRUSS DESIGNER SHALL INCLUDE A DESIGN ALLOWANCE AS FOLLOWS:
   - AHU'S, BOILERS AND WATER TANKS = 100PSF DL
   - PHOTO-VOLTAIC (SOLAR) ARRAYS = 30PSF DL
   - UNSCHEDULED MAINTENANCE TIE-BACKS = 3,100 LBS ASD IMPACT LOAD APPLIED IN ANY DIRECTION
3. CONTRACTOR TO VERIFY ALL DIMENSIONS, ELEVATIONS, DEPRESSIONS, STEPS, FINISHES, PLATE HEIGHTS AND SPECIFIC/OVERALL BUILDING GEOMETRY WITH THE ARCHITECTURAL DRAWINGS AND NOTIFY ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION START
4. SEE WOOD FRAMING SPECIFICATIONS FOR FLOOR / ROOF SHEATHING AND MINIMUM LUMBER GRADE / SPECIES REQUIREMENTS.
5. SECTIONS CUT ON PLAN SHALL BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS. REFER TO TYPICAL DETAILS/SECTIONS FOR DETAILS / SECTIONS NOT SPECIFICALLY SHOWN OR CUT ON PLAN.
6. SEE ROOF UPLIFT SECTIONS FOR FASTENING OF ROOF LEVEL HEADERS, BEAMS AND GIRDER TRUSSES.
7. AT ROOF UPLIFT ROD LOCATIONS, NUTS SHALL BE RE-TIGHTENED AS REQUIRED PRIOR TO INTERIOR WALL SHEATHING INSTALLATION TO ACCOMMODATE WOOD SHRINKAGE
8. AT LOCATIONS WHERE ROOF UPLIFT RODS COINCIDE WITH SHEAR WALL END ANCHORAGE RODS, ONLY THE SHEAR WALL ANCHORAGE ROD NEED BE INSTALLED. ALL OTHER FASTNENING HARDWARE SHOWN ON THE ROOF UPLIFT DETAILS SHALL BE INSTALLED AS INDICATED.
9. TRUSSES SHALL RECEIVE DOUBLE TOP CHORD AT SPECIFIC DIAPHRAGM STRAPPING LOCATIONS PER PLAN AND NOTES. REFER TO WOOD DIAPHRAGM REINFORCING PLANS FOR MORE INFO.
10. TRUSS SUPPLIER SHALL PROVIDE MECHANICAL CHASES WHERE REQUIRED TO FIT MEP DUCTS AND PIPES THROUGH TRUSS WEB OPENINGS. REFER TO MEP DRAWINGS AND COORDINATE WITH MEP WHERE OPENINGS ARE REQUIRED.
11. SEE THIS SHEET FOR POST/HANGER SCHEDULE. ALL BEAM/HEADER BEARING POINTS NOT NOTED ON PLAN SHALL BE SUPPORTED PER NOTE 4 OF THE POST SCHEDULE.
12. ALL HEADERS BEARING ROOF TRUSSES SHALL BE LOCATED DIRECTLY BELOW THE DOUBLE TOP PLATE UNO. SEE HEADER UPLIFT DETAILS.
13. PROVIDE 3/8" MIN HORIZONTAL GAP BETWEEN EXTERIOR WALL SHEATHING PANELS TO ALLOW FOR SHRINKAGE. GAP TO BE PLACED AT THE LEVEL OF THE FLOOR SHEATHING AT EACH LEVEL, TYP.
14. ROOF TIE-BACK AND EBM ANCHORS, WHERE APPLICABLE, SHALL BE ANCHORD TO ROOF TRUSSES PER TYPICAL DETAILS. TRUSS SUPPLIER SHALL LOCATE ALL ANCHORS PER THE EBM/FACADE ACCESS CONSULTANT DRAWINGS IN COORDINATION WITH THE ARCHITECTURAL DRAWINGS. TRUSS SUPPLIER SHALL DESIGN THE SUPPORTING TRUSSES TO MEET THE LOAD DEMAND(S) SHOWN ON THE TYPICAL DETAILS.
15. AT LOCATIONS WITH PARAPET FRAMING INTENDED TO SUPPORT EBM LOADS SHALL BE REINFORCED PER TYPICAL DETAILS. ALL OTHER LOCATIONS HAVE BEEN DESIGNED TO MEET WIND/SEISMIC LATERAL FORCES ONLY.
16. SEE TYPICAL DETAILS FOR MECHANICAL AND HVAC PLATFORM FRAMING.
17. REFER TO WOOD SHRINKAGE NOTES IN GENERAL NOTES FOR ESTIMATED BUILDING MOVEMENT DUE TO WOOD SHRINKAGE. CONTRACTOR SHALL COORDINATE ESTIMATED VERTICAL MOVEMENT WITH OTHER TRADES AS NEEDED. IN THE ABSENCE OF ESTIMATED BUILDING VERTICAL MOVEMENT, ASSUME 1/4" PER FLOOR.
18. ROOF TRUSSES SHALL BE DESIGNED FOR DRAG LOADS SHOWN ON PLAN. IN THE ABSENSE OF D.F. (DRAG FORCES), ALL TRUSSES PARALLEL TO SHEARWALLS SHALL BE DESIGNED TO RESIST MINIMUM 400 PLF DRAG FORCE (TYPICAL).

WOOD FRAMING SCHEDULE

LOCATION	FRAMING TYPE (U.N.O. ON PLAN)	MAX SPACING (U.N.O. ON PLAN)
FLOOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
CORRIDOR	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MAIN-LANDING	18" DEEP OPEN WEB WOOD TRUSSES	24" O.C.
STAIR MID-LANDING	2X12 JOIST (U.N.O. ON PLAN)	24" O.C.
ROOF	24" DEEP (MIN) SLOPED TOP OPEN WEB WOOD TRUSSES	24" O.C.

NOTES:
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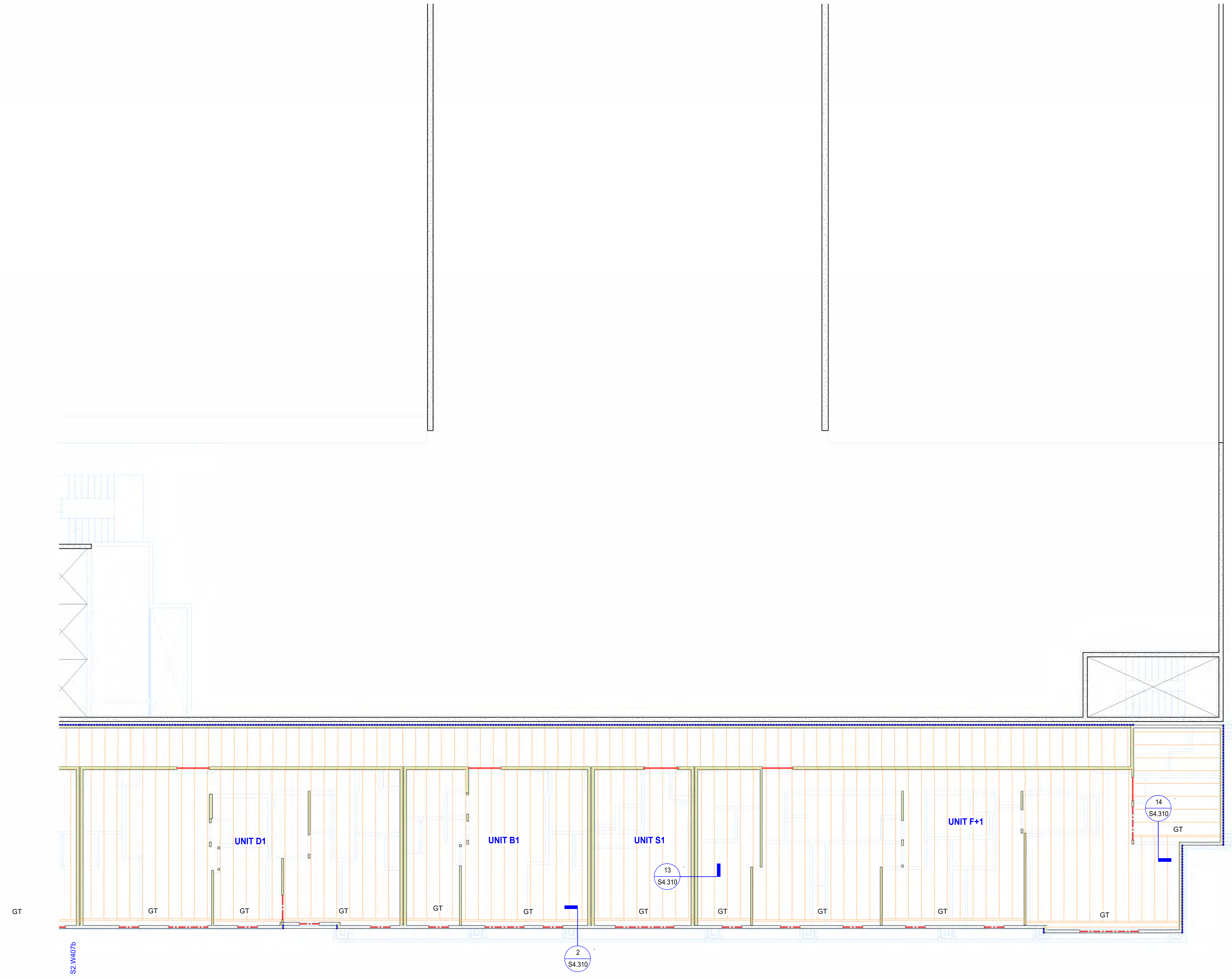
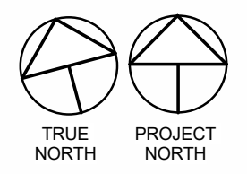
= LOAD BEARING WOOD FRAMED STUD WALL
= EXTERIOR 2 HOUR RATED LOAD BEARING WOOD FRAMED STUD WALL
= INDICATES 3HR FIRE SEPARATION WALL. REFER TO ARCHITECTURAL DETAILS FOR MORE INFORMATION.
= HEADER (DROP BEAM)
= FLUSH BEAM
= LEDGER
= TRUSS / JOIST
= GIRDER TRUSS
HSS / W = STEEL BEAM





1 Roof Framing Plan - West Building - Level 07 - Area C
1/8" = 1'-0"

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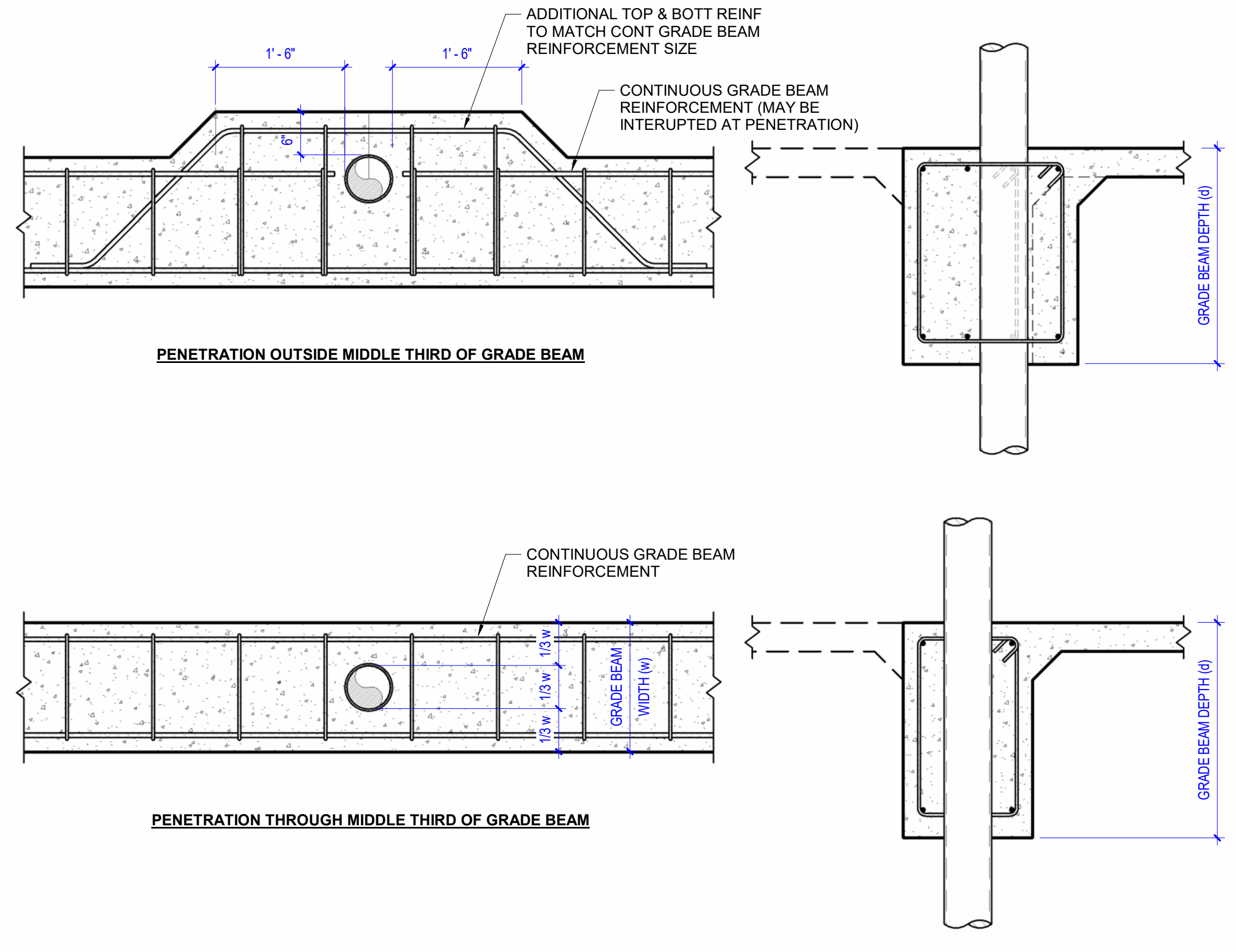
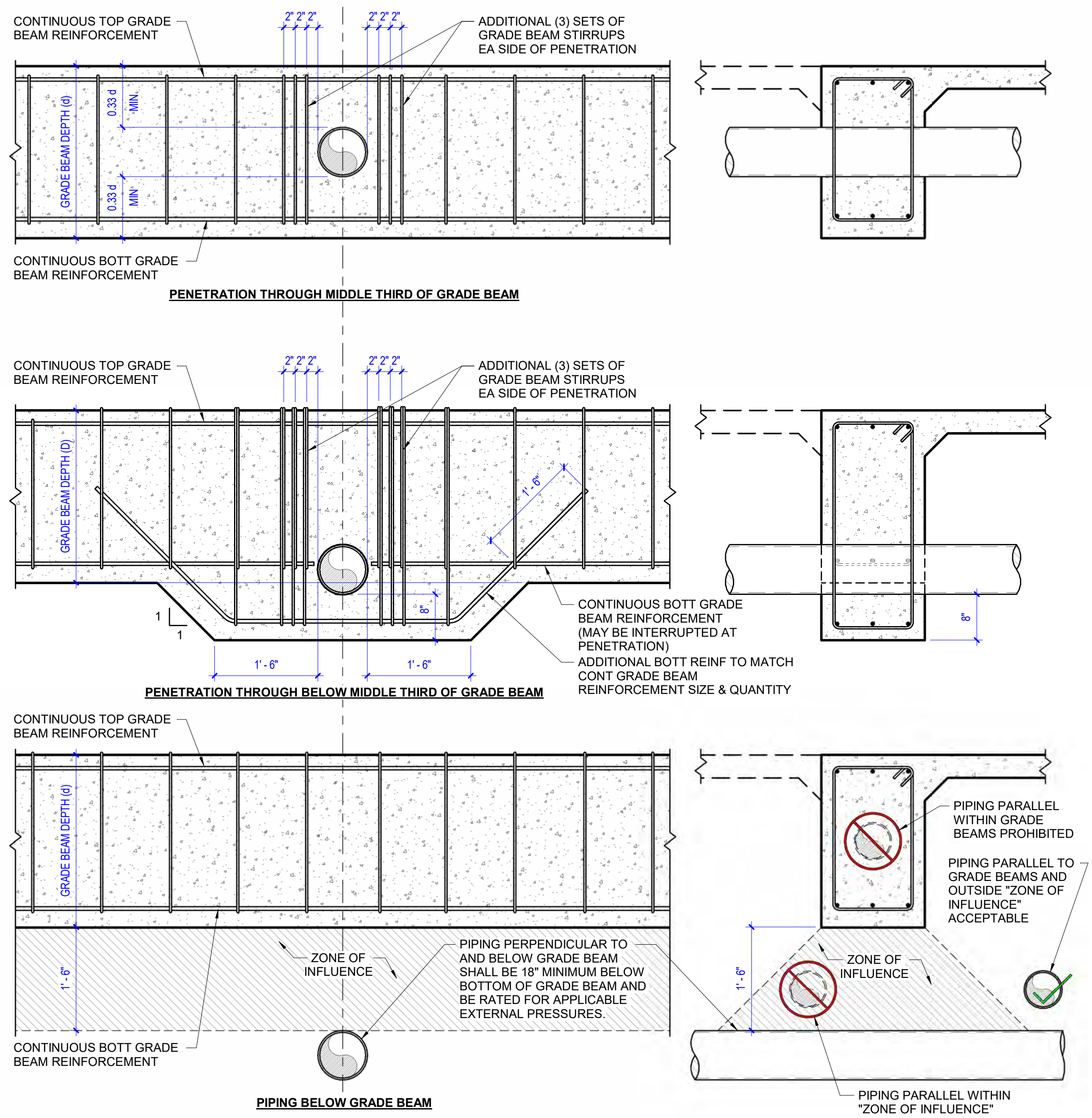

ADDITIONAL TOP & BOTT REINF TO MATCH CONT GRADE BEAM REINFORCEMENT SIZE
1' - 6"
1' - 6"
6"
CONTINUOUS GRADE BEAM REINFORCEMENT (MAY BE INTERUPTED AT PENETRATION)
GRADE BEAM DEPTH (d)
PENETRATION OUTSIDE MIDDLE THIRD OF GRADE BEAM
CONTINUOUS GRADE BEAM REINFORCEMENT
1/3 w
1/3 w
1/3 w
GRADE BEAM WIDTH (w)
GRADE BEAM DEPTH (d)
PENETRATION THROUGH MIDDLE THIRD OF GRADE BEAM

VERTICAL PLUMBING THROUGH GRADE BEAM

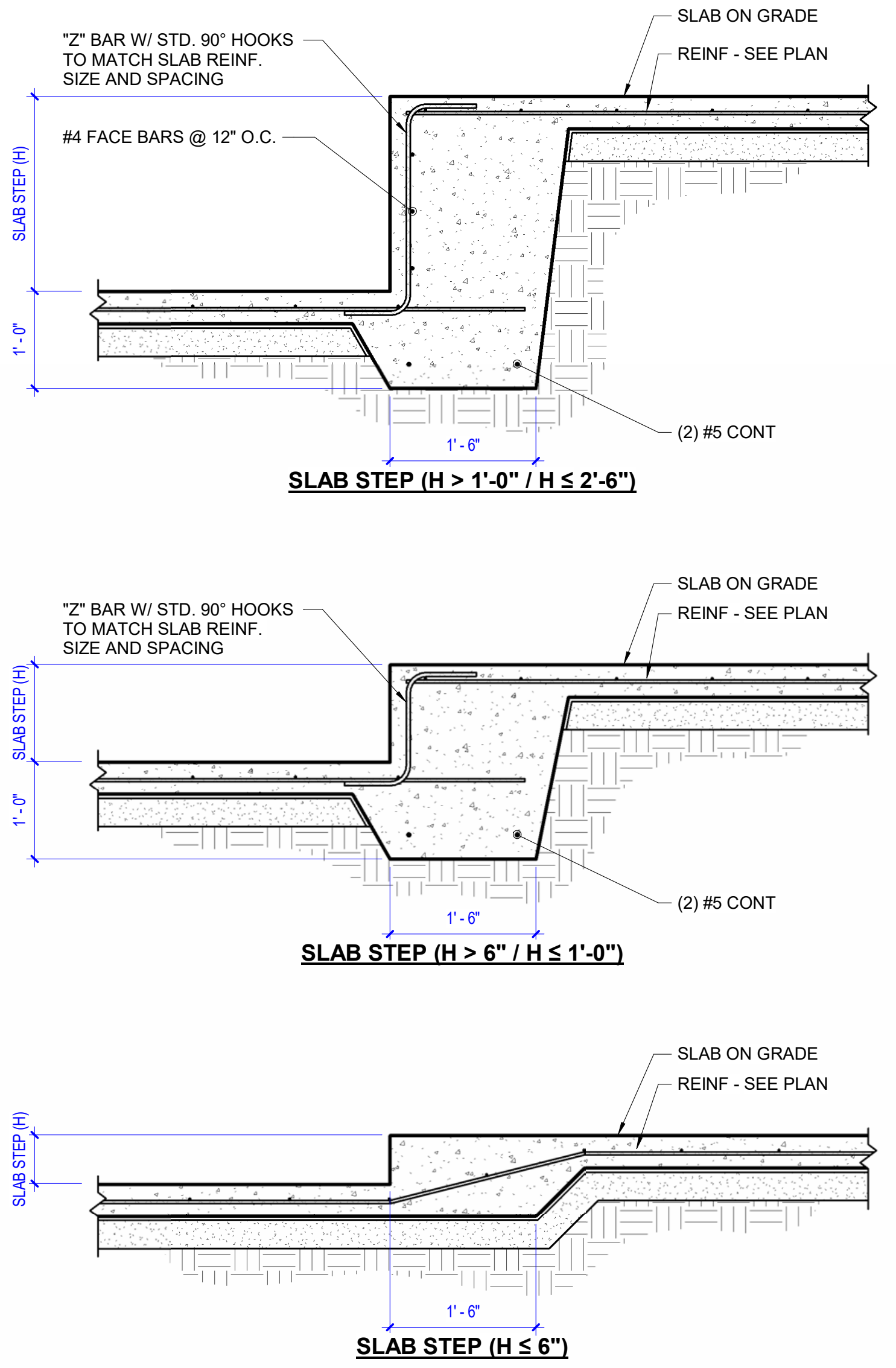
10 Typical Penetration In Grade Beam

3/4" = 1'-0"


CONTINUOUS TOP GRADE BEAM REINFORCEMENT
2" 2" 2"
2" 2" 2"
ADDITIONAL (3) SETS OF GRADE BEAM STIRRUPS EA SIDE OF PENETRATION
GRADE BEAM DEPTH (d)
0.33 d MIN.
0.33 d MIN.
CONTINUOUS BOTT GRADE BEAM REINFORCEMENT
PENETRATION THROUGH MIDDLE THIRD OF GRADE BEAM
CONTINUOUS TOP GRADE BEAM REINFORCEMENT
2" 2" 2"
2" 2" 2"
ADDITIONAL (3) SETS OF GRADE BEAM STIRRUPS EA SIDE OF PENETRATION
GRADE BEAM DEPTH (D)
1' - 6"
8"
1
1
1' - 6"
1' - 6"
8"
CONTINUOUS BOTT GRADE BEAM REINFORCEMENT (MAY BE INTERRUPTED AT PENETRATION)
ADDITIONAL BOTT REINF TO MATCH CONT GRADE BEAM REINFORCEMENT SIZE & QUANTITY
PENETRATION THROUGH BELOW MIDDLE THIRD OF GRADE BEAM
CONTINUOUS TOP GRADE BEAM REINFORCEMENT
GRADE BEAM DEPTH (d)
1' - 6"
ZONE OF INFLUENCE
PIPING PERPENDICULAR TO AND BELOW GRADE BEAM SHALL BE 18" MINIMUM BELOW BOTTOM OF GRADE BEAM AND BE RATED FOR APPLICABLE EXTERNAL PRESSURES.
CONTINUOUS BOTT GRADE BEAM REINFORCEMENT
PIPING BELOW GRADE BEAM
PIPING PARALLEL WITHIN GRADE BEAMS PROHIBITED
PIPING PARALLEL TO GRADE BEAMS AND OUTSIDE "ZONE OF INFLUENCE" ACCEPTABLE
1' - 6"
ZONE OF INFLUENCE
PIPING PARALLEL WITHIN "ZONE OF INFLUENCE" PROHIBITED

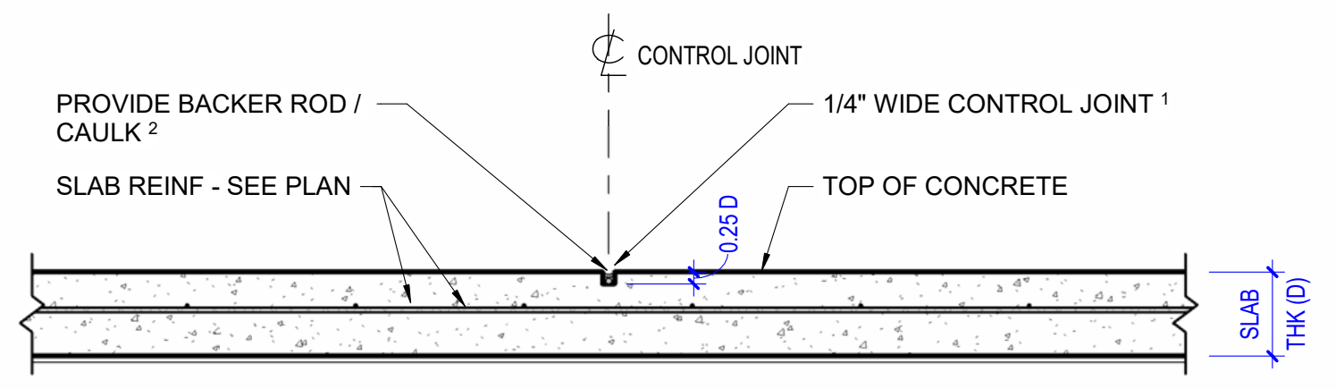
9 Typical Penetration In Grade Beam

3/4" = 1'-0"



8 Typical Step in Slab on Grade

3/4" = 1'-0"

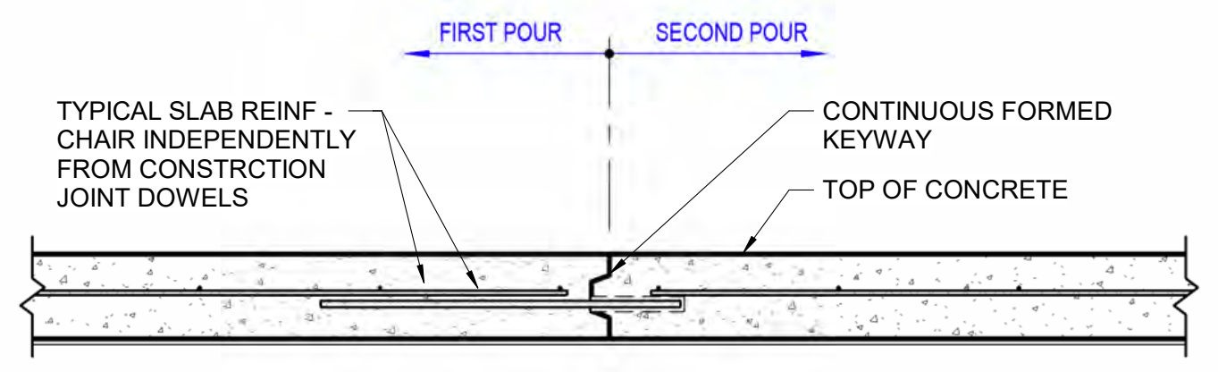


NOTE:

1. AT CONTRATORS OPTION, CONTROL JOINT MAY BE:
   - PRE-FORMED ZIP-STRIP OR EQUAL - PRE-FORMED CONTROL JOINTS TO BE INSTALLED IN FRESH CONCRETE PER MANUFACTURERS RECOMENDATIONS.
   - SAW CUT - JOINTS TO BE CUT INTO SLAB AS SOON AS SLAB IS HARDENED ENOUGH TO SUPPORT THE WEIGHT OF SAWING EQUIPMENT WITHOUT DAMAGING SLAB (APPROXIMATLY 4 TO 8 HOURS AFTER PLACEMENT).
2. CLEAN CONTROL JOINTS TO ENSURE JOINT IS CLEAR OF ALL LOOSE DEBRIS PRIOR TOP FILLING THE JOINTS.

7 Slab on Grade Control Joint

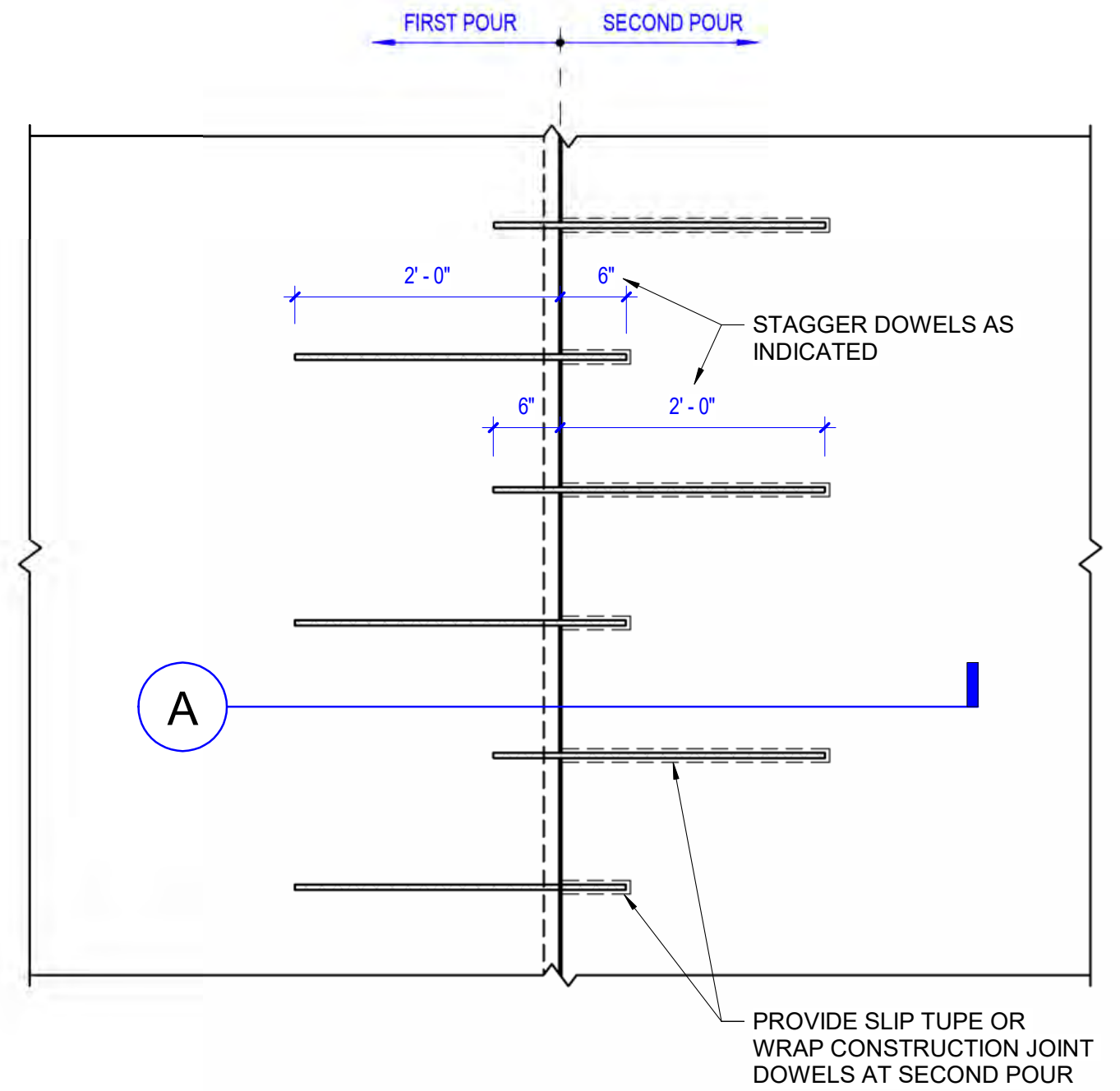
3/4" = 1'-0"


FIRST POUR
SECOND POUR
TYPICAL SLAB REINF - CHAIR INDEPENDENTLY FROM CONSTRCTION JOINT DOWELS
CONTINUOUS FORMED KEYWAY
TOP OF CONCRETE

A SECTION

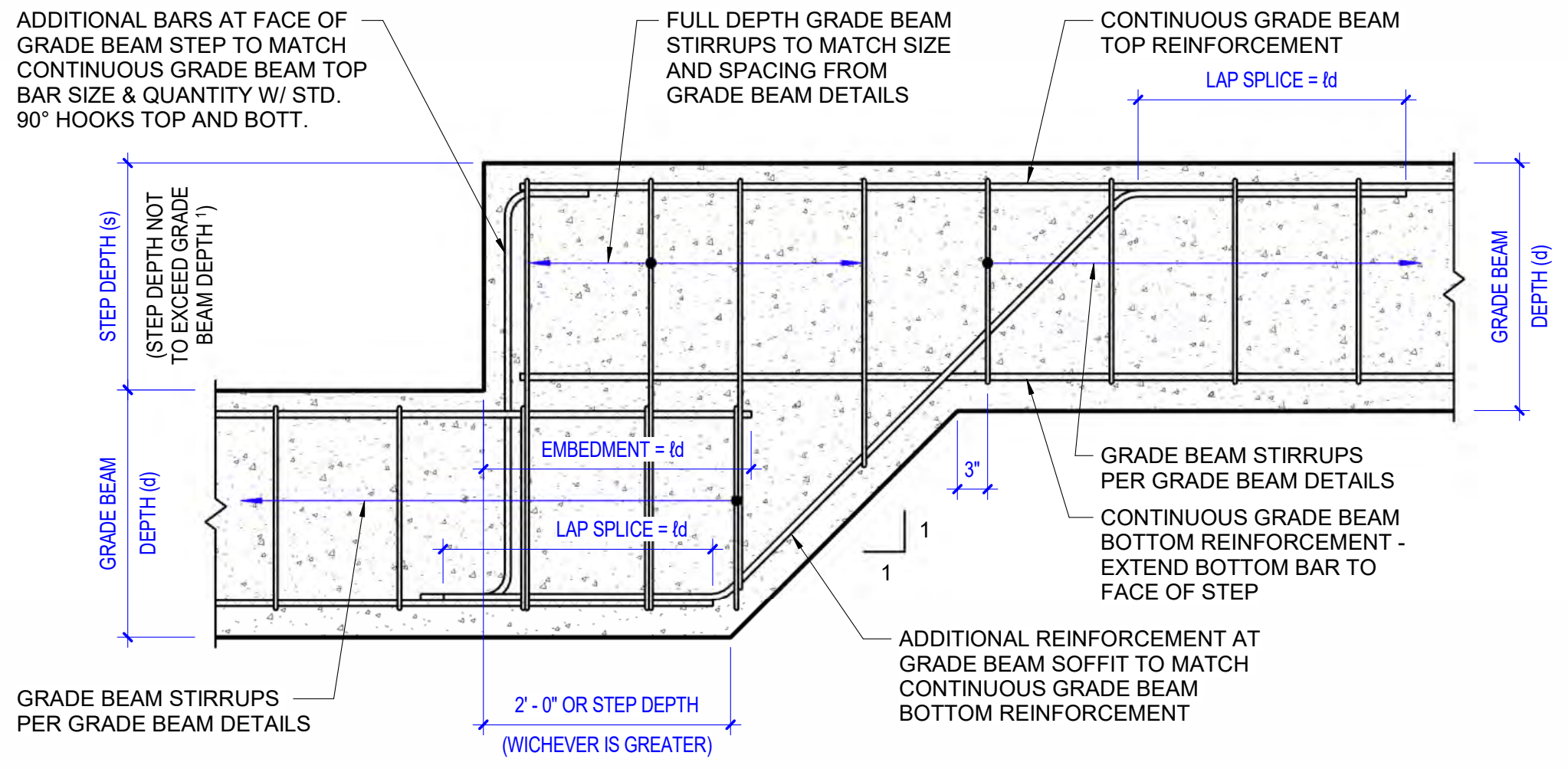
NOTE:

1. CONTRACTOR TO SUBMIT CONSTRUCTION JOINT LAYOUT TO EOR FOR APPROVAL.
2. DOWELS SHALL BE PLACED PERPENDICULAR TO FACE OF SLAB AT CONSTRUCTION JOINT TO ALLOW FOR SLIPPAGE DURING SHRINKAGE.


FIRST POUR
SECOND POUR
2' - 0"
6"
6"
2' - 0"
STAGGER DOWELS AS INDICATED
A
PROVIDE SLIP TUPE OR WRAP CONSTRUCTION JOINT DOWELS AT SECOND POUR

6 Slab on Grade Construction Joint

3/4" = 1'-0"

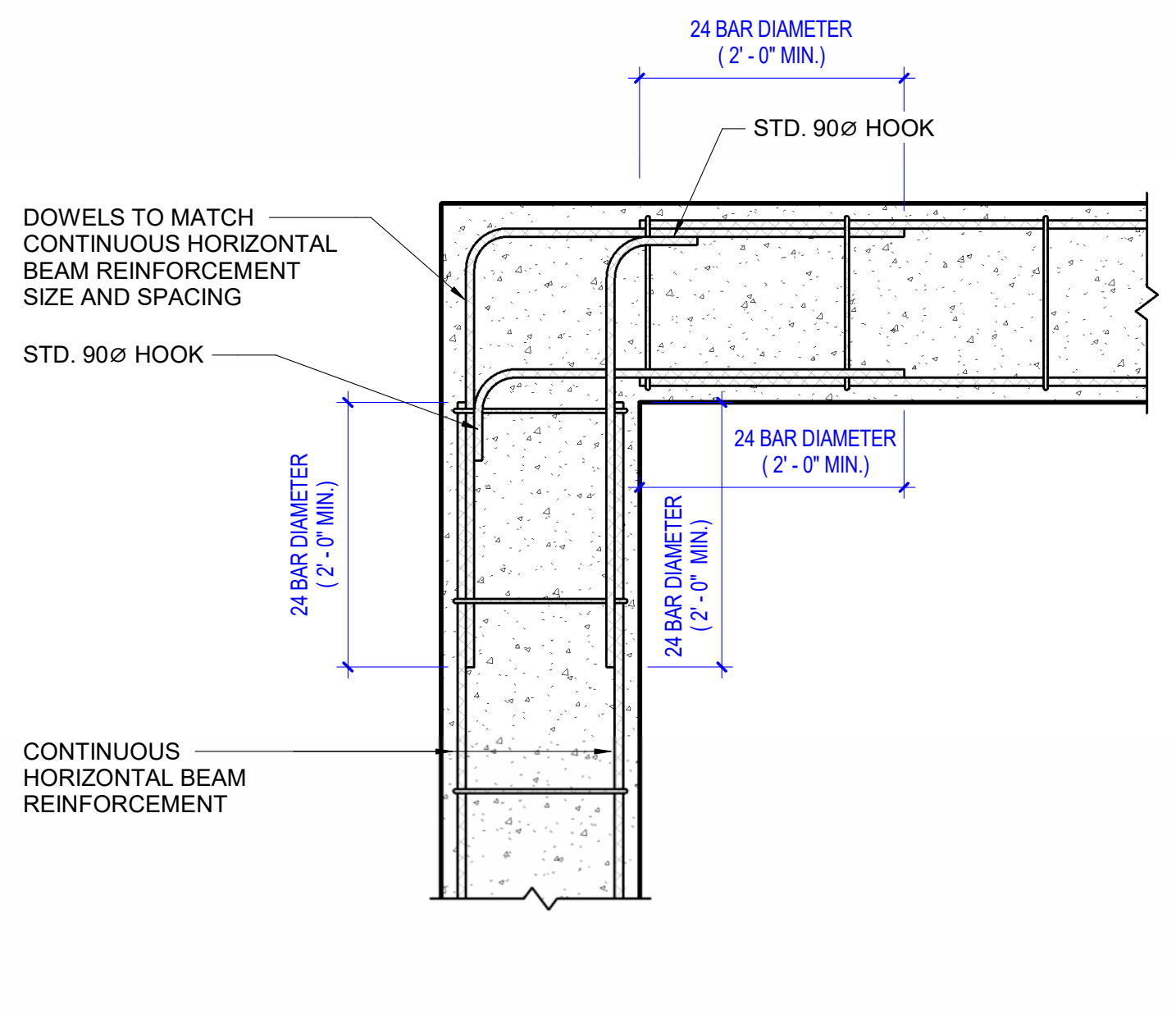

ADDITIONAL BARS AT FACE OF GRADE BEAM STEP TO MATCH CONTINUOUS GRADE BEAM TOP BAR SIZE & QUANTITY W/ STD. 90° HOOKS TOP AND BOTT.
FULL DEPTH GRADE BEAM STIRRUPS TO MATCH SIZE AND SPACING FROM GRADE BEAM DETAILS
CONTINUOUS GRADE BEAM TOP REINFORCEMENT
LAP SPLICE = ℓd
STEP DEPTH (s)
(STEP DEPTH NOT TO EXCEED GRADE BEAM DEPTH 1)
GRADE BEAM DEPTH (d)
GRADE BEAM DEPTH (d)
EMBEDMENT = ℓd
LAP SPLICE = ℓd
3"
1
1
GRADE BEAM STIRRUPS PER GRADE BEAM DETAILS
CONTINUOUS GRADE BEAM BOTTOM REINFORCEMENT - EXTEND BOTTOM BAR TO FACE OF STEP
ADDITIONAL REINFORCEMENT AT GRADE BEAM SOFFIT TO MATCH CONTINUOUS GRADE BEAM BOTTOM REINFORCEMENT
GRADE BEAM STIRRUPS PER GRADE BEAM DETAILS
2' - 0" OR STEP DEPTH
(WICHEVER IS GREATER)

NOTE:

1. IF STEP IS GREATER THAN BEAM DEPTH, PROVIDE MULTIPLE EQUAL STEPS IN GRADE BEAM MIN 3 GRADE BEAM DEPTH'S APART.

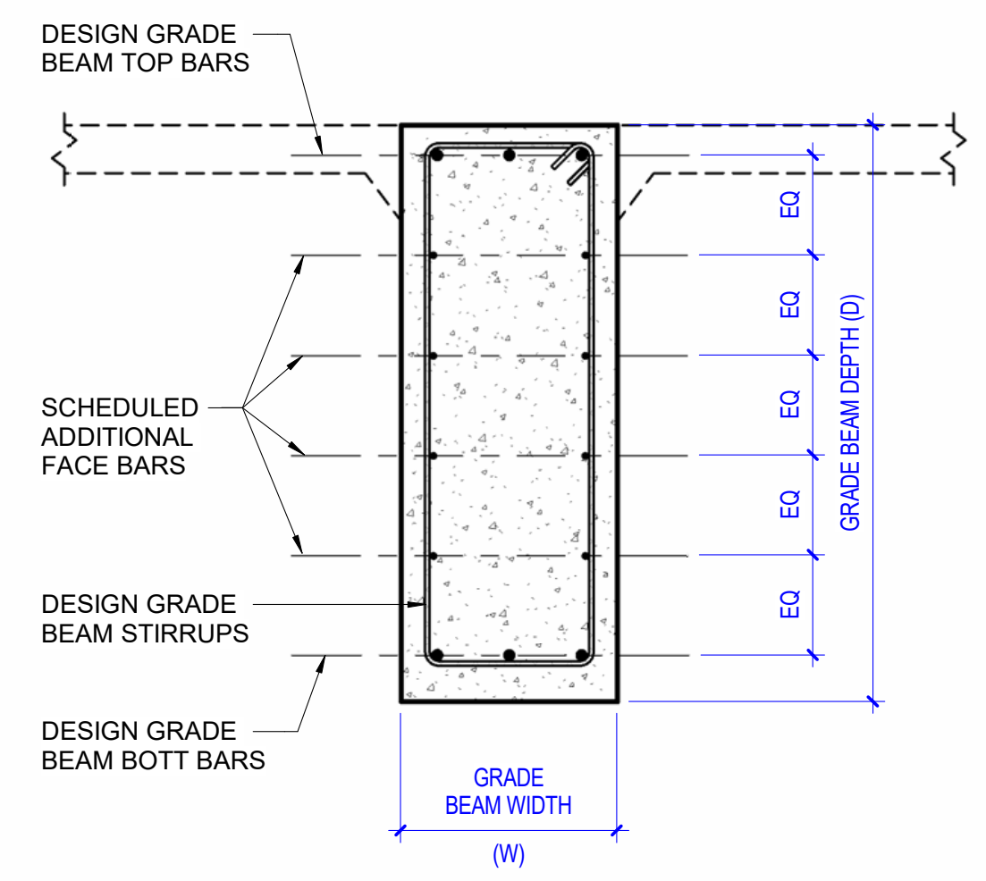
5 Typical Step in Grade Beam

3/4" = 1'-0"


24 BAR DIAMETER (2' - 0" MIN.)
STD. 90⌀ HOOK
DOWELS TO MATCH CONTINUOUS HORIZONTAL BEAM REINFORCEMENT SIZE AND SPACING
STD. 90⌀ HOOK
GRADE BEAM DEPTH (d)
24 BAR DIAMETER (2' - 0" MIN.)
24 BAR DIAMETER (2' - 0" MIN.)
24 BAR DIAMETER (2' - 0" MIN.)
CONTINUOUS HORIZONTAL BEAM REINFORCEMENT

3 Grade Beam Corner

3/4" = 1'-0"

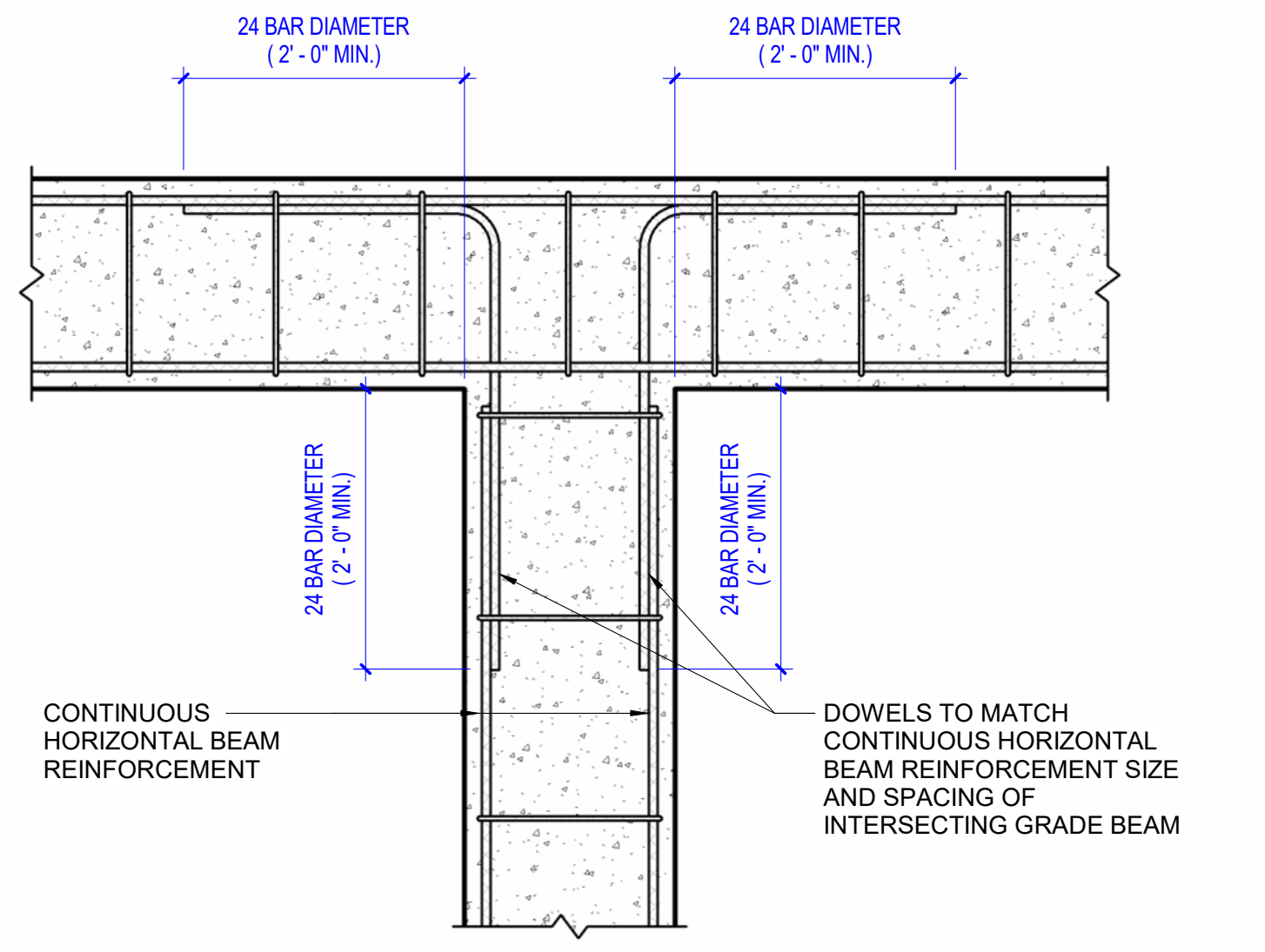

DESIGN GRADE BEAM TOP BARS
SCHEDULED ADDITIONAL FACE BARS
DESIGN GRADE BEAM STIRRUPS
DESIGN GRADE BEAM BOTT BARS
EQ
EQ
EQ
EQ
EQ
GRADE BEAM DEPTH (D)
GRADE BEAM WIDTH
(W)

FACE BAR SCHEDULE

GRADE BEAM DEPTH (D)	ADDITIONAL FACE BARS (EA FACE)
D < 36"	NONE
36" ≤ D < 42"	(3) #4
42" ≤ D < 48"	(4) #4
48" ≤ D ≤ 54"	(5) #4

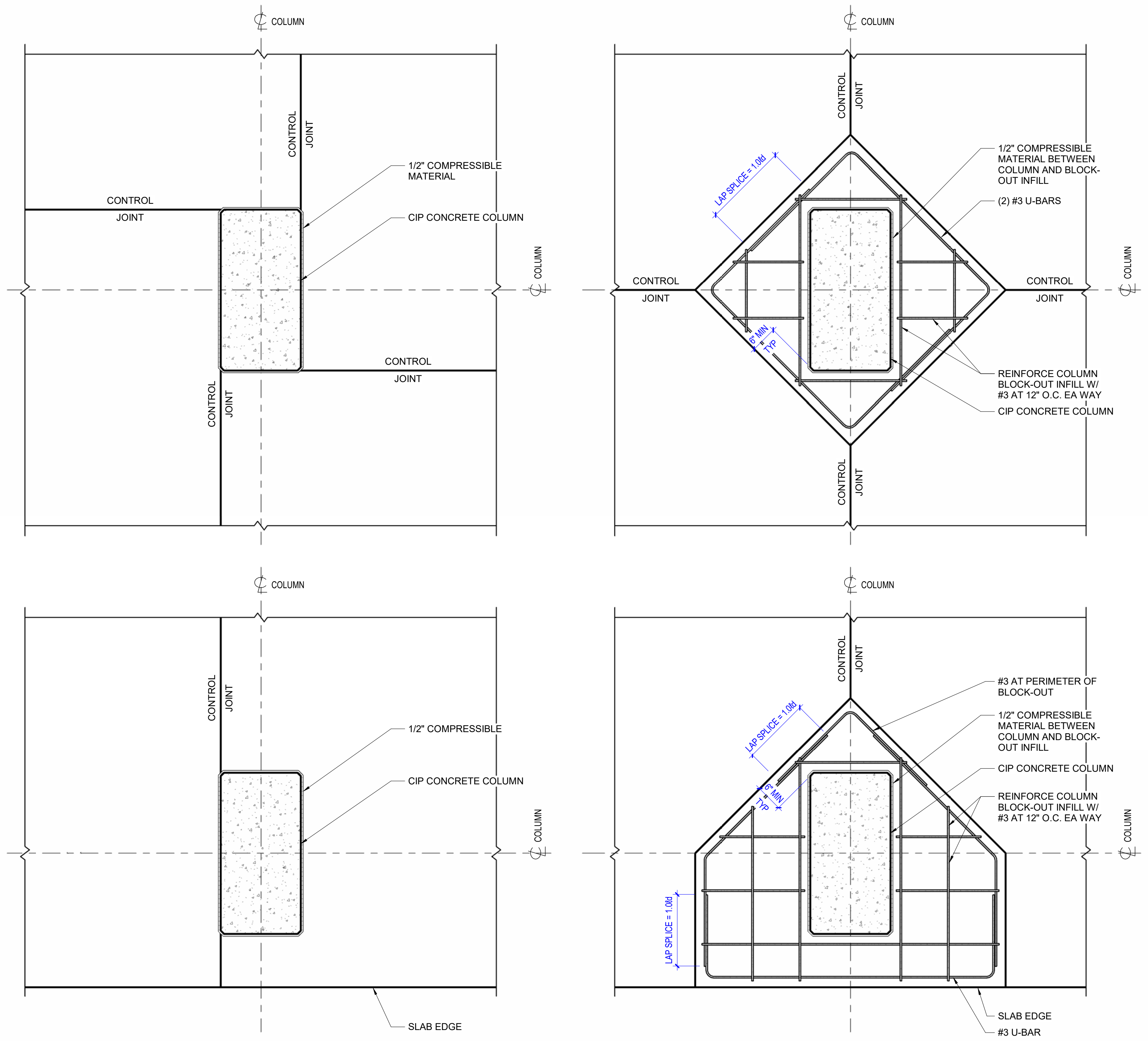
4 Grade Beam Face Reinforcement

3/4" = 1'-0"


24 BAR DIAMETER (2' - 0" MIN.)
24 BAR DIAMETER (2' - 0" MIN.)
24 BAR DIAMETER (2' - 0" MIN.)
24 BAR DIAMETER (2' - 0" MIN.)
CONTINUOUS HORIZONTAL BEAM REINFORCEMENT
DOWELS TO MATCH CONTINUOUS HORIZONTAL BEAM REINFORCEMENT SIZE AND SPACING OF INTERSECTING GRADE BEAM

2 Grade Beam Intersection

3/4" = 1'-0"



PIN WHEEL OPTION

BLOCKOUT OPTION

1 Column Block-Out - CIP Column

3/4" = 1'-0"

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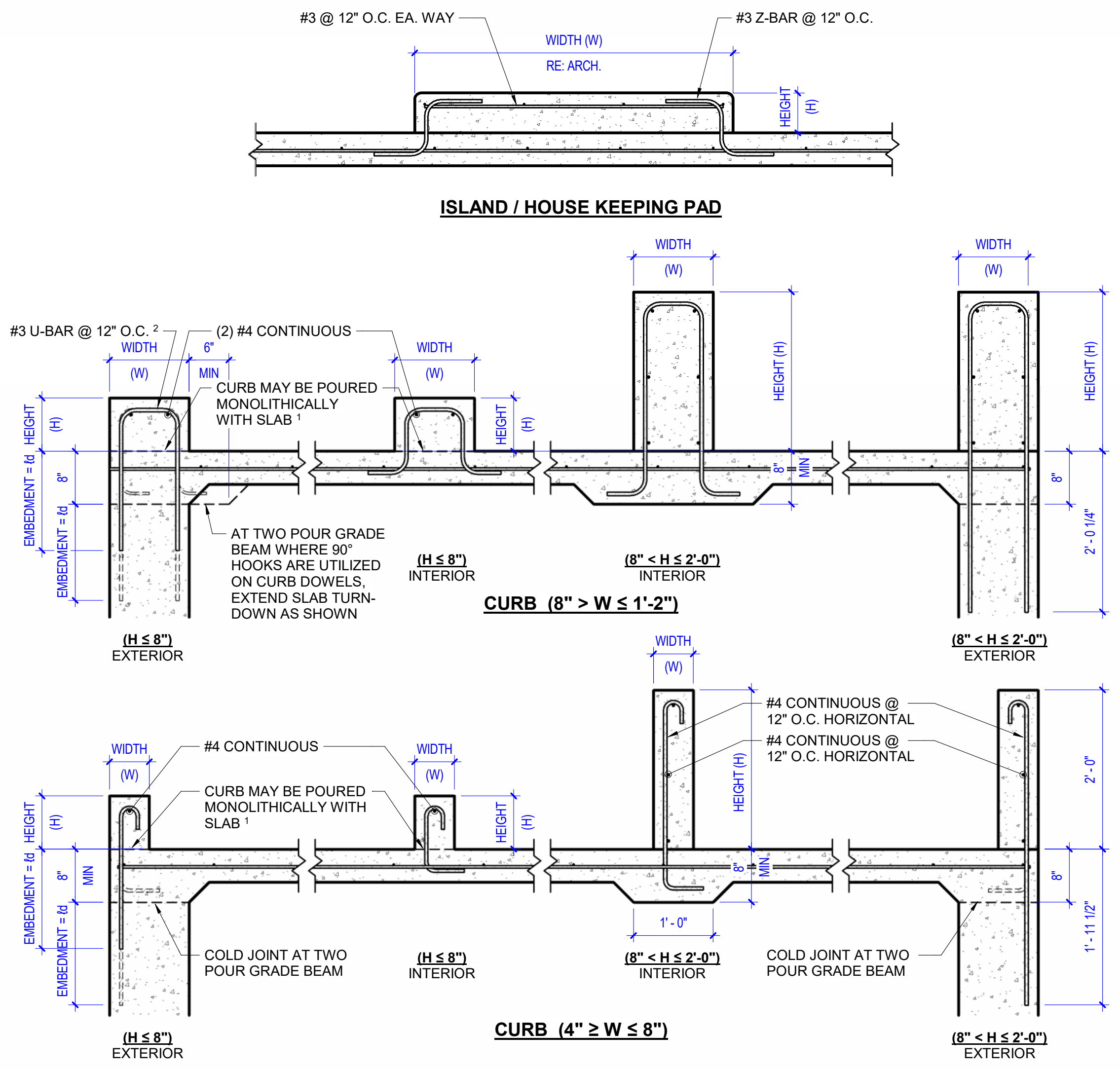
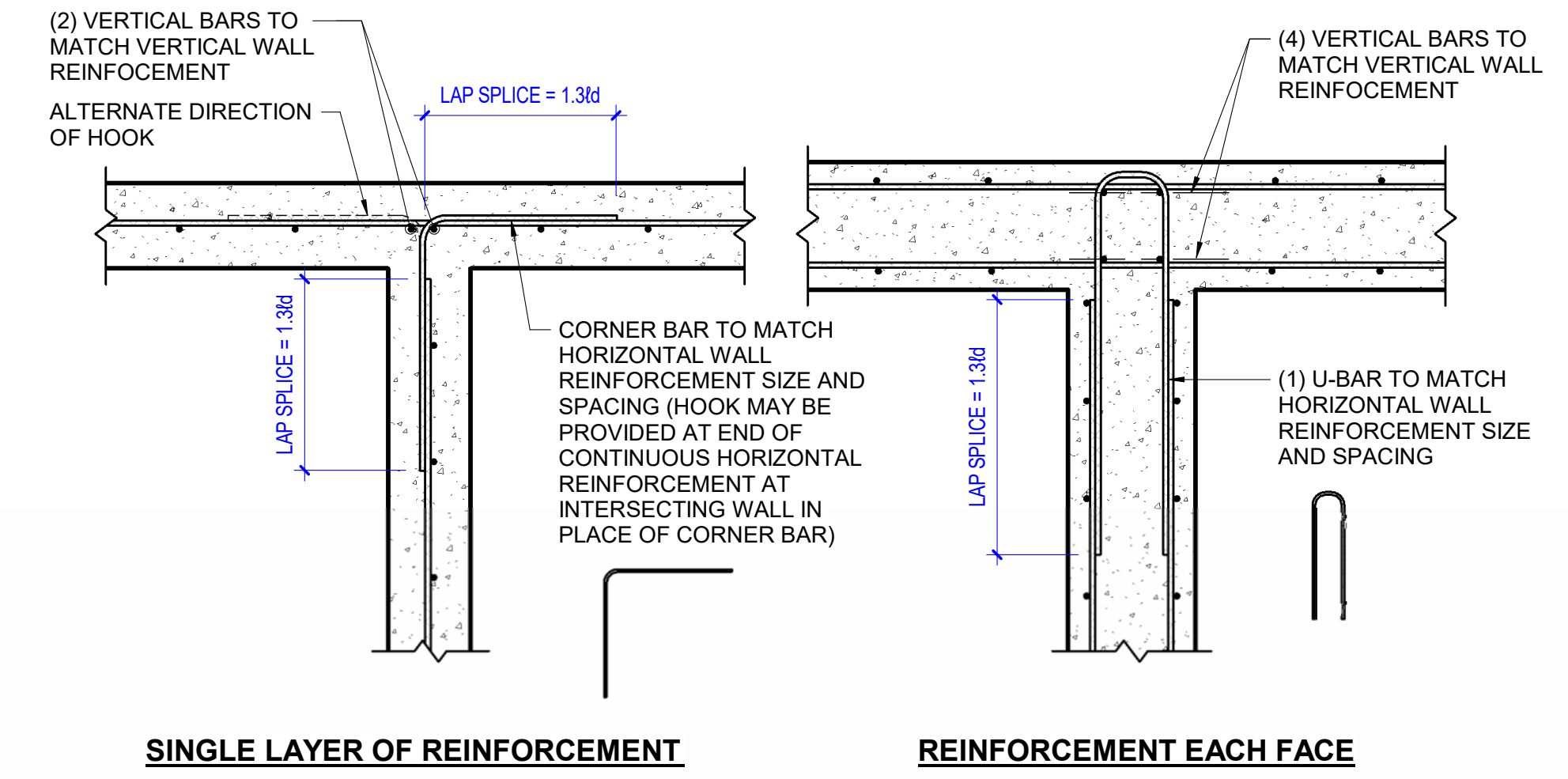
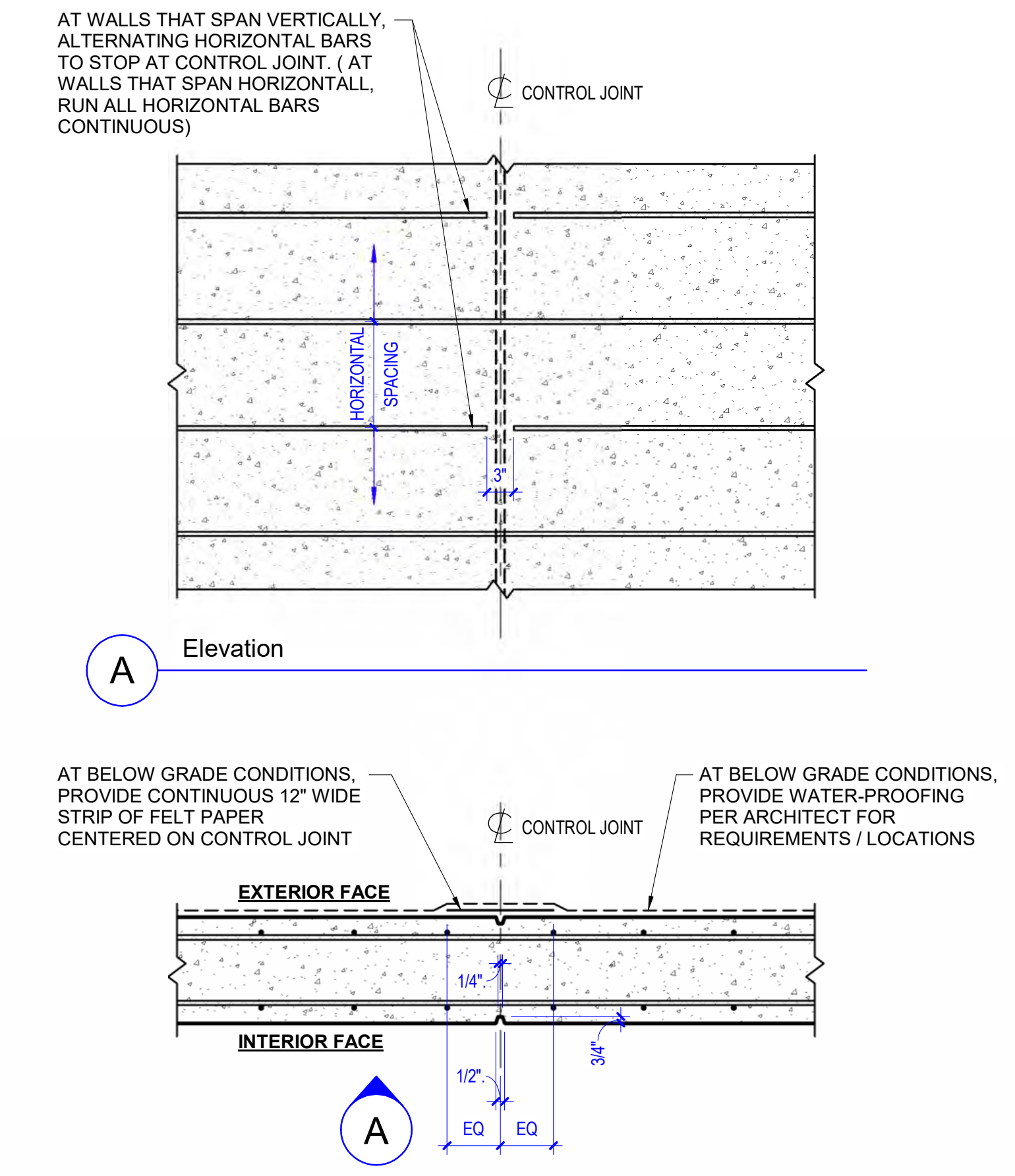

#3 @ 12" O.C. EA. WAY
#3 Z-BAR @ 12" O.C.
WIDTH (W)
RE: ARCH.
HEIGHT (H)
ISLAND / HOUSE KEEPING PAD
#3 U-BAR @ 12" O.C. 2
(2) #4 CONTINUOUS
WIDTH (W)
6" MIN
CURB MAY BE POURED MONOLITHICALLY WITH SLAB 1
HEIGHT (H)
EMBEDMENT = ℓd
8"
AT TWO POUR GRADE BEAM WHERE 90° HOOKS ARE UTILIZED ON CURB DOWELS, EXTEND SLAB TURN-DOWN AS SHOWN
(H ≤ 8") EXTERIOR
(H ≤ 8") INTERIOR
(8" < H ≤ 2'-0") INTERIOR
(8" < H ≤ 2'-0") EXTERIOR
CURB (8" > W ≤ 1'-2")
8" MIN
2' - 0 1/4"
#4 CONTINUOUS
CURB MAY BE POURED MONOLITHICALLY WITH SLAB 1
#4 CONTINUOUS @ 12" O.C. HORIZONTAL
#4 CONTINUOUS @ 12" O.C. HORIZONTAL
MIN
1' - 0"
2' - 0"
1' - 11 1/2"
COLD JOINT AT TWO POUR GRADE BEAM
COLD JOINT AT TWO POUR GRADE BEAM
CURB (4" ≥ W ≤ 8")

NOTE:
1. WHEN CURB HEIGHT (H) IS 8" OR LESS, CURB MAY BE POURED AS SECOND POUR OR MONOLITHIC WITH SLAB AT CONTRACTORS OPTION.
2. U-BAR MAY BE CAST INTO SLAB POUR OVER WITH 90° HOOKS ON EACH LEG ON TWO POUR GRADE BEAMS AT CONTRACTORS OPTION WHERE INDICATED.
3. WHEN CURB IS UTILIZED AT THE BASE OF CMU WALL, CMU SLAB BEAM IS TO BE USED IN LIEU OF THICKENED SLAB INDICATED HERE.

9 Typical Curb / House Keeping Pad
3/4" = 1'-0"

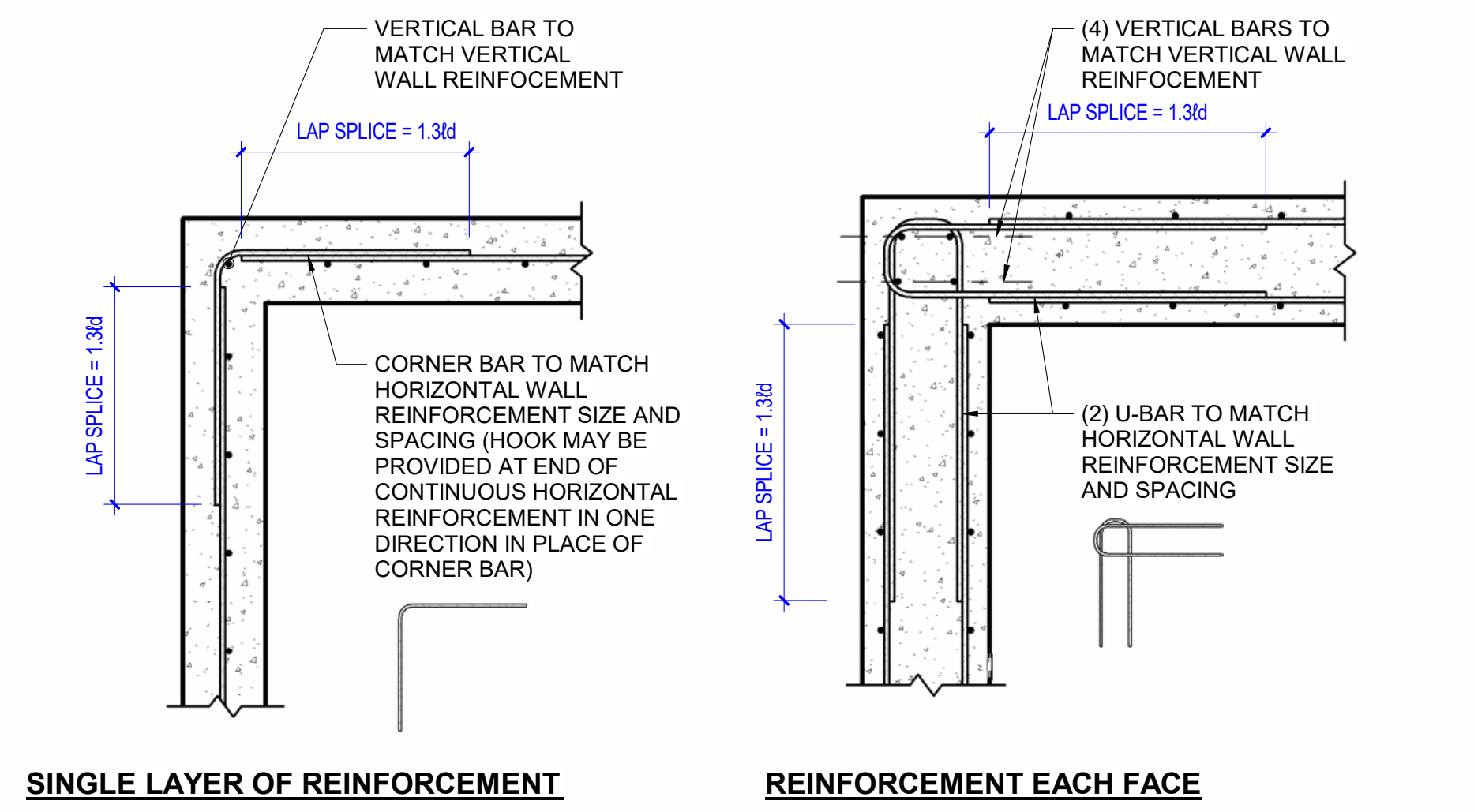

(2) VERTICAL BARS TO MATCH VERTICAL WALL REINFOCEMENT
ALTERNATE DIRECTION OF HOOK
LAP SPLICE = 1.3ℓd
CORNER BAR TO MATCH HORIZONTAL WALL REINFORCEMENT SIZE AND SPACING (HOOK MAY BE PROVIDED AT END OF CONTINUOUS HORIZONTAL REINFORCEMENT AT INTERSECTING WALL IN PLACE OF CORNER BAR)
(4) VERTICAL BARS TO MATCH VERTICAL WALL REINFOCEMENT
(1) U-BAR TO MATCH HORIZONTAL WALL REINFORCEMENT SIZE AND SPACING
SINGLE LAYER OF REINFORCEMENT
REINFORCEMENT EACH FACE

6 Concrete Wall Intersection
3/4" = 1'-0"

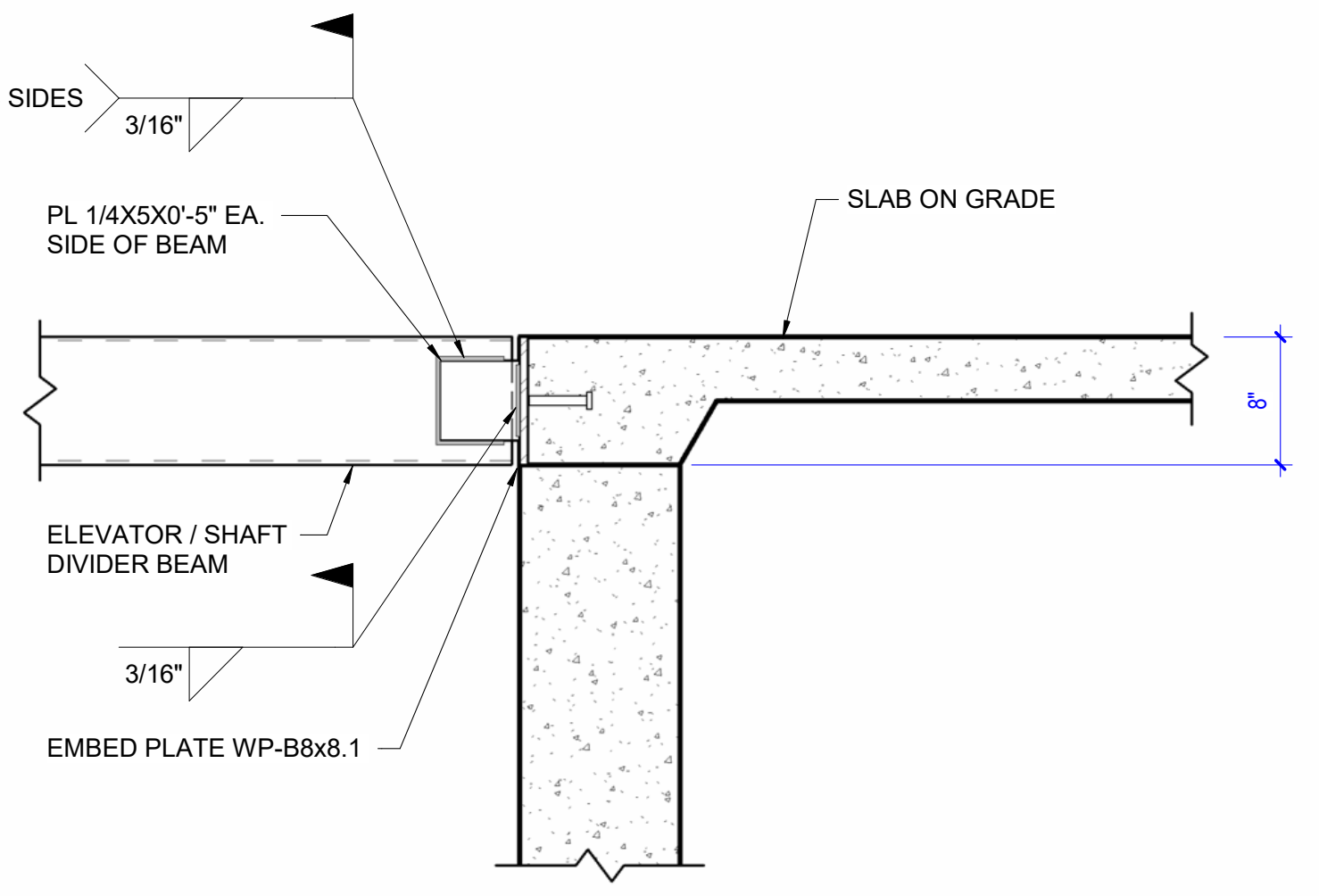

AT WALLS THAT SPAN VERTICALLY, ALTERNATING HORIZONTAL BARS TO STOP AT CONTROL JOINT. (AT WALLS THAT SPAN HORIZONTALL, RUN ALL HORIZONTAL BARS CONTINUOUS)
℄ CONTROL JOINT
HORIZONTAL SPACING
3"
A Elevation
AT BELOW GRADE CONDITIONS, PROVIDE CONTINUOUS 12" WIDE STRIP OF FELT PAPER CENTERED ON CONTROL JOINT
℄ CONTROL JOINT
AT BELOW GRADE CONDITIONS, PROVIDE WATER-PROOFING PER ARCHITECT FOR REQUIREMENTS / LOCATIONS
EXTERIOR FACE
INTERIOR FACE
1/4"
3/4"
1/2"
EQ
EQ
A

NOTES:
1. PROVIDE VERTICAL WALL CONTROL JOINTS AT 60'-0" O.C. MAXIMUM SPACING (UNO.)
2. AT CONDITIONS WHERE WALL SPANS HORIZONTALLY, CONTROL JOINTS TO BE LOCATED IN MIDDLE THIRD OF WALL SPANS (UNO).
3. CONTRACTOR TO SUBMIT CONTROL JOINT LOCATIONS PRIOR TO CONSTRUCTION TO EOR FOR REVIEW / APPROVAL.
4. RE: ARCHITECT FOR ADDITIONAL CONTROL JOINT REQUIREMENTS AND TREATMENT OF EXPOSED CONTROL JOINTS. (1/2" EXTERIOR CONTROL JOINT WIDTH MAY BE INCREASED TO MATCH ARCHITECTURAL INTENT.
5. AT BELOW GRADE CONDITIONS, CONTROL JOINT TO BE SEALED W/ WEATHER-RESISTANT SEALANT

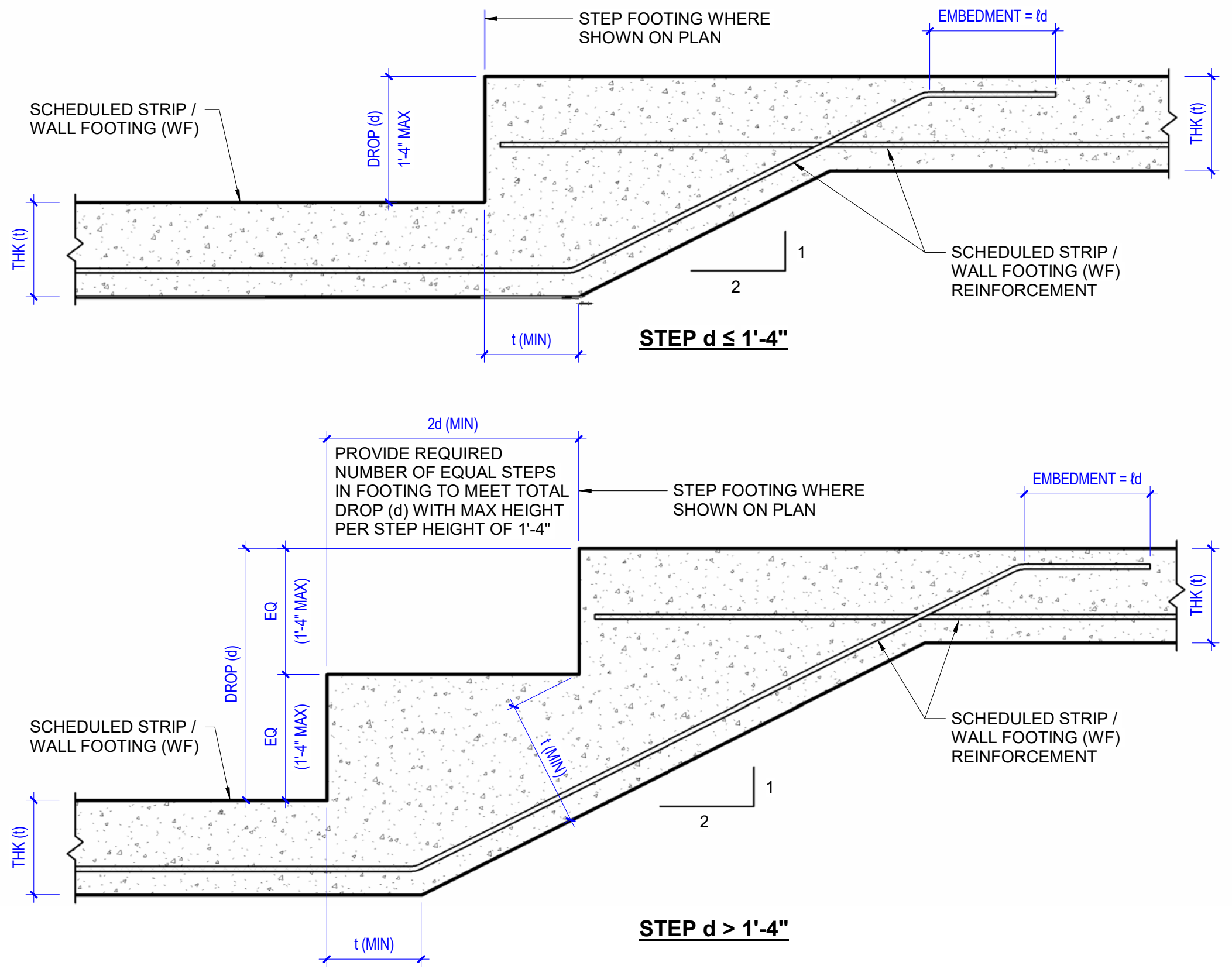
2 Concrete Wall Control Joint
3/4" = 1'-0"


VERTICAL BAR TO MATCH VERTICAL WALL REINFOCEMENT
LAP SPLICE = 1.3ℓd
LAP SPLICE = 1.3ℓd
CORNER BAR TO MATCH HORIZONTAL WALL REINFORCEMENT SIZE AND SPACING (HOOK MAY BE PROVIDED AT END OF CONTINUOUS HORIZONTAL REINFORCEMENT IN ONE DIRECTION IN PLACE OF CORNER BAR)
(4) VERTICAL BARS TO MATCH VERTICAL WALL REINFOCEMENT
LAP SPLICE = 1.3ℓd
LAP SPLICE = 1.3ℓd
(2) U-BAR TO MATCH HORIZONTAL WALL REINFORCEMENT SIZE AND SPACING
SINGLE LAYER OF REINFORCEMENT
REINFORCEMENT EACH FACE

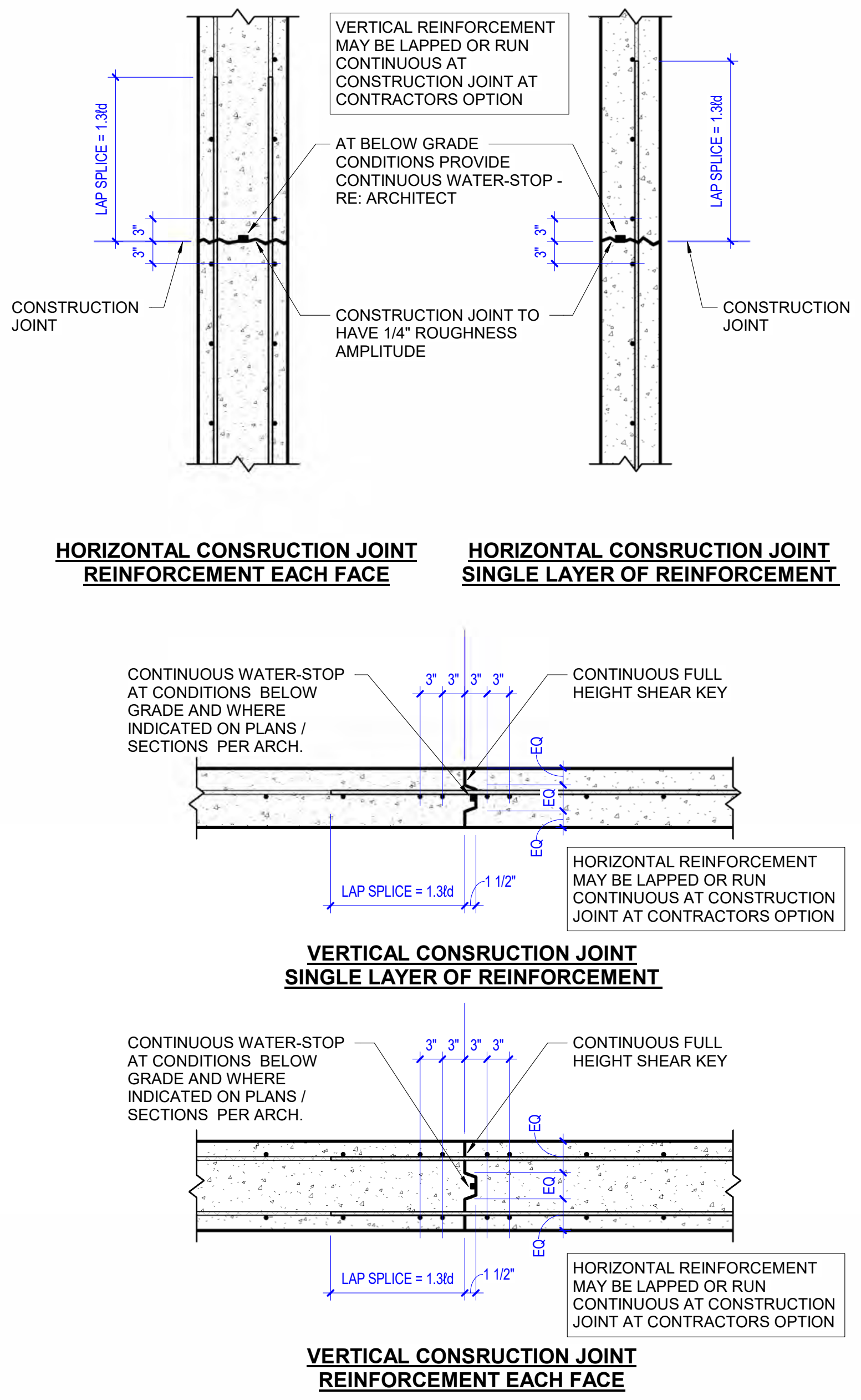
5 Concrete Wall Corner
3/4" = 1'-0"


3 SIDES
3/16"
PL 1/4X5X0'-5" EA. SIDE OF BEAM
SLAB ON GRADE
8"
ELEVATOR / SHAFT DIVIDER BEAM
3/16"
EMBED PLATE WP-B8x8.1

8 Elevator Sill Angle
1" = 1'-0"

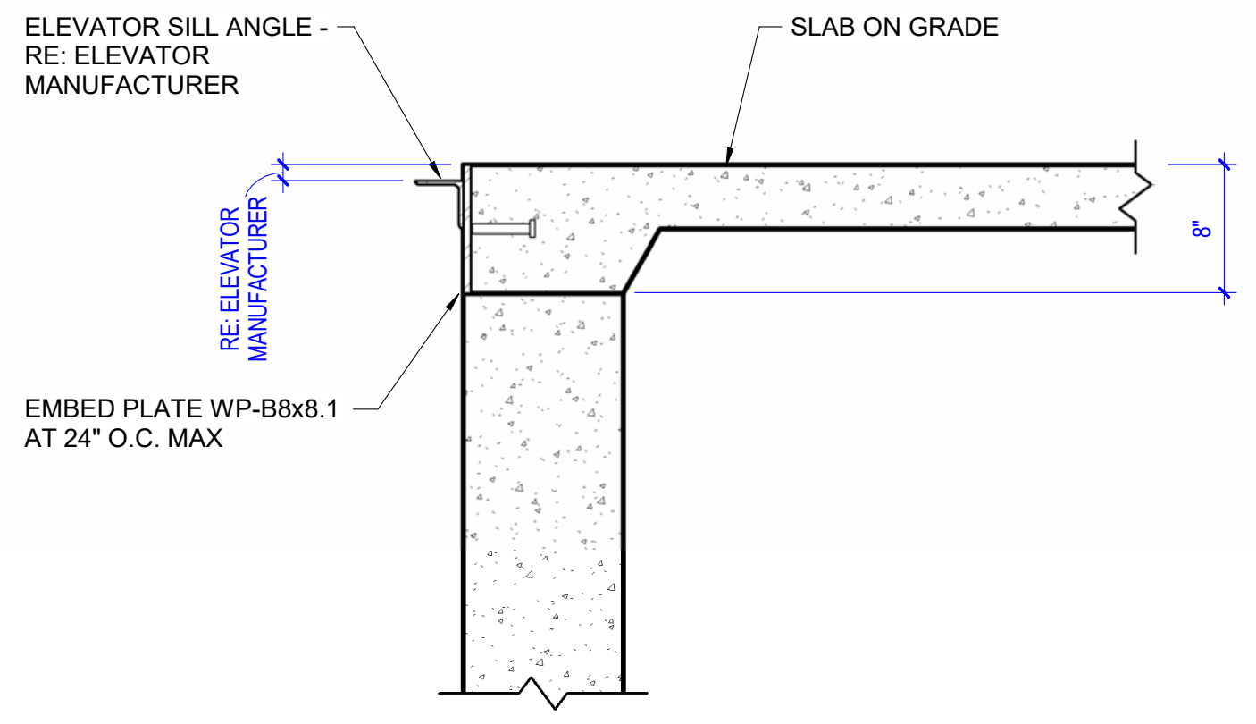

STEP FOOTING WHERE SHOWN ON PLAN
EMBEDMENT = ℓd
SCHEDULED STRIP / WALL FOOTING (WF)
DROP (d) 1'-4" MAX
THK (t)
THK (t)
SCHEDULED STRIP / WALL FOOTING (WF) REINFORCEMENT
1
2
t (MIN)
STEP d ≤ 1'-4"
2d (MIN)
PROVIDE REQUIRED NUMBER OF EQUAL STEPS IN FOOTING TO MEET TOTAL DROP (d) WITH MAX HEIGHT PER STEP HEIGHT OF 1'-4"
STEP FOOTING WHERE SHOWN ON PLAN
EMBEDMENT = ℓd
EQ (1'-4" MAX)
EQ (1'-4" MAX)
DROP (d)
SCHEDULED STRIP / WALL FOOTING (WF)
t (MIN)
THK (t)
THK (t)
SCHEDULED STRIP / WALL FOOTING (WF) REINFORCEMENT
1
2
t (MIN)
STEP d > 1'-4"

4 Typical Step in Strip / Wall Footing
3/4" = 1'-0"

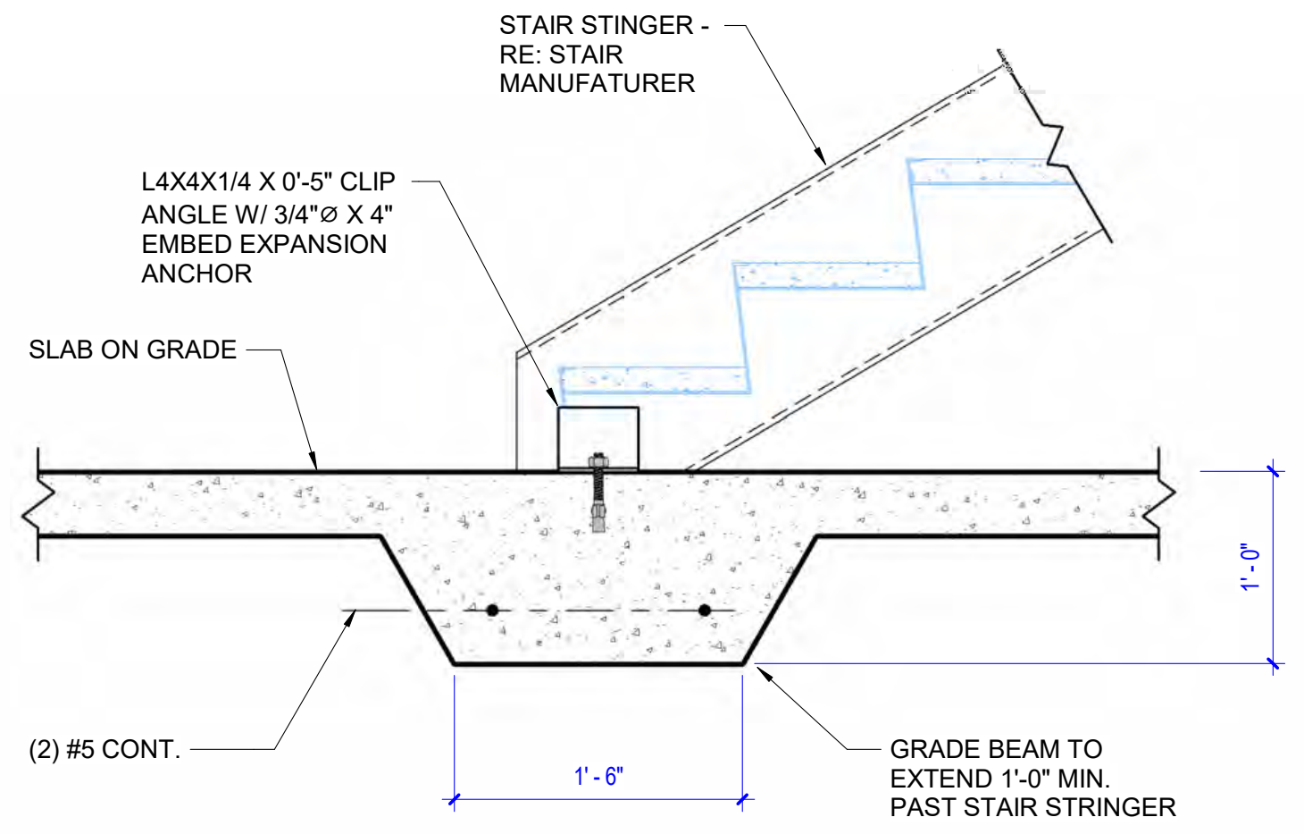

VERTICAL REINFORCEMENT MAY BE LAPPED OR RUN CONTINUOUS AT CONSTRUCTION JOINT AT CONTRACTORS OPTION
LAP SPLICE = 1.3ℓd
AT BELOW GRADE CONDITIONS PROVIDE CONTINUOUS WATER-STOP - RE: ARCHITECT
3"
3"
CONSTRUCTION JOINT
CONSTRUCTION JOINT TO HAVE 1/4" ROUGHNESS AMPLITUDE
CONSTRUCTION JOINT
HORIZONTAL CONSRUCTION JOINT REINFORCEMENT EACH FACE
HORIZONTAL CONSRUCTION JOINT SINGLE LAYER OF REINFORCEMENT
CONTINUOUS WATER-STOP AT CONDITIONS BELOW GRADE AND WHERE INDICATED ON PLANS / SECTIONS PER ARCH.
3" 3" 3" 3"
CONTINUOUS FULL HEIGHT SHEAR KEY
EQ
EQ
EQ
LAP SPLICE = 1.3ℓd
1 1/2"
HORIZONTAL REINFORCEMENT MAY BE LAPPED OR RUN CONTINUOUS AT CONSTRUCTION JOINT AT CONTRACTORS OPTION
VERTICAL CONSRUCTION JOINT SINGLE LAYER OF REINFORCEMENT
CONTINUOUS WATER-STOP AT CONDITIONS BELOW GRADE AND WHERE INDICATED ON PLANS / SECTIONS PER ARCH.
3" 3" 3" 3"
CONTINUOUS FULL HEIGHT SHEAR KEY
EQ
EQ
EQ
LAP SPLICE = 1.3ℓd
1 1/2"
HORIZONTAL REINFORCEMENT MAY BE LAPPED OR RUN CONTINUOUS AT CONSTRUCTION JOINT AT CONTRACTORS OPTION
VERTICAL CONSRUCTION JOINT REINFORCEMENT EACH FACE

NOTES:
1. AT CONDITIONS WHERE WALL SPANS HORIZONTALLY, CONTROL JOINTS TO BE LOCATED IN MIDDLE THIRD OF WALL SPANS (UNO).
2. CONTRACTOR TO SUBMIT CONTROL JOINT LOCATIONS PRIOR TO CONSTRUCTION TO EOR FOR REVIEW / APPROVAL.
3. RE: ARCHITECT FOR ADDITIONAL CONSTRUCTION JOINT REQUIREMENTS AND TREATMENT OF EXPOSED CONSTRUCTIO JOINTS.

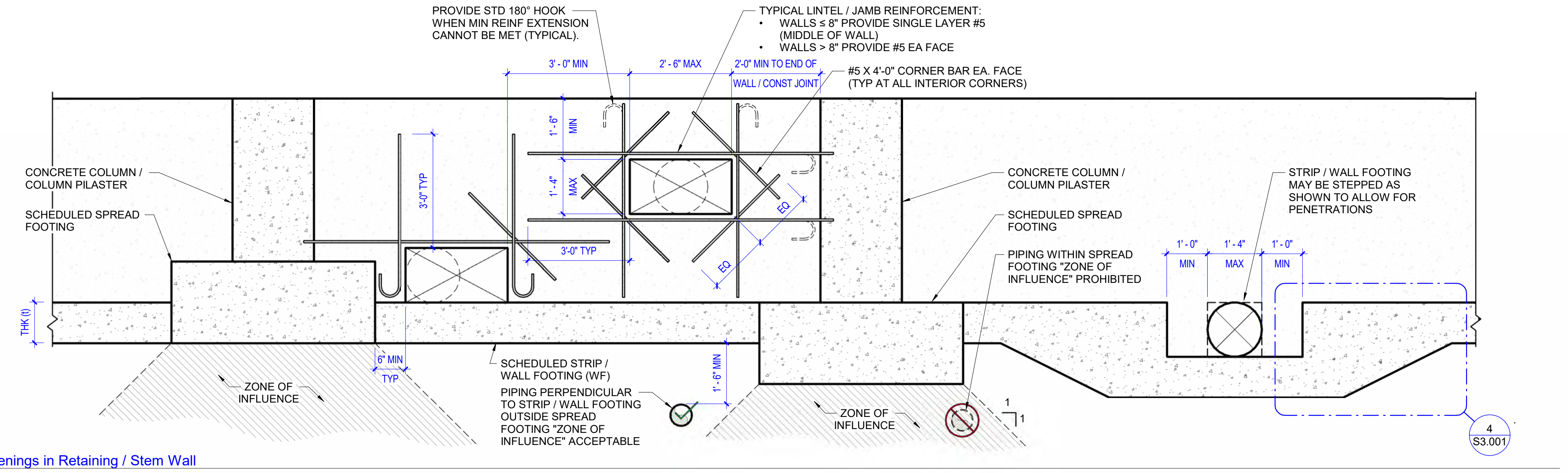
1 Concrete Wall Construction Joint
3/4" = 1'-0"


ELEVATOR SILL ANGLE - RE: ELEVATOR MANUFACTURER
SLAB ON GRADE
RE: ELEVATOR MANUFACTURER
8"
EMBED PLATE WP-B8x8.1 AT 24" O.C. MAX

7 Elevator Sill Angle
1" = 1'-0"


STAIR STINGER - RE: STAIR MANUFATURER
L4X4X1/4 X 0'-5" CLIP ANGLE W/ 3/4"Ø X 4" EMBED EXPANSION ANCHOR
SLAB ON GRADE
1' - 0"
(2) #5 CONT.
1' - 6"
GRADE BEAM TO EXTEND 1'-0" MIN. PAST STAIR STRINGER

10 Stair Stringer Attachment
1" = 1'-0"


PROVIDE STD 180° HOOK WHEN MIN REINF EXTENSION CANNOT BE MET (TYPICAL).
TYPICAL LINTEL / JAMB REINFORCEMENT:
• WALLS ≤ 8" PROVIDE SINGLE LAYER #5 (MIDDLE OF WALL)
• WALLS > 8" PROVIDE #5 EA FACE
3' - 0" MIN
2' - 6" MAX
2'-0" MIN TO END OF WALL / CONST JOINT
#5 X 4'-0" CORNER BAR EA. FACE (TYP AT ALL INTERIOR CORNERS)
1' - 6" MIN
1' - 4" MAX
3'-0" TYP
3'-0" TYP
EQ
EQ
CONCRETE COLUMN / COLUMN PILASTER
SCHEDULED SPREAD FOOTING
THK (t)
6" MIN TYP
ZONE OF INFLUENCE
SCHEDULED STRIP / WALL FOOTING (WF)
PIPING PERPENDICULAR TO STRIP / WALL FOOTING OUTSIDE SPREAD FOOTING "ZONE OF INFLUENCE" ACCEPTABLE
1' - 6" MIN
CONCRETE COLUMN / COLUMN PILASTER
SCHEDULED SPREAD FOOTING
PIPING WITHIN SPREAD FOOTING "ZONE OF INFLUENCE" PROHIBITED
ZONE OF INFLUENCE
1
1
1' - 0" MIN
1' - 4" MAX
1' - 0" MIN
STRIP / WALL FOOTING MAY BE STEPPED AS SHOWN TO ALLOW FOR PENETRATIONS
4 S3.001

3 Openings in Retaining / Stem Wall
1/2" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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	100% DD	09/30/2026

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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Typical Foundation Details

SHEET NUMBER:
S3.001

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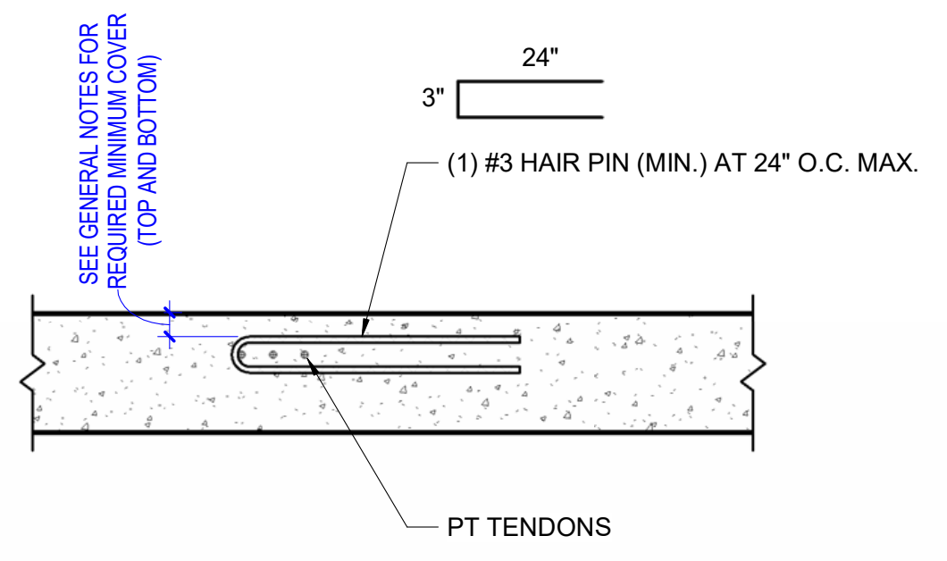
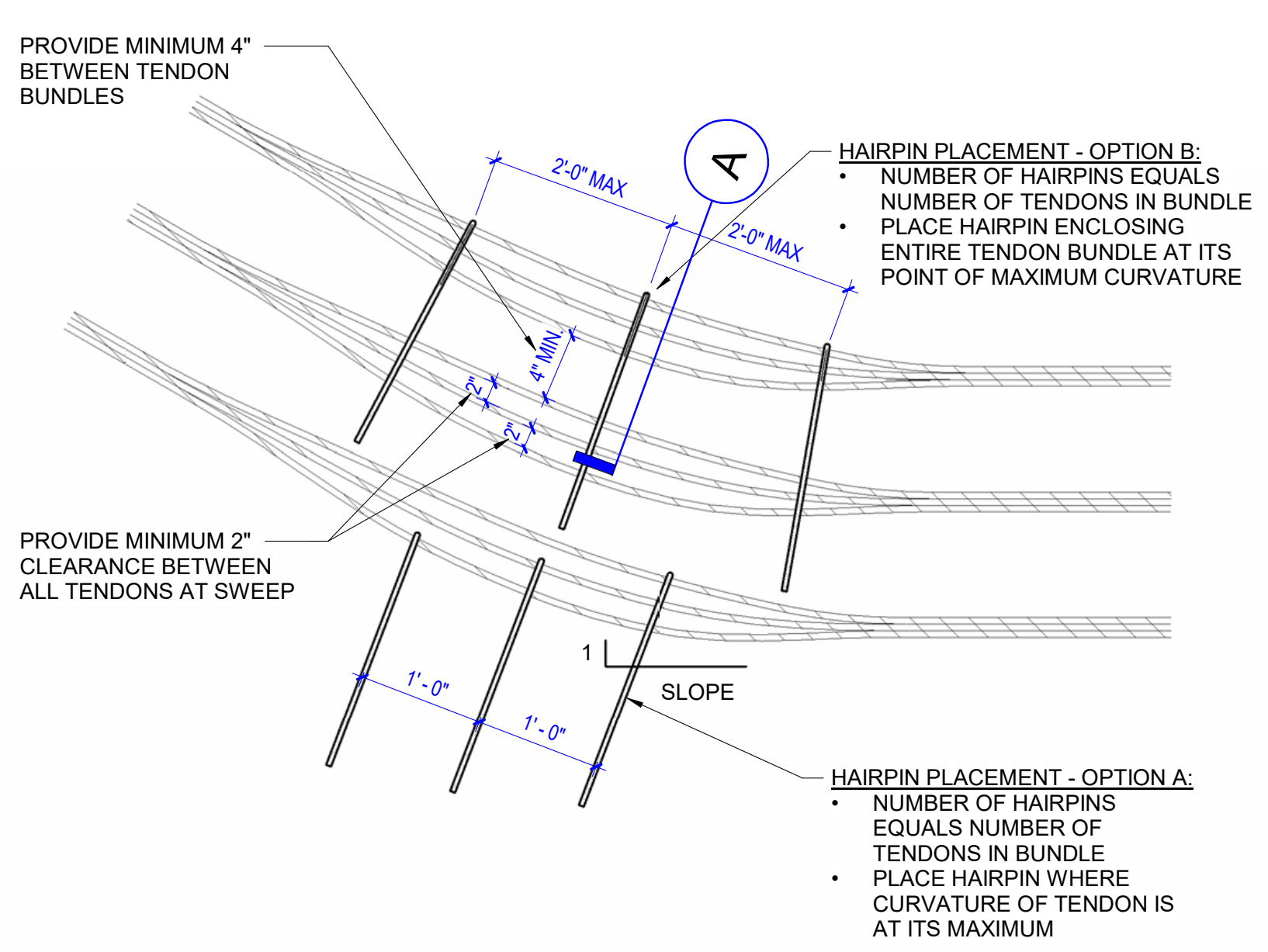

SEE GENERAL NOTES FOR REQUIRED MINIMUM COVER (TOP AND BOTTOM)
24"
3"
(1) #3 HAIR PIN (MIN.) AT 24" O.C. MAX.
PT TENDONS

A Detail at Hairpin

HAIRPIN PLACEMENT CHART			
TENDON SLOPE	PLACEMENT		
	PROCEDURE	OPTION	
LESS THAN 1:12	SPLAY TENDONS NOT REQUIRED	NO HAIRPINS REQUIRED	
1:12 - 1:6	SPLAY TENDONS	OPTION A OR OPTION B	

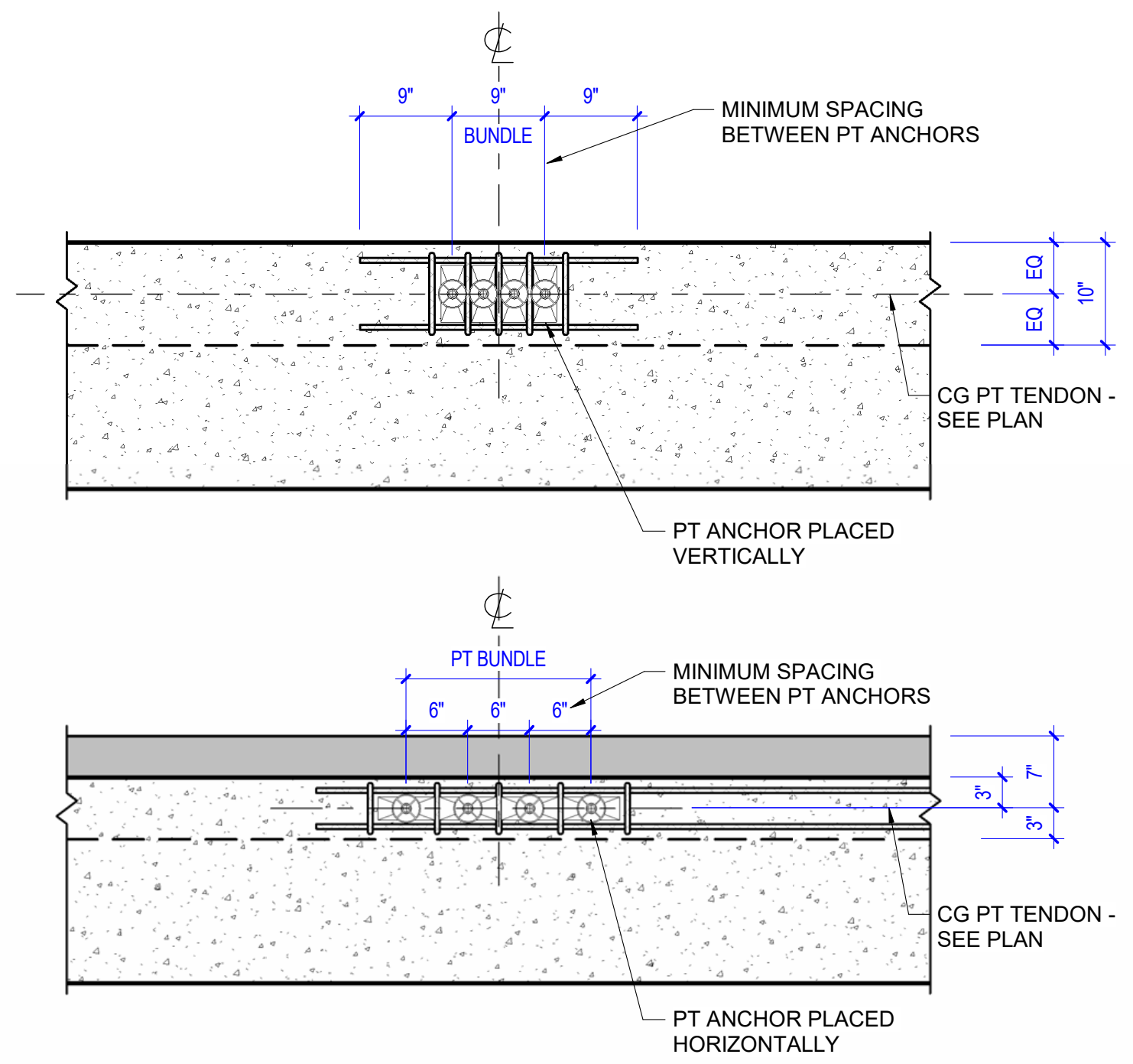
NOTE: FOR SLOPES GREATER THAN 1:6 REFERENCE PT SUPPLIER APPROVED SUBMITTAL


PROVIDE MINIMUM 4" BETWEEN TENDON BUNDLES
A
2'-0" MAX
2'-0" MAX
4" MIN.
2"
2"
PROVIDE MINIMUM 2" CLEARANCE BETWEEN ALL TENDONS AT SWEEP
1'-0"
1'-0"
1
SLOPE

HAIRPIN PLACEMENT - OPTION B:
- NUMBER OF HAIRPINS EQUALS NUMBER OF TENDONS IN BUNDLE
- PLACE HAIRPIN ENCLOSING ENTIRE TENDON BUNDLE AT ITS POINT OF MAXIMUM CURVATURE

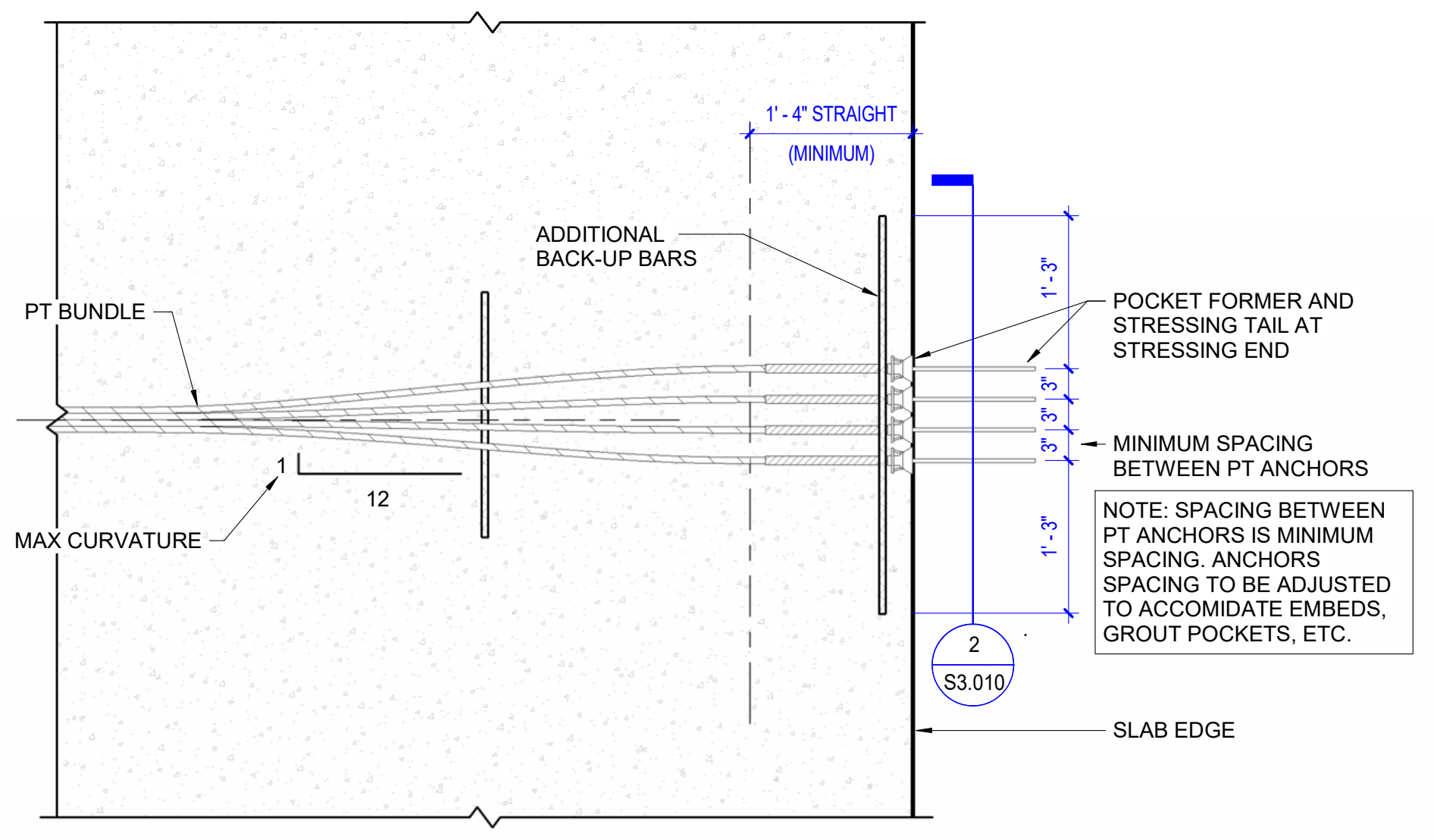
HAIRPIN PLACEMENT - OPTION A:
- NUMBER OF HAIRPINS EQUALS NUMBER OF TENDONS IN BUNDLE
- PLACE HAIRPIN WHERE CURVATURE OF TENDON IS AT ITS MAXIMUM

3 Tendon Curvature
3/4" = 1'-0"


℄
9"
9"
9"
BUNDLE
MINIMUM SPACING BETWEEN PT ANCHORS
EQ
EQ
10"
CG PT TENDON - SEE PLAN
PT ANCHOR PLACED VERTICALLY
℄
PT BUNDLE
6"
6"
6"
MINIMUM SPACING BETWEEN PT ANCHORS
7"
3"
3"
CG PT TENDON - SEE PLAN
PT ANCHOR PLACED HORIZONTALLY

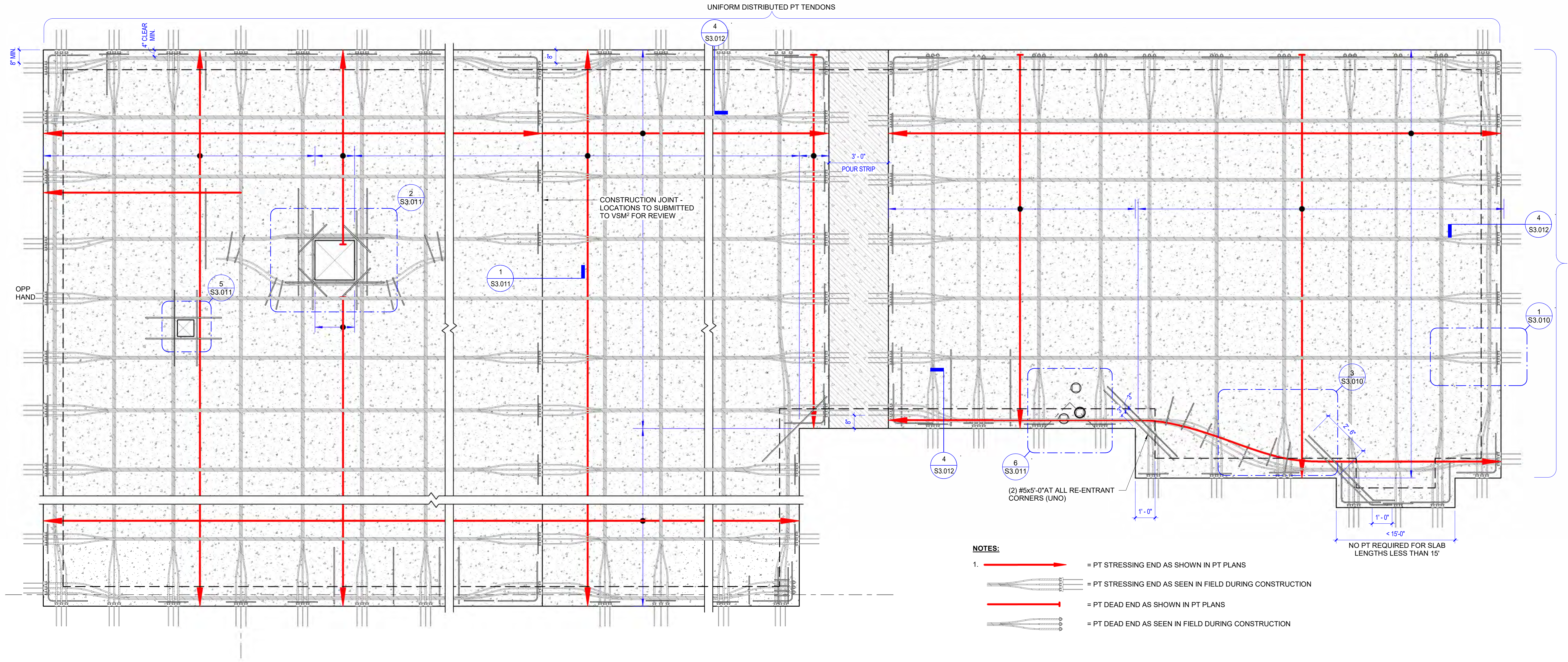
NOTE:
1. STRESSING SEQUENCE; START AT CENTER OF STRAND GROUP THEN GO TO RIGHT AND LEFT OF CENTER PER DIAGRAM ABOVE. CONTINUE THIS SEQUENCE UNTIL ALL TENDONS ARE STRESSED.
2. MINIMUM SPACING BETWEEN PT ANCHORS AND PT BUNDLES TO BE INCREASED AS REQUIRED TO AVOID CONFLICT WITH VERTICAL COLUMN REINFORCEMENT, EMBEDS, ETC.

2 PT Bundle Elevation at Slab Edge
3/4" = 1'-0"



NOTE: SPACING BETWEEN PT ANCHORS IS MINIMUM SPACING. ANCHORS SPACING TO BE ADJUSTED TO ACCOMIDATE EMBEDS, GROUT POCKETS, ETC.

1 Flaring of Tendons at Slab Edge
3/4" = 1'-0"


UNIFORM DISTRIBUTED PT TENDONS
4
S3.012
8" MIN.
4" CLEAR MIN.
2
S3.011
CONSTRUCTION JOINT - LOCATIONS TO SUBMITTED TO VSM² FOR REVIEW
3'-0"
POUR STRIP
1
S3.011
5
S3.011
OPP HAND
4
S3.012
UNIFORM DISTRIBUTED PT TENDONS
1
S3.010
3
S3.010
4
S3.012
6
S3.011
(2) #5x5'-0"AT ALL RE-ENTRANT CORNERS (UNO)
1'-0"
2'-6"
1'-0"
< 15'-0"
NO PT REQUIRED FOR SLAB LENGTHS LESS THAN 15'

NOTES:

1. ⟶ = PT STRESSING END AS SHOWN IN PT PLANS

⋲ = PT STRESSING END AS SEEN IN FIELD DURING CONSTRUCTION

━┫ = PT DEAD END AS SHOWN IN PT PLANS

⋲ = PT DEAD END AS SEEN IN FIELD DURING CONSTRUCTION

FN3.0 STRUCTURAL POST TENSIONED SLAB ON GRADE (UNIFORM SLAB THICKNESS):

MATERIALS:

1. CONCRETE:
   - REFER TO GENERAL NOTES FOR ALL POST-TENSIONED SLAB ON GRADE MINIMUM COMPRESSIVE STRENGTH.
2. REINFORCEMENT:
   - PRESTRESSED REINFORCEMENT: TENDONS SHALL CONFORM TO PTI M10.6-15 AND BE FULLY ENCAPSULATED REGARDLESS OF EXPSOURE ENVIRONMENT.
   - ALLOWABLE STRESSES: AT JACKING FORCE, TENDONS SHALL NOT EXCEED 0.94fpy OR 0.80fpu
   - IMMEDIATELY AFTER PRESTRESS TRANSFER, TENSILE STRESS AT ANCHORAGE DEVICES SHALL NOT EXCEED 0.70fpu
   - DEFORMED REINFORCEMENT SHALL CONFORM TO ASTM A615 OR A706 GRADE 60 UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.
   - WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185
   - COVER TO REINFORCEMENT: MINIMUM CONCRETE COVER TO TENDONS (EXCLUDNG ANCHORS AND STRAND TAILS) AND NON-PRESTRESSED REINFORCEMENT SHALL BE AS FOLLOWS:
     a. RIBS
        - TOP: 1 INCH
        - BOTTOM: 3 INCHES
        - SIDES: 2.5 INCHES
     b. SLABS
        - TOP: 1 INCH
        - BOTTOM: 1.5 INCHES
        - BOTTOM (UTF): 3 INCHES
        - SIDES (UFT): 3 INCHES

ANCHORS:

1. BEARING STRESSES ON CONCRETE CREATED BY ANCHORS SHALL NOT EXCEED THE MAXIMUM PERMISSIBLE VALUES PER PTI DC10.5-19 CONSIDERING AT TRANSFER OF PRESTRESS FORCE AND AFTER ALL PRESTRESS LOSS CONDITIONS.
2. ANCHORAGES AND COUPLERS SHALL CONFORM TO PTI SPECIFICATIONS FOR SINGLE-STRAND UNBONDED TENDONS SECTIONS 2.2. AND 6.2.

DURABILITY:

1. FOUNDATION CONCRETE EXPOSED TO FREEZE AND THAW CYCLES OR TO DEICING CHEMICALS SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS. REFER TO CONCRETE SPECIFICATION FOR MORE INFORMATION.

DURABILITY (CONTINUED):

2. CONCRETE IN DIRECT CONTACT WITH SOIL CONTAINING WATER-SOLUBLE SULFATES OR CHLORIDES SHALL CONFORM TO THE FOLLOWING:
   A. SOIL SULFATES: FOR SOIL SULFATE CONCENTRATONS GREATER THAN OR EQUAL TO 0.1% BUT LESS THAN 0.2% BY WEIGHT, CONCRETE SHALL BE MADE WITH TYPE II OR TYPE IV CEMENT.
   B. FOR SOIL SULFATE CONCENTRATIONS EQUAL TO OR GREATER THAN 0.2% BY WEIGHT, CONCRETE SHALL BE MADE WITH TYPE V CEMENT (OR APPROVED EQUIVALENT) AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI @ 28 DAYS.
   C. CONCENTRATIONS OF WATER-SOLUBLE SOIL SULFATES SHALL BE DETERMINED BY CALIFORNIA DEPARTMENT OF TRANSPORTATION TEST 417 OR ANOTHER CURRENT TEST METHOD RECOGNIZED BY THE LOCAL BUILDONG CODE OR COMMONLY USED AT OR NEAR THE PROJECT LOCATION.
   D. SOIL CHLORIDES. WHEN CONCRETE IS IN DIRECT CONTACT WITH SOIL CONTAINING A LEVEL OF CHLORIDE IONS THAT IS KNOWN TO HAVE CAUSED TENDON FAILURE DUE TO CORROSION IN THE LOCAL AREA AS DETERMINED BY LOCAL EXPERIENCE AND PRACTICE, TENDONS AND REINFORCING STEEL SHALL BE PROTECTED FROM CORROSION BY INCREASING CONCRETE COVER BASED ON CHLORIDE CONCENTRATION MEASURED IN PPM AND / OR OTHER MEANS AS APPROVED BY THE SEOR AND IN CONFORMANCE WITH ACI 222.3R-03:
      - 500 TO 5000 PPM: 3 INCHES
      - 5001 TO 10,000 PPM: 4 INCHES
      - OVER 10,000: 5 INCHES

UNIFORM THICKNESS POST TENSION SLAB ON GRADE FOUNDATION (UTF):

1. THE UTF HAS BEEN DESIGNED USING A (3) DIMENSIONAL FINITE ELEMENT ANALYSIS WITH A FINAL RESIDUAL COMPRESSIVE STRESS OF 150 PSI. ALL PRESTRESS LOSSES INCLUDING, BUT NOT LIMITED TO, INTERNAL FRICTION, ELASTIC SHORTENING, SEATING LOSSES AND LONG TERM LOSSES SHALL BE INCLUDED IN THE POST TENSION SHOP DRAWING CALCULATION SUBMITTAL FOR REVIEW AND APPROVAL BY THE SEOR PRIOR TO FABRICATION.
2. SUBGRADE FRICTION LOSSES CONSIDER (2) LAYERS OF 6 MIL MIN. POLYTHYLENE SHEETING WITH A COEFFICIENT OF FRICTION OF 0.75 at CONDITIONS WHERE THE UTF SLAB IS CAST DIRECTLY OVER A 2" MINIMUM THICK SAND LAYER, THE COEFFICIENT OF FRICTION SHALL BE TAKEN AS 1.0. ANY OTHER SUBGRADE MATERIAL MUST BE REVIEWED AND APPROVED BY BOTH THE GEOTECHNICAL ENGINEER AND STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION START. SEE SUBGRADE FRICTION LOSS TABLES FOR REQUIRED ADDED FORCE BASED ON SLAB ON GRADE THICKNESS AND TOTAL LENGTH OF PT TENDONS MEASURED FROM DEAD/STRESSING END TO DEAD/STRESSING END.

UNIFORM THICKNESS POST TENSION SLAB ON GRADE FOUNDATION (UTF) CONTINUED:

3. REFER TO ELEVATED POST TENSION SLAB GENERAL NOTES FOR REQUIRED SUBMITTALS, CONCRETE, TENDON PLACEMENT, STRESSING, GROUTING, TESTING AND INSPECTION NOT LISTED HEREIN.
4. THE CONCRETE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION OF ALL ANCHOR BOLTS, HOLDOWNS ANCHORS, TIE-DOWN ANCHORS, POST ANCHORS AND OTHER CAST-IN OR EMBEDDED ITEMS WITH THE APPROVED ARCHITECTURAL PLANS PRIOR TO WORK START. ALL EMBEDDED ITEM LOCATIONS SHALL BE CROSS-REFERENCED WITH THE STRUCTURAL DRAWINGS. WHERE CONFLICTS ARE ENCOUNTERED, THE CONTRACTOR SHALL BRING THE CONFLICT(S) TO THE ATTENTION OF THE ARCHITECT AND SEOR FOR REVIEW AND COORDINATION WITH THE CONSTRUCTION TEAM.
5. THE UTF DESIGN DOES NOT ALLOW FOR IMPROPER DRAINAGE, TREES TOO CLOSE TO THE FOUNDATION EDGE OR IMPROPER MAINTENANCE.
6. ALL SURFACE WATER MUST DRAIN AWAY FROM SLAB AND NO PONDING SHALL BE ALLOWED ANYWHERE ALONG THE FOUNDATION EDGE. IN ADDITION, ORIGINAL GRADING SHALL BE MAINTAINED AND COORDINATED WITH CURRENT AND EXISTING LANDSCAPE FEATURES.
7. THE SOIL UNDER PERIMETER BEAMS AND/O SLAB TURNDOWNS SHALL BE NEAR OPTIMUM MOISTURE CONTENT PRIOR TO CONCRETE PLACEMENT AND SHALL BE VERIFIED BY THE GEOTECHNICAL ENGINEER IN WRITING.
8. THE CONTRACTOR SHALL DAMPEN ANY SAND/MEMBRANE AS REQUIRED JUST PRIOR TO CONCRETE PLACEMENT IN AN EFFORT TO MINIMIZE MOISTURE TRANSFER BETWEEN THE CONCRETE AND THE SAND / MEMBRANE.
9. CONTRACTOR TO PROVIDE MINIMUM OF (2) LAYERS OF POLYETHYLENE SHEETING (6 MIL MIN.) UNDER STRUCTURAL SLAB ON GRADE WITH MINIMUM 12 INCH OVERLAP AT ENDS / SEAMS FASTENED TOGETHER WITH APPROVED 2 INCH MINIMUM WIDE ADHESIVE TAPE.
10. CONTRACTOR MUST VERIFY ALL SLAB EDGES, SLAB STEPS, OFFSETS, RECESSES, DEPRESSIONS, BRICK LEDGES, BLOCKOUTS, ETC. WITH THE APPROVED ARCHITCTURAL PLANS AND NOTIFY ARCHITECT AND SEOR OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION FOR REVIEW.
11. CONTRACTOR MUST ALSO COORDINATE AND ALL MEP OPENINGS, BLOCKOUTS, PENETRATIONS AND EMBEDDED CONDUITS, INSERTS, ETC. WITH MEP, ARCHITECTURAL AND STRUCTURAL DRAWINGS AS REQUIRED PRIOR TO CONSTRUCTION.
12. EXCESS RUNOFF SHALL NOT BE COLLECTED AND DISPOSED OF BY MEANS OF DISCHARGE PIPE BELOW THE STRUCTURAL FOUNDATION. ANY AND ALL CLAY PLUGS SHALL BE PLACED AT EDGE OF FOUNDATION AND FILLED WITH GRANULAR MATERIAL OR AS SPECIFIED BY THE GEOTECHNICAL ENGINEER AND / OR REPORT.

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VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
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SHEET TITLE:
PT Slab on Grade Foundation General Notes / Typical Details

SHEET NUMBER:
S3.010

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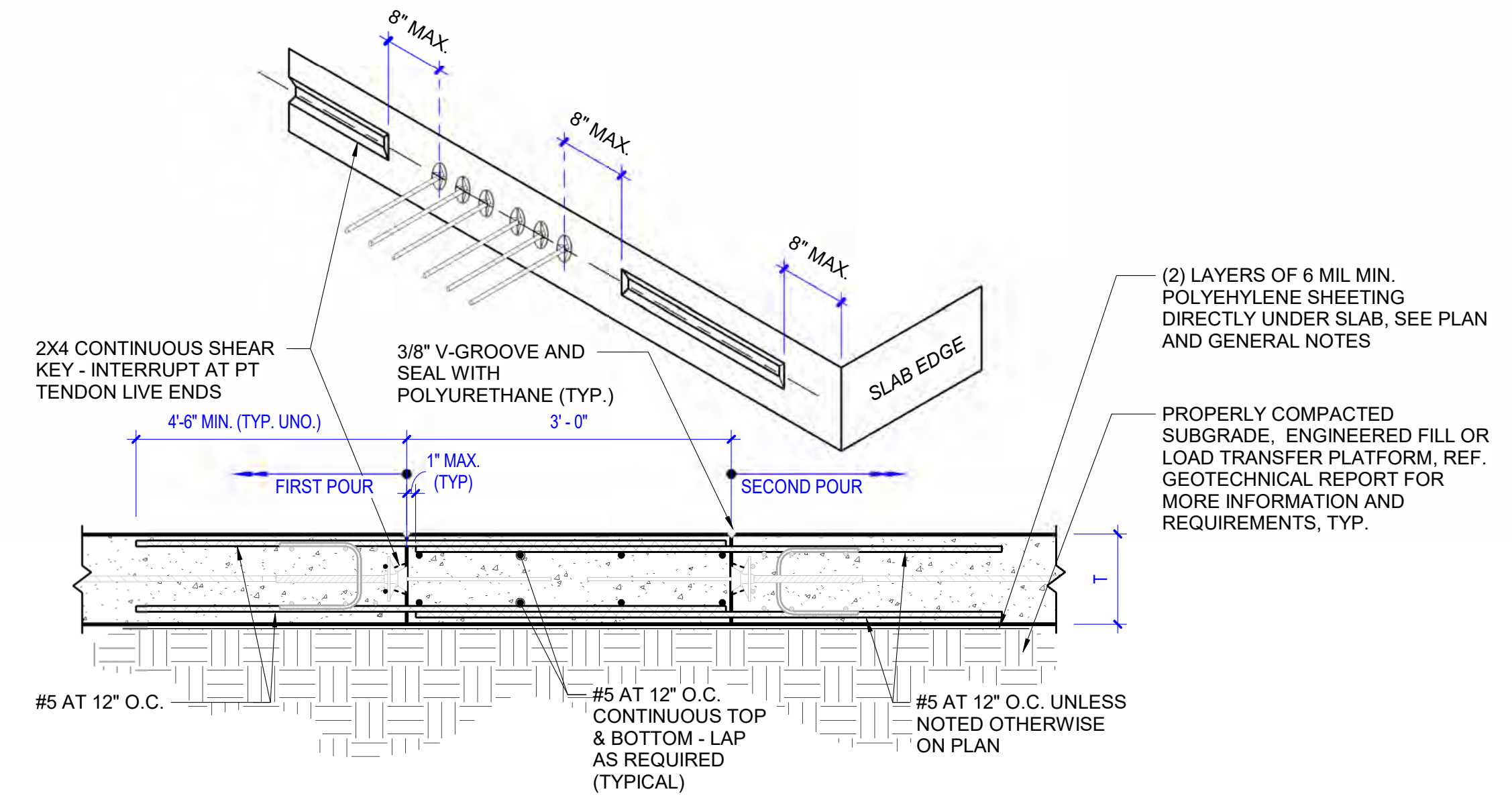
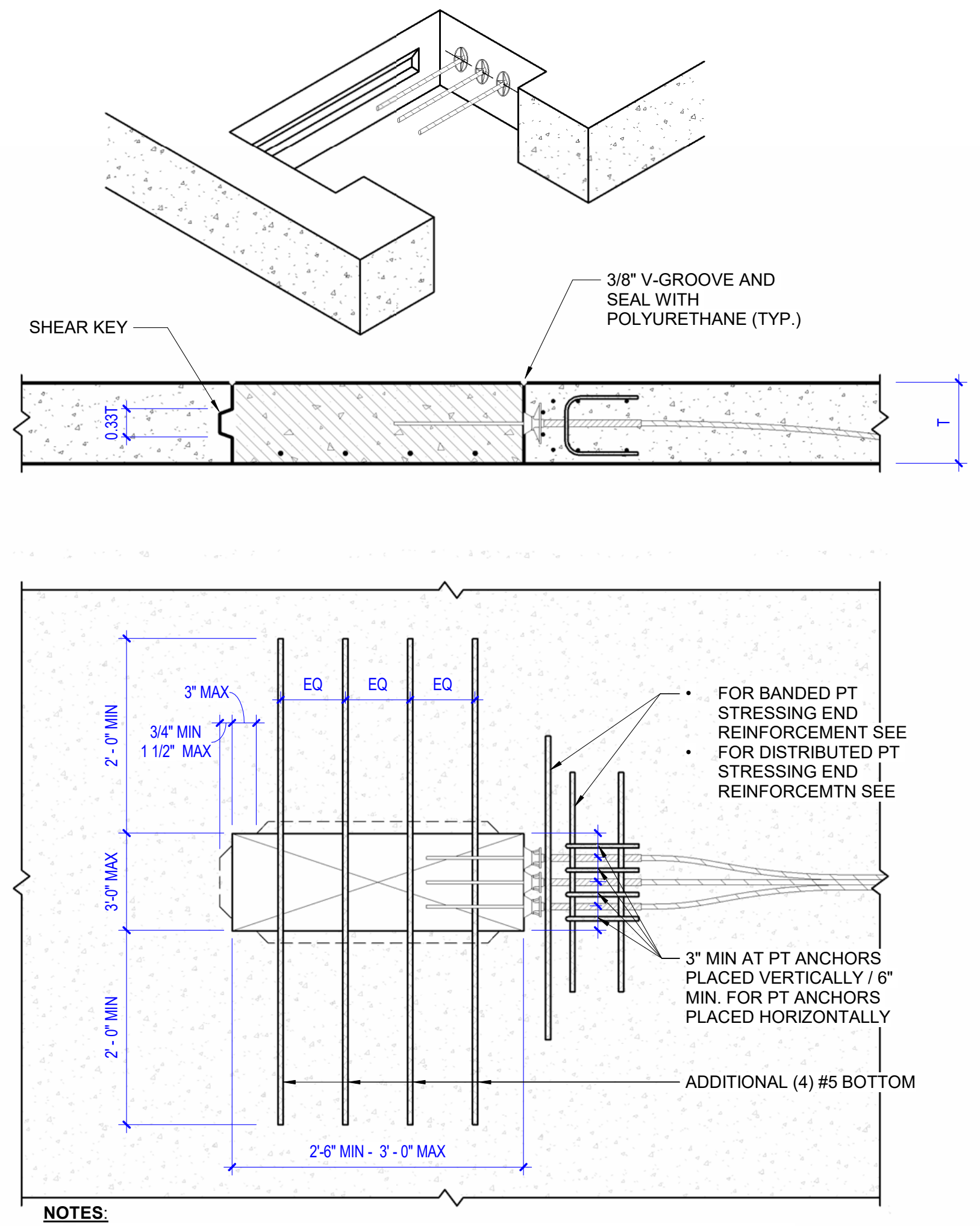

8" MAX.
8" MAX.
8" MAX.
SLAB EDGE
2X4 CONTINUOUS SHEAR KEY - INTERRUPT AT PT TENDON LIVE ENDS
3/8" V-GROOVE AND SEAL WITH POLYURETHANE (TYP.)
4'-6" MIN. (TYP. UNO.)
3' - 0"
1" MAX. (TYP)
FIRST POUR
SECOND POUR
T
(2) LAYERS OF 6 MIL MIN. POLYEHYLENE SHEETING DIRECTLY UNDER SLAB, SEE PLAN AND GENERAL NOTES
PROPERLY COMPACTED SUBGRADE, ENGINEERED FILL OR LOAD TRANSFER PLATFORM, REF. GEOTECHNICAL REPORT FOR MORE INFORMATION AND REQUIREMENTS, TYP.
#5 AT 12" O.C.
#5 AT 12" O.C. CONTINUOUS TOP & BOTTOM - LAP AS REQUIRED (TYPICAL)
#5 AT 12" O.C. UNLESS NOTED OTHERWISE ON PLAN

NOTES:

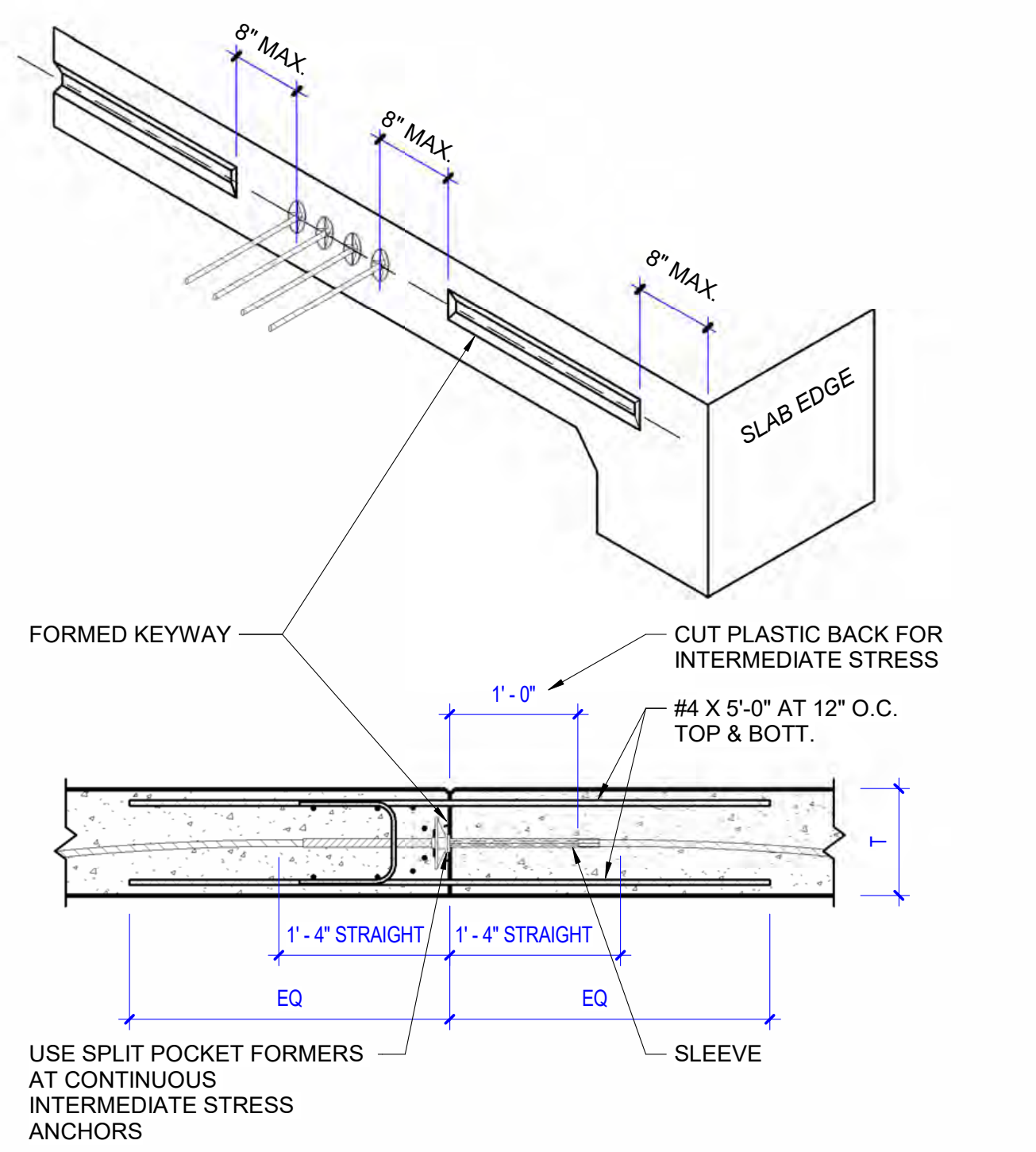
1. POUR STRIP TO BE PLACED WITH LOW-SHRINK CONCRETE. REFER TO GENERAL NOTES FOR LOW-SHRINK CONCRETE REQUIREMENTS.
2. POUR STRIP MAY BE INSTALLED IMMEDIATELY AFTER SECOND POUR TENDONS TAILS HAVE BEEN APPROVED FOR CUTTING BY THE EOR. LONGER DELAYS FOR INSTALLATION OF POUR STRIP CONCRETE ARE STRUCTURALLY ACCEPTABLE AS NEEDED TO SUIT CONSTRUCTION.
3. ROUGHEN, CLEAN AND PREPARE JOINTS TO A SATURATED SURFACE DRY CONDITION PRIOR TO POUR STRIP CONCRETE PLACEMENT, TYP.
4. ELIMINATE ACCIDENTAL MISALIGNMENT BETWEEN EDGE OF SLABS THAT ARE TO BE JOINED WITH POUR STRIP. USE MECHANICAL METHODS SUCH AS JACKING AS NECESSARY.
5. PROVIDE WATERPROOFING MEMBRANE IF REQUIRED AND REF. ARCHITECTURAL DRAWINGS FOR MORE INFO.
6. CUT STRESSED TENDON TAILS WITHIN (24) HOURS AFTER RECEIVING EOR APPROVAL AND GROUT STRESSING POCKETS WITHIN (8) HOURS OF CUTTING STRESSED TENDON TAILS, TYP. REFER TO GENERAL NOTES FOR ADDITIONAL REQUIREMENTS AND INFORMATION WHEN THESE CONDITIONS CANNOT BE MET.

7 Section at PT Slab on Grade Pour Strip
3/4" = 1'-0"


SHEAR KEY
3/8" V-GROOVE AND SEAL WITH POLYURETHANE (TYP.)
0.33T
T
3" MAX.
3/4" MIN
1 1/2" MAX
EQ
EQ
EQ
2' - 0" MIN
3'-0" MAX
2' - 0" MIN
• FOR BANDED PT STRESSING END REINFORCEMENT SEE
• FOR DISTRIBUTED PT STRESSING END REINFORCEMTN SEE
3" MIN AT PT ANCHORS PLACED VERTICALLY / 6" MIN. FOR PT ANCHORS PLACED HORIZONTALLY
ADDITIONAL (4) #5 BOTTOM
2'-6" MIN - 3' - 0" MAX

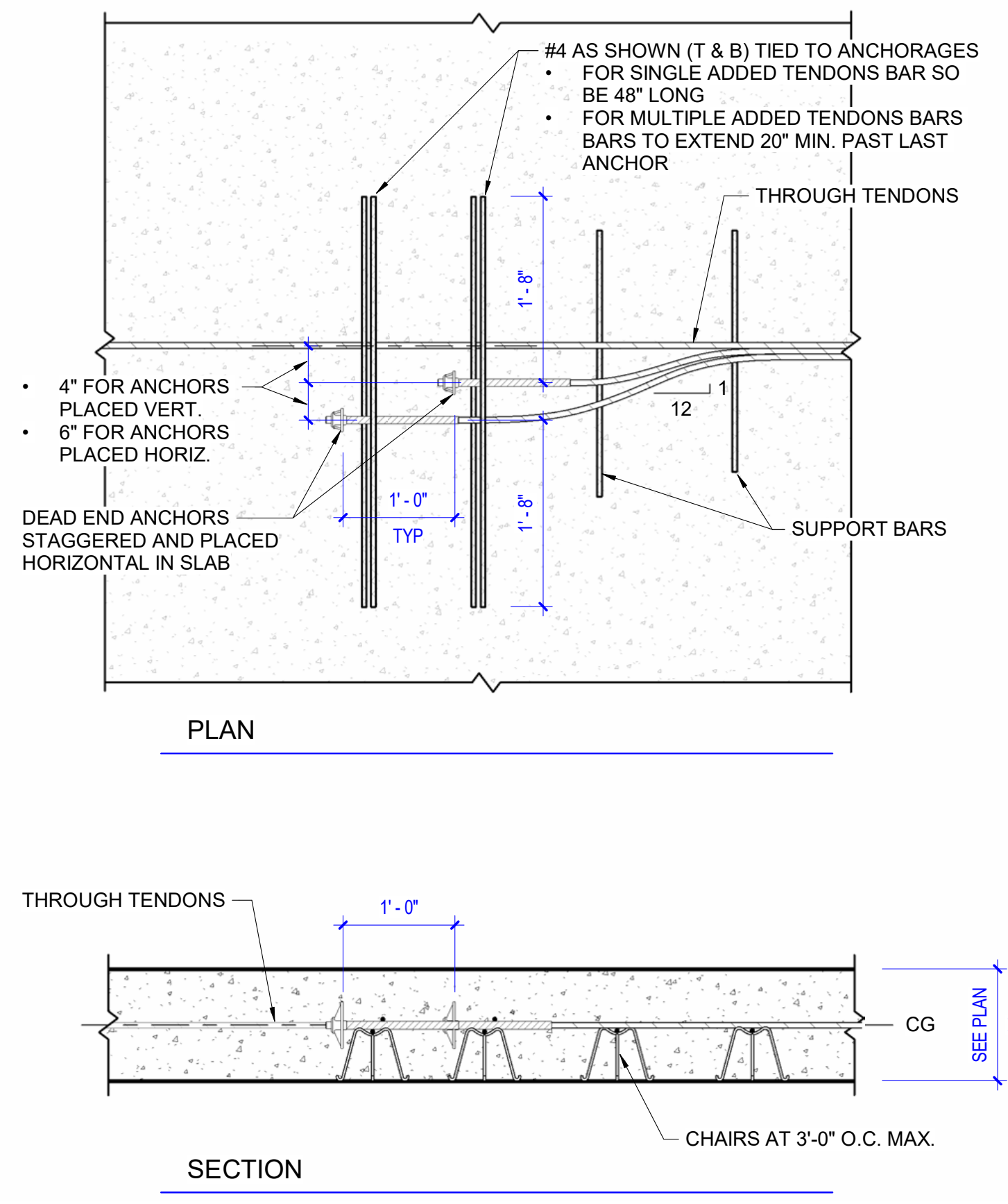
NOTES:
1. FINISH TENDONS AND FILL STRESSING POCKETS PER POST-TENISONED CONCRETE REQUIREMENTS IN GENERAL NOTES
2. CONTINUE ALL REINFORCEMENT SHOWN IN PLANS THROUGH THE BLOCKOUT. IF TOP BARS PREVENT STRESSING RAM ACCESS, CONSULT SEOR.
3. FILL BLOCKOUT W/ CONCRETE SPEIFIED FOR SLAB. BLOCKOUT MAY BE FILLED IMMEDIATELY AFTER STRESSING AND FINISHING TENDONS.

4 PT Stressing Blockout
3/4" = 1'-0"

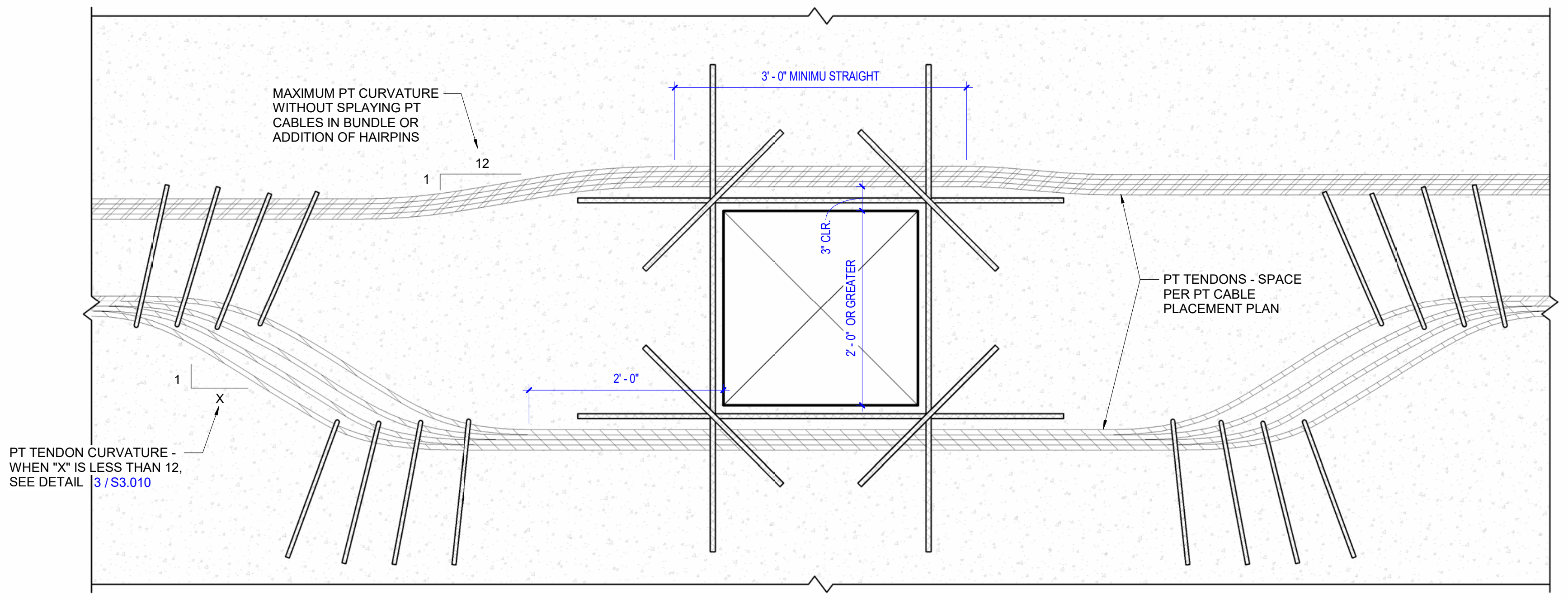

8" MAX.
8" MAX.
8" MAX.
SLAB EDGE
FORMED KEYWAY
CUT PLASTIC BACK FOR INTERMEDIATE STRESS
1' - 0"
#4 X 5'-0" AT 12" O.C. TOP & BOTT.
T
1' - 4" STRAIGHT
1' - 4" STRAIGHT
EQ
EQ
USE SPLIT POCKET FORMERS AT CONTINUOUS INTERMEDIATE STRESS ANCHORS
SLEEVE

NOTE:
LOCATIONS OF PROPOSED SLAB CONSTRIUCTION JOINTS TO BE TO BE SUBMITED TO VSMsq BY CONTACTOR FOR APPROVAL

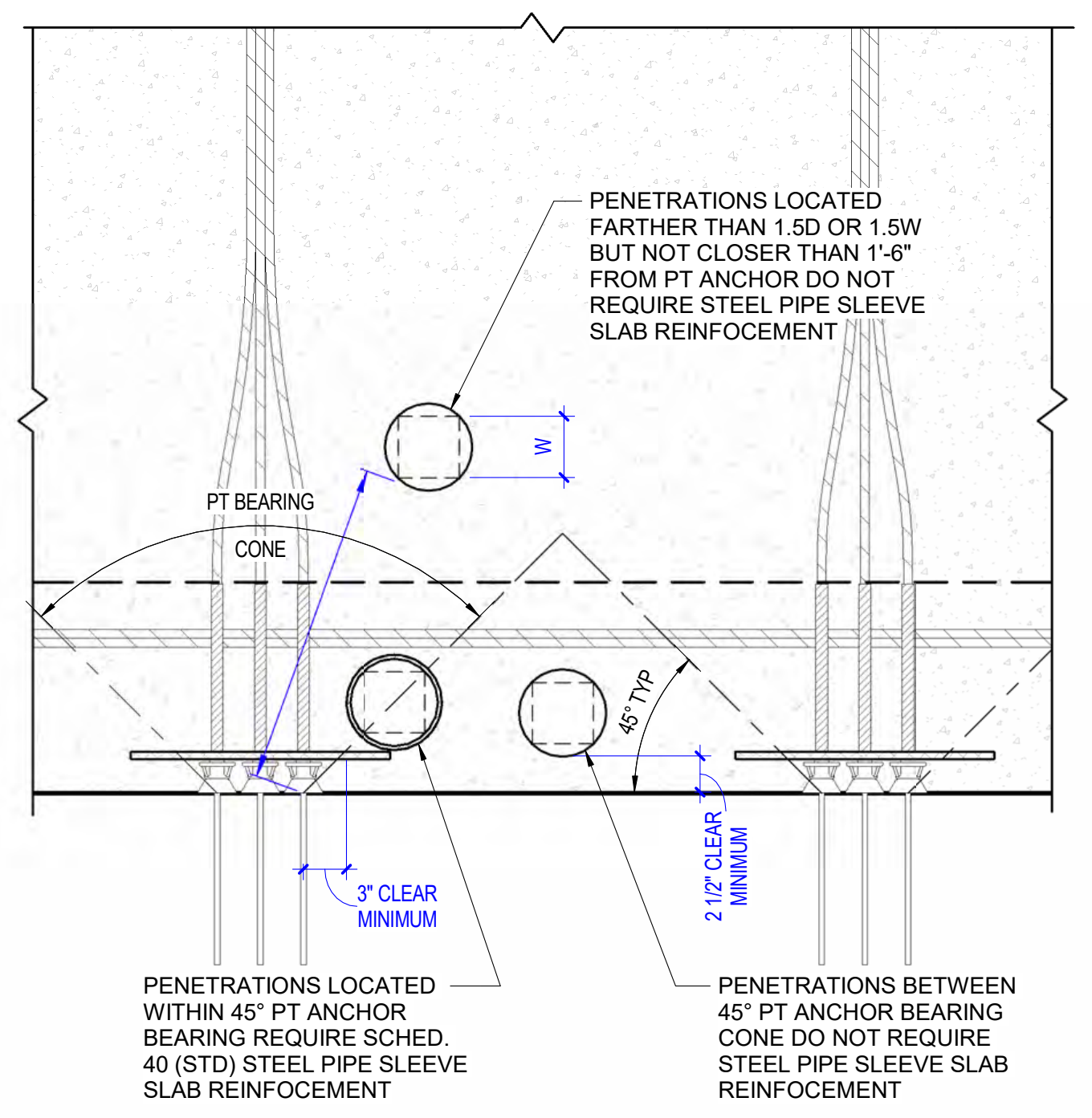
1 Uniform Thickness Foundation Typical Slab Construction Joint
3/4" = 1'-0"


#4 AS SHOWN (T & B) TIED TO ANCHORAGES
• FOR SINGLE ADDED TENDONS BAR SO BE 48" LONG
• FOR MULTIPLE ADDED TENDONS BARS BARS TO EXTEND 20" MIN. PAST LAST ANCHOR
THROUGH TENDONS
1' - 8"
1' - 8"
1
12
• 4" FOR ANCHORS PLACED VERT.
• 6" FOR ANCHORS PLACED HORIZ.
DEAD END ANCHORS STAGGERED AND PLACED HORIZONTAL IN SLAB
1' - 0"
TYP
SUPPORT BARS
PLAN
THROUGH TENDONS
1' - 0"
CG
SEE PLAN (T)
CHAIRS AT 3'-0" O.C. MAX.
SECTION

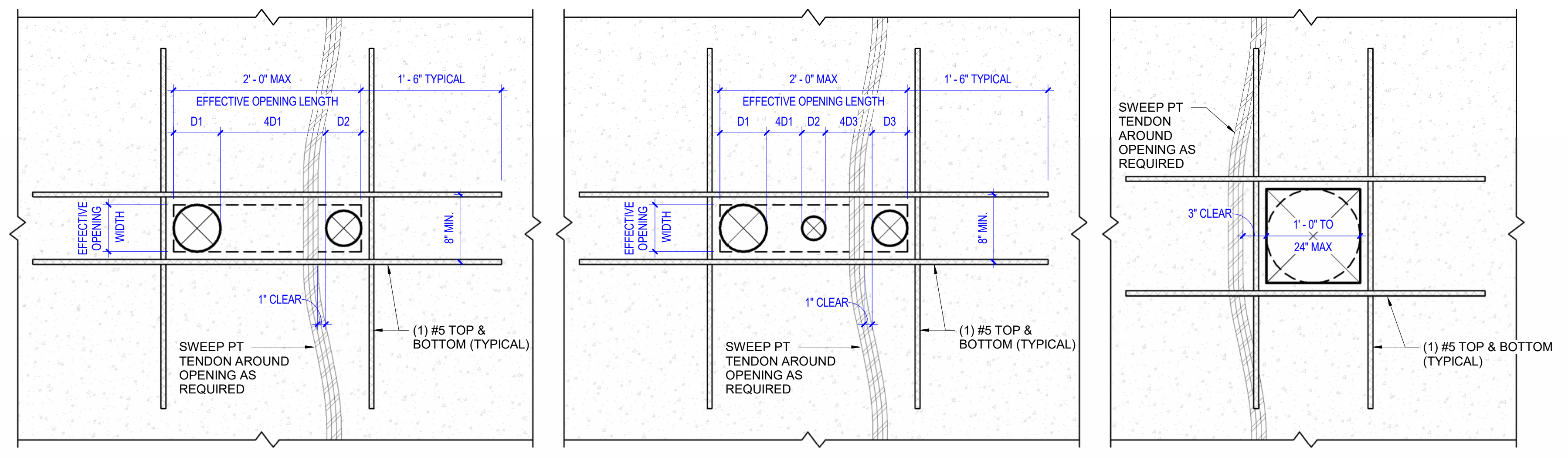
3 Placement of Added Tendon
3/4" = 1'-0"


3' - 0" MINIMU STRAIGHT
MAXIMUM PT CURVATURE WITHOUT SPLAYING PT CABLES IN BUNDLE OR ADDITION OF HAIRPINS
12
1
3" CLR.
2' - 0" OR GREATER
PT TENDONS - SPACE PER PT CABLE PLACEMENT PLAN
2' - 0"
1
X
PT TENDON CURVATURE - WHEN "X" IS LESS THAN 12, SEE DETAIL 3 / S3.010

2 Tendon Placement at Opening 24" or Larger
1" = 1'-0"


PENETRATIONS LOCATED FARTHER THAN 1.5D OR 1.5W BUT NOT CLOSER THAN 1'-6" FROM PT ANCHOR DO NOT REQUIRE STEEL PIPE SLEEVE SLAB REINFOCEMENT
W
PT BEARING
CONE
45° TYP
3" CLEAR MINIMUM
2 1/2" CLEAR MINIMUM
PENETRATIONS LOCATED WITHIN 45° PT ANCHOR BEARING REQUIRE SCHED. 40 (STD) STEEL PIPE SLEEVE SLAB REINFOCEMENT
PENETRATIONS BETWEEN 45° PT ANCHOR BEARING CONE DO NOT REQUIRE STEEL PIPE SLEEVE SLAB REINFOCEMENT

6 Penetration Near Tendon Anchorage Zone
1" = 1'-0"



MULTIPLE SLAB OPENINGS 10" TO 24"

MULTIPLE SLAB OPENINGS 10" TO 24"

SINGLE SLAB OPENINGS 12" TO 24"

NOTE:
1. FOR SLAB OPENINGS LESS THAN 12" NO ADDITIONAL REINFOREMENT REQUIRED.
2. FOR OPENINGS LARGER THAN 24" AND MULTIPLE ADJACENT OPENINGS, SEE

5 Trim Bars at Slab Openings 12" to 24"
1" = 1'-0"

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Structural Engineers
VSMsq Structural Engineers, LLC
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Suite 350
Houston, TX 77077
281-407-5734
www.vsmsq.com

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SHEET TITLE:
PT Slab on Grade Foundation Typical Details

SHEET NUMBER:
S3.011

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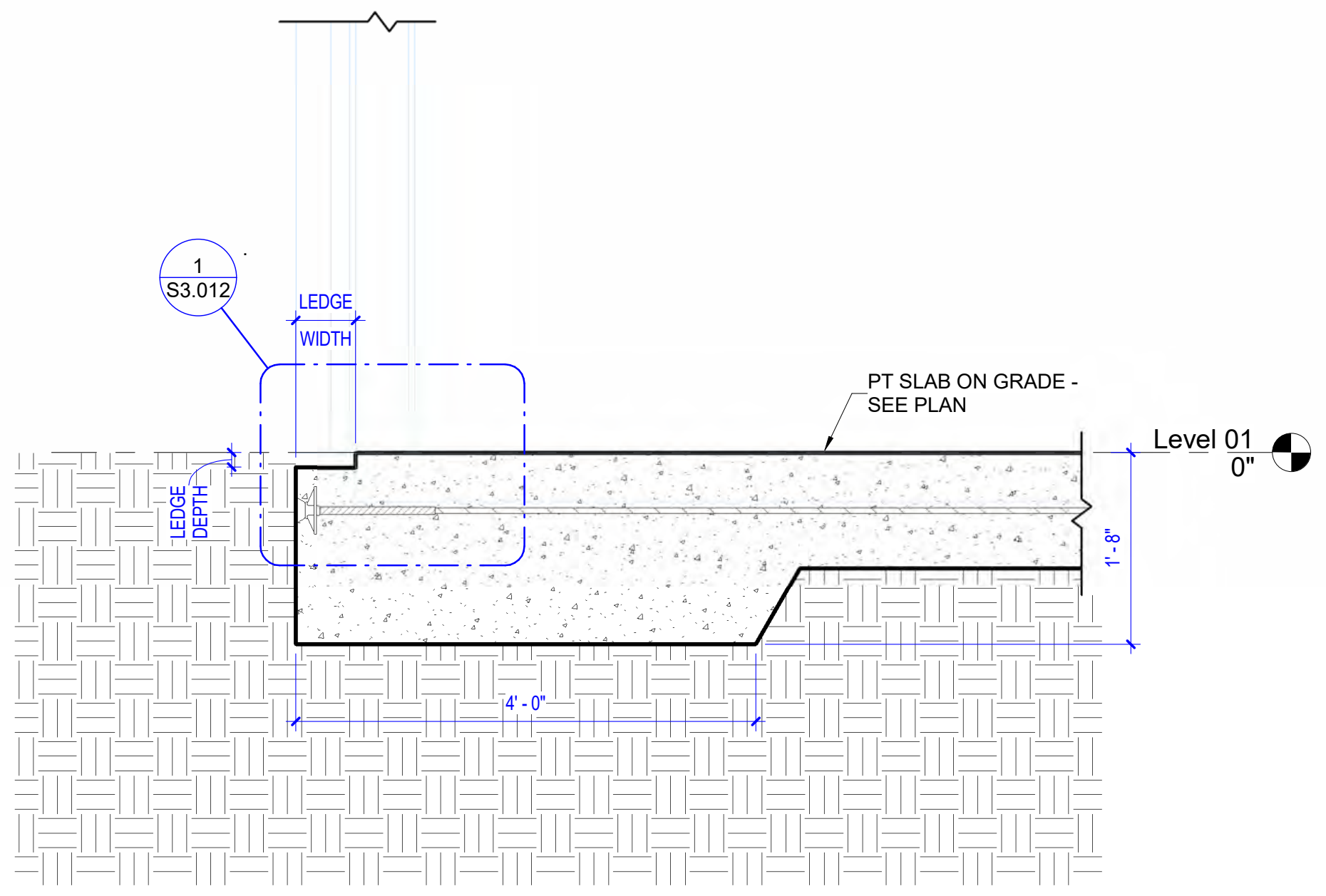
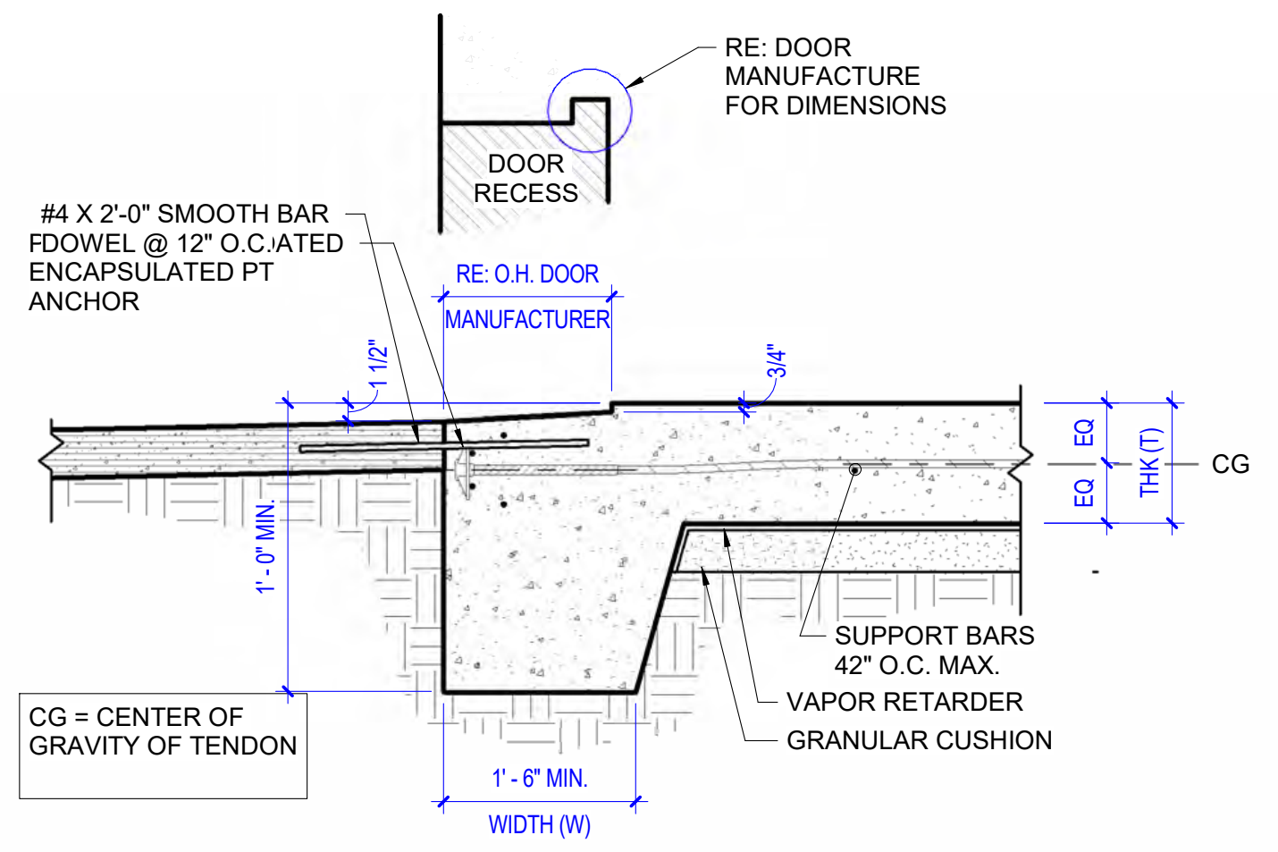
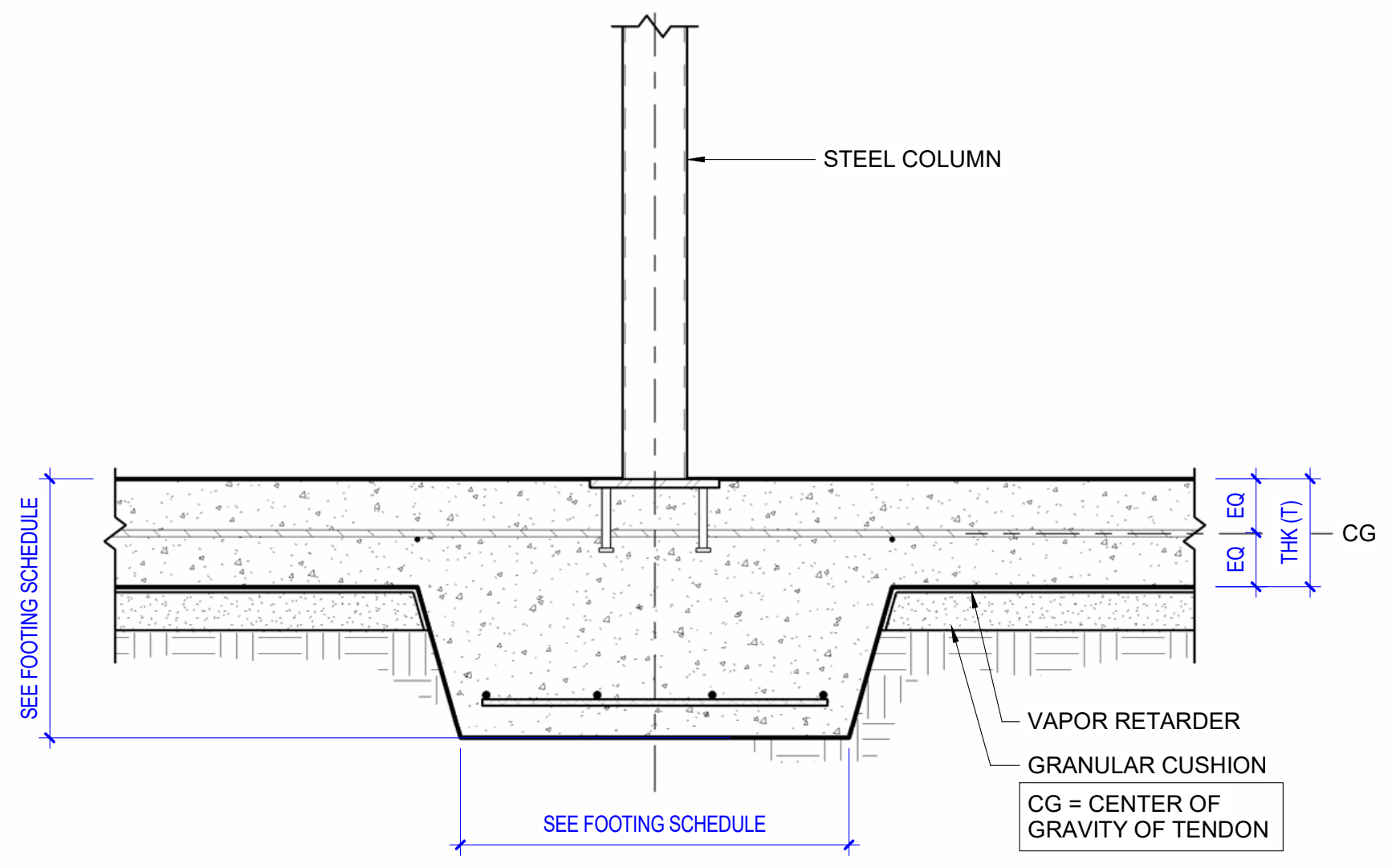


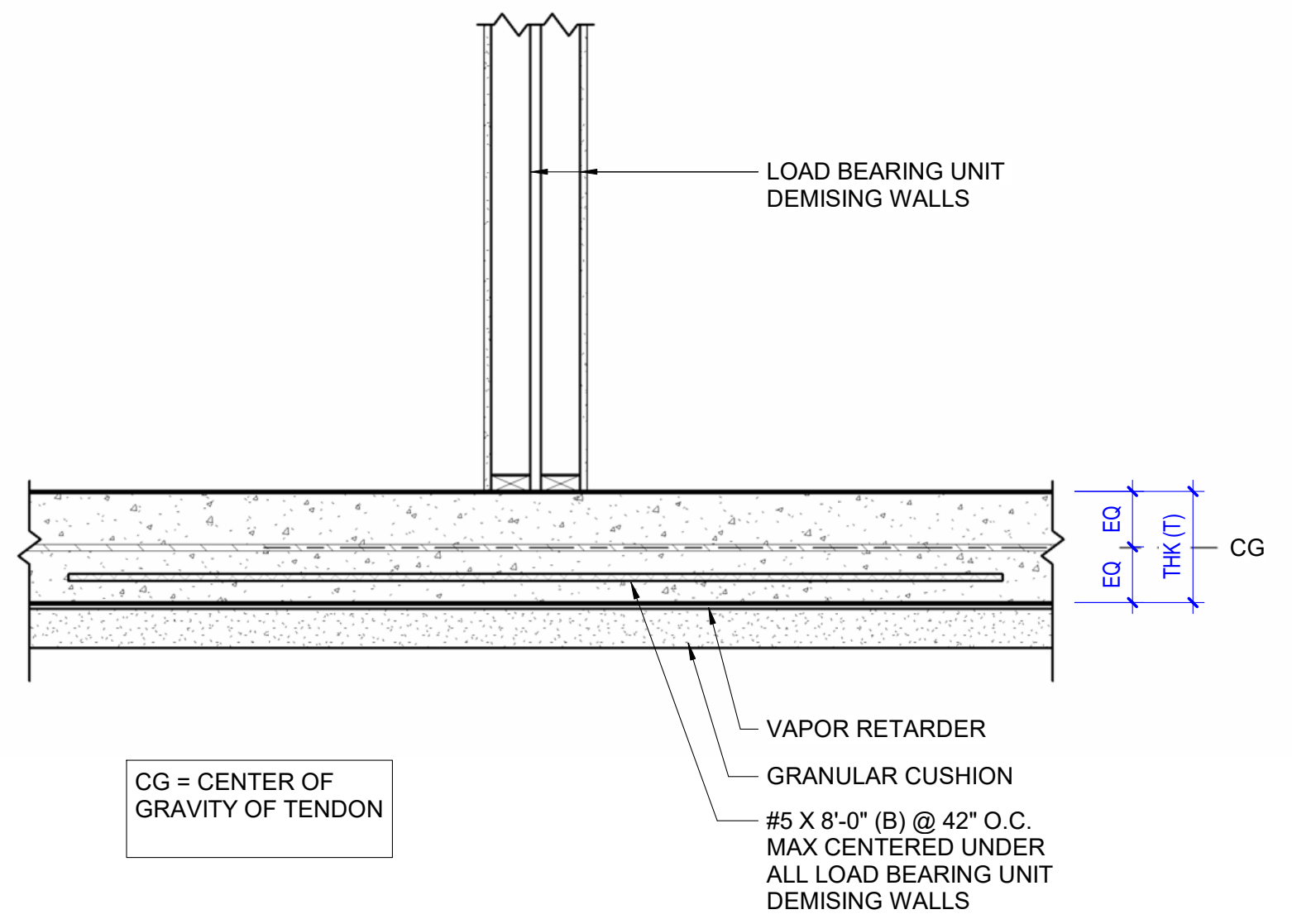
11 Section at Wide Beam
3/4" = 1'-0"


RE: DOOR MANUFACTURE FOR DIMENSIONS
DOOR RECESS
#4 X 2'-0" SMOOTH BAR FDOWEL @ 12" O.C.ATED ENCAPSULATED PT ANCHOR
RE: O.H. DOOR
MANUFACTURER
1 1/2"
3/4"
1'-0" MIN.
EQ
EQ
THK (T)
CG
SUPPORT BARS 42" O.C. MAX.
VAPOR RETARDER
GRANULAR CUSHION
CG = CENTER OF GRAVITY OF TENDON
1'-6" MIN.
WIDTH (W)

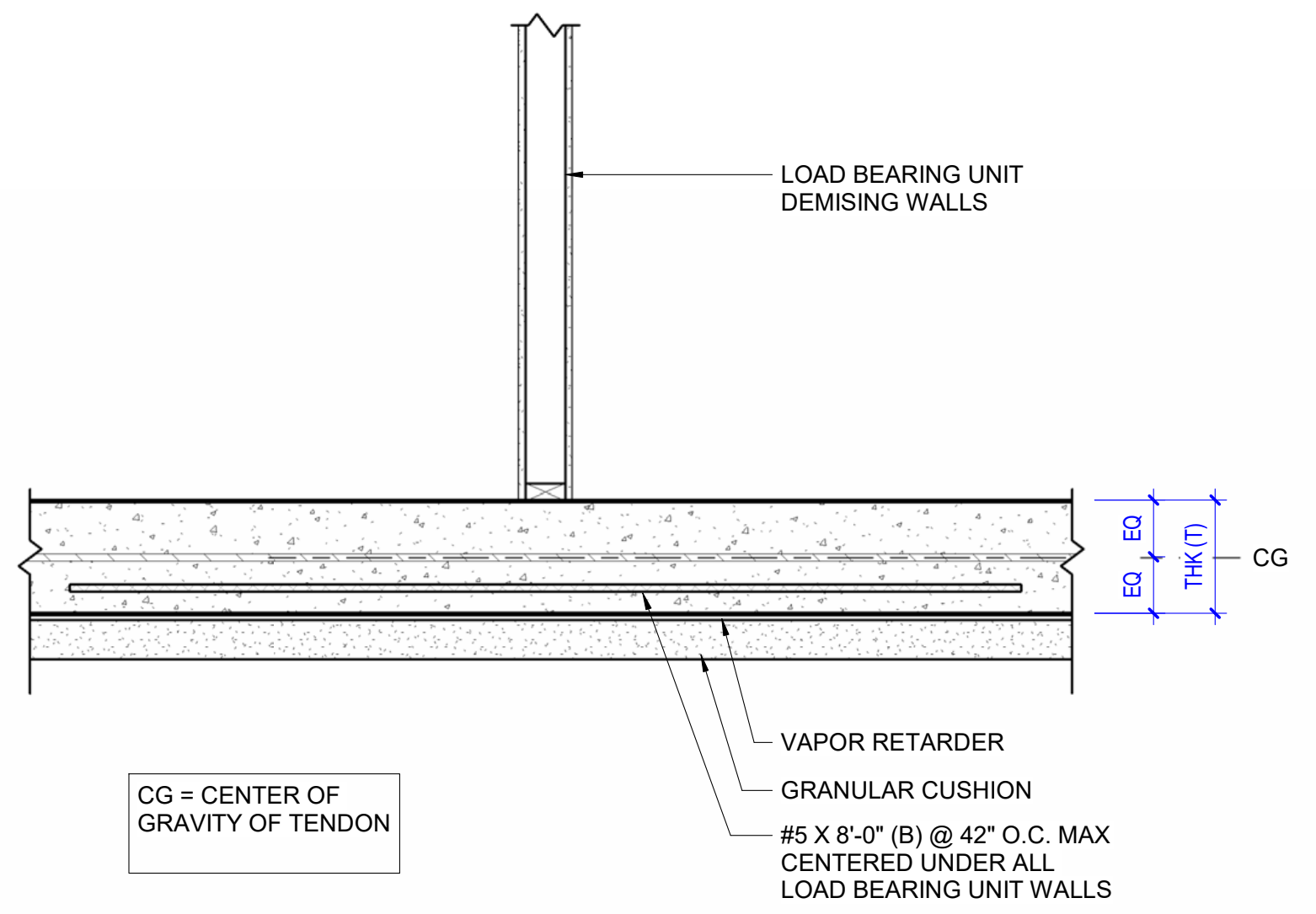
10 Exterior Grade Beam at OH Door
3/4" = 1'-0"



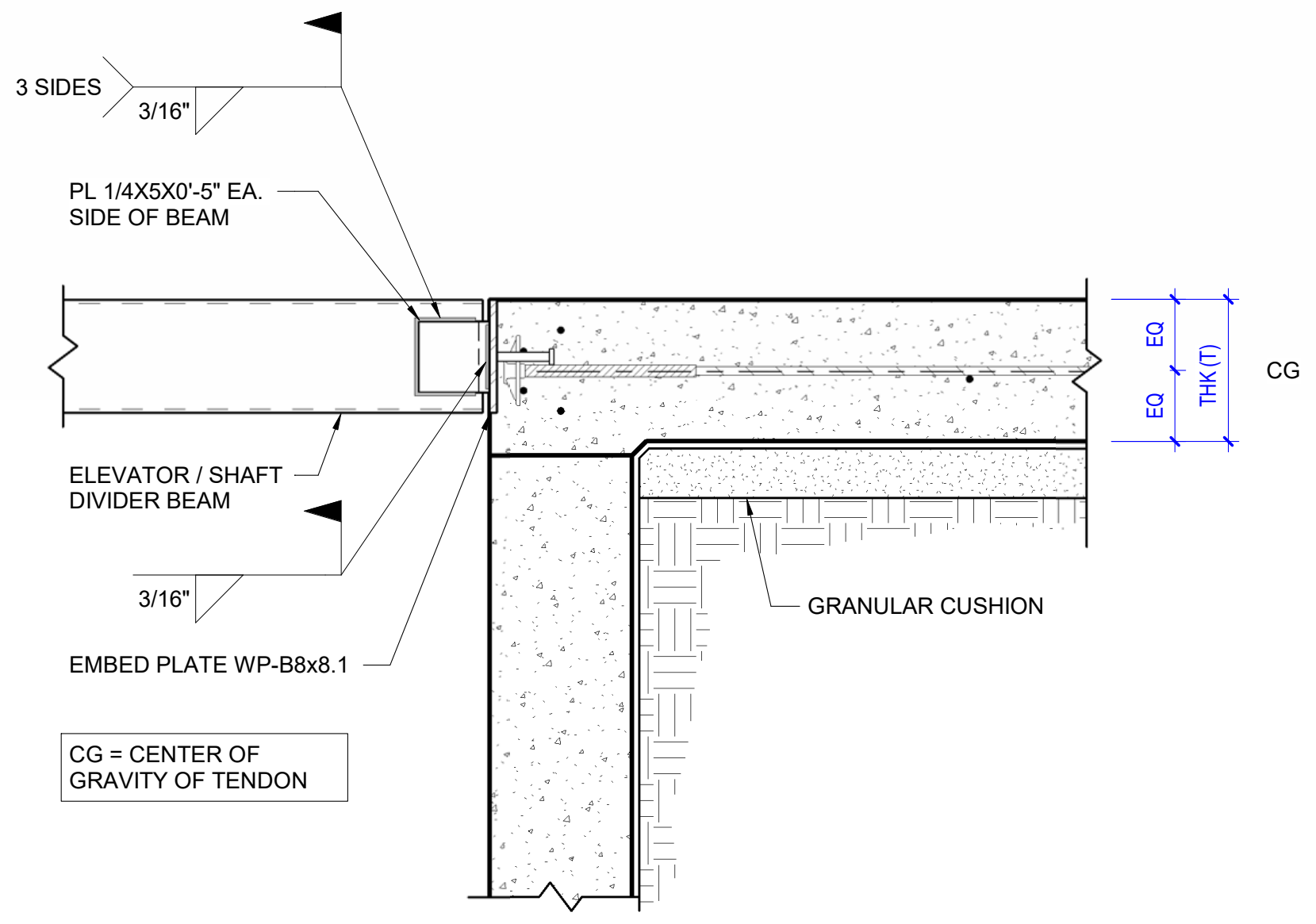
9 Exterior Grade Beam at OH Door
3/4" = 1'-0"


LOAD BEARING UNIT DEMISING WALLS
EQ
EQ
THK (T)
CG
VAPOR RETARDER
GRANULAR CUSHION
CG = CENTER OF GRAVITY OF TENDON
#5 X 8'-0" (B) @ 42" O.C. MAX CENTERED UNDER ALL LOAD BEARING UNIT DEMISING WALLS

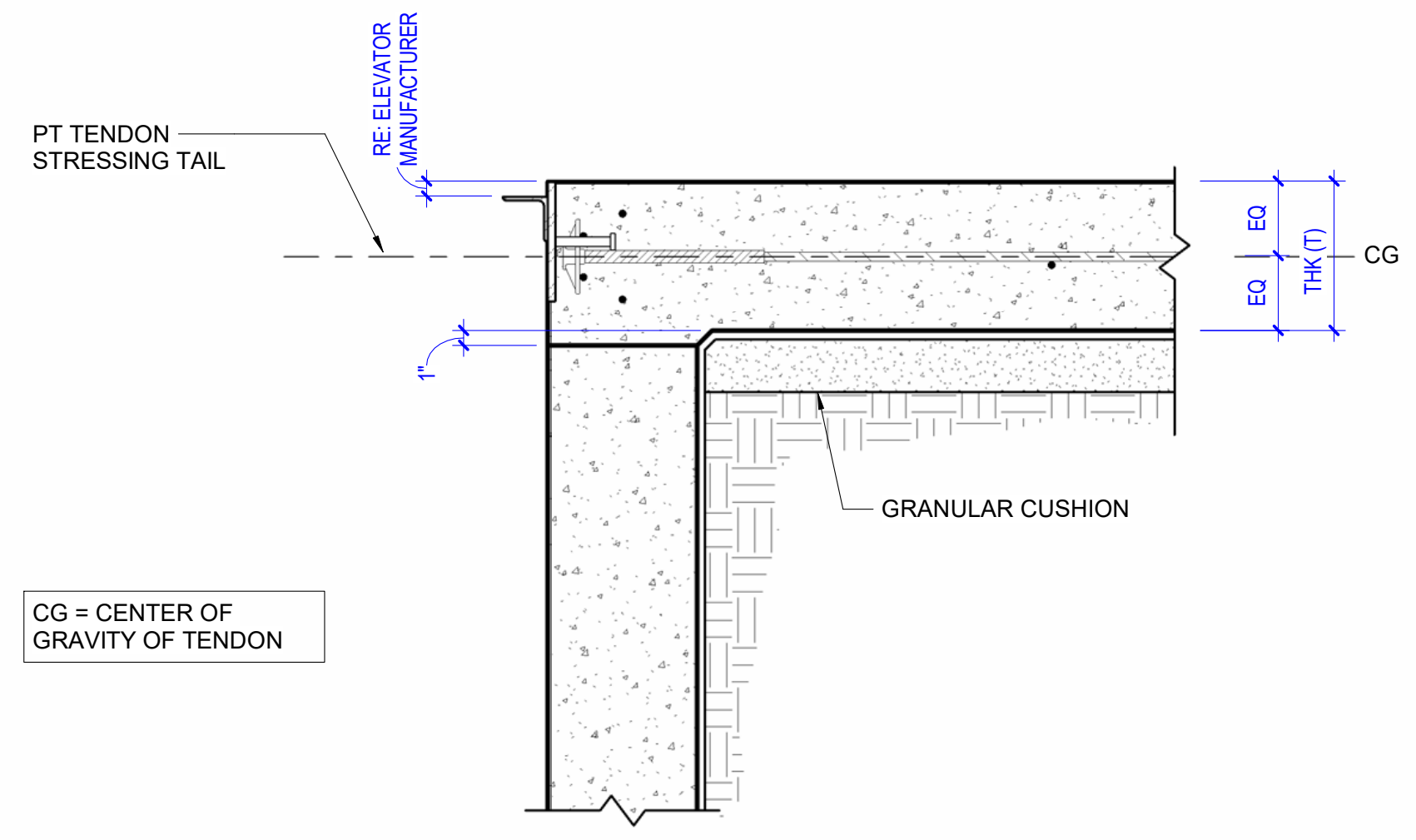
8 Interior at Unit Bearing Walls
3/4" = 1'-0"


LOAD BEARING UNIT DEMISING WALLS
EQ
EQ
THK (T)
CG
VAPOR RETARDER
GRANULAR CUSHION
CG = CENTER OF GRAVITY OF TENDON
#5 X 8'-0" (B) @ 42" O.C. MAX CENTERED UNDER ALL LOAD BEARING UNIT WALLS

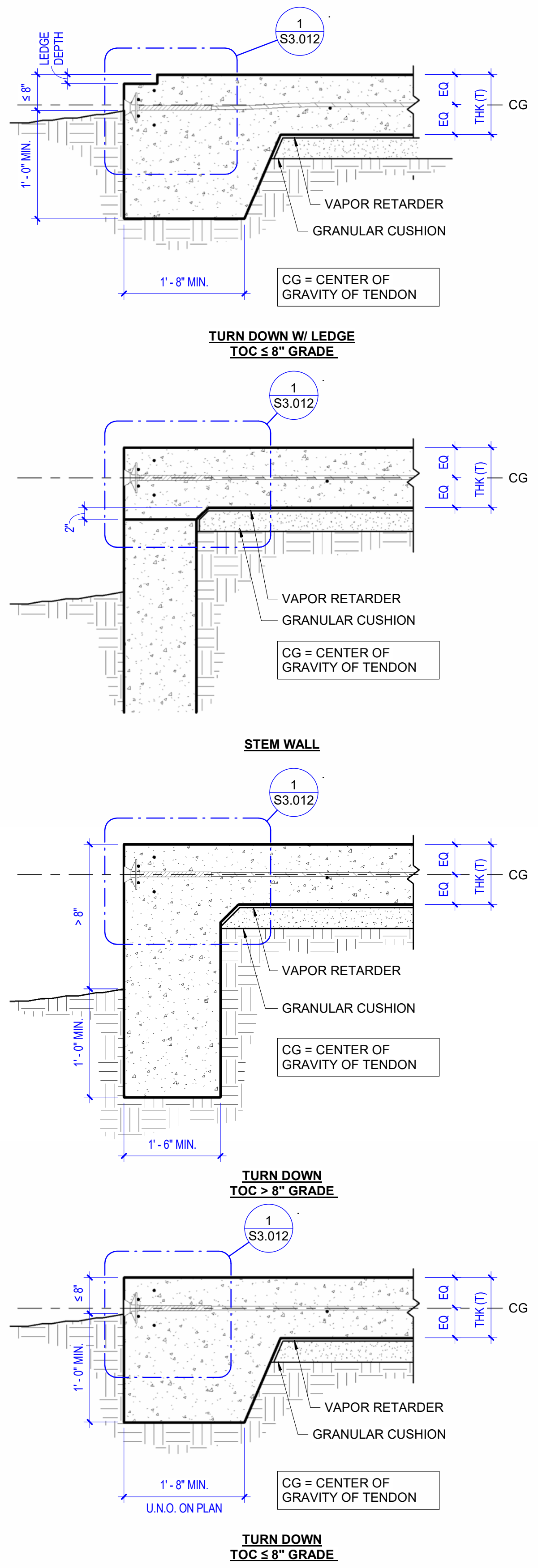
7 Interior Section at Unit Bearing Walls
3/4" = 1'-0"


3 SIDES
3/16"
PL 1/4X5X0'-5" EA. SIDE OF BEAM
Level 01
0"
ELEVATOR / SHAFT DIVIDER BEAM
3/16"
EMBED PLATE WP-B8x8.1
CG = CENTER OF GRAVITY OF TENDON
EQ
EQ
THK (T)
CG
GRANULAR CUSHION

6 Exterior Foundation Stressing End at Distributed Tendons
1" = 1'-0"



5 Exterior Foundation Stressing End at Distributed Tendons
1" = 1'-0"



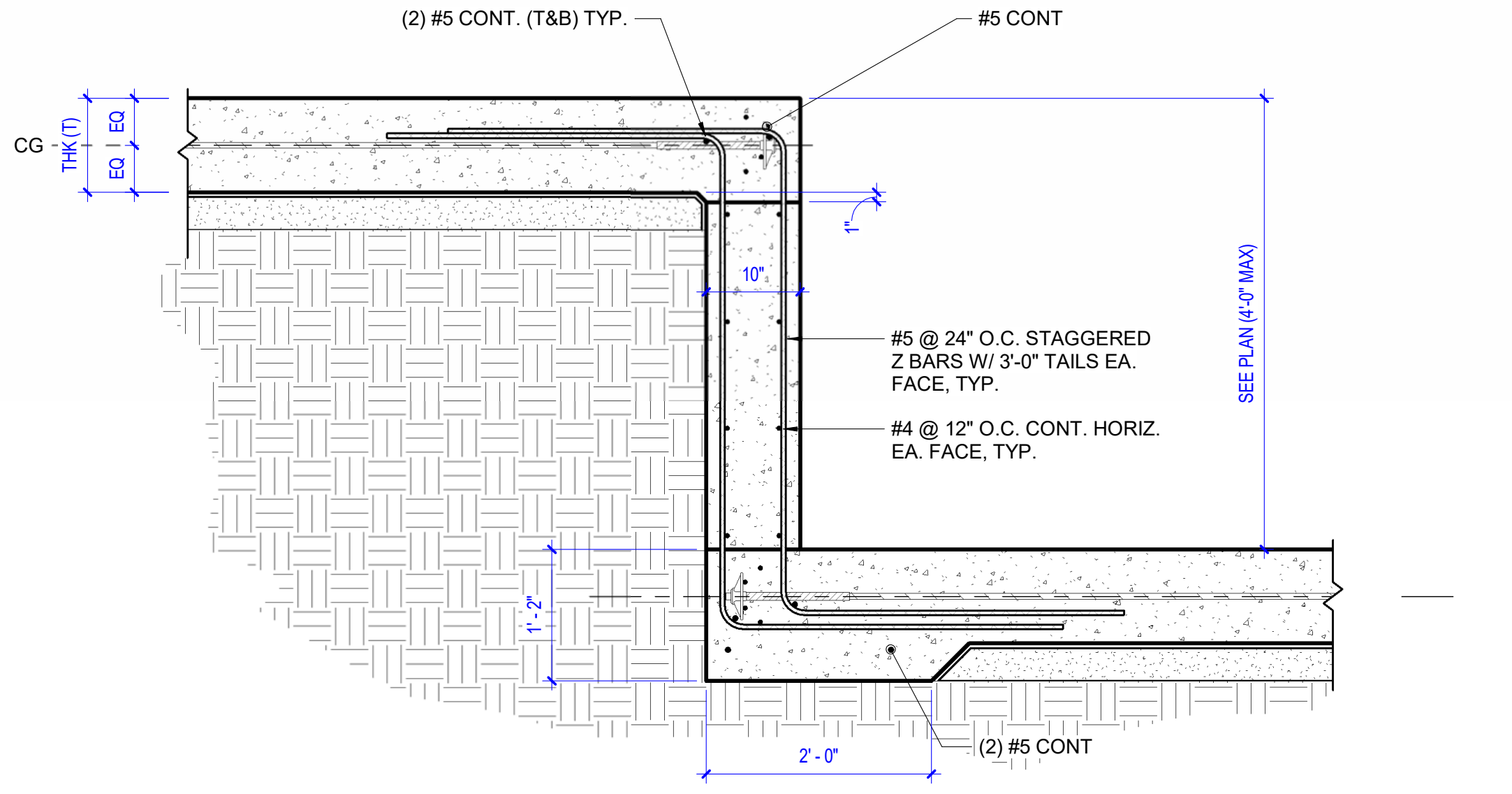
TURN DOWN W/ LEDGE
TOC ≤ 8" GRADE

STEM WALL

TURN DOWN
TOC > 8" GRADE

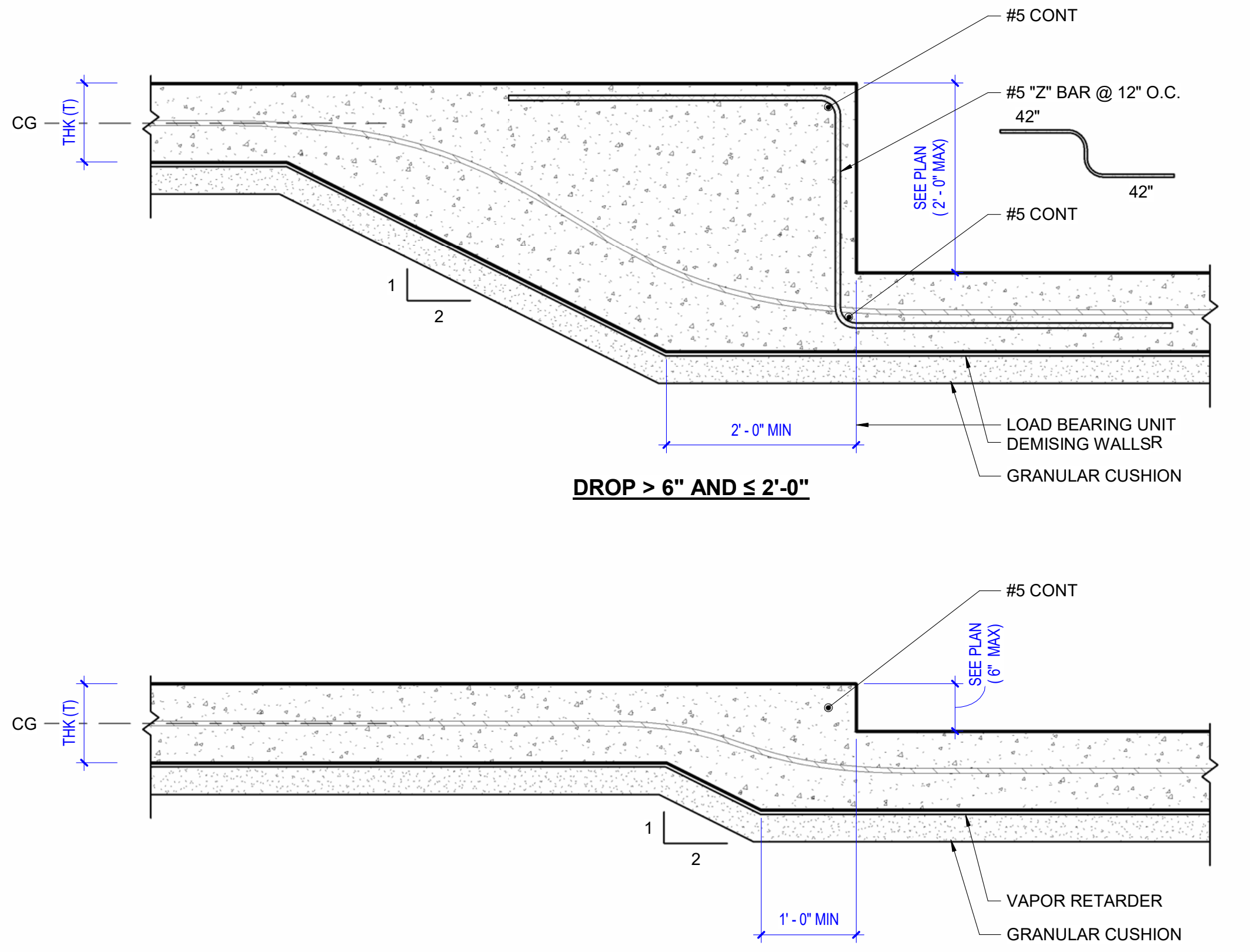
TURN DOWN
TOC ≤ 8" GRADE

4 Exterior Foundation
3/4" = 1'-0"


(2) #5 CONT. (T&B) TYP.
#5 CONT
CG
THK (T)
EQ
EQ
1"
10"
#5 @ 24" O.C. STAGGERED Z BARS W/ 3'-0" TAILS EA. FACE, TYP.
#4 @ 12" O.C. CONT. HORIZ. EA. FACE, TYP.
SEE PLAN (4'-0" MAX)
1'-2"
2'-0"
(2) #5 CONT

DROP > 2'-0" AND ≤ 4'-0"

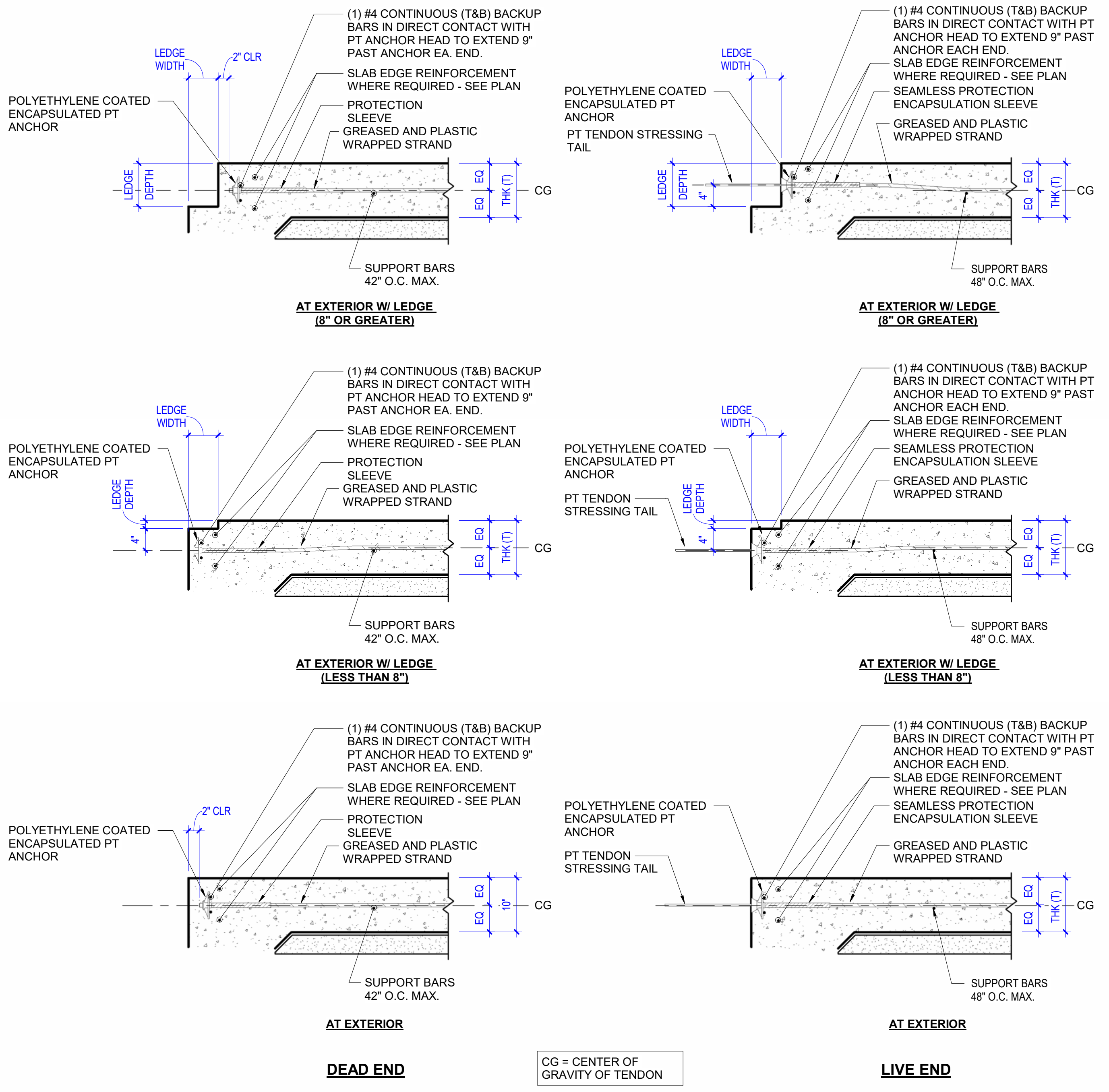
3 Step in Foundation (2'-0" or Less)
3/4" = 1'-0"



DROP > 6" AND ≤ 2'-0"

DROP ≤ 6"

2 Step in Foundation (2'-0" or Less)
3/4" = 1'-0"


LEDGE
WIDTH
2" CLR
(1) #4 CONTINUOUS (T&B) BACKUP BARS IN DIRECT CONTACT WITH PT ANCHOR HEAD TO EXTEND 9" PAST ANCHOR EA. END.
SLAB EDGE REINFORCEMENT WHERE REQUIRED - SEE PLAN
PROTECTION SLEEVE
GREASED AND PLASTIC WRAPPED STRAND
POLYETHYLENE COATED ENCAPSULATED PT ANCHOR
LEDGE
DEPTH
EQ
EQ
THK (T)
CG
SUPPORT BARS 42" O.C. MAX.
(1) #4 CONTINUOUS (T&B) BACKUP BARS IN DIRECT CONTACT WITH PT ANCHOR HEAD TO EXTEND 9" PAST ANCHOR EACH END.
SLAB EDGE REINFORCEMENT WHERE REQUIRED - SEE PLAN
SEAMLESS PROTECTION ENCAPSULATION SLEEVE
GREASED AND PLASTIC WRAPPED STRAND
POLYETHYLENE COATED ENCAPSULATED PT ANCHOR
PT TENDON STRESSING TAIL
4"
SUPPORT BARS 48" O.C. MAX.
10"
CG = CENTER OF GRAVITY OF TENDON

AT EXTERIOR W/ LEDGE (8" OR GREATER)

AT EXTERIOR W/ LEDGE (8" OR GREATER)

AT EXTERIOR W/ LEDGE (LESS THAN 8")

AT EXTERIOR W/ LEDGE (LESS THAN 8")

AT EXTERIOR

AT EXTERIOR

DEAD END

LIVE END

1 Exterior Foundation Dead End at Distributed Tendons
3/4" = 1'-0"

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SHEET TITLE:
PT Slab on Grade Foundation Typical Details

SHEET NUMBER:
S3.012

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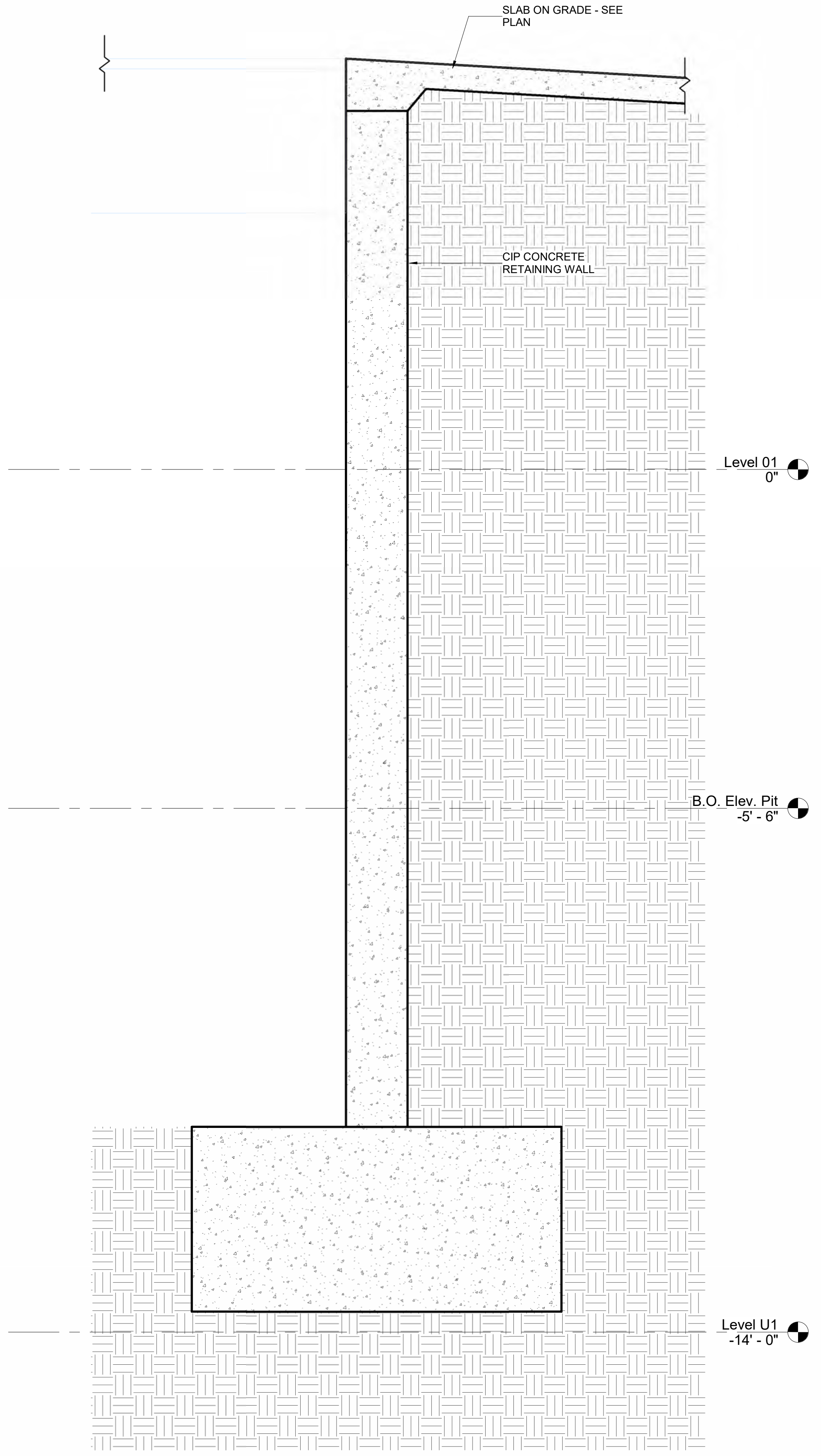
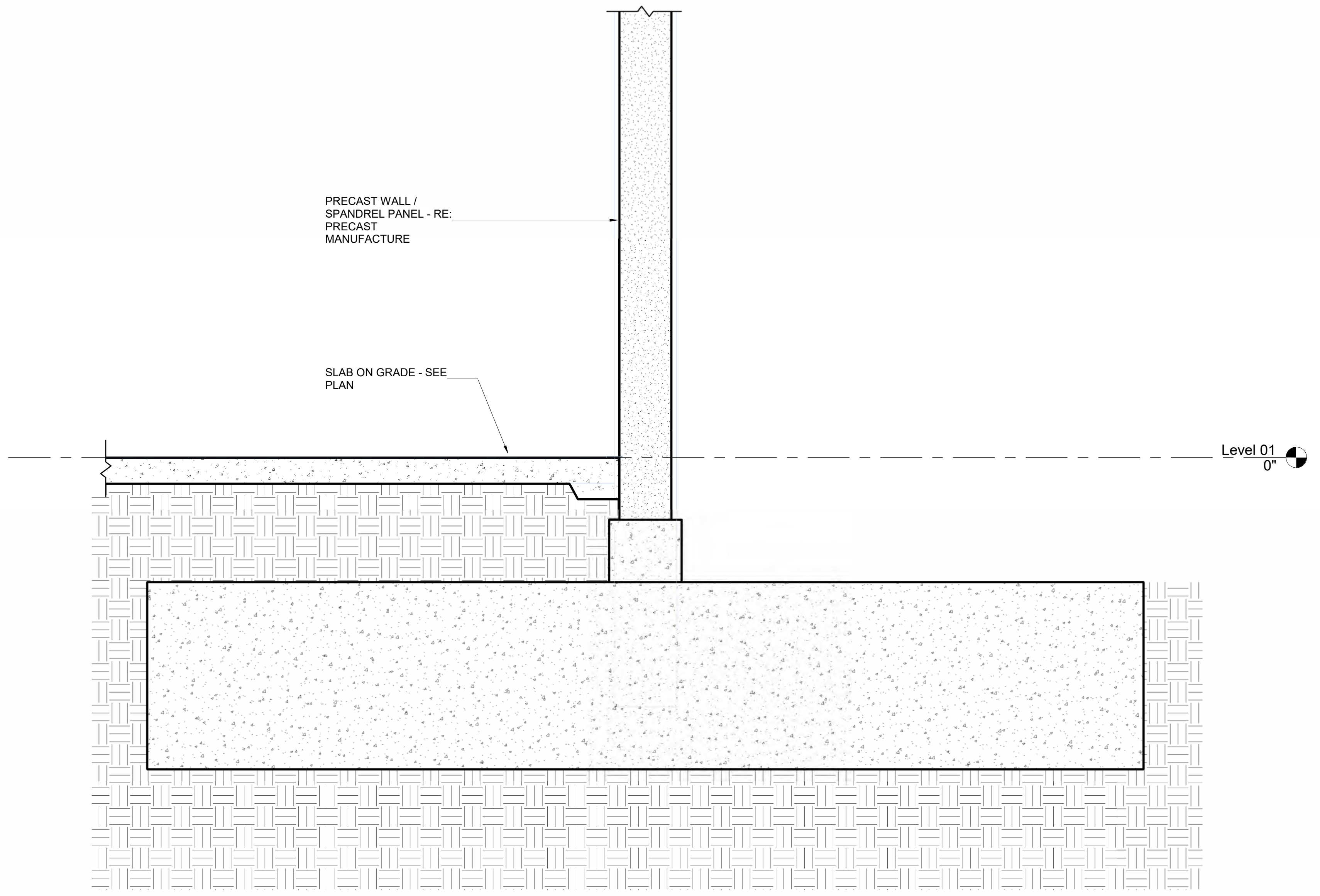
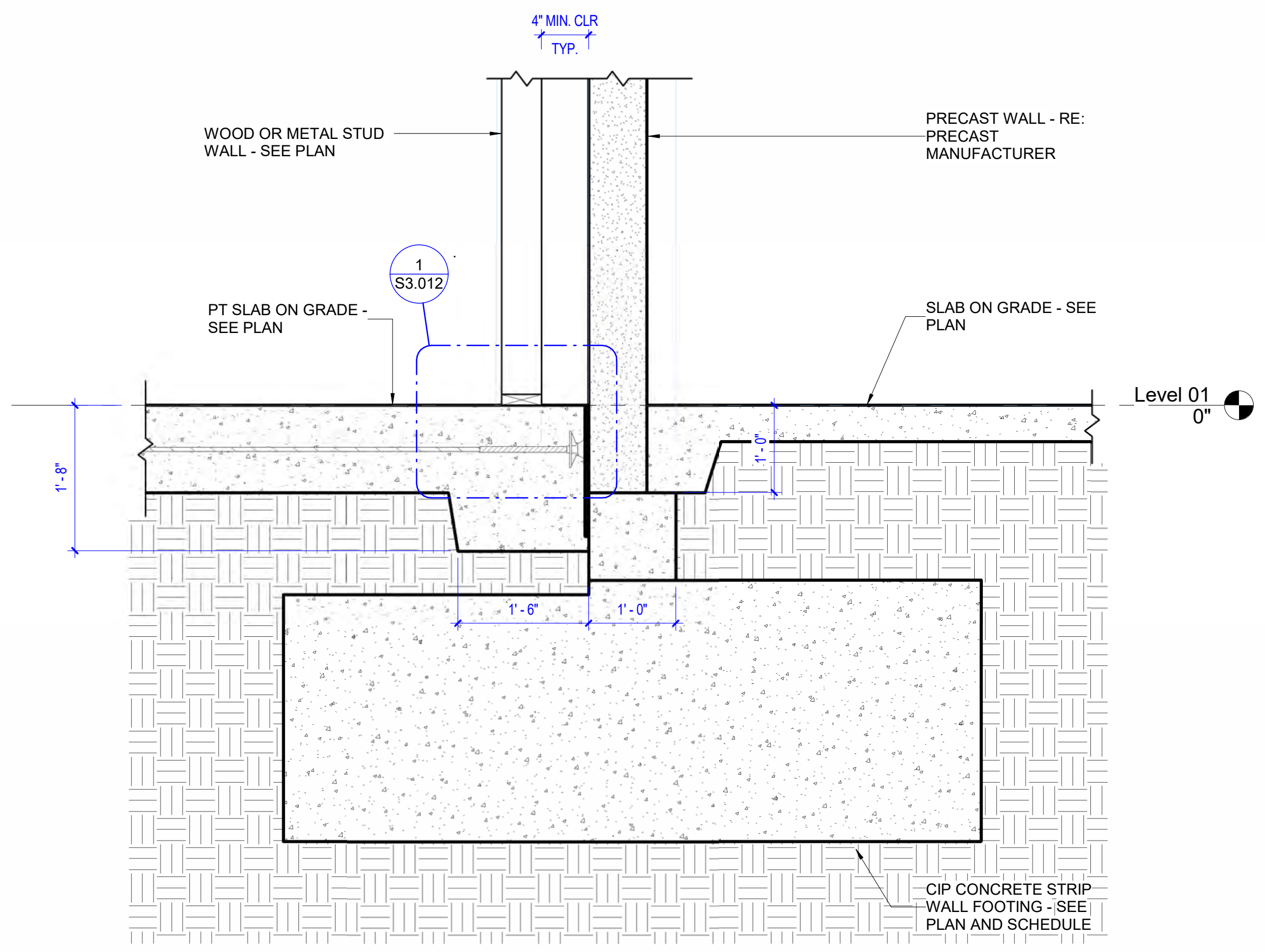


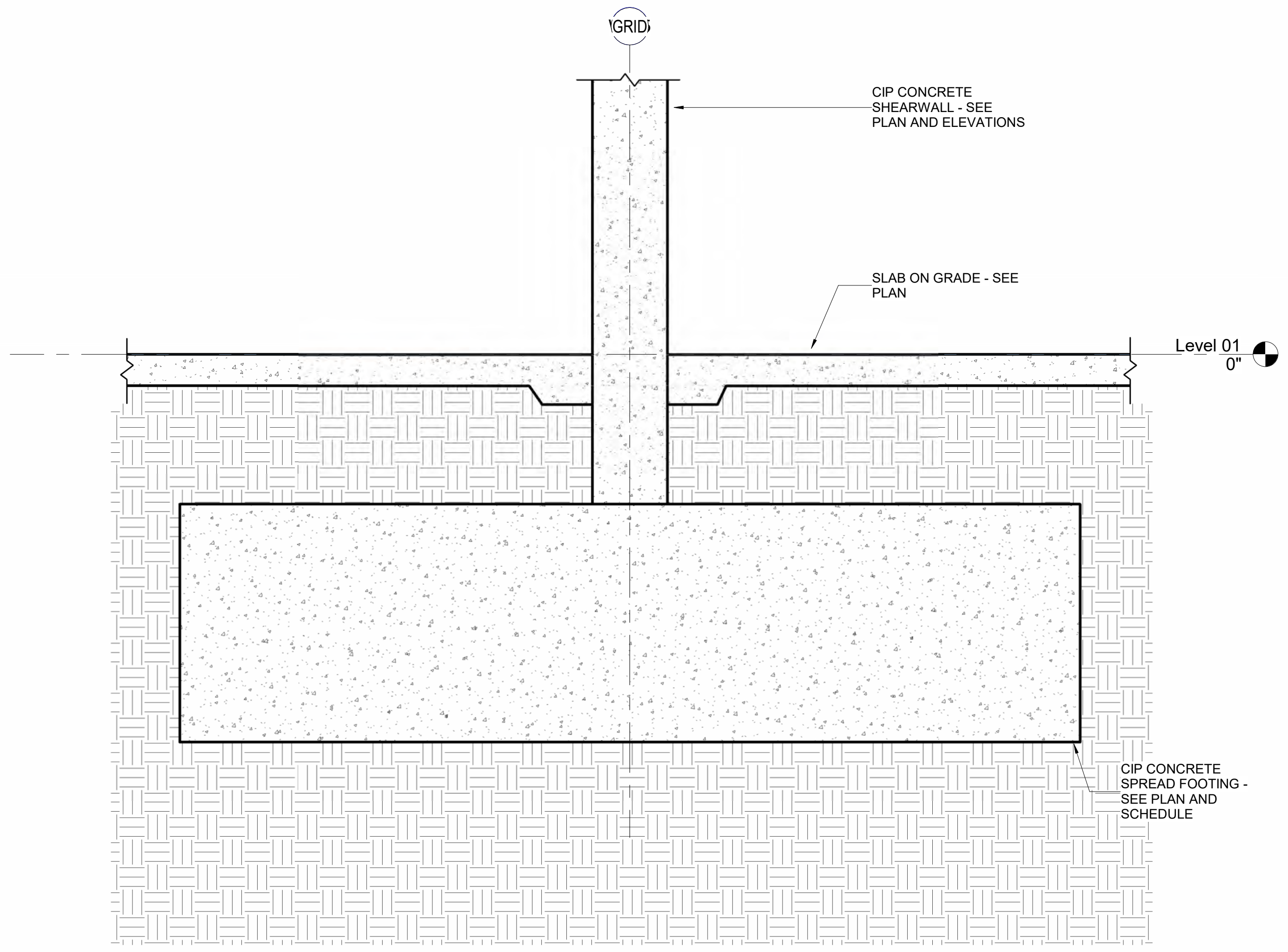
9 Section at Ramp
3/4" = 1'-0"



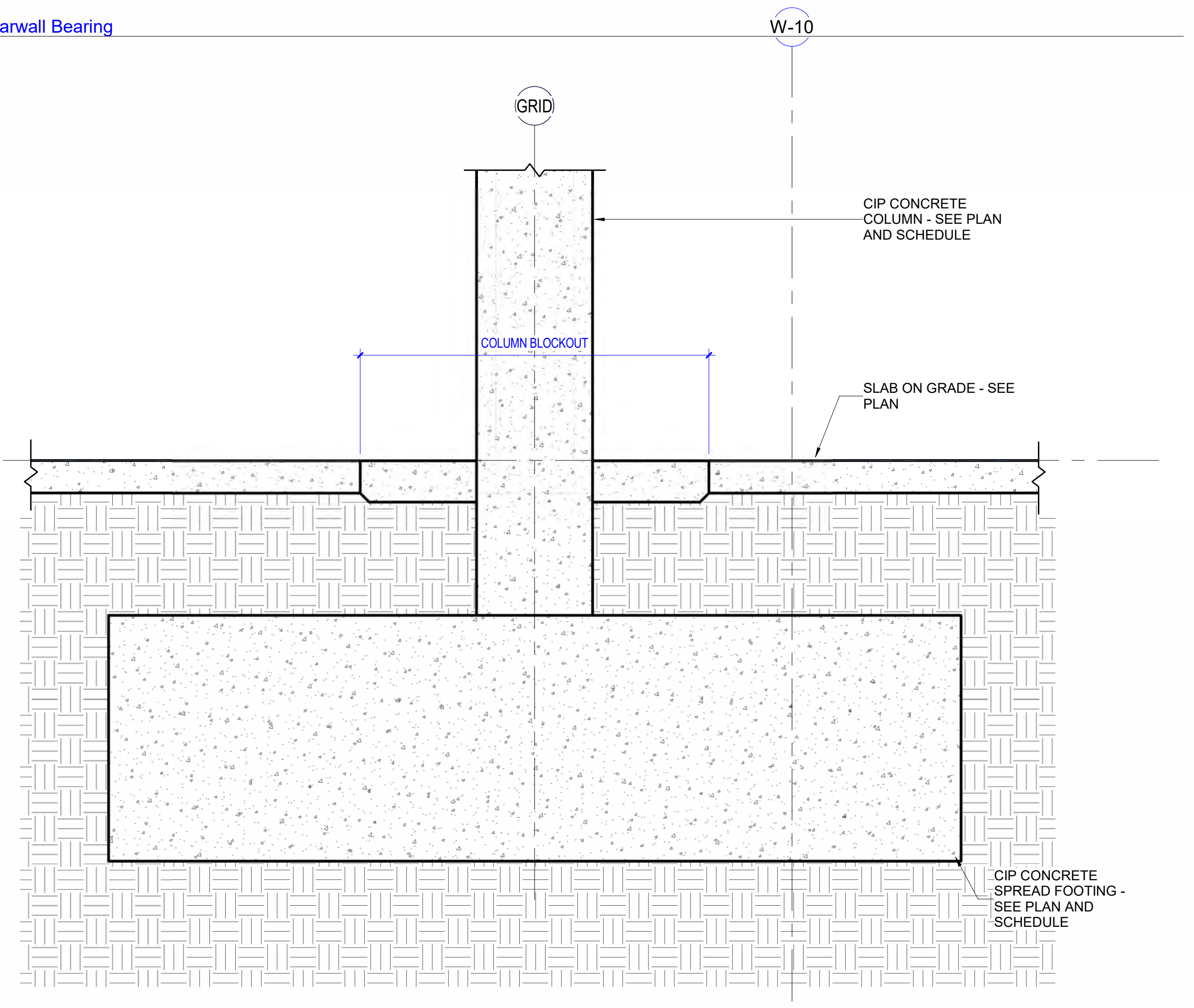
8 Section at Precast Wall
3/4" = 1'-0"


4" MIN. CLR
TYP.
WOOD OR METAL STUD WALL - SEE PLAN
PRECAST WALL - RE: PRECAST MANUFACTURER
1
S3.012
PT SLAB ON GRADE - SEE PLAN
SLAB ON GRADE - SEE PLAN
Level 01
0"
1' - 8"
1' - 0"
1' - 6"
1' - 0"
CIP CONCRETE STRIP WALL FOOTING - SEE PLAN AND SCHEDULE

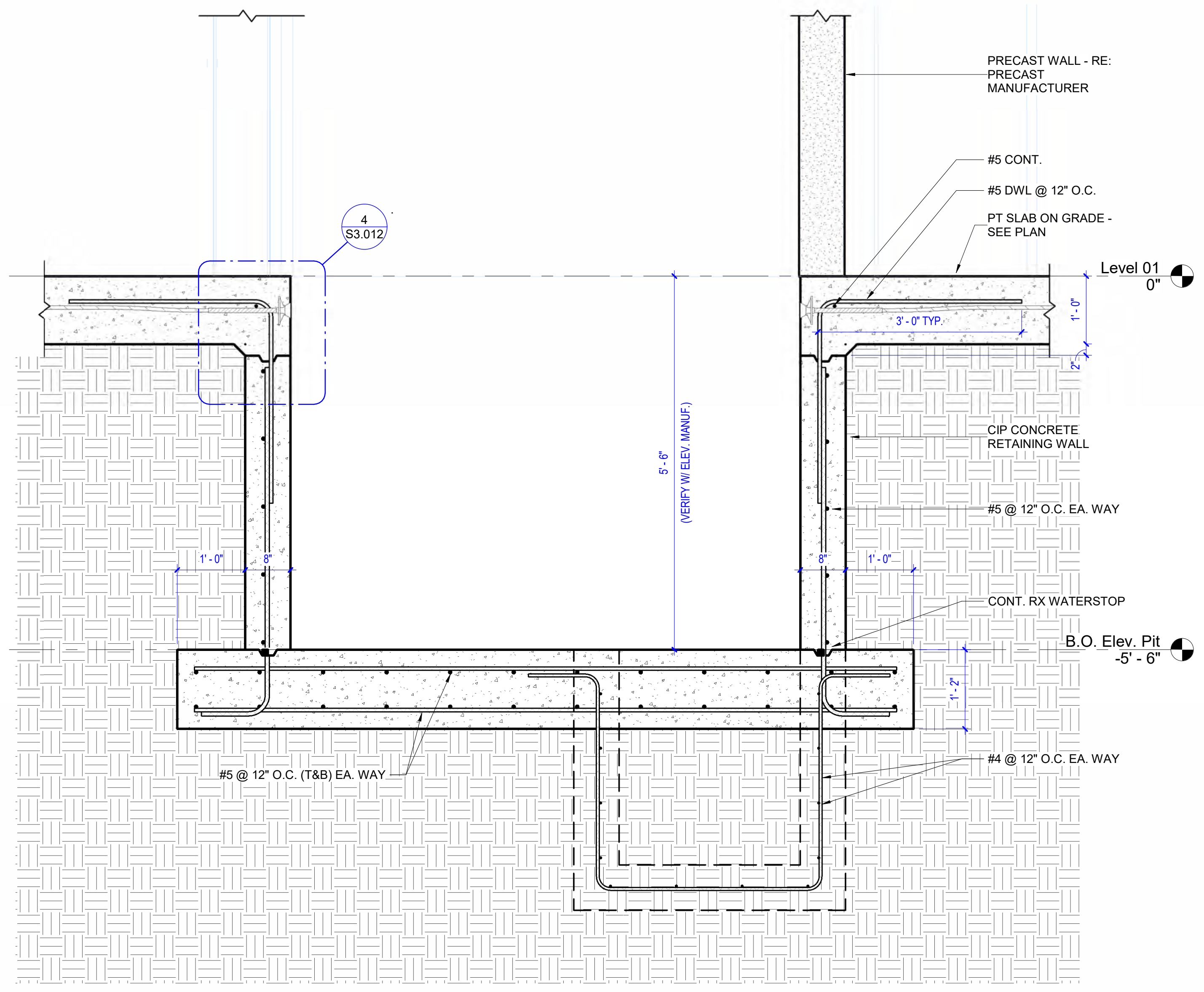
7 Section at Garage Expansion Joint
3/4" = 1'-0"


GRID
CIP CONCRETE SHEARWALL - SEE PLAN AND ELEVATIONS
SLAB ON GRADE - SEE PLAN
Level 01
0"
CIP CONCRETE SPREAD FOOTING - SEE PLAN AND SCHEDULE

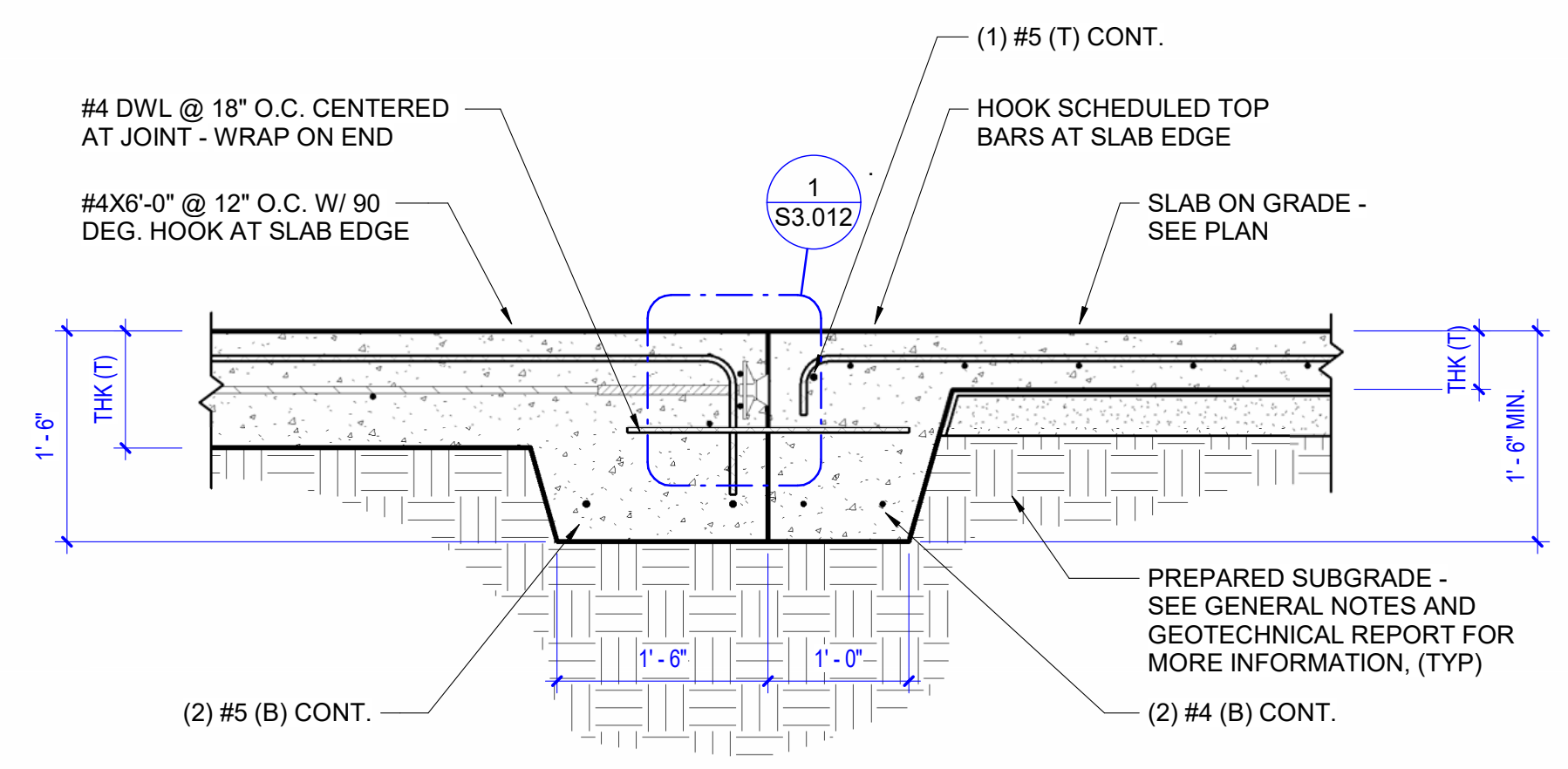
6 Section at Shearwall Bearing
3/4" = 1'-0"


W-10
GRID
CIP CONCRETE COLUMN - SEE PLAN AND SCHEDULE
COLUMN BLOCKOUT
SLAB ON GRADE - SEE PLAN
CIP CONCRETE SPREAD FOOTING - SEE PLAN AND SCHEDULE

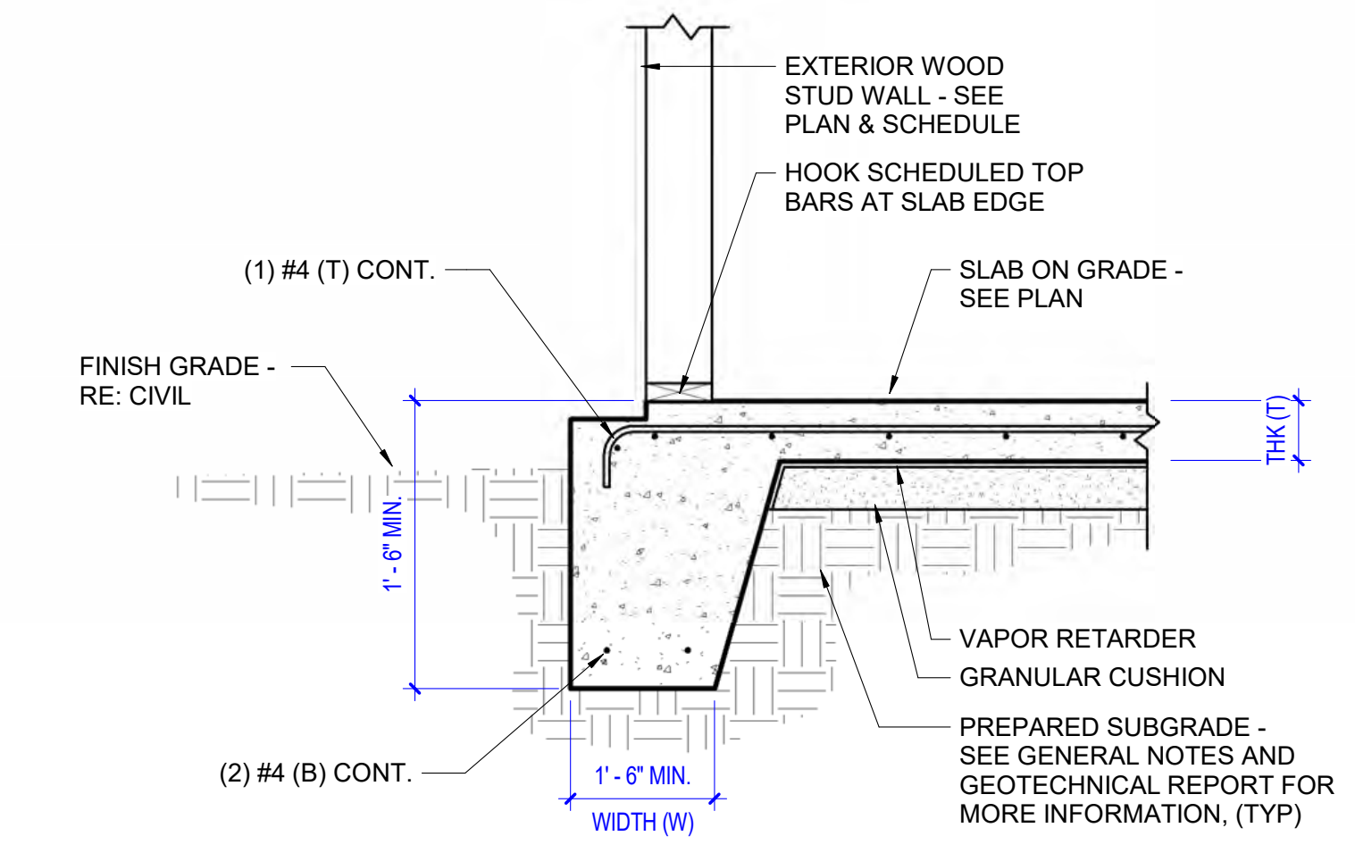
5 Section at Column Bearing
3/4" = 1'-0"



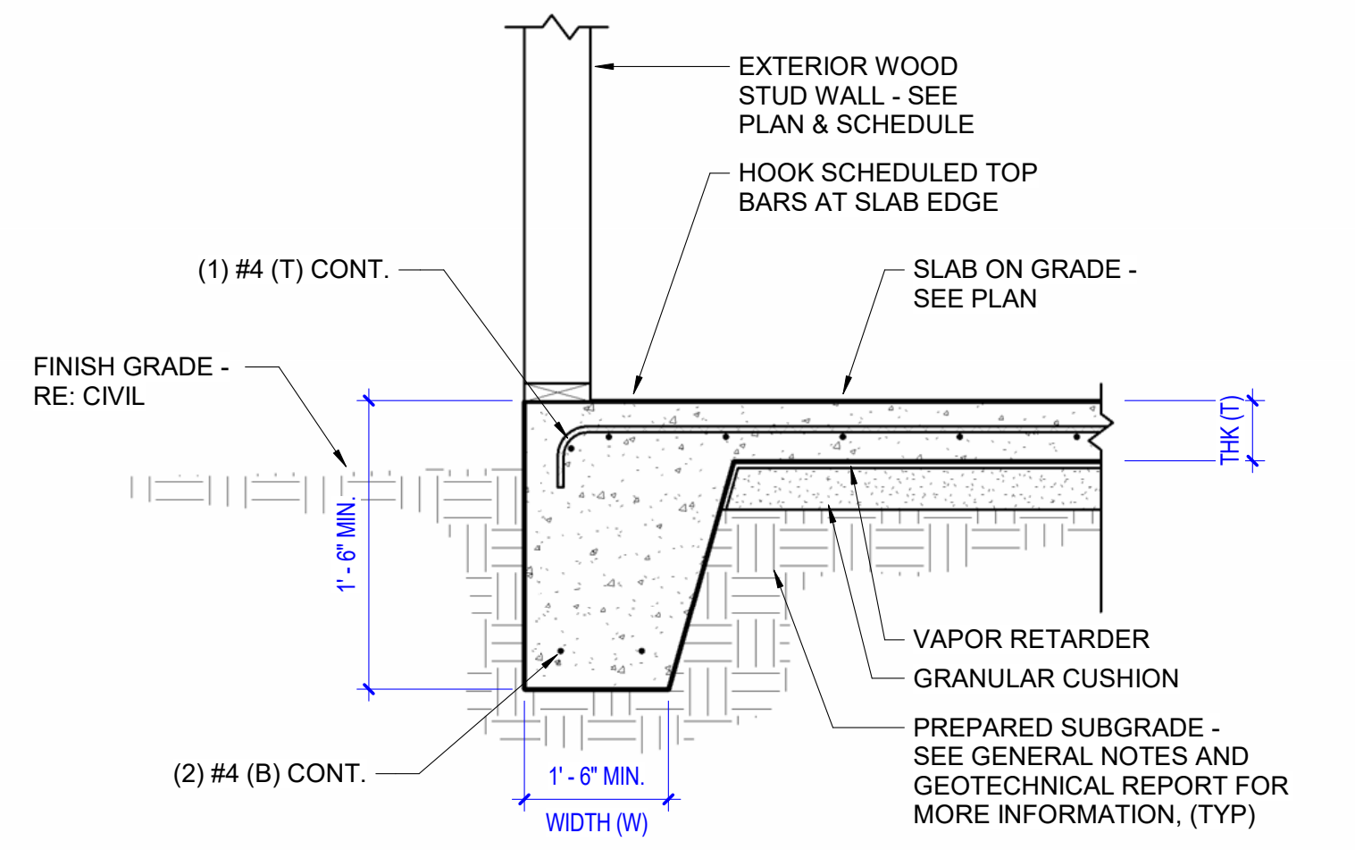
4 Section at Elevator Pit
3/4" = 1'-0"


(1) #5 (T) CONT.
#4 DWL @ 18" O.C. CENTERED AT JOINT - WRAP ON END
HOOK SCHEDULED TOP BARS AT SLAB EDGE
#4X6'-0" @ 12" O.C. W/ 90 DEG. HOOK AT SLAB EDGE
1
S3.012
SLAB ON GRADE - SEE PLAN
1' - 6"
THK (T)
THK (T)
1' - 6" MIN.
PREPARED SUBGRADE - SEE GENERAL NOTES AND GEOTECHNICAL REPORT FOR MORE INFORMATION, (TYP)
1' - 6"
1' - 0"
(2) #5 (B) CONT.
(2) #4 (B) CONT.

3 Typ Exterior Grade Beam
3/4" = 1'-0"


EXTERIOR WOOD STUD WALL - SEE PLAN & SCHEDULE
HOOK SCHEDULED TOP BARS AT SLAB EDGE
(1) #4 (T) CONT.
SLAB ON GRADE - SEE PLAN
FINISH GRADE - RE: CIVIL
THK (T)
1' - 6" MIN.
VAPOR RETARDER
GRANULAR CUSHION
PREPARED SUBGRADE - SEE GENERAL NOTES AND GEOTECHNICAL REPORT FOR MORE INFORMATION, (TYP)
(2) #4 (B) CONT.
1' - 6" MIN.
WIDTH (W)

2 Typ. Exterior Grade Beam w/ Ledge
3/4" = 1'-0"


EXTERIOR WOOD STUD WALL - SEE PLAN & SCHEDULE
HOOK SCHEDULED TOP BARS AT SLAB EDGE
(1) #4 (T) CONT.
SLAB ON GRADE - SEE PLAN
FINISH GRADE - RE: CIVIL
THK (T)
1' - 6" MIN.
VAPOR RETARDER
GRANULAR CUSHION
PREPARED SUBGRADE - SEE GENERAL NOTES AND GEOTECHNICAL REPORT FOR MORE INFORMATION, (TYP)
(2) #4 (B) CONT.
1' - 6" MIN.
WIDTH (W)

1 Typ Exterior Grade Beam
3/4" = 1'-0"

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Structural Engineers

PROJECT # : 26154
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SHEET TITLE:
Foundations Sections

SHEET NUMBER:
S3.100

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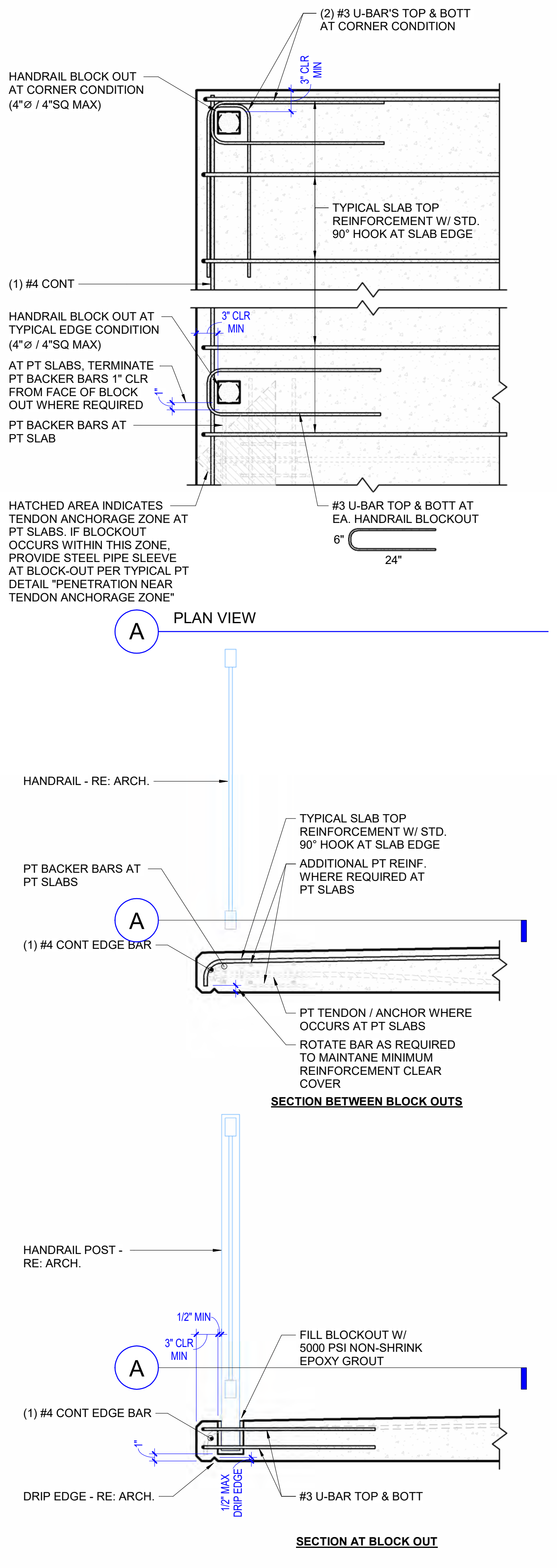
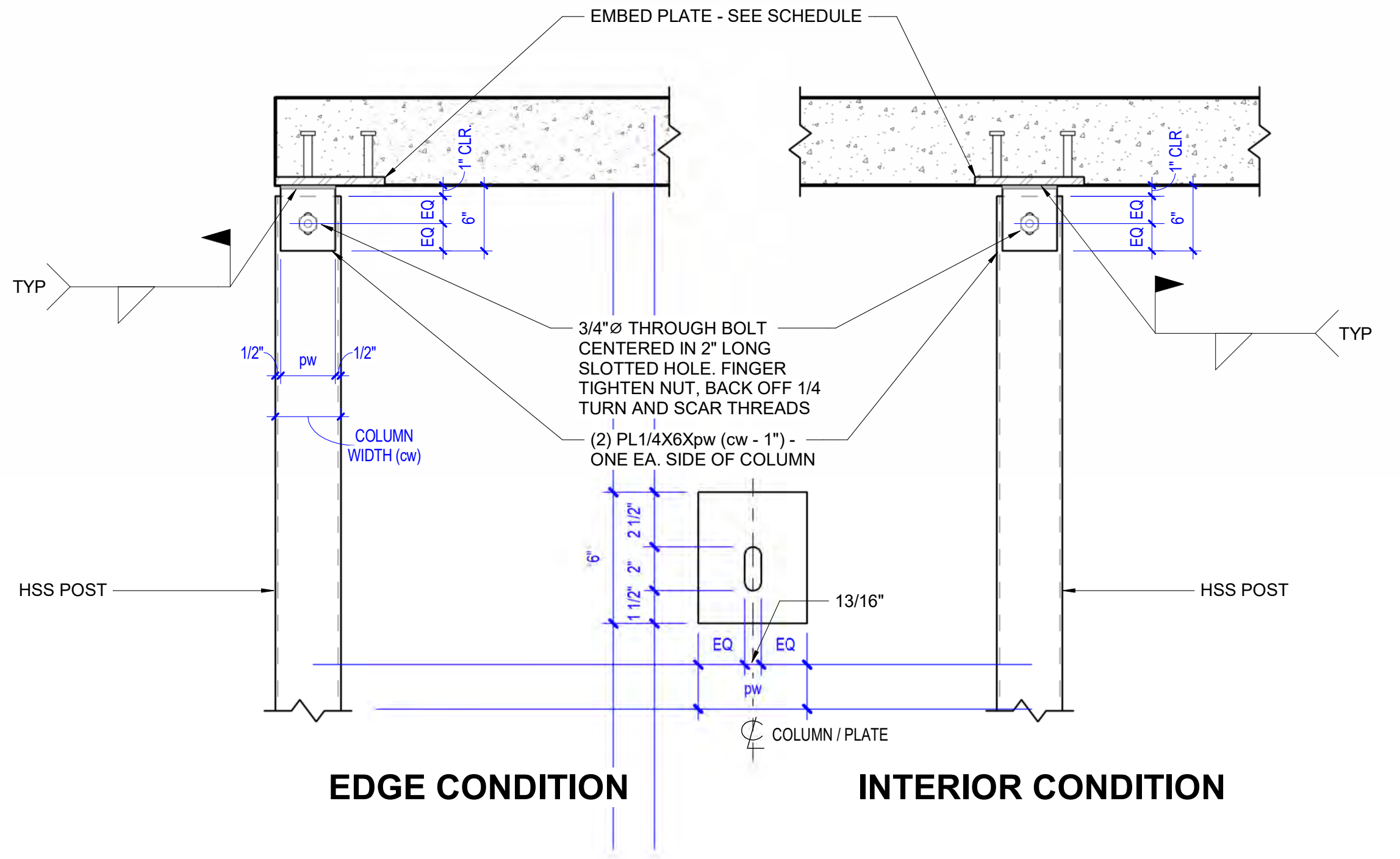

(2) #3 U-BAR'S TOP & BOTT AT CORNER CONDITION
3" CLR MIN
HANDRAIL BLOCK OUT AT CORNER CONDITION (4"Ø / 4"SQ MAX)
TYPICAL SLAB TOP REINFORCEMENT W/ STD. 90° HOOK AT SLAB EDGE
(1) #4 CONT
HANDRAIL BLOCK OUT AT TYPICAL EDGE CONDITION (4"Ø / 4"SQ MAX)
3" CLR MIN
AT PT SLABS, TERMINATE PT BACKER BARS 1" CLR FROM FACE OF BLOCK OUT WHERE REQUIRED
1"
PT BACKER BARS AT PT SLAB
HATCHED AREA INDICATES TENDON ANCHORAGE ZONE AT PT SLABS. IF BLOCKOUT OCCURS WITHIN THIS ZONE, PROVIDE STEEL PIPE SLEEVE AT BLOCK-OUT PER TYPICAL PT DETAIL "PENETRATION NEAR TENDON ANCHORAGE ZONE"
#3 U-BAR TOP & BOTT AT EA. HANDRAIL BLOCKOUT
6"
24"
A
PLAN VIEW
HANDRAIL - RE: ARCH.
TYPICAL SLAB TOP REINFORCEMENT W/ STD. 90° HOOK AT SLAB EDGE
ADDITIONAL PT REINF. WHERE REQUIRED AT PT SLABS
PT BACKER BARS AT PT SLABS
A
(1) #4 CONT EDGE BAR
PT TENDON / ANCHOR WHERE OCCURS AT PT SLABS
ROTATE BAR AS REQUIRED TO MAINTANE MINIMUM REINFORCEMENT CLEAR COVER
SECTION BETWEEN BLOCK OUTS
HANDRAIL POST - RE: ARCH.
1/2" MIN
3" CLR MIN
A
FILL BLOCKOUT W/ 5000 PSI NON-SHRINK EPOXY GROUT
(1) #4 CONT EDGE BAR
1"
DRIP EDGE - RE: ARCH.
1/2" MAX DRIP EDGE
#3 U-BAR TOP & BOTT
SECTION AT BLOCK OUT

10 Handrail Connection - Grout Pocket

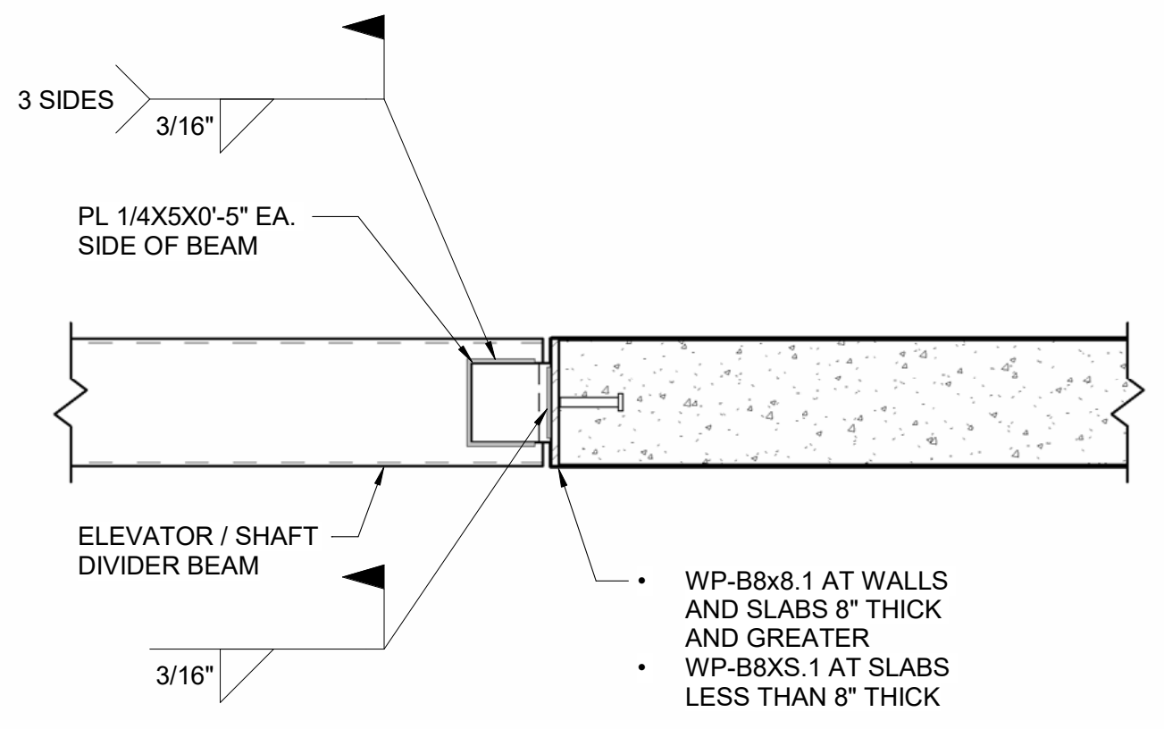
1" = 1'-0"

EMBED PLATE		
COLUMN SIZE	EDGE CONDITION	INTERIOR CONDITION
HSS4X4	WP-E10X10.1	WP-D10X10.1
HSS6X4	WP-E12X12.1	WP-D12X12.2
HSS6X6	WP-E12X12.1	WP-D12X12.2
HSS8X6	WP-E14X14.1	WP-D14X14.1
HSS8X8	WP-E14X14.1	WP-D14X14.1


EMBED PLATE - SEE SCHEDULE
1" CLR.
EQ
EQ
6"
TYP
3/4"Ø THROUGH BOLT CENTERED IN 2" LONG SLOTTED HOLE. FINGER TIGHTEN NUT, BACK OFF 1/4 TURN AND SCAR THREADS
(2) PL1/4X6Xpw (cw - 1") - ONE EA. SIDE OF COLUMN
1/2"
pw
1/2"
COLUMN WIDTH (cw)
6"
2 1/2"
2"
1 1/2"
13/16"
EQ
EQ
pw
COLUMN / PLATE
HSS POST
HSS POST
TYP
EDGE CONDITION
INTERIOR CONDITION

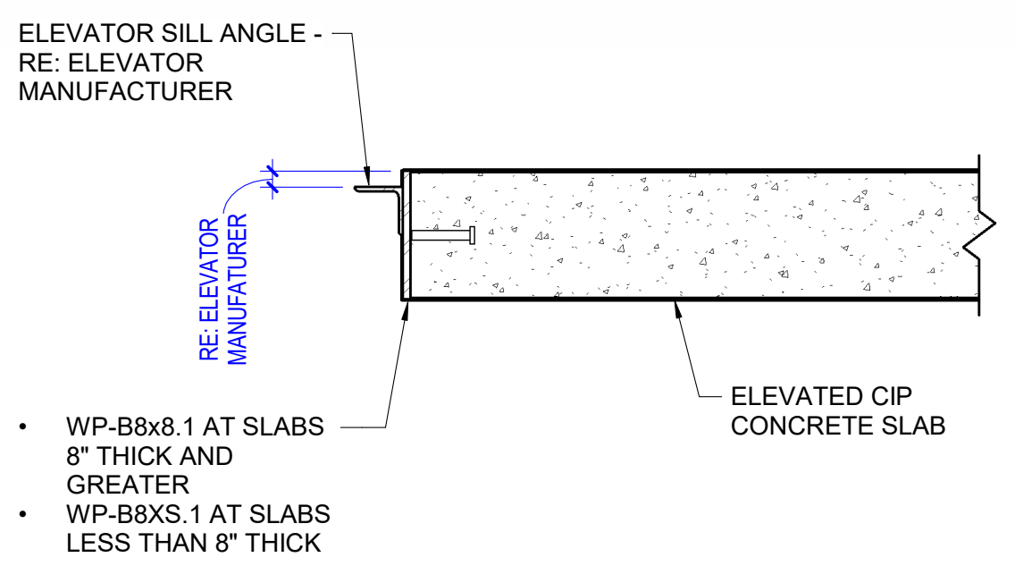
8 HSS Post Head Slip Connection

1" = 1'-0"


3 SIDES
3/16"
PL 1/4X5X0'-5" EA. SIDE OF BEAM
ELEVATOR / SHAFT DIVIDER BEAM
3/16"
WP-B8x8.1 AT WALLS AND SLABS 8" THICK AND GREATER
WP-B8XS.1 AT SLABS LESS THAN 8" THICK

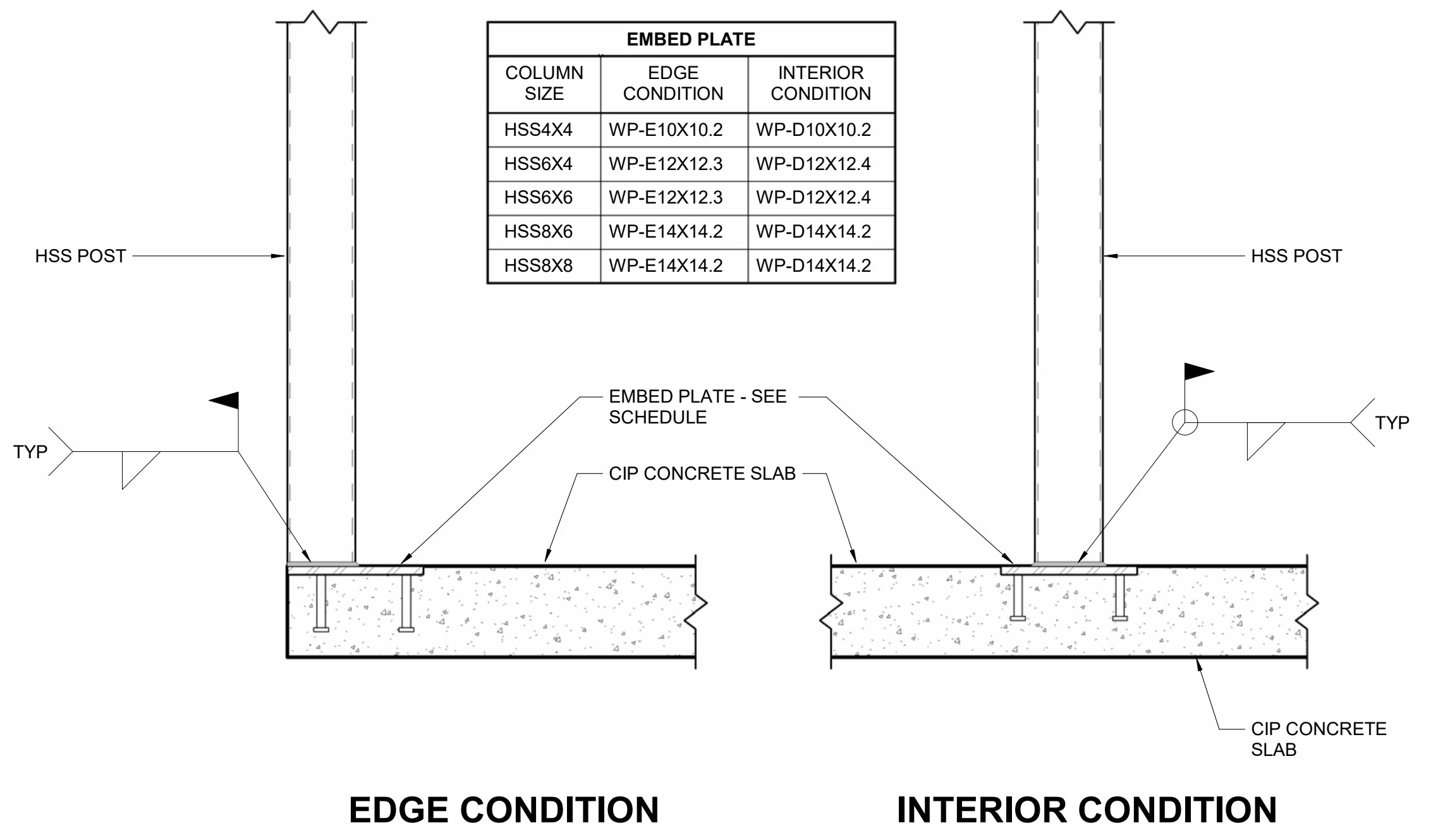
5 Elevator / Shaft Divider Beam Connection at Slab

1" = 1'-0"


ELEVATOR SILL ANGLE - RE: ELEVATOR MANUFACTURER
RE: ELEVATOR MANUFATURER
WP-B8x8.1 AT SLABS 8" THICK AND GREATER
WP-B8XS.1 AT SLABS LESS THAN 8" THICK
ELEVATED CIP CONCRETE SLAB

4 Elevator Sill Angle

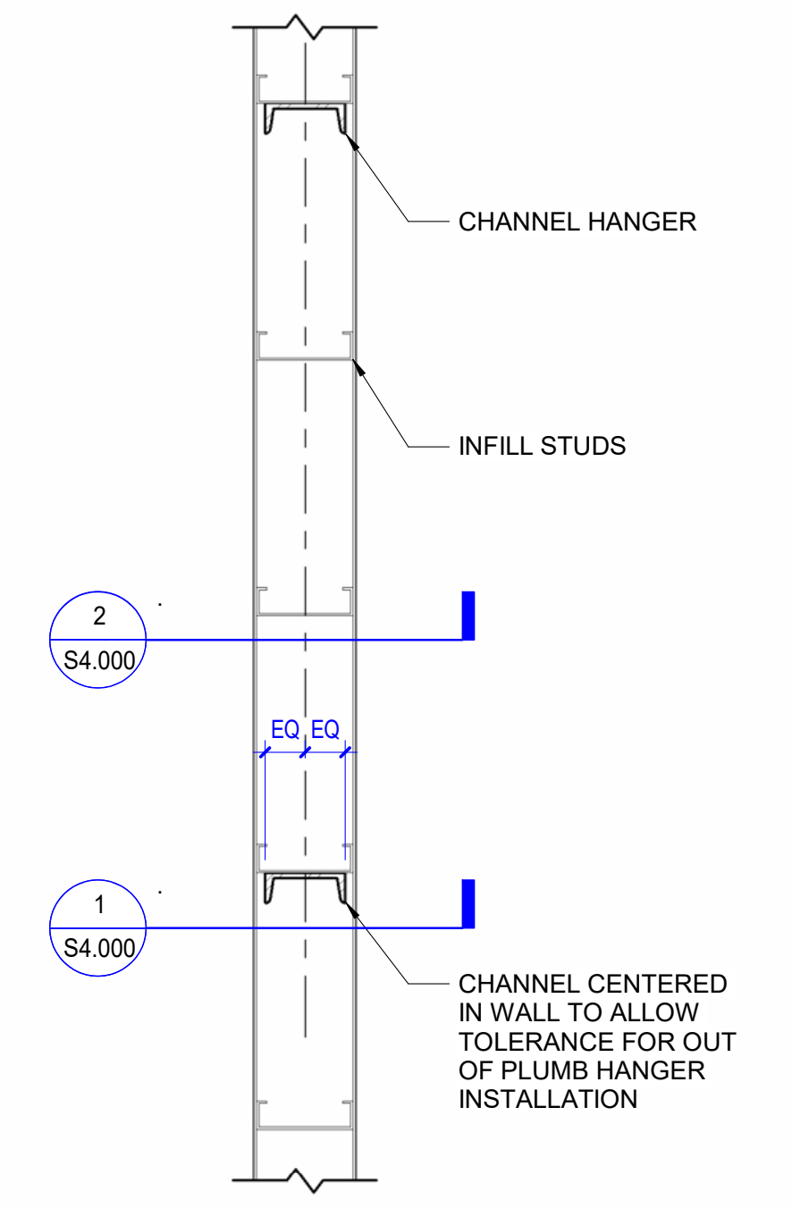
1" = 1'-0"



EMBED PLATE		
COLUMN SIZE	EDGE CONDITION	INTERIOR CONDITION
HSS4X4	WP-E10X10.2	WP-D10X10.2
HSS6X4	WP-E12X12.3	WP-D12X12.4
HSS6X6	WP-E12X12.3	WP-D12X12.4
HSS8X6	WP-E14X14.2	WP-D14X14.2
HSS8X8	WP-E14X14.2	WP-D14X14.2

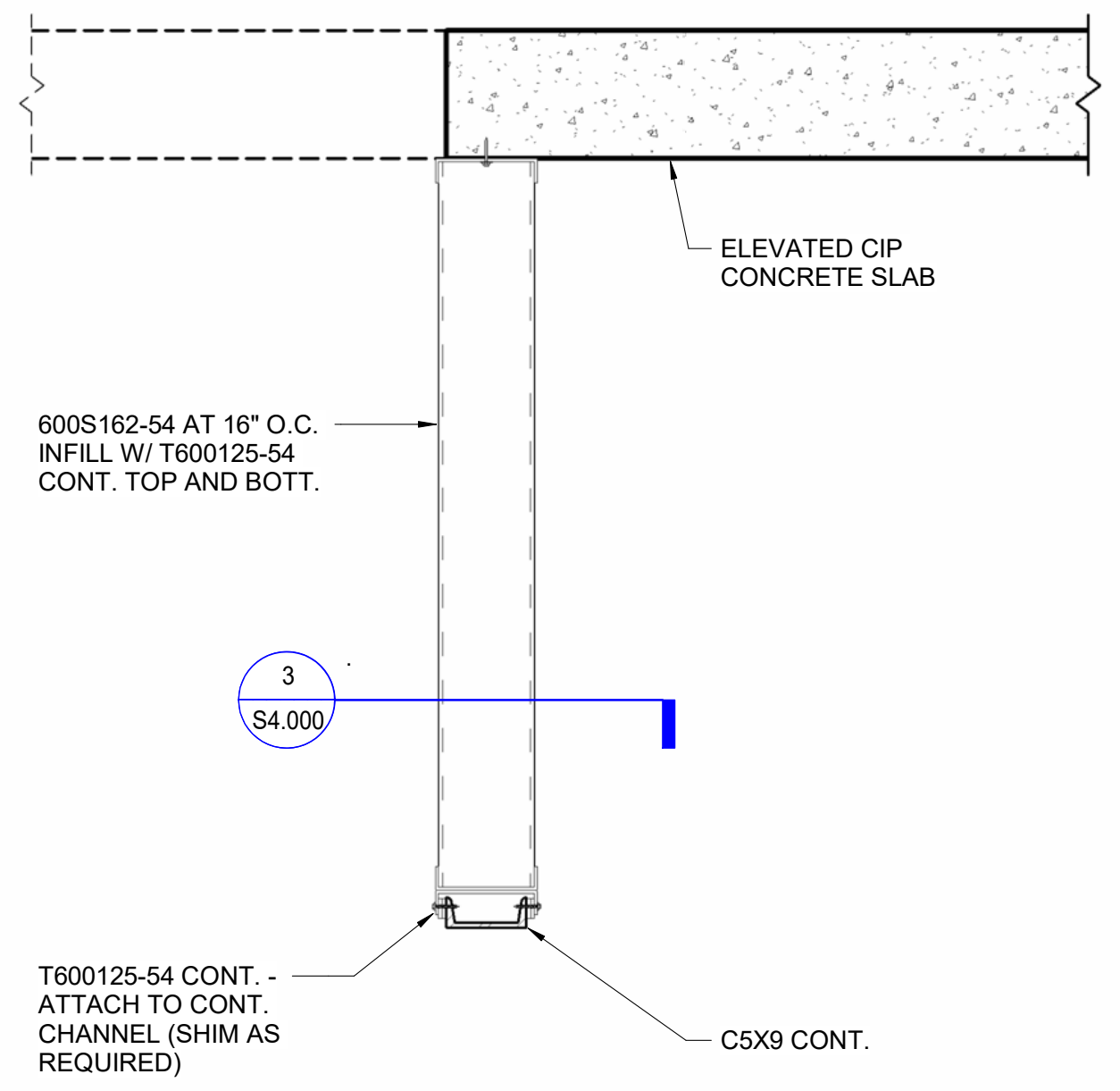
7 HSS Post Base Connection

1" = 1'-0"


CHANNEL HANGER
INFILL STUDS
2
S4.000
EQ
EQ
1
S4.000
CHANNEL CENTERED IN WALL TO ALLOW TOLERANCE FOR OUT OF PLUMB HANGER INSTALLATION

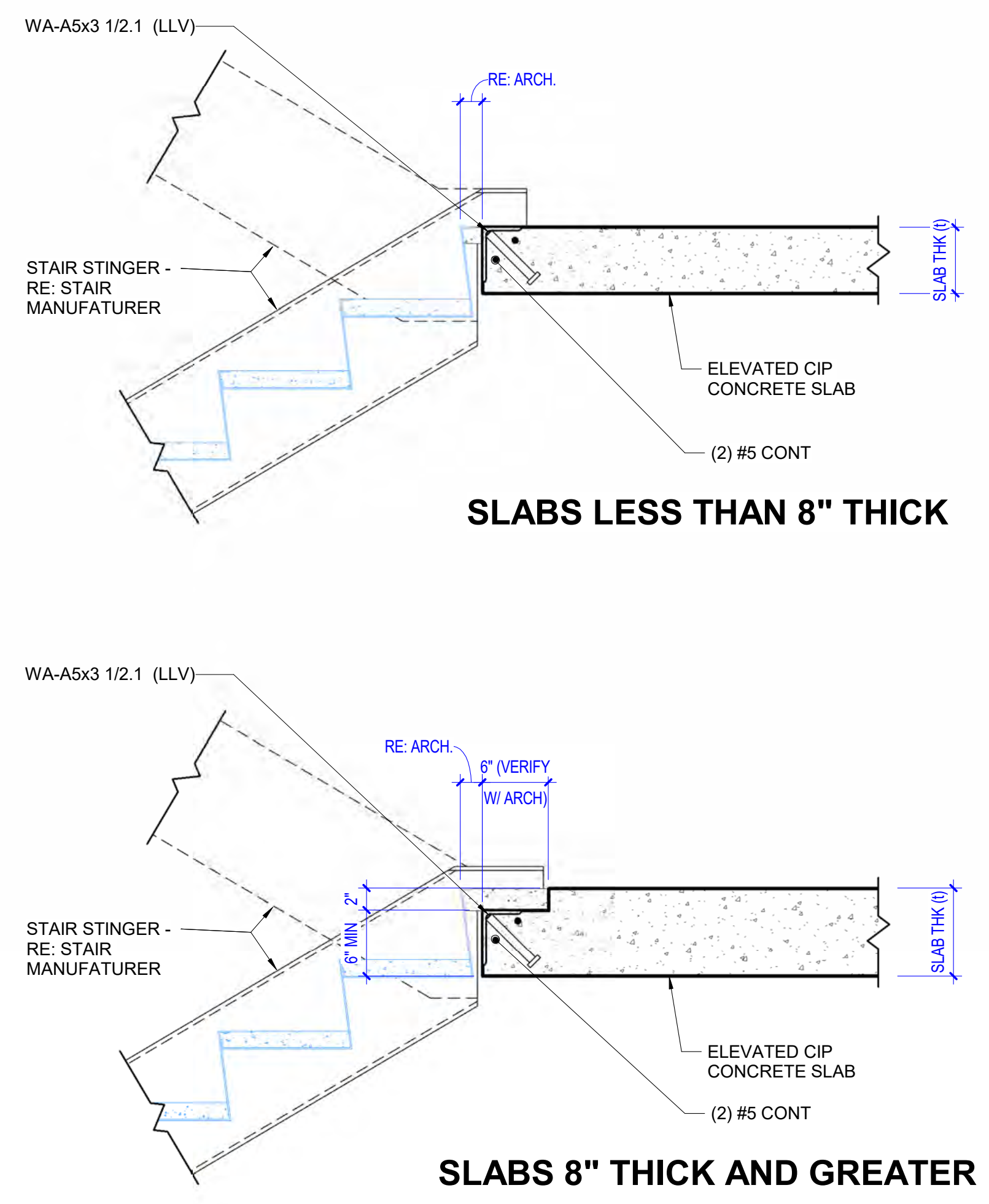
3 Channel Hanger - Detail

1" = 1'-0"


ELEVATED CIP CONCRETE SLAB
600S162-54 AT 16" O.C. INFILL W/ T600125-54 CONT. TOP AND BOTT.
3
S4.000
T600125-54 CONT. - ATTACH TO CONT. CHANNEL (SHIM AS REQUIRED)
C5X9 CONT.

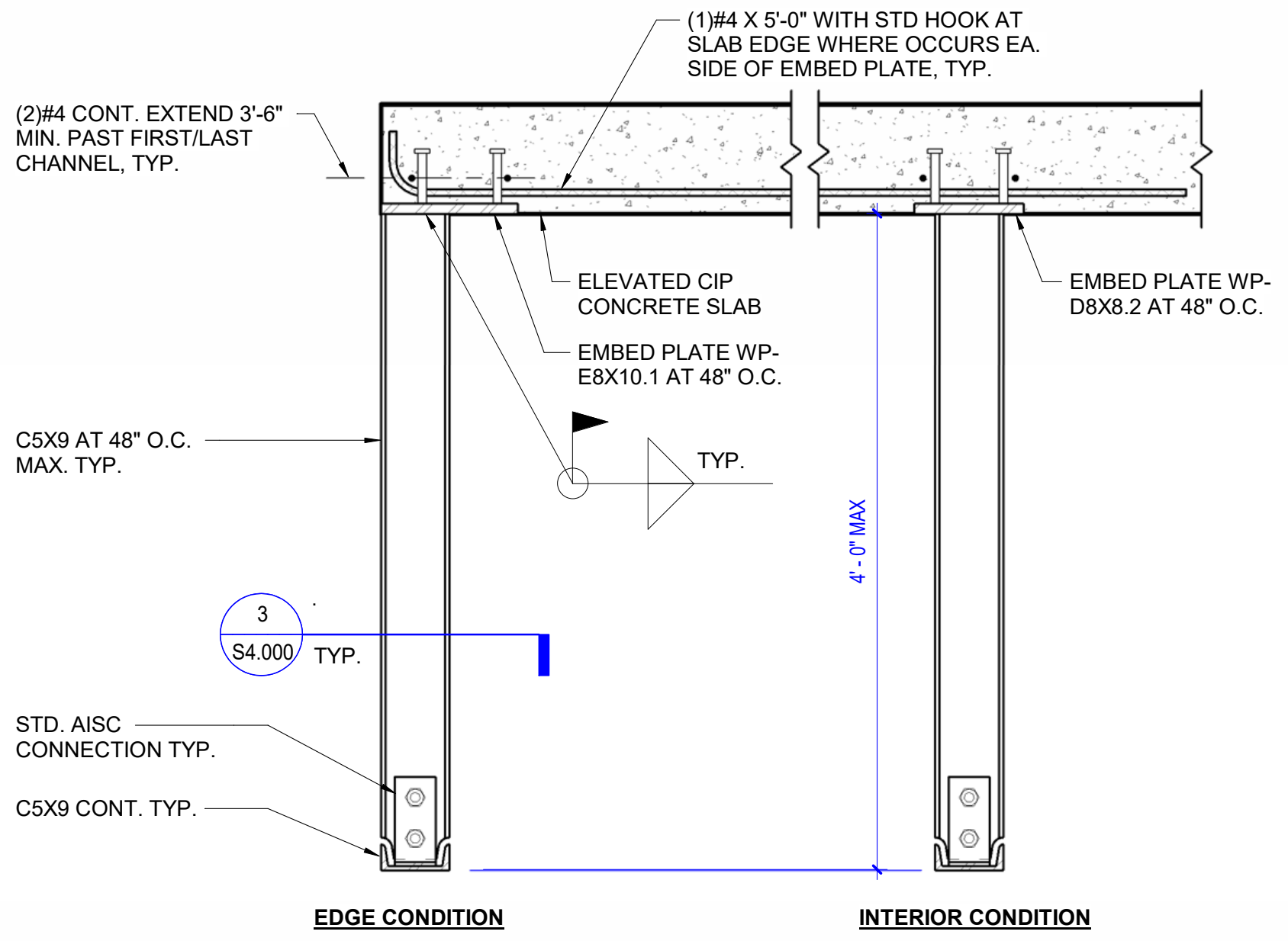
2 Channel Hanger - Infill

1" = 1'-0"



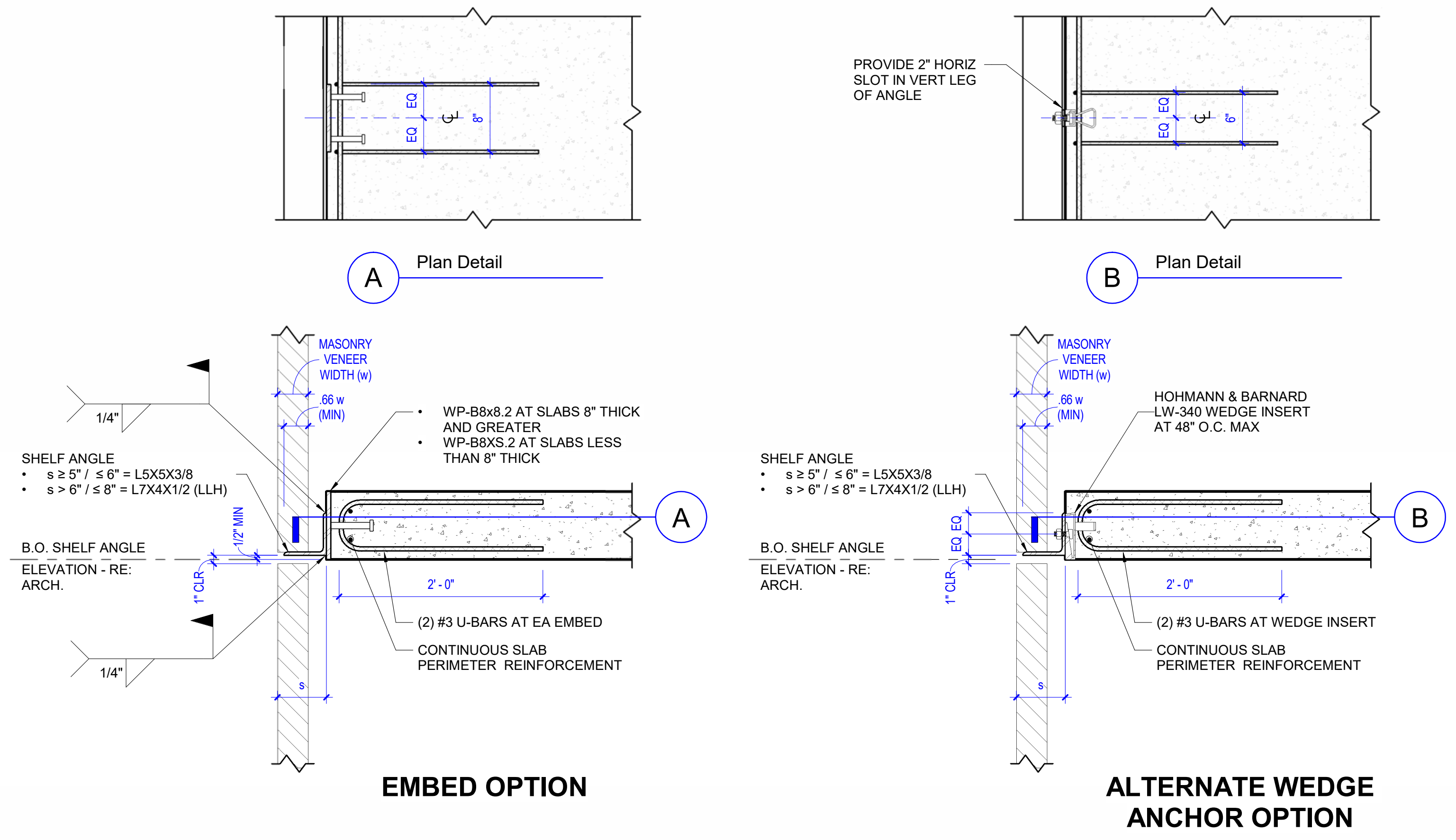
6 Steel Stair Stringer Connection

1" = 1'-0"


(1)#4 X 5'-0" WITH STD HOOK AT SLAB EDGE WHERE OCCURS EA. SIDE OF EMBED PLATE, TYP.
(2)#4 CONT. EXTEND 3'-6" MIN. PAST FIRST/LAST CHANNEL, TYP.
ELEVATED CIP CONCRETE SLAB
EMBED PLATE WP-E8X10.1 AT 48" O.C.
EMBED PLATE WP-D8X8.2 AT 48" O.C.
C5X9 AT 48" O.C. MAX. TYP.
TYP.
4' - 0" MAX
3
S4.000
TYP.
STD. AISC CONNECTION TYP.
C5X9 CONT. TYP.
EDGE CONDITION
INTERIOR CONDITION

1 Channel Hanger - 4' Max

1" = 1'-0"


EQ
EQ
8"
A
Plan Detail
MASONRY VENEER WIDTH (w)
.66 w (MIN)
1/4"
SHELF ANGLE
s ≥ 5" / ≤ 6" = L5X5X3/8
s > 6" / ≤ 8" = L7X4X1/2 (LLH)
WP-B8x8.2 AT SLABS 8" THICK AND GREATER
WP-B8XS.2 AT SLABS LESS THAN 8" THICK
A
B.O. SHELF ANGLE
ELEVATION - RE: ARCH.
1/2" MIN
1" CLR
2' - 0"
1/4"
(2) #3 U-BARS AT EA EMBED
CONTINUOUS SLAB PERIMETER REINFORCEMENT
s
EMBED OPTION
PROVIDE 2" HORIZ SLOT IN VERT LEG OF ANGLE
EQ
EQ
6"
B
Plan Detail
MASONRY VENEER WIDTH (w)
.66 w (MIN)
HOHMANN & BARNARD LW-340 WEDGE INSERT AT 48" O.C. MAX
SHELF ANGLE
s ≥ 5" / ≤ 6" = L5X5X3/8
s > 6" / ≤ 8" = L7X4X1/2 (LLH)
B
B.O. SHELF ANGLE
ELEVATION - RE: ARCH.
EQ
EQ
1" CLR
2' - 0"
(2) #3 U-BARS AT WEDGE INSERT
CONTINUOUS SLAB PERIMETER REINFORCEMENT
s
ALTERNATE WEDGE ANCHOR OPTION

9 Masonry Veneer Shelf Angle

1" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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No.	Description	Date
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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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SHEET TITLE:
Typical Concrete Connections

SHEET NUMBER:
S4.000

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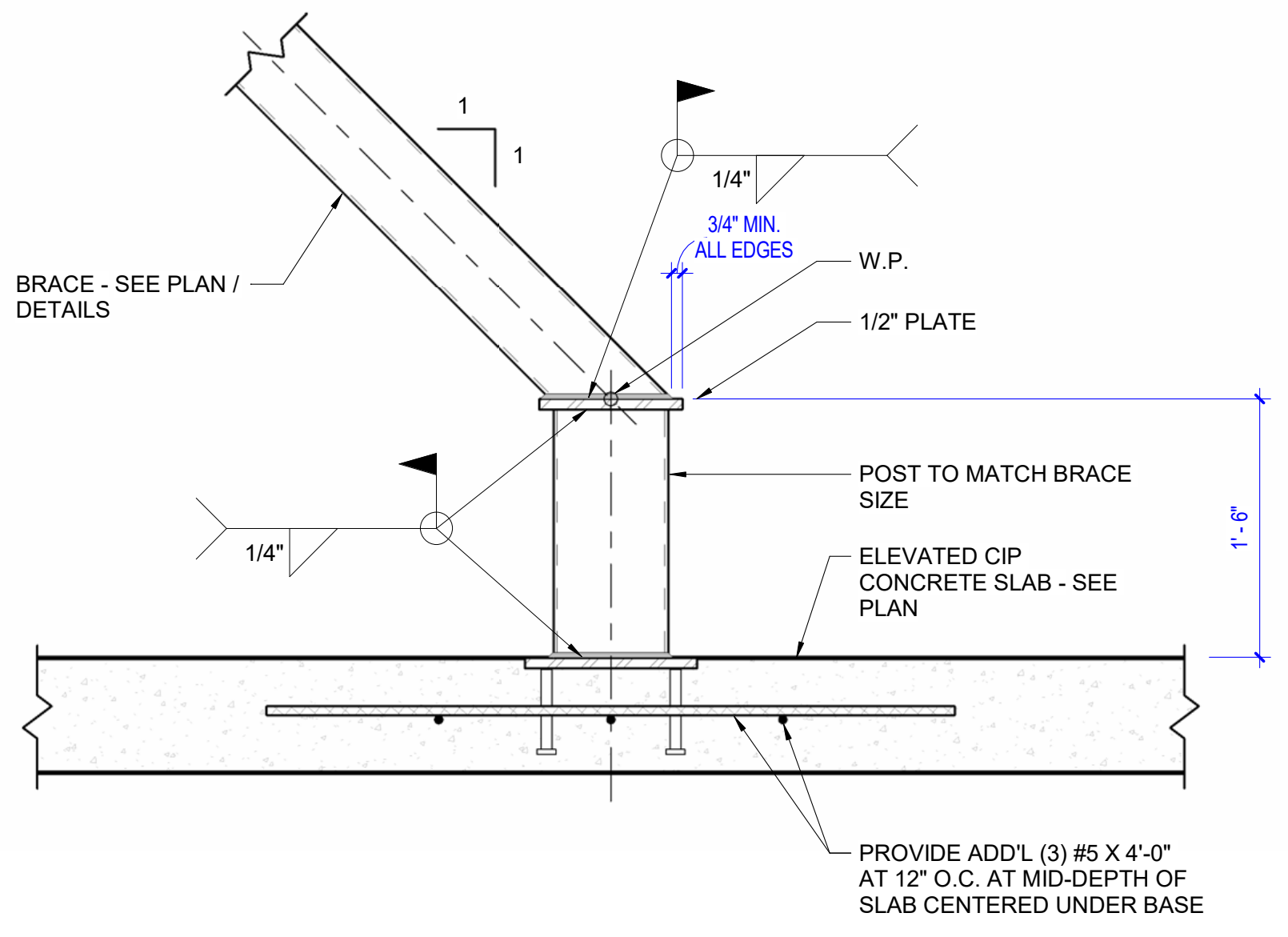
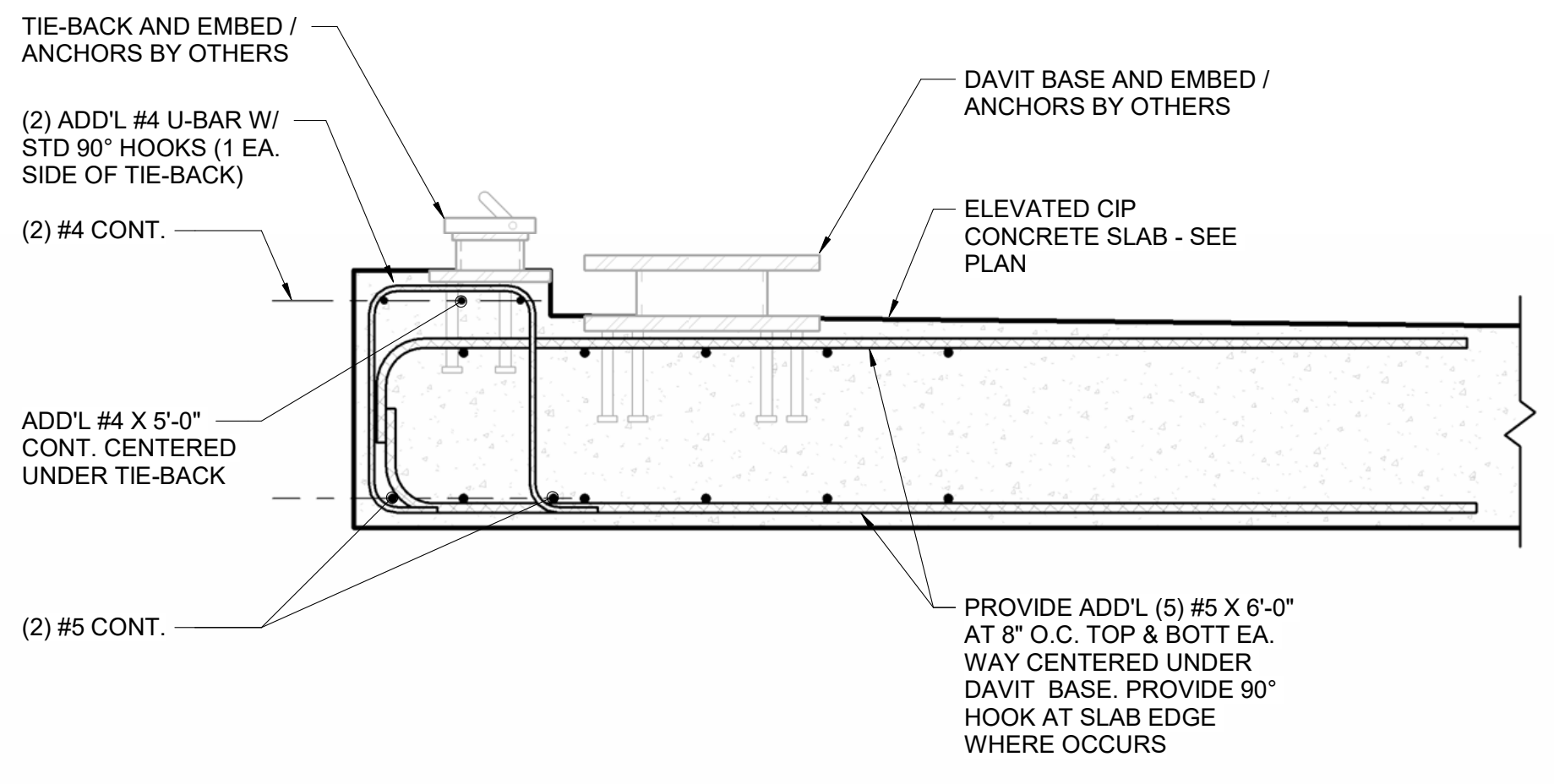
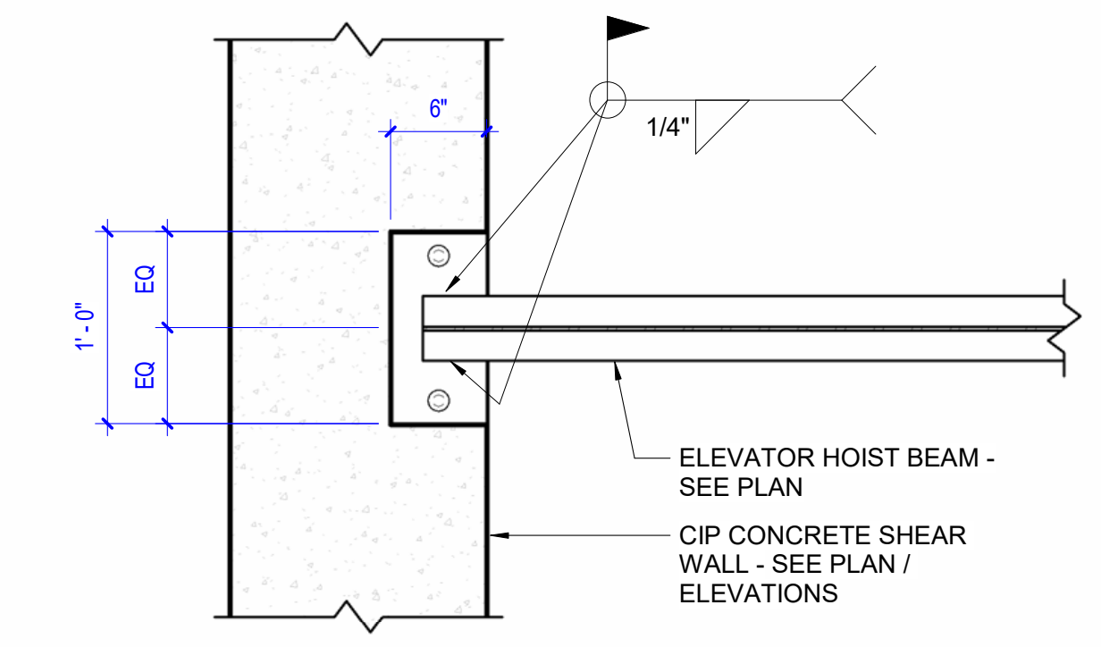

BRACE - SEE PLAN / DETAILS
1
1
1/4"
3/4" MIN. ALL EDGES
W.P.
1/2" PLATE
1/4"
POST TO MATCH BRACE SIZE
ELEVATED CIP CONCRETE SLAB - SEE PLAN
1' - 6"
PROVIDE ADD'L (3) #5 X 4'-0" AT 12" O.C. AT MID-DEPTH OF SLAB CENTERED UNDER BASE

④ Roof Screen Brace Post
1" = 1'-0"

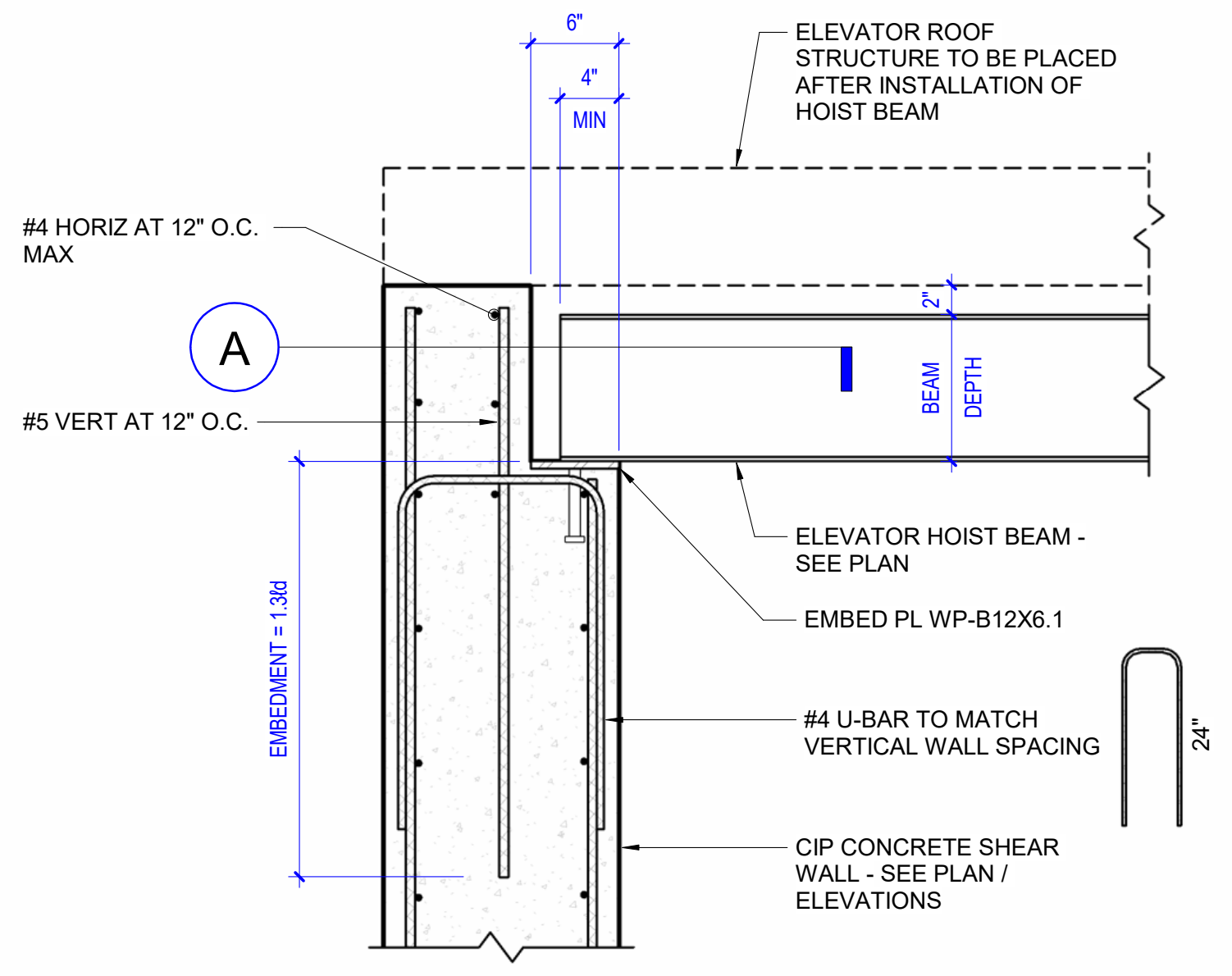

TIE-BACK AND EMBED / ANCHORS BY OTHERS
(2) ADD'L #4 U-BAR W/ STD 90° HOOKS (1 EA. SIDE OF TIE-BACK)
(2) #4 CONT.
ADD'L #4 X 5'-0" CONT. CENTERED UNDER TIE-BACK
(2) #5 CONT.
DAVIT BASE AND EMBED / ANCHORS BY OTHERS
ELEVATED CIP CONCRETE SLAB - SEE PLAN
PROVIDE ADD'L (5) #5 X 6'-0" AT 8" O.C. TOP & BOTT EA. WAY CENTERED UNDER DAVIT BASE. PROVIDE 90° HOOK AT SLAB EDGE WHERE OCCURS

WINDOW WASHING DAVIT BASE / TIE-BACK

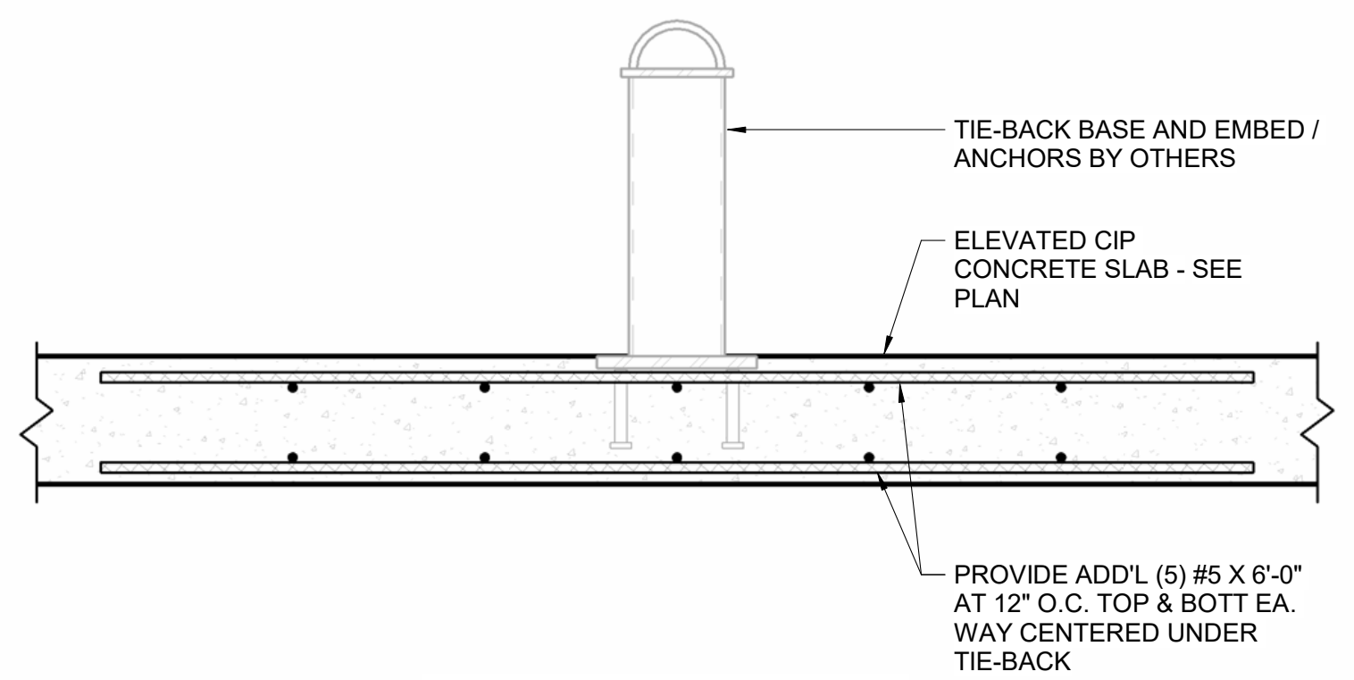
③ Davit_Tie Back At Plaza Deck
1" = 1'-0"


6"
1/4"
1' - 0"
EQ
EQ
ELEVATOR HOIST BEAM - SEE PLAN
CIP CONCRETE SHEAR WALL - SEE PLAN / ELEVATIONS

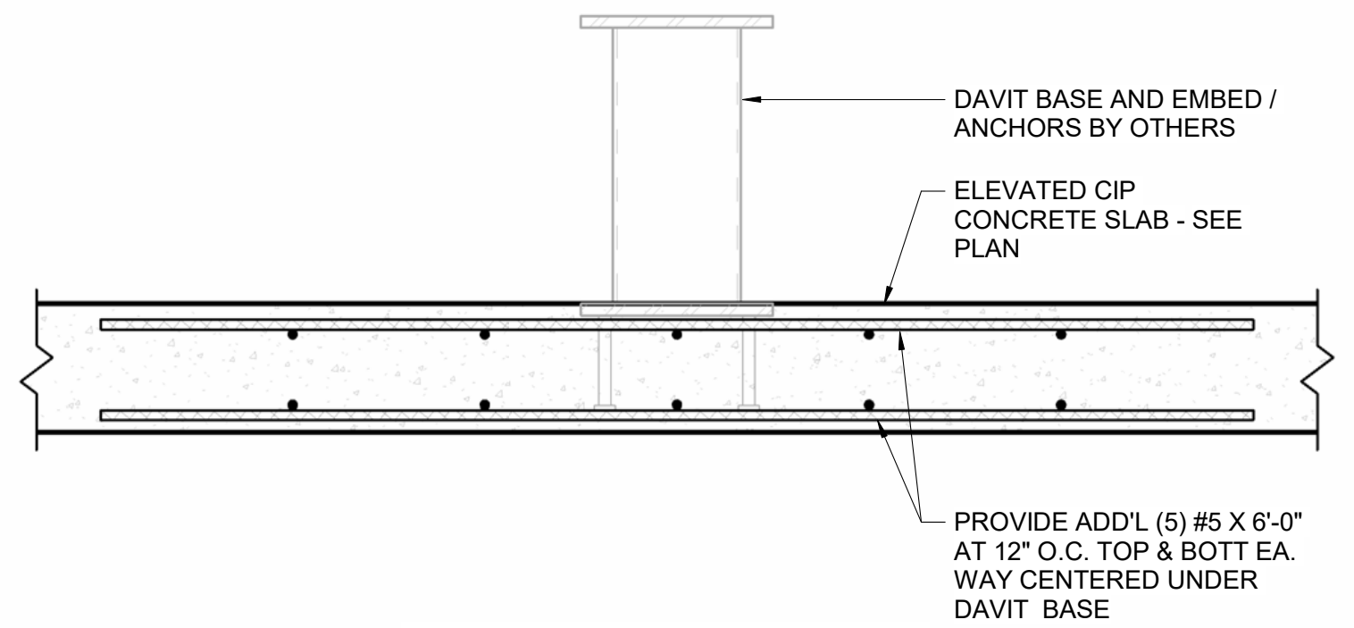
A PLAN VIEW


6"
4"
MIN
ELEVATOR ROOF STRUCTURE TO BE PLACED AFTER INSTALLATION OF HOIST BEAM
#4 HORIZ AT 12" O.C. MAX
A
#5 VERT AT 12" O.C.
2"
BEAM DEPTH
EMBEDMENT = 1.3ℓd
ELEVATOR HOIST BEAM - SEE PLAN
EMBED PL WP-B12X6.1
#4 U-BAR TO MATCH VERTICAL WALL SPACING
24"
CIP CONCRETE SHEAR WALL - SEE PLAN / ELEVATIONS

⑤ Elevator Hoist Beam Bearing
1" = 1'-0"

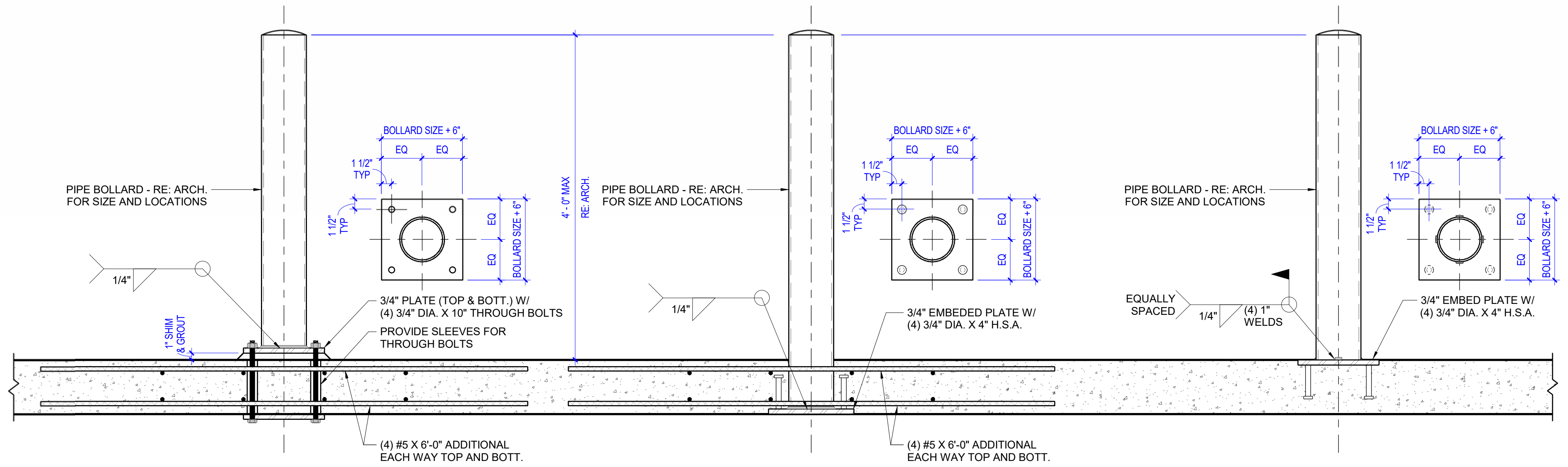

TIE-BACK BASE AND EMBED / ANCHORS BY OTHERS
ELEVATED CIP CONCRETE SLAB - SEE PLAN
PROVIDE ADD'L (5) #5 X 6'-0" AT 12" O.C. TOP & BOTT EA. WAY CENTERED UNDER TIE-BACK

WINDOW WASHING TIE-BACK


DAVIT BASE AND EMBED / ANCHORS BY OTHERS
ELEVATED CIP CONCRETE SLAB - SEE PLAN
PROVIDE ADD'L (5) #5 X 6'-0" AT 12" O.C. TOP & BOTT EA. WAY CENTERED UNDER DAVIT BASE

WINDOW WASHING DAVIT BASE

② Davit_Tie Back Base
1" = 1'-0"


PIPE BOLLARD - RE: ARCH. FOR SIZE AND LOCATIONS
BOLLARD SIZE + 6"
EQ
EQ
1 1/2" TYP
1 1/2" TYP
BOLLARD SIZE + 6"
1/4"
1" SHIM & GROUT
3/4" PLATE (TOP & BOTT.) W/ (4) 3/4" DIA. X 10" THROUGH BOLTS
PROVIDE SLEEVES FOR THROUGH BOLTS
(4) #5 X 6'-0" ADDITIONAL EACH WAY TOP AND BOTT.
4' - 0" MAX RE: ARCH.
3/4" EMBEDED PLATE W/ (4) 3/4" DIA. X 4" H.S.A.
EQUALLY SPACED
(4) 1" WELDS
3/4" EMBED PLATE W/ (4) 3/4" DIA. X 4" H.S.A.

VEHICLE IMPACT BOLLARD CONNECTION THROUGH BOLT OPTION

VEHICLE IMPACT BOLLARD CONNECTION EMBED OPTION

PEDESTIAN / MISC PROTECTION BOLLARD (NON VEHICLE IMPACT AREAS ONLY)

① Bollard Detail
1" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
Typical Concrete Connections

SHEET NUMBER:
S4.001

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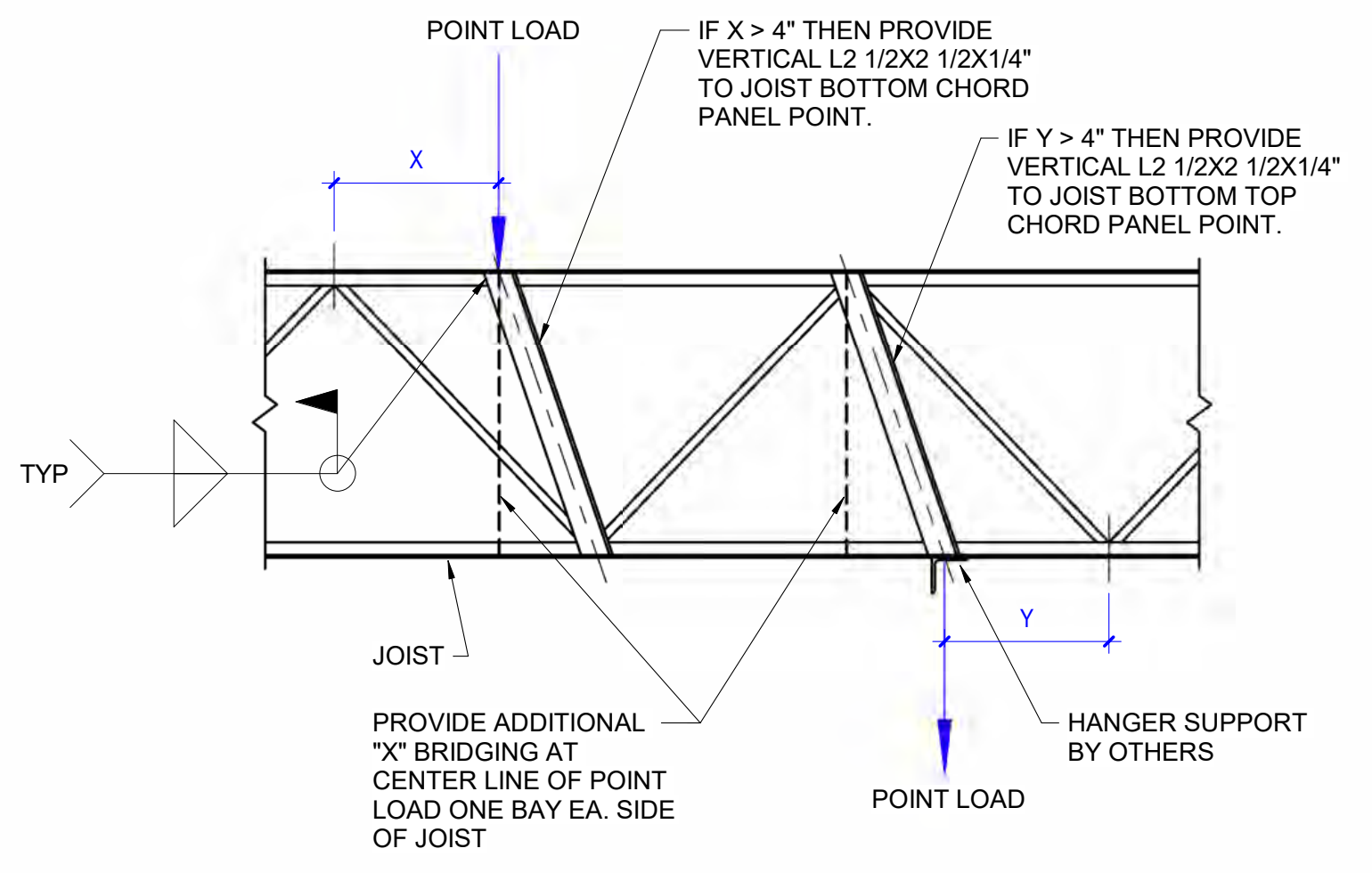
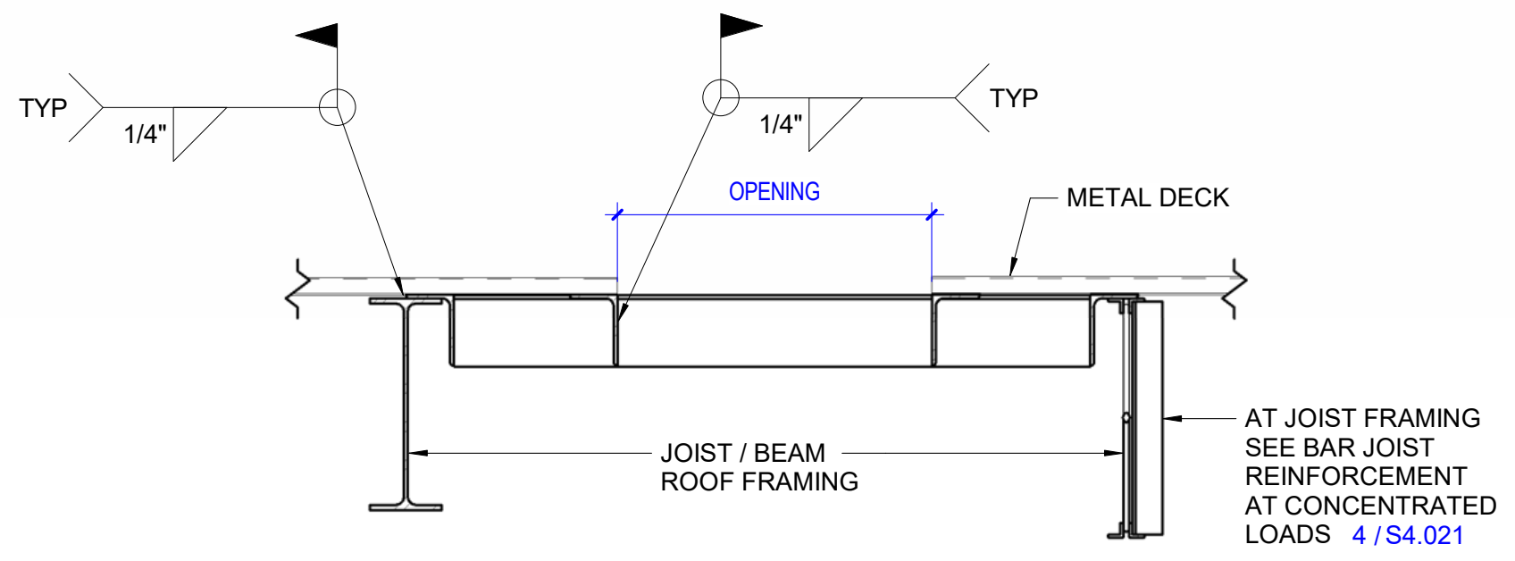
9/30/2026 7:34:07 PM

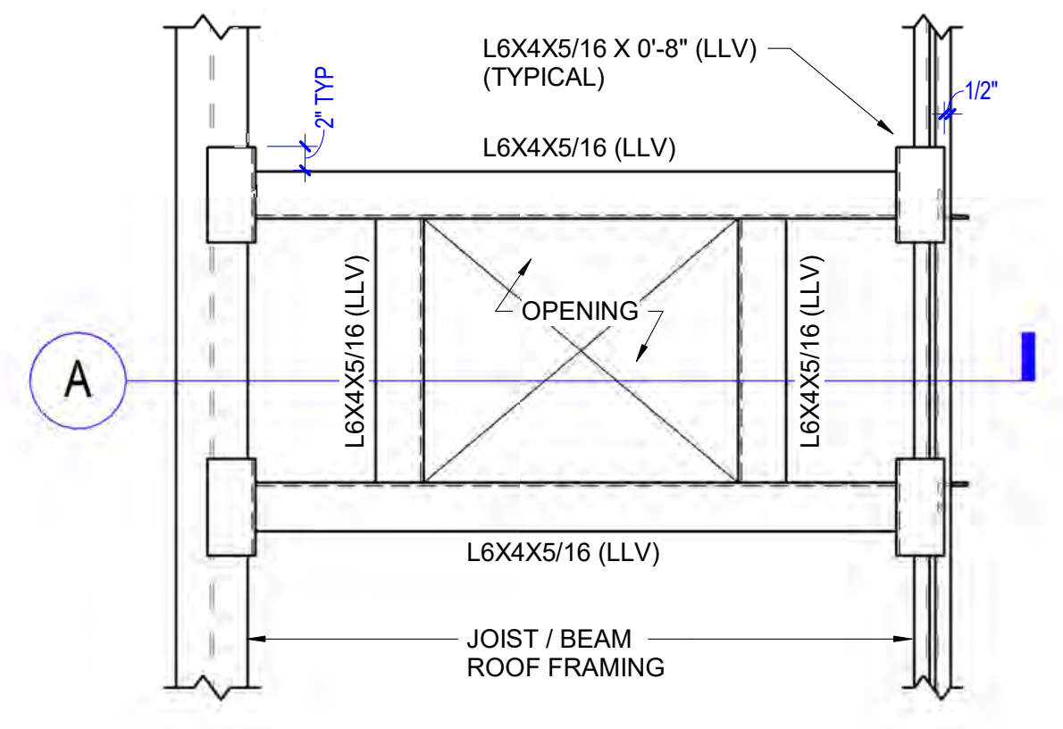

POINT LOAD
IF X > 4" THEN PROVIDE VERTICAL L2 1/2X2 1/2X1/4" TO JOIST BOTTOM CHORD PANEL POINT.
IF Y > 4" THEN PROVIDE VERTICAL L2 1/2X2 1/2X1/4" TO JOIST BOTTOM TOP CHORD PANEL POINT.
X
Y
TYP
JOIST
PROVIDE ADDITIONAL "X" BRIDGING AT CENTER LINE OF POINT LOAD ONE BAY EA. SIDE OF JOIST
POINT LOAD
HANGER SUPPORT BY OTHERS

NOTE:
1. COORDINATE ALL CONCENTRATE LOAD WITH MEP DRAWINGS

④ Bar Joist Reinfocement at Concentrated Loads
3/4" = 1'-0"

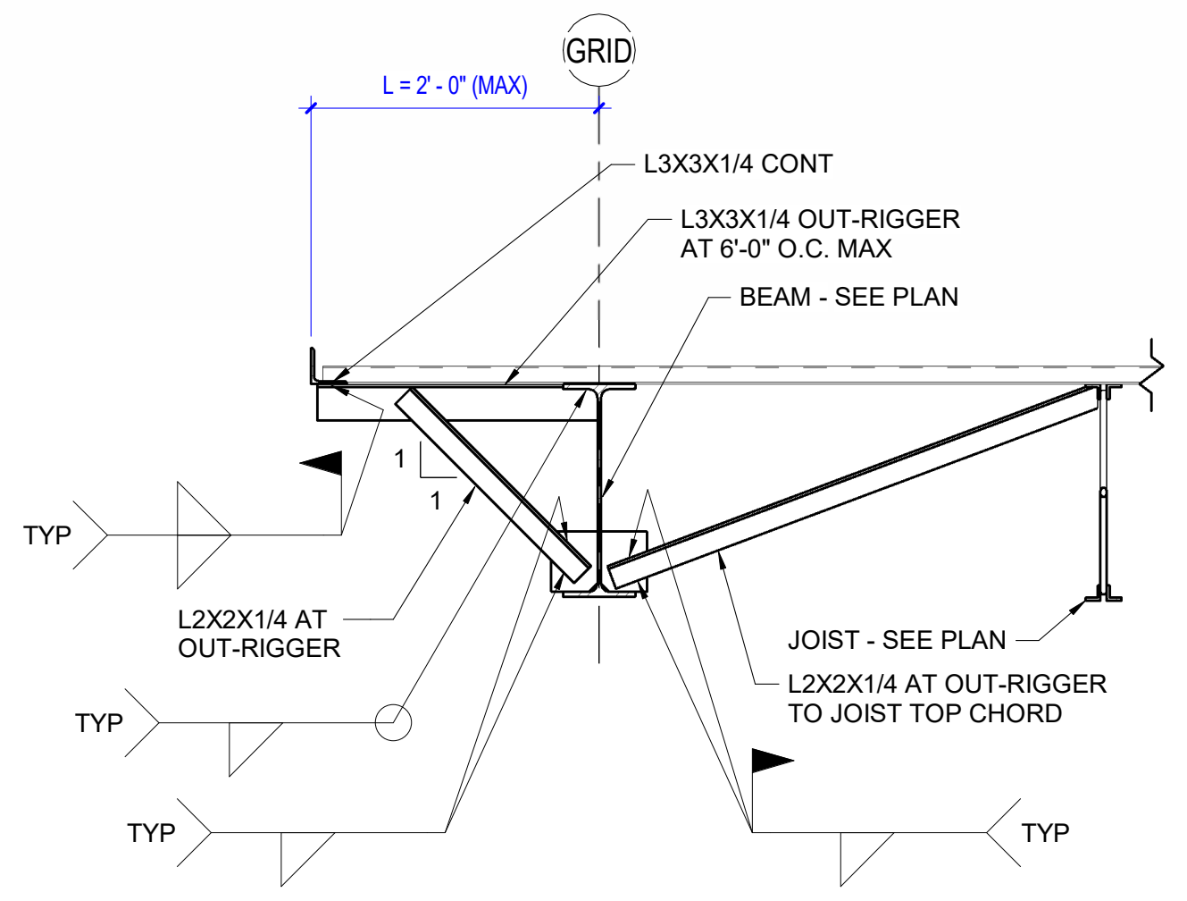

TYP
1/4"
1/4"
TYP
OPENING
METAL DECK
JOIST / BEAM ROOF FRAMING
AT JOIST FRAMING SEE BAR JOIST REINFORCEMENT AT CONCENTRATED LOADS 4 / S4.021

Ⓐ Detail at Opening

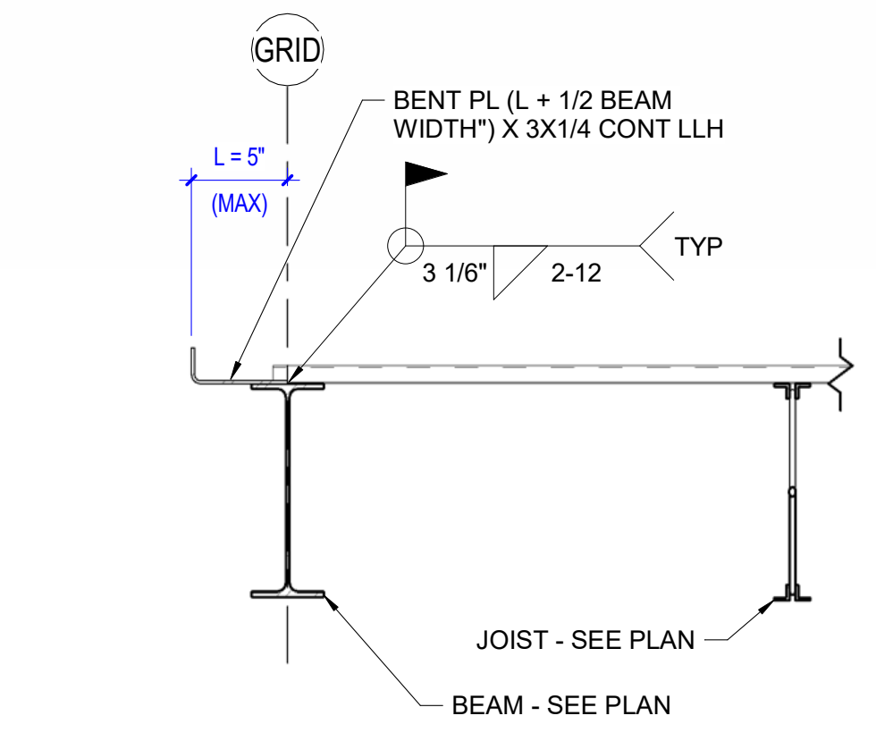


NOTE:
1. PROVIDE OPENING FRAMING AT ALL ROOF TOP MECHANICAL UNITS (RTU) AND OTHER OPENINGS
2. RE: ARCH / MECHANICAL DRAWINGS FOR OPENING SIZE AND LOCATIONS
3. OPENING IN DECK TO BE CUT IMMEDIATELY PRIOR TO INSTALLATION OF RTU'S AND MECHANICAL CURBS

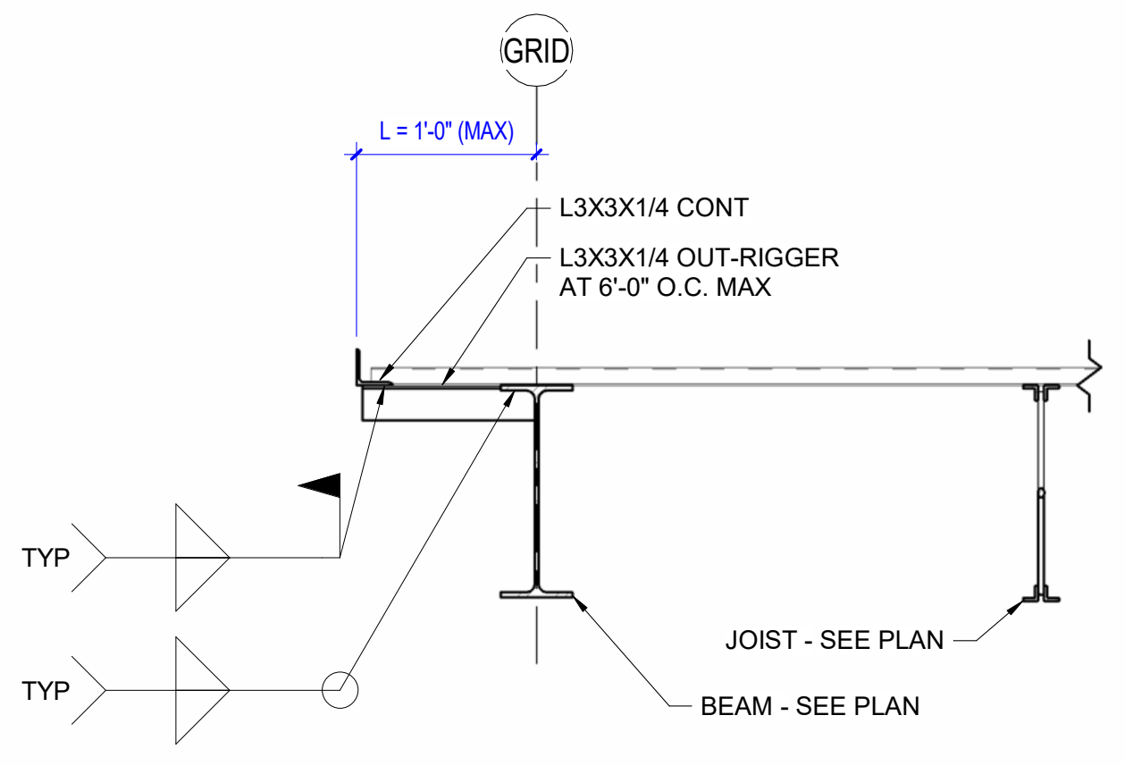
③ Typical Framing at Opening
3/4" = 1'-0"



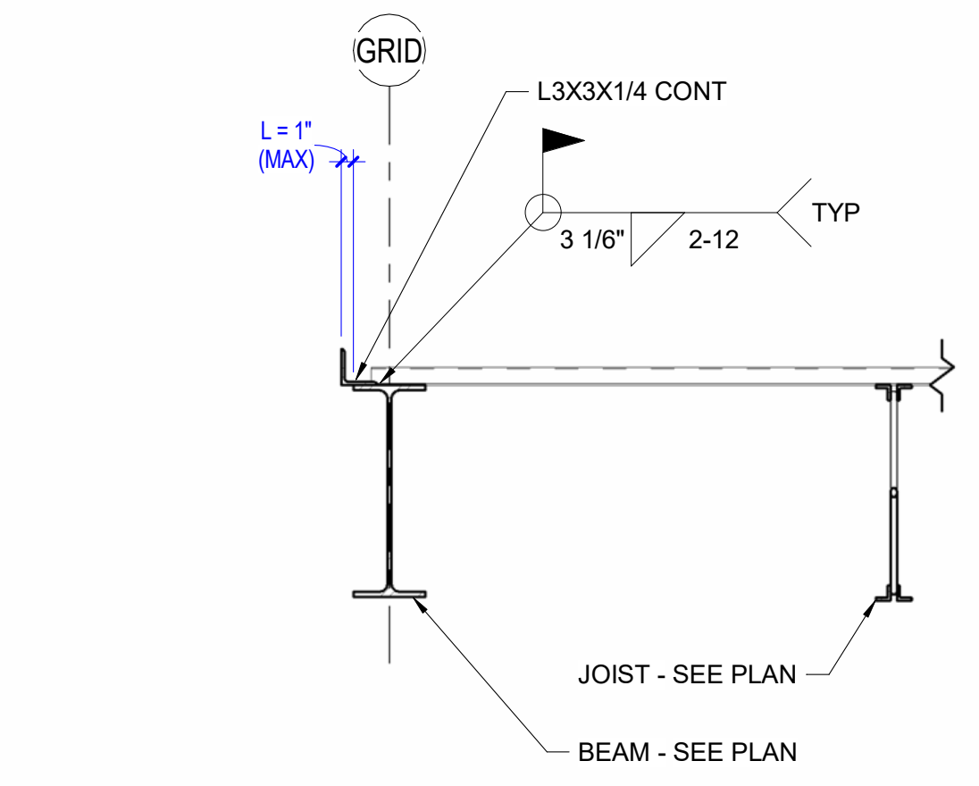
2D Joist Parallel WF Beam - Edge Condition
3/4" = 1'-0"



2B Joist Parallel WF Beam - Edge Condition
3/4" = 1'-0"



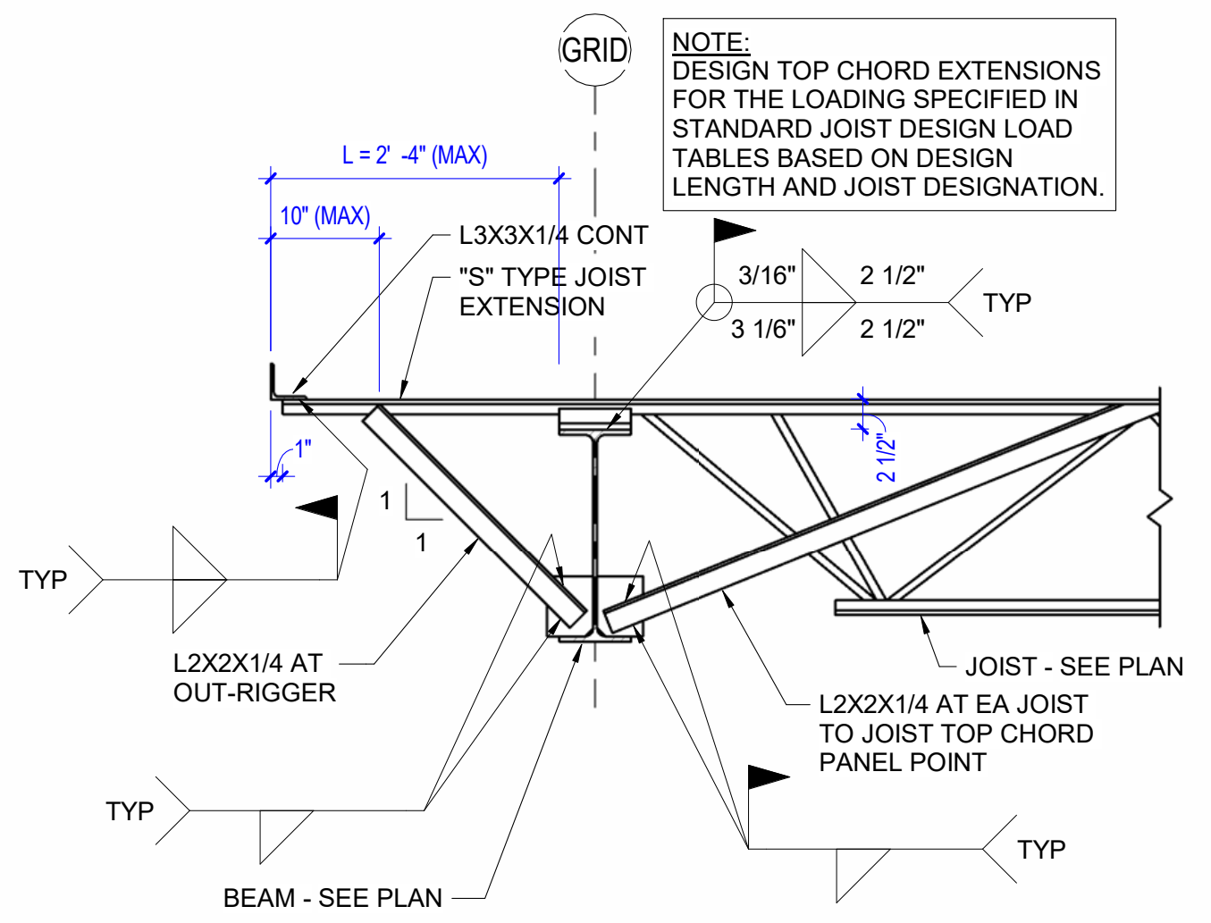
2C Joist Parallel WF Beam - Edge Condition
3/4" = 1'-0"



2A Joist Parallel WF Beam - Edge Condition
3/4" = 1'-0"

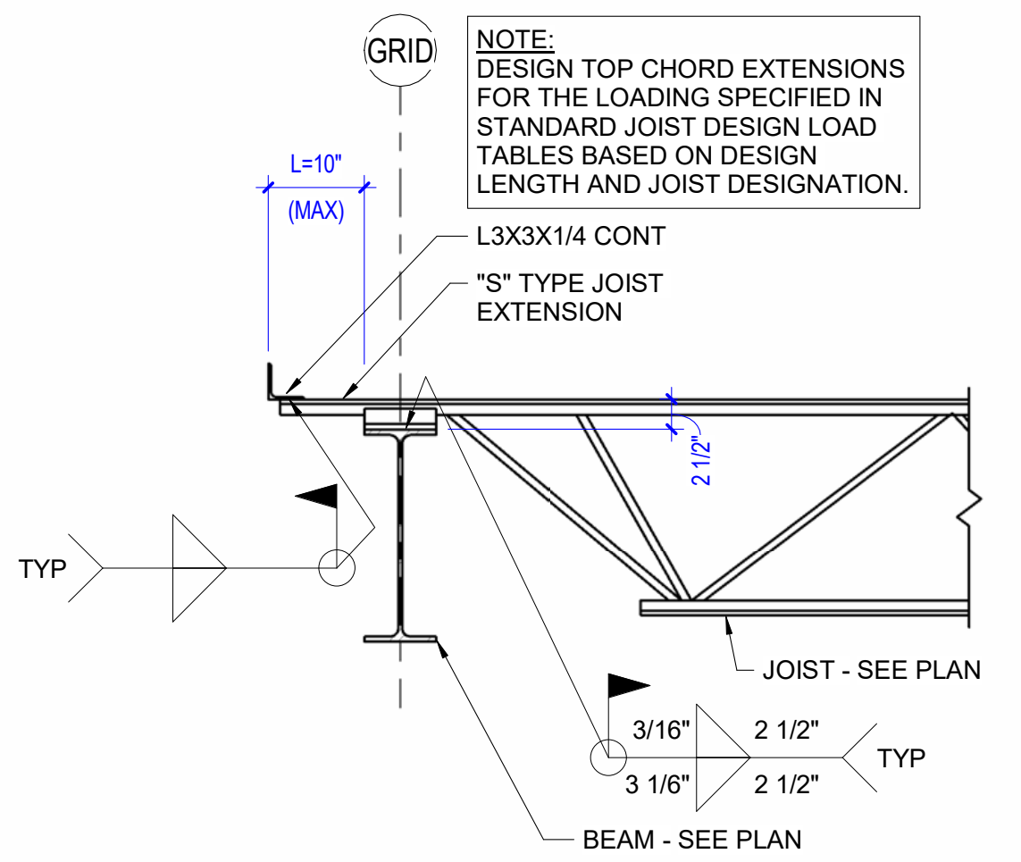
EDGE DISTANCE	
EDGE DISTANCE "L"	DETAIL
1" OR LESS	2A / S4.021
>1" TO 5"	2B / S4.021
>5" TO 1'-0"	2C / S4.021
>1'-0" TO 2'-0"	2D / S4.021

② Joist Parallel WF Beam - Edge Condition
3/4" = 1'-0"



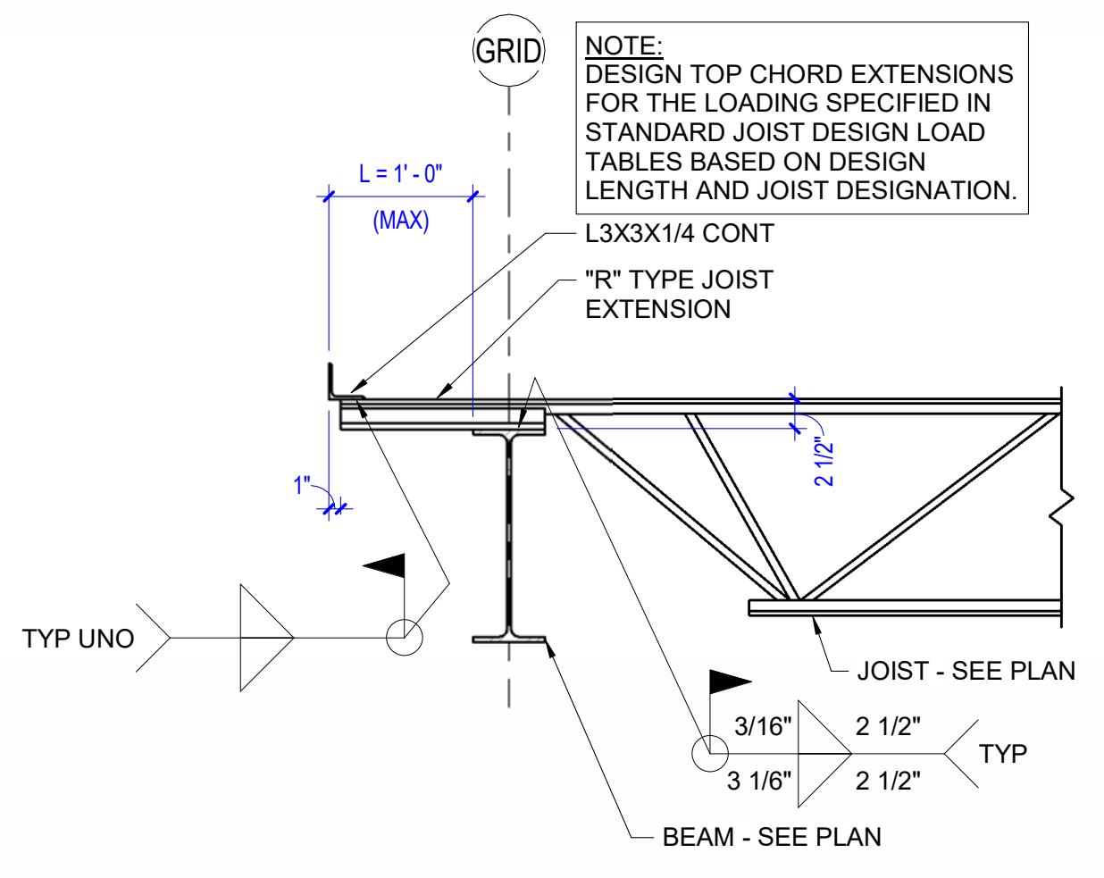
NOTE:
DESIGN TOP CHORD EXTENSIONS FOR THE LOADING SPECIFIED IN STANDARD JOIST DESIGN LOAD TABLES BASED ON DESIGN LENGTH AND JOIST DESIGNATION.

1D Joist Bearing WF Beam - Edge Condition
3/4" = 1'-0"



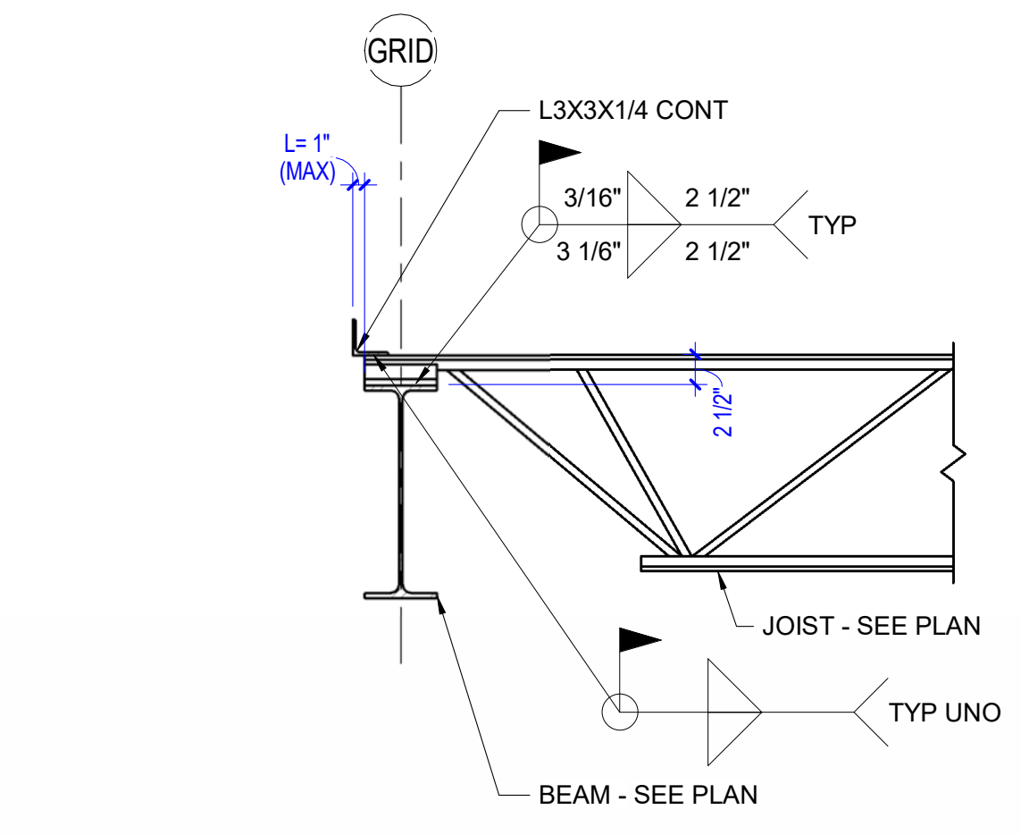
NOTE:
DESIGN TOP CHORD EXTENSIONS FOR THE LOADING SPECIFIED IN STANDARD JOIST DESIGN LOAD TABLES BASED ON DESIGN LENGTH AND JOIST DESIGNATION.

1B Joist Bearing WF Beam - Edge Condition
3/4" = 1'-0"



NOTE:
DESIGN TOP CHORD EXTENSIONS FOR THE LOADING SPECIFIED IN STANDARD JOIST DESIGN LOAD TABLES BASED ON DESIGN LENGTH AND JOIST DESIGNATION.

1C Joist Bearing WF Beam - Edge Condition
3/4" = 1'-0"



1A Joist Bearing WF Beam - Edge Condition
3/4" = 1'-0"

EDGE DISTANCE	
EDGE DISTANCE "L"	DETAIL
1" OR LESS	1A / S4.021
>1" TO 10"	1B / S4.021
>10" TO 1'-0"	1C / S4.021
>1'-0" TO 2'-4"	1D / S4.021

① Joist Bearing WF Beam - Edge Condition
3/4" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMeg Structural Engineers, LLC
1080 Eldridge Parkway
Suite 350
Houston, TX 77077
281-407-5734
www.vsmeg.com

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DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
Typical Steel Bar Joist Details

SHEET NUMBER:
S4.021

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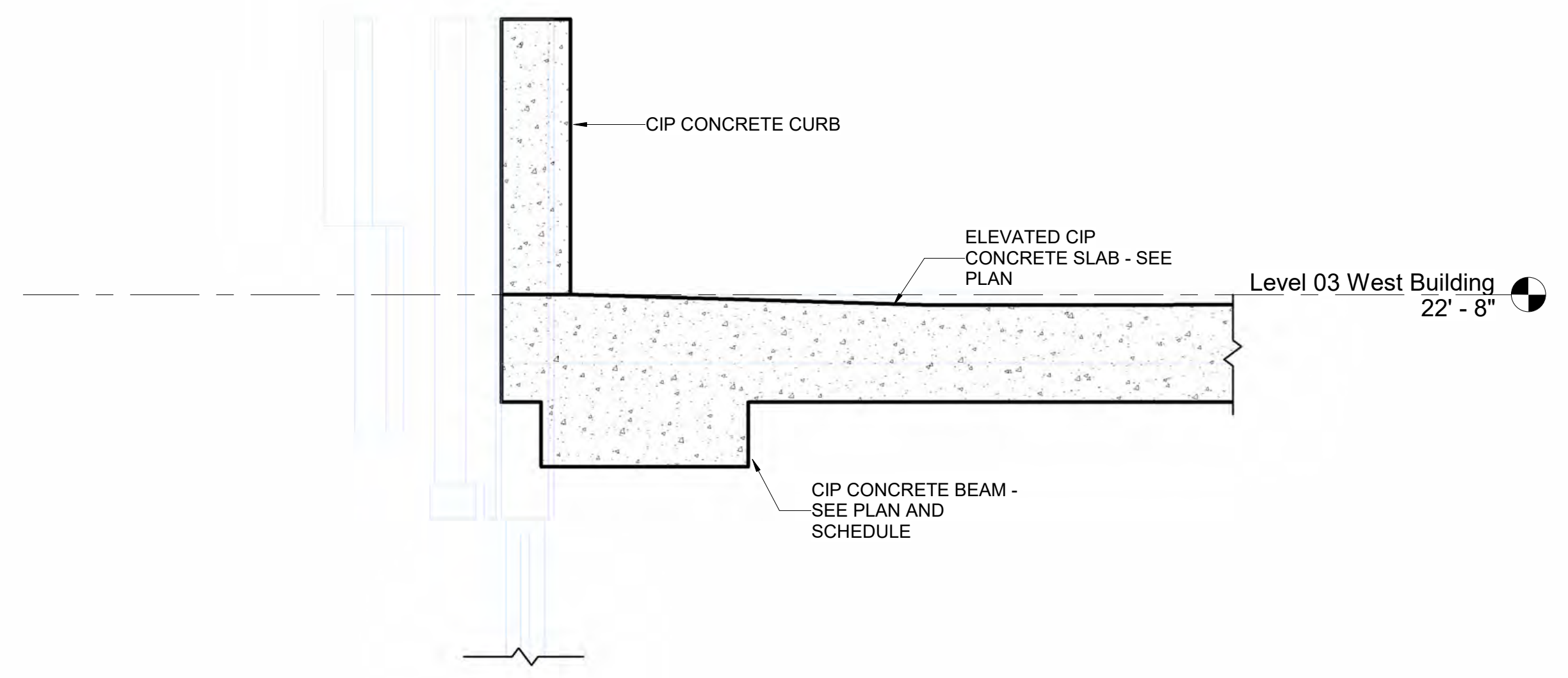
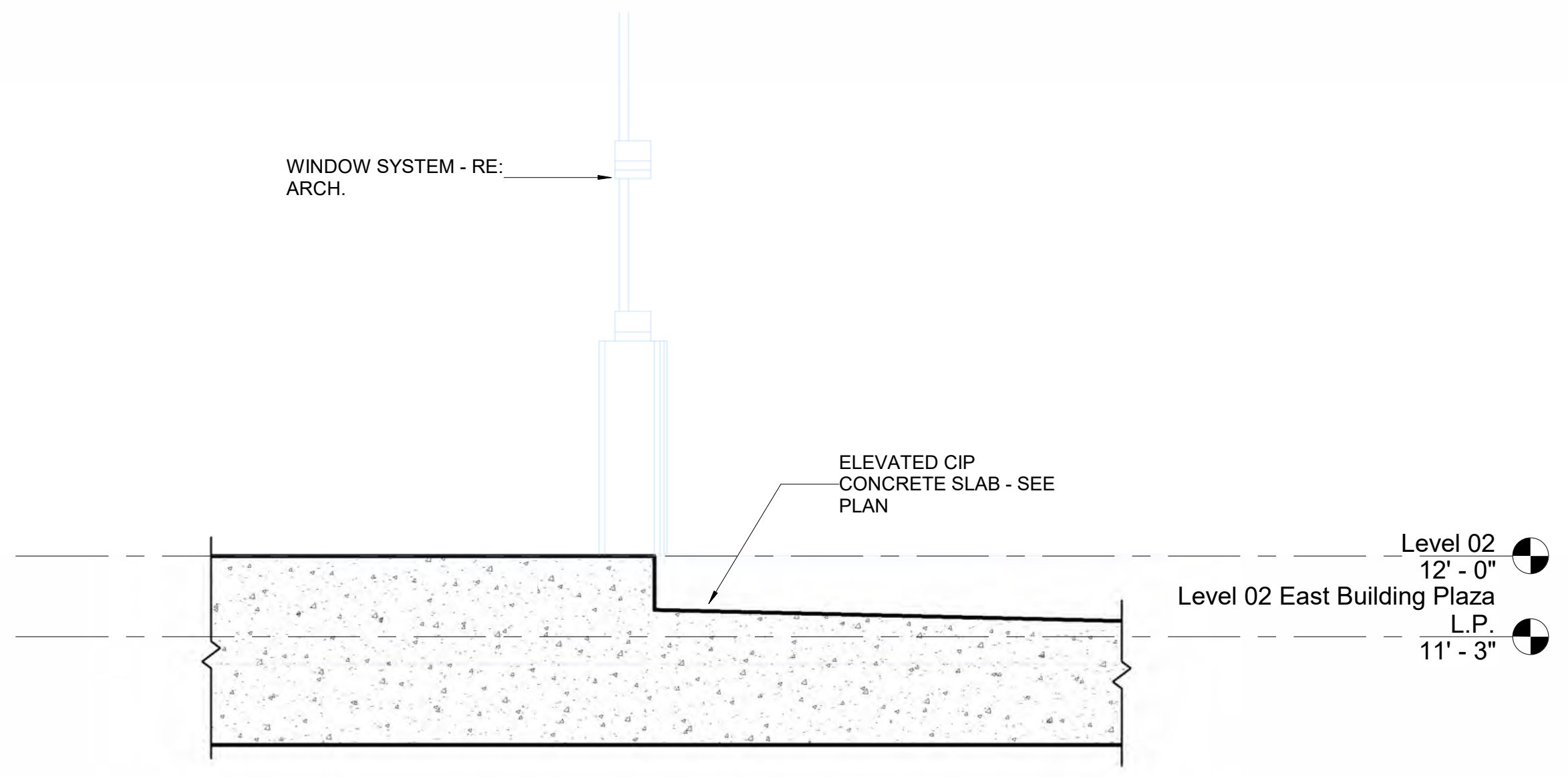
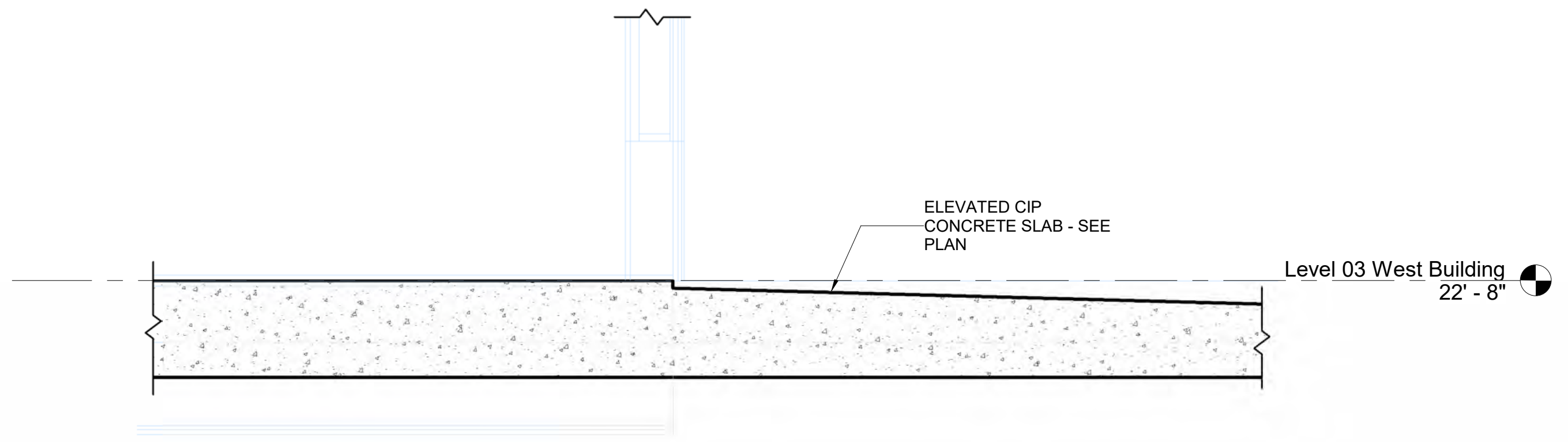

CIP CONCRETE CURB
ELEVATED CIP CONCRETE SLAB - SEE PLAN
Level 03 West Building
22' - 8"
CIP CONCRETE BEAM - SEE PLAN AND SCHEDULE

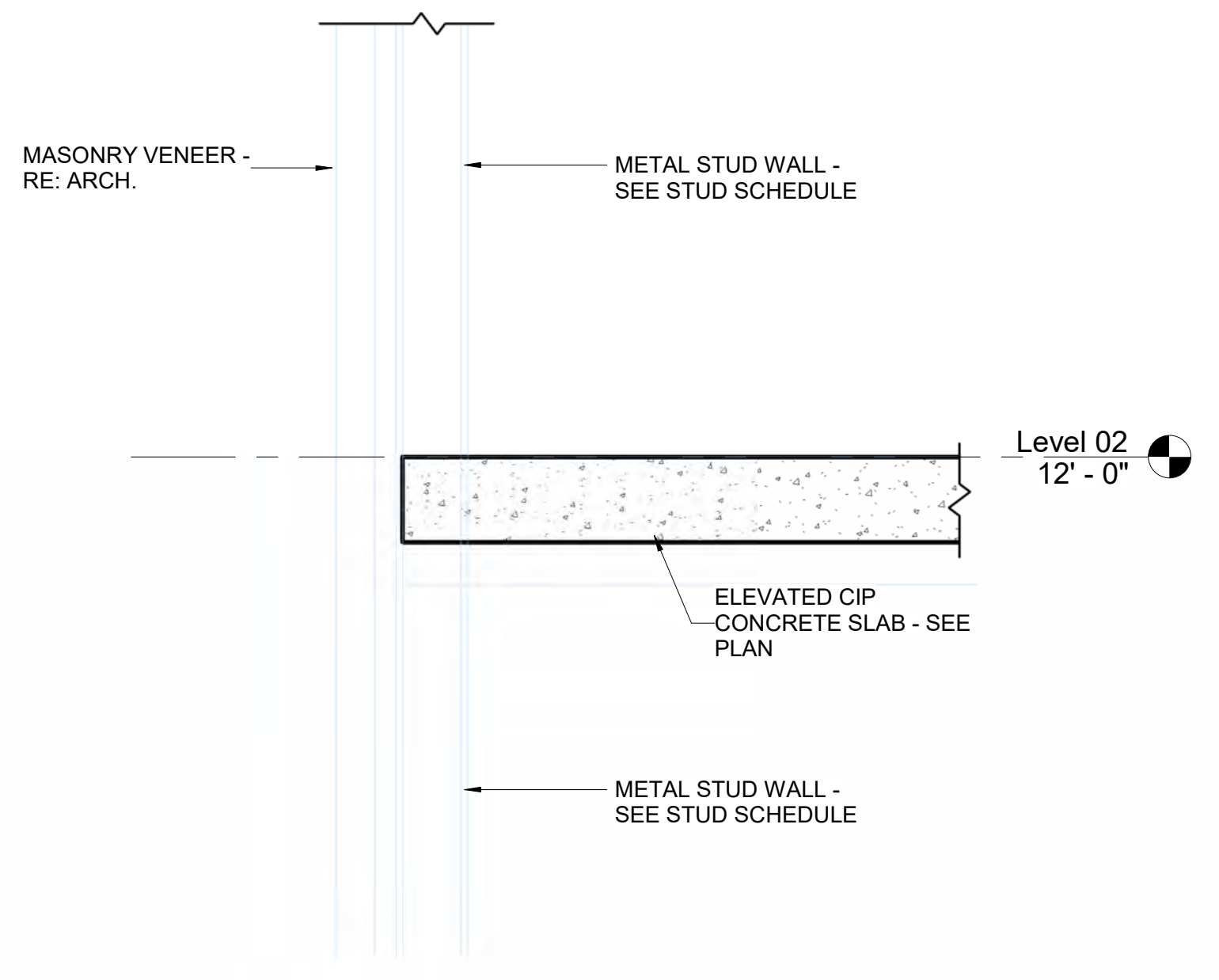
10 Section at Level 3 Roof
3/4" = 1'-0"


WINDOW SYSTEM - RE: ARCH.
ELEVATED CIP CONCRETE SLAB - SEE PLAN
Level 02
12' - 0"
Level 02 East Building Plaza L.P.
11' - 3"

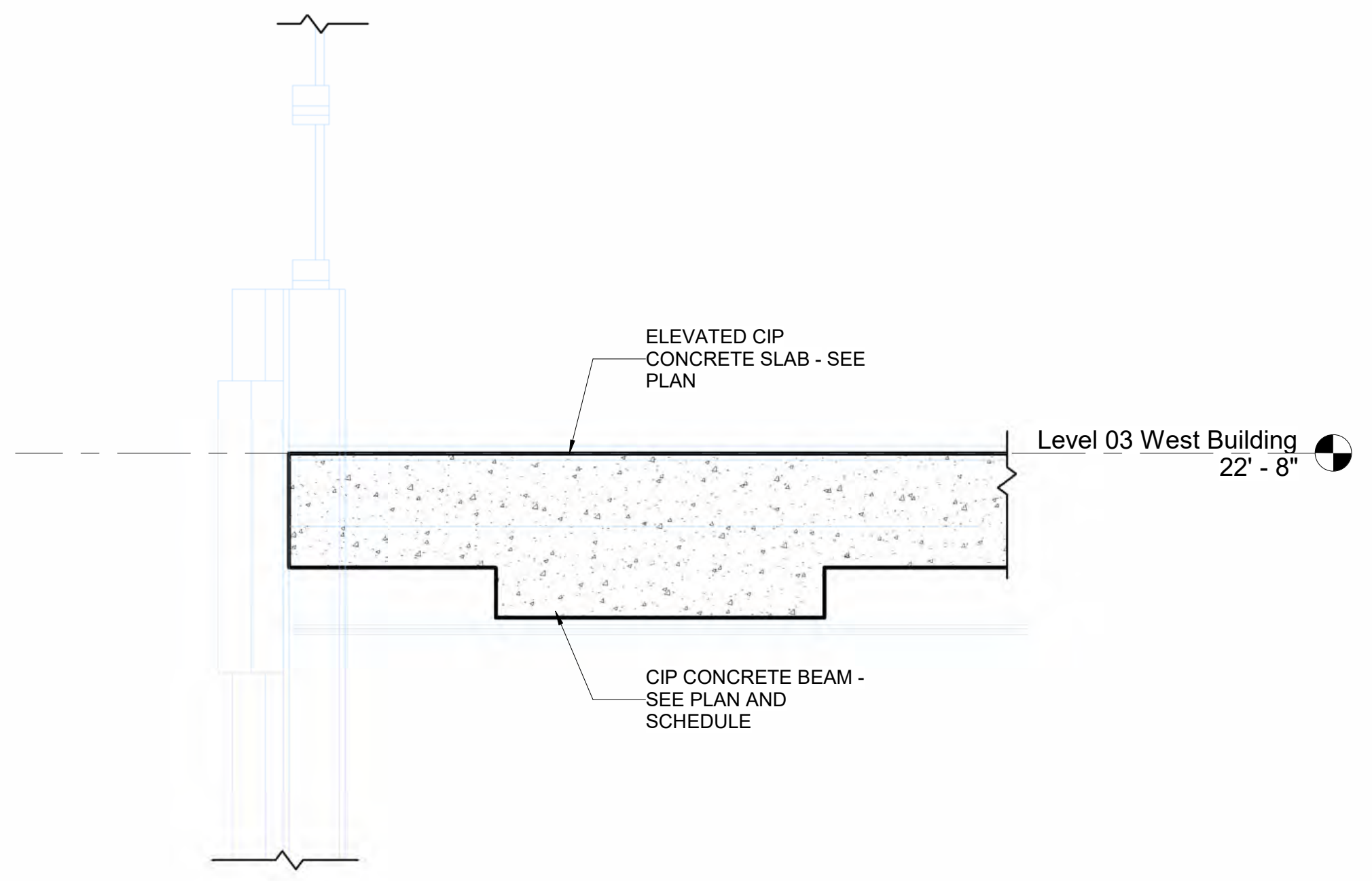
13 Section at Plaza
3/4" = 1'-0"



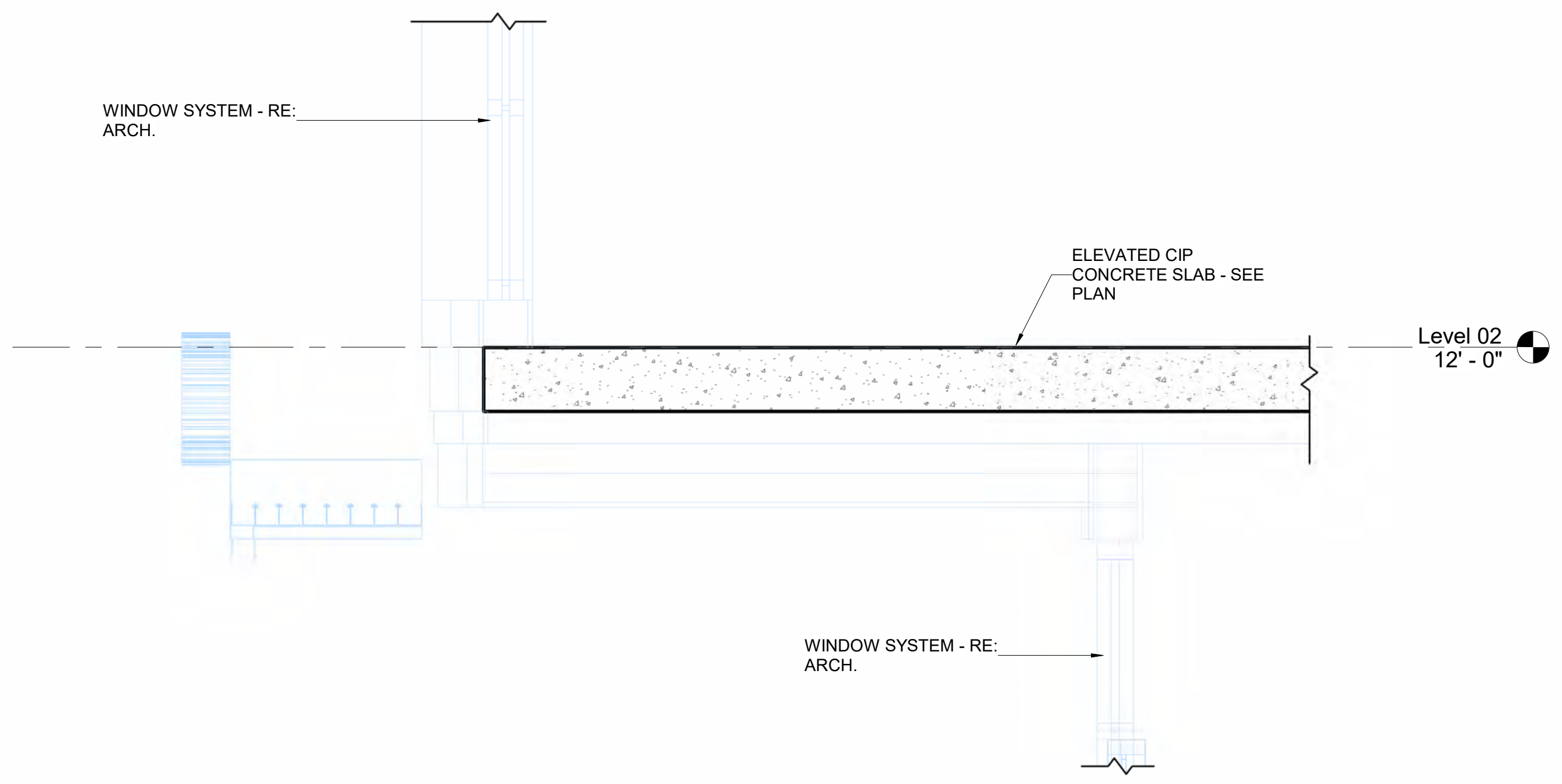
9 Section at Level 3 Roof
3/4" = 1'-0"


MASONRY VENEER - RE: ARCH.
METAL STUD WALL - SEE STUD SCHEDULE
Level 02
12' - 0"
ELEVATED CIP CONCRETE SLAB - SEE PLAN
METAL STUD WALL - SEE STUD SCHEDULE

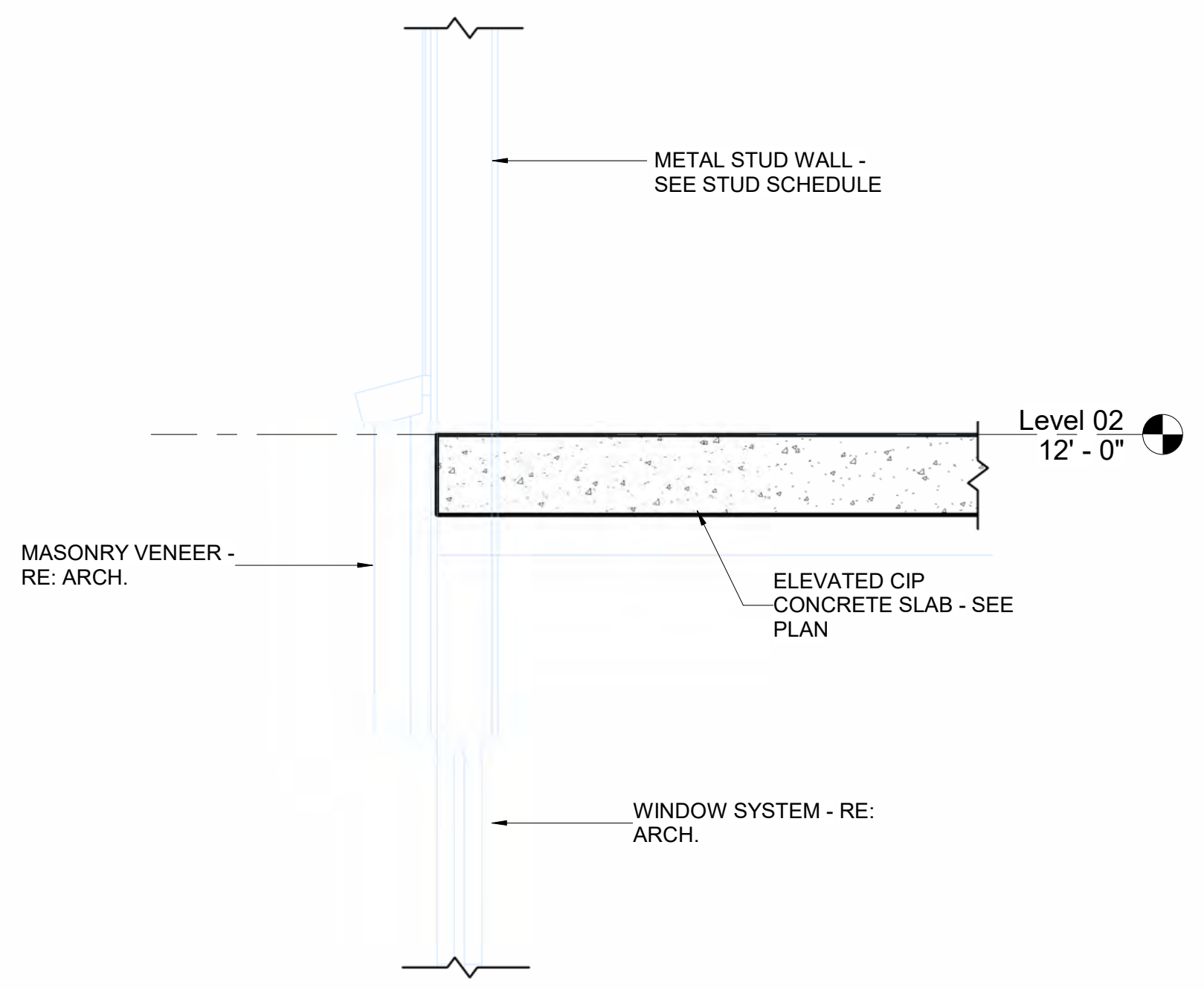
8 Section at Exterior
3/4" = 1'-0"


ELEVATED CIP CONCRETE SLAB - SEE PLAN
Level 03 West Building
22' - 8"
CIP CONCRETE BEAM - SEE PLAN AND SCHEDULE

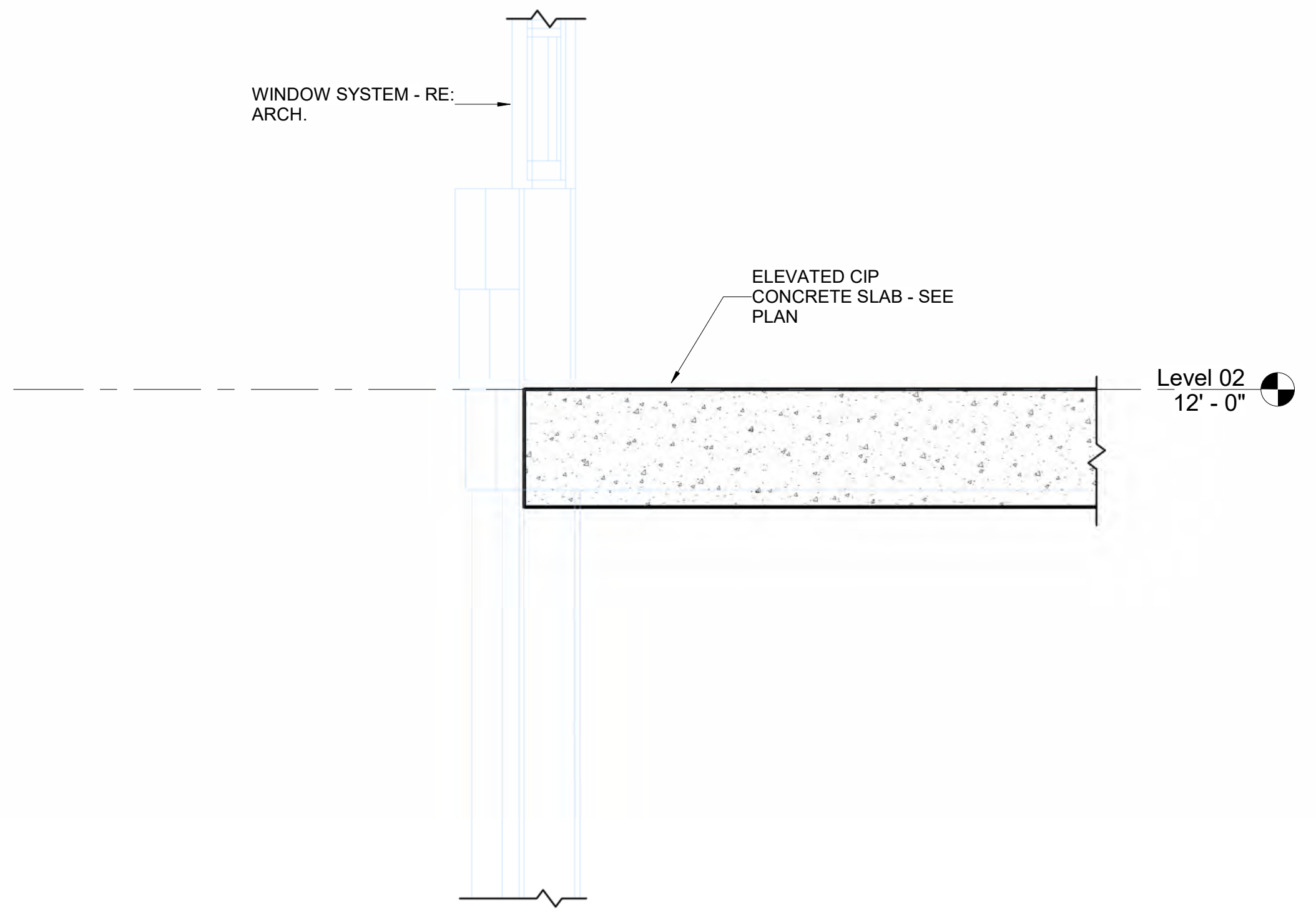
12 Section at Exterior
3/4" = 1'-0"


WINDOW SYSTEM - RE: ARCH.
ELEVATED CIP CONCRETE SLAB - SEE PLAN
Level 02
12' - 0"
WINDOW SYSTEM - RE: ARCH.

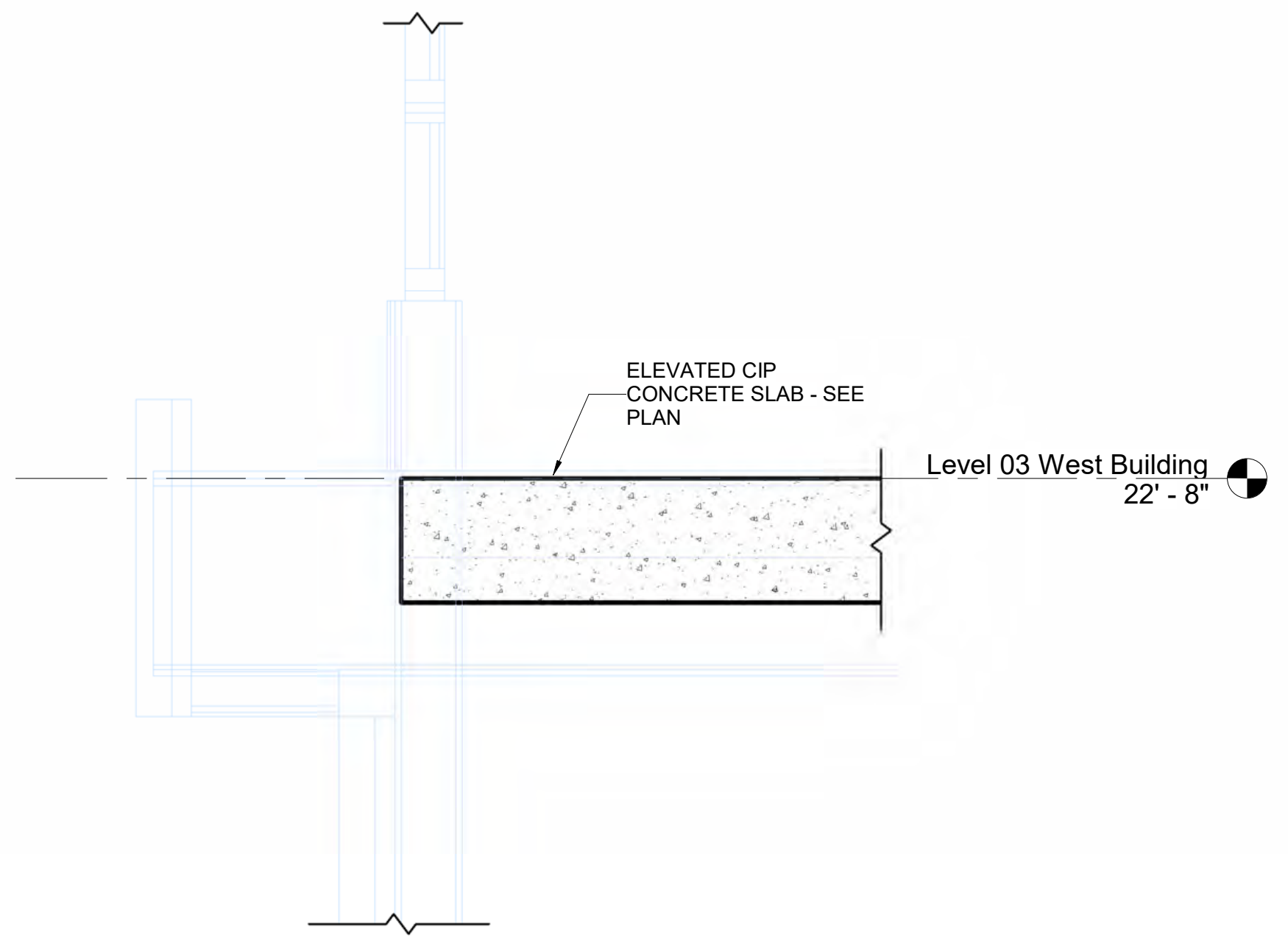
6 Section at Exterior
3/4" = 1'-0"


METAL STUD WALL - SEE STUD SCHEDULE
Level 02
12' - 0"
MASONRY VENEER - RE: ARCH.
ELEVATED CIP CONCRETE SLAB - SEE PLAN
WINDOW SYSTEM - RE: ARCH.

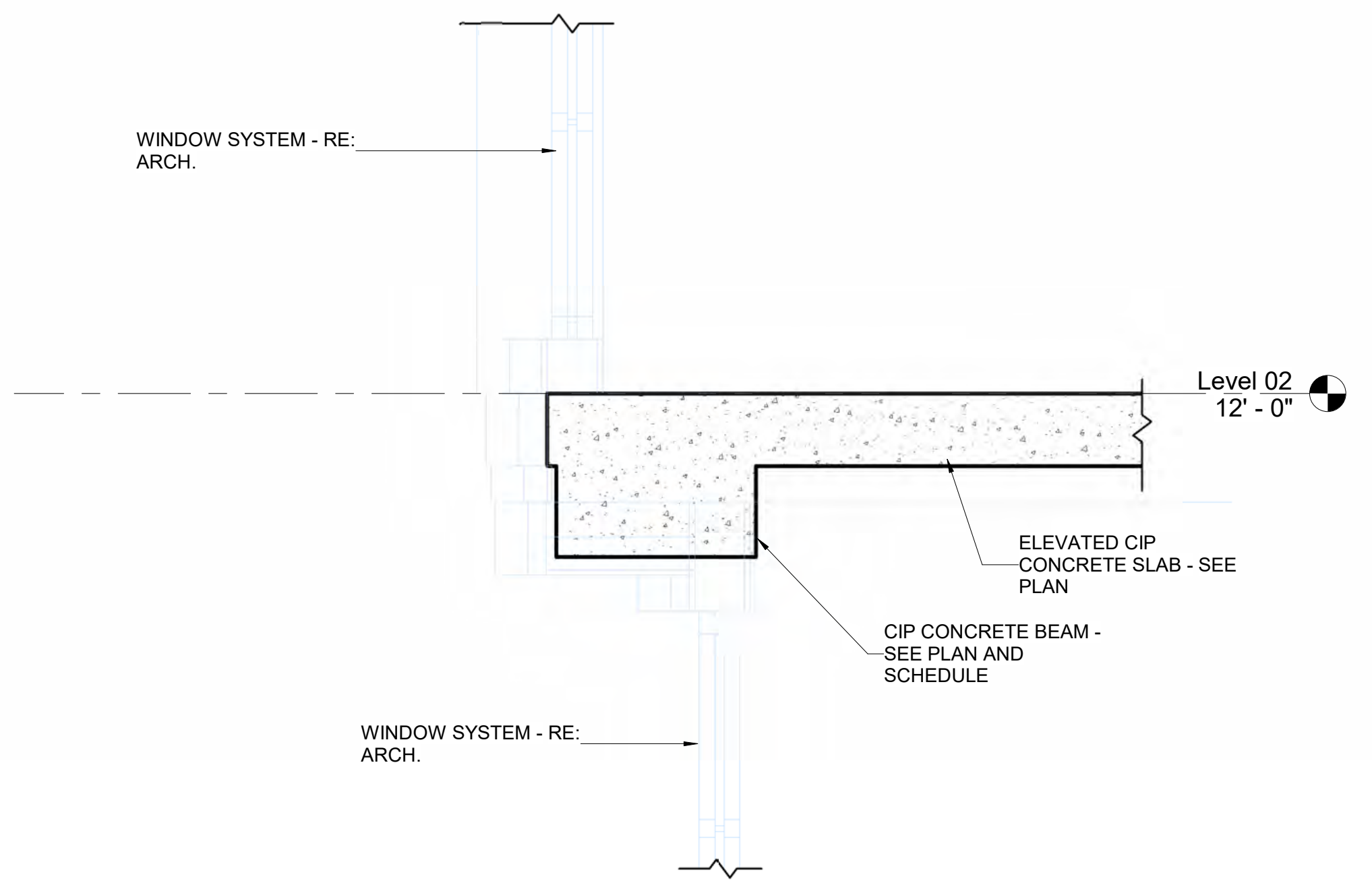
3 Section at Exterior
3/4" = 1'-0"


WINDOW SYSTEM - RE: ARCH.
ELEVATED CIP CONCRETE SLAB - SEE PLAN
Level 02
12' - 0"

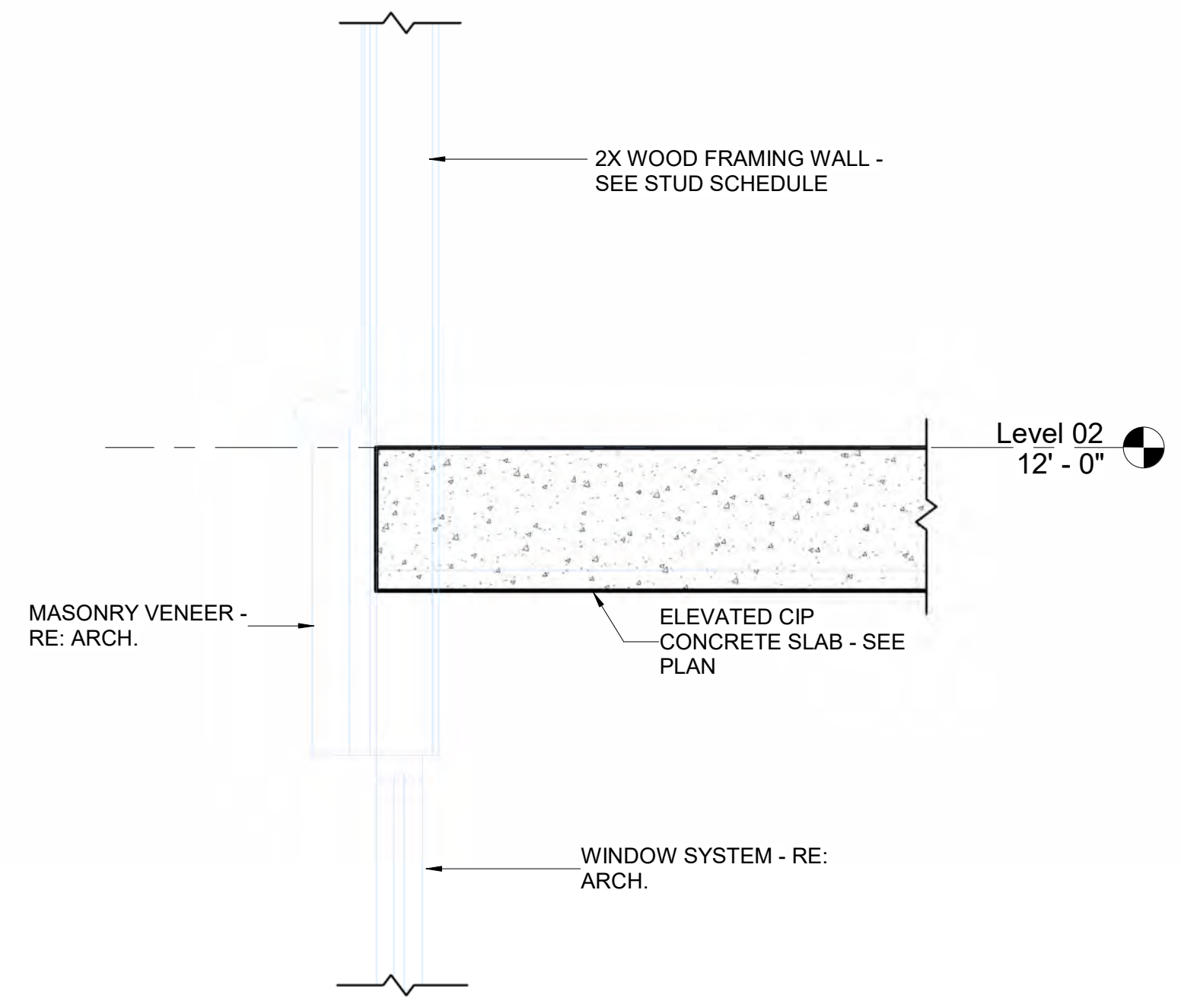
15 Detail 65
3/4" = 1'-0"



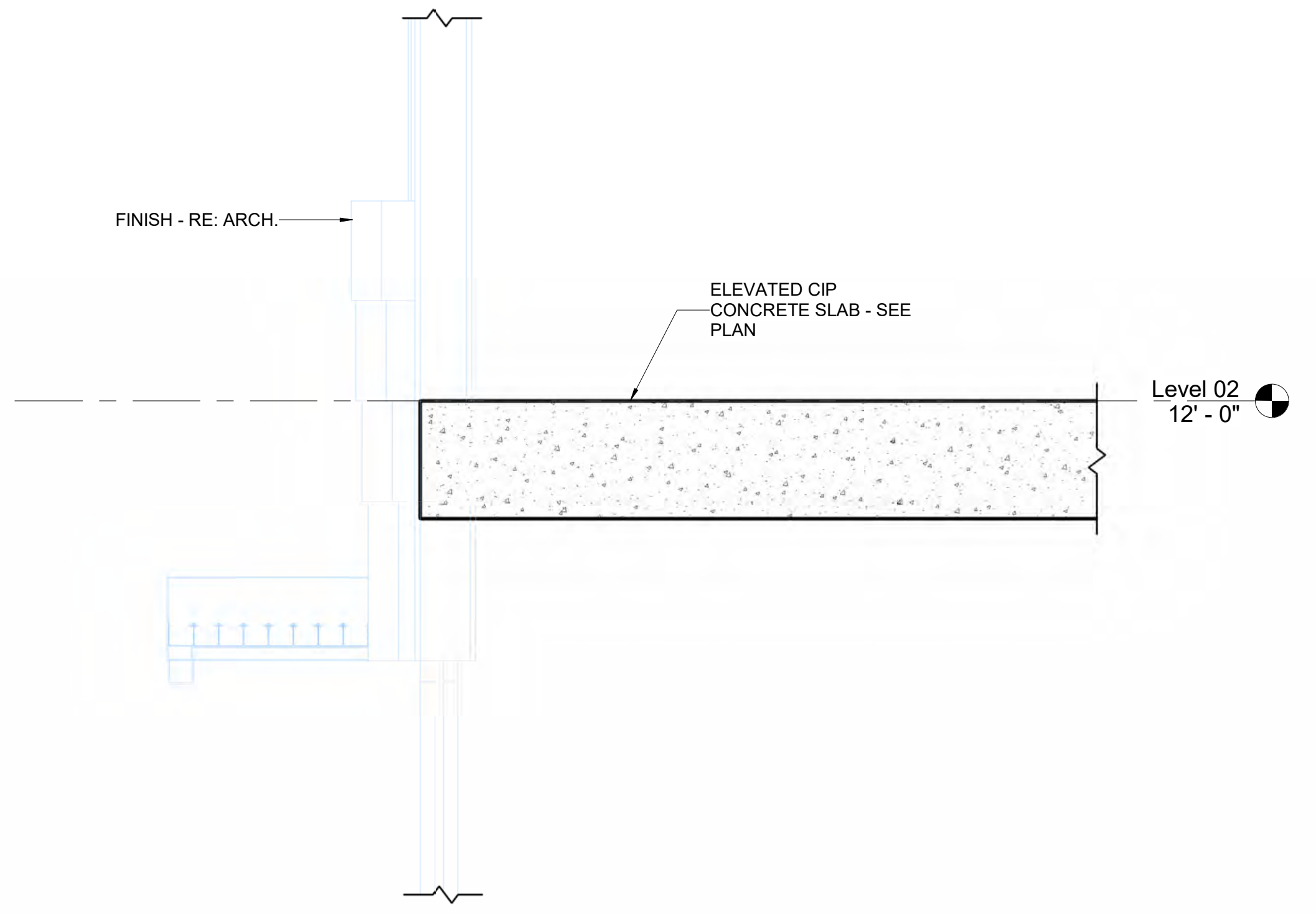
11 Section at Exterior
3/4" = 1'-0"


WINDOW SYSTEM - RE: ARCH.
Level 02
12' - 0"
ELEVATED CIP CONCRETE SLAB - SEE PLAN
CIP CONCRETE BEAM - SEE PLAN AND SCHEDULE
WINDOW SYSTEM - RE: ARCH.

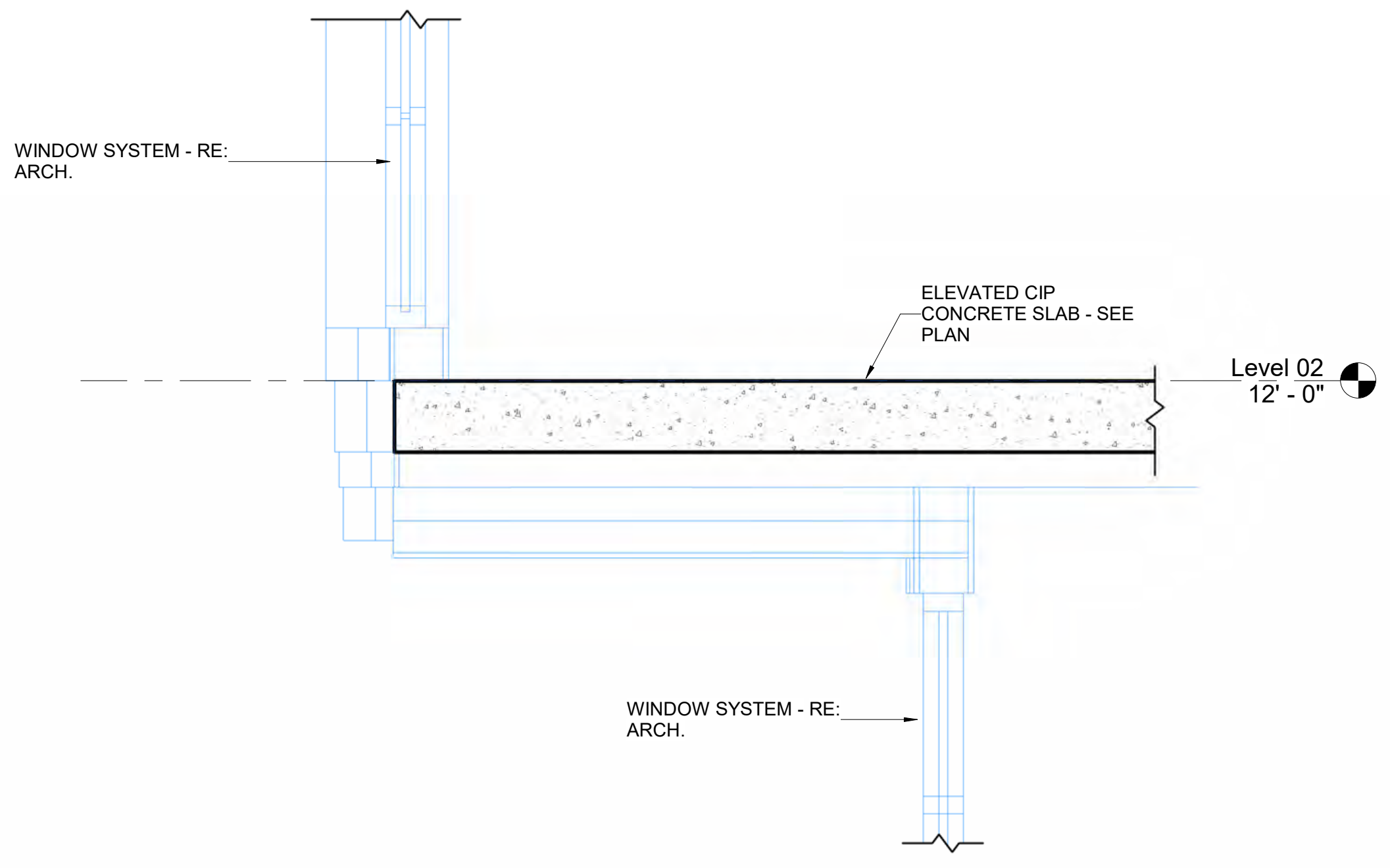
5 Section at Exterior
3/4" = 1'-0"


2X WOOD FRAMING WALL - SEE STUD SCHEDULE
Level 02
12' - 0"
MASONRY VENEER - RE: ARCH.
ELEVATED CIP CONCRETE SLAB - SEE PLAN
WINDOW SYSTEM - RE: ARCH.

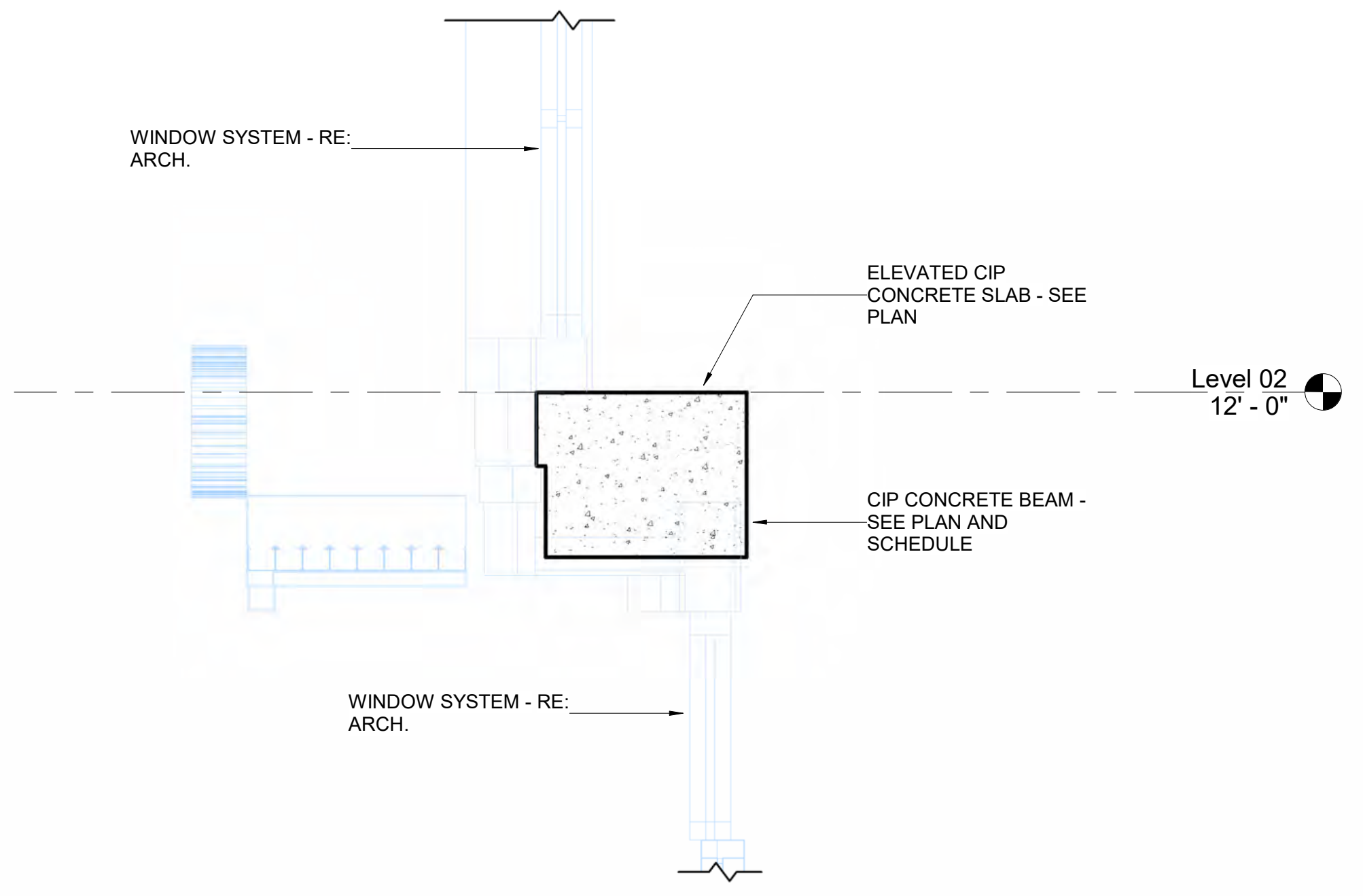
2 Section at Exterior
3/4" = 1'-0"



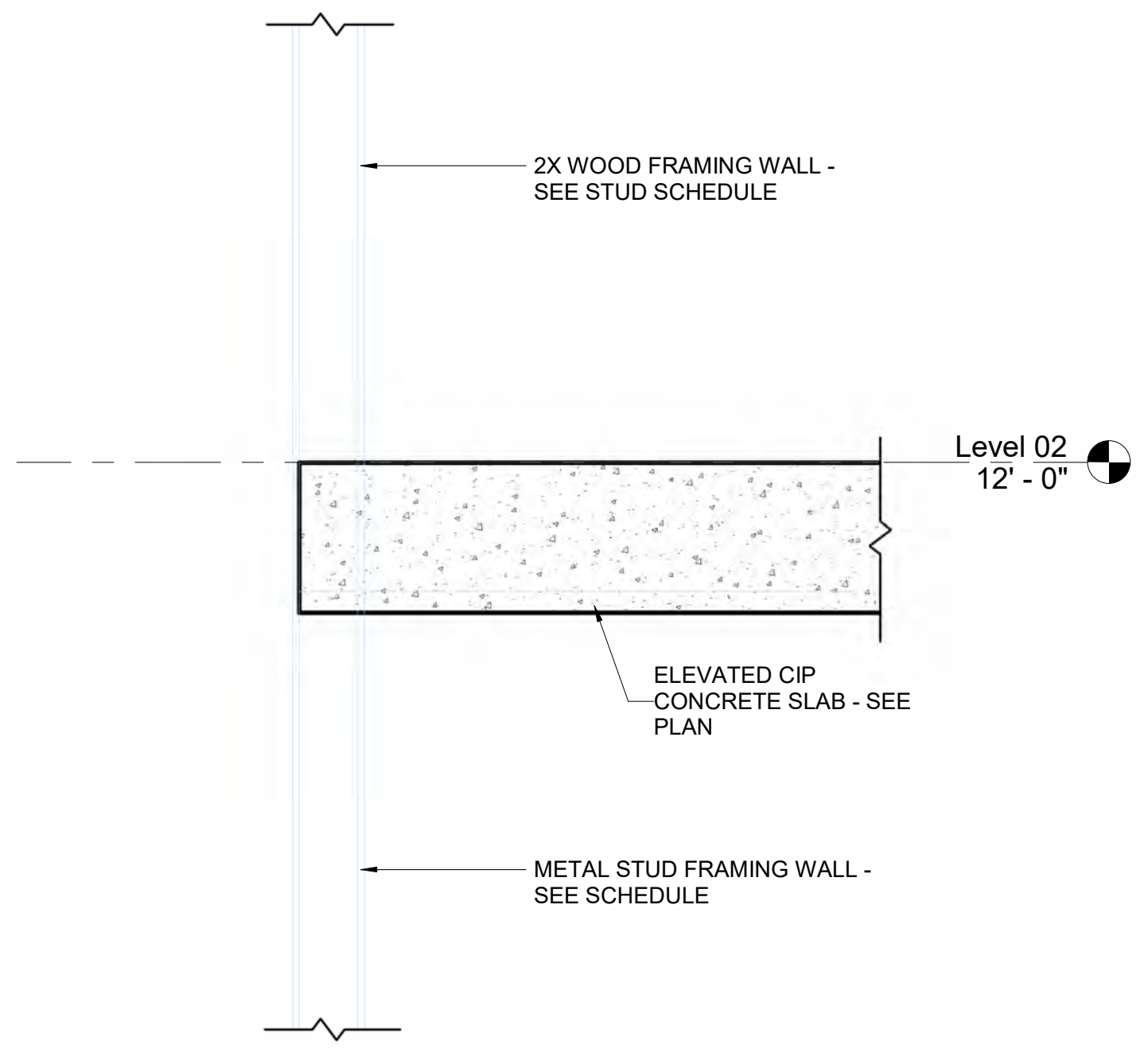
14 Detail 63
3/4" = 1'-0"


WINDOW SYSTEM - RE: ARCH.
ELEVATED CIP CONCRETE SLAB - SEE PLAN
Level 02
12' - 0"
WINDOW SYSTEM - RE: ARCH.

7 Section at Exterior
3/4" = 1'-0"


WINDOW SYSTEM - RE: ARCH.
ELEVATED CIP CONCRETE SLAB - SEE PLAN
Level 02
12' - 0"
CIP CONCRETE BEAM - SEE PLAN AND SCHEDULE
WINDOW SYSTEM - RE: ARCH.

4 Section at Exterior
3/4" = 1'-0"


2X WOOD FRAMING WALL - SEE STUD SCHEDULE
Level 02
12' - 0"
ELEVATED CIP CONCRETE SLAB - SEE PLAN
METAL STUD FRAMING WALL - SEE SCHEDULE

1 Section at Exterior
3/4" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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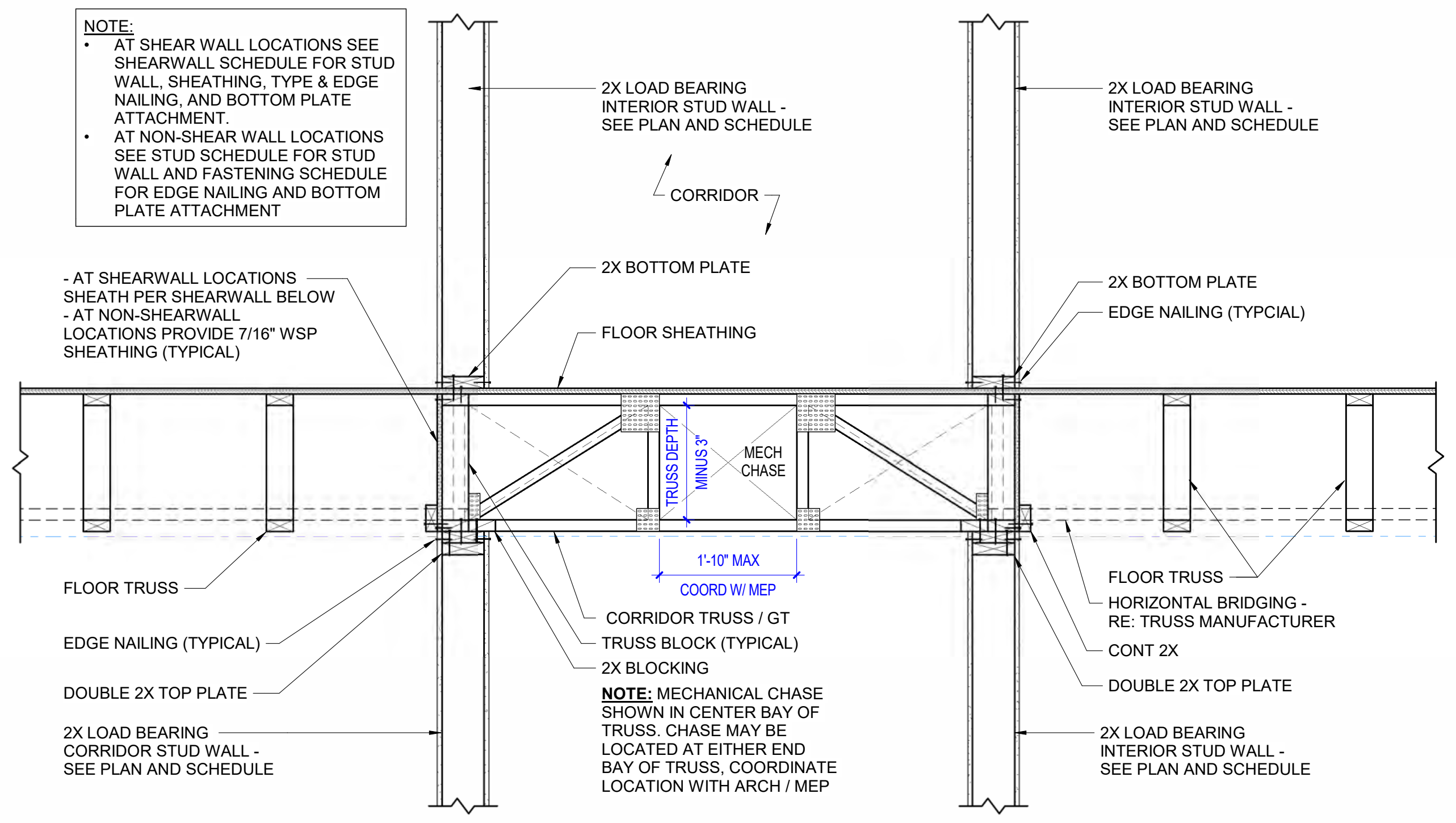
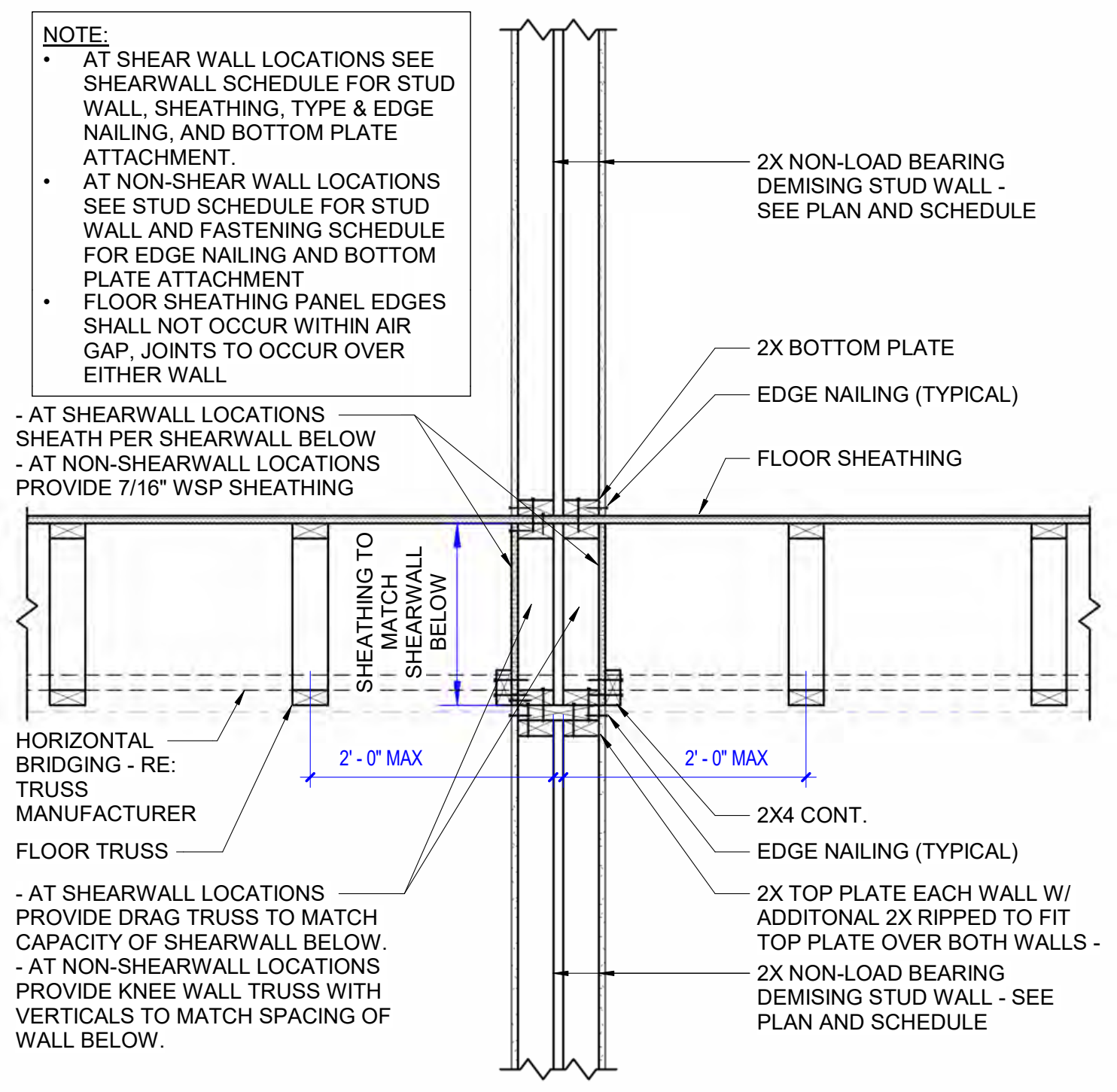

2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE
CORRIDOR
2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE
- AT SHEARWALL LOCATIONS SHEATH PER SHEARWALL BELOW - AT NON-SHEARWALL LOCATIONS PROVIDE 7/16" WSP SHEATHING (TYPICAL)
2X BOTTOM PLATE
FLOOR SHEATHING
2X BOTTOM PLATE
EDGE NAILING (TYPCIAL)
TRUSS DEPTH MINUS 3"
MECH CHASE
1'-10" MAX
COORD W/ MEP
FLOOR TRUSS
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE
2X LOAD BEARING CORRIDOR STUD WALL - SEE PLAN AND SCHEDULE
CORRIDOR TRUSS / GT
TRUSS BLOCK (TYPICAL)
2X BLOCKING
FLOOR TRUSS
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
CONT 2X
DOUBLE 2X TOP PLATE
2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

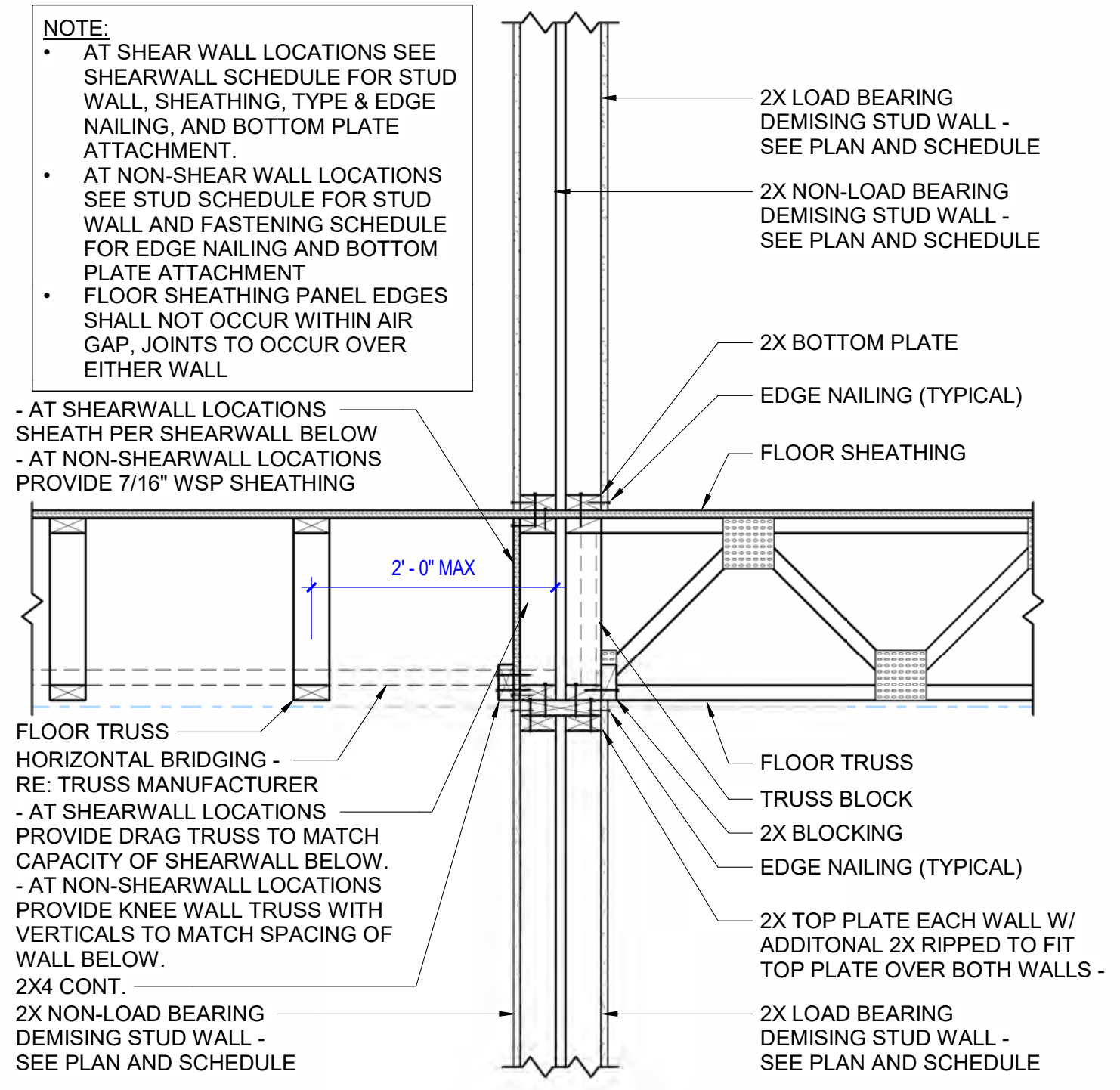
NOTE: MECHANICAL CHASE SHOWN IN CENTER BAY OF TRUSS. CHASE MAY BE LOCATED AT EITHER END BAY OF TRUSS, COORDINATE LOCATION WITH ARCH / MEP

11 Truss Bearing at Corridor
3/4" = 1'-0"


2X NON-LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
- AT SHEARWALL LOCATIONS SHEATH PER SHEARWALL BELOW - AT NON-SHEARWALL LOCATIONS PROVIDE 7/16" WSP SHEATHING
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
SHEATHING TO MATCH SHEARWALL BELOW
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
FLOOR TRUSS
2' - 0" MAX
2' - 0" MAX
2X4 CONT.
EDGE NAILING (TYPICAL)
- AT SHEARWALL LOCATIONS PROVIDE DRAG TRUSS TO MATCH CAPACITY OF SHEARWALL BELOW. - AT NON-SHEARWALL LOCATIONS PROVIDE KNEE WALL TRUSS WITH VERTICALS TO MATCH SPACING OF WALL BELOW.
2X TOP PLATE EACH WALL W/ ADDITONAL 2X RIPPED TO FIT TOP PLATE OVER BOTH WALLS -
2X NON-LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE

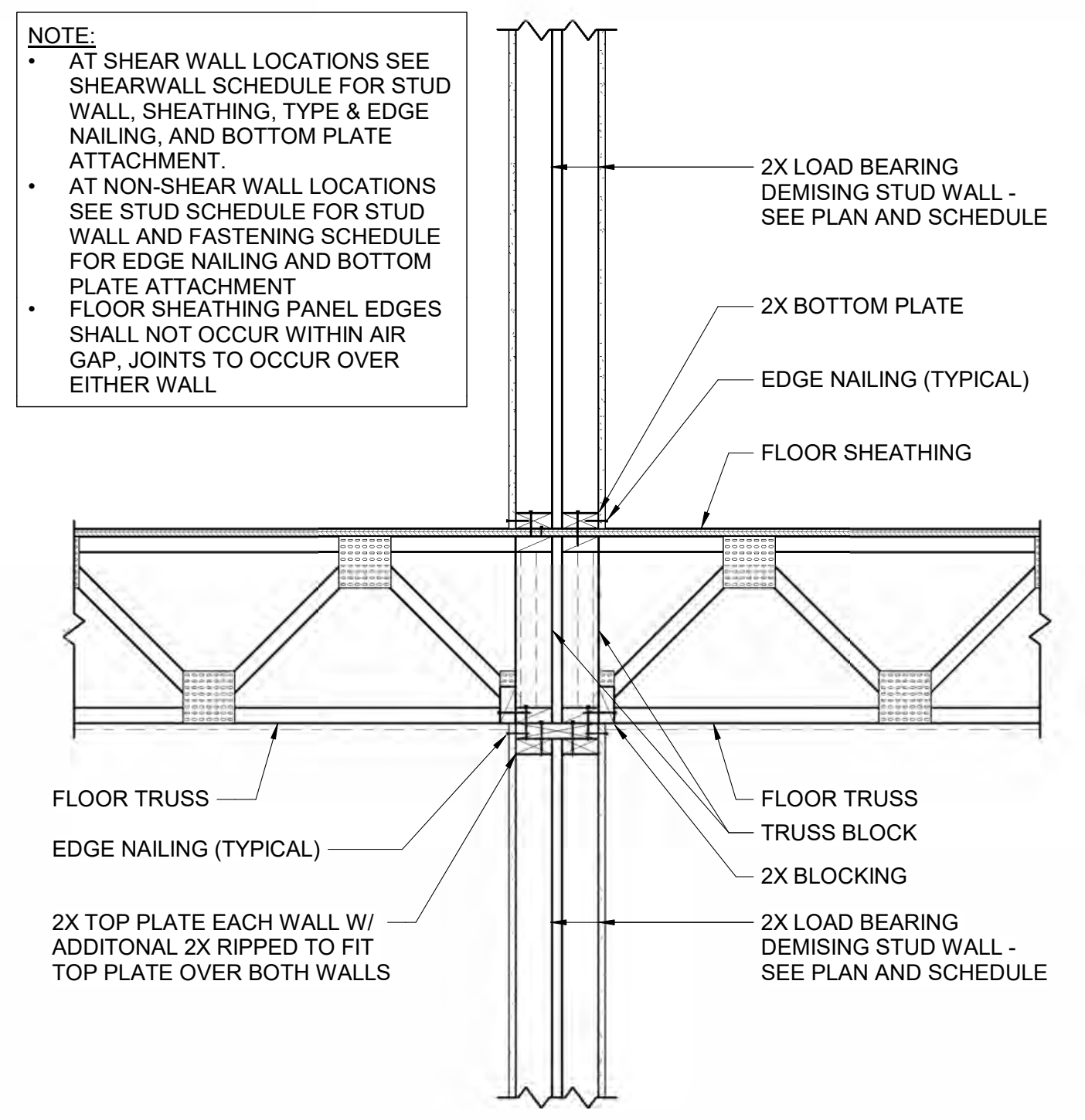
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT
- FLOOR SHEATHING PANEL EDGES SHALL NOT OCCUR WITHIN AIR GAP, JOINTS TO OCCUR OVER EITHER WALL

10 Truss Parallel Interior - Demising Wall
3/4" = 1'-0"


2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
2X NON-LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
- AT SHEARWALL LOCATIONS SHEATH PER SHEARWALL BELOW - AT NON-SHEARWALL LOCATIONS PROVIDE 7/16" WSP SHEATHING
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
2' - 0" MAX
FLOOR TRUSS
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
- AT SHEARWALL LOCATIONS PROVIDE DRAG TRUSS TO MATCH CAPACITY OF SHEARWALL BELOW. - AT NON-SHEARWALL LOCATIONS PROVIDE KNEE WALL TRUSS WITH VERTICALS TO MATCH SPACING OF WALL BELOW.
2X4 CONT.
2X NON-LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
FLOOR TRUSS
TRUSS BLOCK
2X BLOCKING
EDGE NAILING (TYPICAL)
2X TOP PLATE EACH WALL W/ ADDITONAL 2X RIPPED TO FIT TOP PLATE OVER BOTH WALLS -
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE

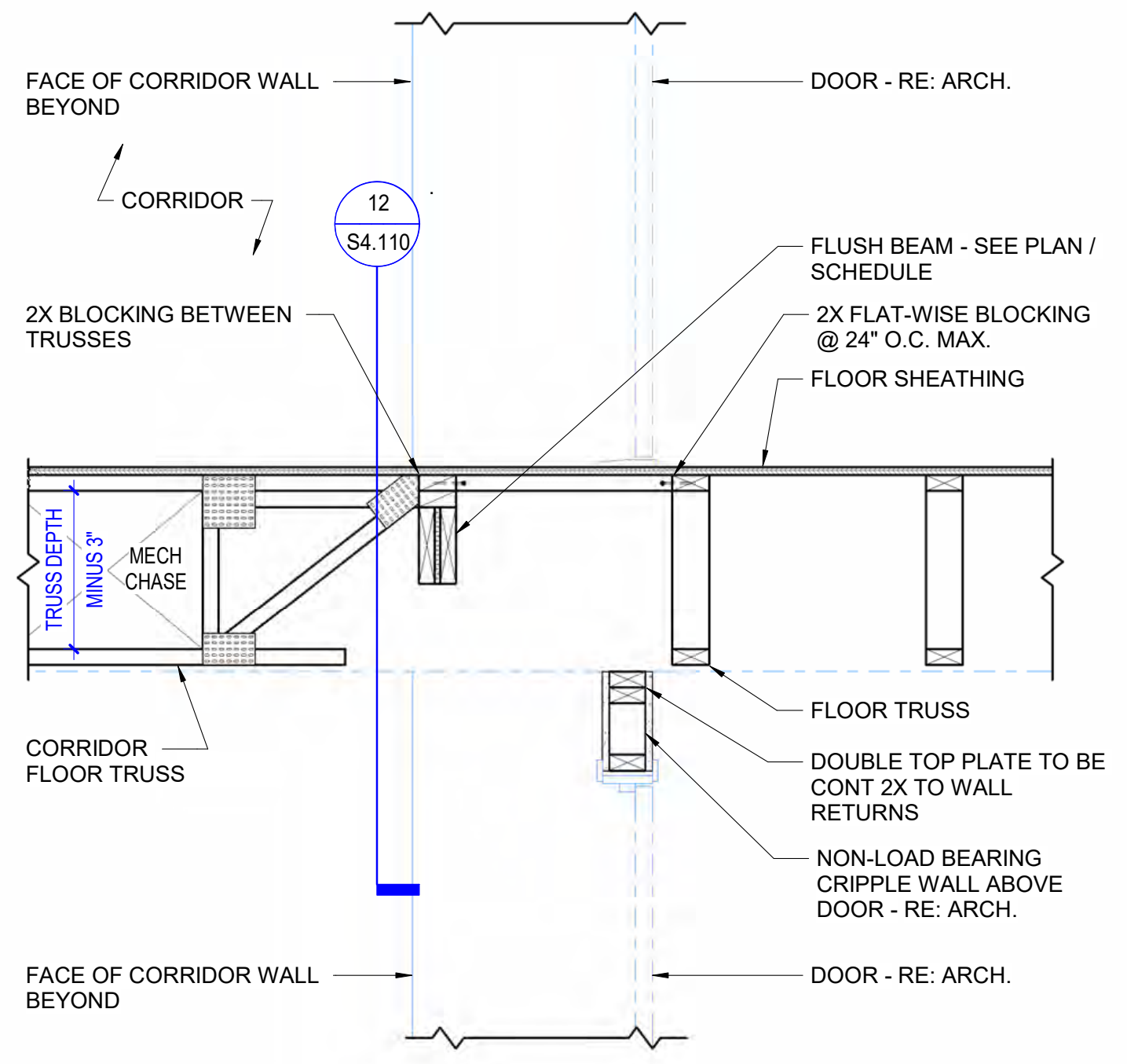
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT
- FLOOR SHEATHING PANEL EDGES SHALL NOT OCCUR WITHIN AIR GAP, JOINTS TO OCCUR OVER EITHER WALL

9 Truss Bearing Interior - Demising Wall - One Side
3/4" = 1'-0"

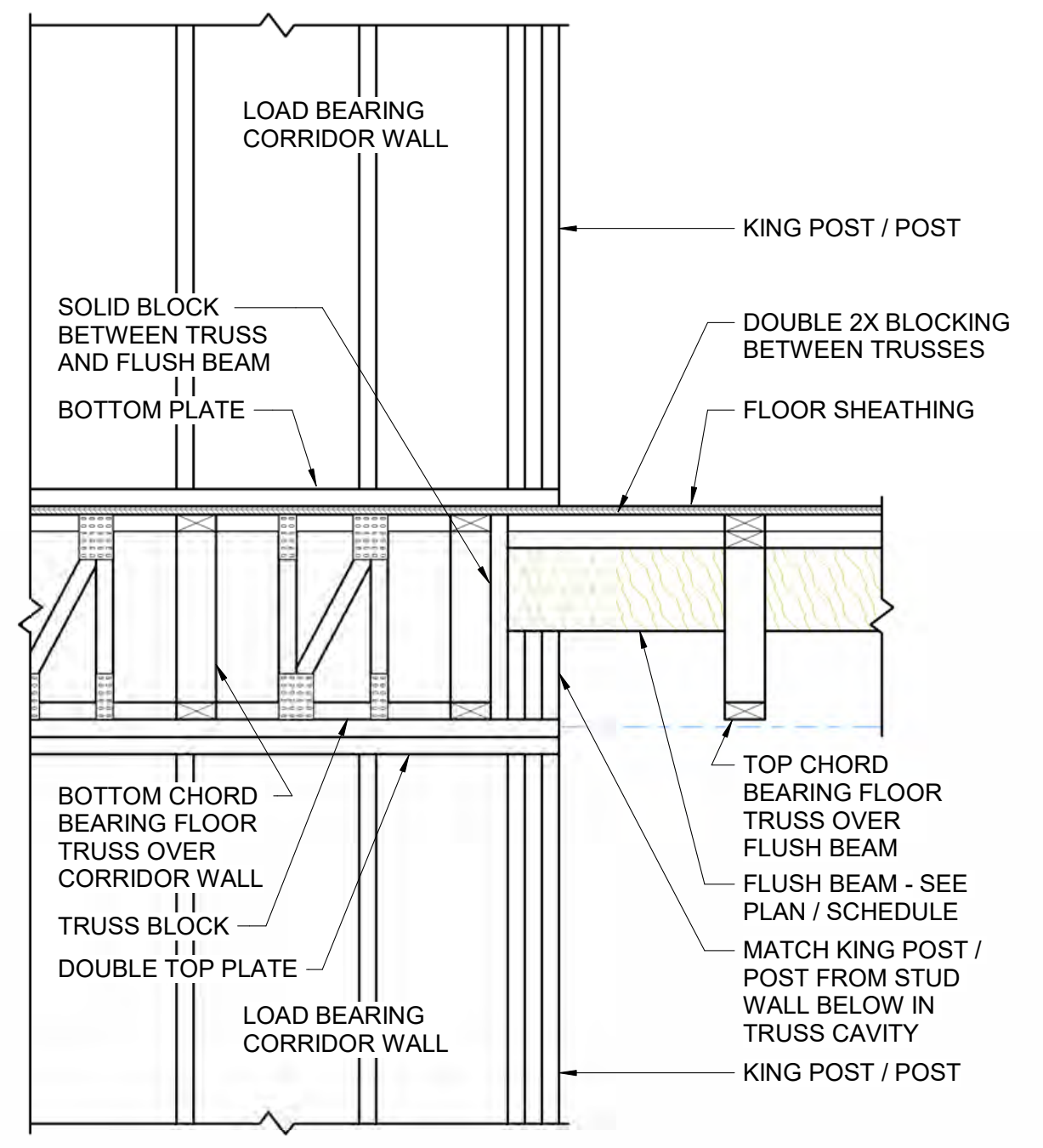

2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
FLOOR TRUSS
EDGE NAILING (TYPICAL)
2X TOP PLATE EACH WALL W/ ADDITONAL 2X RIPPED TO FIT TOP PLATE OVER BOTH WALLS
FLOOR TRUSS
TRUSS BLOCK
2X BLOCKING
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT
- FLOOR SHEATHING PANEL EDGES SHALL NOT OCCUR WITHIN AIR GAP, JOINTS TO OCCUR OVER EITHER WALL

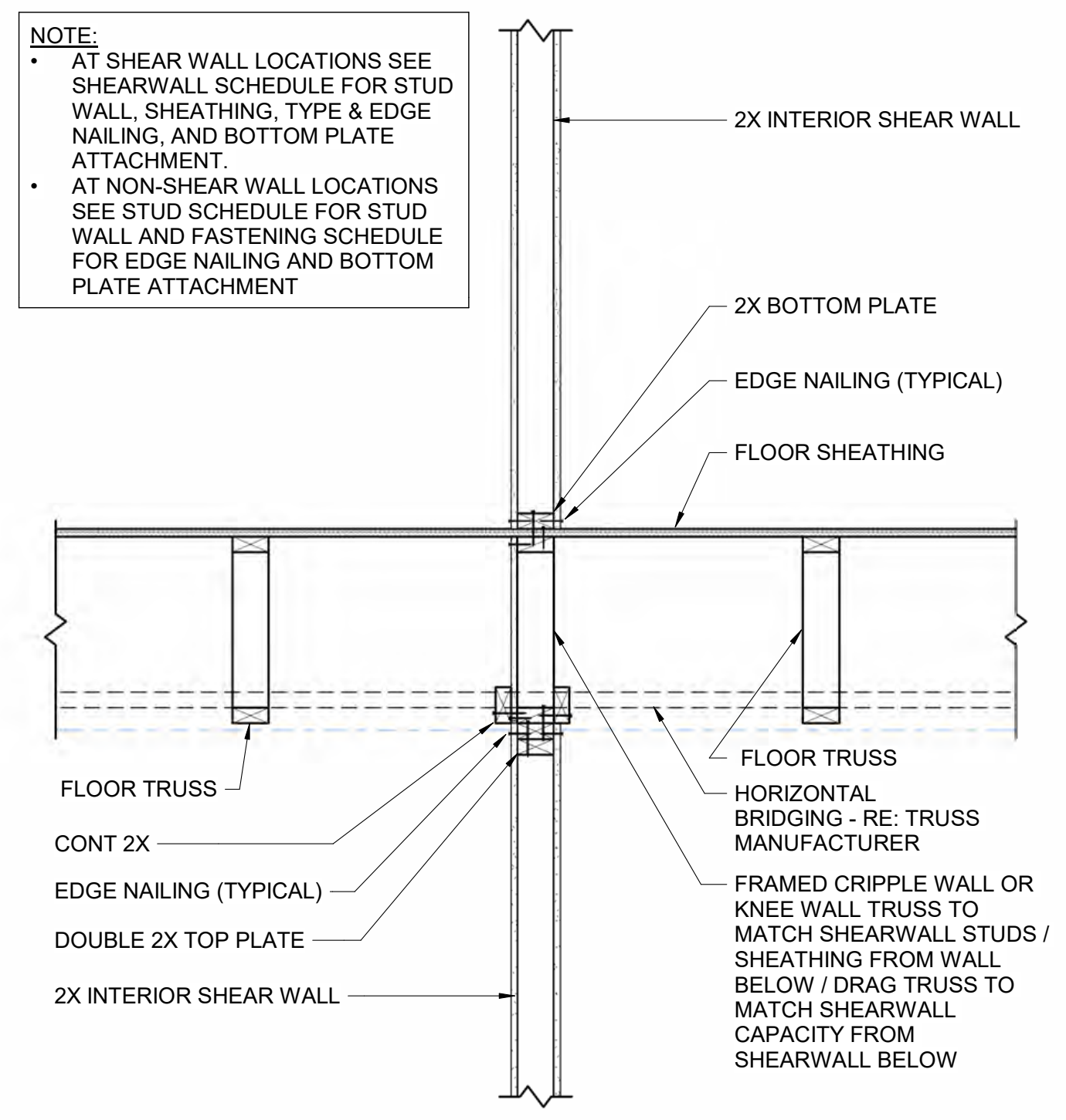
8 Truss Bearing Interior - Demising Wall - Both Sides
3/4" = 1'-0"


FACE OF CORRIDOR WALL BEYOND
DOOR - RE: ARCH.
CORRIDOR
12
S4.110
FLUSH BEAM - SEE PLAN / SCHEDULE
2X BLOCKING BETWEEN TRUSSES
2X FLAT-WISE BLOCKING @ 24" O.C. MAX.
FLOOR SHEATHING
TRUSS DEPTH MINUS 3"
MECH CHASE
FLOOR TRUSS
CORRIDOR FLOOR TRUSS
DOUBLE TOP PLATE TO BE CONT 2X TO WALL RETURNS
NON-LOAD BEARING CRIPPLE WALL ABOVE DOOR - RE: ARCH.
FACE OF CORRIDOR WALL BEYOND
DOOR - RE: ARCH.

13 Truss Bearing at Corridor - Unit Entry Door
3/4" = 1'-0"

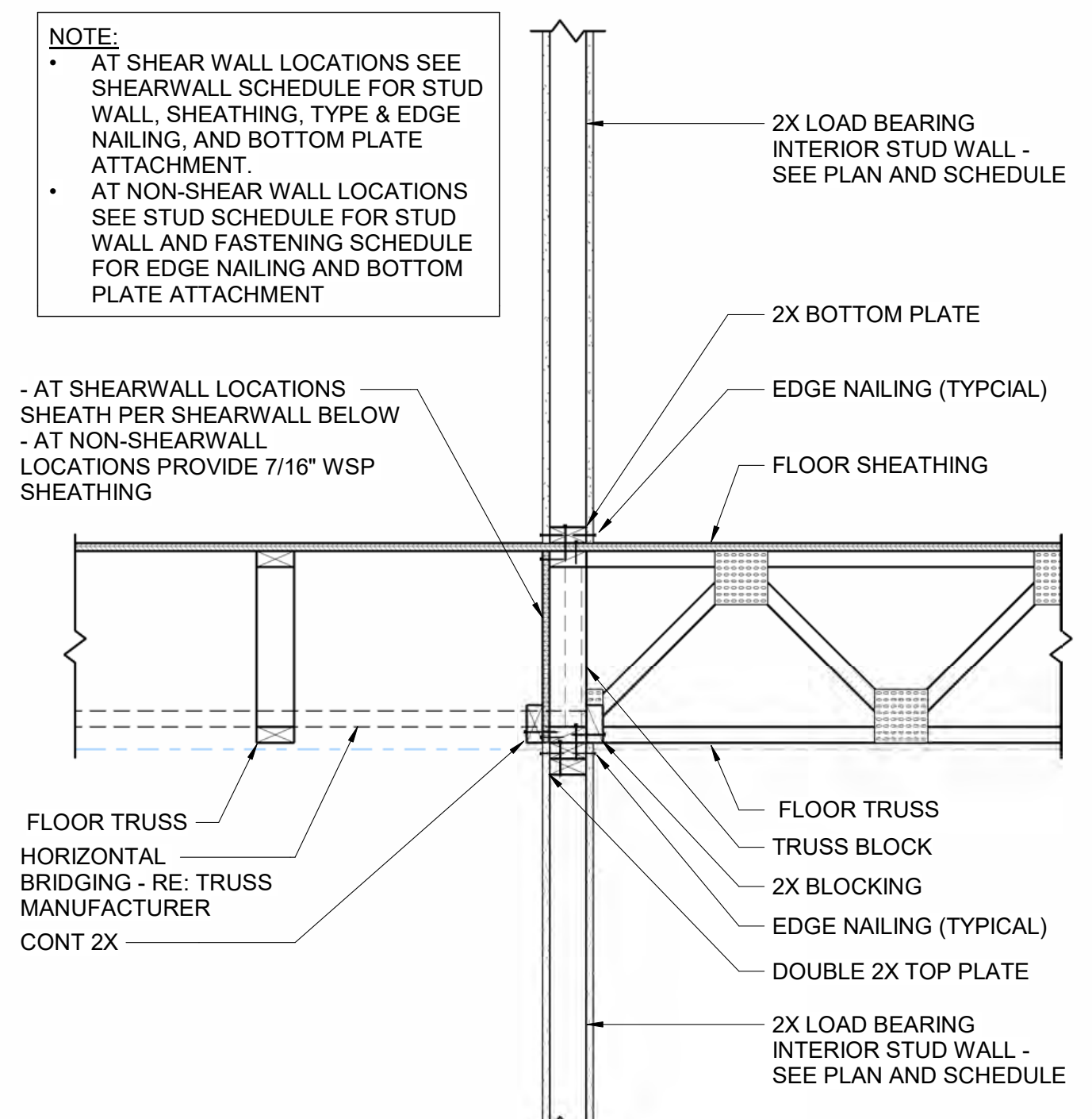

LOAD BEARING CORRIDOR WALL
KING POST / POST
SOLID BLOCK BETWEEN TRUSS AND FLUSH BEAM
DOUBLE 2X BLOCKING BETWEEN TRUSSES
BOTTOM PLATE
FLOOR SHEATHING
BOTTOM CHORD BEARING FLOOR TRUSS OVER CORRIDOR WALL
TOP CHORD BEARING FLOOR TRUSS OVER FLUSH BEAM
FLUSH BEAM - SEE PLAN / SCHEDULE
TRUSS BLOCK
MATCH KING POST / POST FROM STUD WALL BELOW IN TRUSS CAVITY
DOUBLE TOP PLATE
LOAD BEARING CORRIDOR WALL
KING POST / POST

12 Truss Bearing at Corridor - Unit Entry Door
3/4" = 1'-0"


2X INTERIOR SHEAR WALL
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
FLOOR TRUSS
FLOOR TRUSS
CONT 2X
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE
FRAMED CRIPPLE WALL OR KNEE WALL TRUSS TO MATCH SHEARWALL STUDS / SHEATHING FROM WALL BELOW / DRAG TRUSS TO MATCH SHEARWALL CAPACITY FROM SHEARWALL BELOW
2X INTERIOR SHEAR WALL

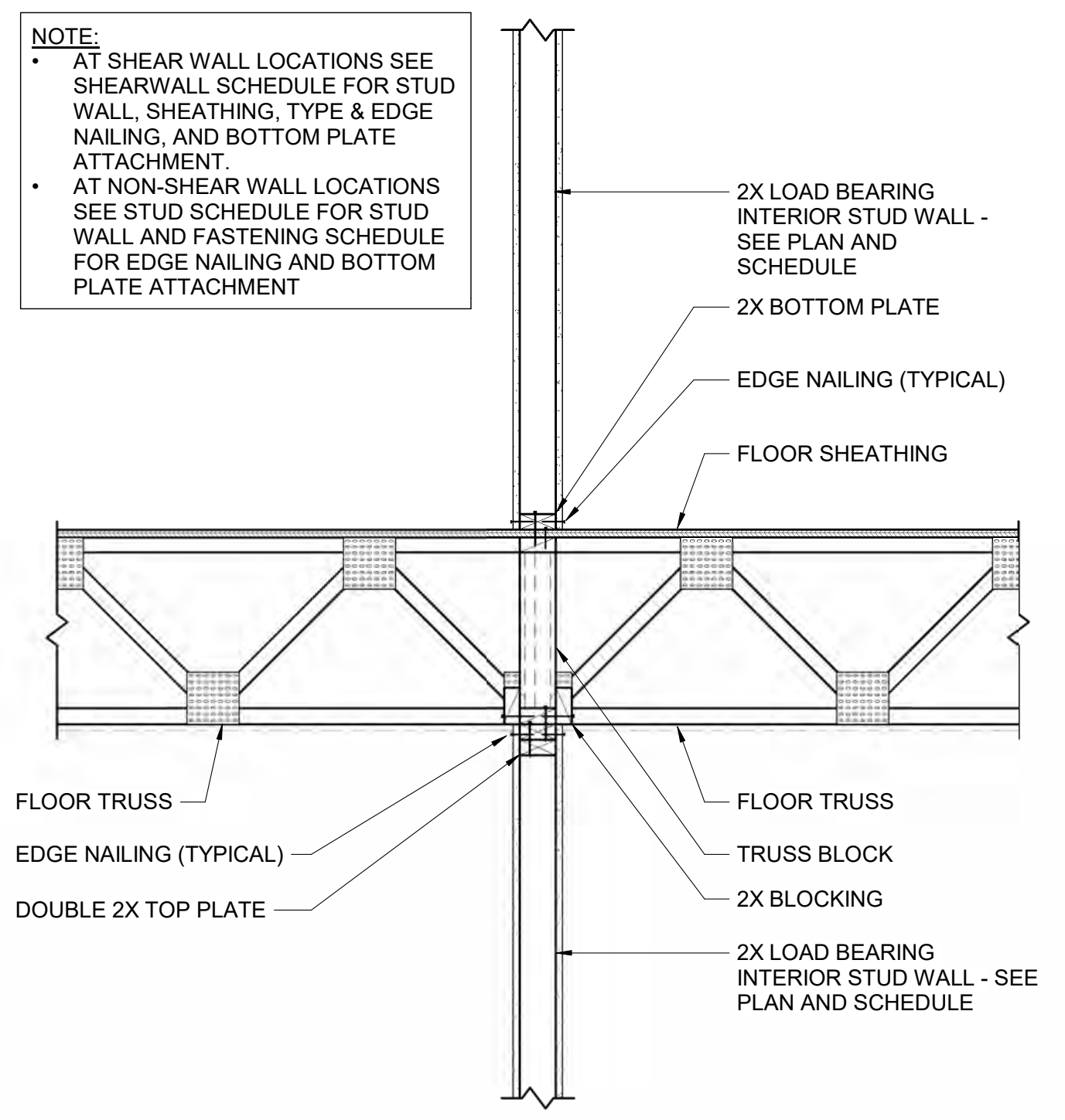
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

7 Truss Parallel to Interior Shear Wall
3/4" = 1'-0"


2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE
- AT SHEARWALL LOCATIONS SHEATH PER SHEARWALL BELOW - AT NON-SHEARWALL LOCATIONS PROVIDE 7/16" WSP SHEATHING
2X BOTTOM PLATE
EDGE NAILING (TYPCIAL)
FLOOR SHEATHING
FLOOR TRUSS
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
CONT 2X
FLOOR TRUSS
TRUSS BLOCK
2X BLOCKING
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE
2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE

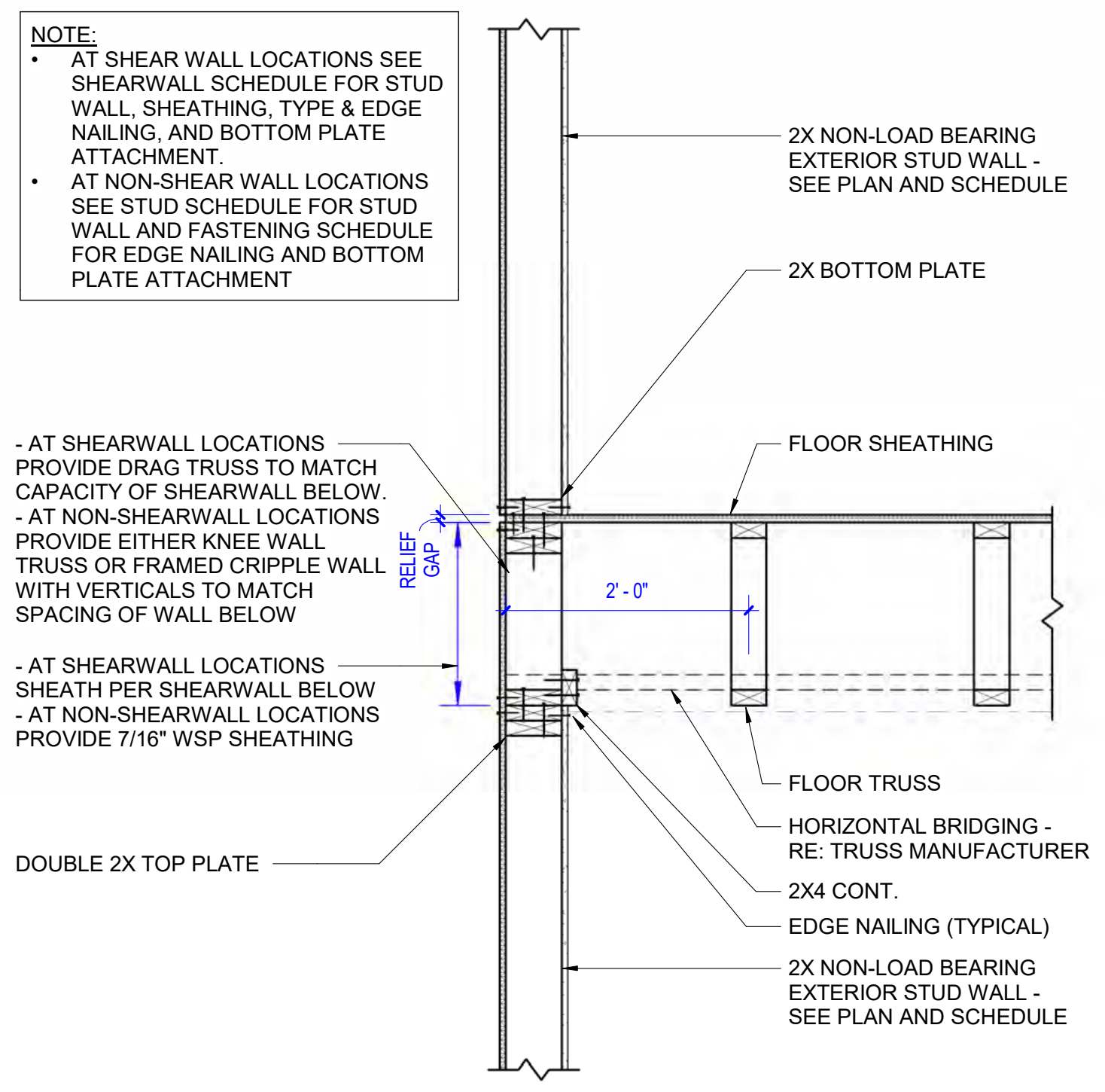
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

6 BC Truss Bearing Interior - One Side
3/4" = 1'-0"



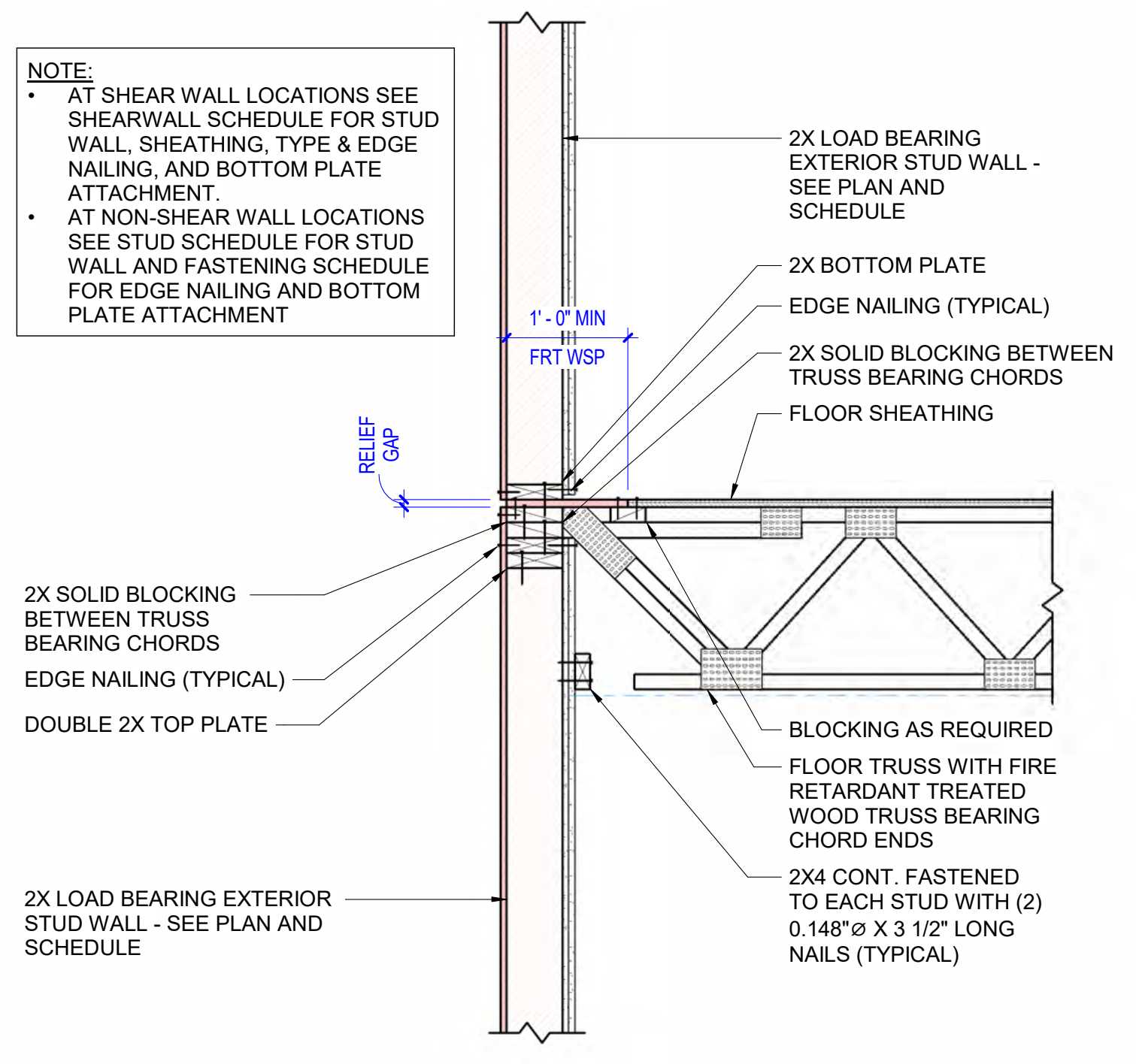
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

5 Truss Bearing Interior - Both Sides
3/4" = 1'-0"


2X NON-LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE
- AT SHEARWALL LOCATIONS PROVIDE DRAG TRUSS TO MATCH CAPACITY OF SHEARWALL BELOW. - AT NON-SHEARWALL LOCATIONS PROVIDE EITHER KNEE WALL TRUSS OR FRAMED CRIPPLE WALL WITH VERTICALS TO MATCH SPACING OF WALL BELOW
FLOOR SHEATHING
RELIEF GAP
2' - 0"
- AT SHEARWALL LOCATIONS SHEATH PER SHEARWALL BELOW - AT NON-SHEARWALL LOCATIONS PROVIDE 7/16" WSP SHEATHING
FLOOR TRUSS
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
DOUBLE 2X TOP PLATE
2X4 CONT.
EDGE NAILING (TYPICAL)
2X NON-LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE

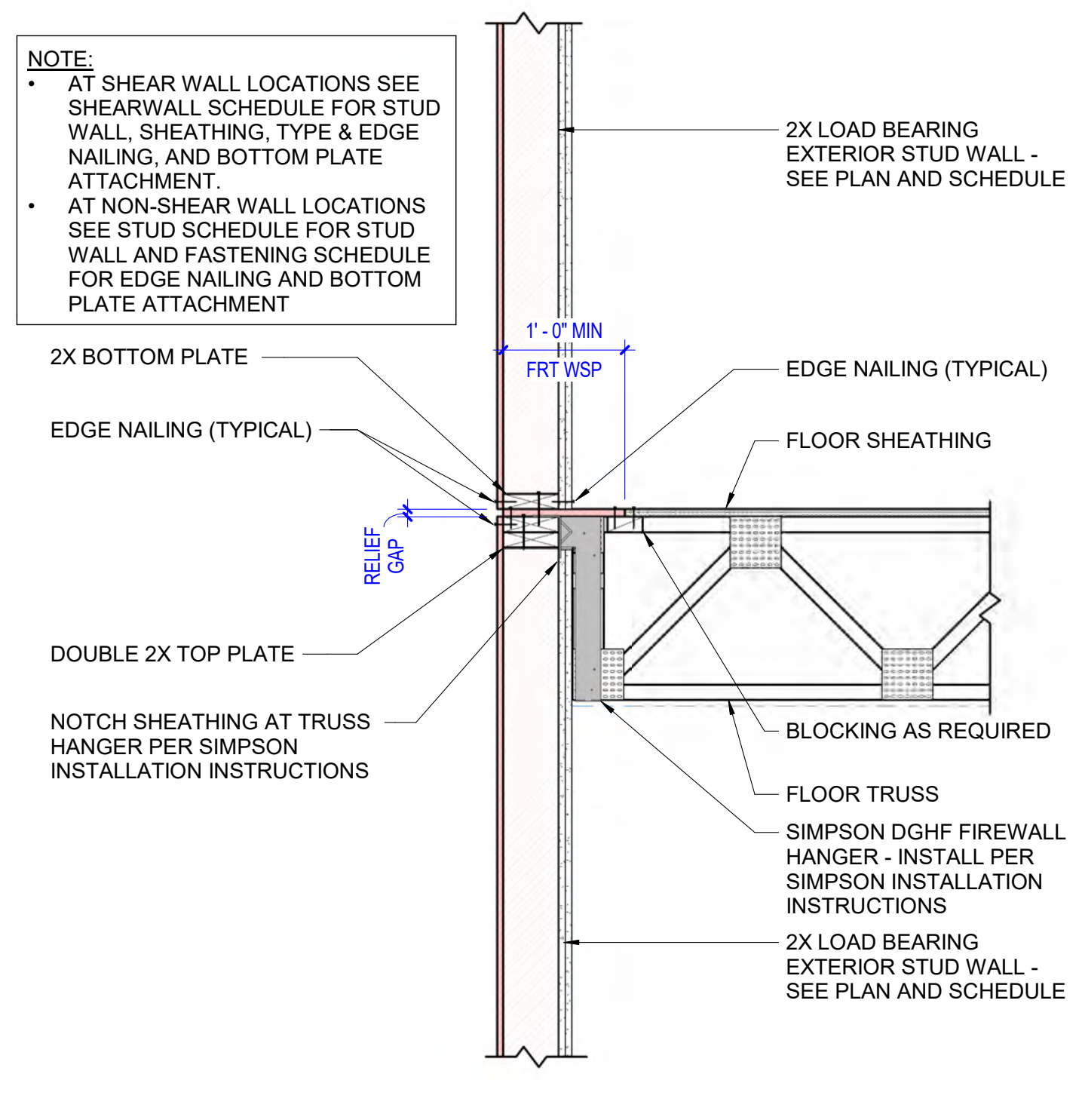
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

2 Truss Parallel to Exterior
3/4" = 1'-0"


2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
1' - 0" MIN
FRT WSP
2X SOLID BLOCKING BETWEEN TRUSS BEARING CHORDS
FLOOR SHEATHING
RELIEF GAP
2X SOLID BLOCKING BETWEEN TRUSS BEARING CHORDS
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE
BLOCKING AS REQUIRED
FLOOR TRUSS WITH FIRE RETARDANT TREATED WOOD TRUSS BEARING CHORD ENDS
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
2X4 CONT. FASTENED TO EACH STUD WITH (2) 0.148"Ø X 3 1/2" LONG NAILS (TYPICAL)

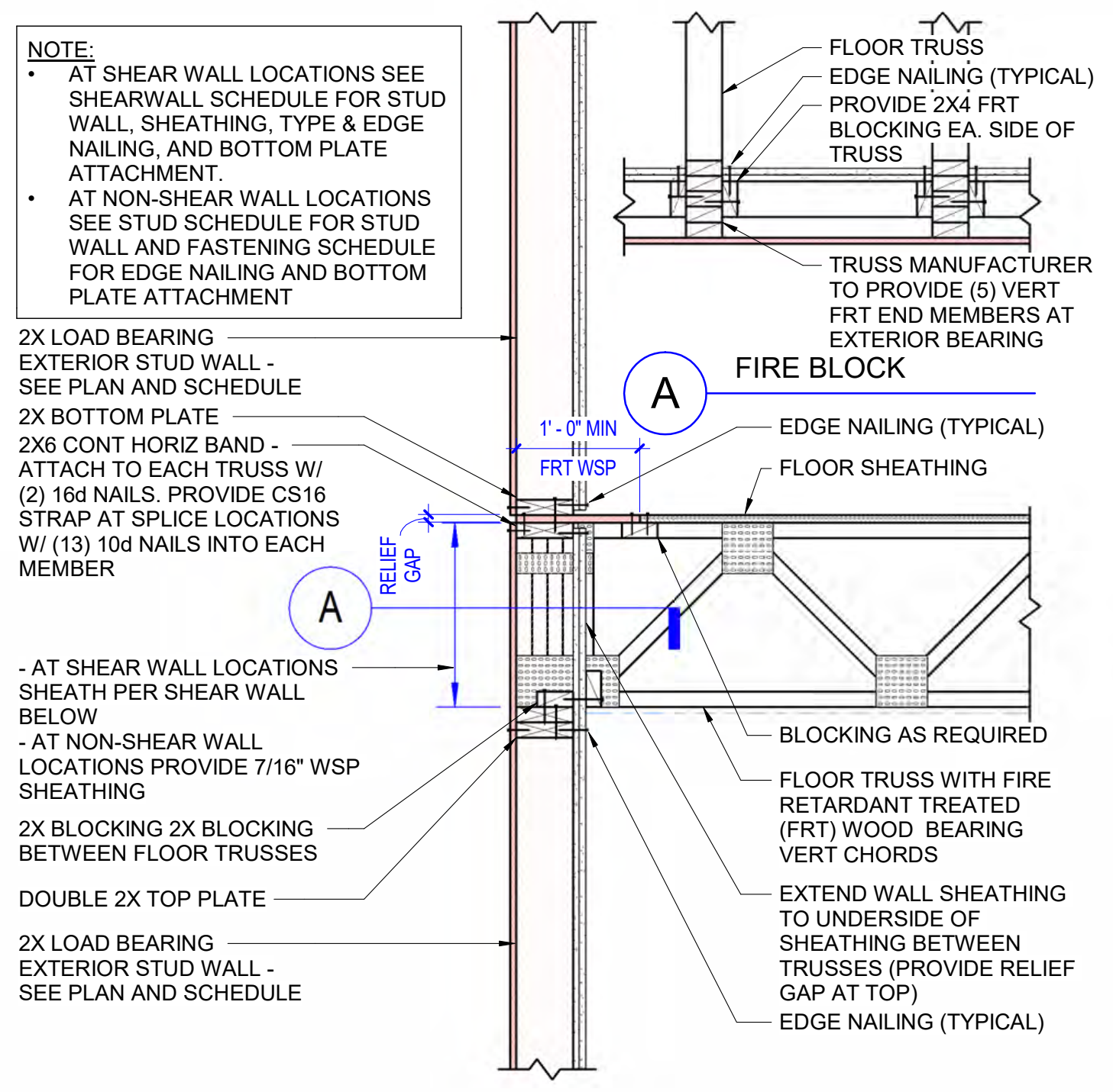
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

1B Truss Bearing Exterior Aletrnate
3/4" = 1'-0"


2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
1' - 0" MIN
FRT WSP
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
RELIEF GAP
DOUBLE 2X TOP PLATE
NOTCH SHEATHING AT TRUSS HANGER PER SIMPSON INSTALLATION INSTRUCTIONS
BLOCKING AS REQUIRED
FLOOR TRUSS
SIMPSON DGHF FIREWALL HANGER - INSTALL PER SIMPSON INSTALLATION INSTRUCTIONS
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

1A Truss Bearing Exterior With Hanger - Alternate
3/4" = 1'-0"


FLOOR TRUSS
EDGE NAILING (TYPICAL)
PROVIDE 2X4 FRT BLOCKING EA. SIDE OF TRUSS
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
TRUSS MANUFACTURER TO PROVIDE (5) VERT FRT END MEMBERS AT EXTERIOR BEARING
2X BOTTOM PLATE
2X6 CONT HORIZ BAND - ATTACH TO EACH TRUSS W/ (2) 16d NAILS. PROVIDE CS16 STRAP AT SPLICE LOCATIONS W/ (13) 10d NAILS INTO EACH MEMBER
A
FIRE BLOCK
1' - 0" MIN
FRT WSP
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
RELIEF GAP
A
- AT SHEAR WALL LOCATIONS SHEATH PER SHEAR WALL BELOW - AT NON-SHEAR WALL LOCATIONS PROVIDE 7/16" WSP SHEATHING
BLOCKING AS REQUIRED
FLOOR TRUSS WITH FIRE RETARDANT TREATED (FRT) WOOD BEARING VERT CHORDS
2X BLOCKING 2X BLOCKING BETWEEN FLOOR TRUSSES
DOUBLE 2X TOP PLATE
EXTEND WALL SHEATHING TO UNDERSIDE OF SHEATHING BETWEEN TRUSSES (PROVIDE RELIEF GAP AT TOP)
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
EDGE NAILING (TYPICAL)

NOTE:
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- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

1 Truss Bearing Exterior
3/4" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
VSMeg Structural Engineers, LLC
1080 Eldridge Parkway
Suite 350
Houston, TX 77077
281-407-5734
www.vsmeg.com

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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No.	Description	Date
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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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- NO. 85617

SHEET TITLE:
Framing Sections - Wood Floor

SHEET NUMBER:
S4.110

NOT RELEASED FOR CONSTRUCTION

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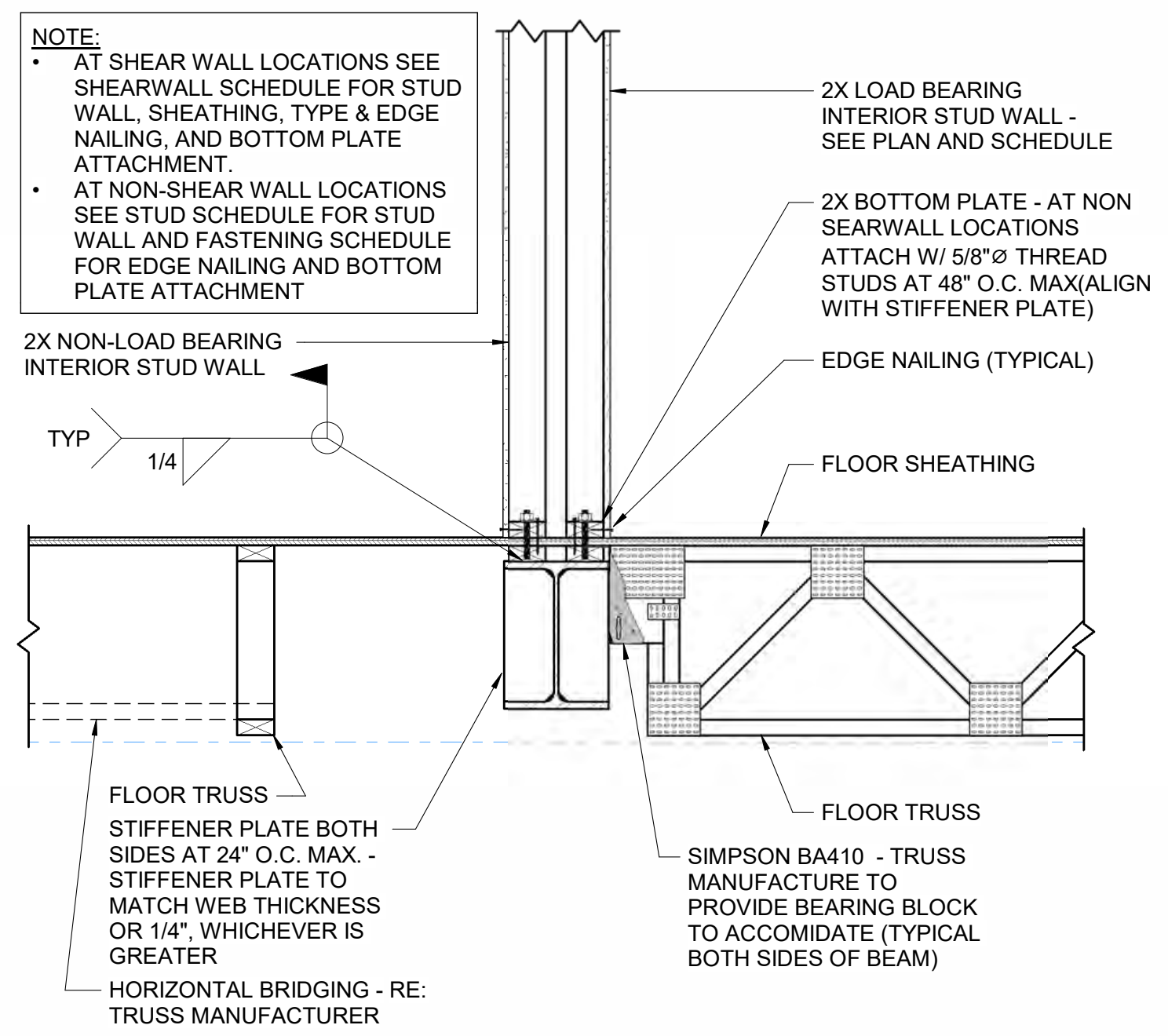
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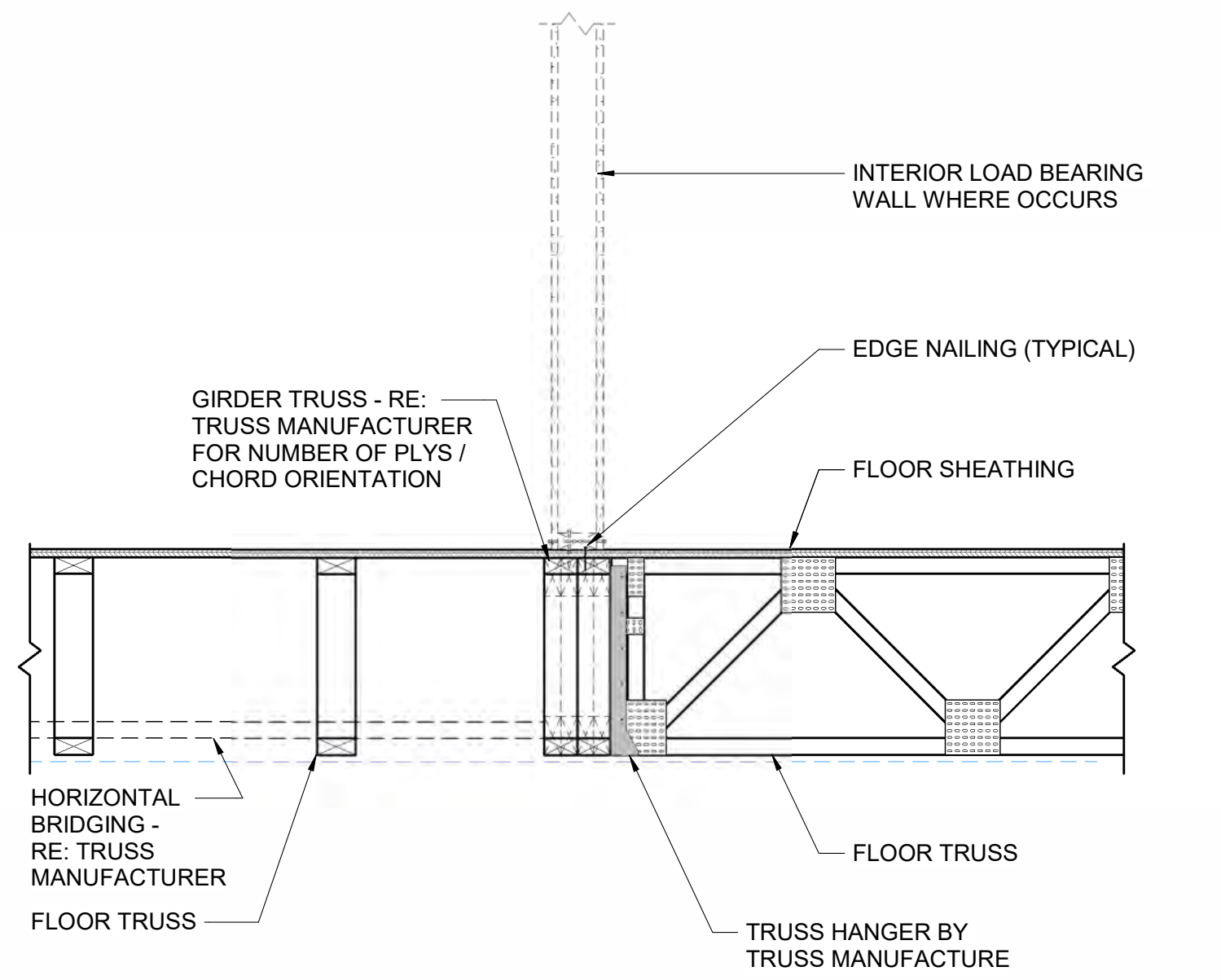
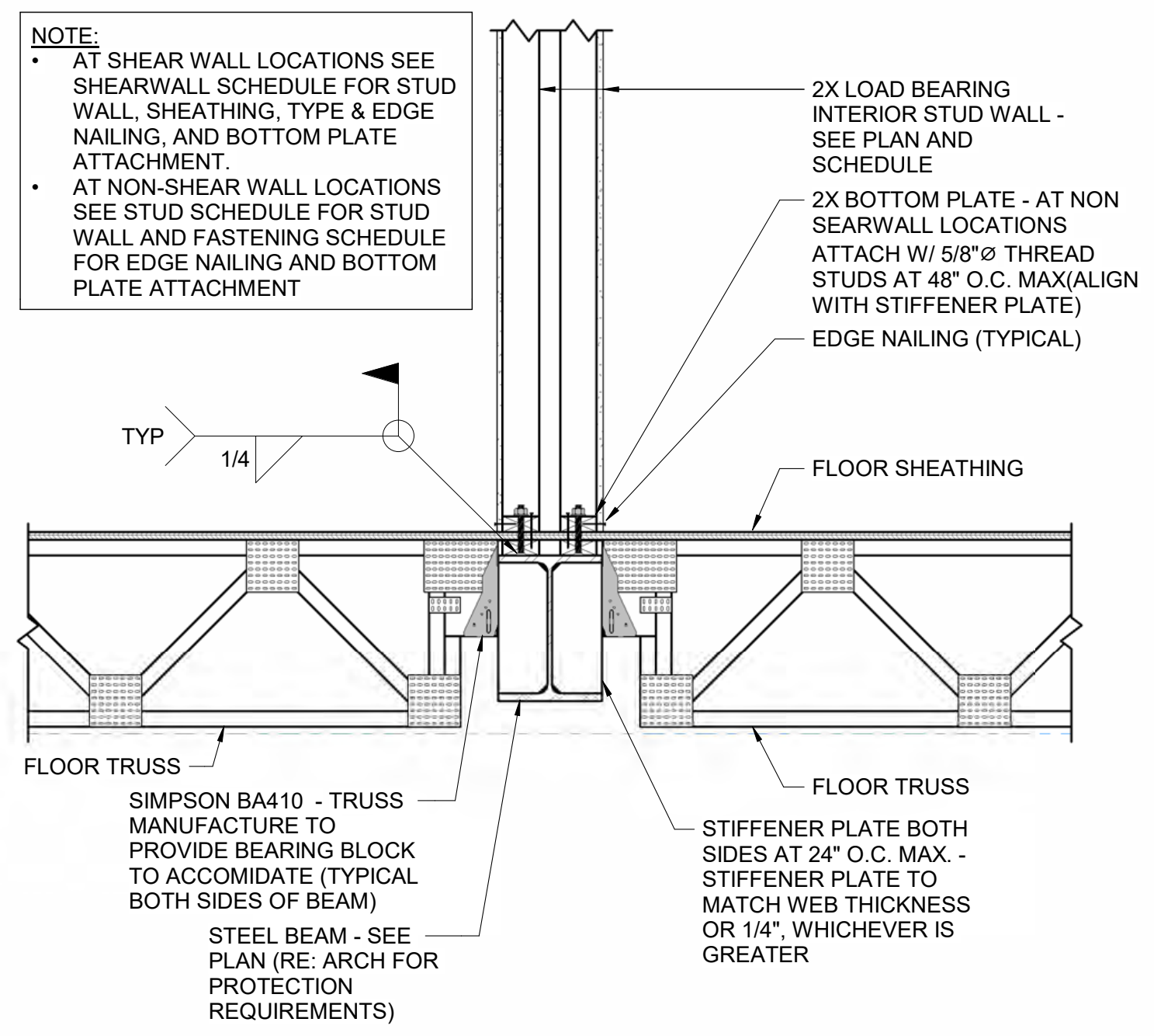
2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE - AT NON SEARWALL LOCATIONS ATTACH W/ 5/8"Ø THREAD STUDS AT 48" O.C. MAX(ALIGN WITH STIFFENER PLATE)
2X NON-LOAD BEARING INTERIOR STUD WALL
EDGE NAILING (TYPICAL)
TYP
1/4
FLOOR SHEATHING
FLOOR TRUSS
STIFFENER PLATE BOTH SIDES AT 24" O.C. MAX. - STIFFENER PLATE TO MATCH WEB THICKNESS OR 1/4", WHICHEVER IS GREATER
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
FLOOR TRUSS
SIMPSON BA410 - TRUSS MANUFACTURE TO PROVIDE BEARING BLOCK TO ACCOMIDATE (TYPICAL BOTH SIDES OF BEAM)

8 Truss Bearing Interior at Steel Beam - Demising Wall - One Side
3/4" = 1'-0"


INTERIOR LOAD BEARING WALL WHERE OCCURS
EDGE NAILING (TYPICAL)
GIRDER TRUSS - RE: TRUSS MANUFACTURER FOR NUMBER OF PLYS / CHORD ORIENTATION
FLOOR SHEATHING
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
FLOOR TRUSS
FLOOR TRUSS
TRUSS HANGER BY TRUSS MANUFACTURE

4 Truss Bearing at Girder Truss - One Side
3/4" = 1'-0"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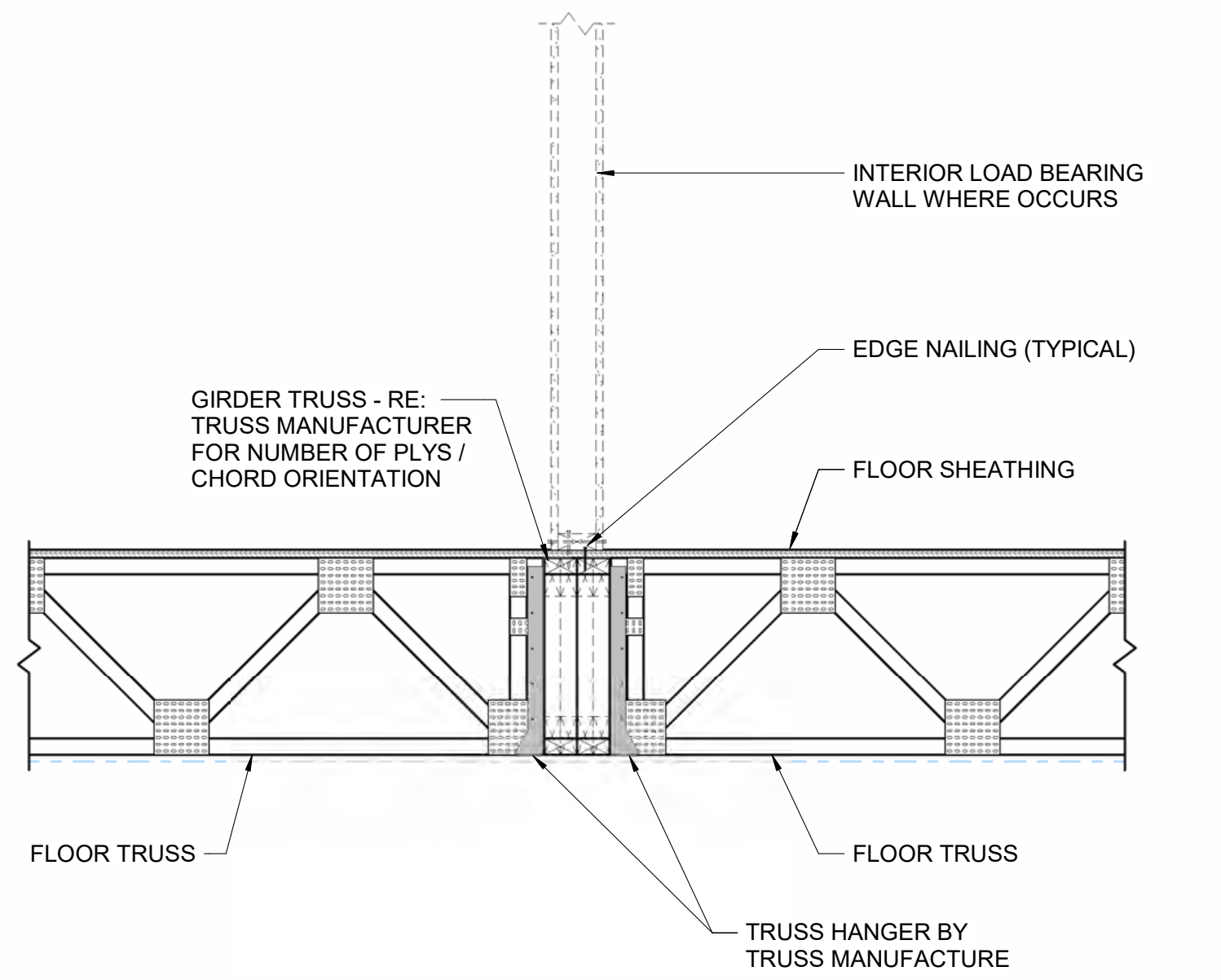
EDGE NAILING (TYPICAL)
TYP
1/4
FLOOR SHEATHING
FLOOR TRUSS
SIMPSON BA410 - TRUSS MANUFACTURE TO PROVIDE BEARING BLOCK TO ACCOMIDATE (TYPICAL BOTH SIDES OF BEAM)
STEEL BEAM - SEE PLAN (RE: ARCH FOR PROTECTION REQUIREMENTS)
FLOOR TRUSS
STIFFENER PLATE BOTH SIDES AT 24" O.C. MAX. - STIFFENER PLATE TO MATCH WEB THICKNESS OR 1/4", WHICHEVER IS GREATER

7 Truss Bearing Interior at Steel Beam - Demising Wall - Both Sides
3/4" = 1'-0"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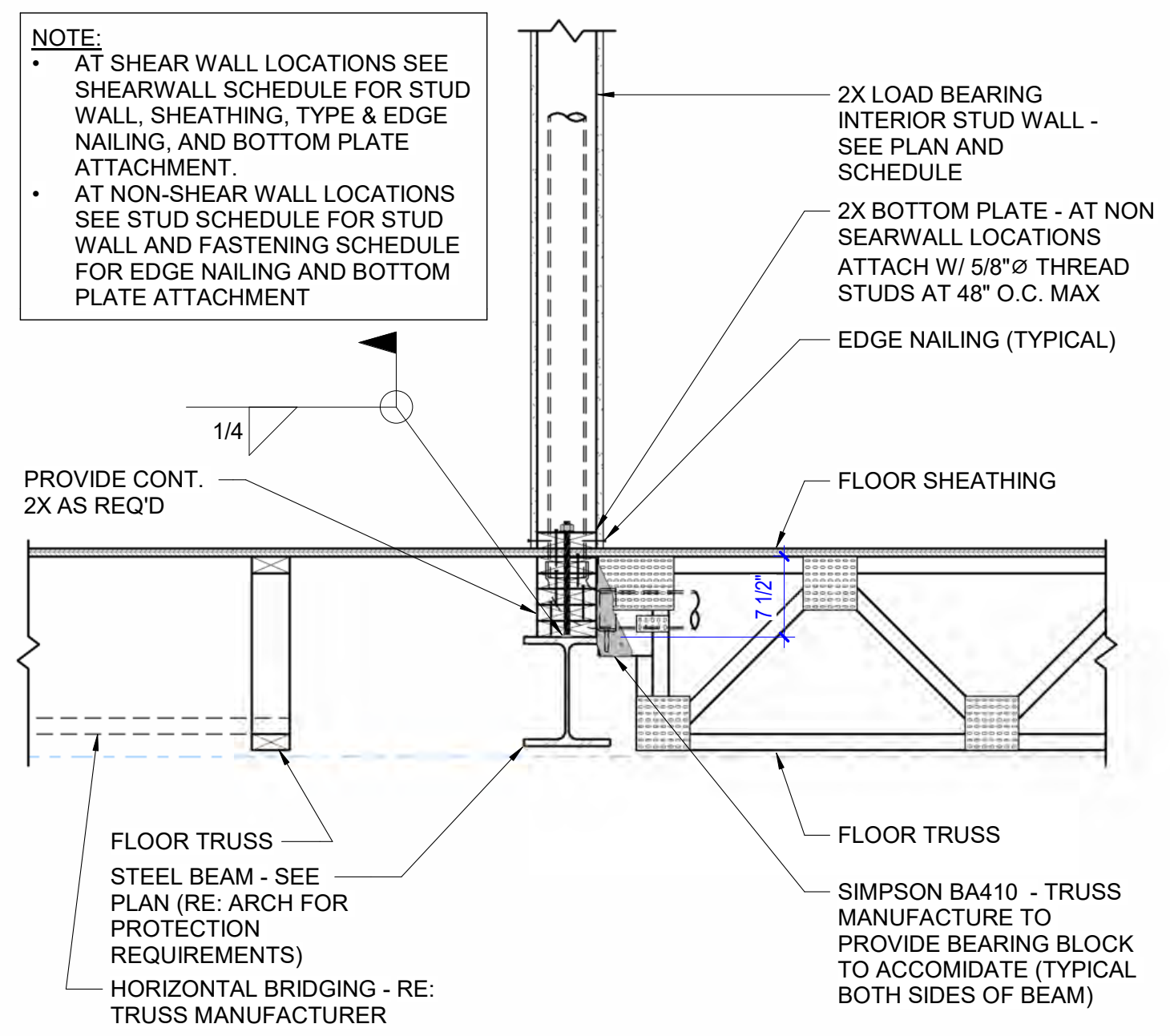
EDGE NAILING (TYPICAL)
GIRDER TRUSS - RE: TRUSS MANUFACTURER FOR NUMBER OF PLYS / CHORD ORIENTATION
FLOOR SHEATHING
FLOOR TRUSS
FLOOR TRUSS
TRUSS HANGER BY TRUSS MANUFACTURE

3 Truss Bearing at Girder Truss - Both Sides
3/4" = 1'-0"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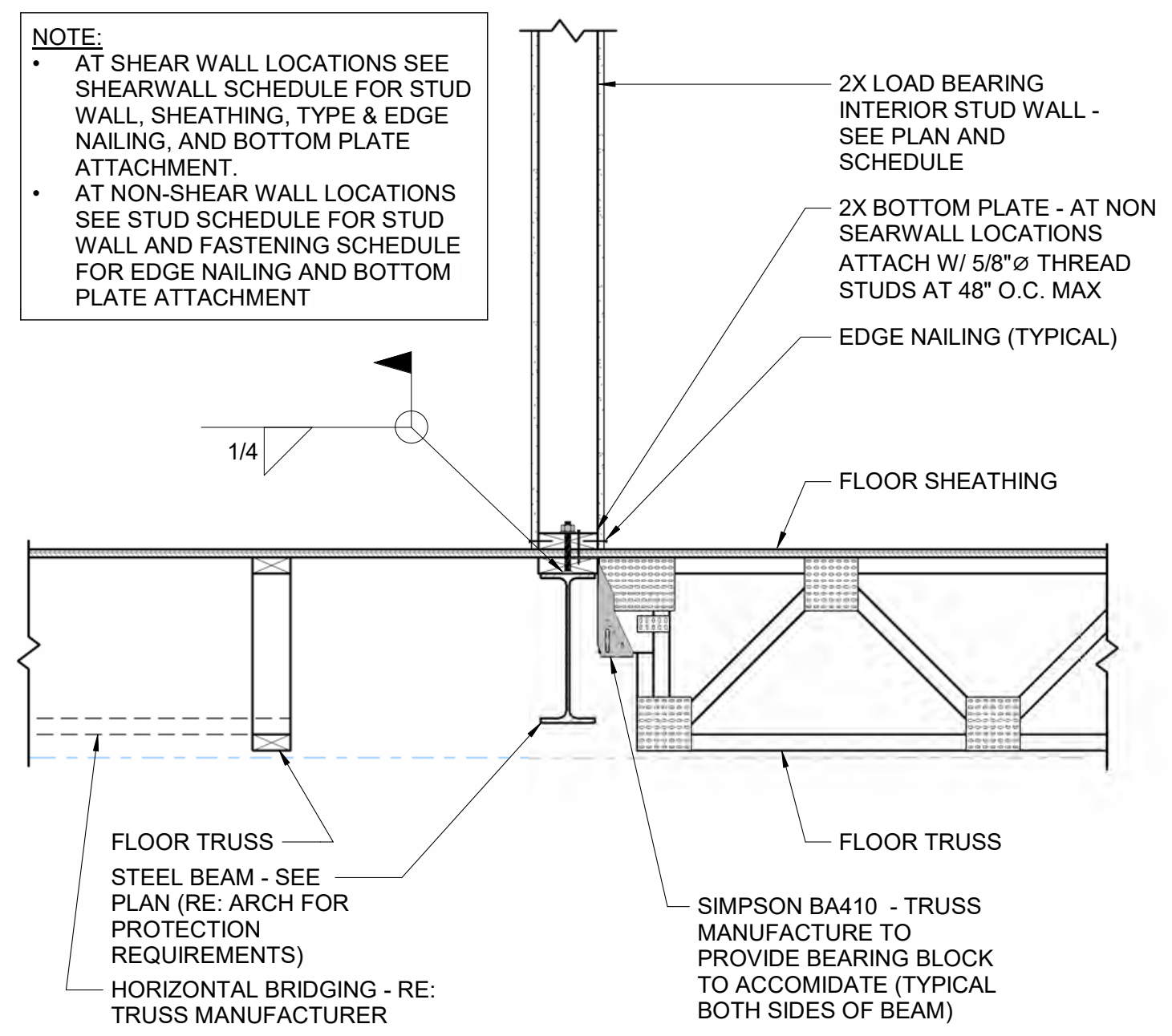
2X BOTTOM PLATE - AT NON SEARWALL LOCATIONS ATTACH W/ 5/8"Ø THREAD STUDS AT 48" O.C. MAX
EDGE NAILING (TYPICAL)
1/4
PROVIDE CONT. 2X AS REQ'D
FLOOR SHEATHING
7 1/2"
FLOOR TRUSS
STEEL BEAM - SEE PLAN (RE: ARCH FOR PROTECTION REQUIREMENTS)
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
FLOOR TRUSS
SIMPSON BA410 - TRUSS MANUFACTURE TO PROVIDE BEARING BLOCK TO ACCOMIDATE (TYPICAL BOTH SIDES OF BEAM)

10 Truss Bearing Interior Plumbing Wall at Steel Beam - One Side
3/4" = 1'-0"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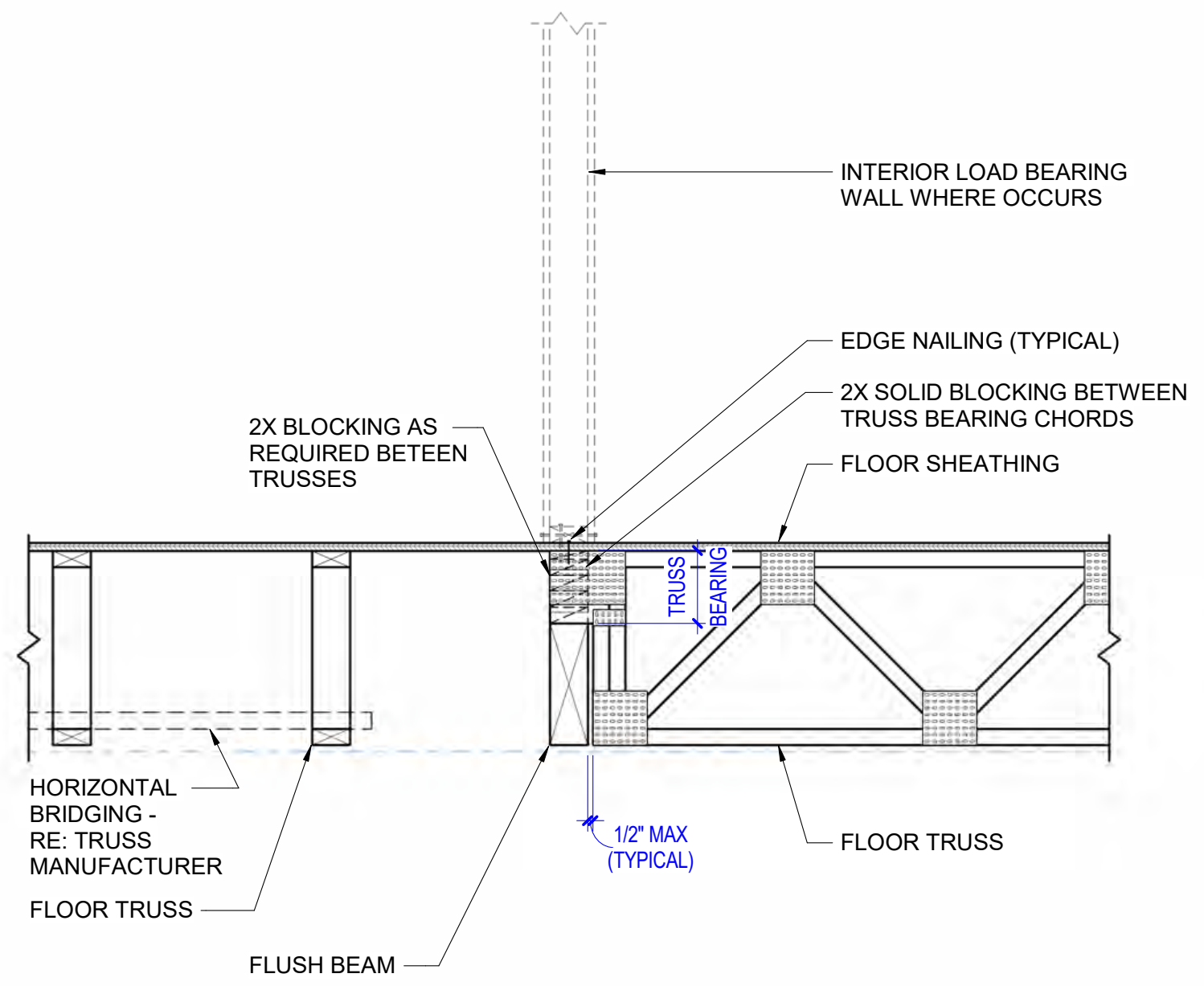
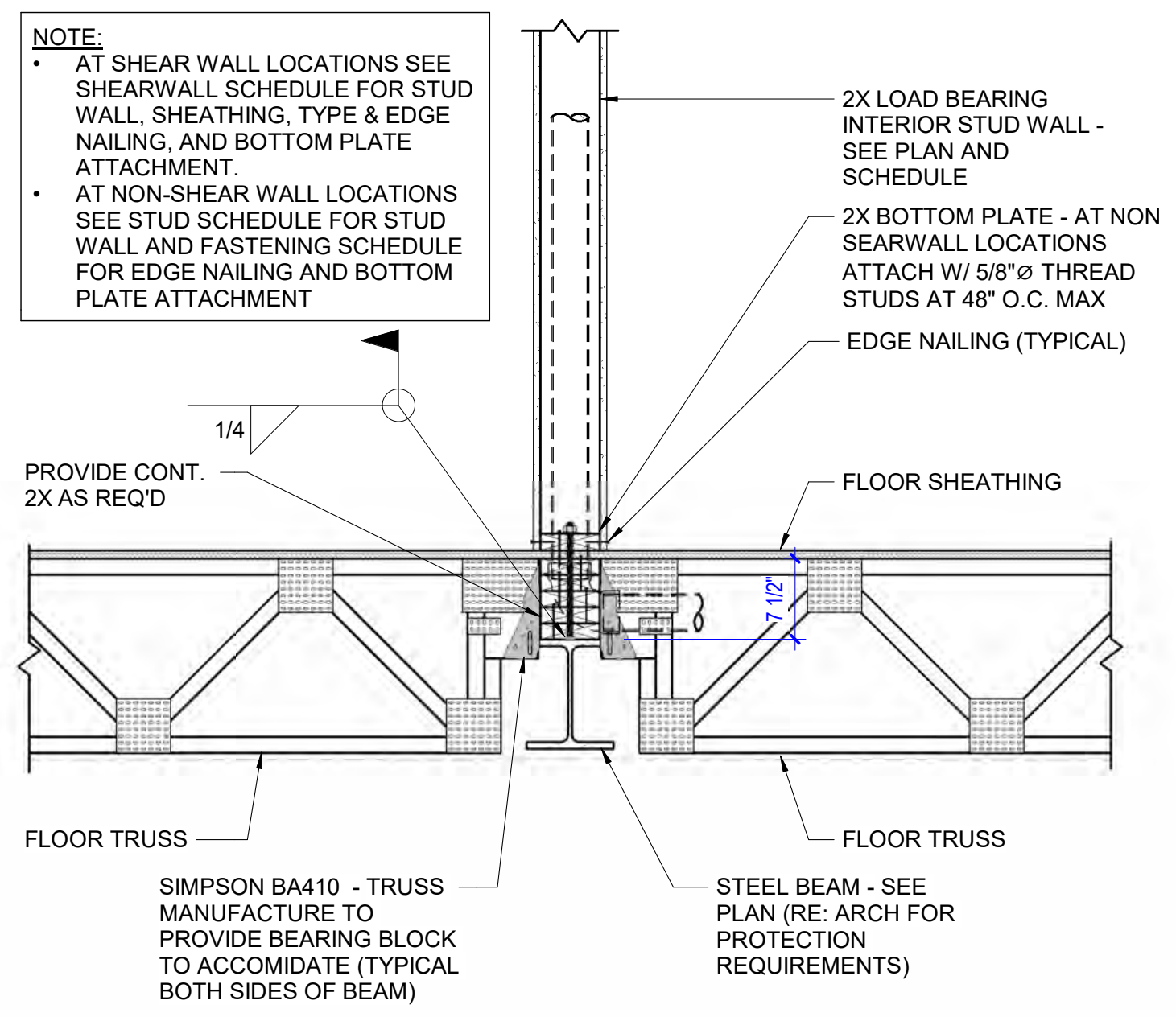
2X BOTTOM PLATE - AT NON SEARWALL LOCATIONS ATTACH W/ 5/8"Ø THREAD STUDS AT 48" O.C. MAX
EDGE NAILING (TYPICAL)
1/4
FLOOR SHEATHING
FLOOR TRUSS
STEEL BEAM - SEE PLAN (RE: ARCH FOR PROTECTION REQUIREMENTS)
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
FLOOR TRUSS
SIMPSON BA410 - TRUSS MANUFACTURE TO PROVIDE BEARING BLOCK TO ACCOMIDATE (TYPICAL BOTH SIDES OF BEAM)

6 Truss Bearing Interior at Steel Beam - One Side
3/4" = 1'-0"


INTERIOR LOAD BEARING WALL WHERE OCCURS
EDGE NAILING (TYPICAL)
2X SOLID BLOCKING BETWEEN TRUSS BEARING CHORDS
2X BLOCKING AS REQUIRED BETEEN TRUSSES
FLOOR SHEATHING
TRUSS BEARING
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
FLOOR TRUSS
1/2" MAX (TYPICAL)
FLOOR TRUSS
FLUSH BEAM

2 Truss Bearing at Flush Beam - One Side
3/4" = 1'-0"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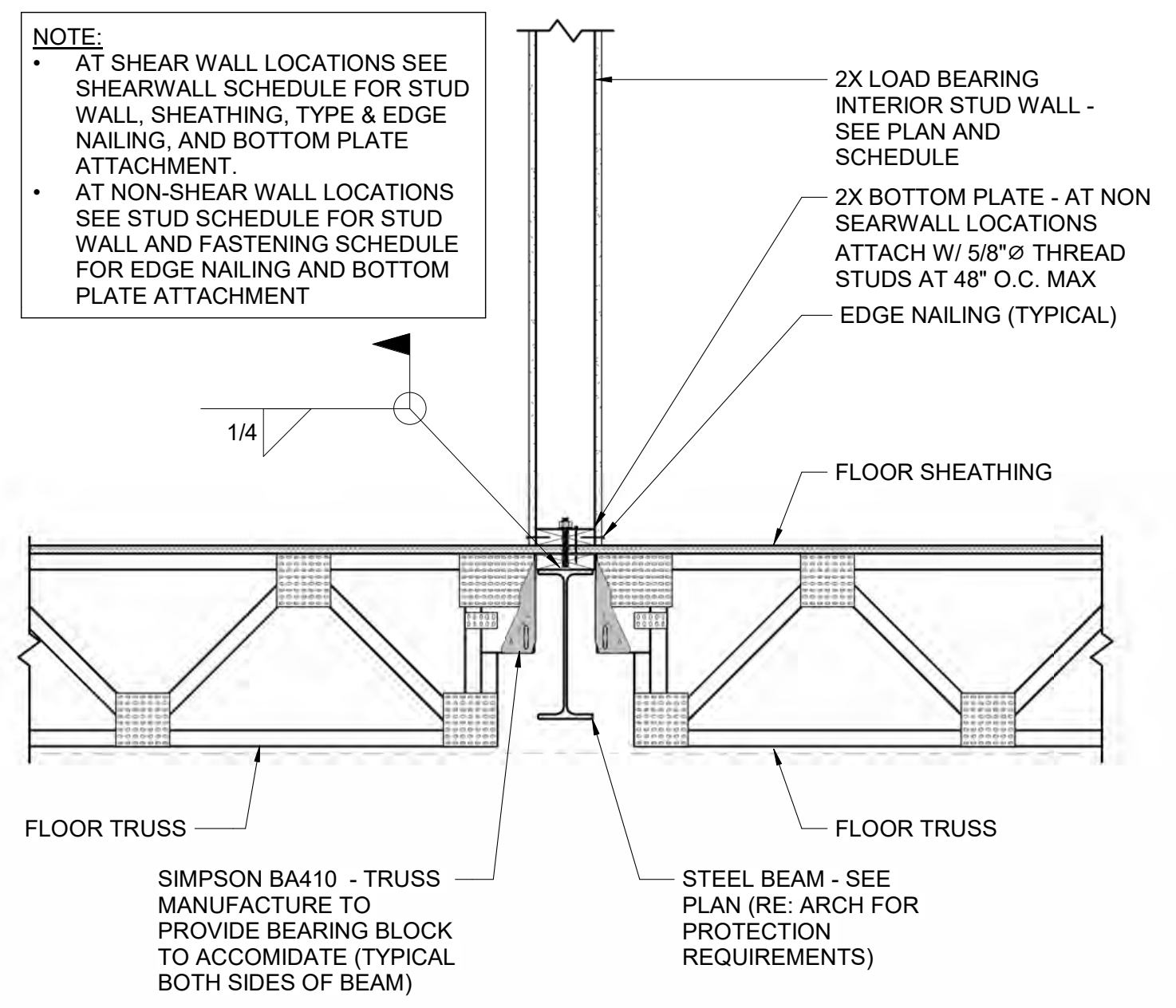
2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE - AT NON SEARWALL LOCATIONS ATTACH W/ 5/8"Ø THREAD STUDS AT 48" O.C. MAX
EDGE NAILING (TYPICAL)
1/4
PROVIDE CONT. 2X AS REQ'D
FLOOR SHEATHING
7 1/2"
FLOOR TRUSS
FLOOR TRUSS
SIMPSON BA410 - TRUSS MANUFACTURE TO PROVIDE BEARING BLOCK TO ACCOMIDATE (TYPICAL BOTH SIDES OF BEAM)
STEEL BEAM - SEE PLAN (RE: ARCH FOR PROTECTION REQUIREMENTS)

9 Truss Bearing Interior Plumbing Wall at Steel Beam - Both Sides
3/4" = 1'-0"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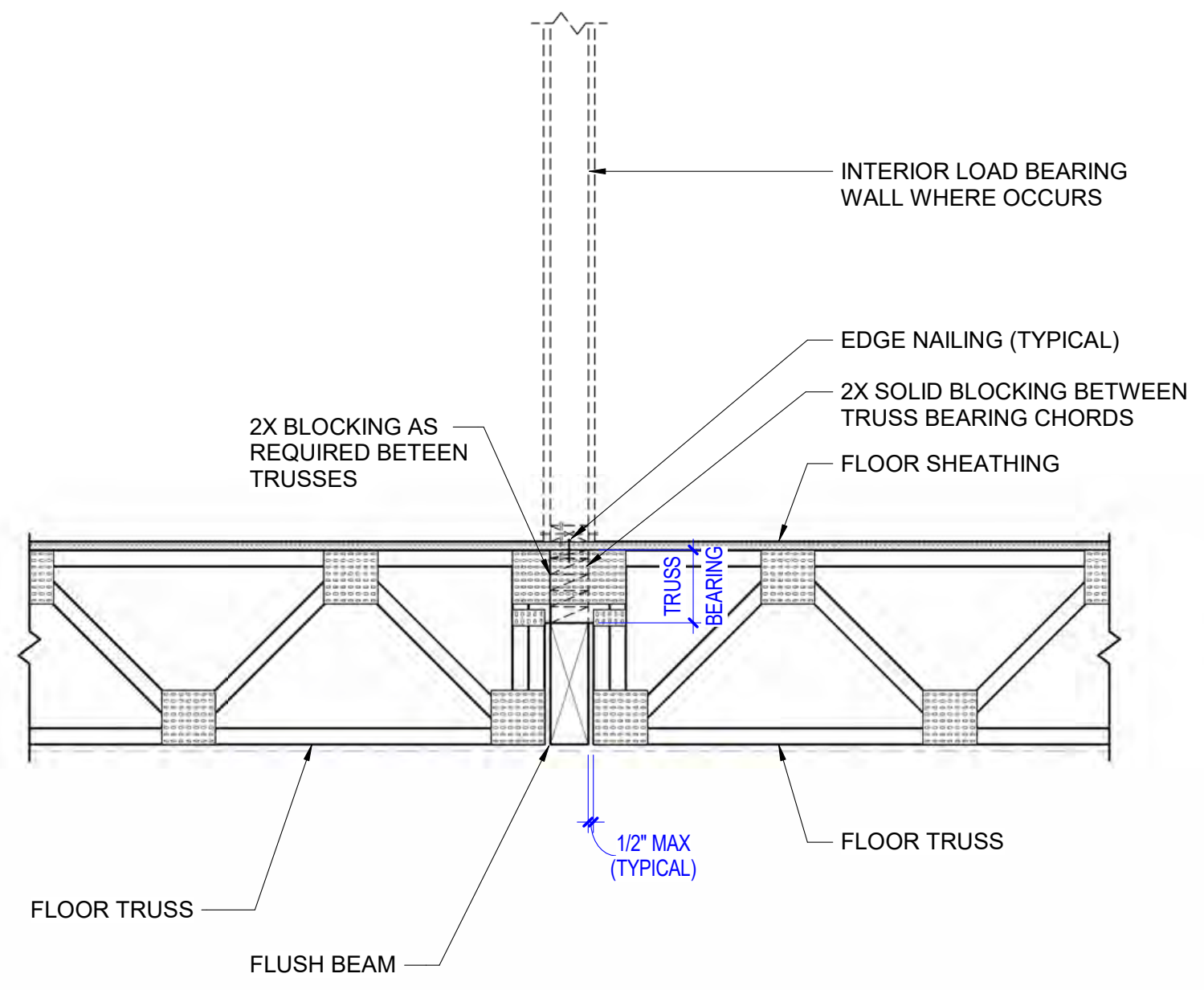
2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE - AT NON SEARWALL LOCATIONS ATTACH W/ 5/8"Ø THREAD STUDS AT 48" O.C. MAX
EDGE NAILING (TYPICAL)
1/4
FLOOR SHEATHING
FLOOR TRUSS
FLOOR TRUSS
SIMPSON BA410 - TRUSS MANUFACTURE TO PROVIDE BEARING BLOCK TO ACCOMIDATE (TYPICAL BOTH SIDES OF BEAM)
STEEL BEAM - SEE PLAN (RE: ARCH FOR PROTECTION REQUIREMENTS)

5 Truss Bearing Interior at Steel Beam - Both Sides
3/4" = 1'-0"


INTERIOR LOAD BEARING WALL WHERE OCCURS
EDGE NAILING (TYPICAL)
2X SOLID BLOCKING BETWEEN TRUSS BEARING CHORDS
2X BLOCKING AS REQUIRED BETEEN TRUSSES
FLOOR SHEATHING
TRUSS BEARING
FLOOR TRUSS
1/2" MAX (TYPICAL)
FLOOR TRUSS
FLUSH BEAM

1 Truss Bearing at Flush Beam - Both Sides
3/4" = 1'-0"

VSM²
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Structural Engineers
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BATON ROUGE, LOUISIANA 70808
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Framing Sections - Wood Floor

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S4.111

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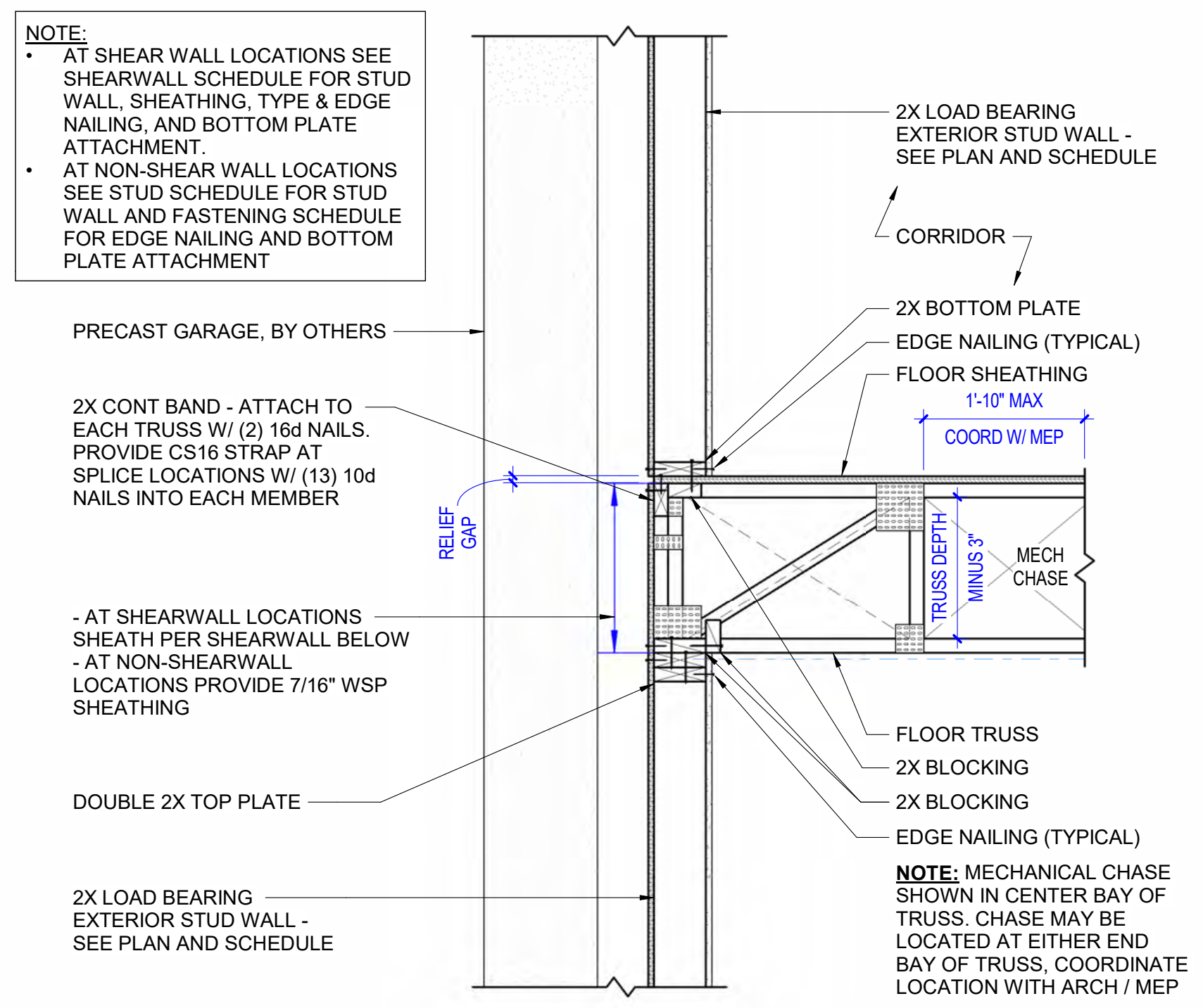
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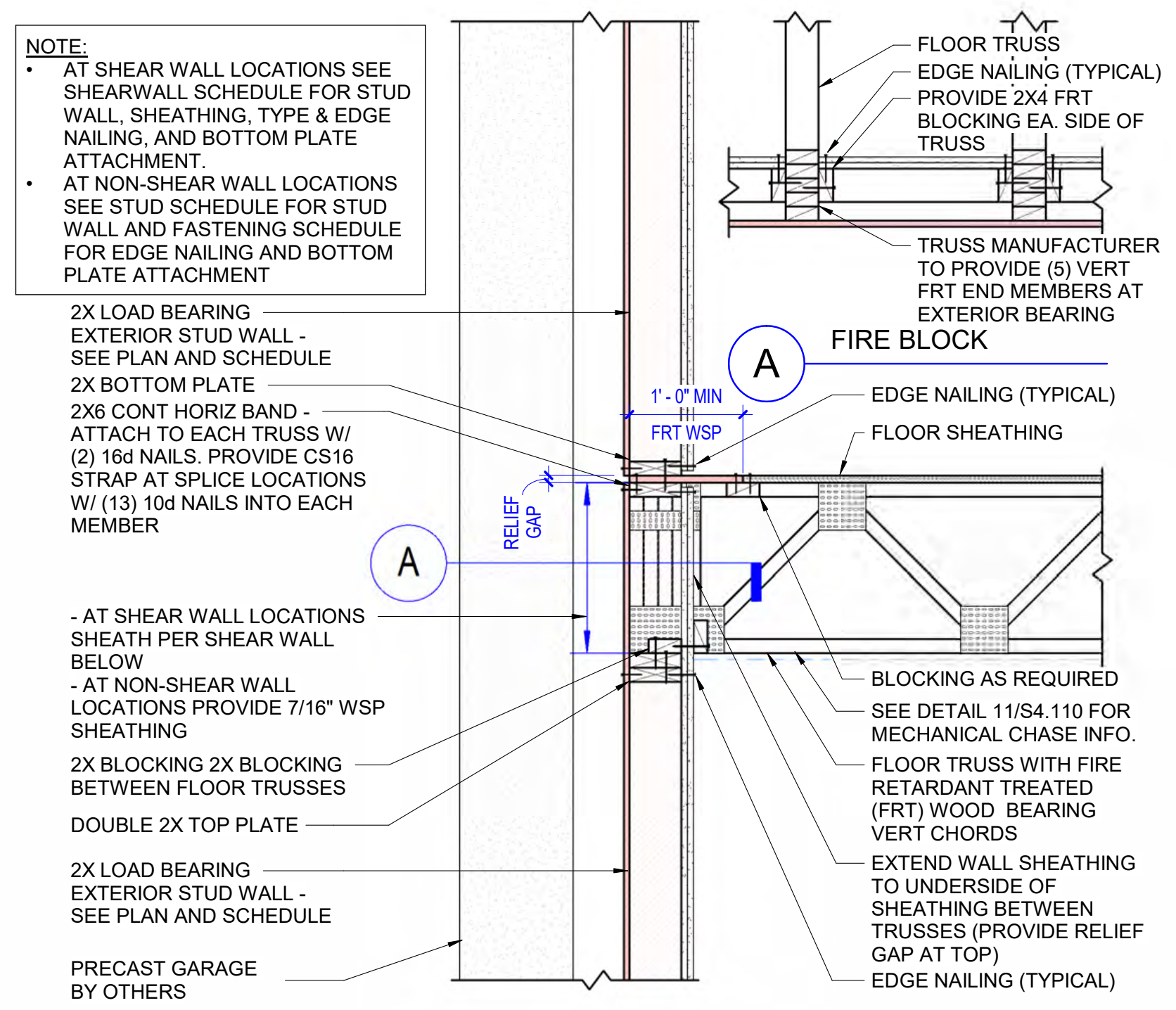
NOTE: MECHANICAL CHASE SHOWN IN CENTER BAY OF TRUSS. CHASE MAY BE LOCATED AT EITHER END BAY OF TRUSS, COORDINATE LOCATION WITH ARCH / MEP

2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
CORRIDOR
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
1'-10" MAX
COORD W/ MEP
MECH CHASE
TRUSS DEPTH MINUS 3"
RELIEF GAP
PRECAST GARAGE, BY OTHERS
2X CONT BAND - ATTACH TO EACH TRUSS W/ (2) 16d NAILS. PROVIDE CS16 STRAP AT SPLICE LOCATIONS W/ (13) 10d NAILS INTO EACH MEMBER
- AT SHEARWALL LOCATIONS SHEATH PER SHEARWALL BELOW - AT NON-SHEARWALL LOCATIONS PROVIDE 7/16" WSP SHEATHING
DOUBLE 2X TOP PLATE
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
FLOOR TRUSS
2X BLOCKING
2X BLOCKING
EDGE NAILING (TYPICAL)

(11) Truss Bearing at Exterior Corridor
3/4" = 1'-0"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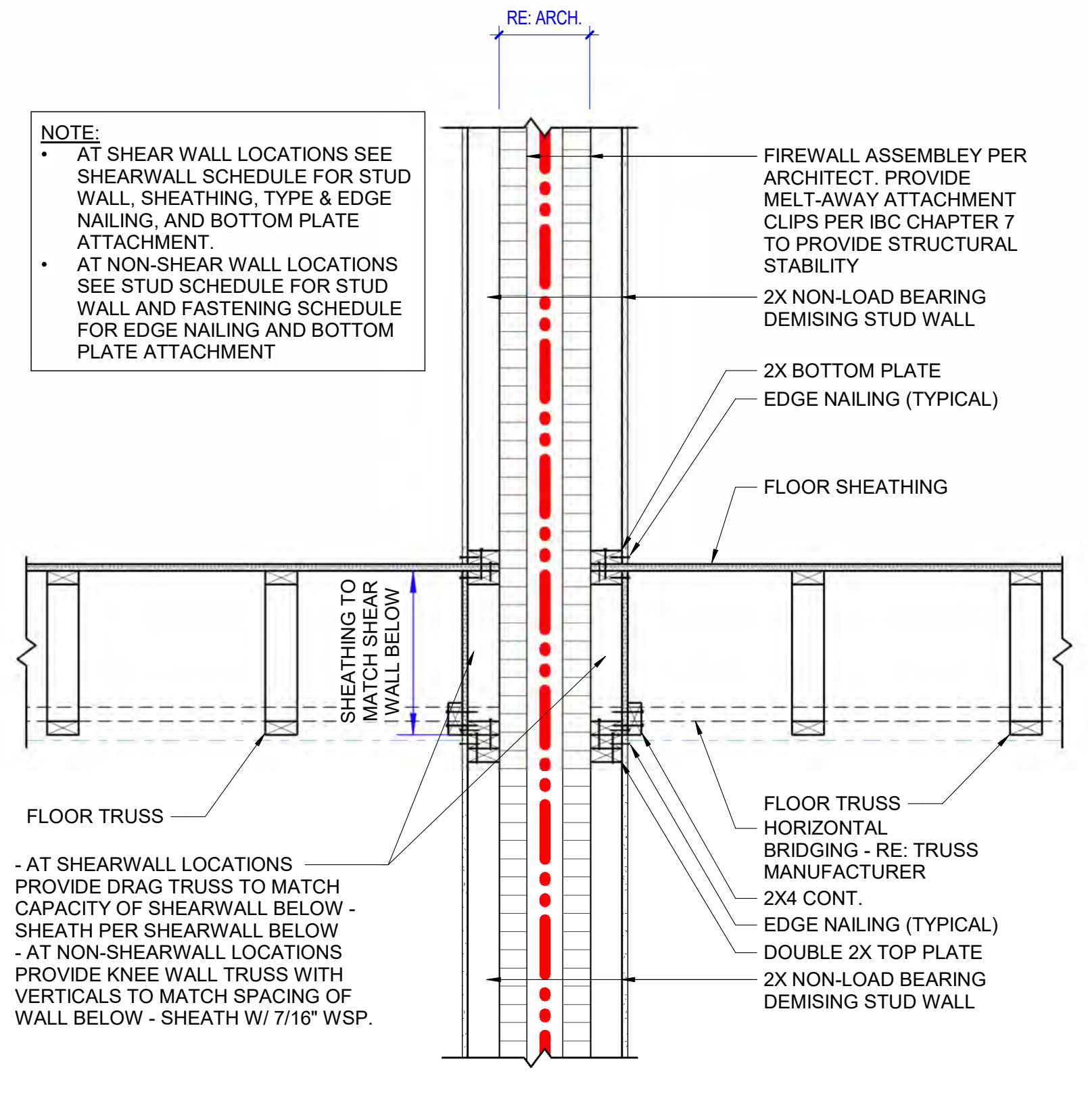
FLOOR TRUSS
EDGE NAILING (TYPICAL)
PROVIDE 2X4 FRT BLOCKING EA. SIDE OF TRUSS
TRUSS MANUFACTURER TO PROVIDE (5) VERT FRT END MEMBERS AT EXTERIOR BEARING
A
FIRE BLOCK
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE
2X6 CONT HORIZ BAND - ATTACH TO EACH TRUSS W/ (2) 16d NAILS. PROVIDE CS16 STRAP AT SPLICE LOCATIONS W/ (13) 10d NAILS INTO EACH MEMBER
1' - 0" MIN
FRT WSP
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
RELIEF GAP
A
- AT SHEAR WALL LOCATIONS SHEATH PER SHEAR WALL BELOW - AT NON-SHEAR WALL LOCATIONS PROVIDE 7/16" WSP SHEATHING
2X BLOCKING 2X BLOCKING BETWEEN FLOOR TRUSSES
DOUBLE 2X TOP PLATE
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
PRECAST GARAGE BY OTHERS
BLOCKING AS REQUIRED
SEE DETAIL 11/S4.110 FOR MECHANICAL CHASE INFO.
FLOOR TRUSS WITH FIRE RETARDANT TREATED (FRT) WOOD BEARING VERT CHORDS
EXTEND WALL SHEATHING TO UNDERSIDE OF SHEATHING BETWEEN TRUSSES (PROVIDE RELIEF GAP AT TOP)
EDGE NAILING (TYPICAL)

(10) Truss Bearing at Exterior Corridor - Fire Rated
3/4" = 1'-0"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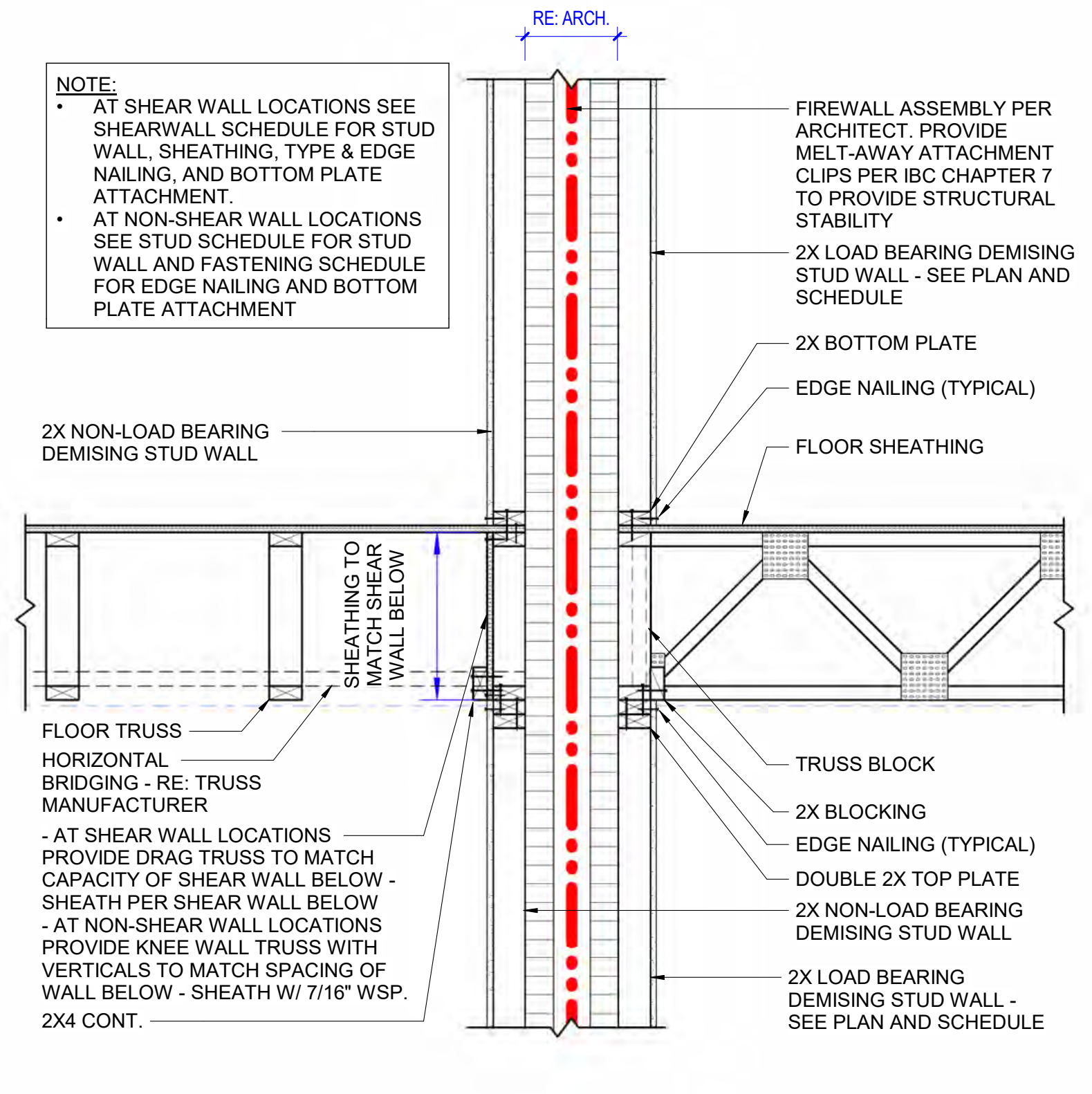
RE: ARCH.
FIREWALL ASSEMBLEY PER ARCHITECT. PROVIDE MELT-AWAY ATTACHMENT CLIPS PER IBC CHAPTER 7 TO PROVIDE STRUCTURAL STABILITY
2X NON-LOAD BEARING DEMISING STUD WALL
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
SHEATHING TO MATCH SHEAR WALL BELOW
FLOOR TRUSS
- AT SHEARWALL LOCATIONS PROVIDE DRAG TRUSS TO MATCH CAPACITY OF SHEARWALL BELOW - SHEATH PER SHEARWALL BELOW - AT NON-SHEARWALL LOCATIONS PROVIDE KNEE WALL TRUSS WITH VERTICALS TO MATCH SPACING OF WALL BELOW - SHEATH W/ 7/16" WSP.
FLOOR TRUSS
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
2X4 CONT.
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE
2X NON-LOAD BEARING DEMISING STUD WALL

(9) Truss Parallel - Firewall
3/4" = 1'-0"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FIREWALL ASSEMBLY PER ARCHITECT. PROVIDE MELT-AWAY ATTACHMENT CLIPS PER IBC CHAPTER 7 TO PROVIDE STRUCTURAL STABILITY
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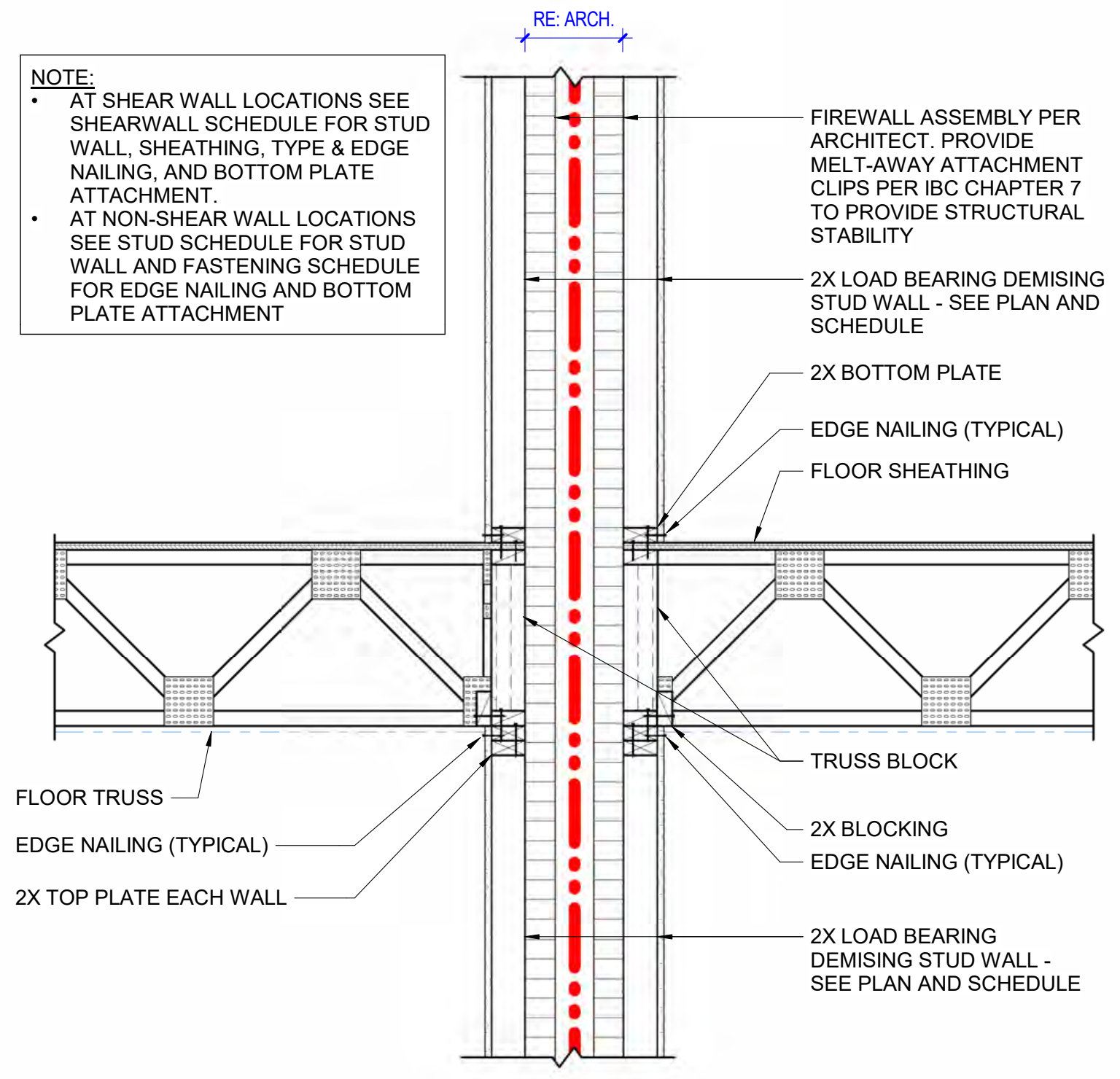
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
2X NON-LOAD BEARING DEMISING STUD WALL
SHEATHING TO MATCH SHEAR WALL BELOW
FLOOR TRUSS
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
- AT SHEAR WALL LOCATIONS PROVIDE DRAG TRUSS TO MATCH CAPACITY OF SHEAR WALL BELOW - SHEATH PER SHEAR WALL BELOW - AT NON-SHEAR WALL LOCATIONS PROVIDE KNEE WALL TRUSS WITH VERTICALS TO MATCH SPACING OF WALL BELOW - SHEATH W/ 7/16" WSP.
2X4 CONT.
TRUSS BLOCK
2X BLOCKING
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE
2X NON-LOAD BEARING DEMISING STUD WALL
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE

(8) Truss Bearing Interior - Firewall - One Side
3/4" = 1'-0"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RE: ARCH.
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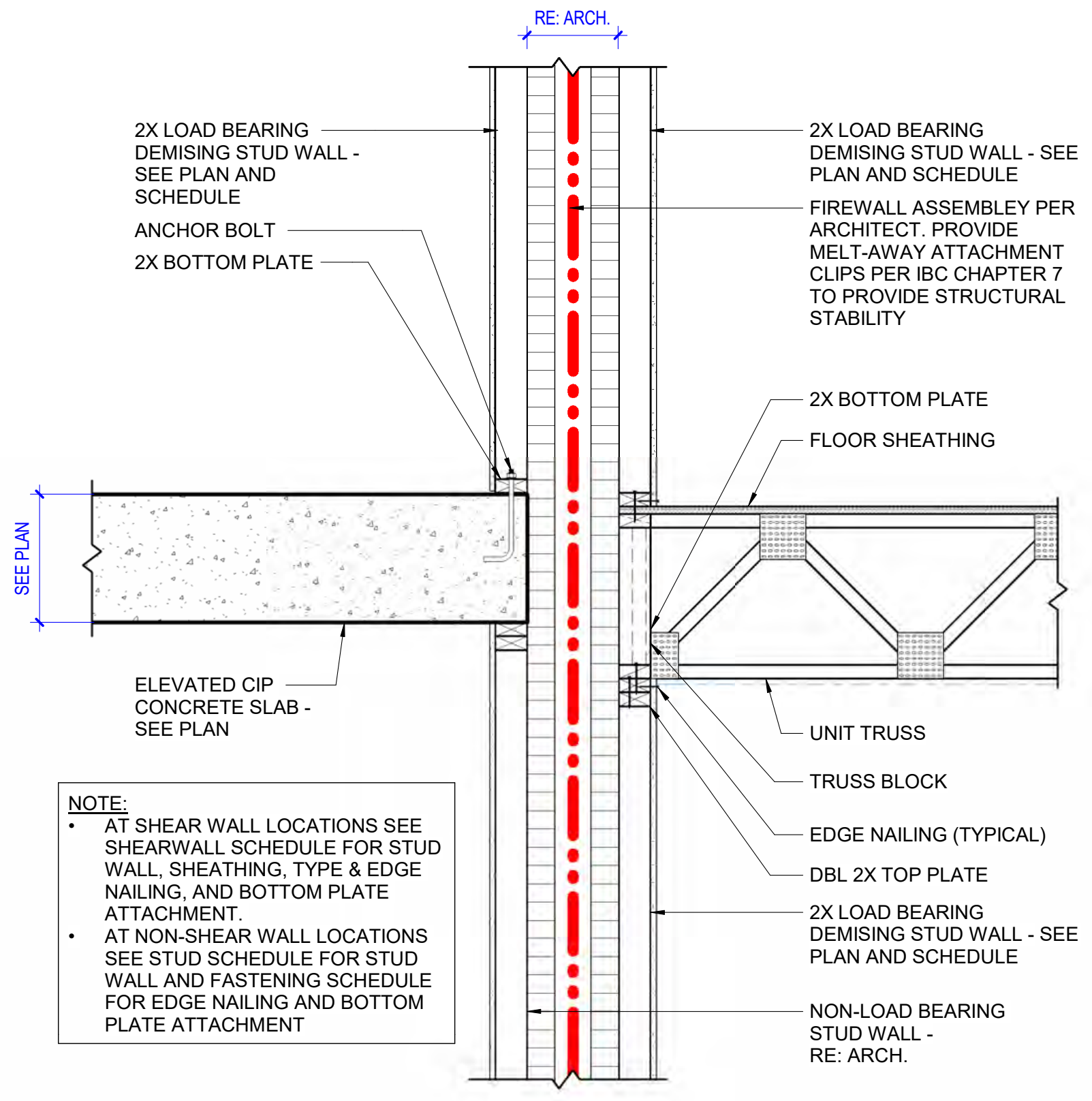
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
FLOOR TRUSS
EDGE NAILING (TYPICAL)
2X TOP PLATE EACH WALL
TRUSS BLOCK
2X BLOCKING
EDGE NAILING (TYPICAL)
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE

(7) Truss Bearing Interior - Firewall - Both Sides
3/4" = 1'-0"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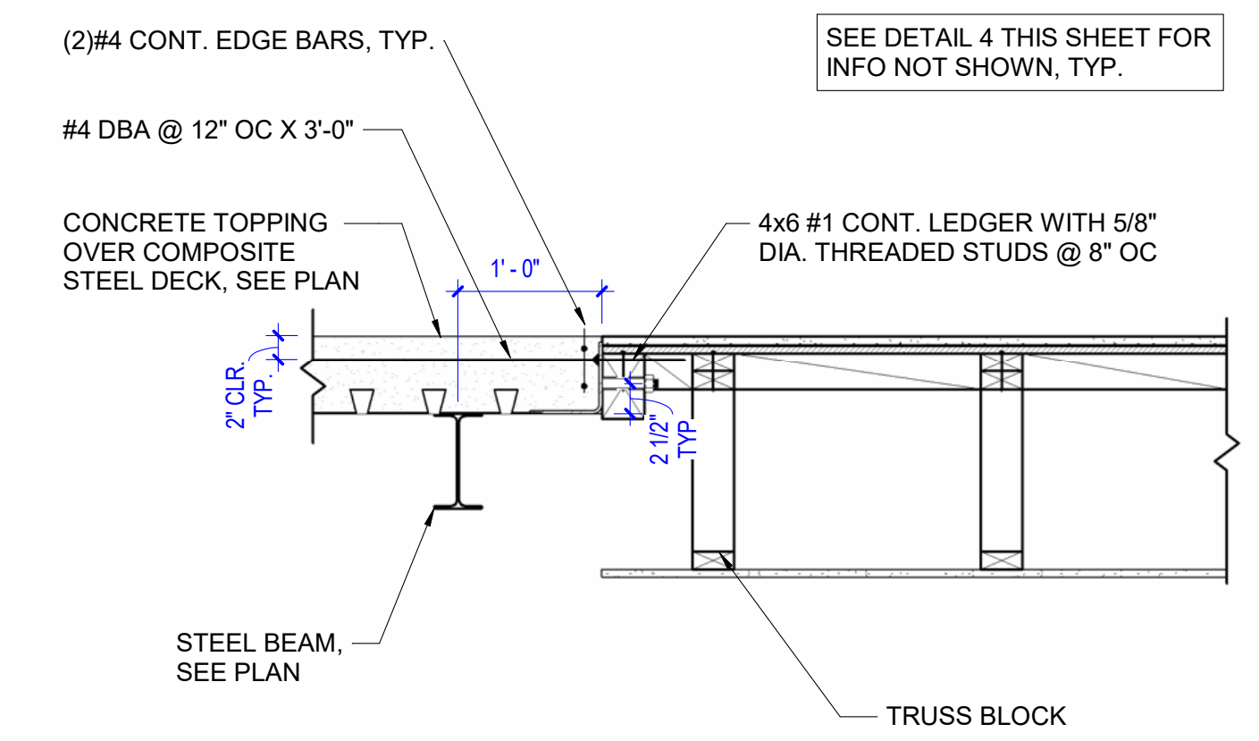
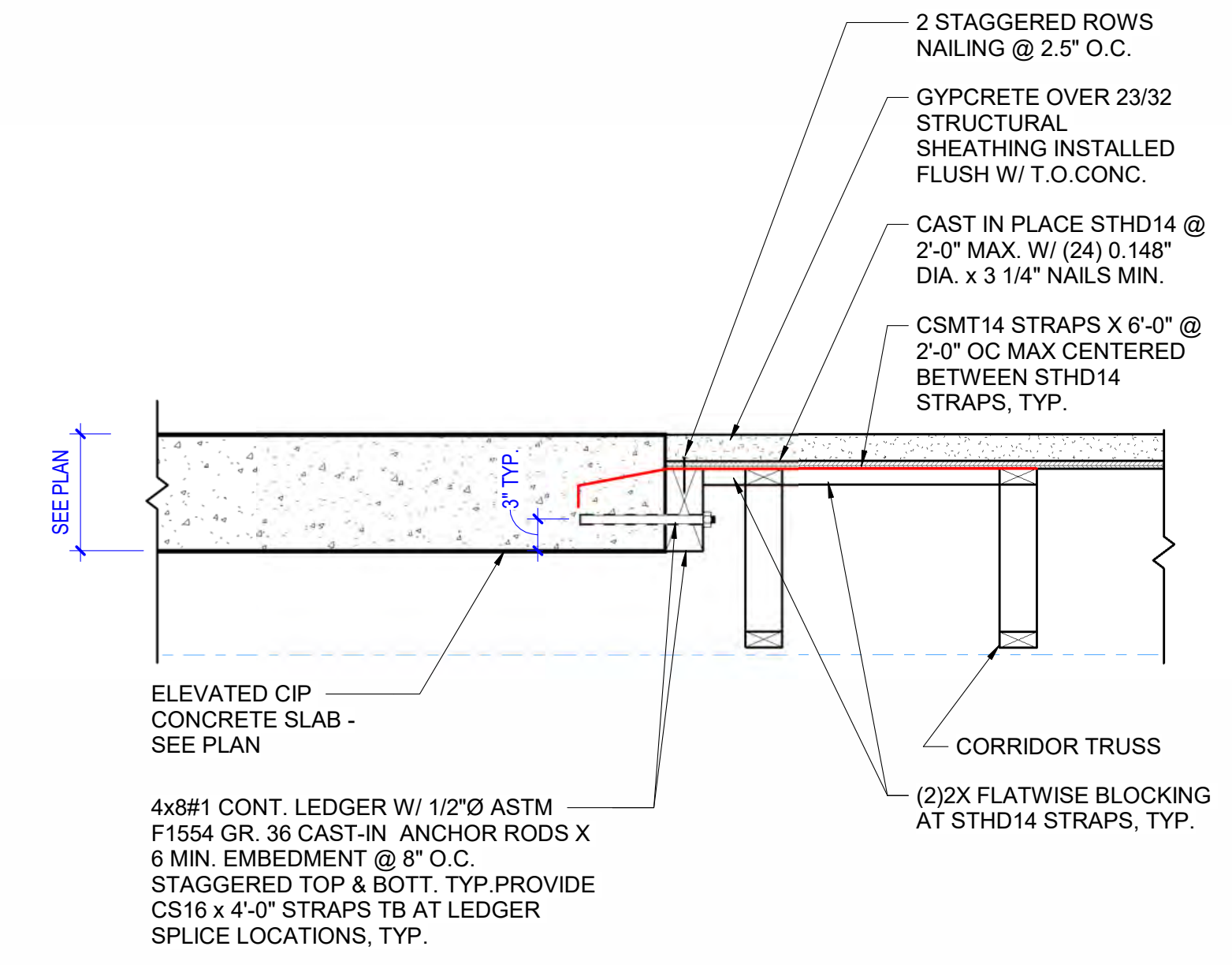
RE: ARCH.
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
ANCHOR BOLT
2X BOTTOM PLATE
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
FIREWALL ASSEMBLEY PER ARCHITECT. PROVIDE MELT-AWAY ATTACHMENT CLIPS PER IBC CHAPTER 7 TO PROVIDE STRUCTURAL STABILITY
2X BOTTOM PLATE
FLOOR SHEATHING
SEE PLAN
ELEVATED CIP CONCRETE SLAB - SEE PLAN
UNIT TRUSS
TRUSS BLOCK
EDGE NAILING (TYPICAL)
DBL 2X TOP PLATE
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
NON-LOAD BEARING STUD WALL - RE: ARCH.

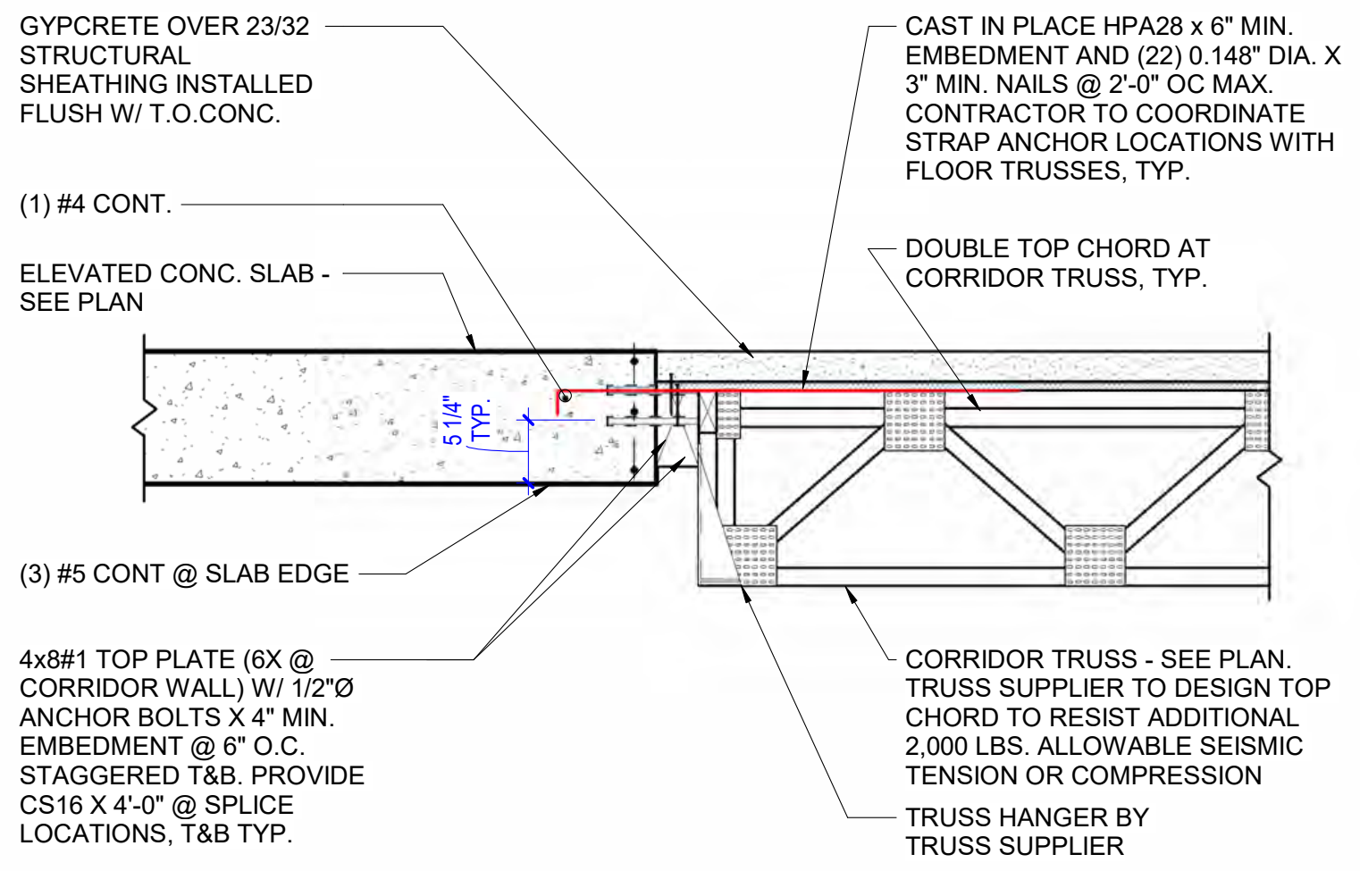
(6) Unit Demising Wall at Podium - Perpendicular Trusses - Firewall
3/4" = 1'-0"


(2)#4 CONT. EDGE BARS, TYP.
#4 DBA @ 12" OC X 3'-0"
CONCRETE TOPPING OVER COMPOSITE STEEL DECK, SEE PLAN
SEE DETAIL 4 THIS SHEET FOR INFO NOT SHOWN, TYP.
4x6 #1 CONT. LEDGER WITH 5/8" DIA. THREADED STUDS @ 8" OC
1' - 0"
2" CLR. TYP.
2 1/2" TYP
STEEL BEAM, SEE PLAN
TRUSS BLOCK

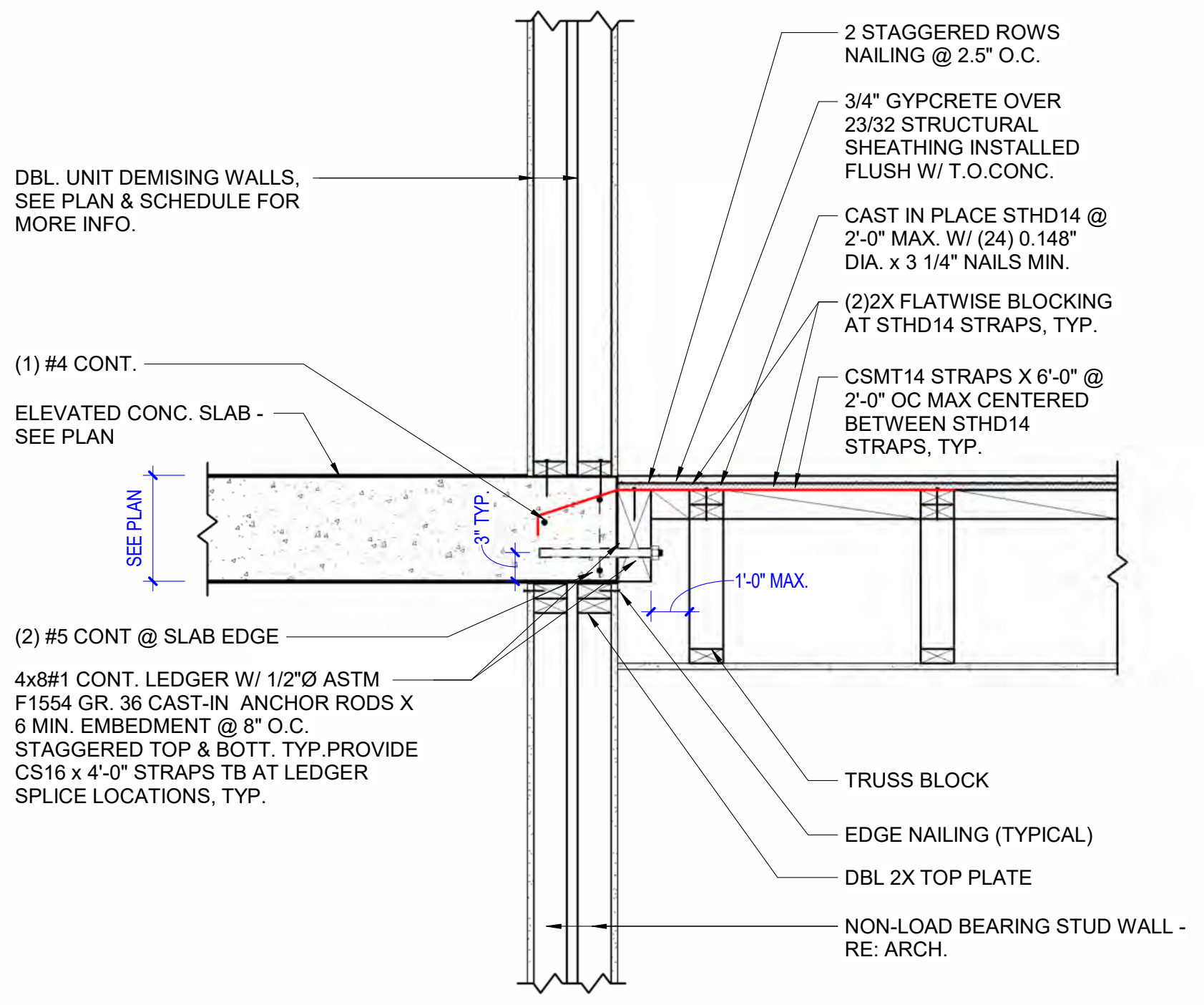
(5) Unit Demising Wall at Podium - Parallel Trusses
3/4" = 1'-0"


2 STAGGERED ROWS NAILING @ 2.5" O.C.
GYPCRETE OVER 23/32 STRUCTURAL SHEATHING INSTALLED FLUSH W/ T.O.CONC.
CAST IN PLACE STHD14 @ 2'-0" MAX. W/ (24) 0.148" DIA. x 3 1/4" NAILS MIN.
CSMT14 STRAPS X 6'-0" @ 2'-0" OC MAX CENTERED BETWEEN STHD14 STRAPS, TYP.
SEE PLAN
3" TYP.
ELEVATED CIP CONCRETE SLAB - SEE PLAN
4x8#1 CONT. LEDGER W/ 1/2"Ø ASTM F1554 GR. 36 CAST-IN ANCHOR RODS X 6 MIN. EMBEDMENT @ 8" O.C. STAGGERED TOP & BOTT. TYP.PROVIDE CS16 x 4'-0" STRAPS TB AT LEDGER SPLICE LOCATIONS, TYP.
CORRIDOR TRUSS
(2)2X FLATWISE BLOCKING AT STHD14 STRAPS, TYP.

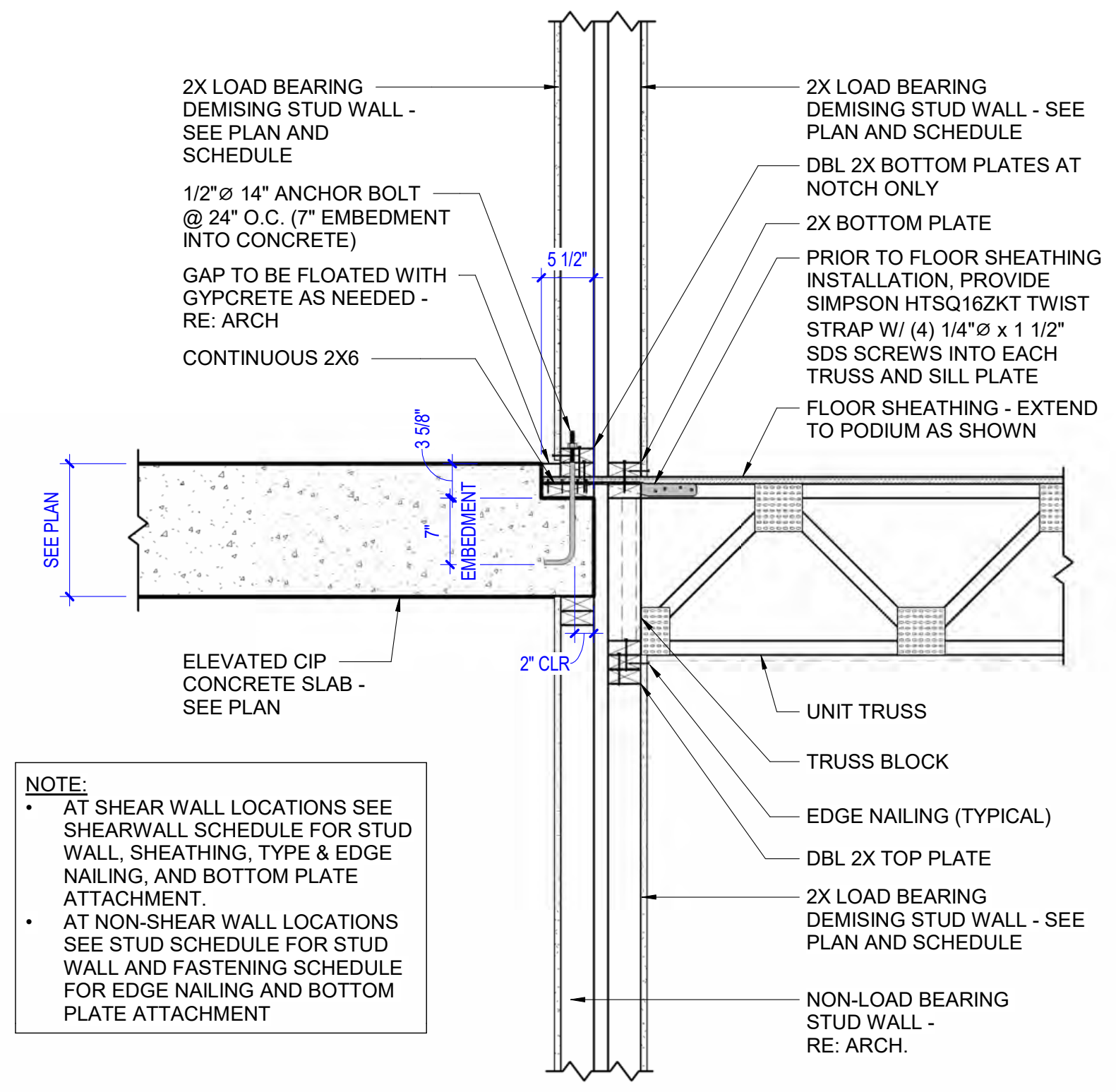
(4) Corridor Wall at Podium - Parallel Trusses
3/4" = 1'-0"


GYPCRETE OVER 23/32 STRUCTURAL SHEATHING INSTALLED FLUSH W/ T.O.CONC.
CAST IN PLACE HPA28 x 6" MIN. EMBEDMENT AND (22) 0.148" DIA. X 3" MIN. NAILS @ 2'-0" OC MAX. CONTRACTOR TO COORDINATE STRAP ANCHOR LOCATIONS WITH FLOOR TRUSSES, TYP.
(1) #4 CONT.
DOUBLE TOP CHORD AT CORRIDOR TRUSS, TYP.
ELEVATED CONC. SLAB - SEE PLAN
5 1/4" TYP.
(3) #5 CONT @ SLAB EDGE
4x8#1 TOP PLATE (6X @ CORRIDOR WALL) W/ 1/2"Ø ANCHOR BOLTS X 4" MIN. EMBEDMENT @ 6" O.C. STAGGERED T&B. PROVIDE CS16 X 4'-0" @ SPLICE LOCATIONS, T&B TYP.
CORRIDOR TRUSS - SEE PLAN. TRUSS SUPPLIER TO DESIGN TOP CHORD TO RESIST ADDITIONAL 2,000 LBS. ALLOWABLE SEISMIC TENSION OR COMPRESSION
TRUSS HANGER BY TRUSS SUPPLIER

(3) Corridor Wall at Podium - Perpendicular Trusses
3/4" = 1'-0"


2 STAGGERED ROWS NAILING @ 2.5" O.C.
3/4" GYPCRETE OVER 23/32 STRUCTURAL SHEATHING INSTALLED FLUSH W/ T.O.CONC.
DBL. UNIT DEMISING WALLS, SEE PLAN & SCHEDULE FOR MORE INFO.
CAST IN PLACE STHD14 @ 2'-0" MAX. W/ (24) 0.148" DIA. x 3 1/4" NAILS MIN.
(2)2X FLATWISE BLOCKING AT STHD14 STRAPS, TYP.
(1) #4 CONT.
CSMT14 STRAPS X 6'-0" @ 2'-0" OC MAX CENTERED BETWEEN STHD14 STRAPS, TYP.
ELEVATED CONC. SLAB - SEE PLAN
SEE PLAN
3" TYP.
1'-0" MAX.
(2) #5 CONT @ SLAB EDGE
4x8#1 CONT. LEDGER W/ 1/2"Ø ASTM F1554 GR. 36 CAST-IN ANCHOR RODS X 6 MIN. EMBEDMENT @ 8" O.C. STAGGERED TOP & BOTT. TYP.PROVIDE CS16 x 4'-0" STRAPS TB AT LEDGER SPLICE LOCATIONS, TYP.
TRUSS BLOCK
EDGE NAILING (TYPICAL)
DBL 2X TOP PLATE
NON-LOAD BEARING STUD WALL - RE: ARCH.

(2) Unit Demising Wall at Podium - Parallel Trusses
3/4" = 1'-0"


NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
1/2"⌀ 14" ANCHOR BOLT @ 24" O.C. (7" EMBEDMENT INTO CONCRETE)
GAP TO BE FLOATED WITH GYPCRETE AS NEEDED - RE: ARCH
CONTINUOUS 2X6
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
DBL 2X BOTTOM PLATES AT NOTCH ONLY
2X BOTTOM PLATE
PRIOR TO FLOOR SHEATHING INSTALLATION, PROVIDE SIMPSON HTSQ16ZKT TWIST STRAP W/ (4) 1/4"⌀ x 1 1/2" SDS SCREWS INTO EACH TRUSS AND SILL PLATE
FLOOR SHEATHING - EXTEND TO PODIUM AS SHOWN
5 1/2"
3 5/8"
7" EMBEDMENT
SEE PLAN
2" CLR
ELEVATED CIP CONCRETE SLAB - SEE PLAN
UNIT TRUSS
TRUSS BLOCK
EDGE NAILING (TYPICAL)
DBL 2X TOP PLATE
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
NON-LOAD BEARING STUD WALL - RE: ARCH.

(1) Unit Demising Wall at Podium - Perpendicular Trusses
3/4" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
Framing Sections - Wood Floor

SHEET NUMBER:
S4.112

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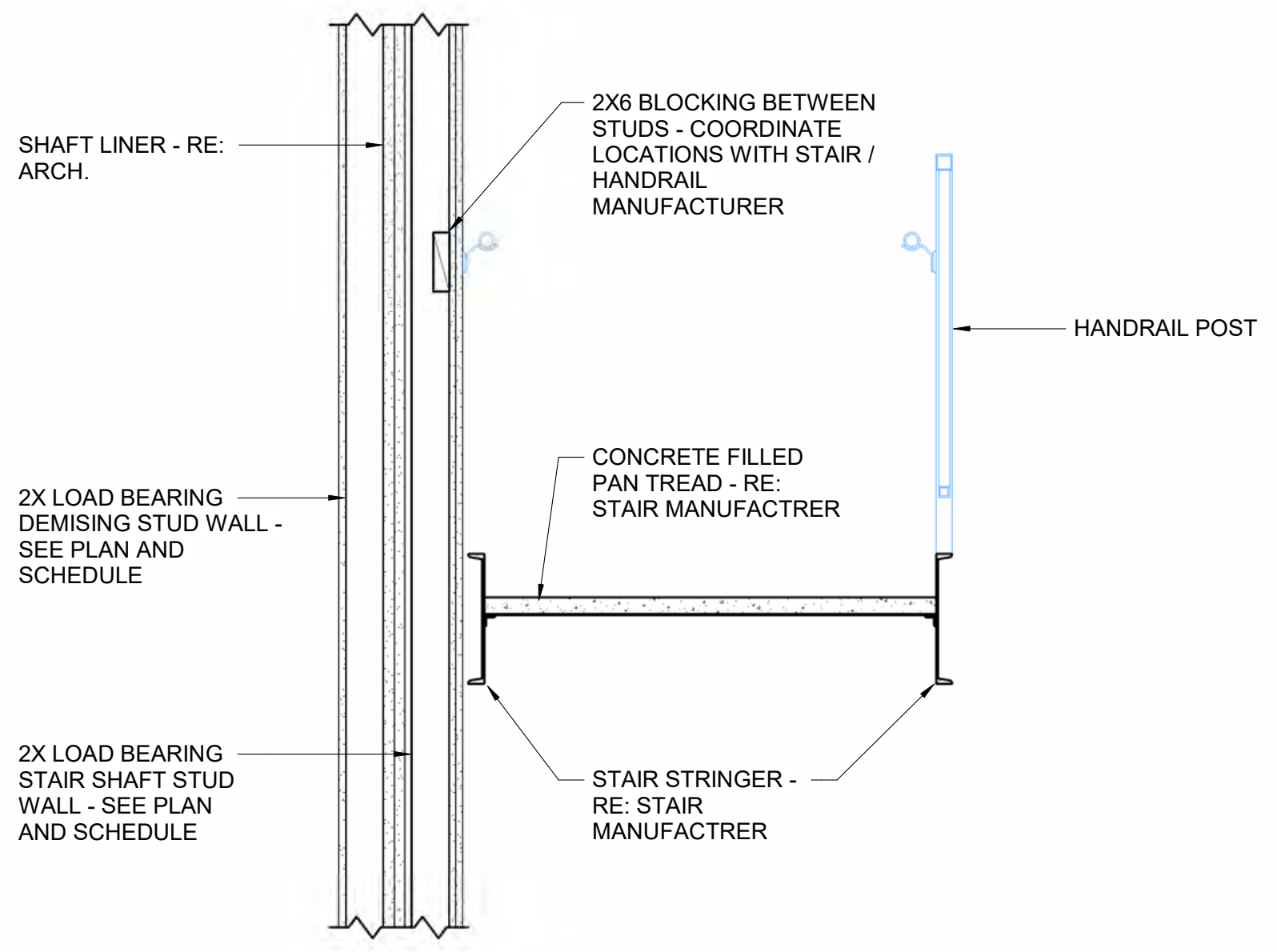
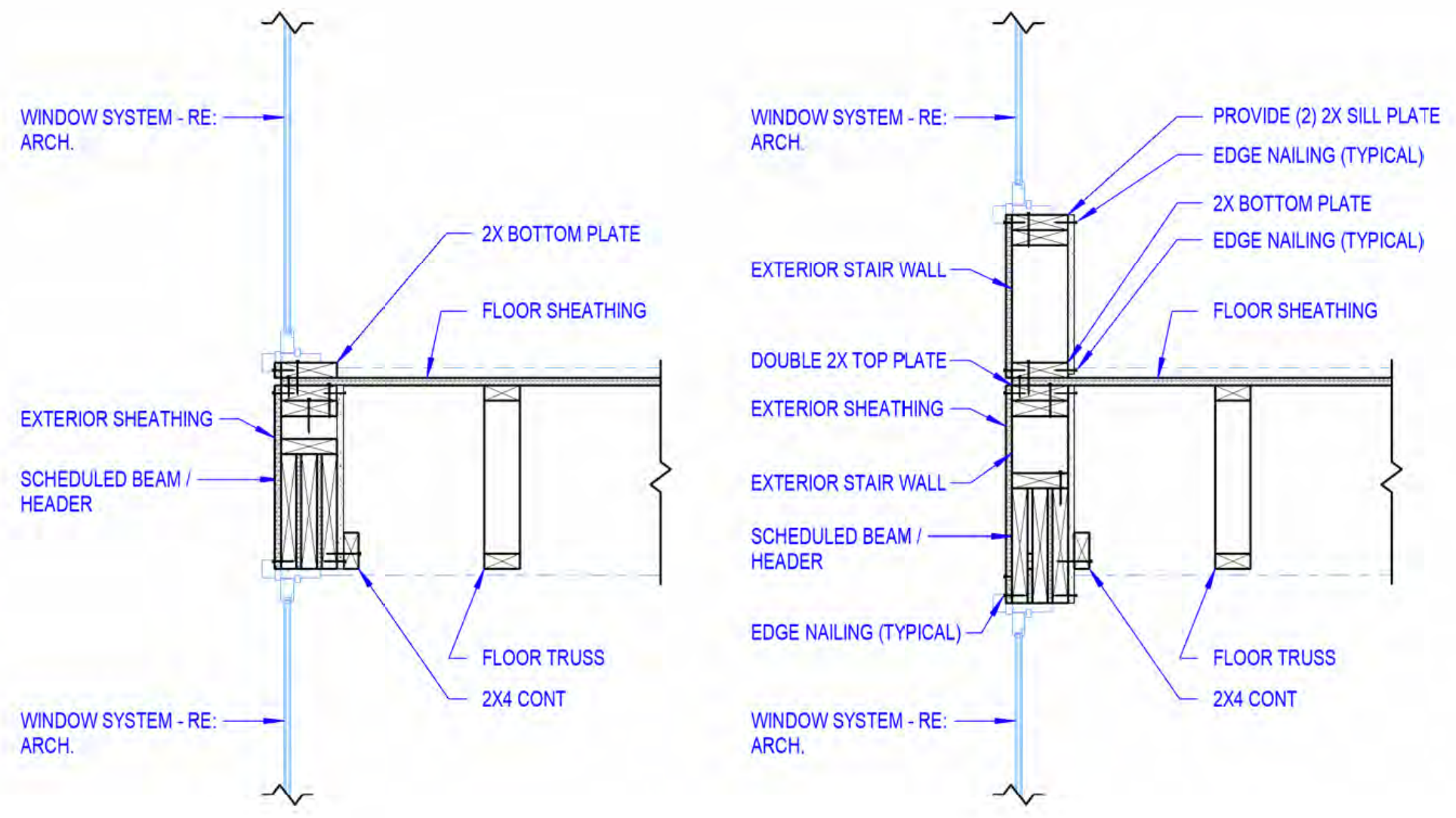
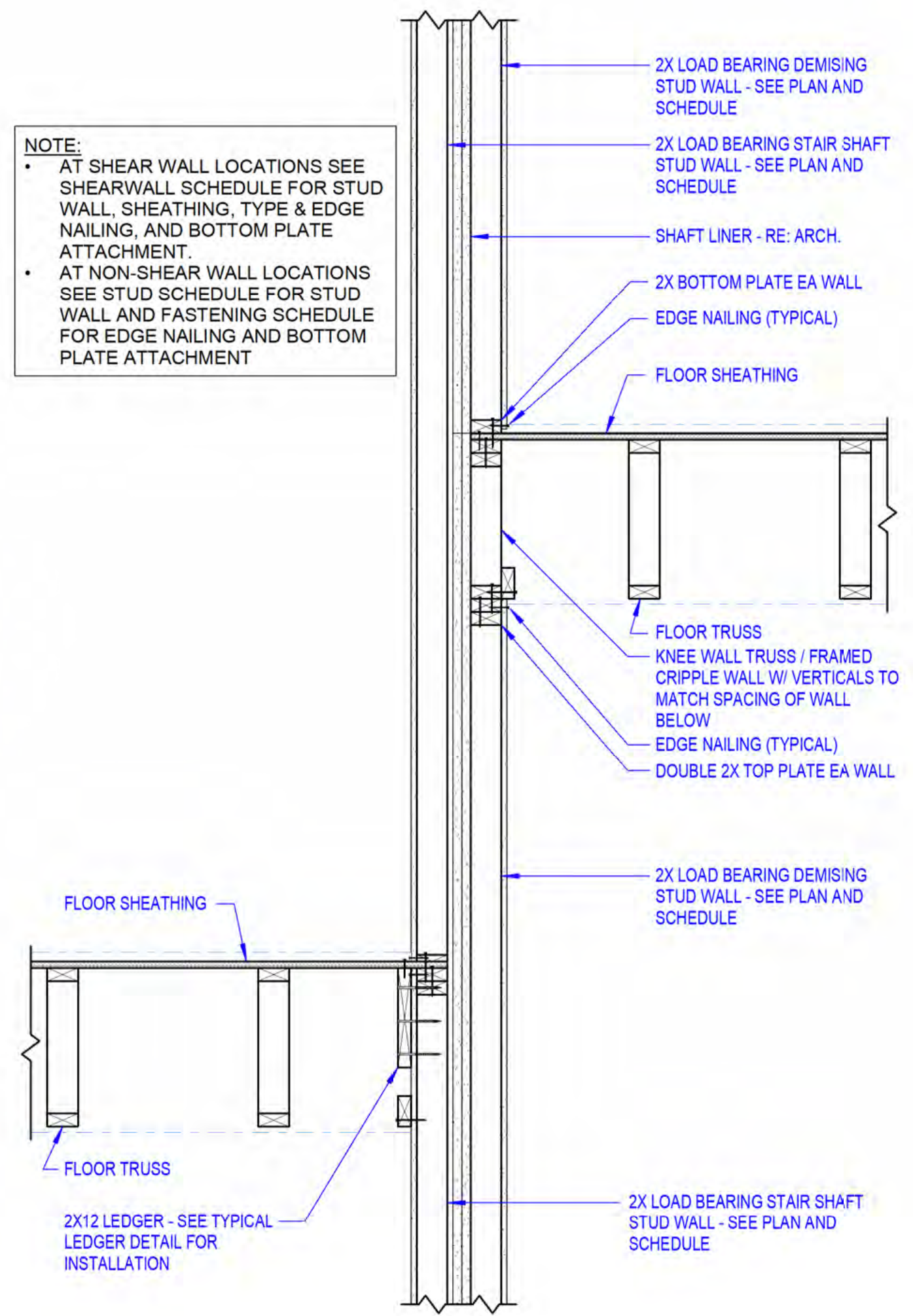

SHAFT LINER - RE: ARCH.
2X6 BLOCKING BETWEEN STUDS - COORDINATE LOCATIONS WITH STAIR / HANDRAIL MANUFACTURER
HANDRAIL POST
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
CONCRETE FILLED PAN TREAD - RE: STAIR MANUFACTRER
2X LOAD BEARING STAIR SHAFT STUD WALL - SEE PLAN AND SCHEDULE
STAIR STRINGER - RE: STAIR MANUFACTRER

11 Section at Stair Stringers
3/4" = 1'-0"



10 Stair Mid-Landing
3/4" = 1'-0"


NOTE:
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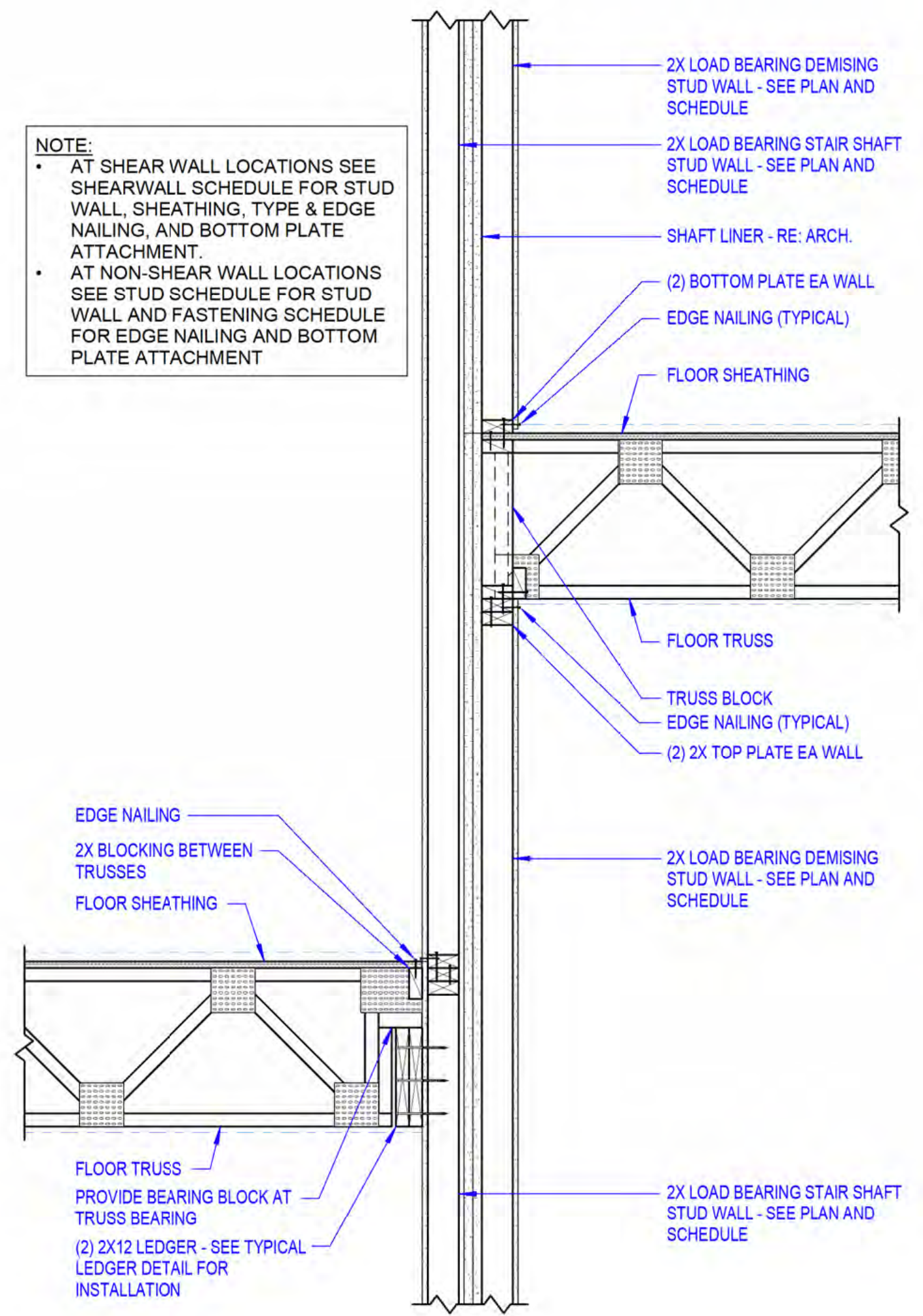
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

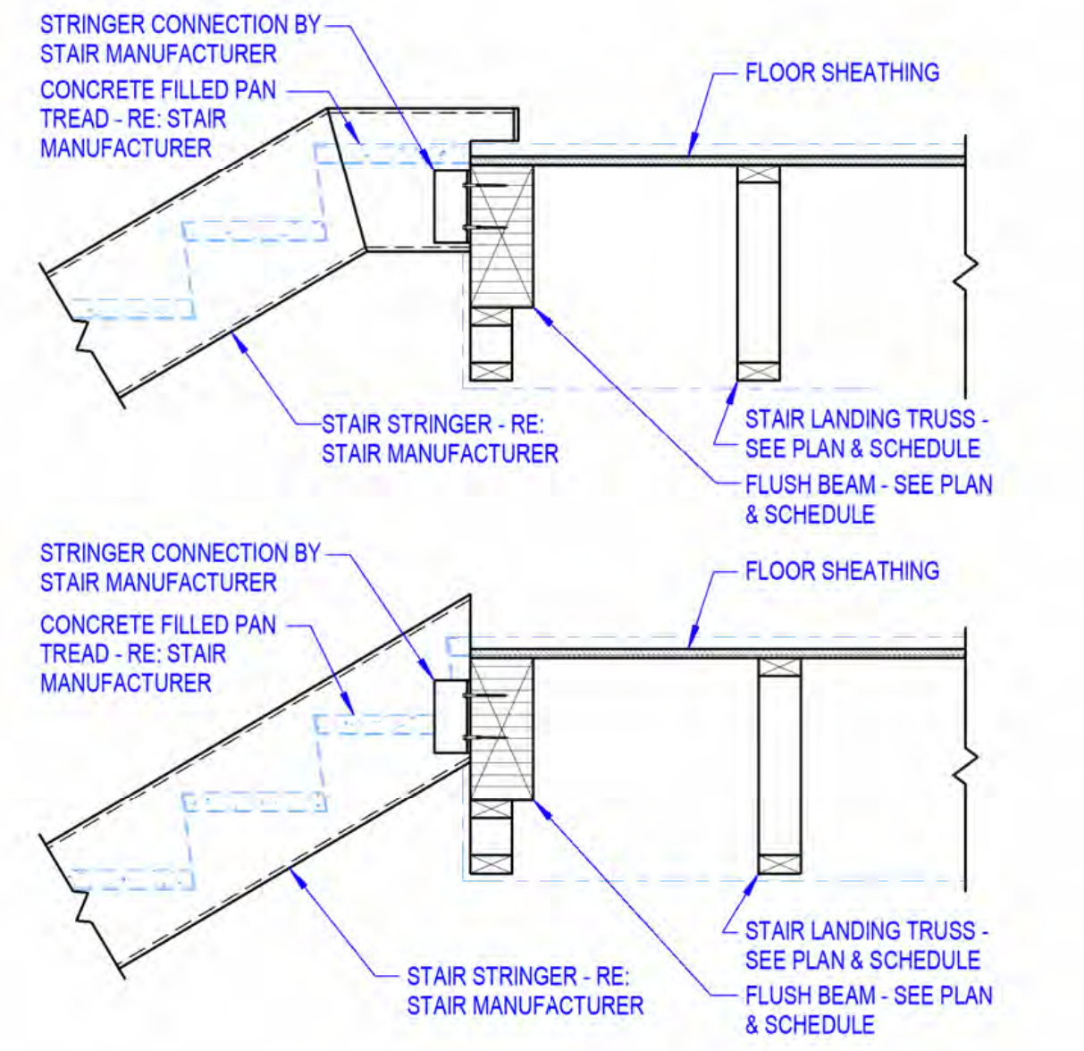
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
2X LOAD BEARING STAIR SHAFT STUD WALL - SEE PLAN AND SCHEDULE
SHAFT LINER - RE: ARCH.
2X BOTTOM PLATE EA WALL
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
FLOOR TRUSS
KNEE WALL TRUSS / FRAMED CRIPPLE WALL W/ VERTICALS TO MATCH SPACING OF WALL BELOW
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE EA WALL
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
FLOOR SHEATHING
FLOOR TRUSS
2X12 LEDGER - SEE TYPICAL LEDGER DETAIL FOR INSTALLATION
2X LOAD BEARING STAIR SHAFT STUD WALL - SEE PLAN AND SCHEDULE

7 Stair Mid-Landing
3/4" = 1'-0"

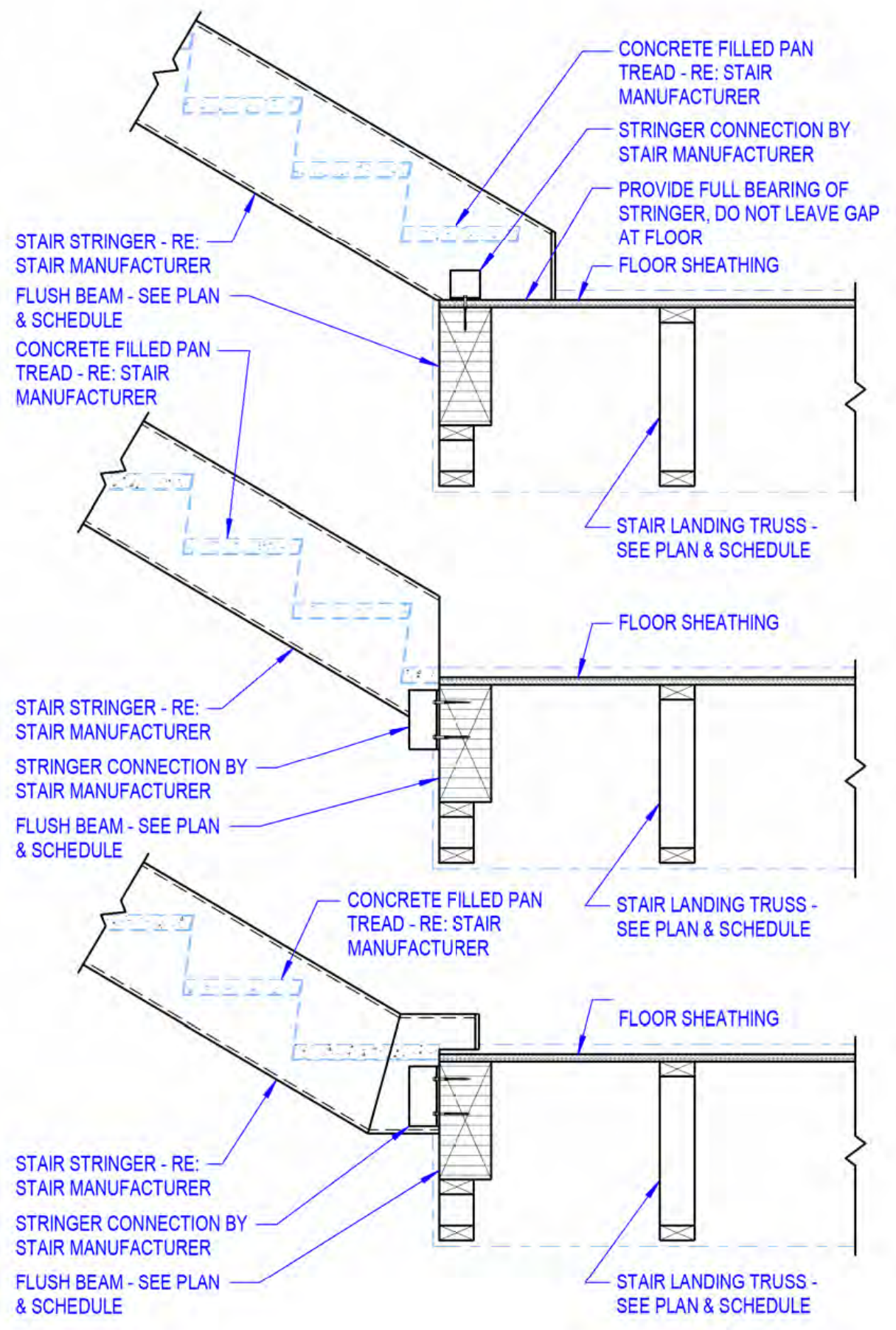

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
2X LOAD BEARING STAIR SHAFT STUD WALL - SEE PLAN AND SCHEDULE
SHAFT LINER - RE: ARCH.
(2) BOTTOM PLATE EA WALL
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
FLOOR TRUSS
TRUSS BLOCK
EDGE NAILING (TYPICAL)
(2) 2X TOP PLATE EA WALL
EDGE NAILING
2X BLOCKING BETWEEN TRUSSES
FLOOR SHEATHING
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
FLOOR TRUSS
PROVIDE BEARING BLOCK AT TRUSS BEARING
(2) 2X12 LEDGER - SEE TYPICAL LEDGER DETAIL FOR INSTALLATION
2X LOAD BEARING STAIR SHAFT STUD WALL - SEE PLAN AND SCHEDULE

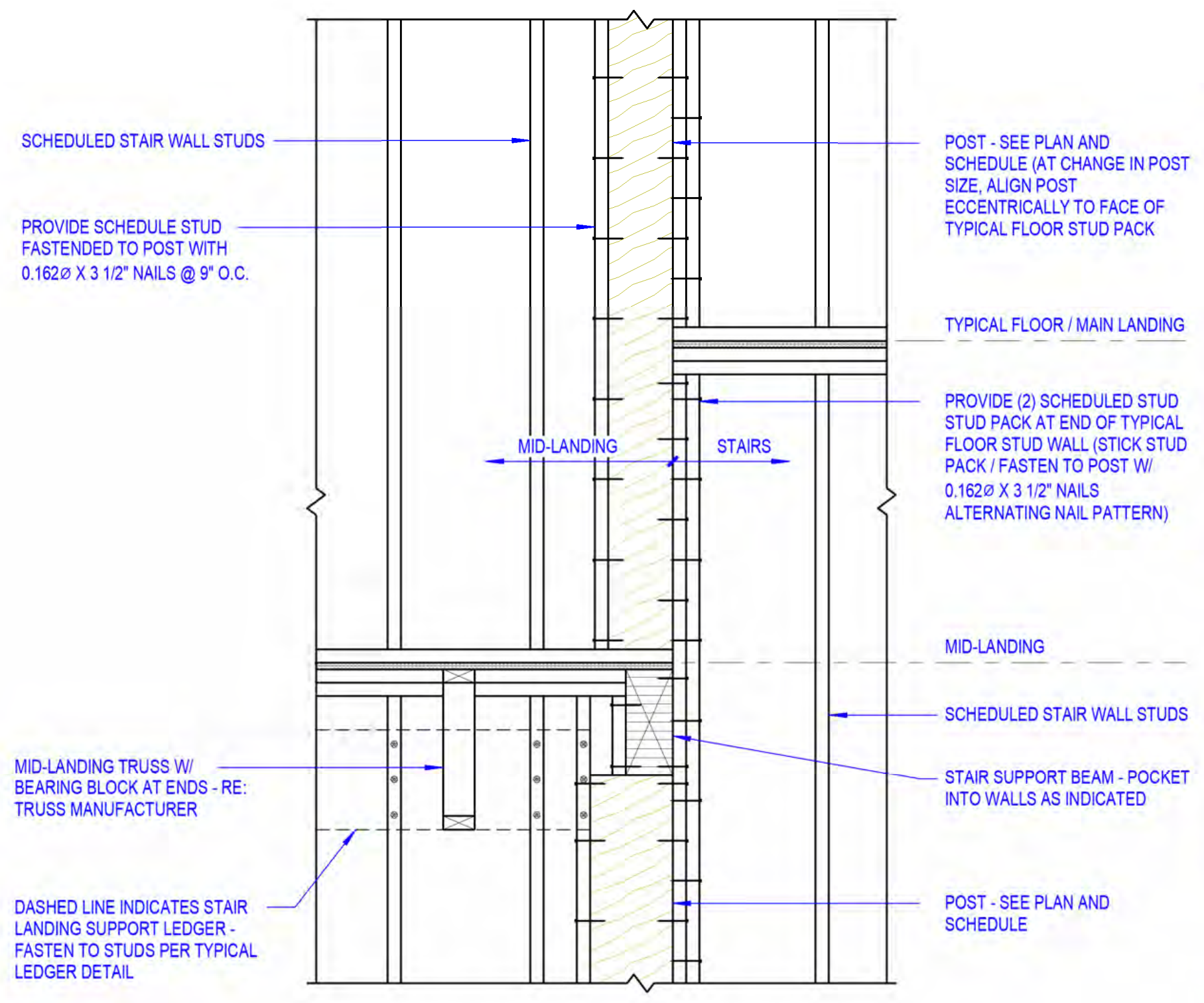
4 Stair Mid-Landing
3/4" = 1'-0"


STRINGER CONNECTION BY STAIR MANUFACTURER
CONCRETE FILLED PAN TREAD - RE: STAIR MANUFACTURER
FLOOR SHEATHING
STAIR STRINGER - RE: STAIR MANUFACTURER
STAIR LANDING TRUSS - SEE PLAN & SCHEDULE
FLUSH BEAM - SEE PLAN & SCHEDULE
STRINGER CONNECTION BY STAIR MANUFACTURER
CONCRETE FILLED PAN TREAD - RE: STAIR MANUFACTURER
FLOOR SHEATHING
STAIR STRINGER - RE: STAIR MANUFACTURER
STAIR LANDING TRUSS - SEE PLAN & SCHEDULE
FLUSH BEAM - SEE PLAN & SCHEDULE

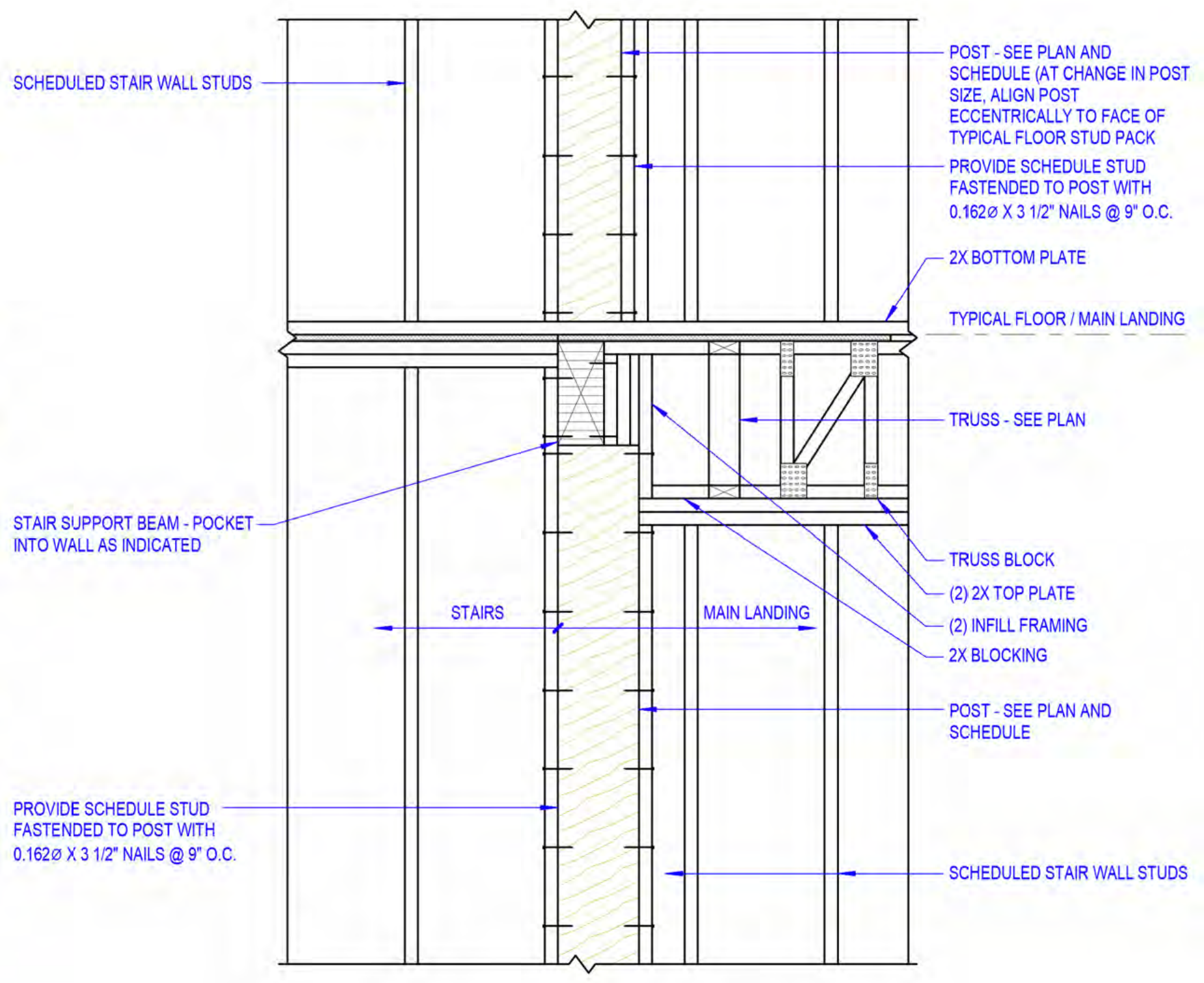
3 Steel Stair Stringer - Dn
3/4" = 1'-0"


CONCRETE FILLED PAN TREAD - RE: STAIR MANUFACTURER
STRINGER CONNECTION BY STAIR MANUFACTURER
PROVIDE FULL BEARING OF STRINGER, DO NOT LEAVE GAP AT FLOOR
FLOOR SHEATHING
STAIR STRINGER - RE: STAIR MANUFACTURER
FLUSH BEAM - SEE PLAN & SCHEDULE
CONCRETE FILLED PAN TREAD - RE: STAIR MANUFACTURER
STAIR LANDING TRUSS - SEE PLAN & SCHEDULE
FLOOR SHEATHING
STAIR STRINGER - RE: STAIR MANUFACTURER
STRINGER CONNECTION BY STAIR MANUFACTURER
FLUSH BEAM - SEE PLAN & SCHEDULE
STAIR LANDING TRUSS - SEE PLAN & SCHEDULE
CONCRETE FILLED PAN TREAD - RE: STAIR MANUFACTURER
FLOOR SHEATHING
STAIR STRINGER - RE: STAIR MANUFACTURER
STRINGER CONNECTION BY STAIR MANUFACTURER
FLUSH BEAM - SEE PLAN & SCHEDULE
STAIR LANDING TRUSS - SEE PLAN & SCHEDULE

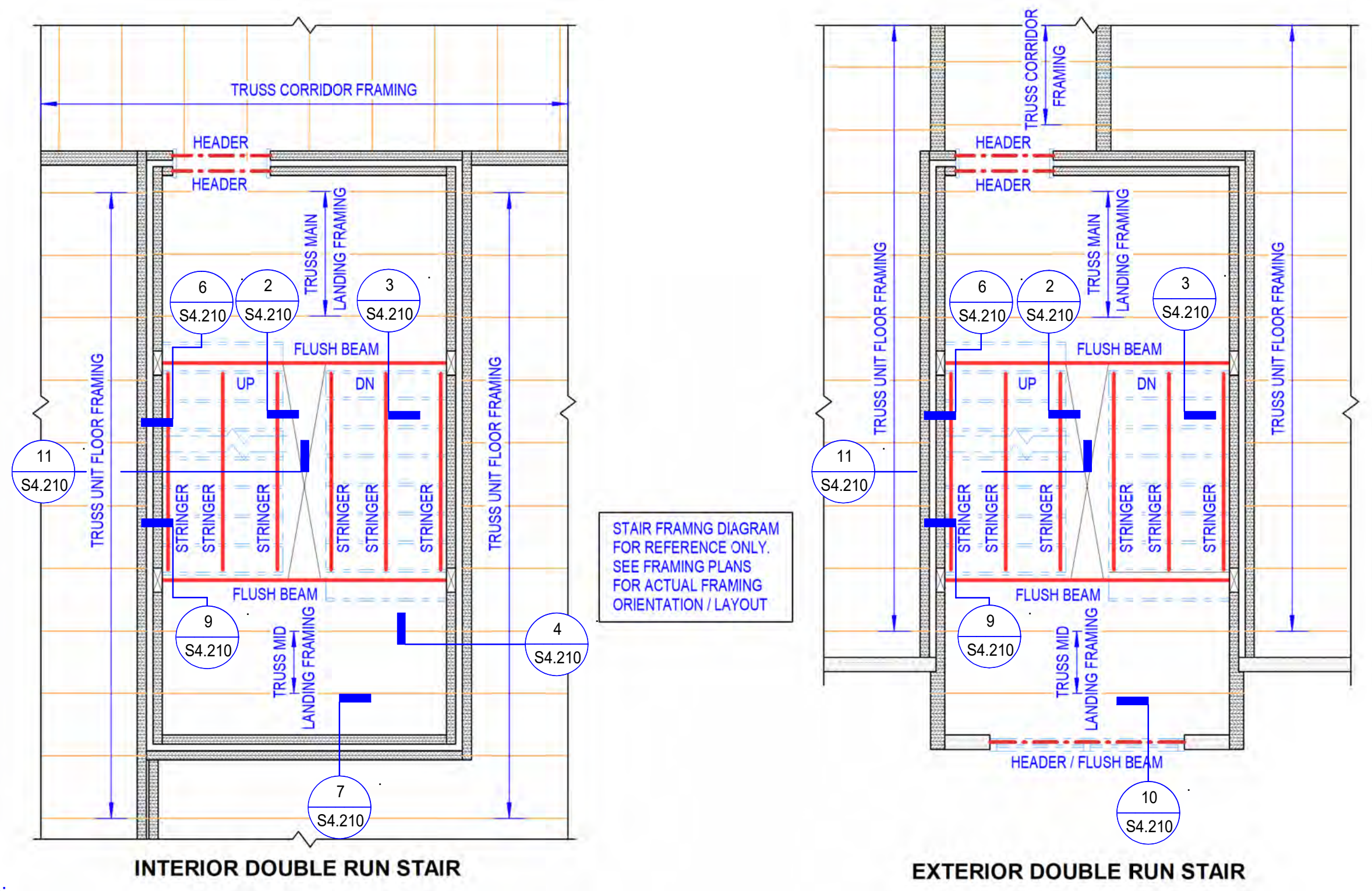
2 Steel Stair Stringer - Up
3/4" = 1'-0"


SCHEDULED STAIR WALL STUDS
PROVIDE SCHEDULE STUD FASTENDED TO POST WITH 0.162Ø X 3 1/2" NAILS @ 9" O.C.
POST - SEE PLAN AND SCHEDULE (AT CHANGE IN POST SIZE, ALIGN POST ECCENTRICALLY TO FACE OF TYPICAL FLOOR STUD PACK
TYPICAL FLOOR / MAIN LANDING
PROVIDE (2) SCHEDULED STUD STUD PACK AT END OF TYPICAL FLOOR STUD WALL (STICK STUD PACK / FASTEN TO POST W/ 0.162Ø X 3 1/2" NAILS ALTERNATING NAIL PATTERN)
MID-LANDING
STAIRS
MID-LANDING
SCHEDULED STAIR WALL STUDS
MID-LANDING TRUSS W/ BEARING BLOCK AT ENDS - RE: TRUSS MANUFACTURER
STAIR SUPPORT BEAM - POCKET INTO WALLS AS INDICATED
DASHED LINE INDICATES STAIR LANDING SUPPORT LEDGER - FASTEN TO STUDS PER TYPICAL LEDGER DETAIL
POST - SEE PLAN AND SCHEDULE

9 Stair Landing Beam Supprt / Plate Offset
3/4" = 1'-0"


SCHEDULED STAIR WALL STUDS
POST - SEE PLAN AND SCHEDULE (AT CHANGE IN POST SIZE, ALIGN POST ECCENTRICALLY TO FACE OF TYPICAL FLOOR STUD PACK
PROVIDE SCHEDULE STUD FASTENDED TO POST WITH 0.162Ø X 3 1/2" NAILS @ 9" O.C.
2X BOTTOM PLATE
TYPICAL FLOOR / MAIN LANDING
TRUSS - SEE PLAN
STAIR SUPPORT BEAM - POCKET INTO WALL AS INDICATED
TRUSS BLOCK
(2) 2X TOP PLATE
STAIRS
MAIN LANDING
(2) INFILL FRAMING
2X BLOCKING
POST - SEE PLAN AND SCHEDULE
PROVIDE SCHEDULE STUD FASTENDED TO POST WITH 0.162Ø X 3 1/2" NAILS @ 9" O.C.
SCHEDULED STAIR WALL STUDS

6 Stair Landing Beam Supprt
3/4" = 1'-0"



STAIR FRAMNG DIAGRAM FOR REFERENCE ONLY. SEE FRAMING PLANS FOR ACTUAL FRAMING ORIENTATION / LAYOUT

1 Stair Framing Diagram
1/4" = 1'-0"

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VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
Stair Framing

SHEET NUMBER:
S4.210

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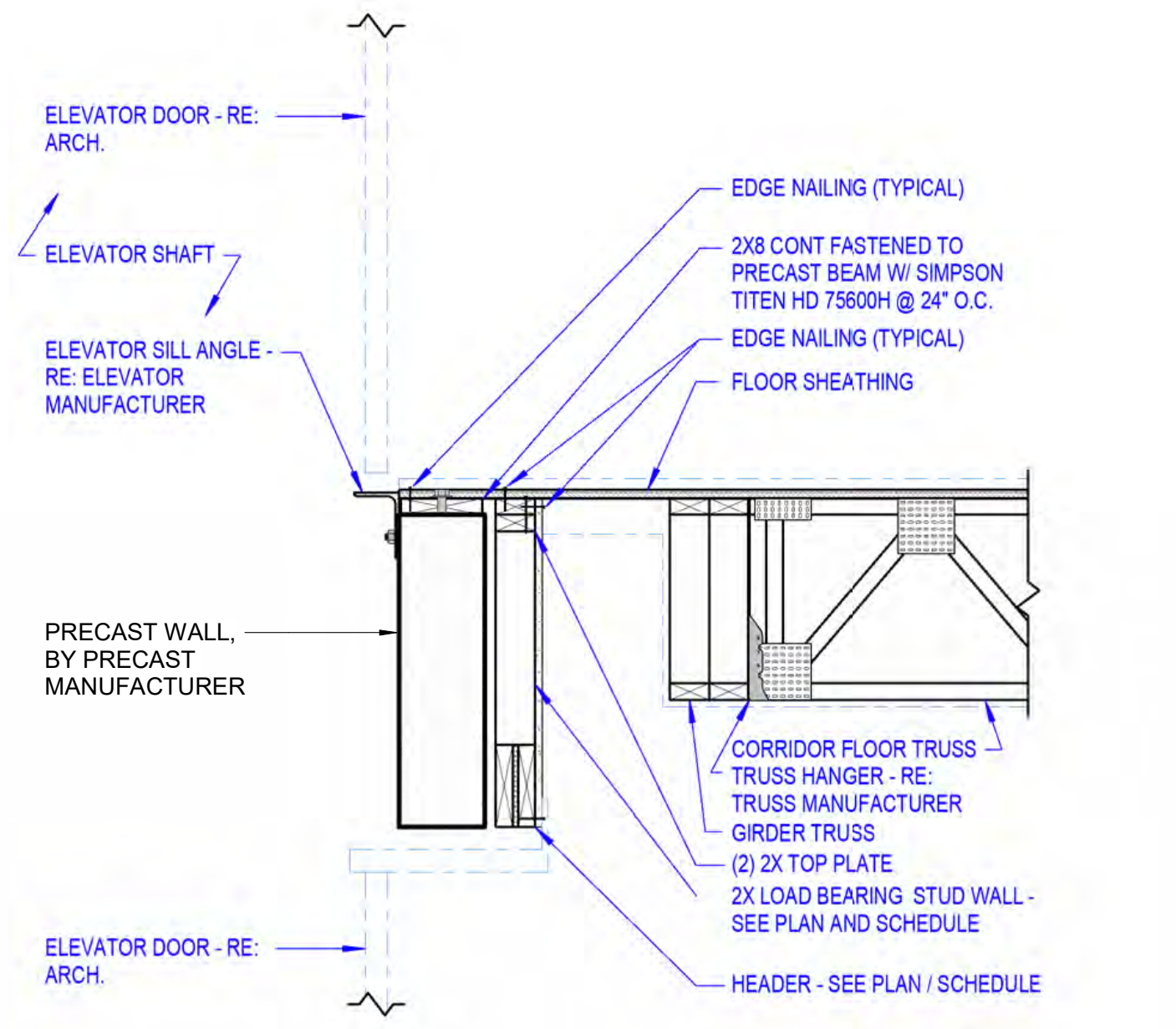
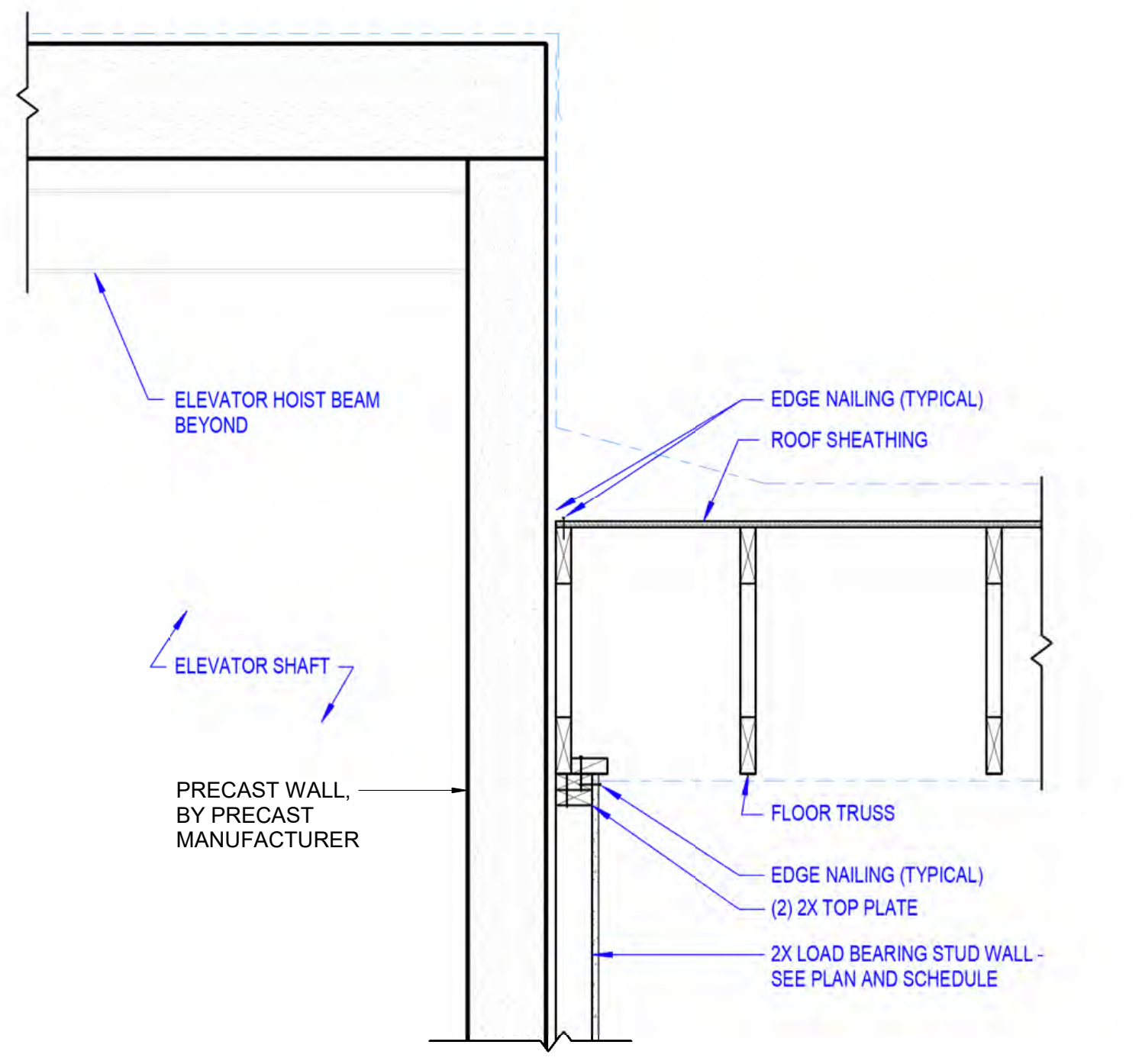
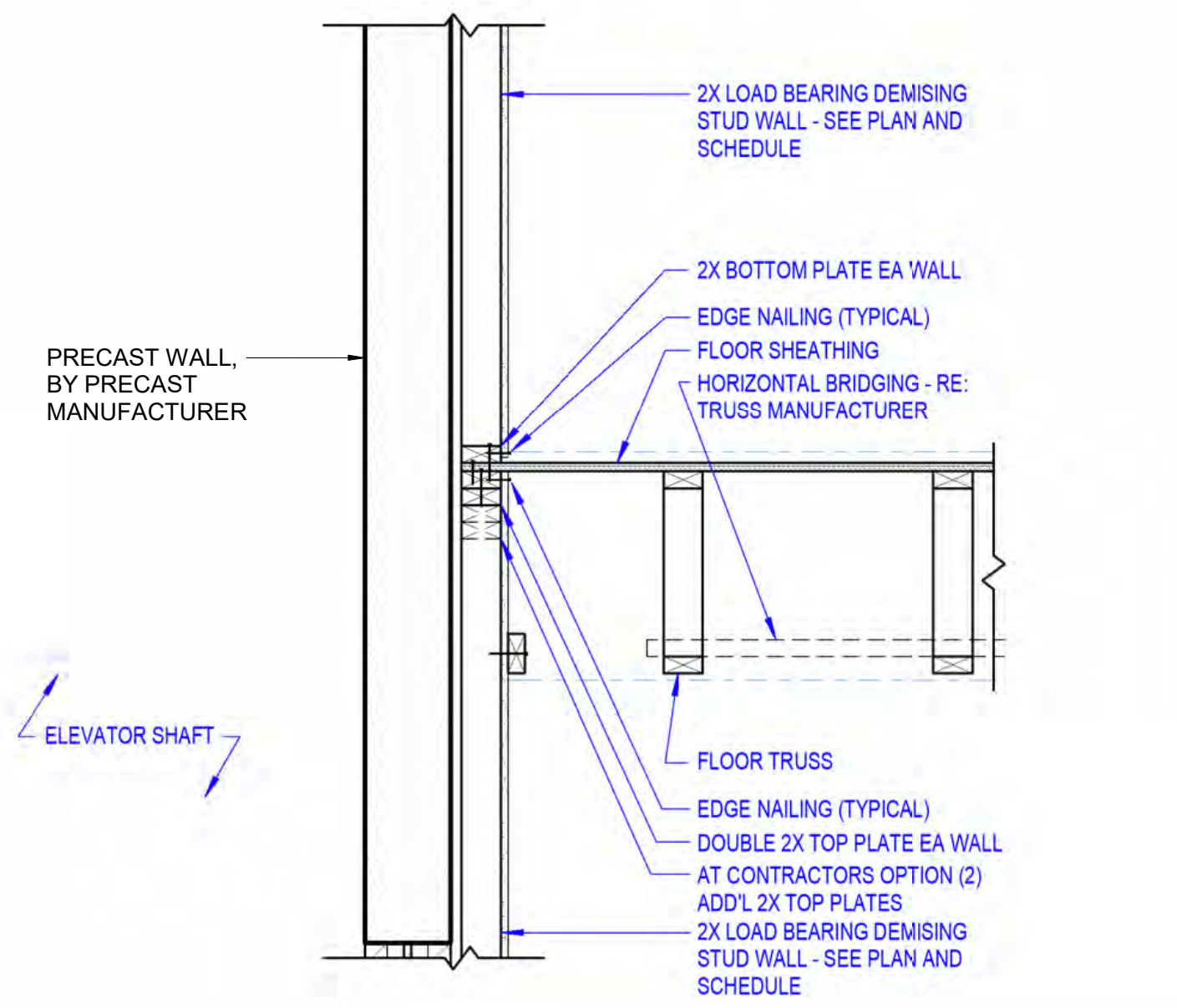

ELEVATOR DOOR - RE: ARCH.
ELEVATOR SHAFT
ELEVATOR SILL ANGLE - RE: ELEVATOR MANUFACTURER
PRECAST WALL, BY PRECAST MANUFACTURER
ELEVATOR DOOR - RE: ARCH.
EDGE NAILING (TYPICAL)
2X8 CONT FASTENED TO PRECAST BEAM W/ SIMPSON TITEN HD 75600H @ 24" O.C.
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
CORRIDOR FLOOR TRUSS
TRUSS HANGER - RE: TRUSS MANUFACTURER
GIRDER TRUSS
(2) 2X TOP PLATE
2X LOAD BEARING STUD WALL - SEE PLAN AND SCHEDULE
HEADER - SEE PLAN / SCHEDULE

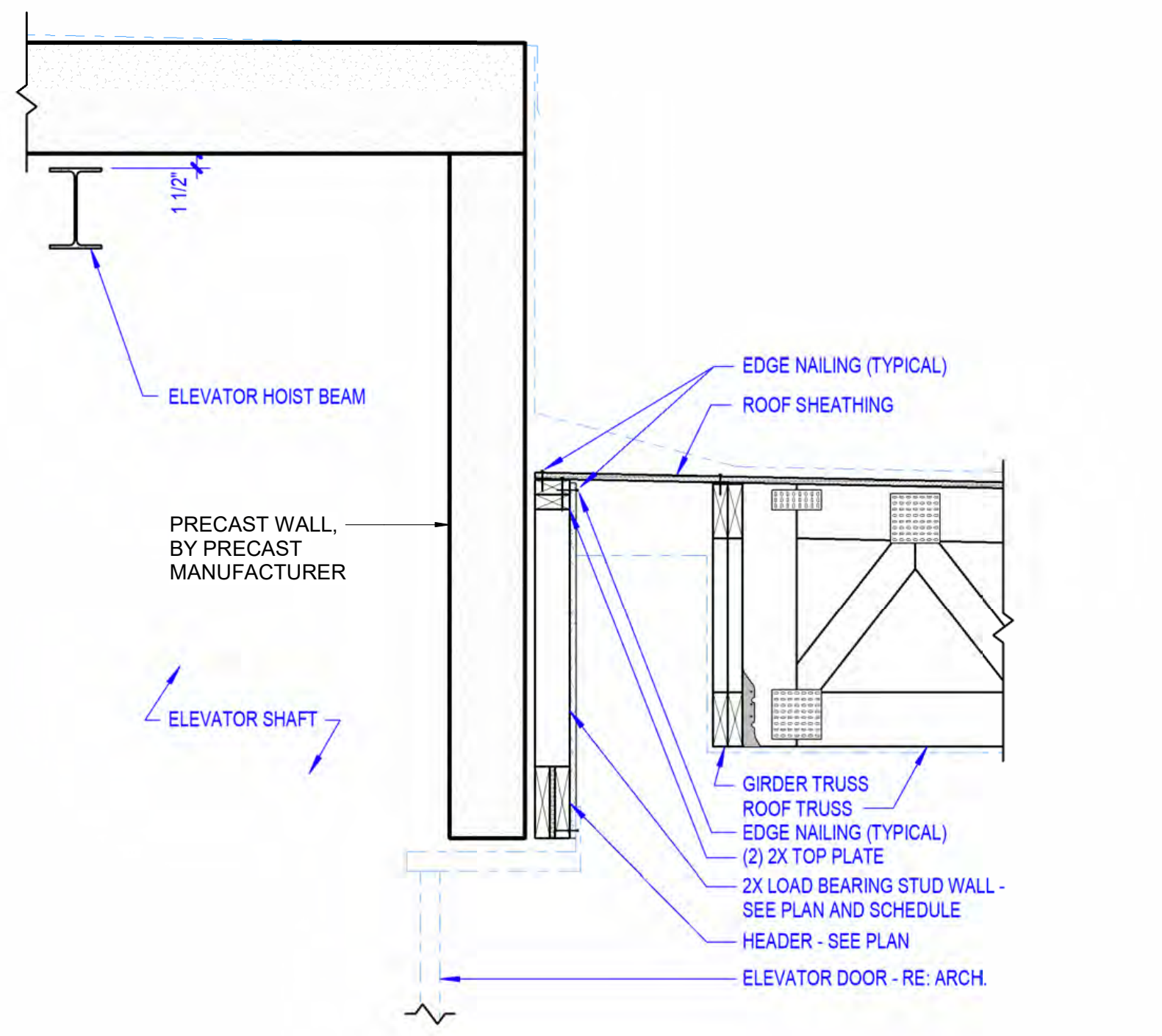
4 Detail at Elevator Sill
3/4" = 1'-0"


ELEVATOR HOIST BEAM BEYOND
ELEVATOR SHAFT
PRECAST WALL, BY PRECAST MANUFACTURER
EDGE NAILING (TYPICAL)
ROOF SHEATHING
FLOOR TRUSS
EDGE NAILING (TYPICAL)
(2) 2X TOP PLATE
2X LOAD BEARING STUD WALL - SEE PLAN AND SCHEDULE

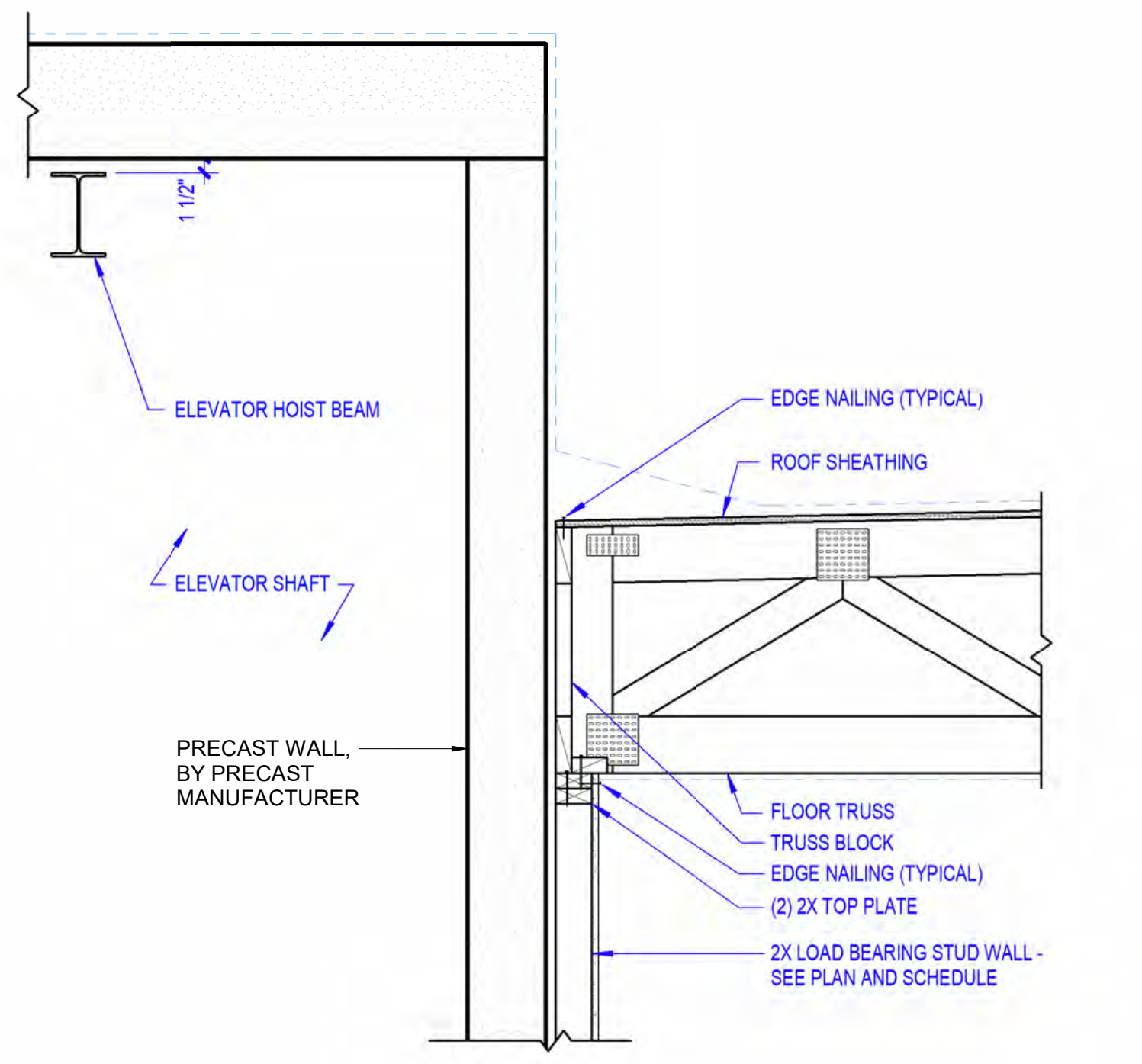
6 Truss Bearing at Elevator Shaft - CMU
3/4" = 1'-0"


2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
2X BOTTOM PLATE EA WALL
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
PRECAST WALL, BY PRECAST MANUFACTURER
ELEVATOR SHAFT
FLOOR TRUSS
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE EA WALL
AT CONTRACTORS OPTION (2) ADD'L 2X TOP PLATES
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE

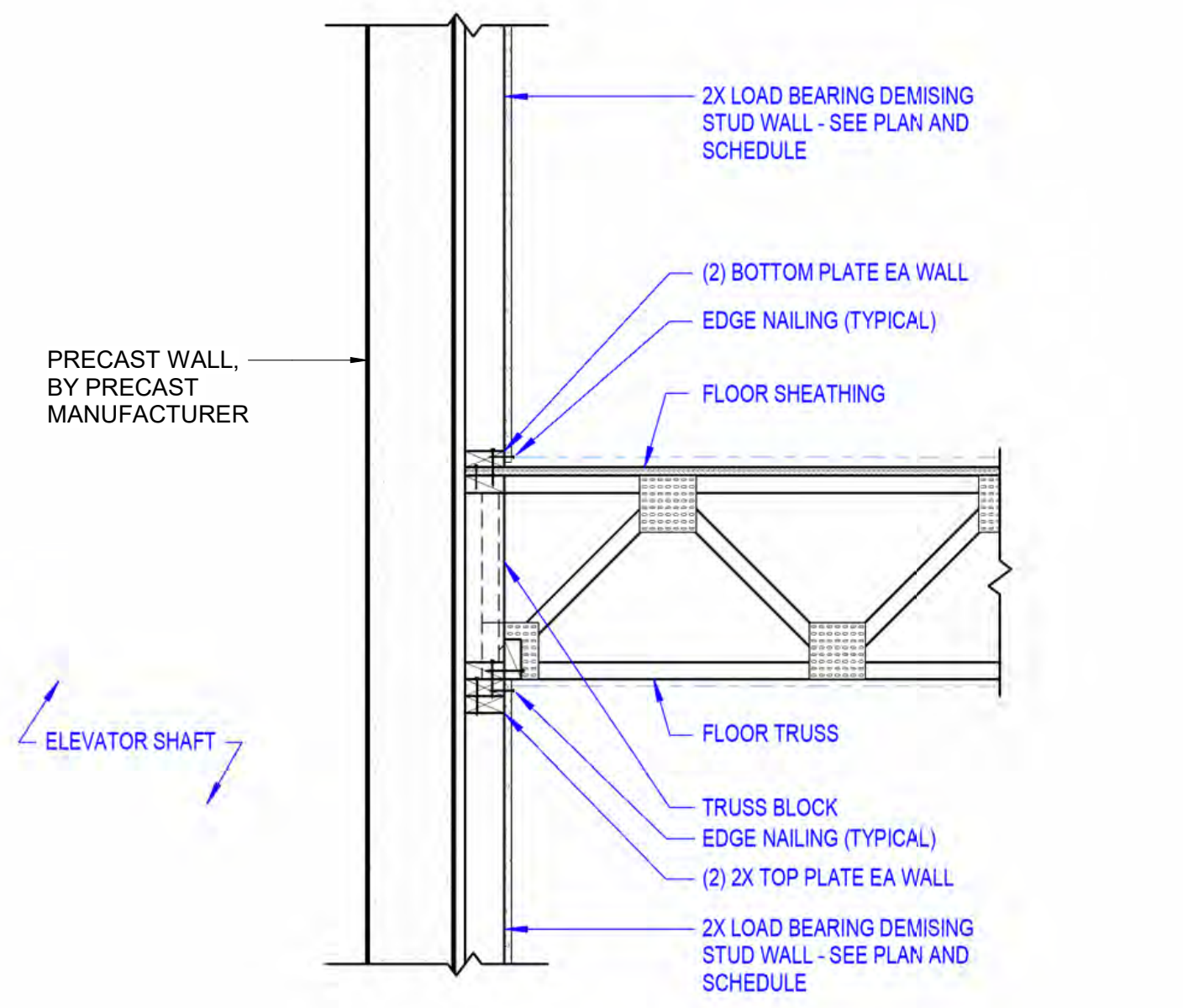
3 Truss Bearing at Elevator Shaft-Double Wall
3/4" = 1'-0"



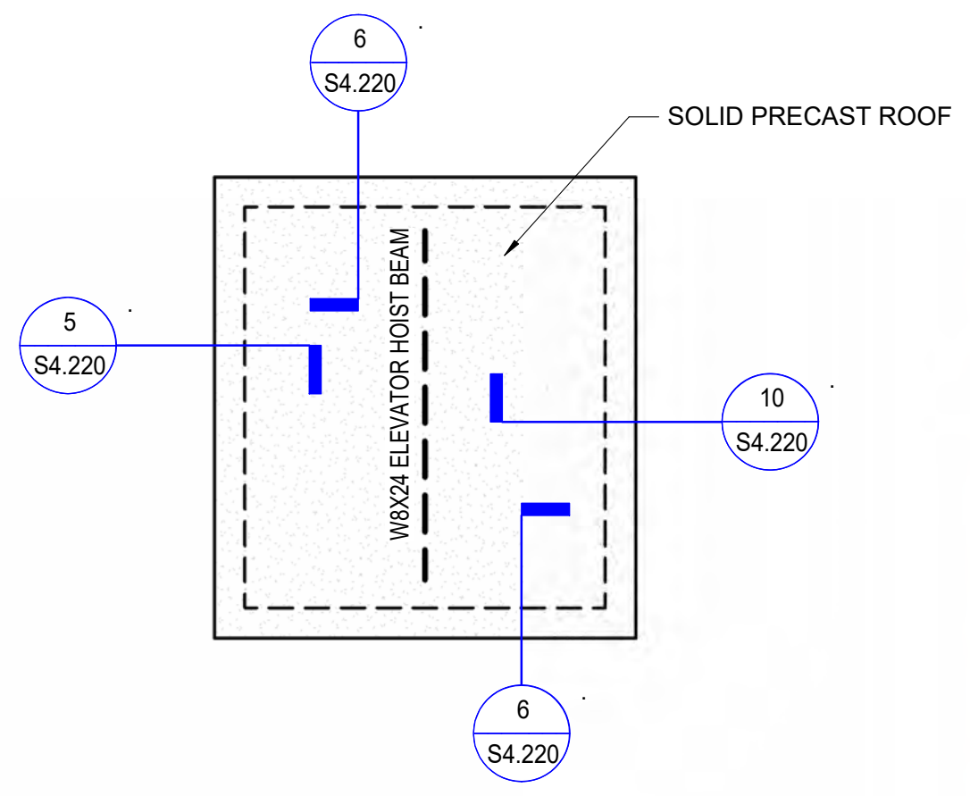
10 Truss Bearing at Elevator Shaft - CMU
3/4" = 1'-0"



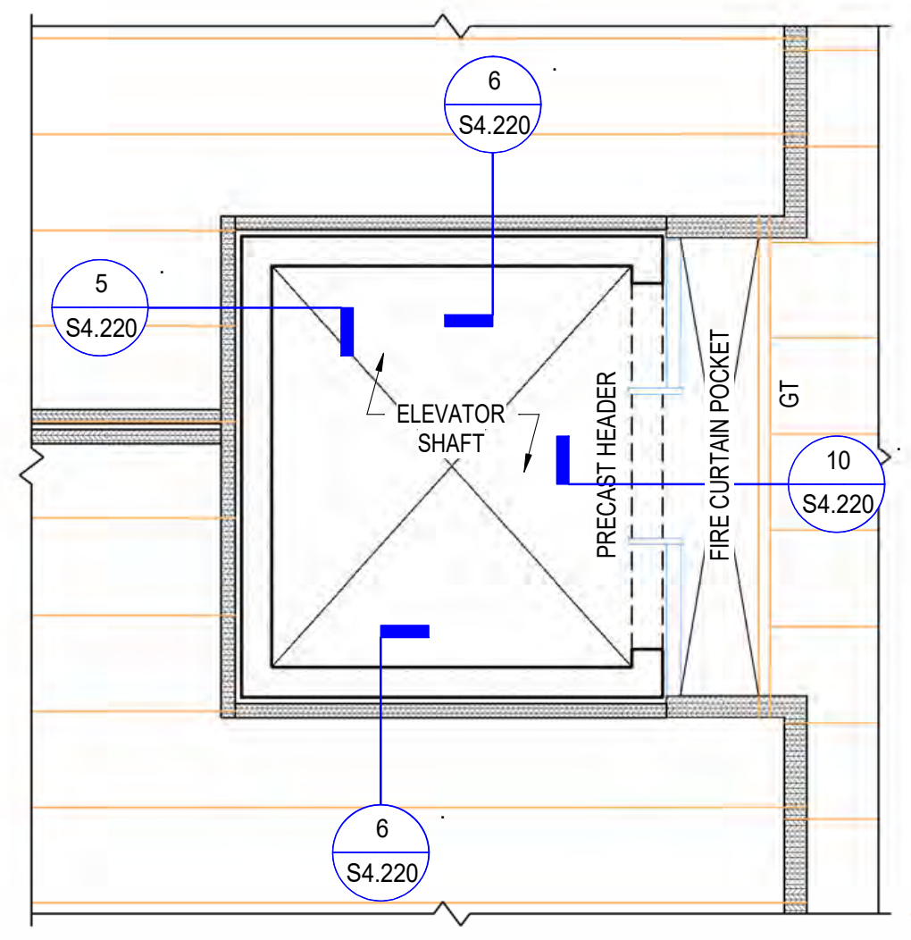
5 Truss Bearing at Elevator Shaft - CMU
3/4" = 1'-0"


2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
(2) BOTTOM PLATE EA WALL
EDGE NAILING (TYPICAL)
FLOOR SHEATHING
PRECAST WALL, BY PRECAST MANUFACTURER
ELEVATOR SHAFT
FLOOR TRUSS
TRUSS BLOCK
EDGE NAILING (TYPICAL)
(2) 2X TOP PLATE EA WALL
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE

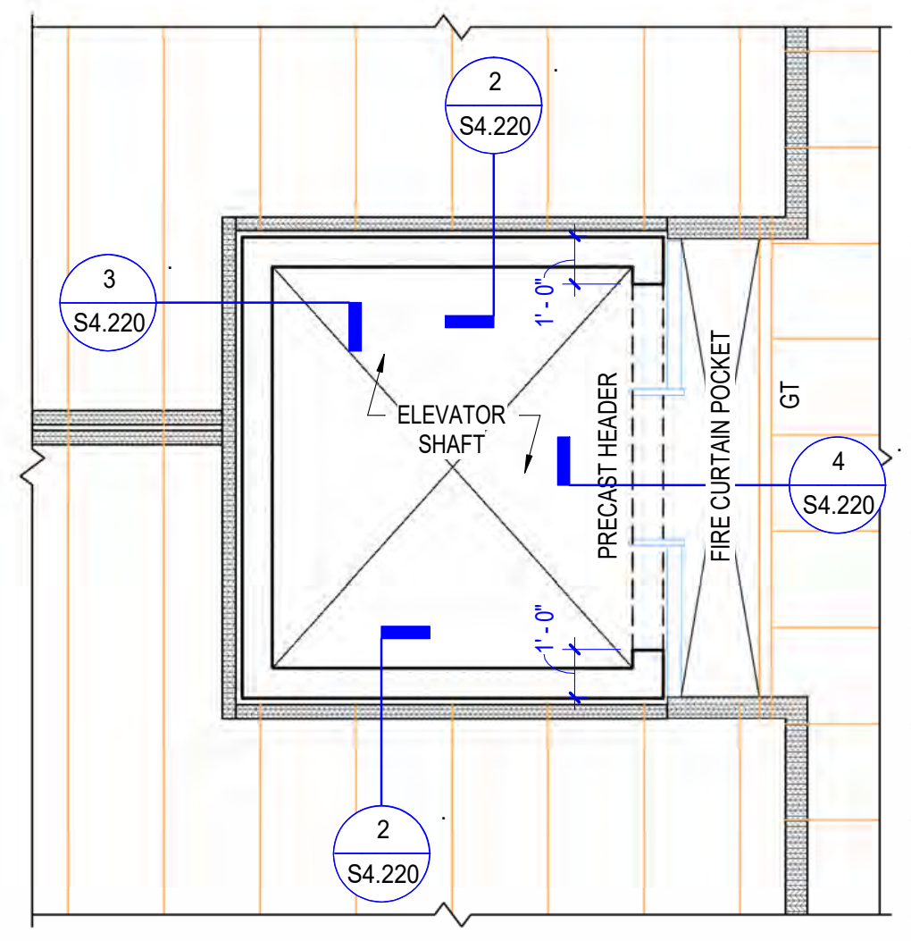
2 Truss Bearing at Elevator Shaft-Double Wall
3/4" = 1'-0"



SINGLE ELEVATOR MACHINE OVER-RUN ROOF FRAMING

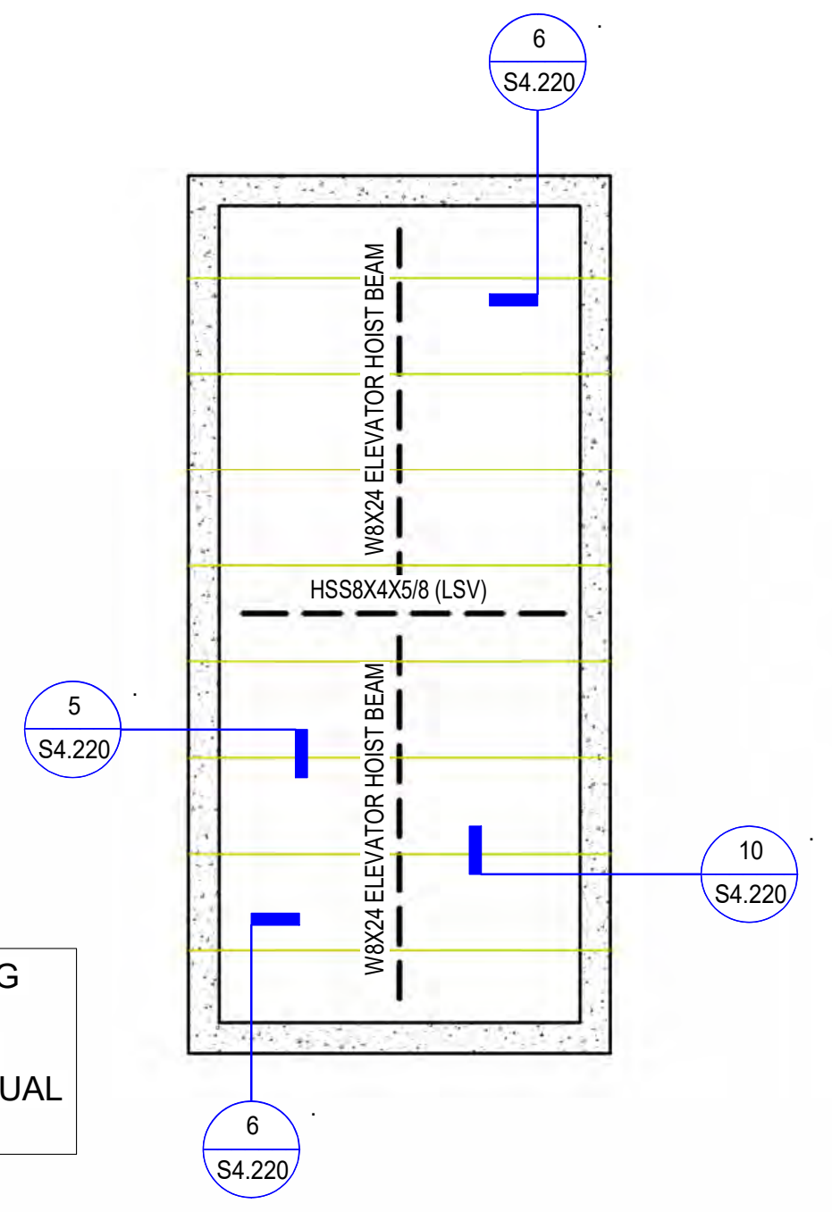


SINGLE ELEVATOR ROOF FRAMING

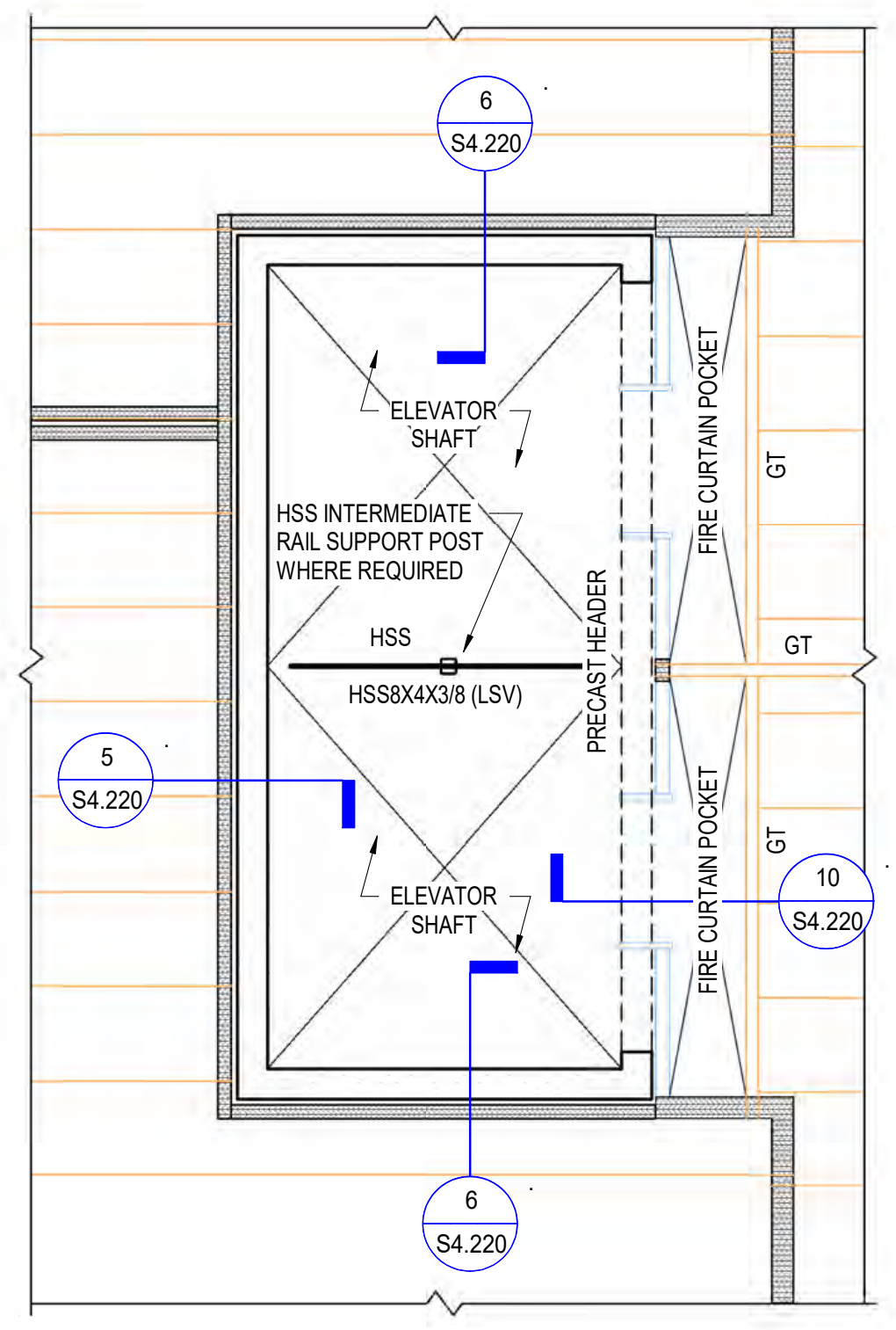


SINGLE ELEVATOR FLOOR FRAMING

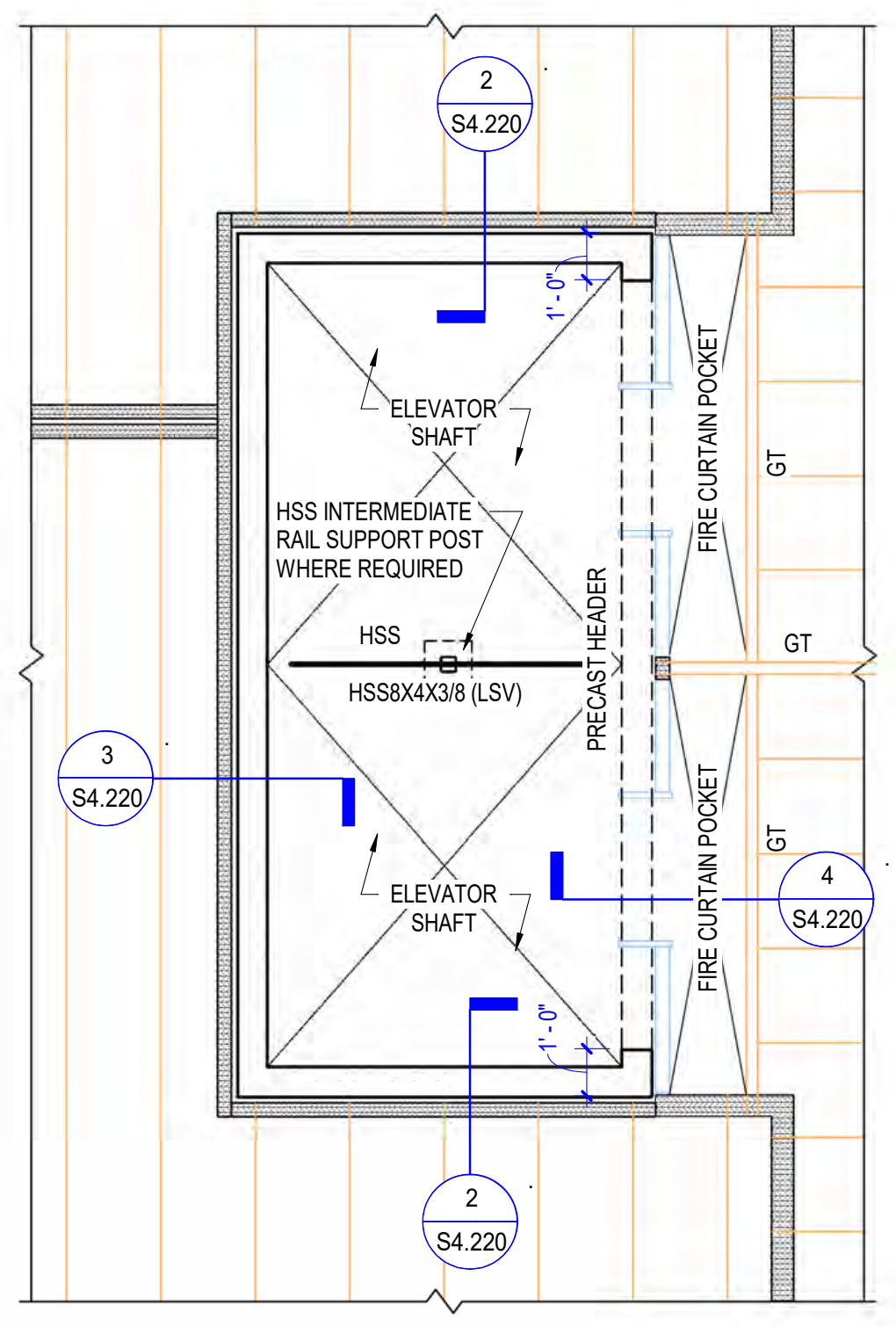
ELEVATOR FRAMING SHOWN IS FOR REFERENCE ONLY. SEE PLAN FOR ACTUAL FRAMING LAYOUT.



DOUBLE ELEVATOR MACHINE OVER-RUN ROOF FRAMING



DOUBLE ELEVATOR ROOF FRAMING



DOUBLE ELEVATOR FLOOR FRAMING

1 Truss Parrallel to Elevator Shaft-Double Wall
1/4" = 1'-0"

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VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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BATON ROUGE, LOUISIANA 70808
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SHEET TITLE:
Elevator Framing

SHEET NUMBER:
S4.220

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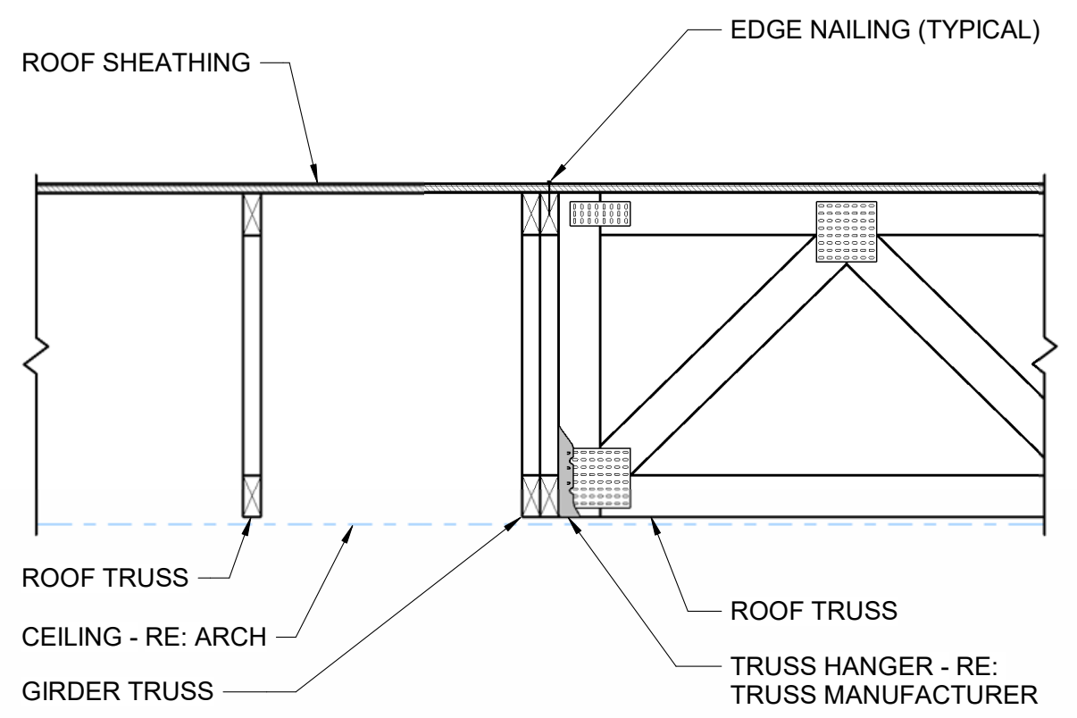
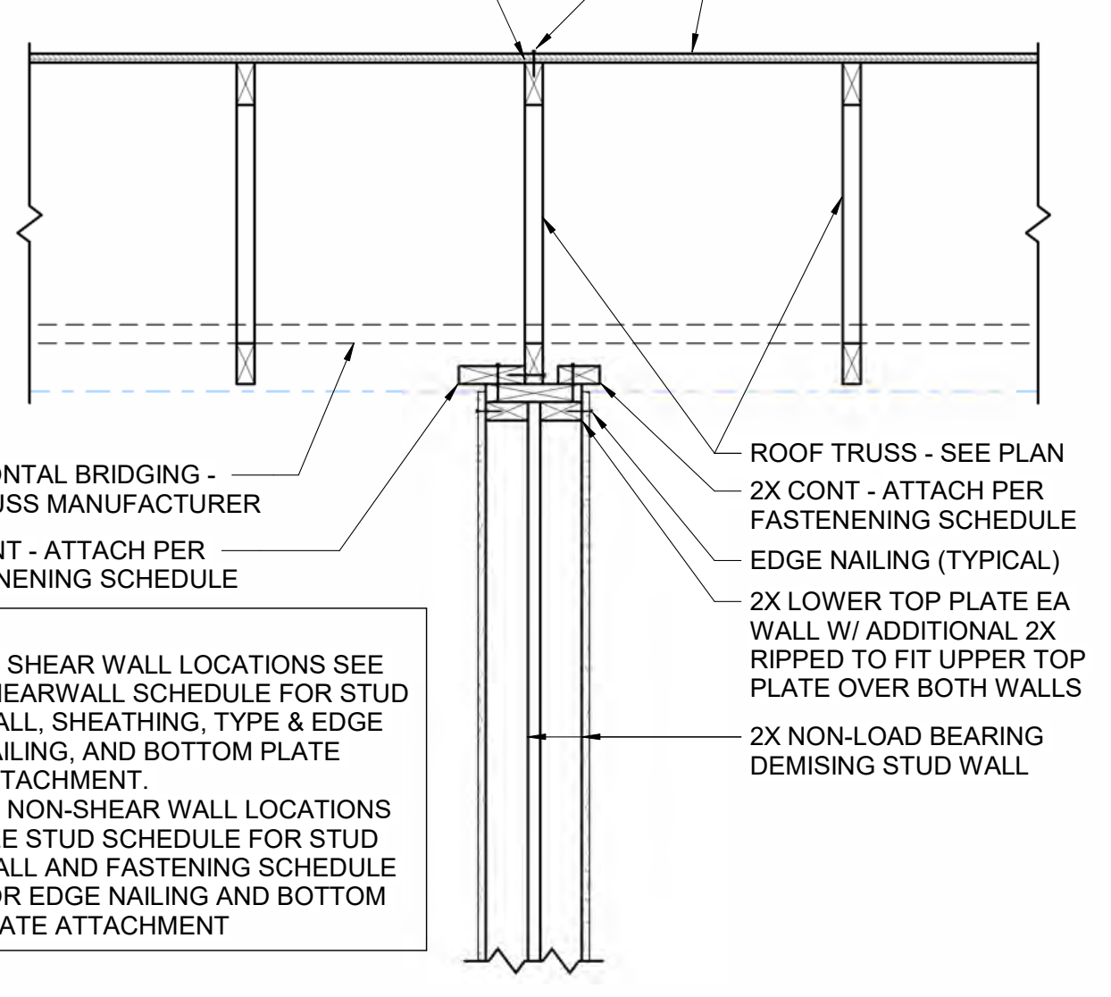


14 Truss Bearing at Girder Truss - Both Side
3/4" = 1'-0"

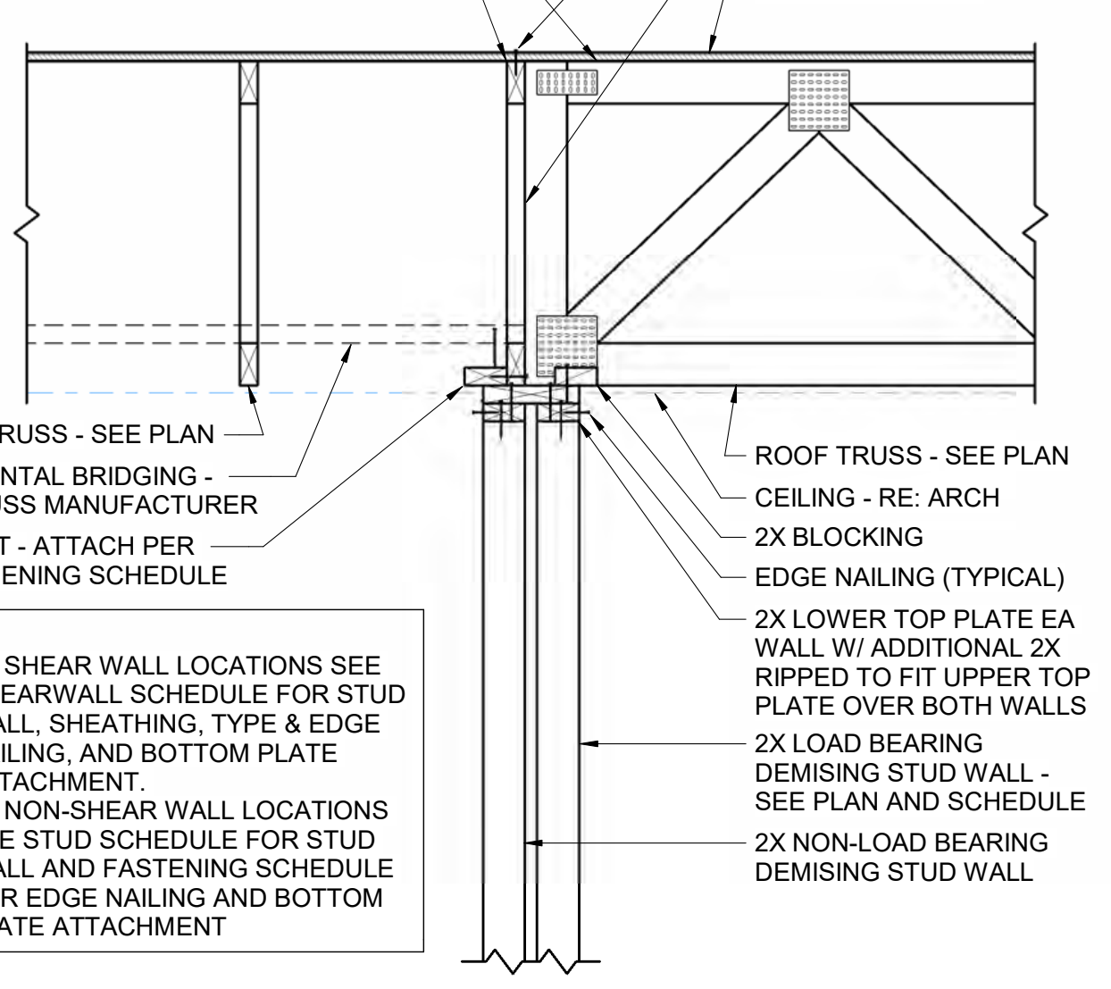
ROOF TRUSSES / TRUSS BLOCKS LOCATED ABOVE SHEAR WALLS SHALL BE DESIGNED FOR DRAG FORCE (D.F.) - SEE ROOF PLAN FOR REQUIRED LOADS. TRUSSES WITHOUT DESIGNATED LOADS SHALL BE DESIGNED FOR 400 PLF


EDGE NAILING (TYPICAL)
ROOF SHEATHING
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
2X CONT - ATTACH PER FASTENENING SCHEDULE
ROOF TRUSS - SEE PLAN
2X CONT - ATTACH PER FASTENENING SCHEDULE
EDGE NAILING (TYPICAL)
2X LOWER TOP PLATE EA WALL W/ ADDITIONAL 2X RIPPED TO FIT UPPER TOP PLATE OVER BOTH WALLS
2X NON-LOAD BEARING DEMISING STUD WALL

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

13 Truss at Partywall - Parallel
3/4" = 1'-0"

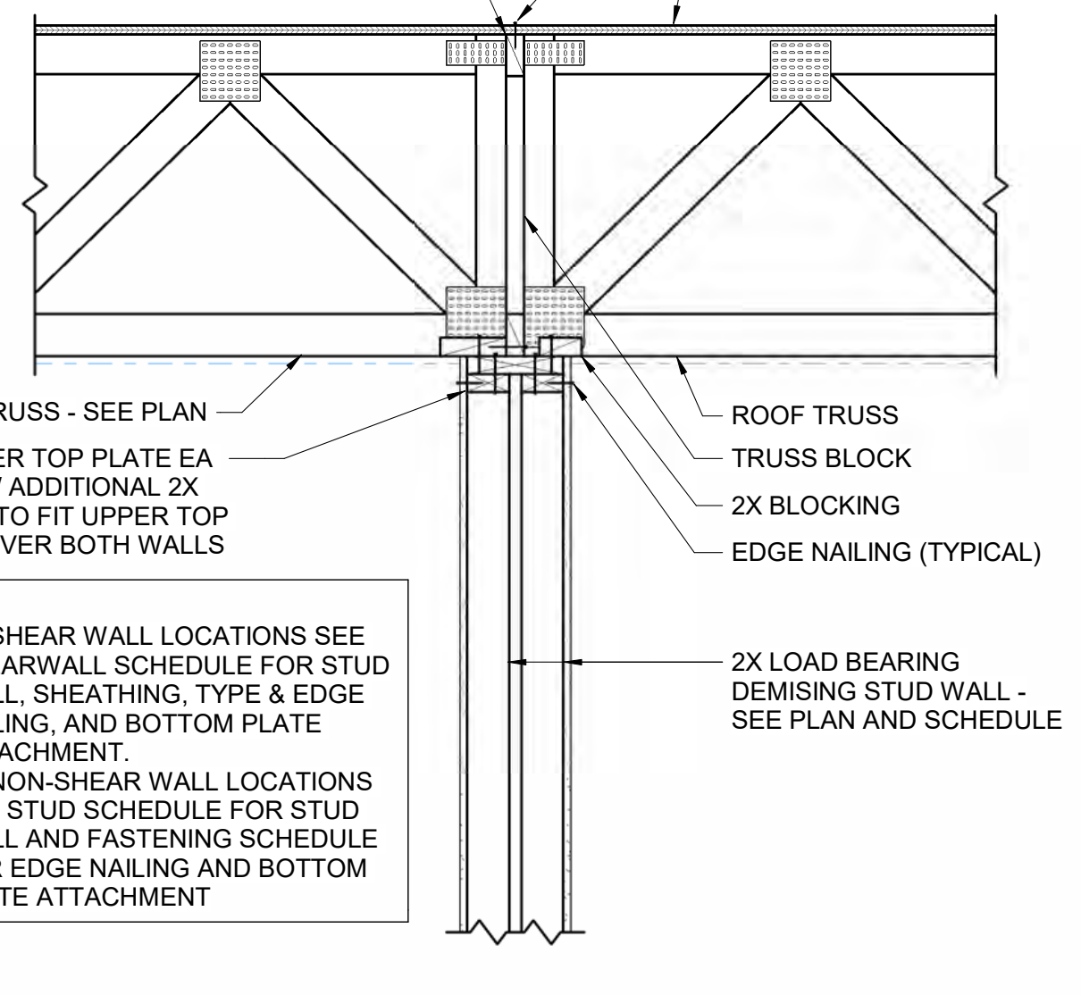
ROOF TRUSSES / TRUSS BLOCKS LOCATED ABOVE SHEAR WALLS SHALL BE DESIGNED FOR DRAG FORCE (D.F.) - SEE ROOF PLAN FOR REQUIRED LOADS. TRUSSES WITHOUT DESIGNATED LOADS SHALL BE DESIGNED FOR 400 PLF


EDGE NAILING (TYPICAL)
ROOF TRUSS - SEE PLAN
ROOF SHEATHING
ROOF TRUSS - SEE PLAN
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
2X CONT - ATTACH PER FASTENENING SCHEDULE
ROOF TRUSS - SEE PLAN
CEILING - RE: ARCH
2X BLOCKING
EDGE NAILING (TYPICAL)
2X LOWER TOP PLATE EA WALL W/ ADDITIONAL 2X RIPPED TO FIT UPPER TOP PLATE OVER BOTH WALLS
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
2X NON-LOAD BEARING DEMISING STUD WALL

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

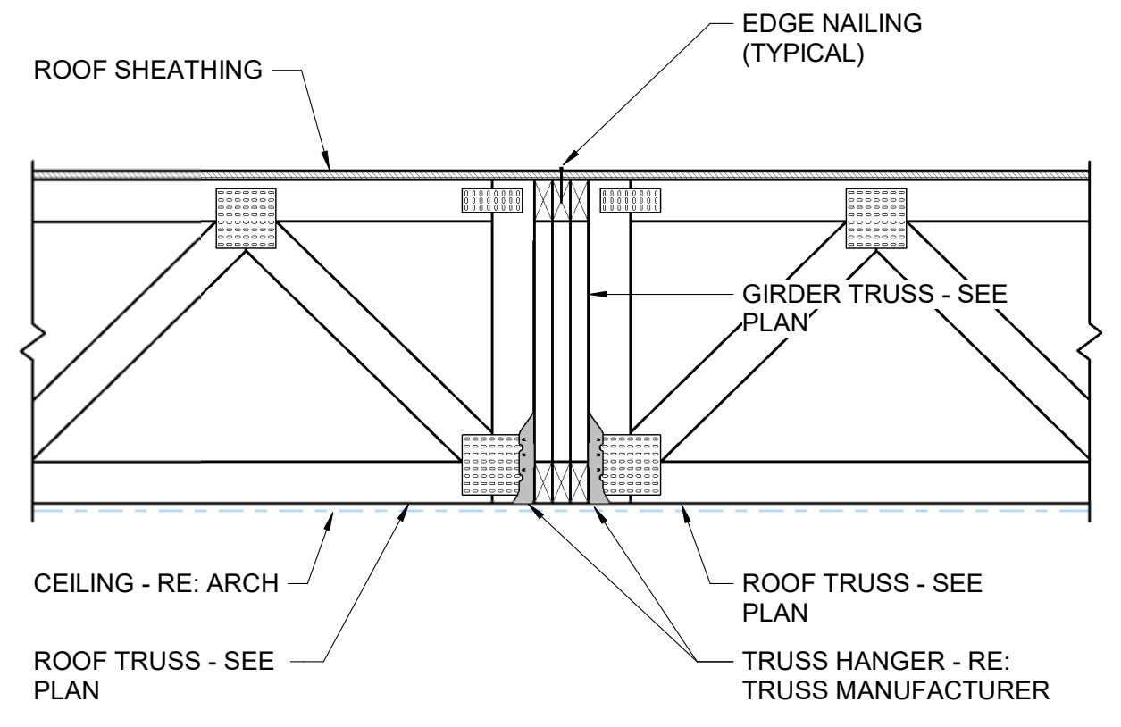
12 Truss Bearing at Party Wall - One Side
3/4" = 1'-0"

ROOF TRUSSES / TRUSS BLOCKS LOCATED ABOVE SHEAR WALLS SHALL BE DESIGNED FOR DRAG FORCE (D.F.) - SEE ROOF PLAN FOR REQUIRED LOADS. TRUSSES WITHOUT DESIGNATED LOADS SHALL BE DESIGNED FOR 400 PLF


EDGE NAILING (TYPICAL)
ROOF SHEATHING
ROOF TRUSS - SEE PLAN
2X LOWER TOP PLATE EA WALL W/ ADDITIONAL 2X RIPPED TO FIT UPPER TOP PLATE OVER BOTH WALLS
ROOF TRUSS
TRUSS BLOCK
2X BLOCKING
EDGE NAILING (TYPICAL)
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE

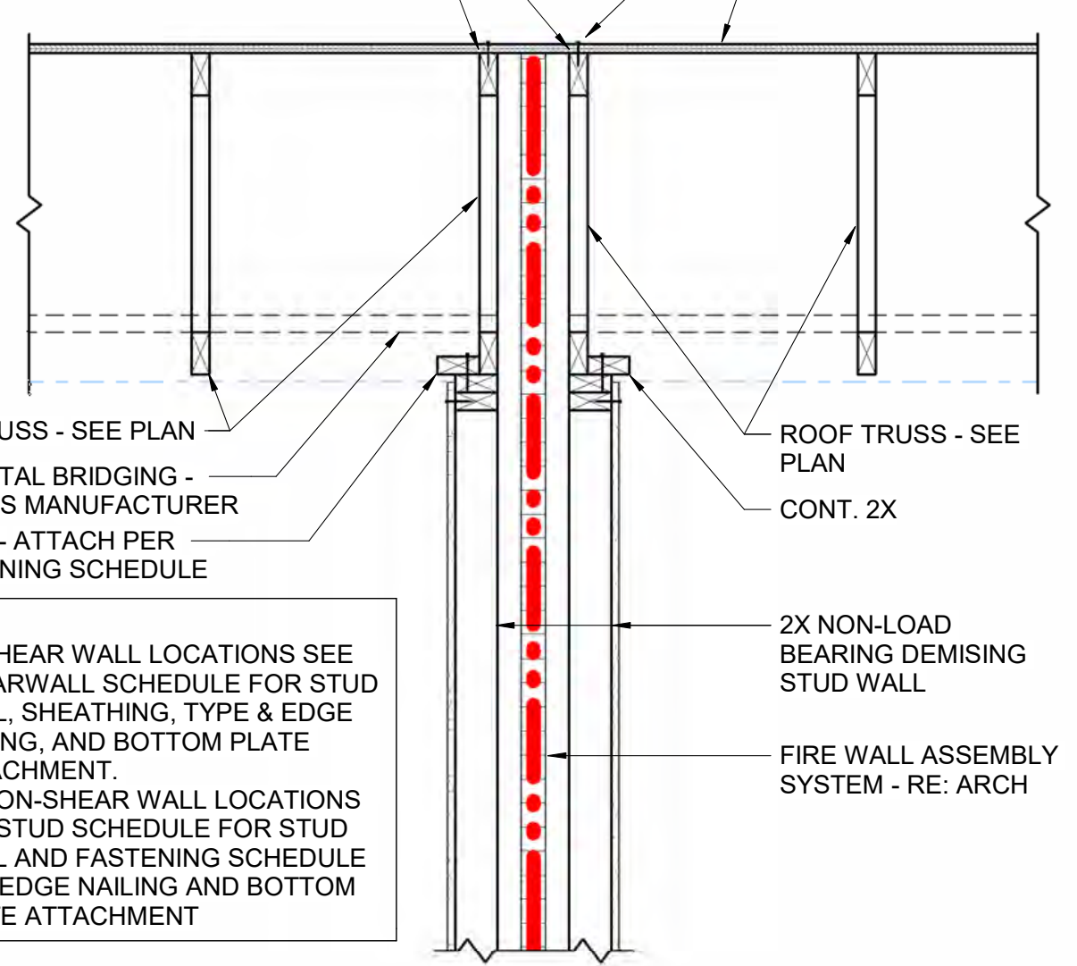
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

11 Truss Bearing at Party Wall - Both Sides
3/4" = 1'-0"



10 Truss Bearing at Girder Truss - Both Side
3/4" = 1'-0"

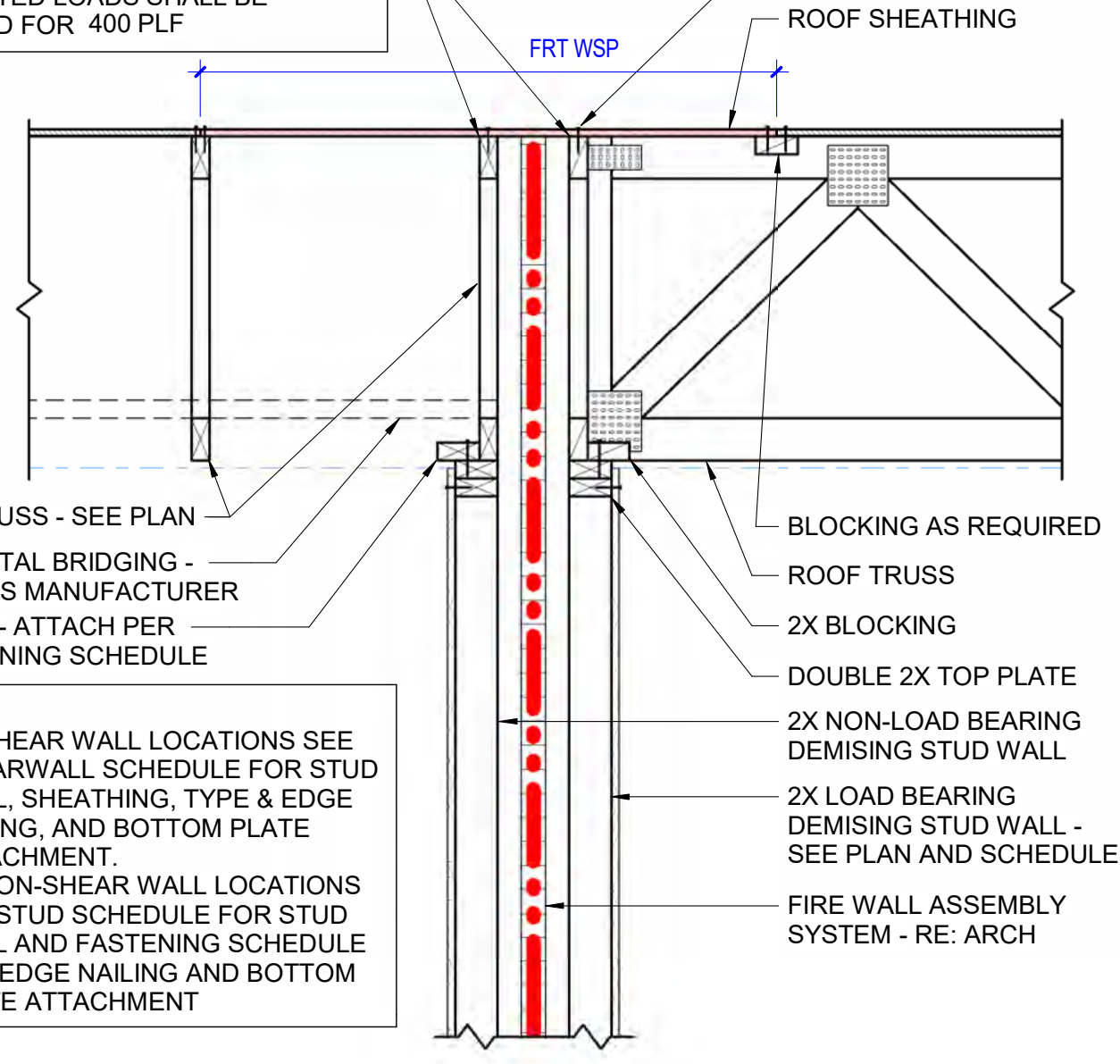
ROOF TRUSSES / TRUSS BLOCKS LOCATED ABOVE SHEAR WALLS SHALL BE DESIGNED FOR DRAG FORCE (D.F.) - SEE ROOF PLAN FOR REQUIRED LOADS. TRUSSES WITHOUT DESIGNATED LOADS SHALL BE DESIGNED FOR 400 PLF


EDGE NAILING (TYPICAL)
ROOF SHEATHING
ROOF TRUSS - SEE PLAN
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
2X CONT - ATTACH PER FASTENENING SCHEDULE
ROOF TRUSS - SEE PLAN
CONT. 2X
2X NON-LOAD BEARING DEMISING STUD WALL
FIRE WALL ASSEMBLY SYSTEM - RE: ARCH

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

9 Truss at Firewall - Parallel
3/4" = 1'-0"

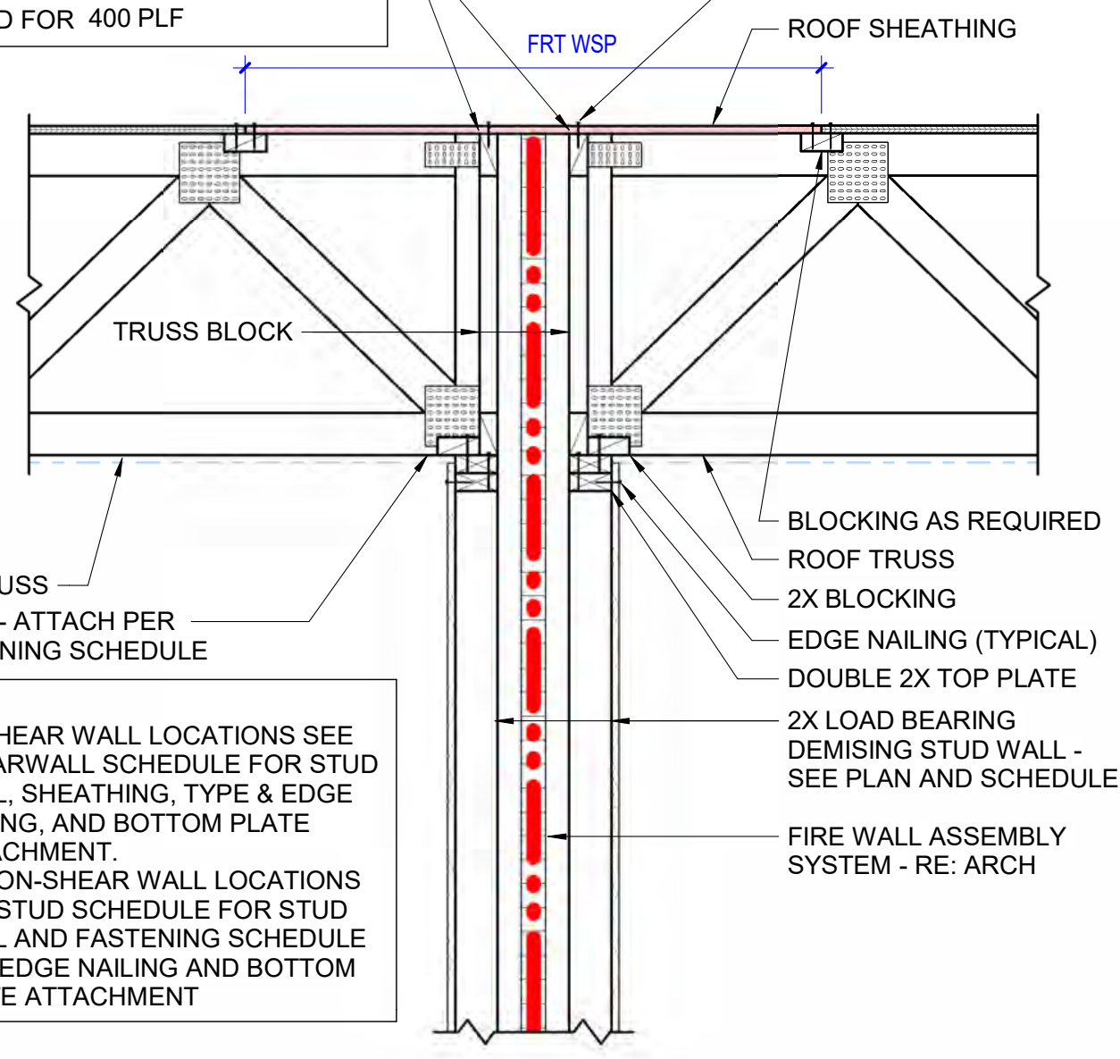
ROOF TRUSSES / TRUSS BLOCKS LOCATED ABOVE SHEAR WALLS SHALL BE DESIGNED FOR DRAG FORCE (D.F.) - SEE ROOF PLAN FOR REQUIRED LOADS. TRUSSES WITHOUT DESIGNATED LOADS SHALL BE DESIGNED FOR 400 PLF


EDGE NAILING (TYPICAL)
ROOF SHEATHING
FRT WSP
ROOF TRUSS - SEE PLAN
HORIZONTAL BRIDGING - RE: TRUSS MANUFACTURER
2X CONT - ATTACH PER FASTENENING SCHEDULE
BLOCKING AS REQUIRED
ROOF TRUSS
2X BLOCKING
DOUBLE 2X TOP PLATE
2X NON-LOAD BEARING DEMISING STUD WALL
2X LOAD BEARING DEMISING STUD WALL - SEE PLAN AND SCHEDULE
FIRE WALL ASSEMBLY SYSTEM - RE: ARCH

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

8 Truss Bearing at Fire Wall - One Side
3/4" = 1'-0"

ROOF TRUSSES / TRUSS BLOCKS LOCATED ABOVE SHEAR WALLS SHALL BE DESIGNED FOR DRAG FORCE (D.F.) - SEE ROOF PLAN FOR REQUIRED LOADS. TRUSSES WITHOUT DESIGNATED LOADS SHALL BE DESIGNED FOR 400 PLF



NOTE:
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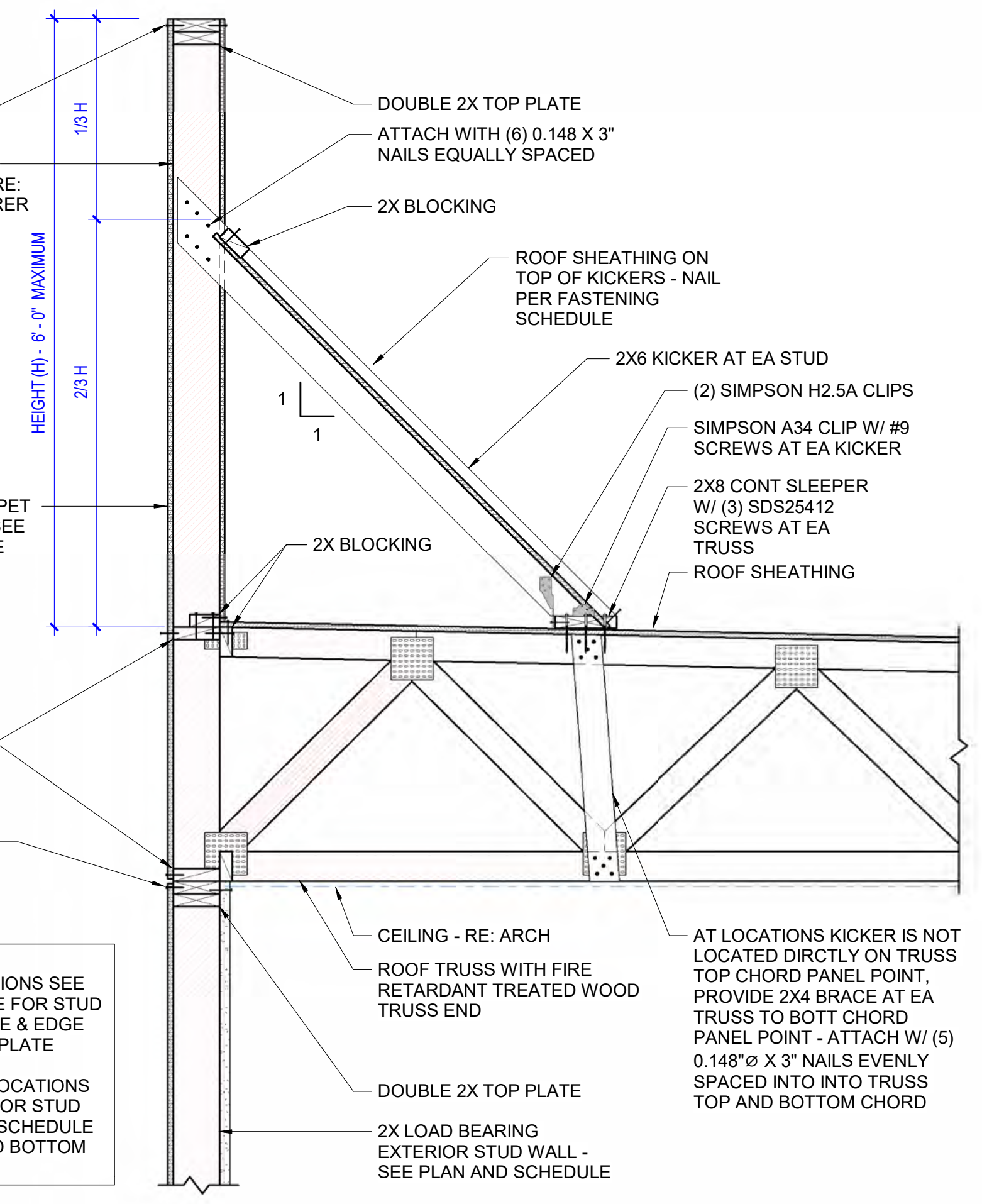
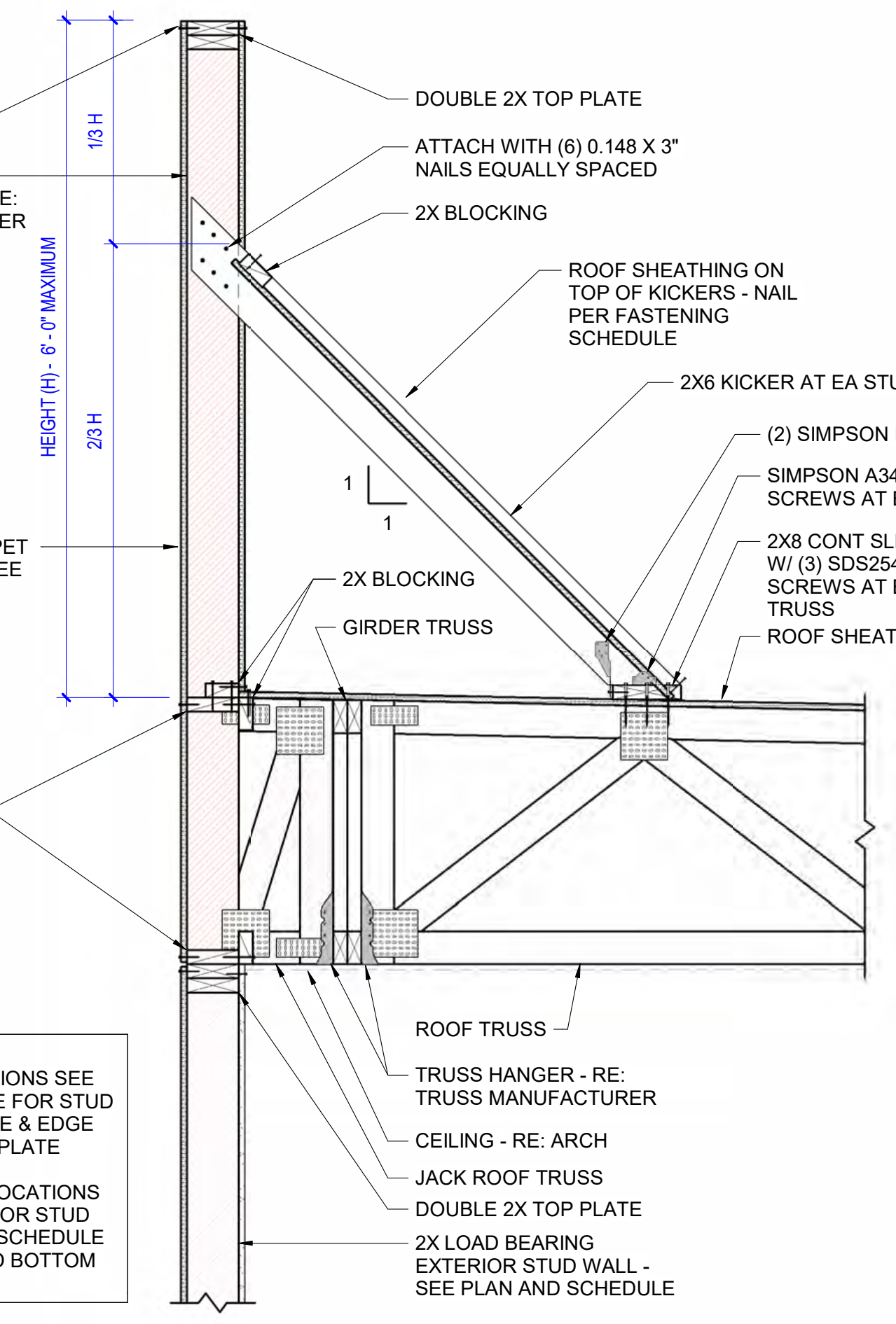
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

7 Truss Bearing at Fire Wall - Both Sides
3/4" = 1'-0"


EDGE NAILING - TYPICAL
2X6 ROOF PARAPET BUILT INTO TRUSS - RE: TRUSS MANUFACTURER
1/3 H
2/3 H
HEIGHT (H) - 6' - 0" MAXIMUM
SHEATH ROOF PARAPET PER WALL BELOW - SEE PLAN AND SCHEDULE
2X BLOCKING
EDGE NAILING - TYPICAL
DOUBLE 2X TOP PLATE
ATTACH WITH (6) 0.148 X 3" NAILS EQUALLY SPACED
2X BLOCKING
ROOF SHEATHING ON TOP OF KICKERS - NAIL PER FASTENING SCHEDULE
2X6 KICKER AT EA STUD
(2) SIMPSON H2.5A CLIPS
SIMPSON A34 CLIP W/ #9 SCREWS AT EA KICKER
2X8 CONT SLEEPER W/ (3) SDS25412 SCREWS AT EA TRUSS
ROOF SHEATHING
1
1
2X BLOCKING
CEILING - RE: ARCH
ROOF TRUSS WITH FIRE RETARDANT TREATED WOOD TRUSS END
DOUBLE 2X TOP PLATE
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE
AT LOCATIONS KICKER IS NOT LOCATED DIRCTLY ON TRUSS TOP CHORD PANEL POINT, PROVIDE 2X4 BRACE AT EA TRUSS TO BOTT CHORD PANEL POINT - ATTACH W/ (5) 0.148"Ø X 3" NAILS EVENLY SPACED INTO INTO TRUSS TOP AND BOTTOM CHORD

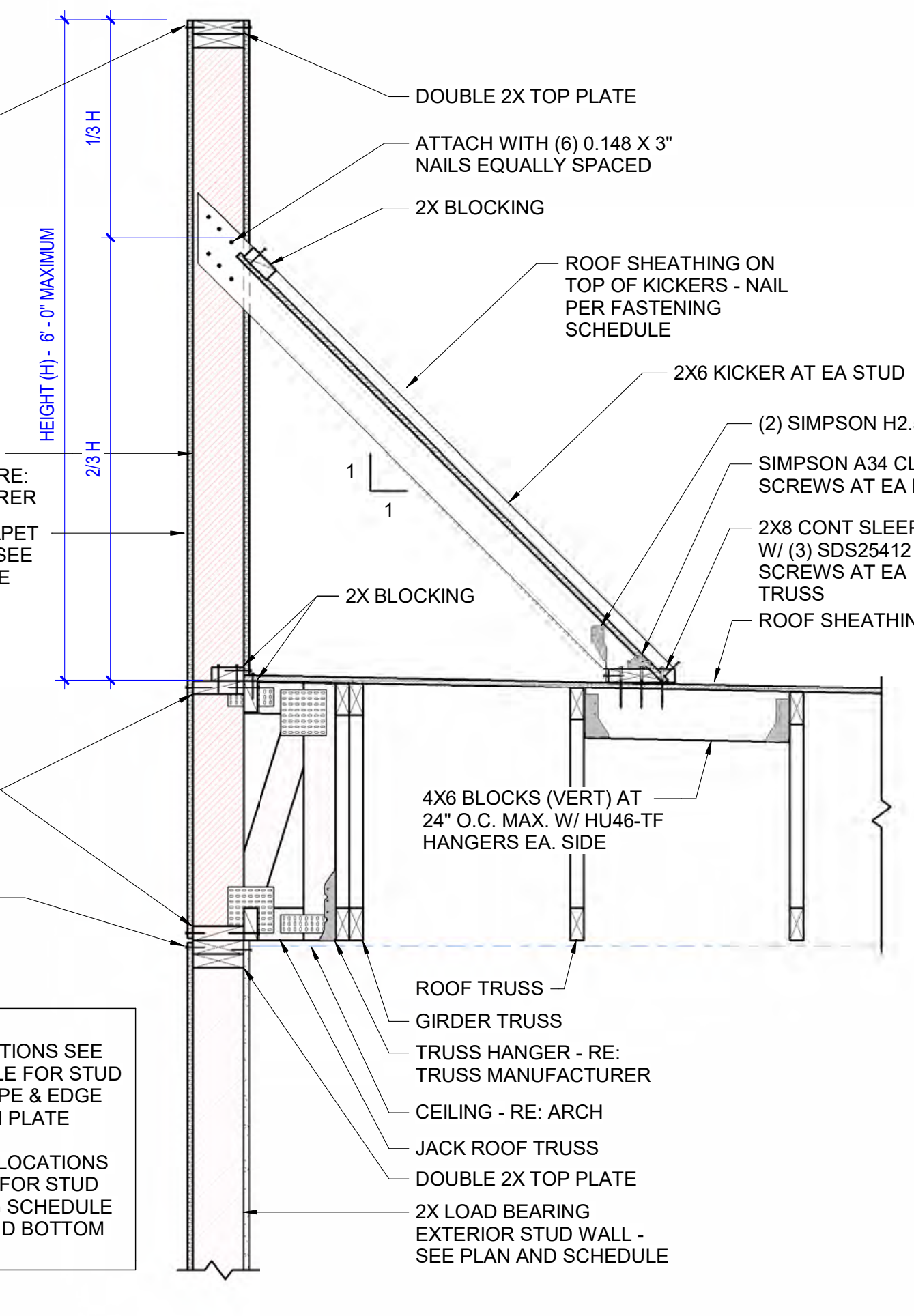
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

6 Exterior Truss Bearing - 6'-0" Max Parapet
3/4" = 1'-0"


EDGE NAILING - TYPICAL
2X6 ROOF PARAPET BUILT INTO TRUSS - RE: TRUSS MANUFACTURER
1/3 H
2/3 H
HEIGHT (H) - 6' - 0" MAXIMUM
SHEATH ROOF PARAPET PER WALL BELOW - SEE PLAN AND SCHEDULE
2X BLOCKING
DOUBLE 2X TOP PLATE
ATTACH WITH (6) 0.148 X 3" NAILS EQUALLY SPACED
2X BLOCKING
ROOF SHEATHING ON TOP OF KICKERS - NAIL PER FASTENING SCHEDULE
2X6 KICKER AT EA STUD
(2) SIMPSON H2.5A CLIPS
SIMPSON A34 CLIP W/ #9 SCREWS AT EA KICKER
2X8 CONT SLEEPER W/ (3) SDS25412 SCREWS AT EA TRUSS
ROOF SHEATHING
1
1
2X BLOCKING
GIRDER TRUSS
ROOF TRUSS
TRUSS HANGER - RE: TRUSS MANUFACTURER
CEILING - RE: ARCH
JACK ROOF TRUSS
DOUBLE 2X TOP PLATE
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE

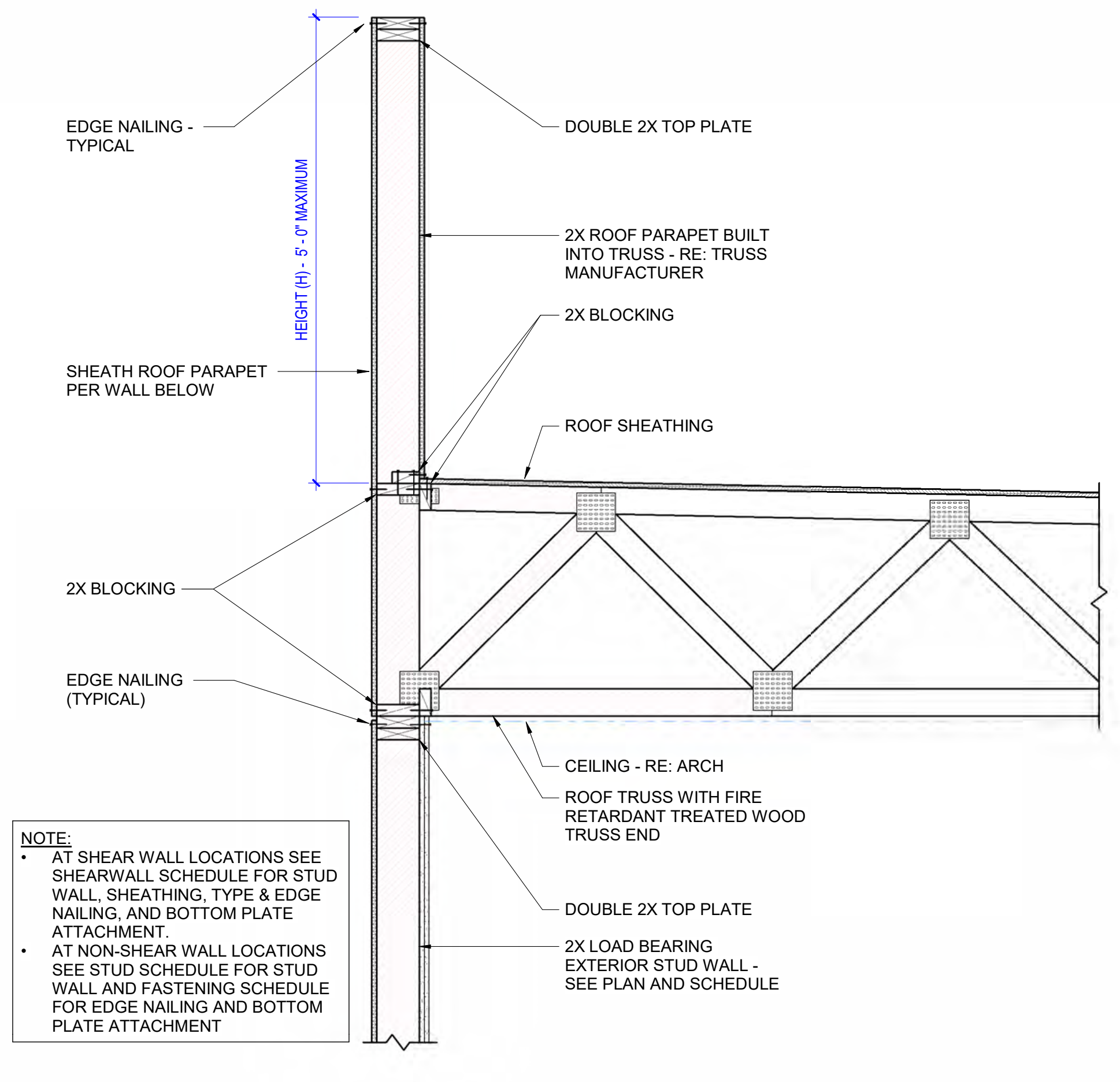
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

5 Exterior Truss Bearing at GT - 6'-0" Max Parapet
3/4" = 1'-0"


EDGE NAILING - TYPICAL
1/3 H
2/3 H
HEIGHT (H) - 6' - 0" MAXIMUM
2X6 ROOF PARAPET BUILT INTO TRUSS - RE: TRUSS MANUFACTURER
SHEATH ROOF PARAPET PER WALL BELOW - SEE PLAN AND SCHEDULE
2X BLOCKING
EDGE NAILING - TYPICAL
DOUBLE 2X TOP PLATE
ATTACH WITH (6) 0.148 X 3" NAILS EQUALLY SPACED
2X BLOCKING
ROOF SHEATHING ON TOP OF KICKERS - NAIL PER FASTENING SCHEDULE
2X6 KICKER AT EA STUD
(2) SIMPSON H2.5A CLIPS
SIMPSON A34 CLIP W/ #9 SCREWS AT EA KICKER
2X8 CONT SLEEPER W/ (3) SDS25412 SCREWS AT EA TRUSS
ROOF SHEATHING
1
1
2X BLOCKING
4X6 BLOCKS (VERT) AT 24" O.C. MAX. W/ HU46-TF HANGERS EA. SIDE
ROOF TRUSS
GIRDER TRUSS
TRUSS HANGER - RE: TRUSS MANUFACTURER
CEILING - RE: ARCH
JACK ROOF TRUSS
DOUBLE 2X TOP PLATE
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE

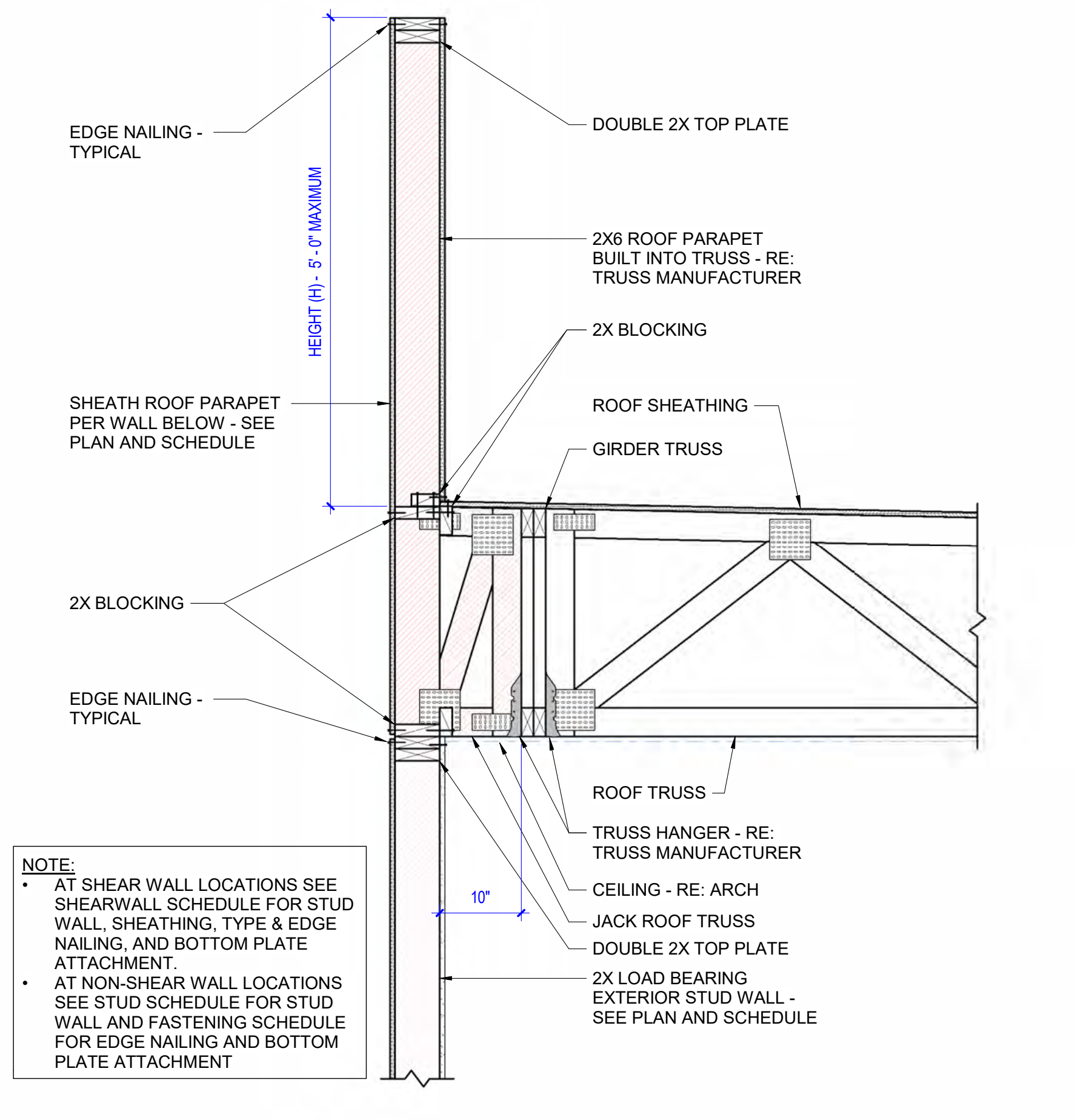
NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

4 Exterior Truss Parallel to GT - 6'-0" Max Parapet
3/4" = 1'-0"


EDGE NAILING - TYPICAL
HEIGHT (H) - 5' - 0" MAXIMUM
SHEATH ROOF PARAPET PER WALL BELOW
2X BLOCKING
EDGE NAILING (TYPICAL)
DOUBLE 2X TOP PLATE
2X ROOF PARAPET BUILT INTO TRUSS - RE: TRUSS MANUFACTURER
2X BLOCKING
ROOF SHEATHING
CEILING - RE: ARCH
ROOF TRUSS WITH FIRE RETARDANT TREATED WOOD TRUSS END
DOUBLE 2X TOP PLATE
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

3 Exterior Truss Bearing - 5'-0" Max Parapet
3/4" = 1'-0"


EDGE NAILING - TYPICAL
HEIGHT (H) - 5' - 0" MAXIMUM
SHEATH ROOF PARAPET PER WALL BELOW - SEE PLAN AND SCHEDULE
2X BLOCKING
EDGE NAILING - TYPICAL
DOUBLE 2X TOP PLATE
2X6 ROOF PARAPET BUILT INTO TRUSS - RE: TRUSS MANUFACTURER
2X BLOCKING
ROOF SHEATHING
GIRDER TRUSS
10"
ROOF TRUSS
TRUSS HANGER - RE: TRUSS MANUFACTURER
CEILING - RE: ARCH
JACK ROOF TRUSS
DOUBLE 2X TOP PLATE
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE

NOTE:
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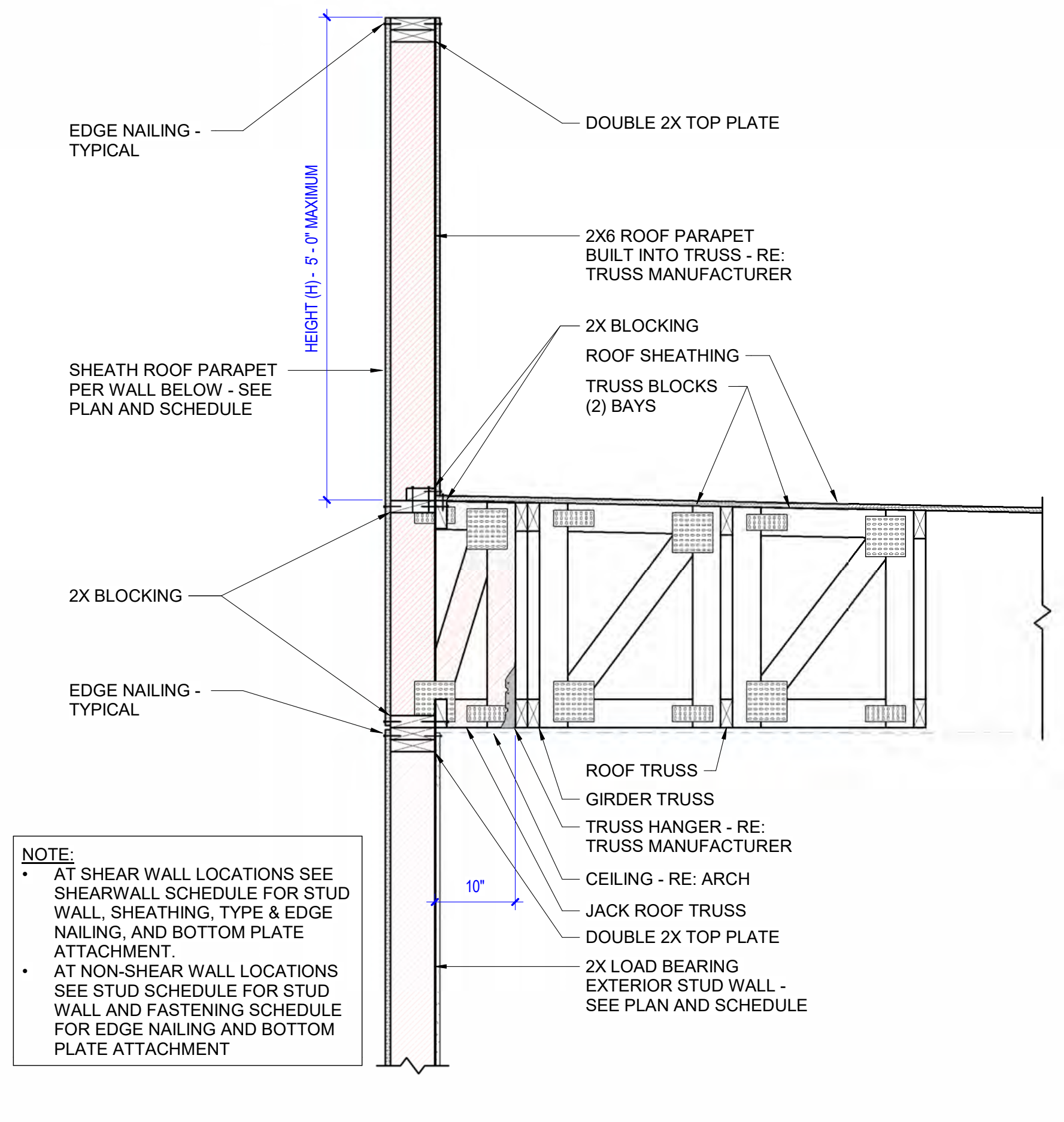
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

2 Exterior Truss Bearing at GT - 5'-0" Max Parapet
3/4" = 1'-0"


EDGE NAILING - TYPICAL
HEIGHT (H) - 5' - 0" MAXIMUM
SHEATH ROOF PARAPET PER WALL BELOW - SEE PLAN AND SCHEDULE
2X BLOCKING
EDGE NAILING - TYPICAL
DOUBLE 2X TOP PLATE
2X6 ROOF PARAPET BUILT INTO TRUSS - RE: TRUSS MANUFACTURER
2X BLOCKING
ROOF SHEATHING
TRUSS BLOCKS (2) BAYS
10"
ROOF TRUSS
GIRDER TRUSS
TRUSS HANGER - RE: TRUSS MANUFACTURER
CEILING - RE: ARCH
JACK ROOF TRUSS
DOUBLE 2X TOP PLATE
2X LOAD BEARING EXTERIOR STUD WALL - SEE PLAN AND SCHEDULE

NOTE:
- AT SHEAR WALL LOCATIONS SEE SHEARWALL SCHEDULE FOR STUD WALL, SHEATHING, TYPE & EDGE NAILING, AND BOTTOM PLATE ATTACHMENT.
- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

1 Exterior Truss Parallel to GT - 5'-0" Max Parapet
3/4" = 1'-0"

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

NILES BOLTON ASSOCIATES

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Suite 600
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T 404 365 7600

www.nilesbolton.com

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EAST BATON ROUGE PARISH

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Roof Framing Sections

SHEET NUMBER:
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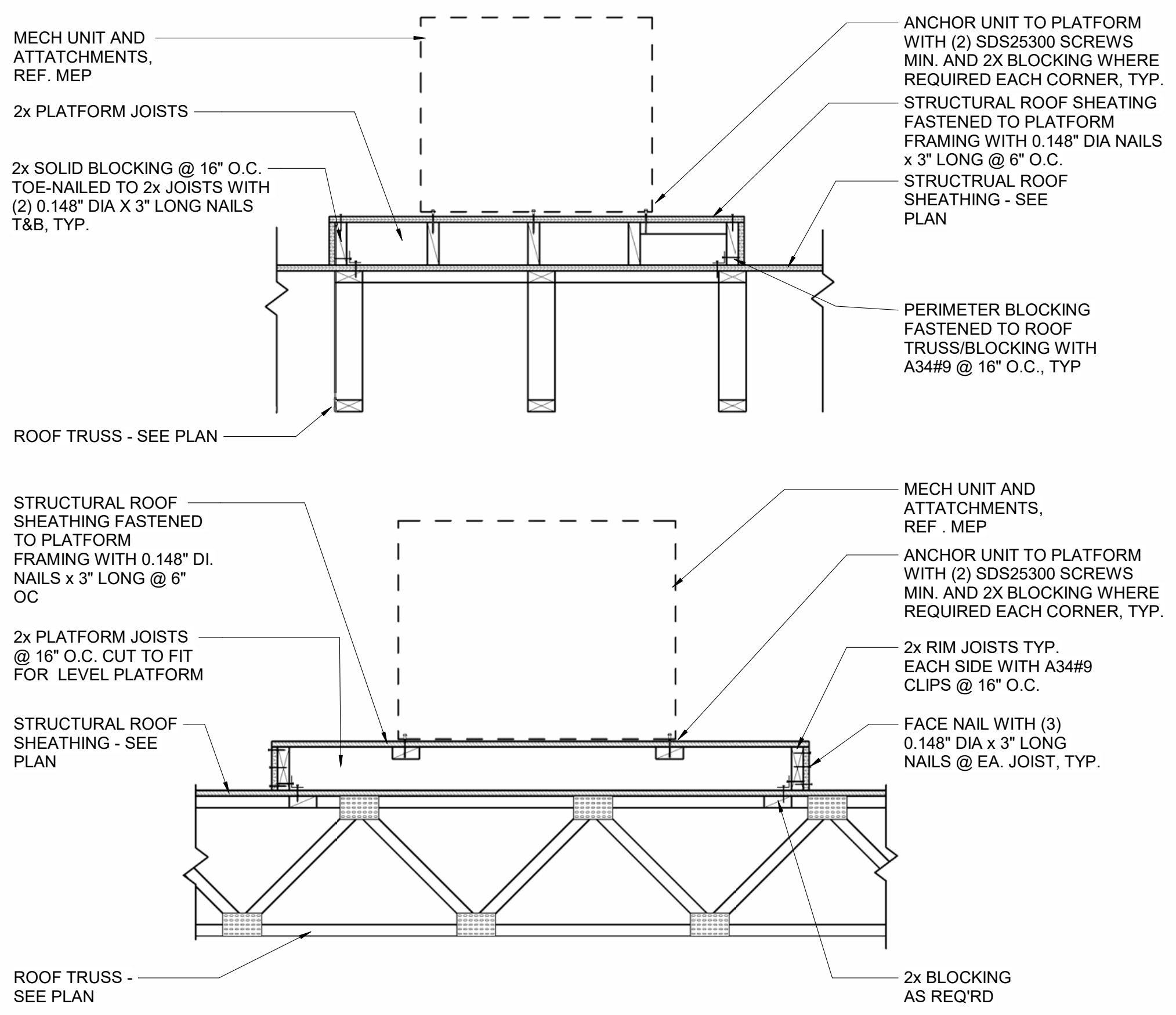
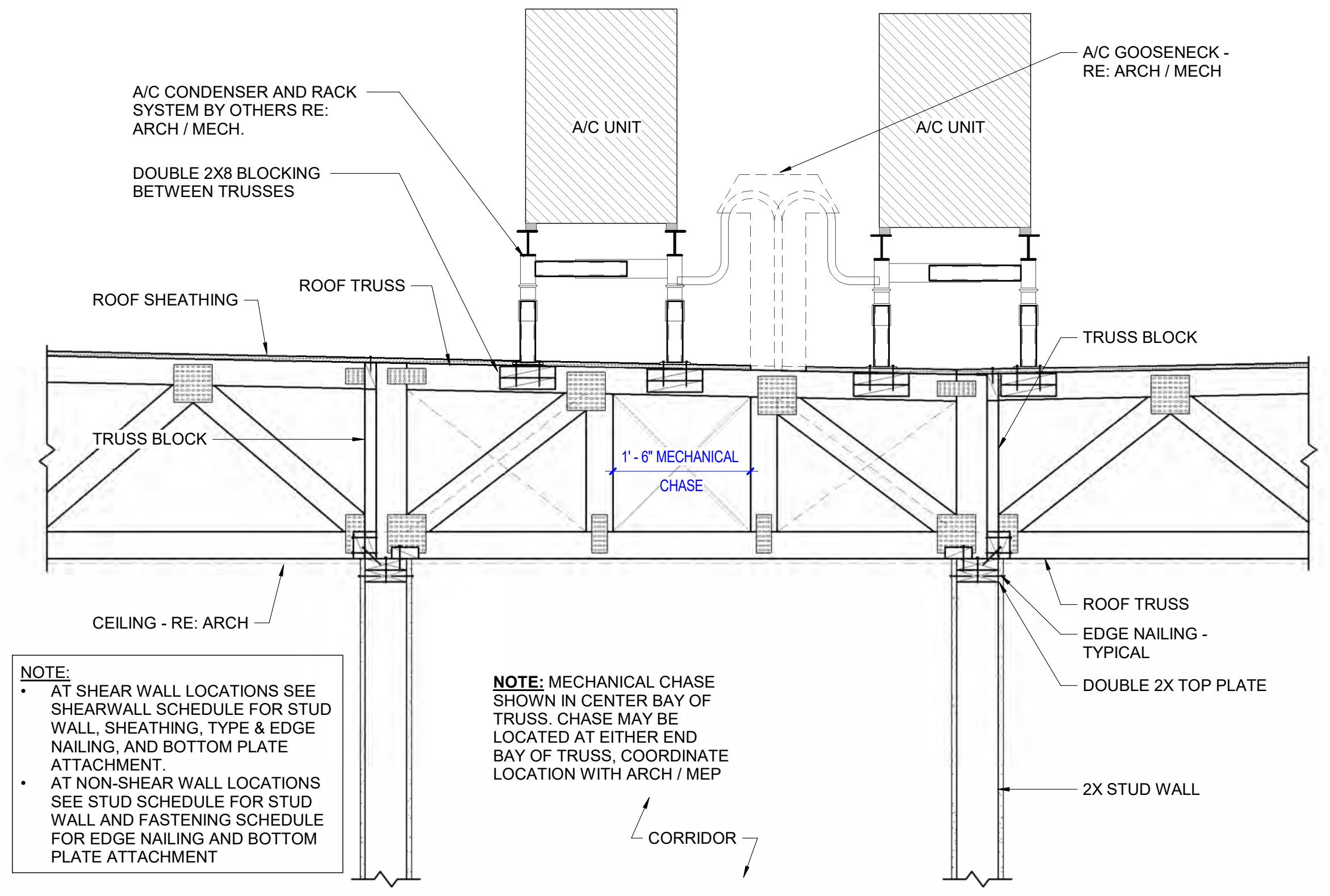

MECH UNIT AND ATTATCHMENTS, REF. MEP
2x PLATFORM JOISTS
2x SOLID BLOCKING @ 16" O.C. TOE-NAILED TO 2x JOISTS WITH (2) 0.148" DIA X 3" LONG NAILS T&B, TYP.
ROOF TRUSS - SEE PLAN
ANCHOR UNIT TO PLATFORM WITH (2) SDS25300 SCREWS MIN. AND 2X BLOCKING WHERE REQUIRED EACH CORNER, TYP.
STRUCTURAL ROOF SHEATING FASTENED TO PLATFORM FRAMING WITH 0.148" DIA NAILS x 3" LONG @ 6" O.C.
STRUCTRUAL ROOF SHEATHING - SEE PLAN
PERIMETER BLOCKING FASTENED TO ROOF TRUSS/BLOCKING WITH A34#9 @ 16" O.C., TYP
STRUCTURAL ROOF SHEATHING FASTENED TO PLATFORM FRAMING WITH 0.148" DI. NAILS x 3" LONG @ 6" OC
2x PLATFORM JOISTS @ 16" O.C. CUT TO FIT FOR LEVEL PLATFORM
STRUCTURAL ROOF SHEATHING - SEE PLAN
ROOF TRUSS - SEE PLAN
MECH UNIT AND ATTATCHMENTS, REF . MEP
ANCHOR UNIT TO PLATFORM WITH (2) SDS25300 SCREWS MIN. AND 2X BLOCKING WHERE REQUIRED EACH CORNER, TYP.
2x RIM JOISTS TYP. EACH SIDE WITH A34#9 CLIPS @ 16" O.C.
FACE NAIL WITH (3) 0.148" DIA x 3" LONG NAILS @ EA. JOIST, TYP.
2x BLOCKING AS REQ'RD

② Typical Wood Framed Mechancial Platform Detail
3/4" = 1'-0"



NOTE:
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- AT NON-SHEAR WALL LOCATIONS SEE STUD SCHEDULE FOR STUD WALL AND FASTENING SCHEDULE FOR EDGE NAILING AND BOTTOM PLATE ATTACHMENT

NOTE: MECHANICAL CHASE SHOWN IN CENTER BAY OF TRUSS. CHASE MAY BE LOCATED AT EITHER END BAY OF TRUSS, COORDINATE LOCATION WITH ARCH / MEP

① Truss Bearing at Corridor w/ Condenser Rack System
3/4" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

NILES BOLTON ASSOCIATES

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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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SHEET TITLE:
Roof Framing
Sections

SHEET NUMBER:
S4.311

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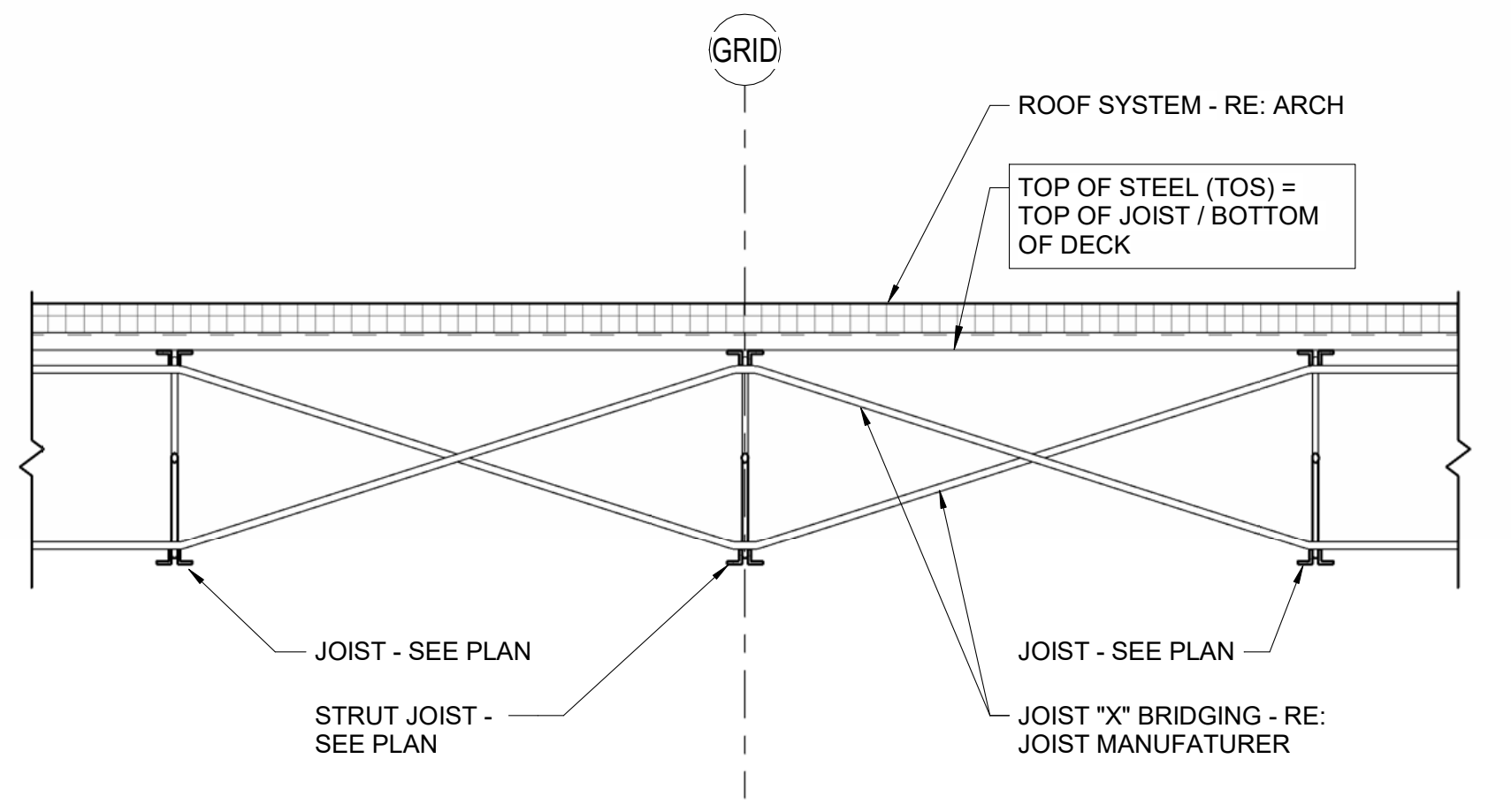
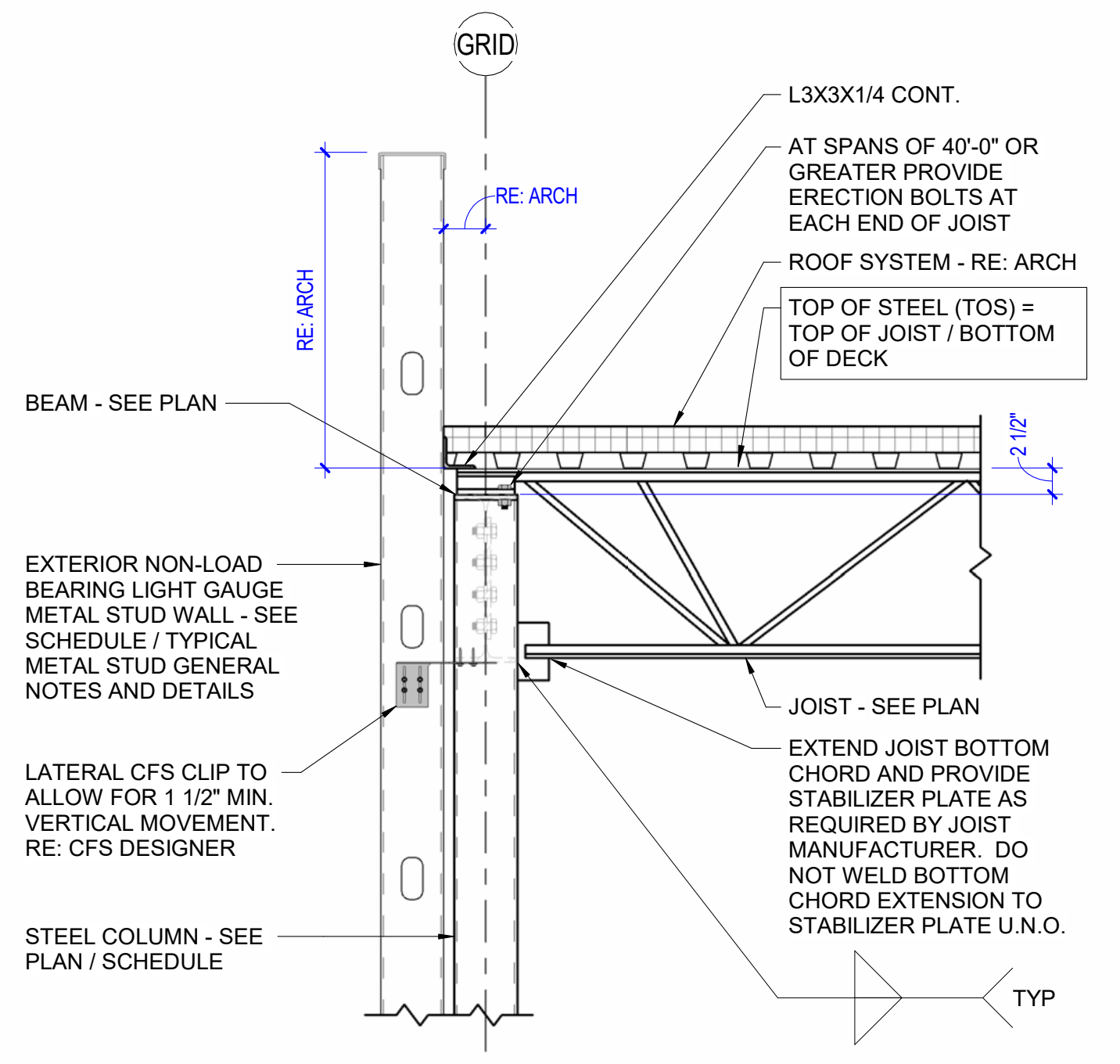
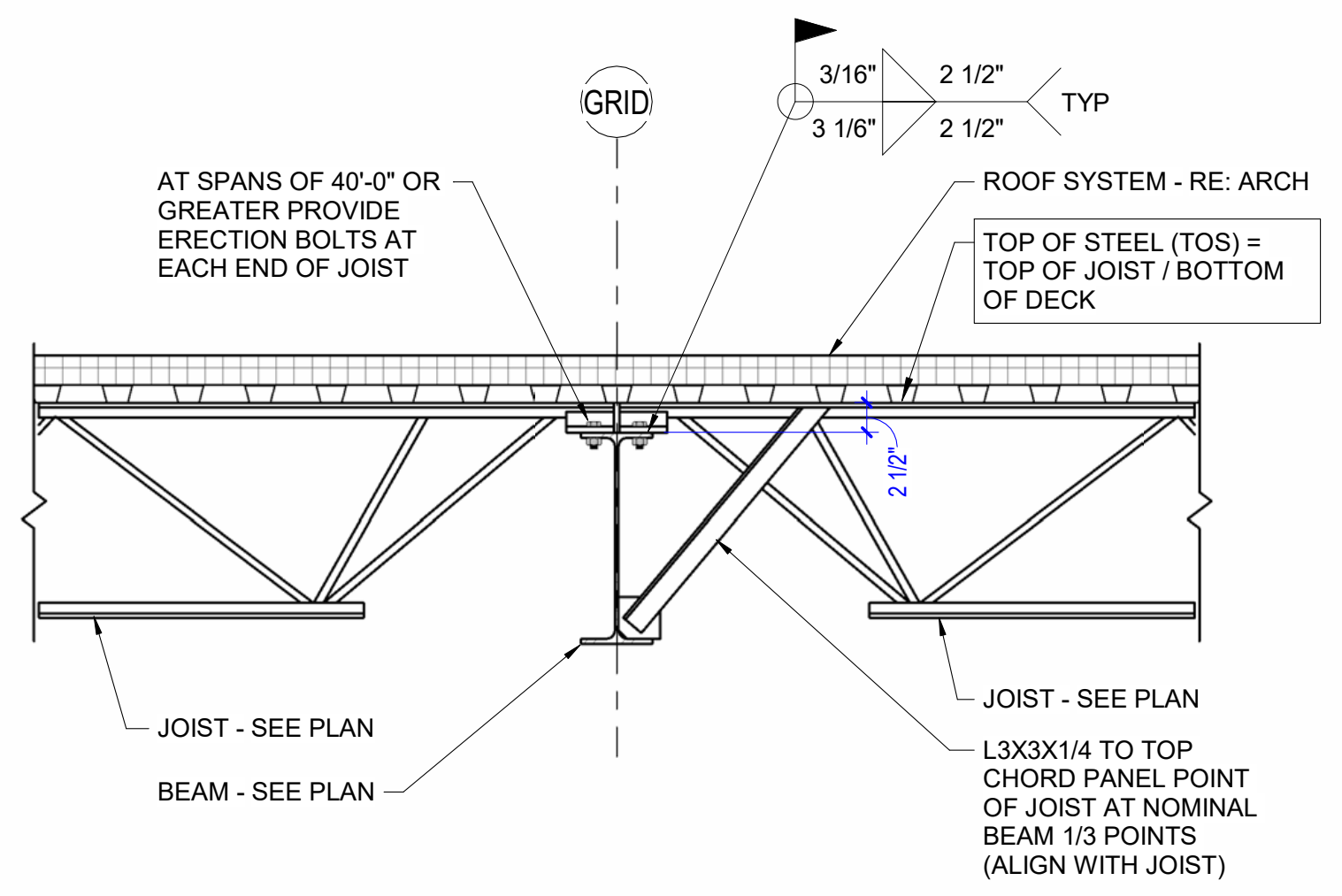

GRID
ROOF SYSTEM - RE: ARCH
TOP OF STEEL (TOS) = TOP OF JOIST / BOTTOM OF DECK
JOIST - SEE PLAN
STRUT JOIST - SEE PLAN
JOIST - SEE PLAN
JOIST "X" BRIDGING - RE: JOIST MANUFATURER

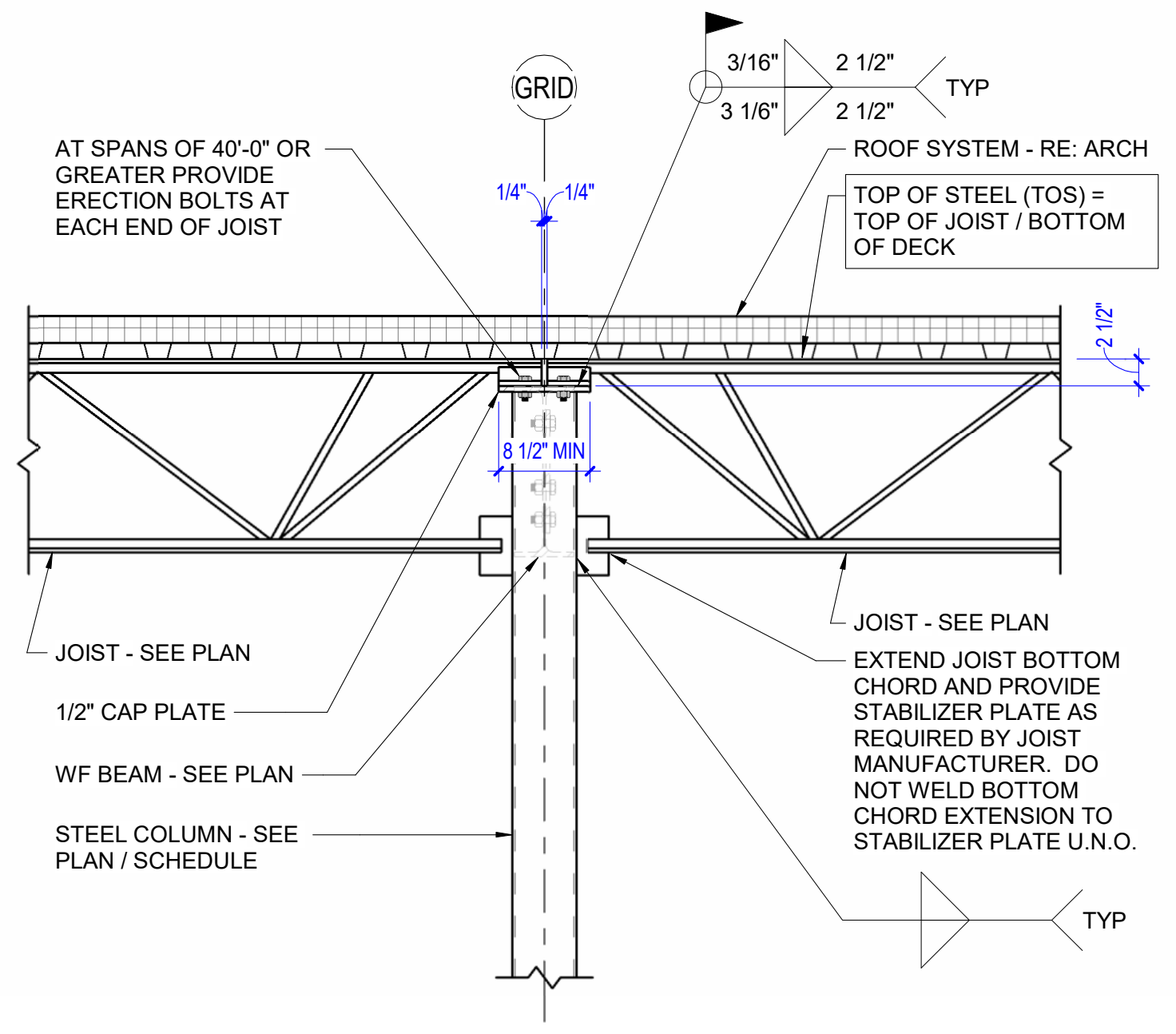
4 Joist Bridging Detail at Strut Joist
3/4" = 1'-0"


GRID
RE: ARCH
RE: ARCH
L3X3X1/4 CONT.
AT SPANS OF 40'-0" OR GREATER PROVIDE ERECTION BOLTS AT EACH END OF JOIST
ROOF SYSTEM - RE: ARCH
TOP OF STEEL (TOS) = TOP OF JOIST / BOTTOM OF DECK
2 1/2"
BEAM - SEE PLAN
EXTERIOR NON-LOAD BEARING LIGHT GAUGE METAL STUD WALL - SEE SCHEDULE / TYPICAL METAL STUD GENERAL NOTES AND DETAILS
LATERAL CFS CLIP TO ALLOW FOR 1 1/2" MIN. VERTICAL MOVEMENT. RE: CFS DESIGNER
STEEL COLUMN - SEE PLAN / SCHEDULE
JOIST - SEE PLAN
EXTEND JOIST BOTTOM CHORD AND PROVIDE STABILIZER PLATE AS REQUIRED BY JOIST MANUFACTURER. DO NOT WELD BOTTOM CHORD EXTENSION TO STABILIZER PLATE U.N.O.
TYP

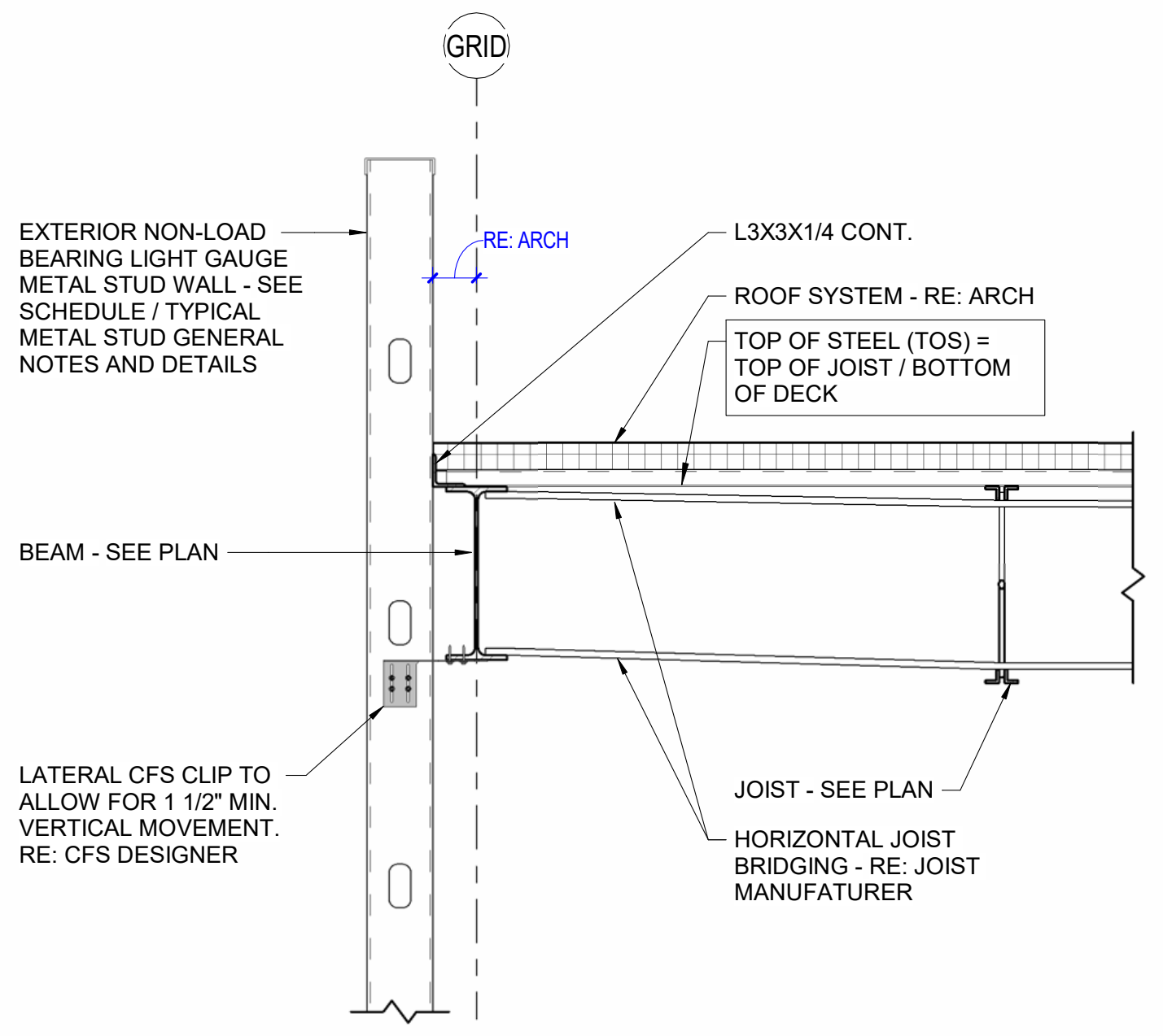
7 Strut Joist Connection at Exterior HSS Column
3/4" = 1'-0"


3/16"
2 1/2"
3 1/6"
2 1/2"
TYP
GRID
AT SPANS OF 40'-0" OR GREATER PROVIDE ERECTION BOLTS AT EACH END OF JOIST
ROOF SYSTEM - RE: ARCH
TOP OF STEEL (TOS) = TOP OF JOIST / BOTTOM OF DECK
2 1/2"
JOIST - SEE PLAN
BEAM - SEE PLAN
JOIST - SEE PLAN
L3X3X1/4 TO TOP CHORD PANEL POINT OF JOIST AT NOMINAL BEAM 1/3 POINTS (ALIGN WITH JOIST)

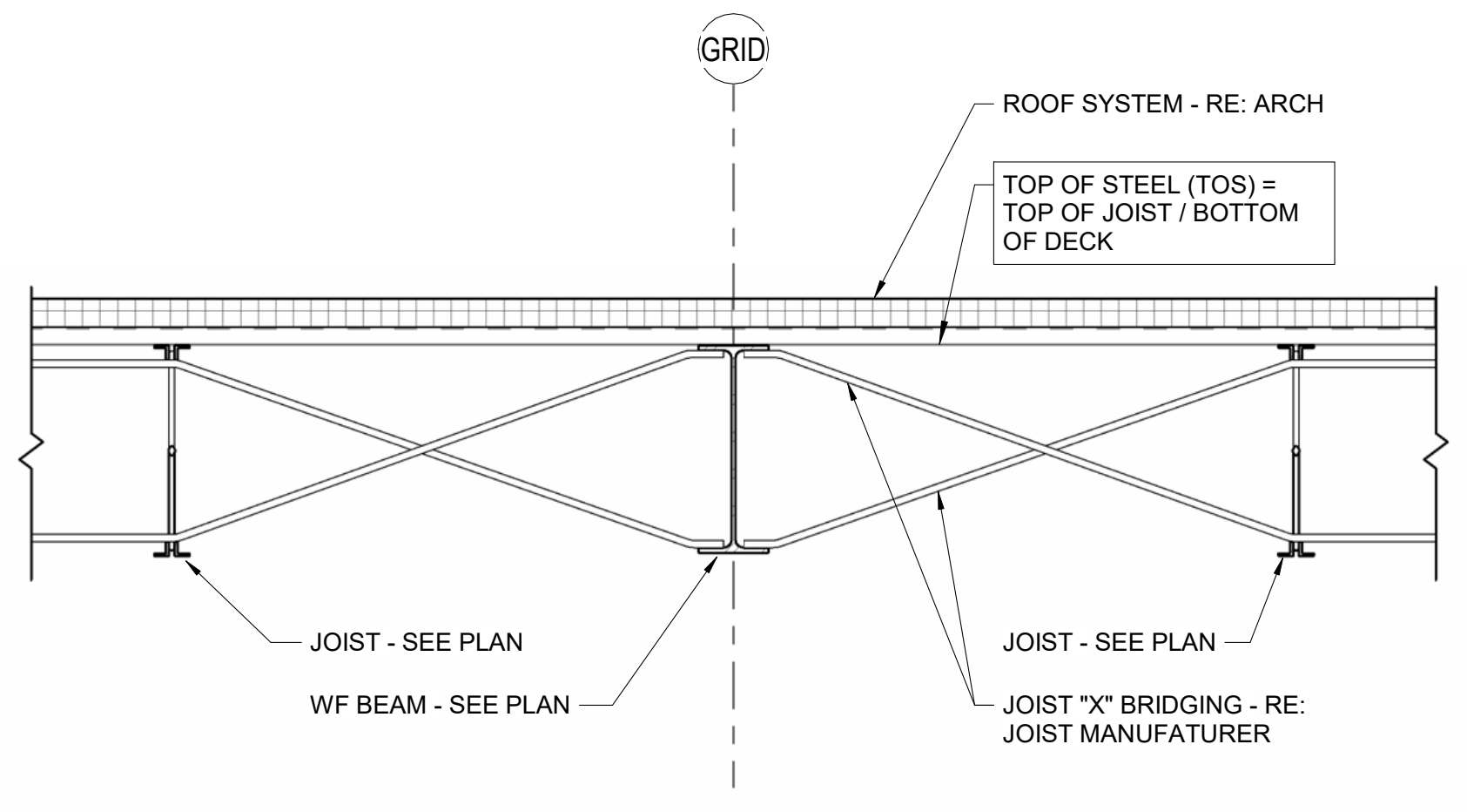
3 Joist Bearing at Interior WF Beam
3/4" = 1'-0"


3/16"
2 1/2"
3 1/6"
2 1/2"
TYP
GRID
1/4"
1/4"
AT SPANS OF 40'-0" OR GREATER PROVIDE ERECTION BOLTS AT EACH END OF JOIST
ROOF SYSTEM - RE: ARCH
TOP OF STEEL (TOS) = TOP OF JOIST / BOTTOM OF DECK
2 1/2"
8 1/2" MIN
JOIST - SEE PLAN
1/2" CAP PLATE
WF BEAM - SEE PLAN
STEEL COLUMN - SEE PLAN / SCHEDULE
JOIST - SEE PLAN
EXTEND JOIST BOTTOM CHORD AND PROVIDE STABILIZER PLATE AS REQUIRED BY JOIST MANUFACTURER. DO NOT WELD BOTTOM CHORD EXTENSION TO STABILIZER PLATE U.N.O.
TYP

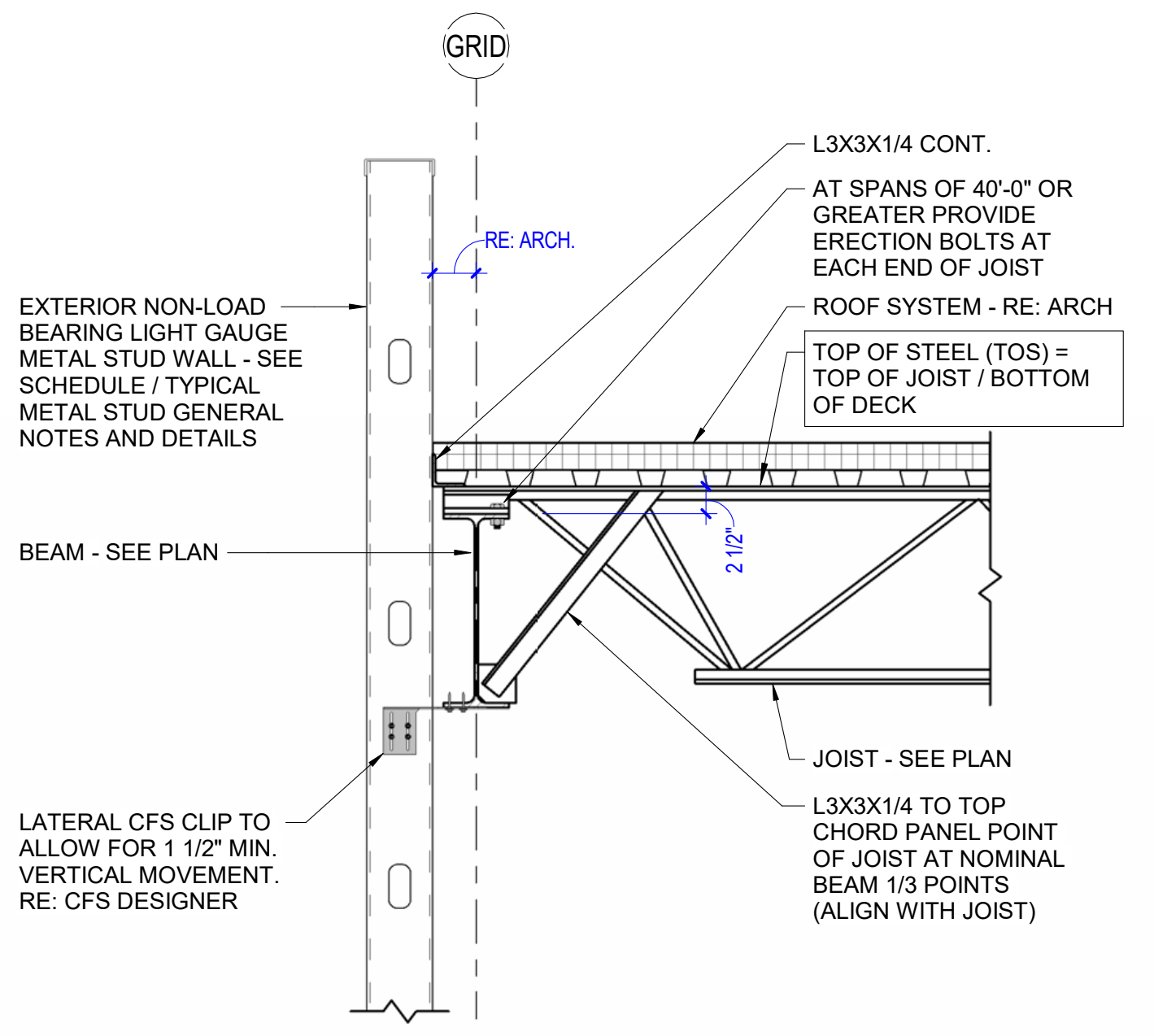
6 Strut Joist Connection at Interior HSS Column - at WF Beam
3/4" = 1'-0"


GRID
RE: ARCH
EXTERIOR NON-LOAD BEARING LIGHT GAUGE METAL STUD WALL - SEE SCHEDULE / TYPICAL METAL STUD GENERAL NOTES AND DETAILS
L3X3X1/4 CONT.
ROOF SYSTEM - RE: ARCH
TOP OF STEEL (TOS) = TOP OF JOIST / BOTTOM OF DECK
BEAM - SEE PLAN
LATERAL CFS CLIP TO ALLOW FOR 1 1/2" MIN. VERTICAL MOVEMENT. RE: CFS DESIGNER
JOIST - SEE PLAN
HORIZONTAL JOIST BRIDGING - RE: JOIST MANUFACTURER

2 Joist Parallel to Exterior WF Beam
3/4" = 1'-0"


GRID
ROOF SYSTEM - RE: ARCH
TOP OF STEEL (TOS) = TOP OF JOIST / BOTTOM OF DECK
JOIST - SEE PLAN
WF BEAM - SEE PLAN
JOIST - SEE PLAN
JOIST "X" BRIDGING - RE: JOIST MANUFATURER

5 Joist Bridging Detail at WF Beam
3/4" = 1'-0"


GRID
RE: ARCH
L3X3X1/4 CONT.
AT SPANS OF 40'-0" OR GREATER PROVIDE ERECTION BOLTS AT EACH END OF JOIST
ROOF SYSTEM - RE: ARCH
TOP OF STEEL (TOS) = TOP OF JOIST / BOTTOM OF DECK
EXTERIOR NON-LOAD BEARING LIGHT GAUGE METAL STUD WALL - SEE SCHEDULE / TYPICAL METAL STUD GENERAL NOTES AND DETAILS
BEAM - SEE PLAN
2 1/2"
LATERAL CFS CLIP TO ALLOW FOR 1 1/2" MIN. VERTICAL MOVEMENT. RE: CFS DESIGNER
JOIST - SEE PLAN
L3X3X1/4 TO TOP CHORD PANEL POINT OF JOIST AT NOMINAL BEAM 1/3 POINTS (ALIGN WITH JOIST)

1 Joist Bearing at Exterior WF Beam
3/4" = 1'-0"

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PROJECT # : 26154
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CHECKED BY: Checker

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- NO. 85617

SHEET TITLE:
Framing - Typical Steel K Bar Joist Roof Details

SHEET NUMBER:
S4.400

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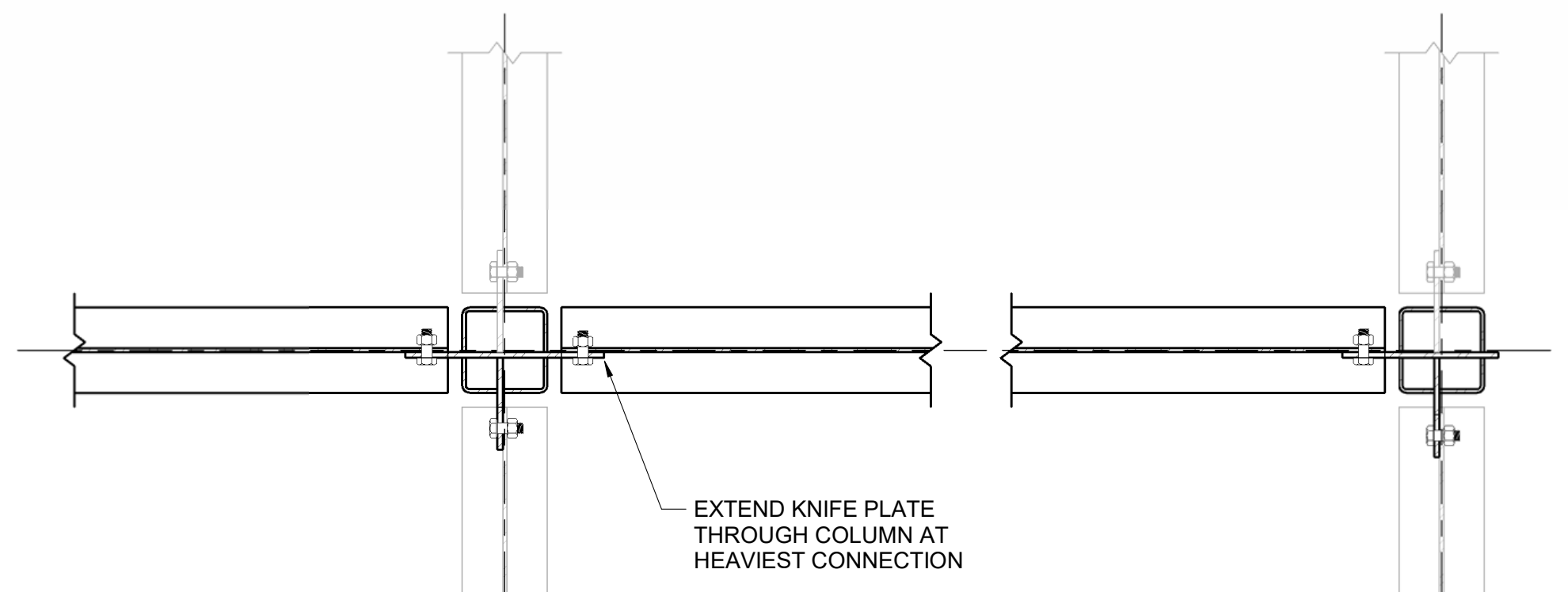
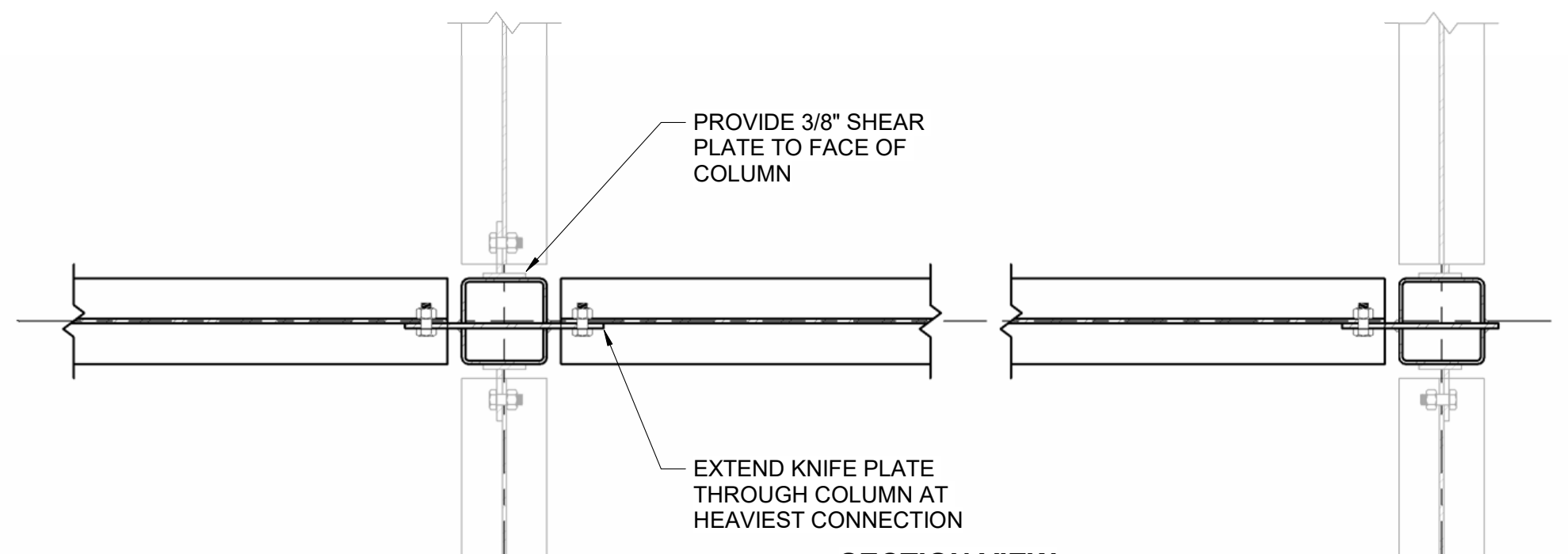
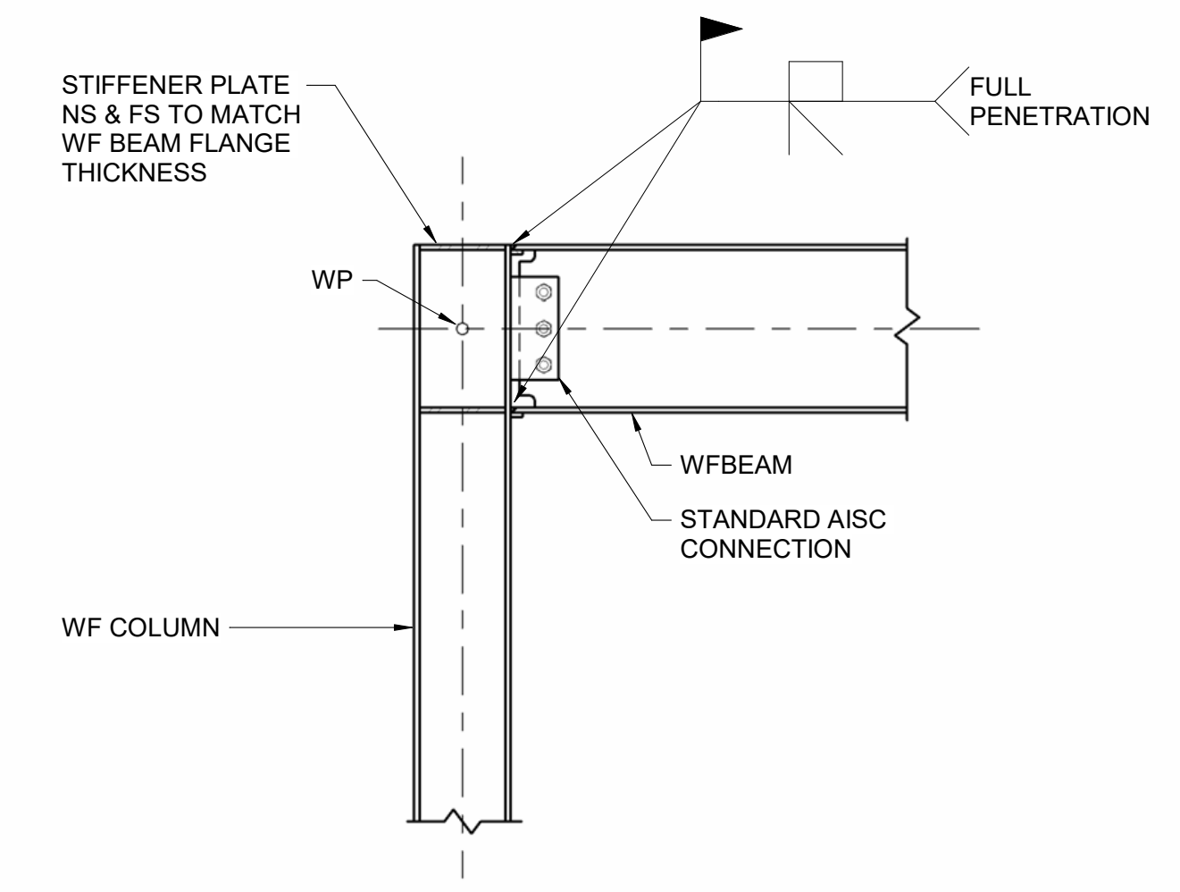


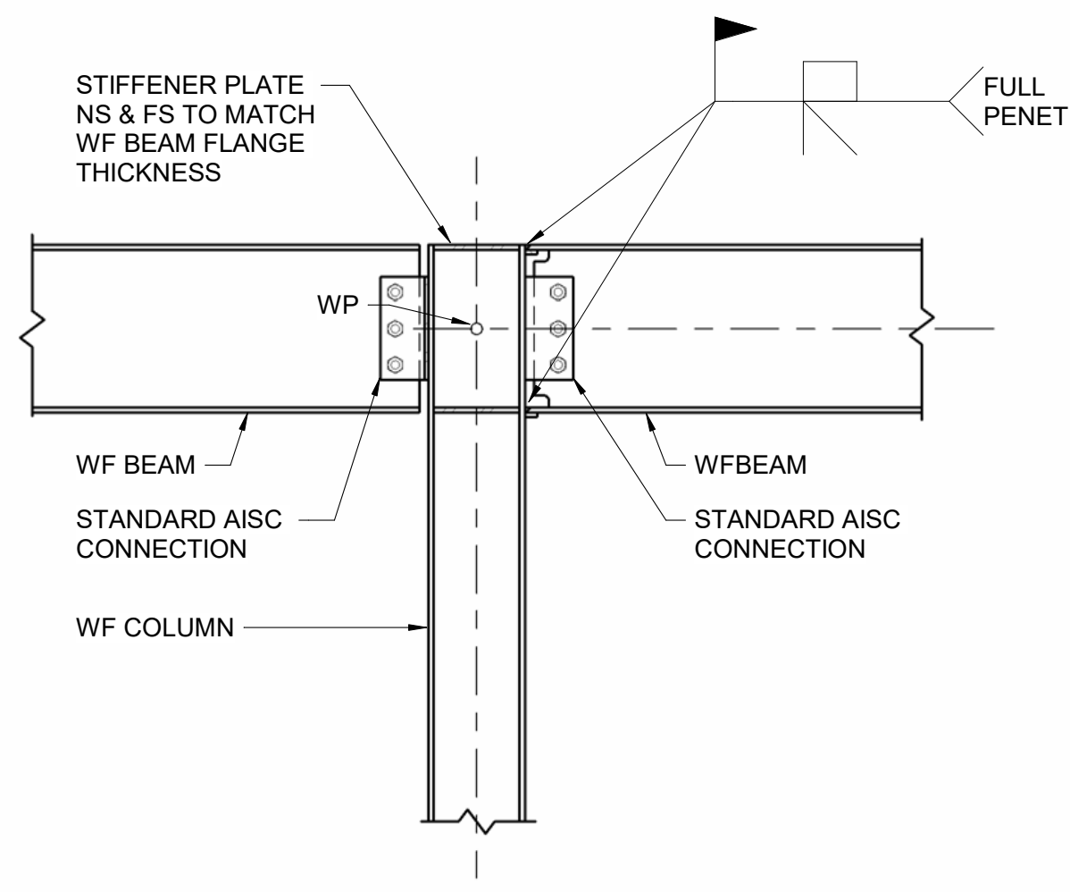
SECTION VIEW
BEAM CONNECTION AT TOP OF COLUMN



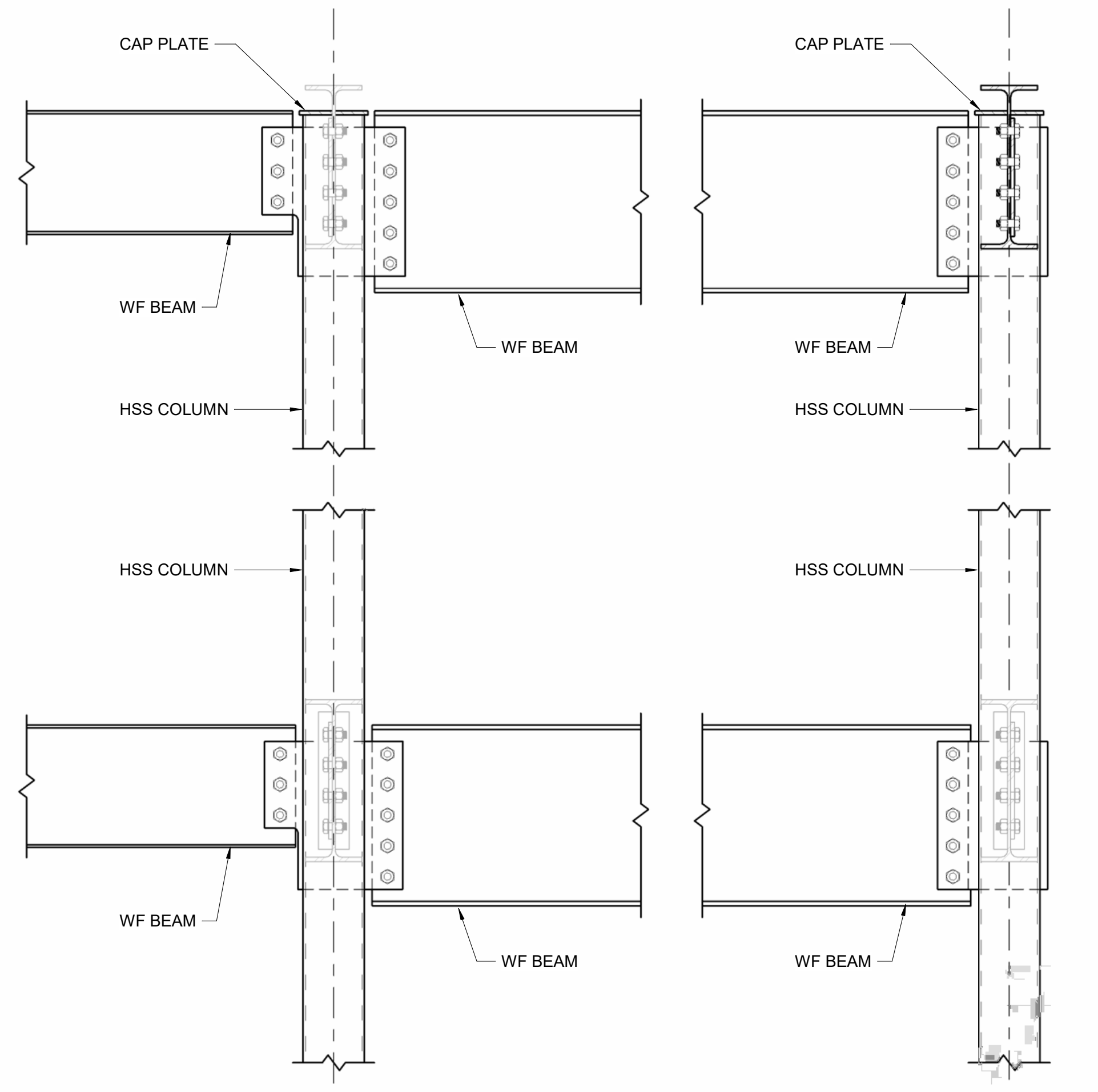
SECTION VIEW
BEAM CONNECTION AT INTERMEDIATE HEIGHT OF COLUMN



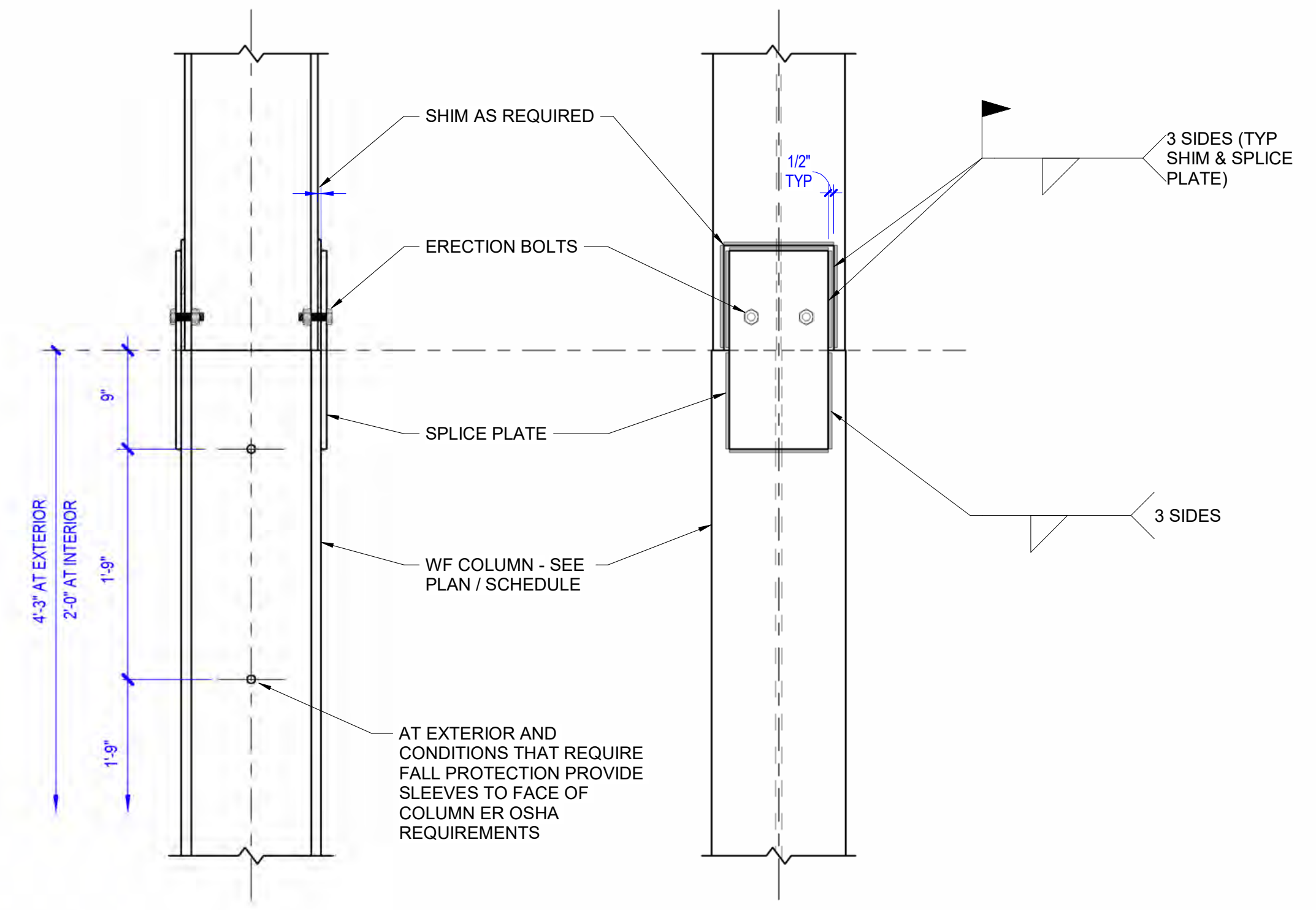
7 MC WF Bm - WF Col
3/4" = 1'-0"



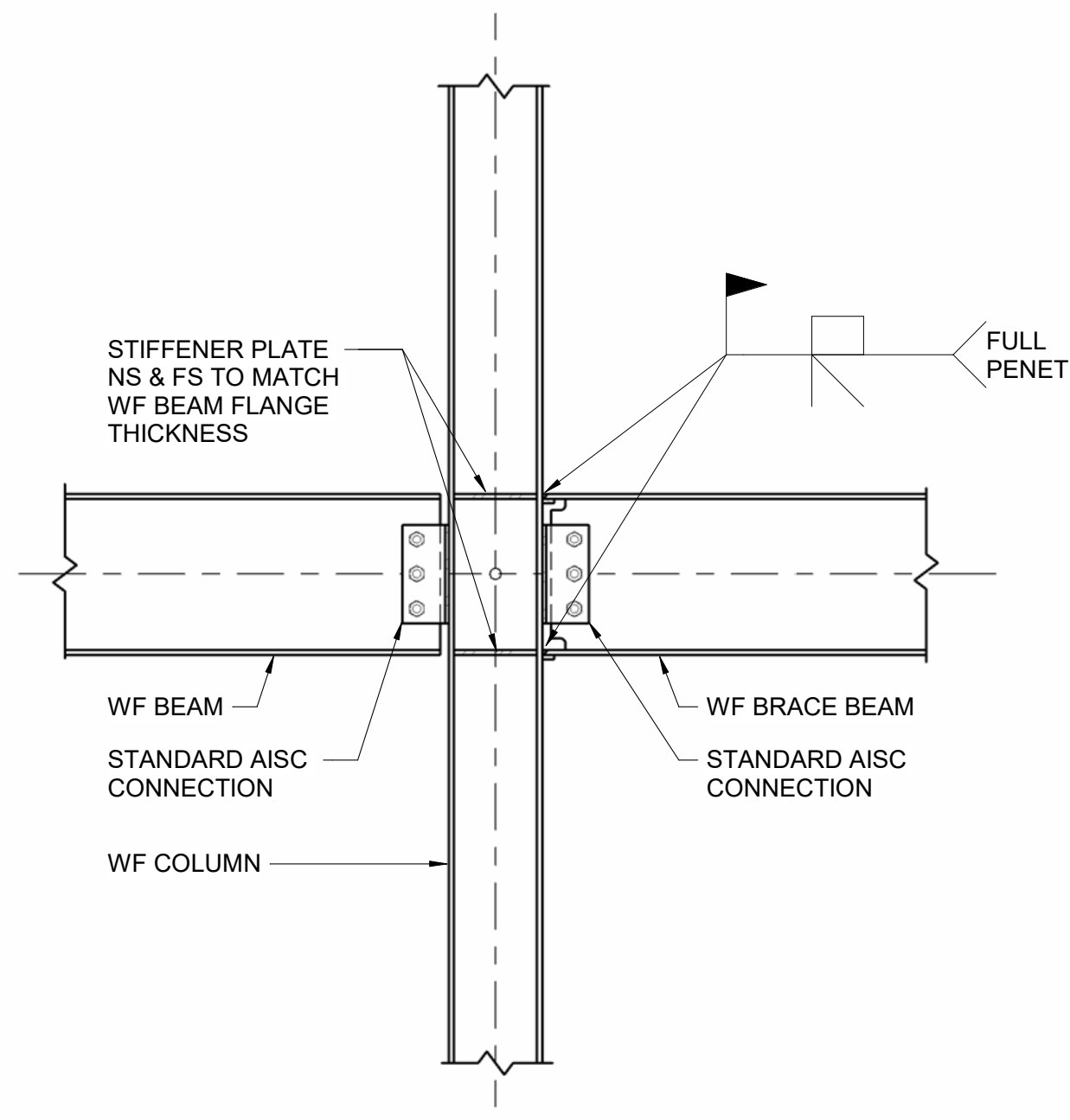
4 MC WF Bm - WF Col
3/4" = 1'-0"



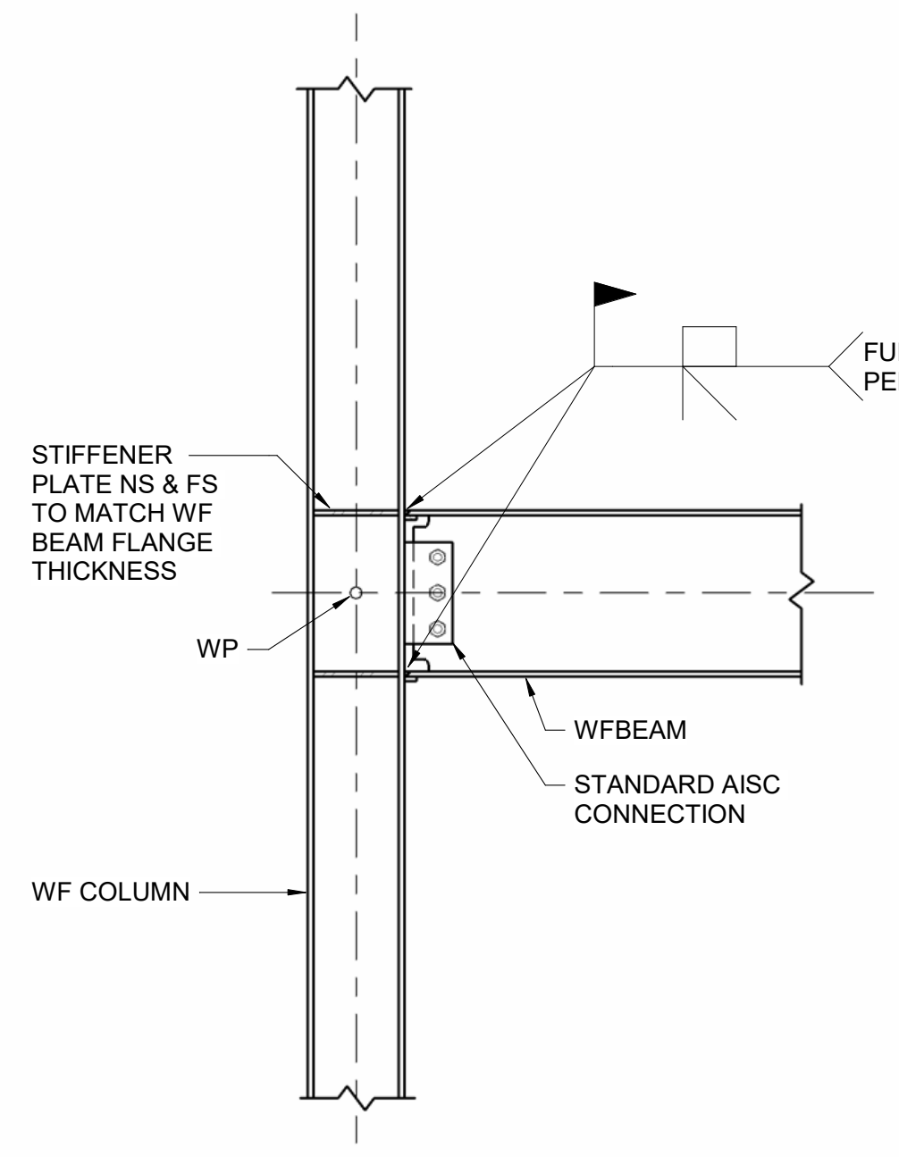
2 WF Beam to HSS Column Connection
1" = 1'-0"



10 WF Column Splice - Exterior Osha Saftey Cables
1" = 1'-0"

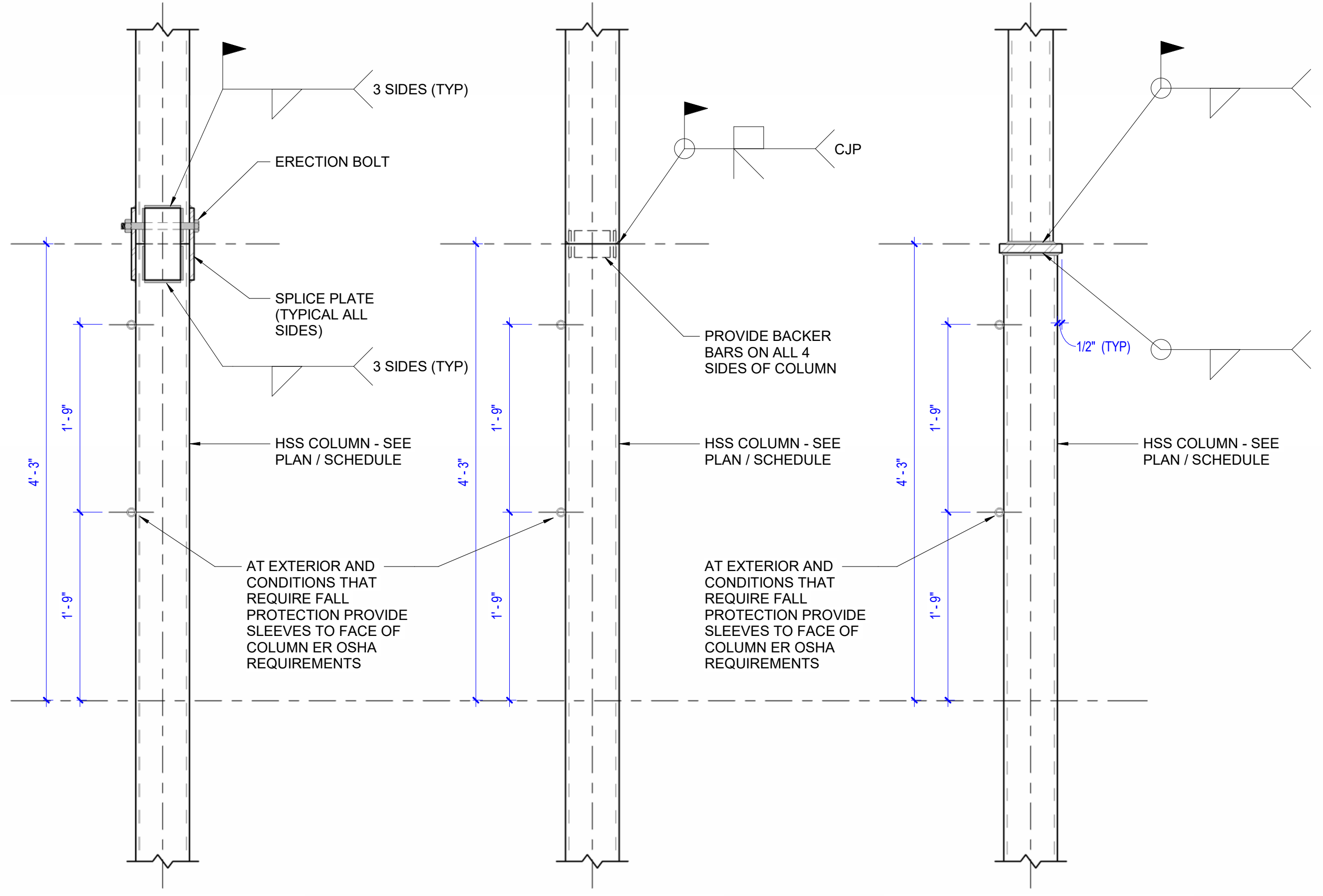


6 MC WF Bm - WF Col
3/4" = 1'-0"



3 MC WF Bm - WF Col
3/4" = 1'-0"

NOTE:
COLUMN SPLICES MUST BE DESIGNED TO RESIST A MINIMUM ECCENTRIC GRAVITY LOAD OF 300 POUNDS:
- LOCATED 18" FROM THE EXTREME OUTER FACE OF THE COLUMN
- IN EACH DIRECTION
- AT TOP OF COLUMN SHAFT

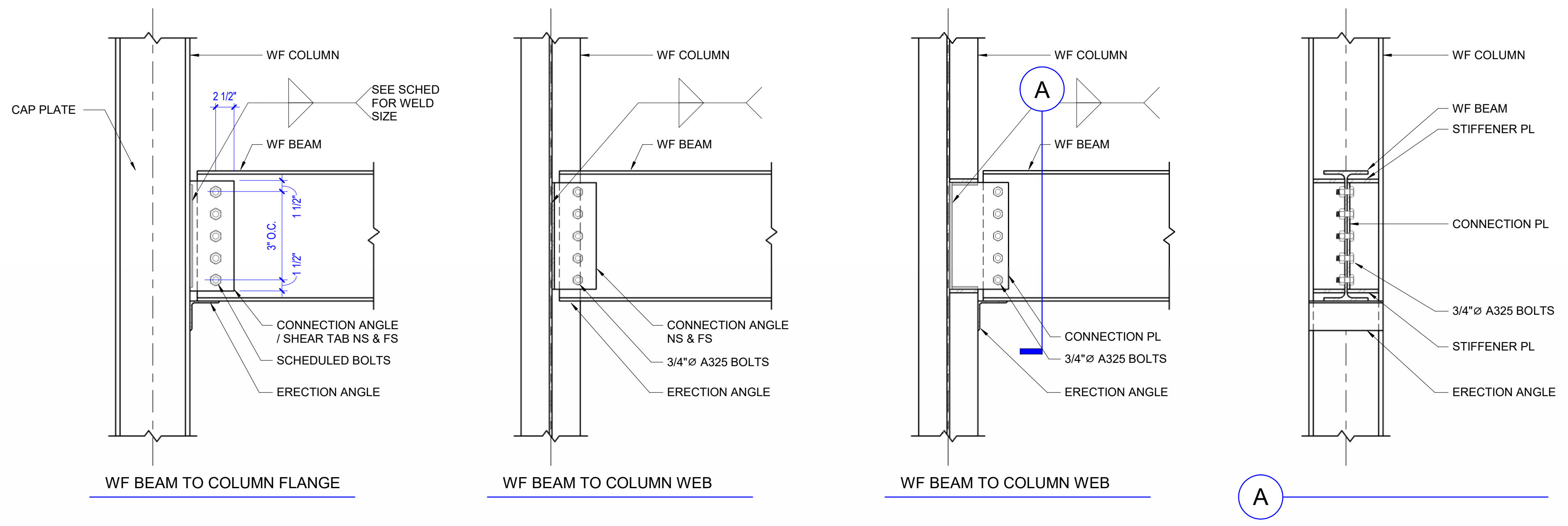


HSS COLUMN OF SAME SIZE

HSS COLUMN OF SAME SIZE

HSS COLUMN OF REDUCING SIZE

9 HSS Column Splice - Exterior Osha Saftey Cables
1" = 1'-0"



WF BEAM TO COLUMN FLANGE

WF BEAM TO COLUMN WEB

WF BEAM TO COLUMN WEB

1 WF Beam to WF Column Connection
1" = 1'-0"

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- NO. 85617

SHEET TITLE:
Typical Steel Details

SHEET NUMBER:
S4.420

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f'c = XXXX PSI

COLUMN ID LEGEND												COLUMN ID LEGEND
SIZE												SIZE
REINFORCEMENT												REINFORCEMENT
SIZE												SIZE
REINFORCEMENT												REINFORCEMENT
SIZE												SIZE
REINFORCEMENT												REINFORCEMENT
SIZE												SIZE
REINFORCEMENT												REINFORCEMENT
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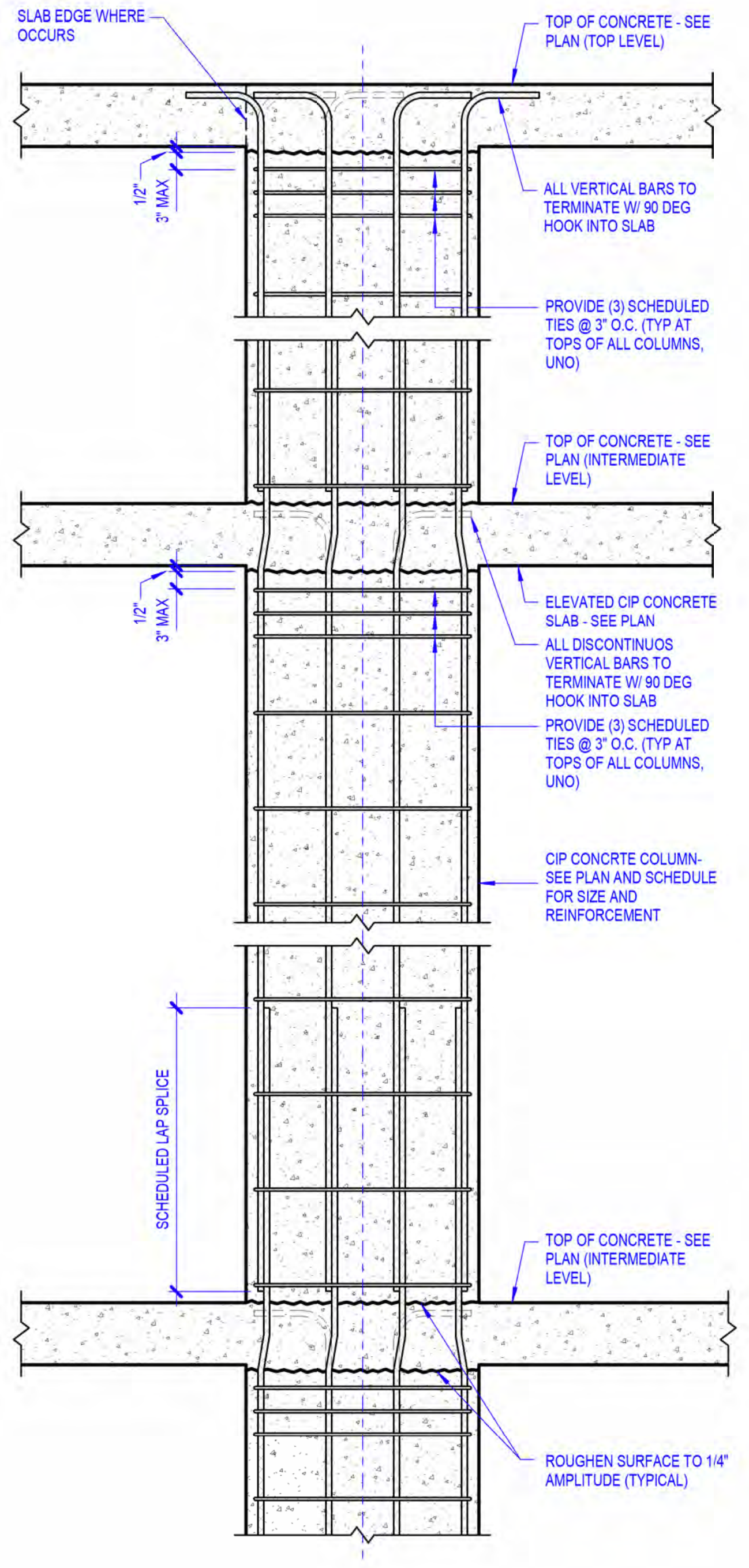
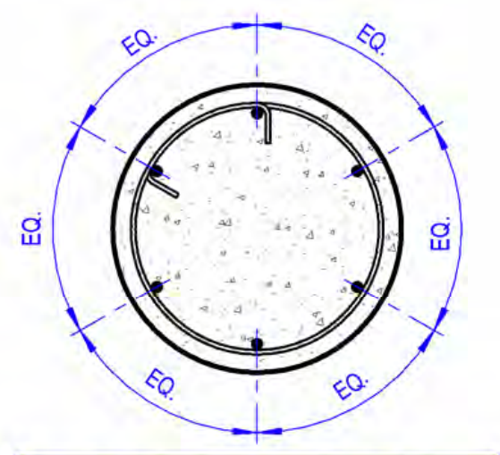
SIZE												SIZE
REINFORCEMENT												REINFORCEMENT
DOWELS TO FOUNDATION												DOWELS TO FOUNDATION
LAP SPLICE												LAP SPLICE
LEGEND COLUMN ID												LEGEND COLUMN ID

f'c = XXXX PSI

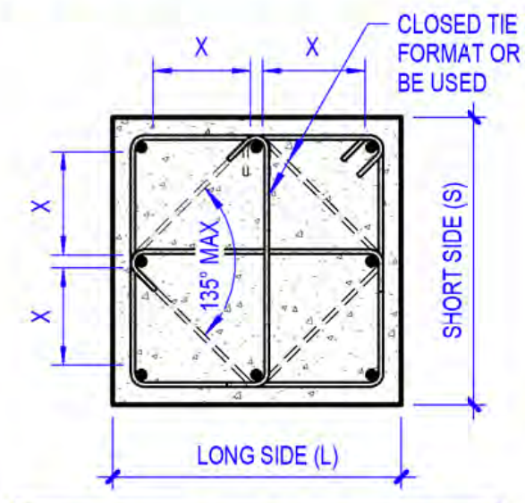
CIP-Column Size Chart -...	
Type	Count


SLAB EDGE WHERE OCCURS
TOP OF CONCRETE - SEE PLAN (TOP LEVEL)
1/2"
3" MAX
ALL VERTICAL BARS TO TERMINATE W/ 90 DEG HOOK INTO SLAB
PROVIDE (3) SCHEDULED TIES @ 3" O.C. (TYP AT TOPS OF ALL COLUMNS, UNO)
TOP OF CONCRETE - SEE PLAN (INTERMEDIATE LEVEL)
1/2"
3" MAX
ELEVATED CIP CONCRETE SLAB - SEE PLAN
ALL DISCONTINUOS VERTICAL BARS TO TERMINATE W/ 90 DEG HOOK INTO SLAB
PROVIDE (3) SCHEDULED TIES @ 3" O.C. (TYP AT TOPS OF ALL COLUMNS, UNO)
CIP CONCRTE COLUMN- SEE PLAN AND SCHEDULE FOR SIZE AND REINFORCEMENT
SCHEDULED LAP SPLICE
TOP OF CONCRETE - SEE PLAN (INTERMEDIATE LEVEL)
ROUGHEN SURFACE TO 1/4" AMPLITUDE (TYPICAL)

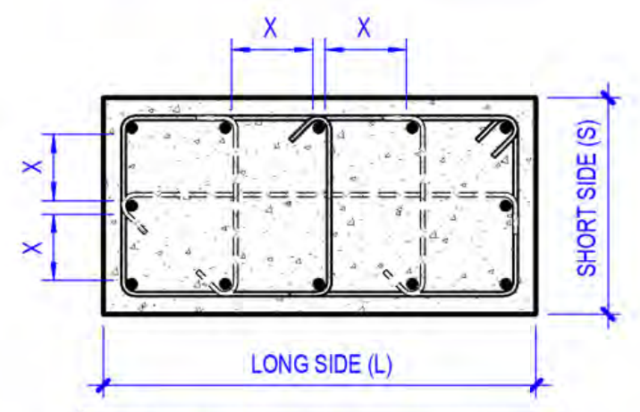


(6) #8(6/RND) W/ #3 @ 3" O.C.

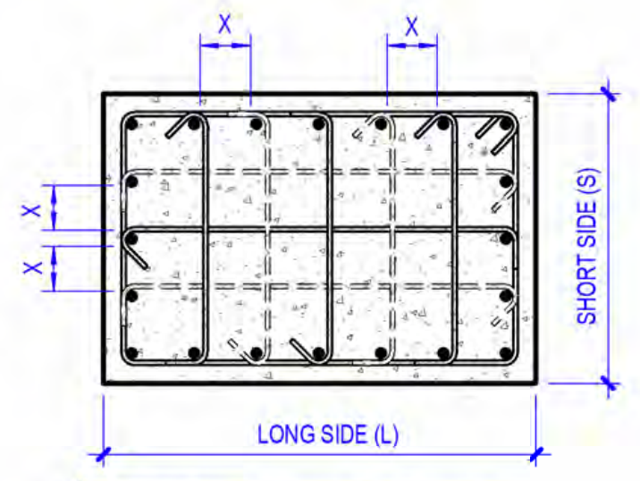
(6) #8(6/RND) W/ #3(SPR) @ 3" O.C.



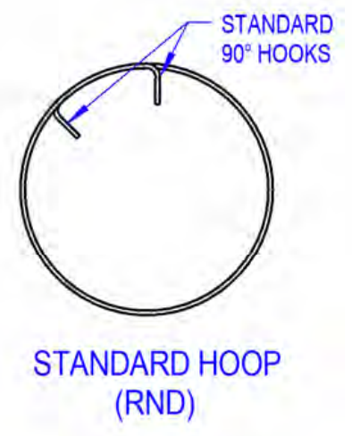
(8) #8 (3/SQ) W/ #3 @ 16" O.C.



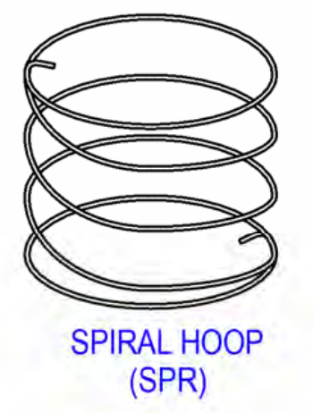
(12) #8 (5/3) W/ #3 @ 16" O.C.



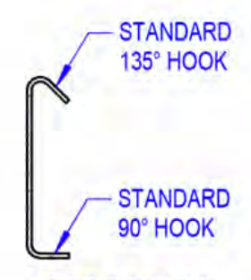
(20) #8 (7/5) W/ #3 @ 16" O.C.



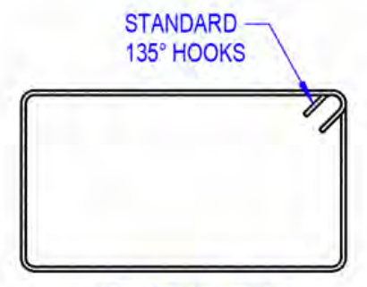
STANDARD HOOP (RND)



SPIRAL HOOP (SPR)



CROSS TIES



CLOSED TIES

COLUMN REINFORCEMENT DIAGRAM

(X) #X (X/X) W/ #X @ X" O.C.

- (X): TOTAL NUMBER OF BARS
- #X: BAR SIZE
- (X/X): NUMBER OF BARS LONG SIDE (L) / NUMBER OF BARS SHORT SIDE (S)
- (X) #X (X/X): VERTICAL REINFORCEMENT
- W/ #X @ X" O.C.: TIES
- #X: BAR SIZE
- X" O.C.: VERTICAL TIE SPACING

NOTE:

1. OUTER TIE TO BE FULLY CLOSED TO CONFINE COLUMN VERTICAL REINFORCEMENT W/ 135 DEGREE HOOKS AS SHOWN. ADDITIONAL LEGS OF TIES NOTED TO BE SINGLE LEG TIES WITH 90 DEGREE HOOKS ONE END AND 135 DEGREE HOOK OPPOSITE END. SINGLE LEG TIES TO ALTERNATE DIRECTION SO THAT 90 DEGREE HOOKS ARE NOT PLACED SIDE BY SIDE.
2. AT ROUND COLUMNS, BARS TO BE EQUALLY SPACED AROUND CIRCUMFERENCE OF COLUMN (X/RND).
3. AT SQUARE COLUMNS, BARS TO BE SYMETRICALLY PLACED ON EACH FACE (X/SQ).
4. 4. RECTANGULAR COLUMN VERTICAL BAR ARRANGEMENT (X/X) TO BE SPACED ALONG THE DESIGNATED FACE EQUALLY.
5. ALL COLUMN REINFORCEMENT TO BE DETAILED IN ACCORDANCE WITH ACI 318 (LATEST EDITION)
6. VERTICAL BARS MUST BE TIED AS SHOWN BY DASHED LINES WHEN CLEAR SPACING (X) GREATER THAN 6"
7. ALTERNATE CROSS TIE HOOK PLACEMENT WHEN ADJACENT
8. THE HOOKS ON STANDARD CIRCULAR HOOPS (STD) SHALL OVERLAP A MINIMUM OF 6" AND EACH MUST ENGAGE A LONGITUDINAL COLUMN BAR. ADJACENT CIRCULAR HOOPS SHOULD NOT ENGAGE THE SAME LONGITUDINAL BAR WITH END HOOKS.
9. MINIMUM COVER TO TIES IS 1 5/8", U.N.O.
10. ALL COLUMN VERTICAL REINFORCEMENT LAP SPLICES TO BE CLASS A (ld = 1.0) UNLESS NOTED DIFFERENTLY IN SCHEDULE. REFERENCE LAP SCHEDULES IN GENERAL NOTES.
11. CONTRACTOR MAY CHOOSE TO PROVIDE COLUMN SPLICES AT EVERY OTHER STORY IN LIEU OF SINGLE STORY HEIGHTS. VERTICAL REINFORCEMENT AND SPLICE REQUIREMENTS SHALL BE THE GREATER OF THE TWO STORIES GIVEN IN THE COLUMN SCHEDULE. TIES MUST STILL BE PROVIDED PER TYPICAL DETAIL AT EACH FLOOR.
12. PROVIDE MECHANICAL COUPLER SPLICES ON VERTICAL REINFORCEMENT WHEN NOTED IN SCHEDULE OR BASED ON BAR SIZE PER THE PROJECT GENERAL NOTES. CONTRACTOR MAY ALSO CHOOSE TO PROVIDE MECHANICAL COUPLER SPLICES TO REDUCE CONGESTION.
13. DOUBLE HEIGHT COLUMNS SHALL BE PLACED IN A SINGLE POUR (NO CONSTRUCTION JOINT IN MIDDLE OF COLUMN HEIGHT).
14. COLUMNS SHOWN ON PLAN ARE CENTERED ON GRID LINES U.N.O.

CIP Column Notes / Diagram
3/4" = 1'-0"

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CIP Concrete Column Schedule

SHEET NUMBER:
S5.100

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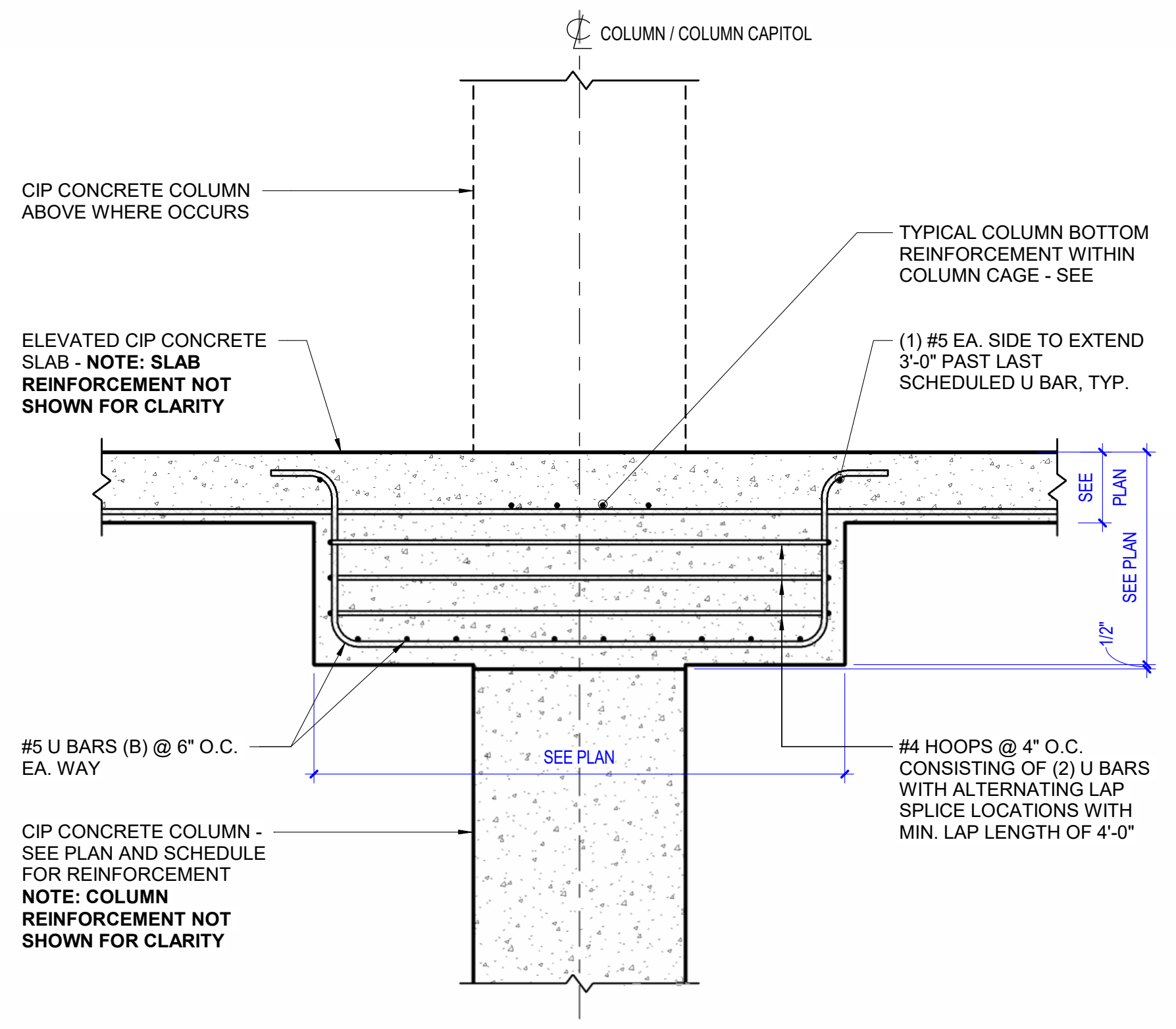


CIP CONCRETE COLUMN ABOVE WHERE OCCURS

ELEVATED CIP CONCRETE SLAB - **NOTE: SLAB REINFORCEMENT NOT SHOWN FOR CLARITY**

TYPICAL COLUMN BOTTOM REINFORCEMENT WITHIN COLUMN CAGE - SEE

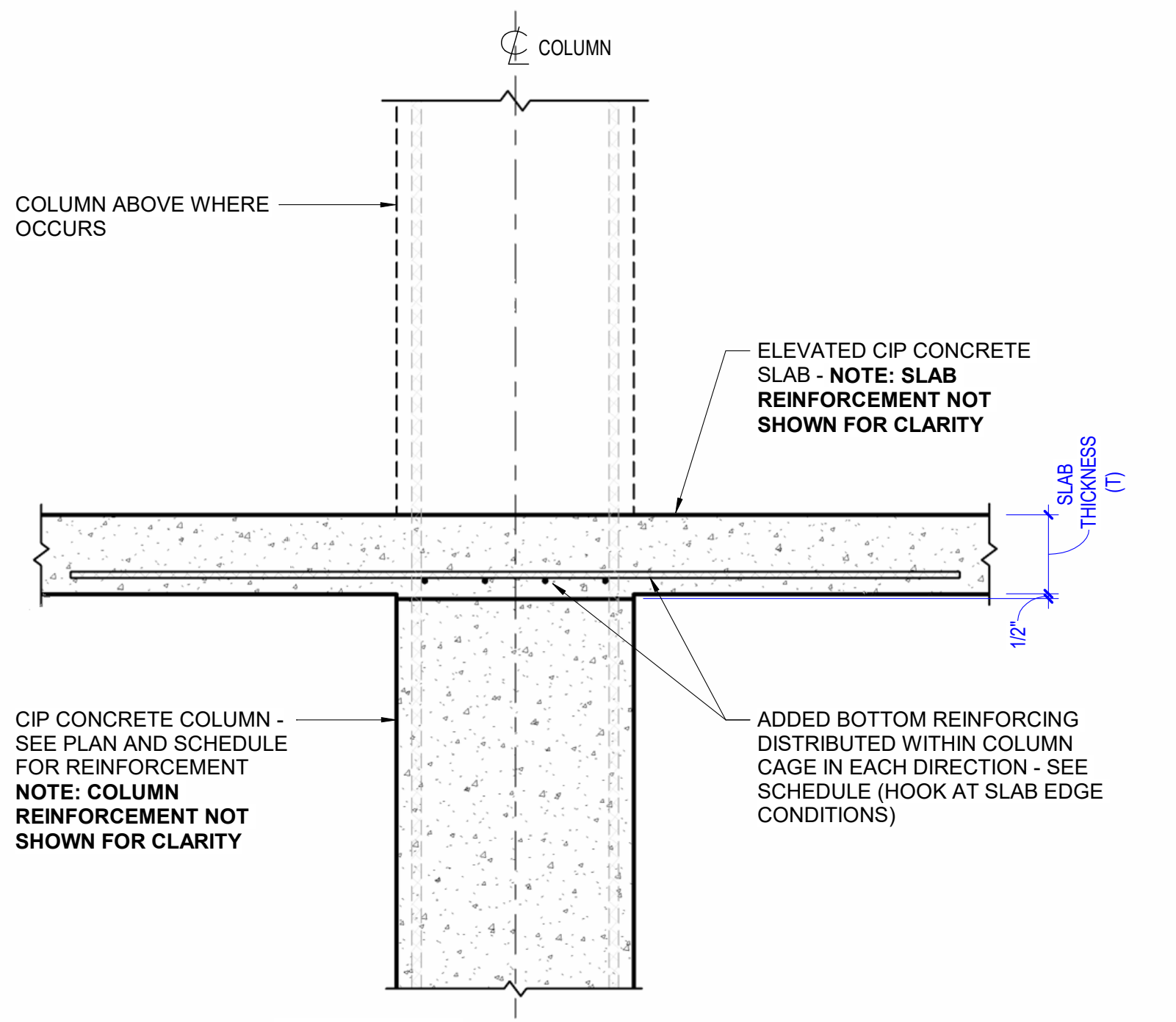
(1) #5 EA. SIDE TO EXTEND 3'-0" PAST LAST SCHEDULED U BAR, TYP.

#5 U BARS (B) @ 6" O.C. EA. WAY

#4 HOOPS @ 4" O.C. CONSISTING OF (2) U BARS WITH ALTERNATING LAP SPLICE LOCATIONS WITH MIN. LAP LENGTH OF 4'-0"

CIP CONCRETE COLUMN - SEE PLAN AND SCHEDULE FOR REINFORCEMENT **NOTE: COLUMN REINFORCEMENT NOT SHOWN FOR CLARITY**

2 Column Transition - Round Over Rectangular
3/4" = 1'-0"



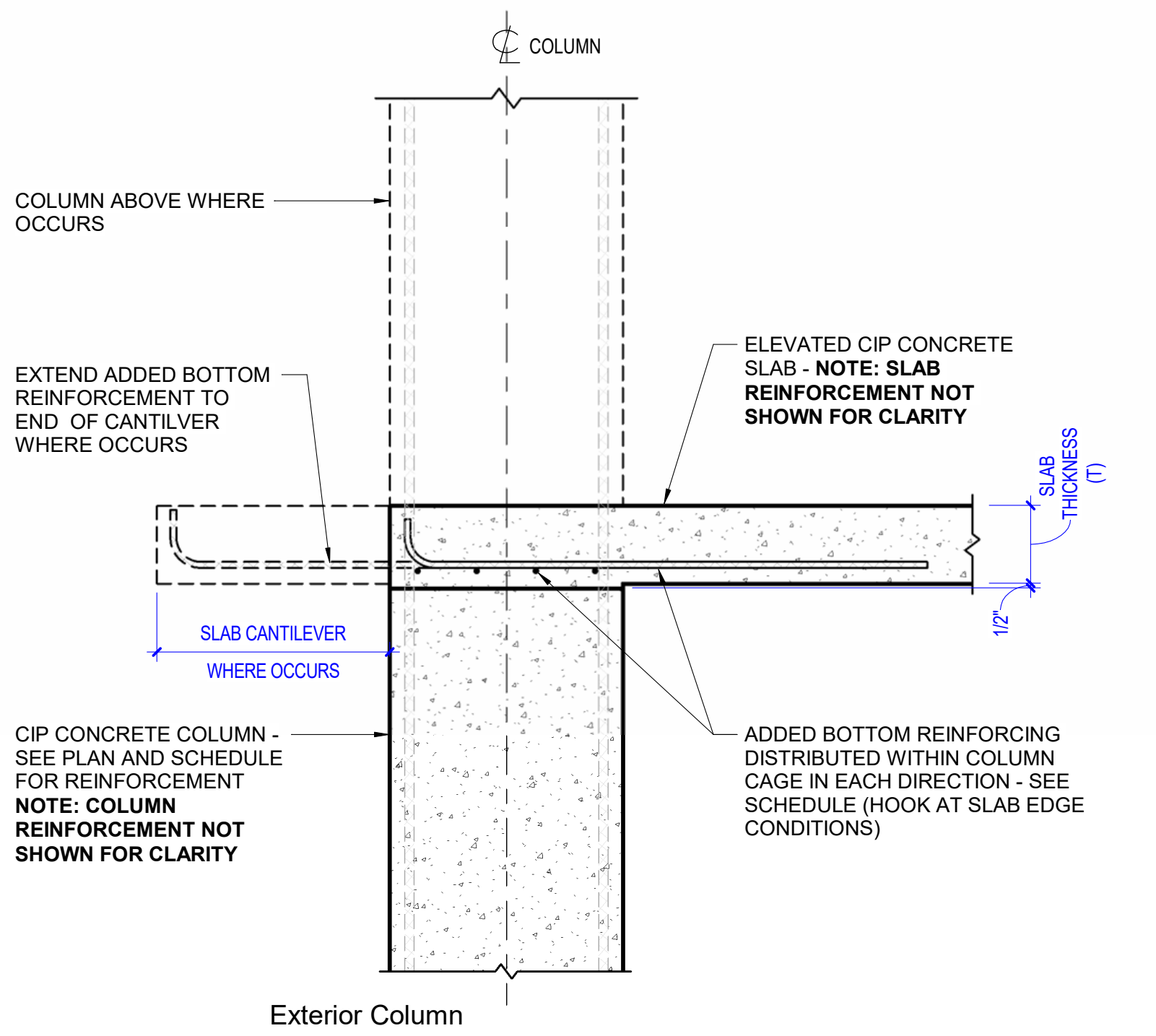
COLUMN ABOVE WHERE OCCURS

ELEVATED CIP CONCRETE SLAB - **NOTE: SLAB REINFORCEMENT NOT SHOWN FOR CLARITY**

CIP CONCRETE COLUMN - SEE PLAN AND SCHEDULE FOR REINFORCEMENT **NOTE: COLUMN REINFORCEMENT NOT SHOWN FOR CLARITY**

ADDED BOTTOM REINFORCING DISTRIBUTED WITHIN COLUMN CAGE IN EACH DIRECTION - SEE SCHEDULE (HOOK AT SLAB EDGE CONDITIONS)

Interior Column



COLUMN ABOVE WHERE OCCURS

EXTEND ADDED BOTTOM REINFORCEMENT TO END OF CANTILVER WHERE OCCURS

ELEVATED CIP CONCRETE SLAB - **NOTE: SLAB REINFORCEMENT NOT SHOWN FOR CLARITY**

CIP CONCRETE COLUMN - SEE PLAN AND SCHEDULE FOR REINFORCEMENT **NOTE: COLUMN REINFORCEMENT NOT SHOWN FOR CLARITY**

ADDED BOTTOM REINFORCING DISTRIBUTED WITHIN COLUMN CAGE IN EACH DIRECTION - SEE SCHEDULE (HOOK AT SLAB EDGE CONDITIONS)

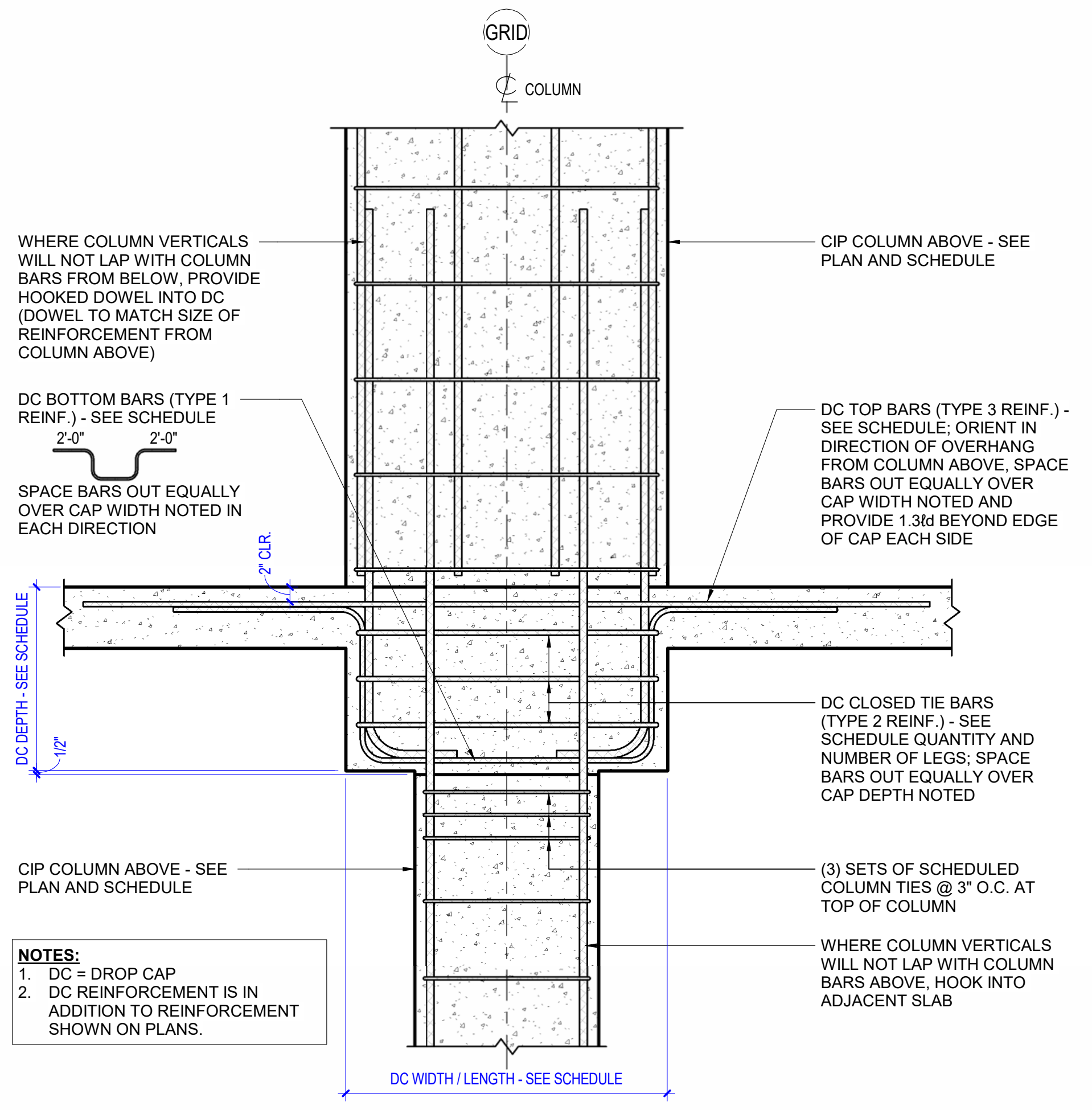
Exterior Column

BOTTOM REINFORCEMENT SCHEDULE 3	
SLAB THICKNESS (INCHES)	BOTTOM BARS 1,2 EA. WAY
8	(3) #5 X 8'-6"
10	(4) #5 X 8'-6"
12	(3) #6 X 10'-6"
14	(4) #6 X 10'-6"
16	(3) #7 X 13'-6"
18	(4) #7 X 13'-6"
20	(4) #7 X 13'-6"
22	(4) #8 X 15'-6"
24	(4) #8 X 15'-6"

SCHEDULE NOTES:

1. COLUMN WIDTH IS 24". FOR COLUMNS WIDTHS GREATER THAN 24", PROVIDE (1) ADDITIONAL BOTTOM BAR FOR EVERY 6" OF ADDITIONAL WIDTH IN THE DIRECTION OF INCREASED WIDTH ONLY (TYPICAL)
2. SPECIFIED CONCRETE STRENGTH IS 5,000 PSI. FOR SPECIFIED CONCRETE STRENGTH GREATER THAN 5,000 PSI, PROVIDE (1) ADDITIONAL BOTTOM BAR IN EACH DIRECTION FOR EVERY 1,000 PSI OF INCREASED CONCRETE STRENGTH, TYP.
3. REINFORCEMENT YIELD STRENGTH IS 60,000 PSI.

1 Typical Bottom Reinforcement at Columns and Ends of Shear Walls
3/4" = 1'-0"



WHERE COLUMN VERTICALS WILL NOT LAP WITH COLUMN BARS FROM BELOW, PROVIDE HOOKED DOWEL INTO DC (DOWEL TO MATCH SIZE OF REINFORCEMENT FROM COLUMN ABOVE)

DC BOTTOM BARS (TYPE 1 REINF.) - SEE SCHEDULE

SPACE BARS OUT EQUALLY OVER CAP WIDTH NOTED IN EACH DIRECTION

CIP COLUMN ABOVE - SEE PLAN AND SCHEDULE

DC TOP BARS (TYPE 3 REINF.) - SEE SCHEDULE; ORIENT IN DIRECTION OF OVERHANG FROM COLUMN ABOVE, SPACE BARS OUT EQUALLY OVER CAP WIDTH NOTED AND PROVIDE 1.3ℓd BEYOND EDGE OF CAP EACH SIDE

DC CLOSED TIE BARS (TYPE 2 REINF.) - SEE SCHEDULE QUANTITY AND NUMBER OF LEGS; SPACE BARS OUT EQUALLY OVER CAP DEPTH NOTED

(3) SETS OF SCHEDULED COLUMN TIES @ 3" O.C. AT TOP OF COLUMN

CIP COLUMN ABOVE - SEE PLAN AND SCHEDULE

WHERE COLUMN VERTICALS WILL NOT LAP WITH COLUMN BARS ABOVE, HOOK INTO ADJACENT SLAB

NOTES:
1. DC = DROP CAP
2. DC REINFORCEMENT IS IN ADDITION TO REINFORCEMENT SHOWN ON PLANS.

DROP CAP AT COLUMN SIZE CHANGE OR ROTATION

DROP CAP (DC) SCHEDULE

ID	DC LENGTH (INCHES)	DC WIDTH (INCHES)	DC DEPTH (INCHES)	TYPE 1 REINF.	TYPE 2 REINF.	TYPE 3 REINF.	DC NOTES
DC-A	42	36	24	(4) #5	(3) #5 - 4 LEG	(9) #7	
DC-B	54	42	22	(5) #5	(3) #5 - 4 LEG	(9) #7	
DC-C	42	36	24	(4) #5	(3) #5 - 2 LEG	(6) #7	
DC-D	36	36	22	(4) #5	(3) #5 - 2 LEG	(5) #7	
DC-E	42	42	24	(5) #5	(3) #5 - 4 LEG	(9) #7	
DC-F	42	28	24	(4) #5	(3) #5 - 4 LEG	(9) #7	
DC-G	42	42	18	(5) #5	(3) #5 - 4 LEG	(7) #7	

3 Typical Drop Cap at Column Rotation
3/4" = 1'-0"

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PROJECT # : 26154
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No.	Description	Date
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BATON ROUGE, LOUISIANA 70808
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SHEET TITLE:
Typical CIP Concrete Column Details

SHEET NUMBER:
S5.110

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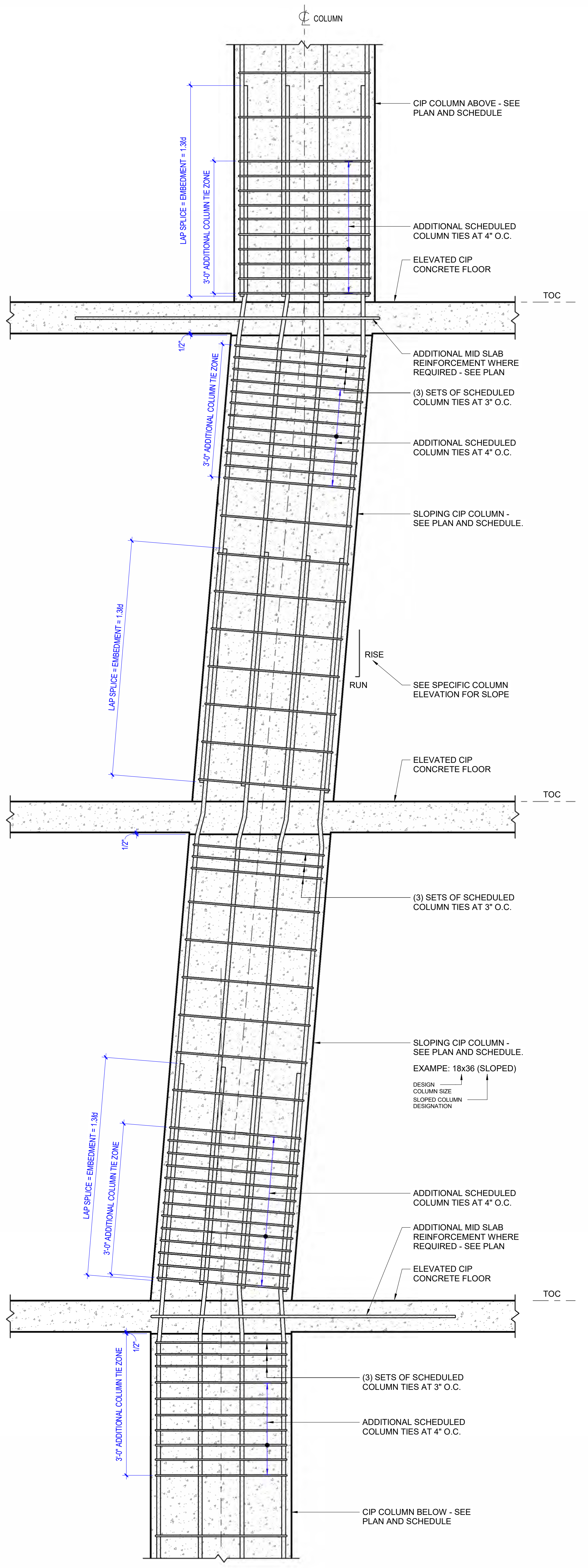
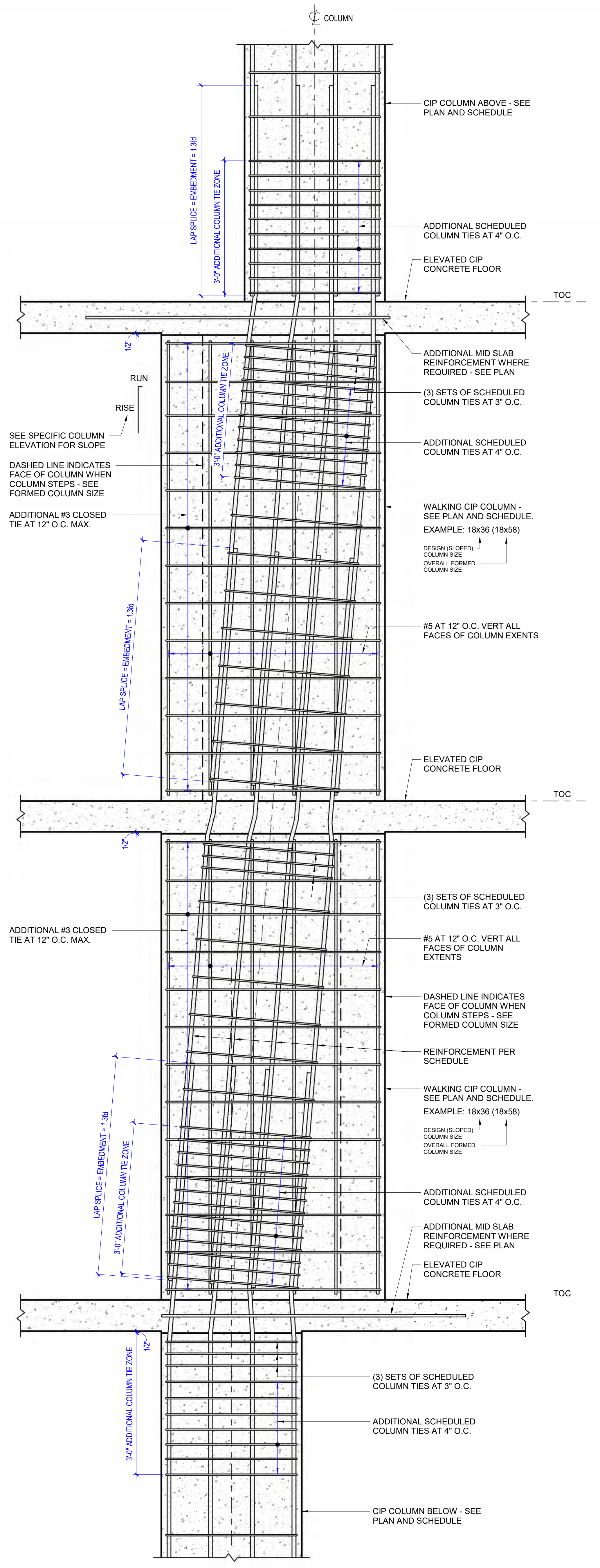

℄ COLUMN
CIP COLUMN ABOVE - SEE PLAN AND SCHEDULE
LAP SPLICE = EMBEDMENT = 1.3ℓd
3'-0" ADDITIONAL COLUMN TIE ZONE
ADDITIONAL SCHEDULED COLUMN TIES AT 4" O.C.
ELEVATED CIP CONCRETE FLOOR
TOC
1/2"
ADDITIONAL MID SLAB REINFORCEMENT WHERE REQUIRED - SEE PLAN
(3) SETS OF SCHEDULED COLUMN TIES AT 3" O.C.
3'-0" ADDITIONAL COLUMN TIE ZONE
ADDITIONAL SCHEDULED COLUMN TIES AT 4" O.C.
SLOPING CIP COLUMN - SEE PLAN AND SCHEDULE.
LAP SPLICE = EMBEDMENT = 1.3ℓd
RISE
RUN
SEE SPECIFIC COLUMN ELEVATION FOR SLOPE
ELEVATED CIP CONCRETE FLOOR
TOC
1/2"
(3) SETS OF SCHEDULED COLUMN TIES AT 3" O.C.
SLOPING CIP COLUMN - SEE PLAN AND SCHEDULE.
EXAMPE: 18x36 (SLOPED)
DESIGN COLUMN SIZE
SLOPED COLUMN DESIGNATION
LAP SPLICE = EMBEDMENT = 1.3ℓd
3'-0" ADDITIONAL COLUMN TIE ZONE
ADDITIONAL SCHEDULED COLUMN TIES AT 4" O.C.
ADDITIONAL MID SLAB REINFORCEMENT WHERE REQUIRED - SEE PLAN
ELEVATED CIP CONCRETE FLOOR
TOC
1/2"
3'-0" ADDITIONAL COLUMN TIE ZONE
(3) SETS OF SCHEDULED COLUMN TIES AT 3" O.C.
ADDITIONAL SCHEDULED COLUMN TIES AT 4" O.C.
CIP COLUMN BELOW - SEE PLAN AND SCHEDULE

① Typical CIP Sloping Column Detail
3/4" = 1'-0"


℄ COLUMN
CIP COLUMN ABOVE - SEE PLAN AND SCHEDULE
LAP SPLICE = EMBEDMENT = 1.3ℓd
3'-0" ADDITIONAL COLUMN TIE ZONE
ADDITIONAL SCHEDULED COLUMN TIES AT 4" O.C.
ELEVATED CIP CONCRETE FLOOR
TOC
1/2"
RUN
RISE
SEE SPECIFIC COLUMN ELEVATION FOR SLOPE
DASHED LINE INDICATES FACE OF COLUMN WHEN COLUMN STEPS - SEE FORMED COLUMN SIZE
ADDITIONAL #3 CLOSED TIE AT 12" O.C. MAX.
3'-0" ADDITIONAL COLUMN TIE ZONE
ADDITIONAL MID SLAB REINFORCEMENT WHERE REQUIRED - SEE PLAN
(3) SETS OF SCHEDULED COLUMN TIES AT 3" O.C.
ADDITIONAL SCHEDULED COLUMN TIES AT 4" O.C.
WALKING CIP COLUMN - SEE PLAN AND SCHEDULE.
EXAMPLE: 18x36 (18x58)
DESIGN (SLOPED) COLUMN SIZE
OVERALL FORMED COLUMN SIZE
LAP SPLICE = EMBEDMENT = 1.3ℓd
#5 AT 12" O.C. VERT ALL FACES OF COLUMN EXENTS
ELEVATED CIP CONCRETE FLOOR
TOC
1/2"
ADDITIONAL #3 CLOSED TIE AT 12" O.C. MAX.
(3) SETS OF SCHEDULED COLUMN TIES AT 3" O.C.
#5 AT 12" O.C. VERT ALL FACES OF COLUMN EXTENTS
DASHED LINE INDICATES FACE OF COLUMN WHEN COLUMN STEPS - SEE FORMED COLUMN SIZE
REINFORCEMENT PER SCHEDULE
WALKING CIP COLUMN - SEE PLAN AND SCHEDULE.
EXAMPLE: 18x36 (18x58)
DESIGN (SLOPED) COLUMN SIZE
OVERALL FORMED COLUMN SIZE
LAP SPLICE = EMBEDMENT = 1.3ℓd
3'-0" ADDITIONAL COLUMN TIE ZONE
ADDITIONAL SCHEDULED COLUMN TIES AT 4" O.C.
ADDITIONAL MID SLAB REINFORCEMENT WHERE REQUIRED - SEE PLAN
ELEVATED CIP CONCRETE FLOOR
TOC
1/2"
3'-0" ADDITIONAL COLUMN TIE ZONE
(3) SETS OF SCHEDULED COLUMN TIES AT 3" O.C.
ADDITIONAL SCHEDULED COLUMN TIES AT 4" O.C.
CIP COLUMN BELOW - SEE PLAN AND SCHEDULE

Typical CIP Walking Column Detail
3/4" = 1'-0"

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SHEET TITLE:
CIP Concrete Column Elevations

SHEET NUMBER:
S5.120

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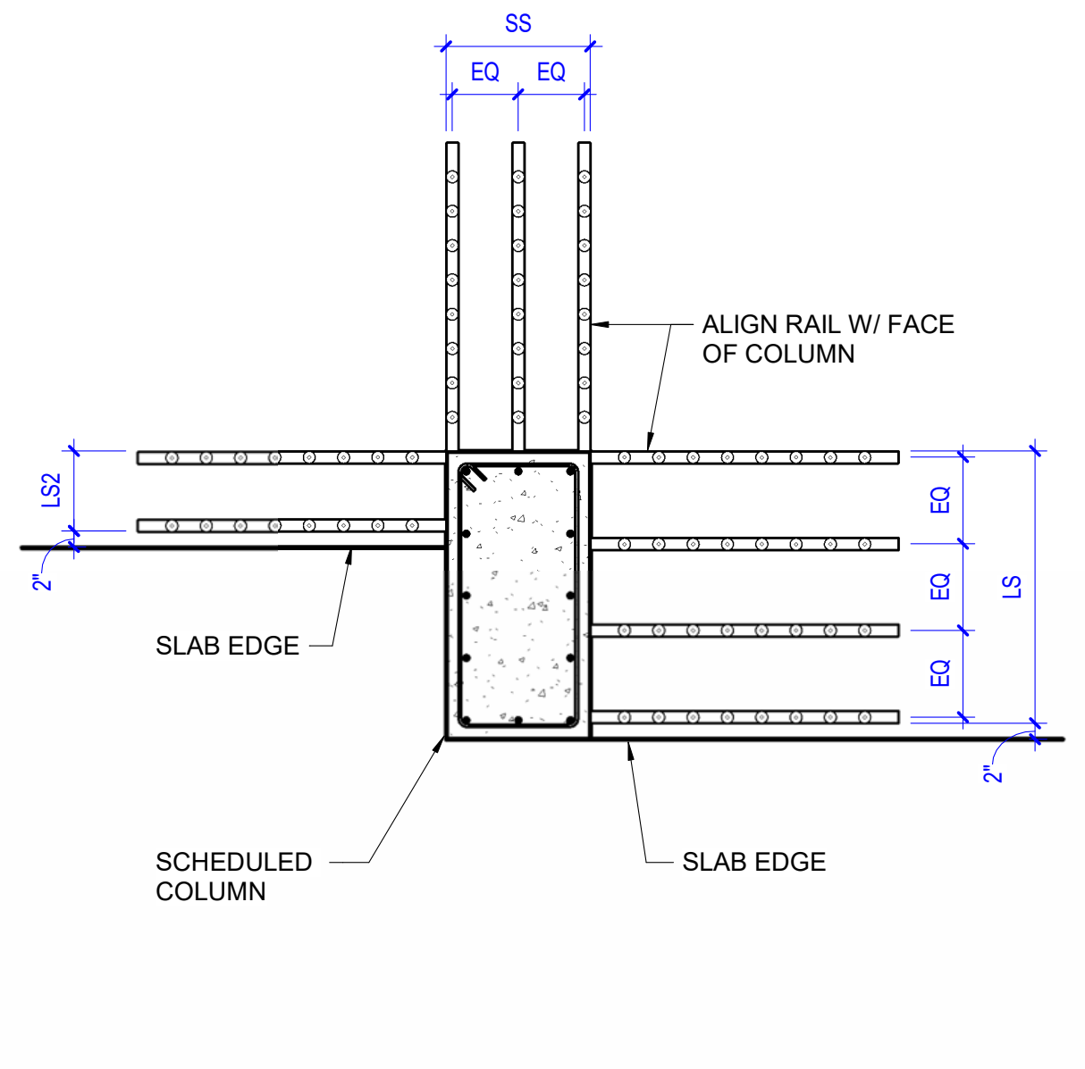
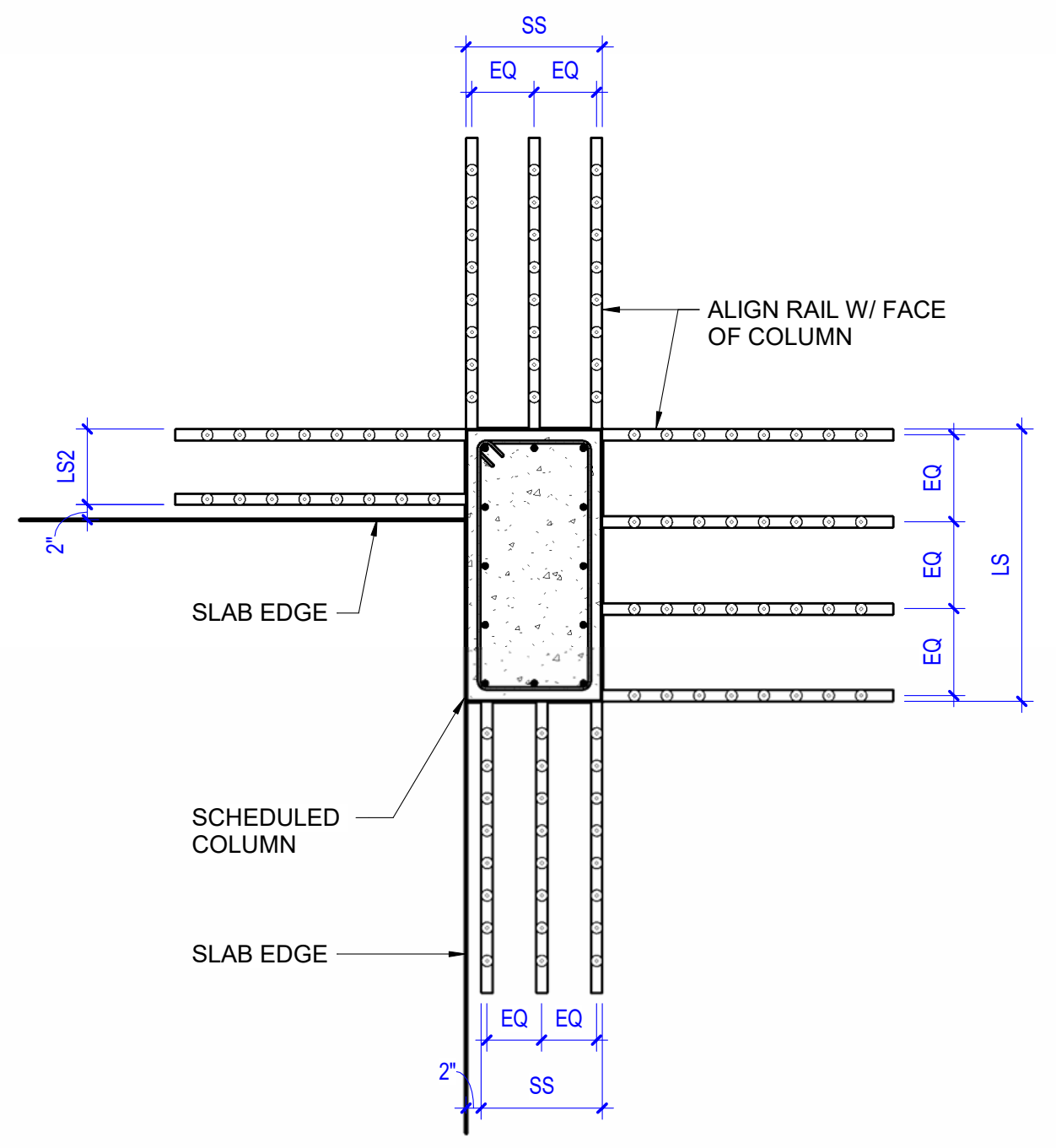
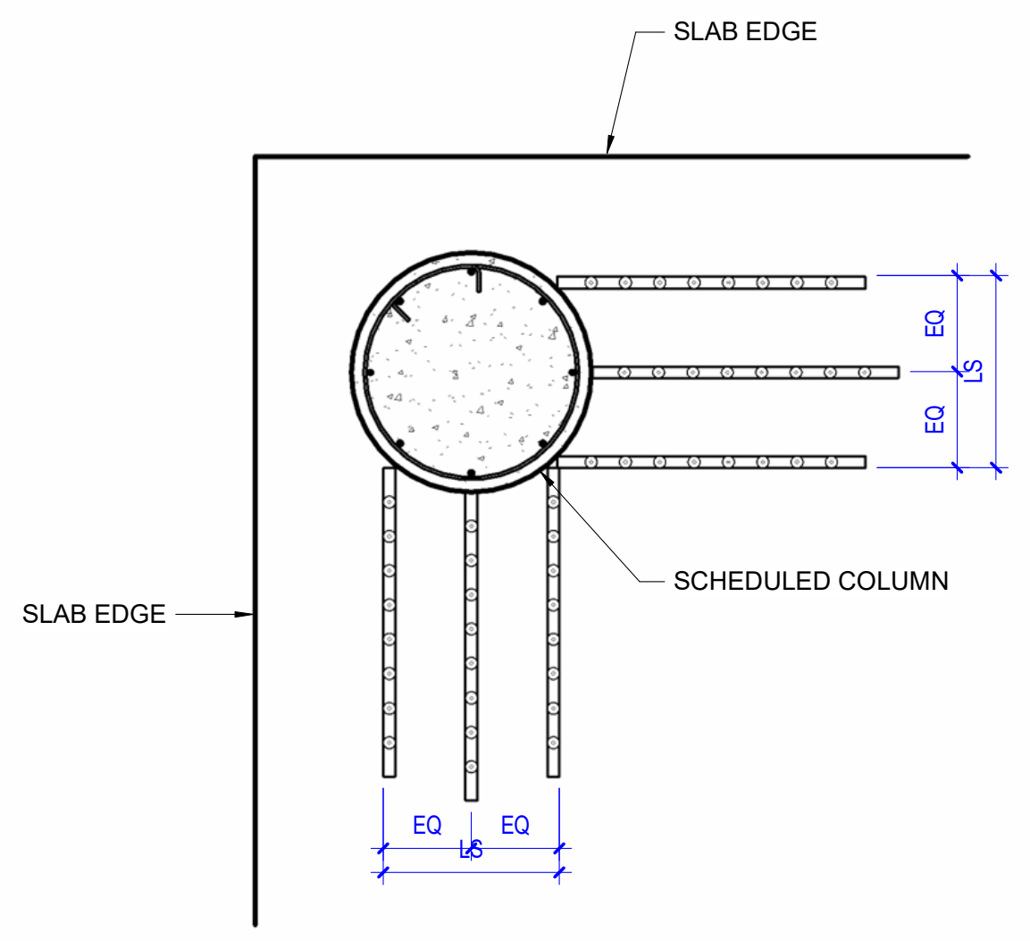


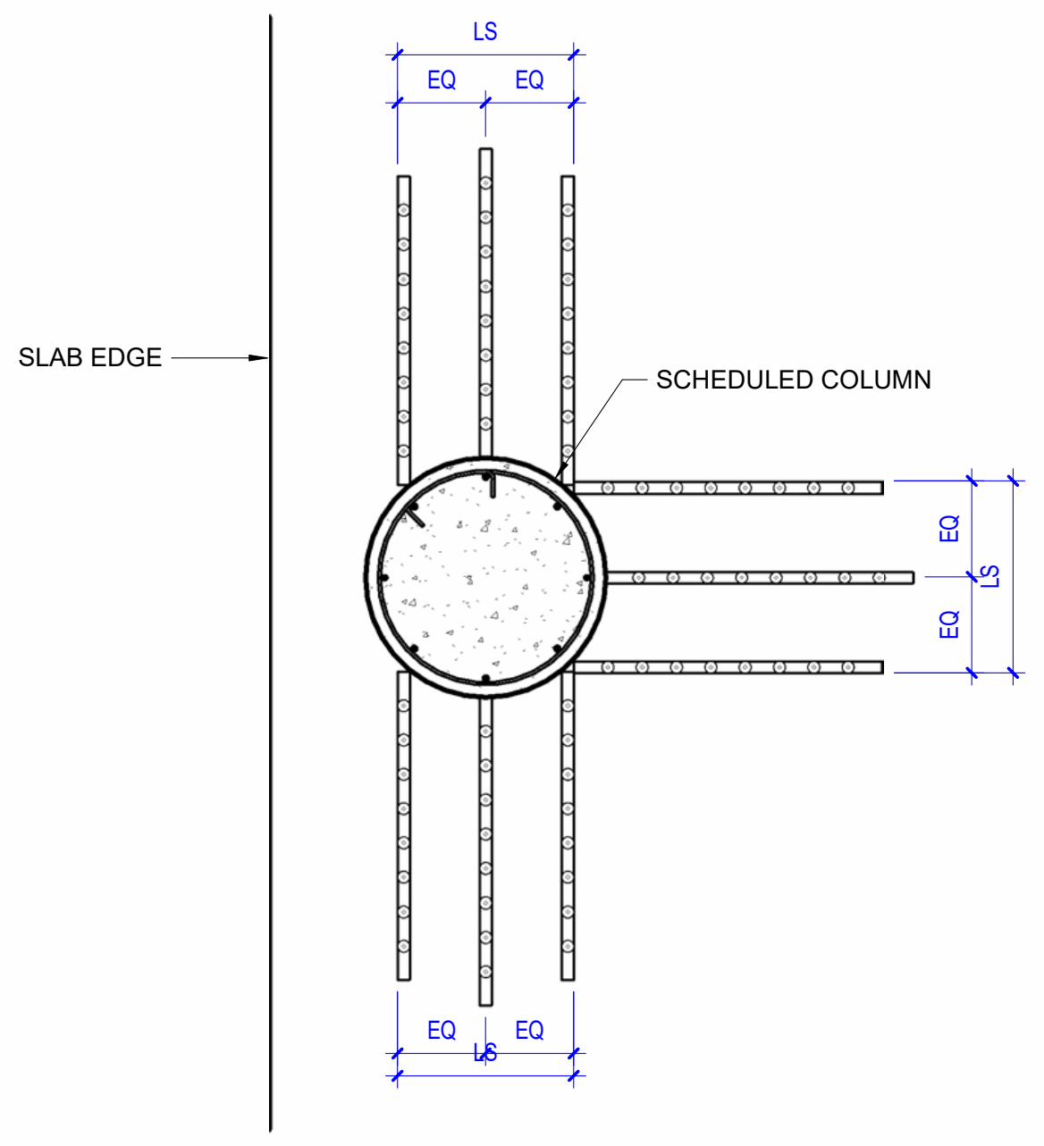
Rectangular Column - Exterior
1/2" = 1'-0"



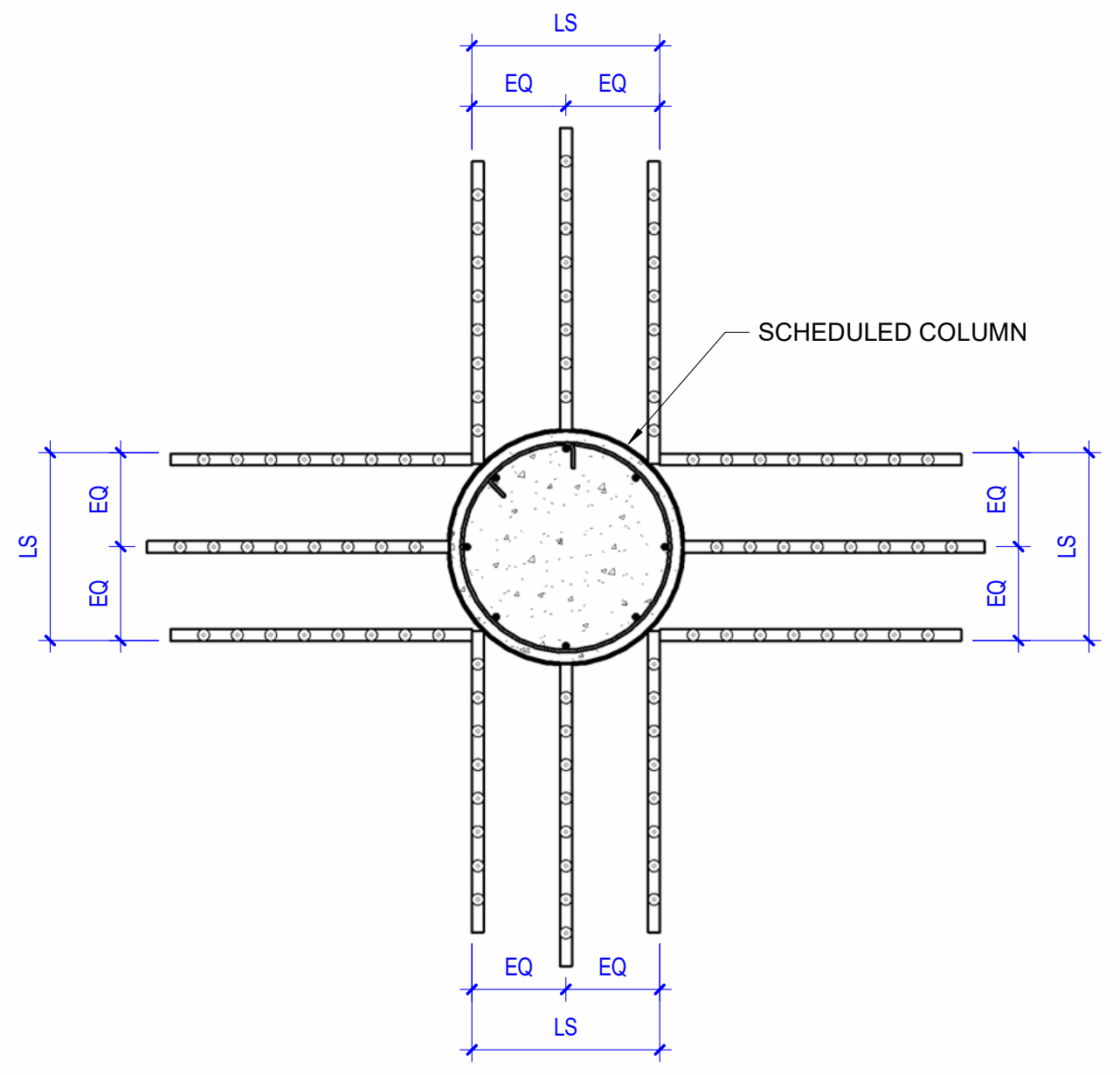
Rectangular Column - Exterior
1/2" = 1'-0"



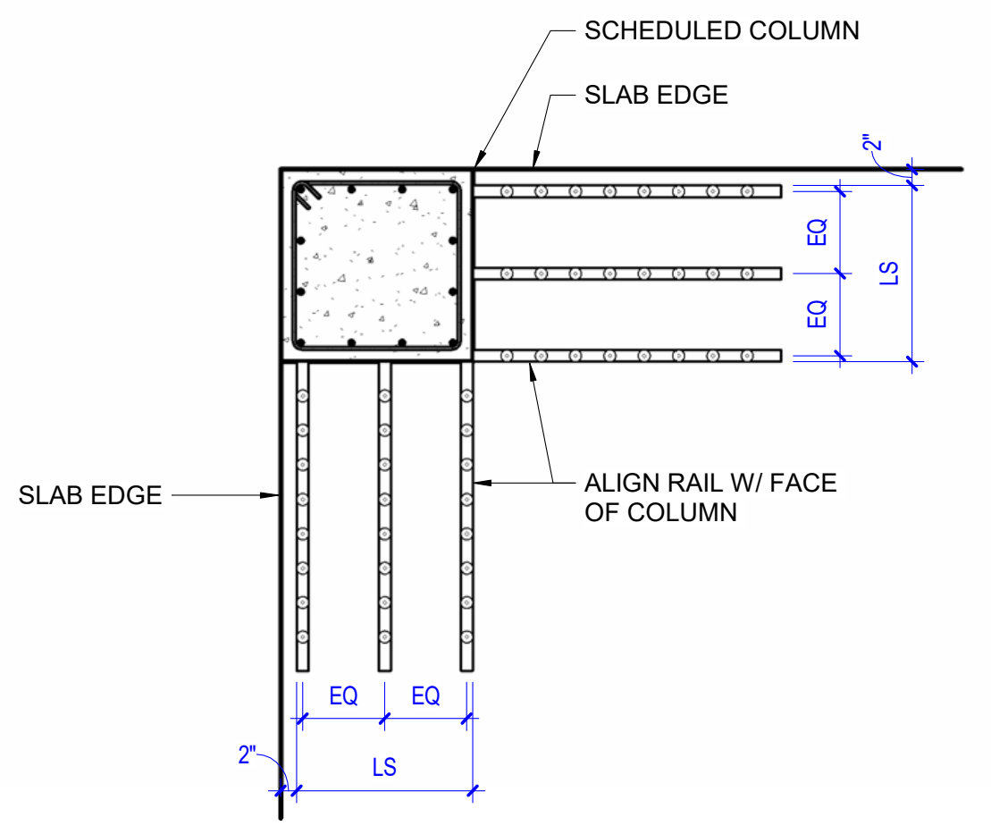
Square Column - Interior
1/2" = 1'-0"



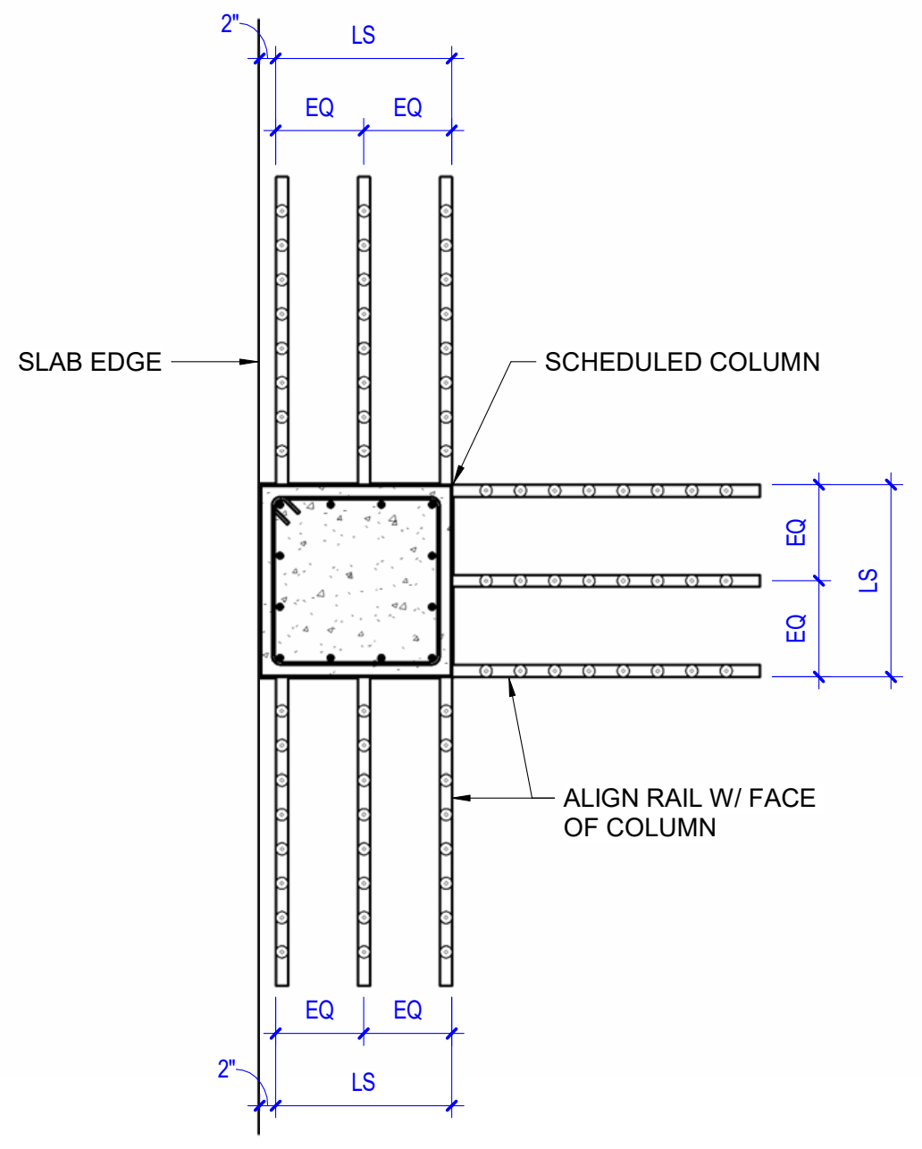
Round Column - Exterior
1/2" = 1'-0"



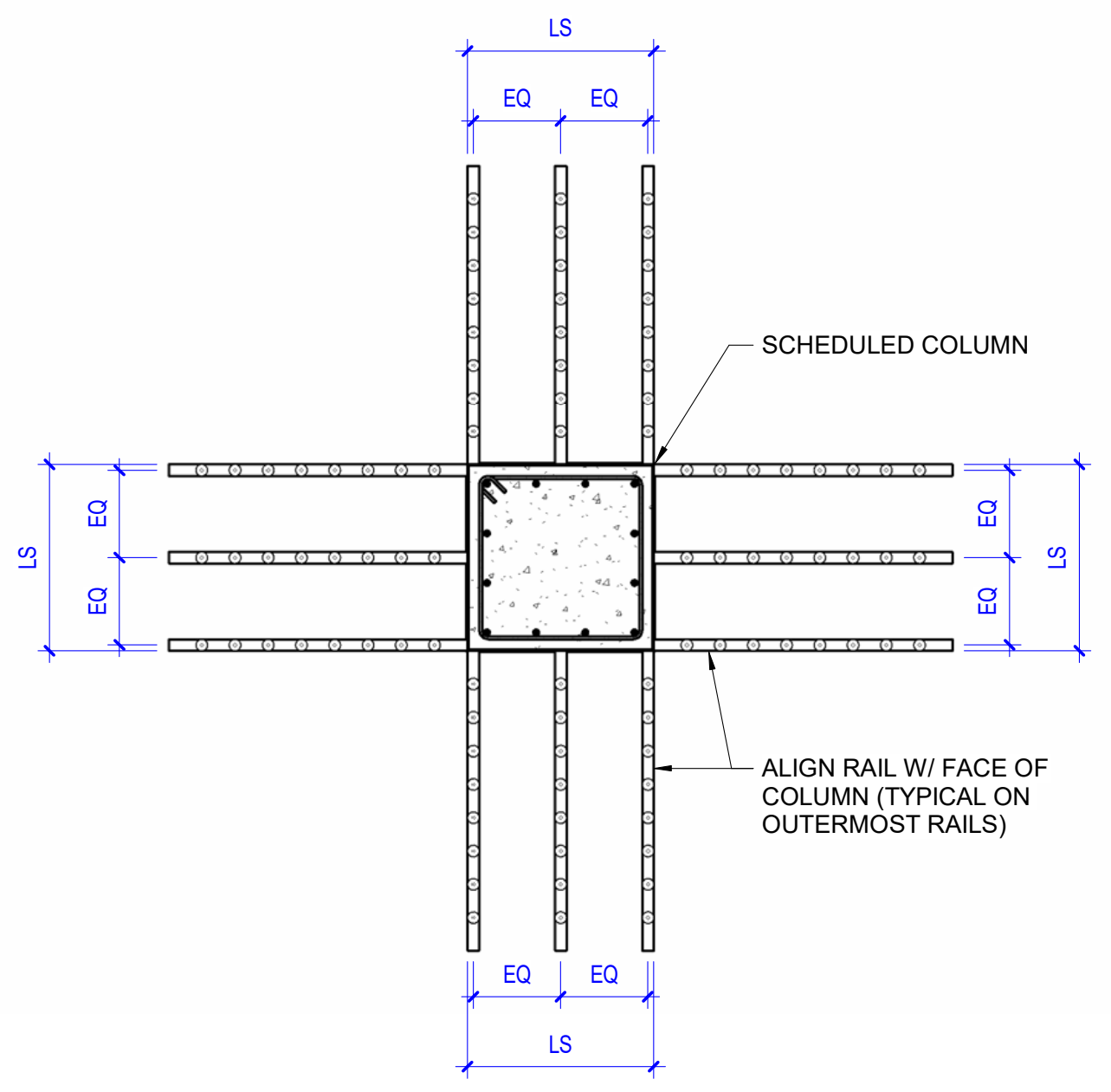
Round Column - Interior
1/2" = 1'-0"



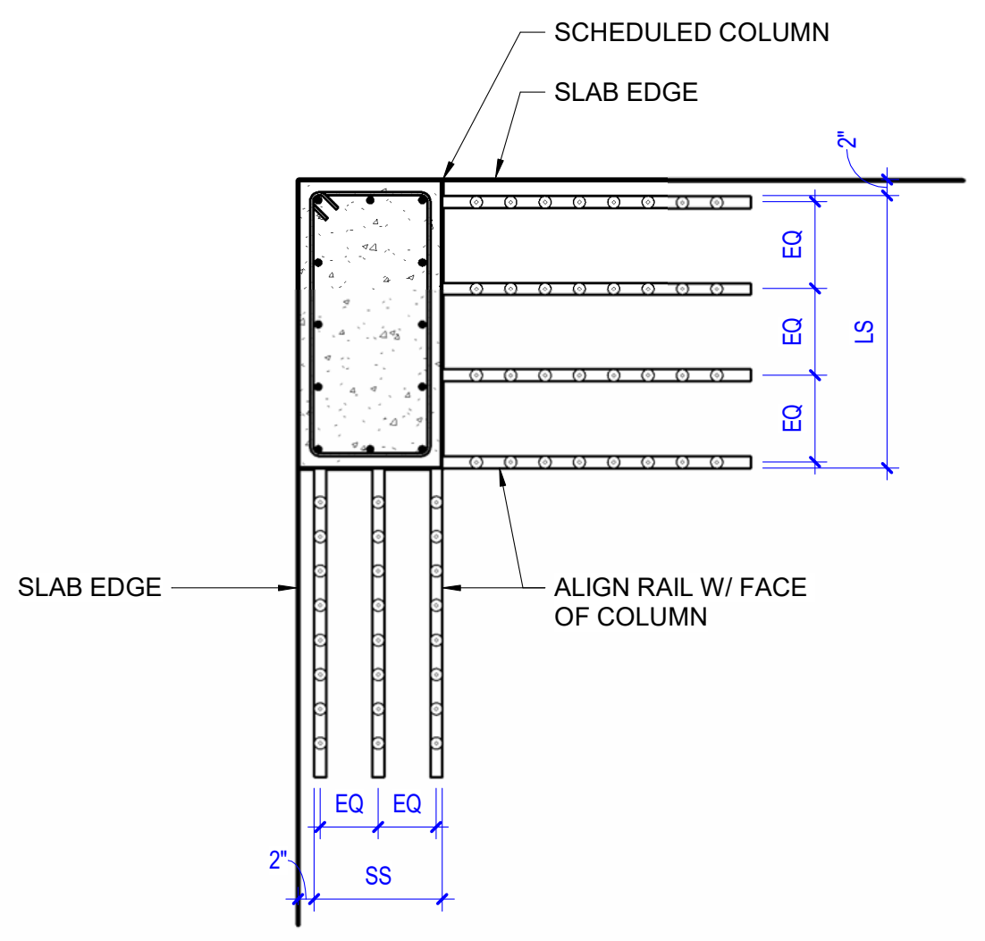
Square Column - Corner
1/2" = 1'-0"



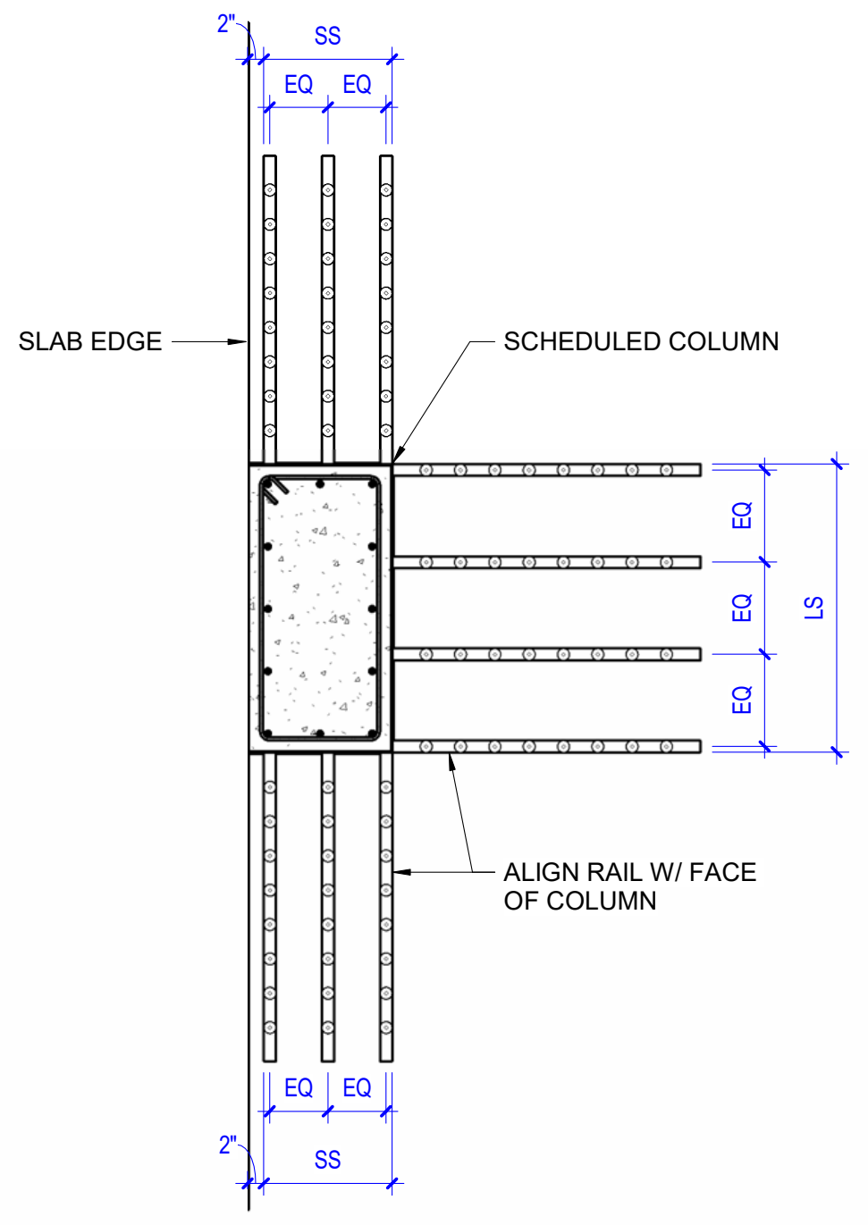
Square Column - Exterior
1/2" = 1'-0"



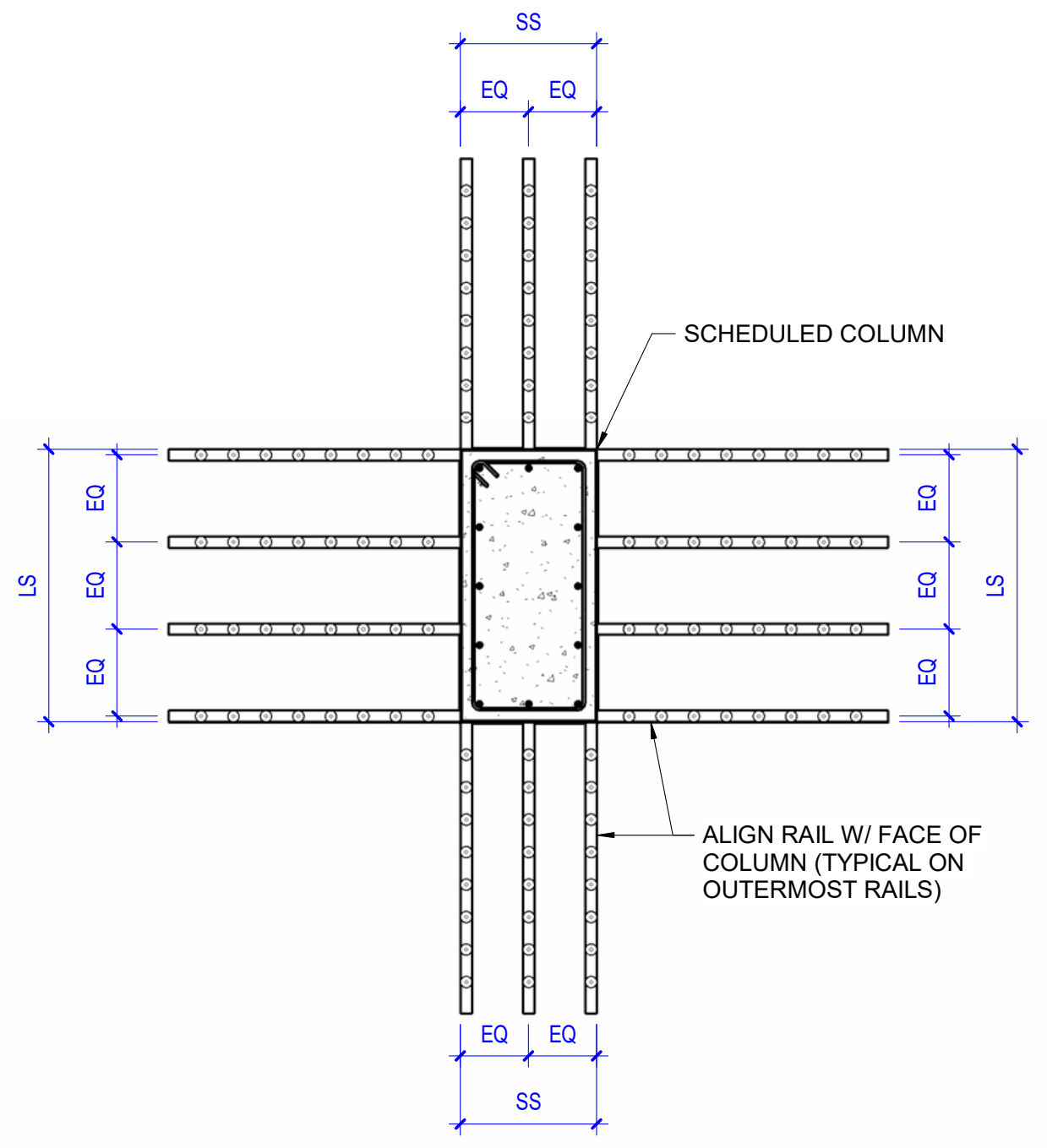
Square Column - Interior
1/2" = 1'-0"



Rectangular Column - Corner
1/2" = 1'-0"

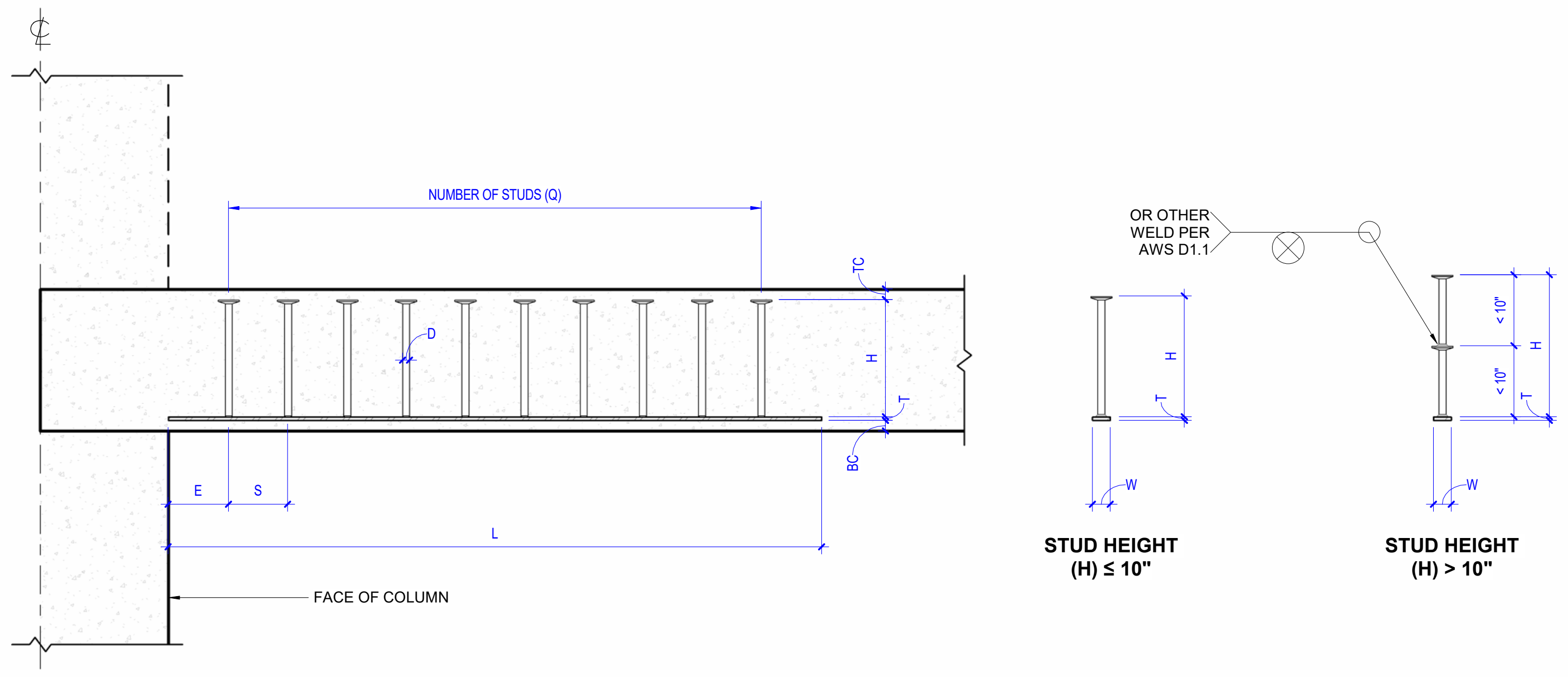


Rectangular Column - Exterior
1/2" = 1'-0"



Rectangular Column - Interior
1/2" = 1'-0"

DECON STUDRAIL SCHEDULE



MARK	TOTAL NUMBER OF RAILS	OVERALL LENGTH OF RAIL (L)	NUMBER OF RAILS LONG SIDE (LS)	NUMBER OF RAILS SHORT SIDE (SS)	NUMBER OF RAILS LONG SIDE (LS2)	NUMBER OF RAILS LONG SIDE (SS2)	NUMBER OF STUDS PER RAIL (Q)	DISTANCE TO FIRST STUD (E)	STUD SPACING (S)	STUD DIAMETER (D)	STUD HEIGHT (H)	TOP COVER (TC)	BOTTOM COVER (BC)	NOTES
SR-X	X	X	X	X	X	X	X	X	X	X	X	X	X	

CC4.0 SHEAR STUD RAIL REINFORCEMENT:

1. MATERIALS USED FOR SHEAR STUD RAIL ASSEMBLIES SHALL CONFORM TO ASTM A108 AND HAVE THE FOLLOWING MINIMUM MATERIAL PROPERTIES:
   A. STUDS:
   - YIELD STRENGTH: 51,000 PSI\
   - TENSILE STRENGTH: 65,000 PSI
   - ELONGATION: 20% IN 2 INCHES
   - REDUCTION OF AREA: 50%

   B. RAILS:
   - YIELD STRENGTH: 44,000 PSI
   - TENSILE STRENGTH: 65,000 PSI
   - ELONGATION IN 8 IN.: 20%
   - REDUCTION OF AREA: 50%
2. WELDING OF RAIL ASSEMBLIES SHALL BE PERFORMED IN ACCORDANCE WITH AMERICAN WELDING SOCIETY (AWS) D1.1, LATEST EDITION.
3. THE OVERALL HEIGHT OF THE STUDRAIL IS DEPENDENT ON THE SLAB THICKNESS AND THE REQUIRED CONCRETE COVER.
4. MAINTAIN A MINIMUM 2" CLEAR SIDE COVER TO SLAB EDGES.
5. ALIGN END OF RAILS WITH FINISHED FACE OF COLUMN AND PLACE PERPENDICULAR TO COLUMN FACES U.N.O..
6. NAIL RAILS TO FORMWORK TO PREVENT MOVEMENT DURING CONCRETE PLACEMENT.
7. THE BASE RAIL PROFILE SHALL CONFORM TO ASTM A1044.
8. THE COMPLETE AND FINISHED STUDRAIL ASSEMBLY SHALL BE ICC-ES EVALUATED WITH WELDING TO TAKE PLACE IN AN ICC-ES APPROVED AND AUDITED FACILITY.

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SHEET TITLE:
Typical Stud Rail Details

SHEET NUMBER:
S5.130

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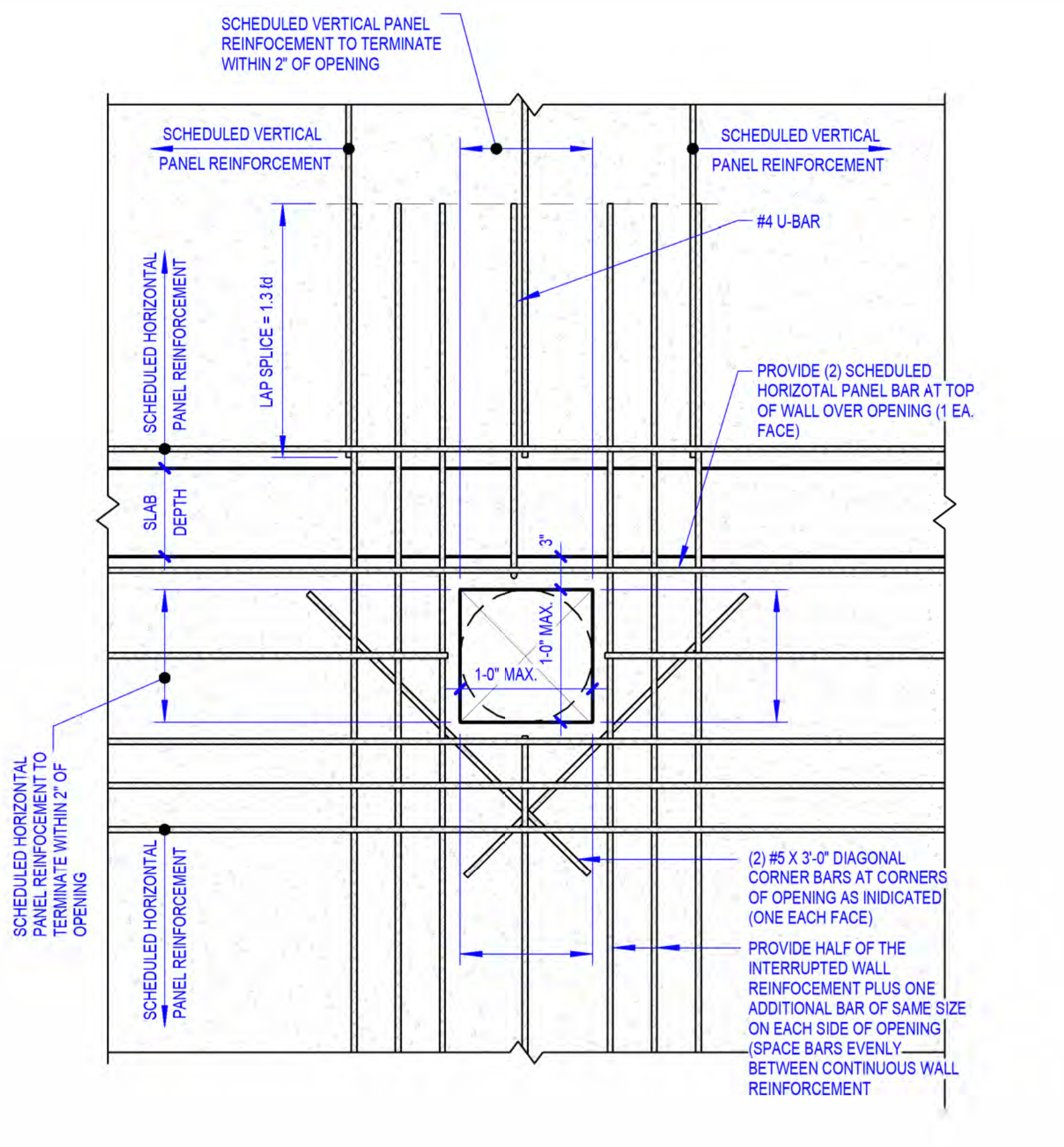
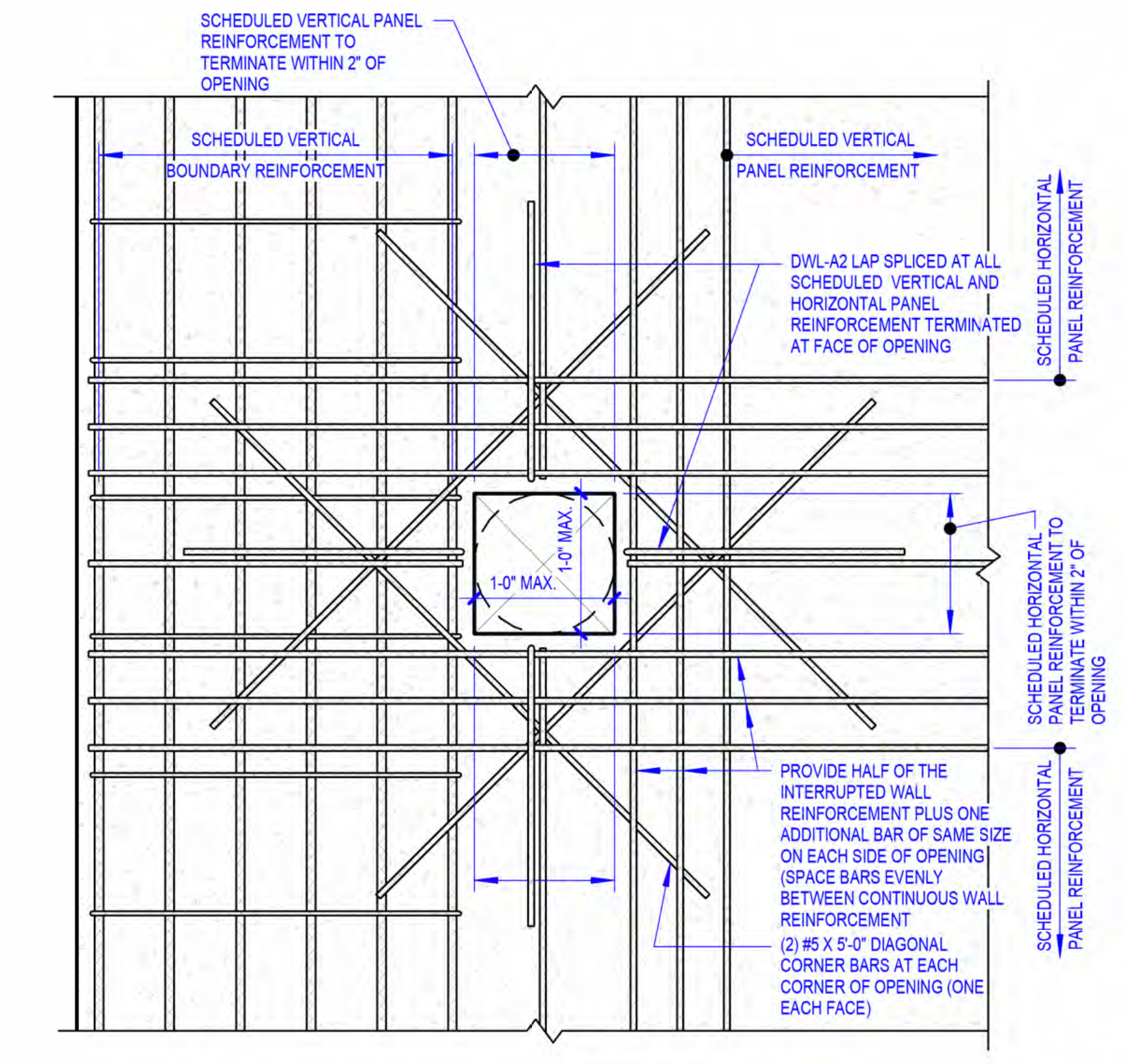
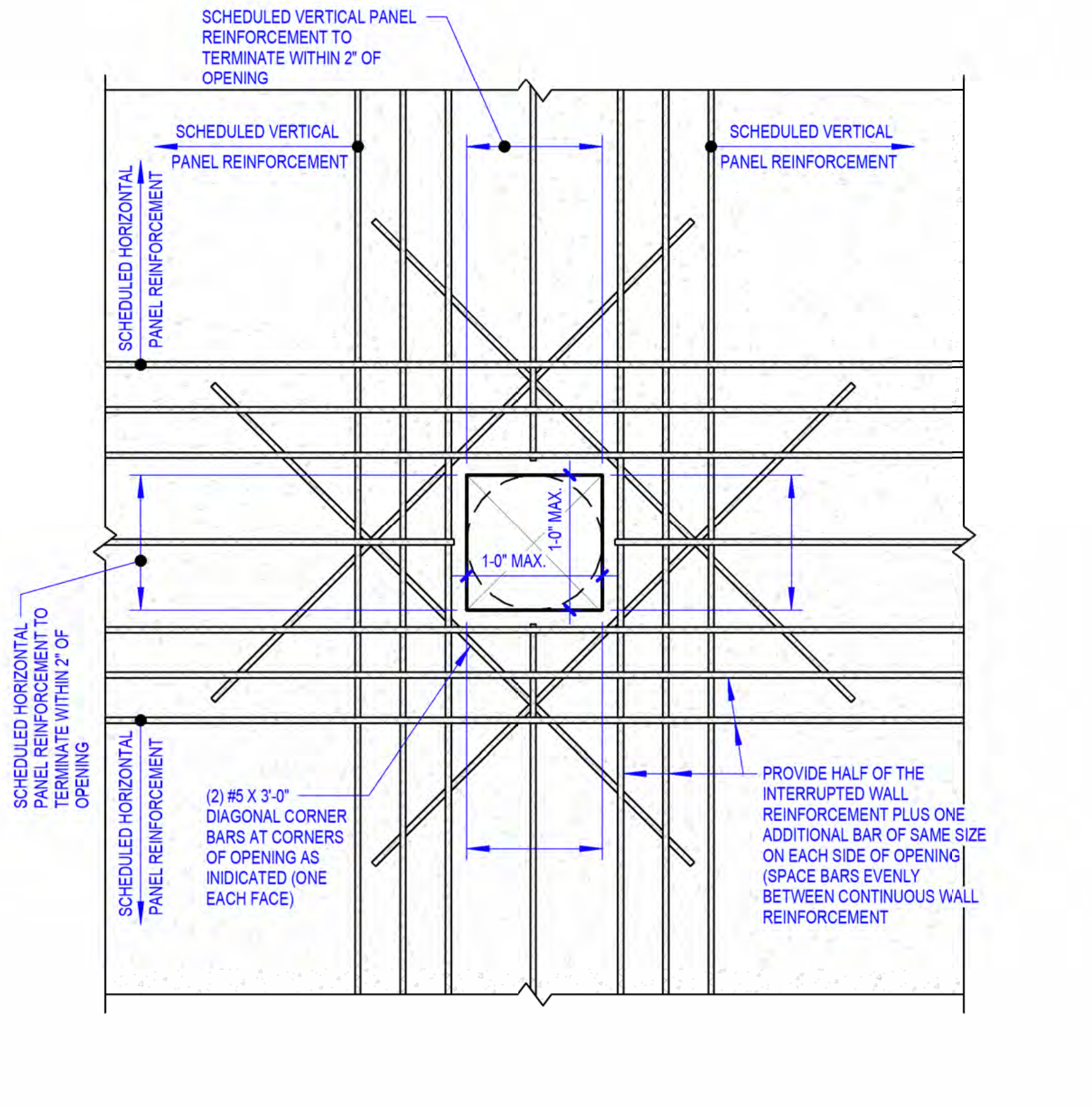

SCHEDULED VERTICAL PANEL REINFOCEMENT TO TERMINATE WITHIN 2" OF OPENING
SCHEDULED VERTICAL PANEL REINFORCEMENT
SCHEDULED VERTICAL PANEL REINFORCEMENT
#4 U-BAR
SCHEDULED HORIZONTAL PANEL REINFORCEMENT
LAP SPLICE = 1.3 ℓd
PROVIDE (2) SCHEDULED HORIZOTAL PANEL BAR AT TOP OF WALL OVER OPENING (1 EA. FACE)
SLAB DEPTH
3"
1-0" MAX.
1-0" MAX.
SCHEDULED HORIZONTAL PANEL REINFOCEMENT TO TERMINATE WITHIN 2" OF OPENING
SCHEDULED HORIZONTAL PANEL REINFORCEMENT
(2) #5 X 3'-0" DIAGONAL CORNER BARS AT CORNERS OF OPENING AS INIDICATED (ONE EACH FACE)
PROVIDE HALF OF THE INTERRUPTED WALL REINFOCEMENT PLUS ONE ADDITIONAL BAR OF SAME SIZE ON EACH SIDE OF OPENING (SPACE BARS EVENLY BETWEEN CONTINUOUS WALL REINFORCEMENT

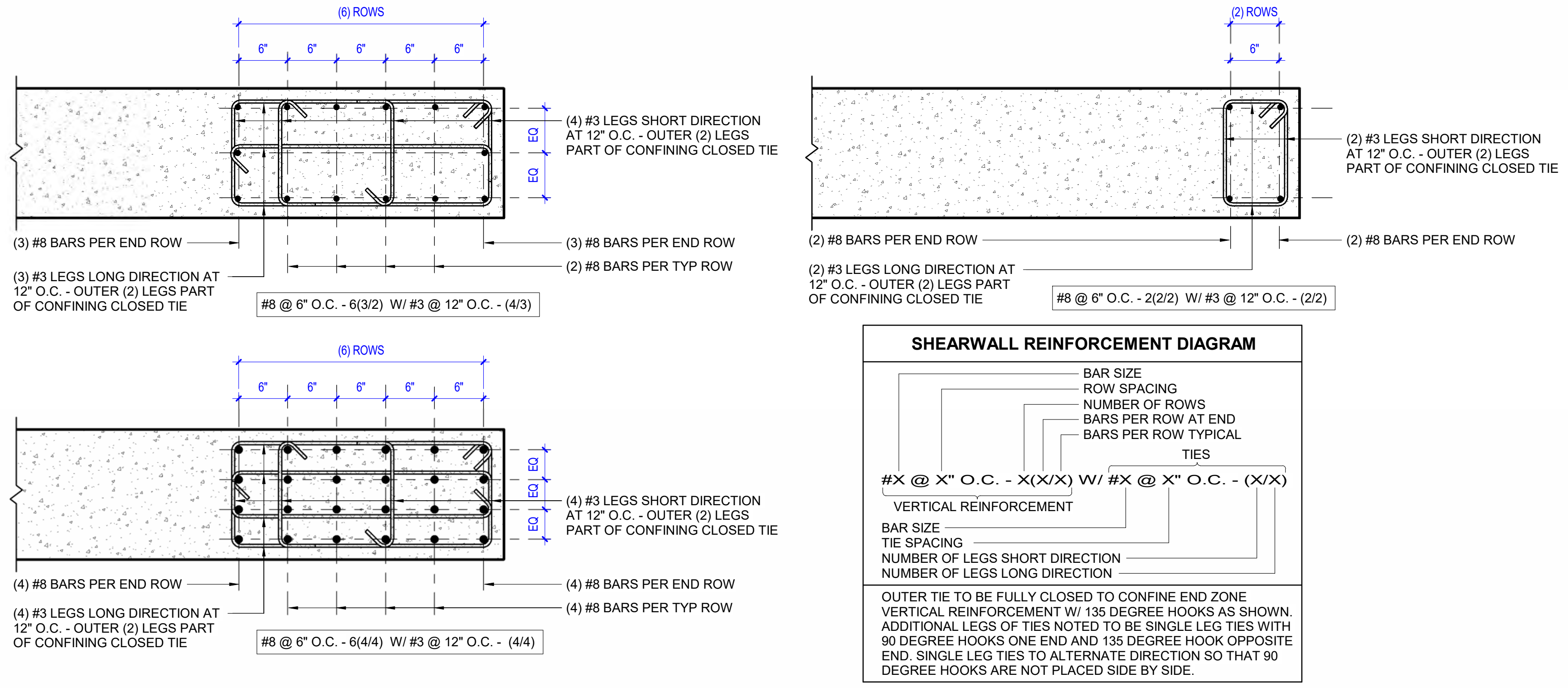
7 Shear Wall Opening Reinforcement - Top / Bottom
1" = 1'-0"


SCHEDULED VERTICAL PANEL REINFORCEMENT TO TERMINATE WITHIN 2" OF OPENING
SCHEDULED VERTICAL BOUNDARY REINFORCEMENT
SCHEDULED VERTICAL PANEL REINFORCEMENT
DWL-A2 LAP SPLICED AT ALL SCHEDULED VERTICAL AND HORIZONTAL PANEL REINFORCEMENT TERMINATED AT FACE OF OPENING
SCHEDULED HORIZONTAL PANEL REINFORCEMENT
1-0" MAX.
1-0" MAX.
SCHEDULED HORIZONTAL PANEL REINFORCEMENT TO TERMINATE WITHIN 2" OF OPENING
PROVIDE HALF OF THE INTERRUPTED WALL REINFORCEMENT PLUS ONE ADDITIONAL BAR OF SAME SIZE ON EACH SIDE OF OPENING (SPACE BARS EVENLY BETWEEN CONTINUOUS WALL REINFORCEMENT
(2) #5 X 5'-0" DIAGONAL CORNER BARS AT EACH CORNER OF OPENING (ONE EACH FACE)
SCHEDULED HORIZONTAL PANEL REINFORCEMENT

6 Shear Wall Opening Reinforcement - Exterior (Within 12" Of End Zone)
1" = 1'-0"


SCHEDULED VERTICAL PANEL REINFORCEMENT TO TERMINATE WITHIN 2" OF OPENING
SCHEDULED VERTICAL PANEL REINFORCEMENT
SCHEDULED VERTICAL PANEL REINFORCEMENT
SCHEDULED HORIZONTAL PANEL REINFORCEMENT
1-0" MAX.
1-0" MAX.
SCHEDULED HORIZONTAL PANEL REINFORCEMENT TO TERMINATE WITHIN 2" OF OPENING
SCHEDULED HORIZONTAL PANEL REINFORCEMENT
(2) #5 X 3'-0" DIAGONAL CORNER BARS AT CORNERS OF OPENING AS INIDICATED (ONE EACH FACE)
PROVIDE HALF OF THE INTERRUPTED WALL REINFORCEMENT PLUS ONE ADDITIONAL BAR OF SAME SIZE ON EACH SIDE OF OPENING (SPACE BARS EVENLY BETWEEN CONTINUOUS WALL REINFORCEMENT

5 Shear Wall Opening Reinforcement - Interior
1" = 1'-0"


(6) ROWS
6"
6"
6"
6"
6"
EQ
EQ
(4) #3 LEGS SHORT DIRECTION AT 12" O.C. - OUTER (2) LEGS PART OF CONFINING CLOSED TIE
(3) #8 BARS PER END ROW
(3) #8 BARS PER END ROW
(2) #8 BARS PER TYP ROW
(3) #3 LEGS LONG DIRECTION AT 12" O.C. - OUTER (2) LEGS PART OF CONFINING CLOSED TIE
#8 @ 6" O.C. - 6(3/2) W/ #3 @ 12" O.C. - (4/3)
(2) ROWS
6"
(2) #3 LEGS SHORT DIRECTION AT 12" O.C. - OUTER (2) LEGS PART OF CONFINING CLOSED TIE
(2) #8 BARS PER END ROW
(2) #8 BARS PER END ROW
(2) #3 LEGS LONG DIRECTION AT 12" O.C. - OUTER (2) LEGS PART OF CONFINING CLOSED TIE
#8 @ 6" O.C. - 2(2/2) W/ #3 @ 12" O.C. - (2/2)
(6) ROWS
6"
6"
6"
6"
6"
EQ
EQ
EQ
(4) #3 LEGS SHORT DIRECTION AT 12" O.C. - OUTER (2) LEGS PART OF CONFINING CLOSED TIE
(4) #8 BARS PER END ROW
(4) #8 BARS PER END ROW
(4) #8 BARS PER TYP ROW
(4) #3 LEGS LONG DIRECTION AT 12" O.C. - OUTER (2) LEGS PART OF CONFINING CLOSED TIE
#8 @ 6" O.C. - 6(4/4) W/ #3 @ 12" O.C. - (4/4)

SHEARWALL REINFORCEMENT DIAGRAM

BAR SIZE
ROW SPACING
NUMBER OF ROWS
BARS PER ROW AT END
BARS PER ROW TYPICAL
TIES

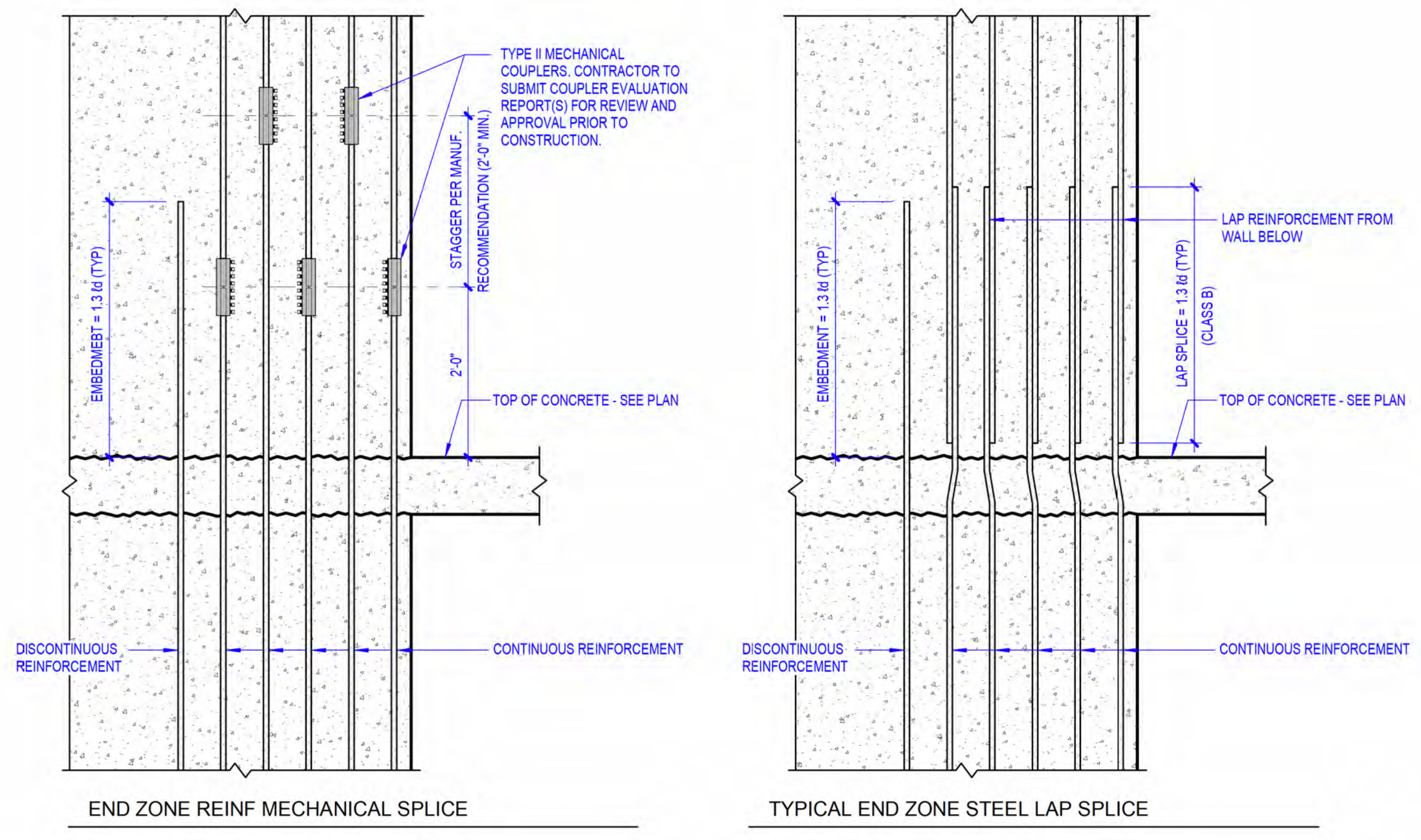
#X @ X" O.C. - X(X/X) W/ #X @ X" O.C. - (X/X)

VERTICAL REINFORCEMENT

BAR SIZE
TIE SPACING
NUMBER OF LEGS SHORT DIRECTION
NUMBER OF LEGS LONG DIRECTION

OUTER TIE TO BE FULLY CLOSED TO CONFINE END ZONE VERTICAL REINFORCEMENT W/ 135 DEGREE HOOKS AS SHOWN. ADDITIONAL LEGS OF TIES NOTED TO BE SINGLE LEG TIES WITH 90 DEGREE HOOKS ONE END AND 135 DEGREE HOOK OPPOSITE END. SINGLE LEG TIES TO ALTERNATE DIRECTION SO THAT 90 DEGREE HOOKS ARE NOT PLACED SIDE BY SIDE.

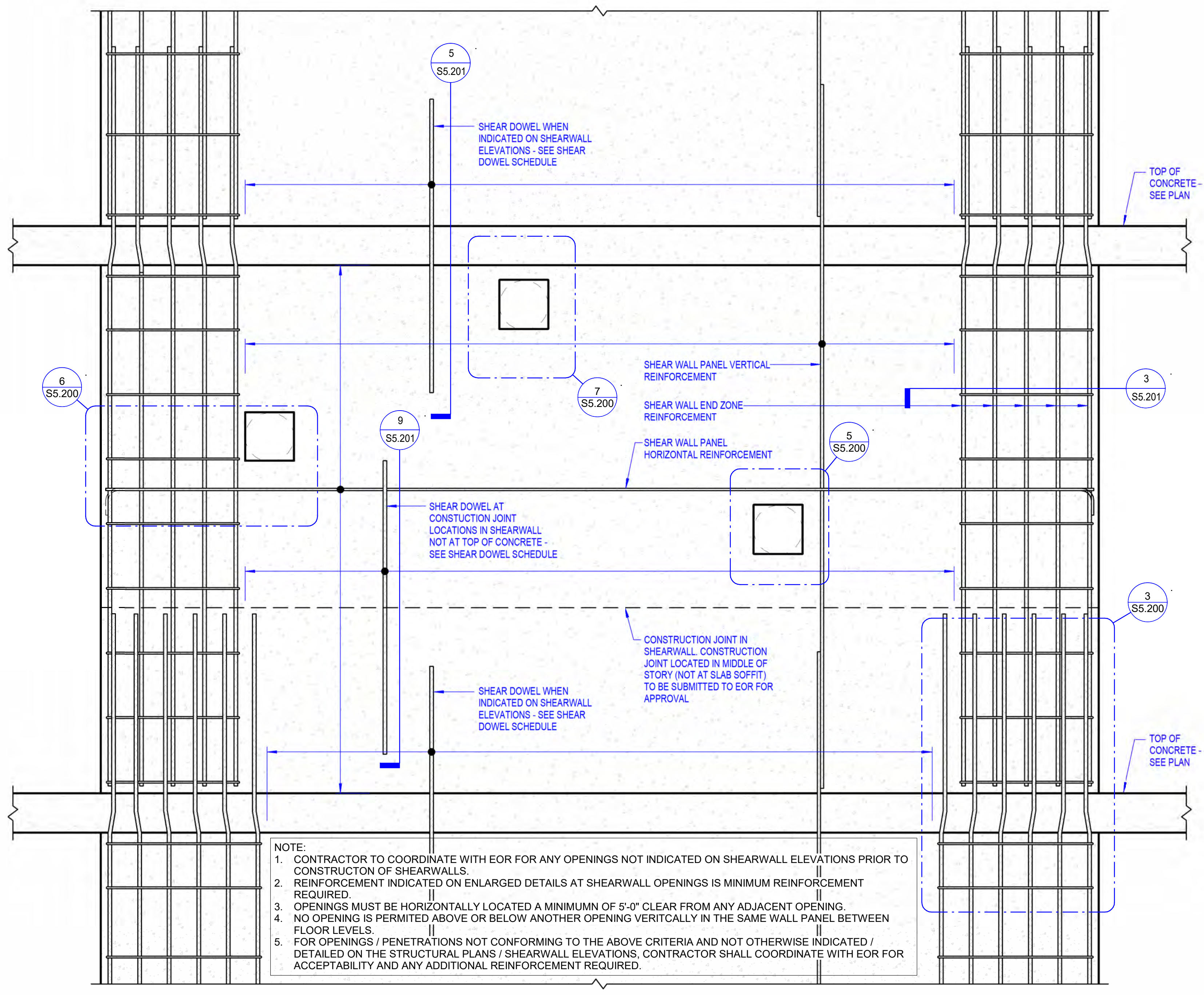
4 Shear Wall End Zone Reinforcement Diagram
1" = 1'-0"


TYPE II MECHANICAL COUPLERS. CONTRACTOR TO SUBMIT COUPLER EVALUATION REPORT(S) FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.
STAGGER PER MANUF. RECOMMENDATION (2'-0" MIN.)
EMBEDMEBT = 1.3 ℓd (TYP)
2'-0"
TOP OF CONCRETE - SEE PLAN
DISCONTINUOUS REINFORCEMENT
CONTINUOUS REINFORCEMENT
LAP REINFORCEMENT FROM WALL BELOW
EMBEDMENT = 1.3 ℓd (TYP)
LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
TOP OF CONCRETE - SEE PLAN
DISCONTINUOUS REINFORCEMENT
CONTINUOUS REINFORCEMENT

END ZONE REINF MECHANICAL SPLICE

TYPICAL END ZONE STEEL LAP SPLICE

3 Shear Wall Boundary Element Lap Splice
3/4" = 1'-0"

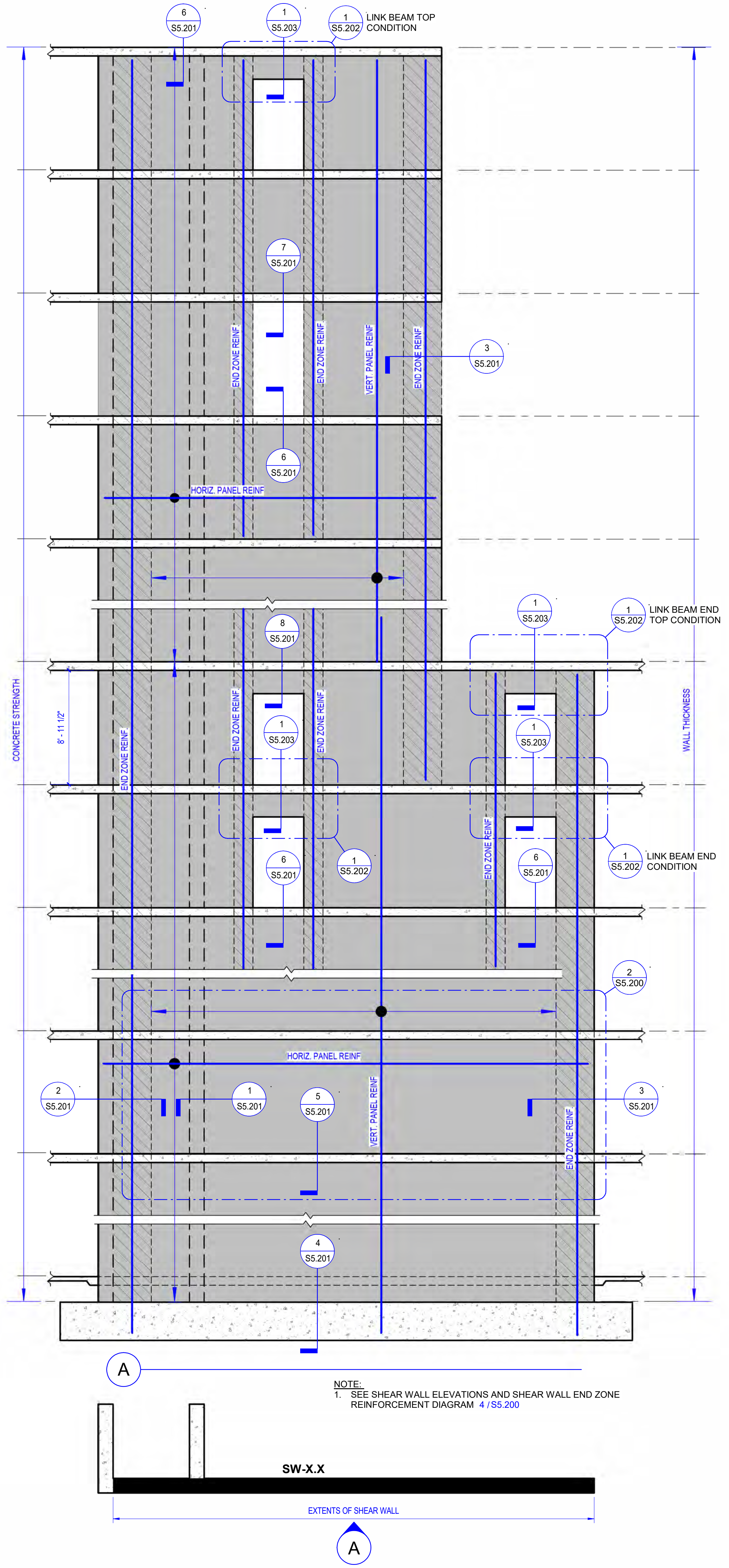

5 S5.201
SHEAR DOWEL WHEN INDICATED ON SHEARWALL ELEVATIONS - SEE SHEAR DOWEL SCHEDULE
TOP OF CONCRETE - SEE PLAN
6 S5.200
7 S5.200
SHEAR WALL PANEL VERTICAL REINFORCEMENT
SHEAR WALL END ZONE REINFORCEMENT
3 S5.201
9 S5.201
SHEAR WALL PANEL HORIZONTAL REINFORCEMENT
5 S5.200
SHEAR DOWEL AT CONSTUCTION JOINT LOCATIONS IN SHEARWALL NOT AT TOP OF CONCRETE - SEE SHEAR DOWEL SCHEDULE
3 S5.200
CONSTRUCTION JOINT IN SHEARWALL. CONSTRUCTION JOINT LOCATED IN MIDDLE OF STORY (NOT AT SLAB SOFFIT) TO BE SUBMITTED TO EOR FOR APPROVAL
SHEAR DOWEL WHEN INDICATED ON SHEARWALL ELEVATIONS - SEE SHEAR DOWEL SCHEDULE
TOP OF CONCRETE - SEE PLAN

NOTE:
1. CONTRACTOR TO COORDINATE WITH EOR FOR ANY OPENINGS NOT INDICATED ON SHEARWALL ELEVATIONS PRIOR TO CONSTRUCTON OF SHEARWALLS.
2. REINFORCEMENT INDICATED ON ENLARGED DETAILS AT SHEARWALL OPENINGS IS MINIMUM REINFORCEMENT REQUIRED.
3. OPENINGS MUST BE HORIZONTALLY LOCATED A MINIMUMN OF 5'-0" CLEAR FROM ANY ADJACENT OPENING.
4. NO OPENING IS PERMITED ABOVE OR BELOW ANOTHER OPENING VERITCALLY IN THE SAME WALL PANEL BETWEEN FLOOR LEVELS.
5. FOR OPENINGS / PENETRATIONS NOT CONFORMING TO THE ABOVE CRITERIA AND NOT OTHERWISE INDICATED / DETAILED ON THE STRUCTURAL PLANS / SHEARWALL ELEVATIONS, CONTRACTOR SHALL COORDINATE WITH EOR FOR ACCEPTABILITY AND ANY ADDITIONAL REINFORCEMENT REQUIRED.

2 Shear Wall Typical Reinforcement Detail
3/4" = 1'-0"

SHEAR WALL SHEAR DOWEL SCHEDULE

WALL THICKNESS (t)	t ≤ 12"	12" < t ≤ 18"	18" < t ≤ 24"	24" < t ≤ 30"	30" < t ≤ 36"
SIZE & SPACING	#6 AT 12" O.C.	#7 AT 12" O.C.	#8 AT 12" O.C.	#9 AT 12" O.C.	#10 AT 12" O.C.



NOTE:
1. SEE SHEAR WALL ELEVATIONS AND SHEAR WALL END ZONE REINFORCEMENT DIAGRAM 4 / S5.200

1 Shear Wall Diagram
3/16" = 1'-0"

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Houston, TX 77077
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PROJECT # : 26154
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No.	Description	Date
	100% DD	09/30/2026

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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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- MARTIN R. MAINGOT

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SHEET TITLE:
CIP Concrete Shear Wall Typical Details

SHEET NUMBER:
S5.200

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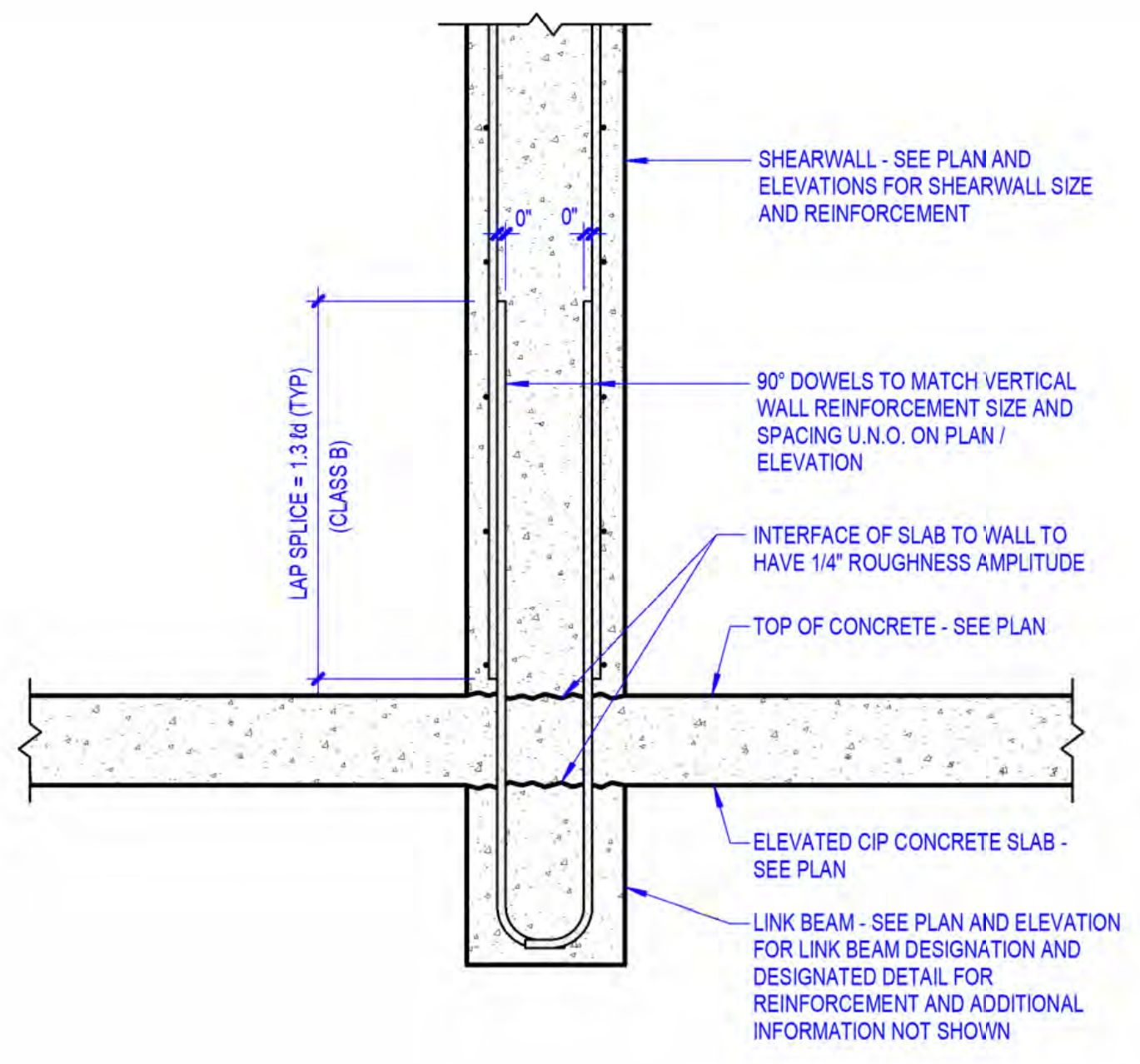
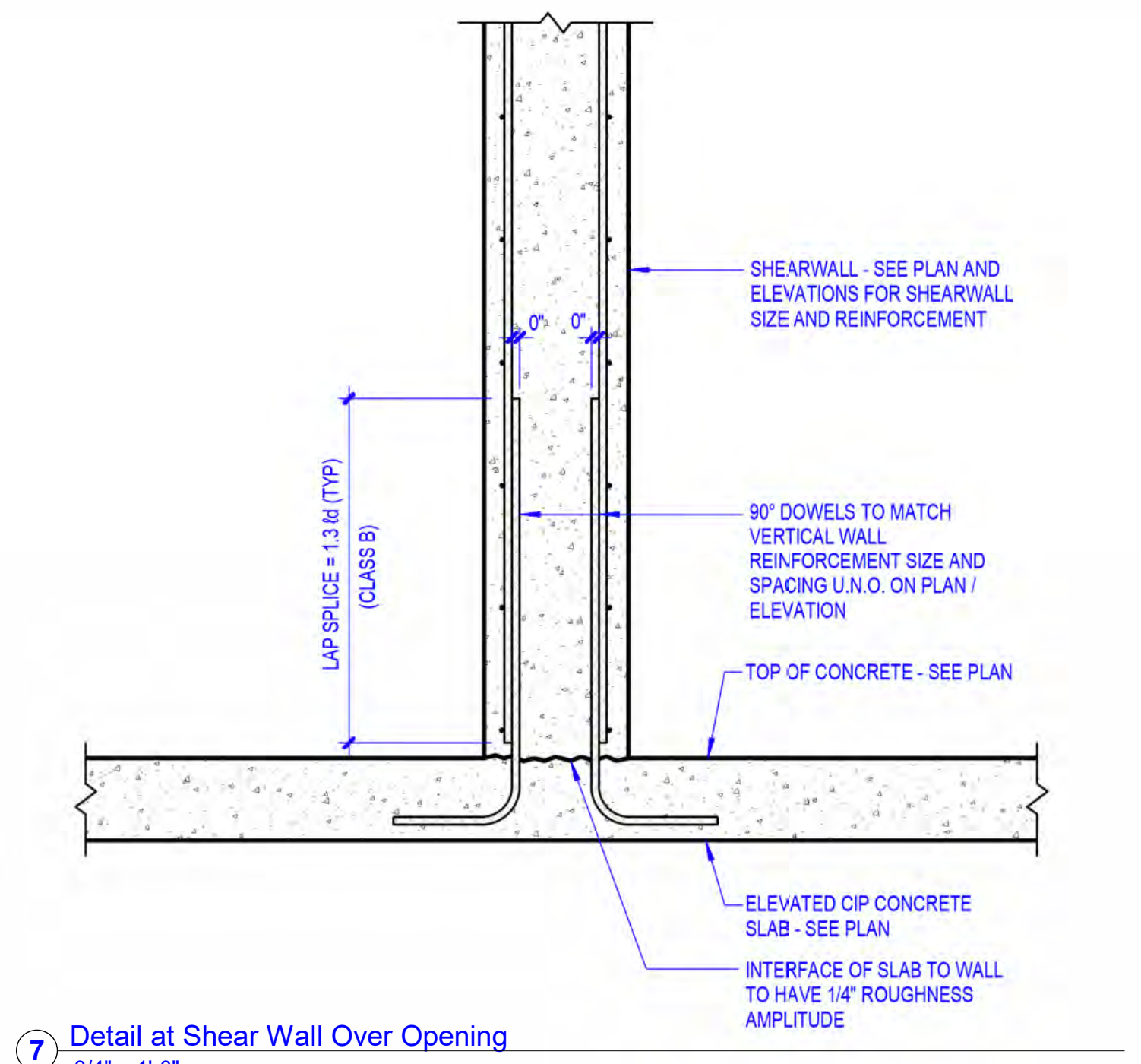
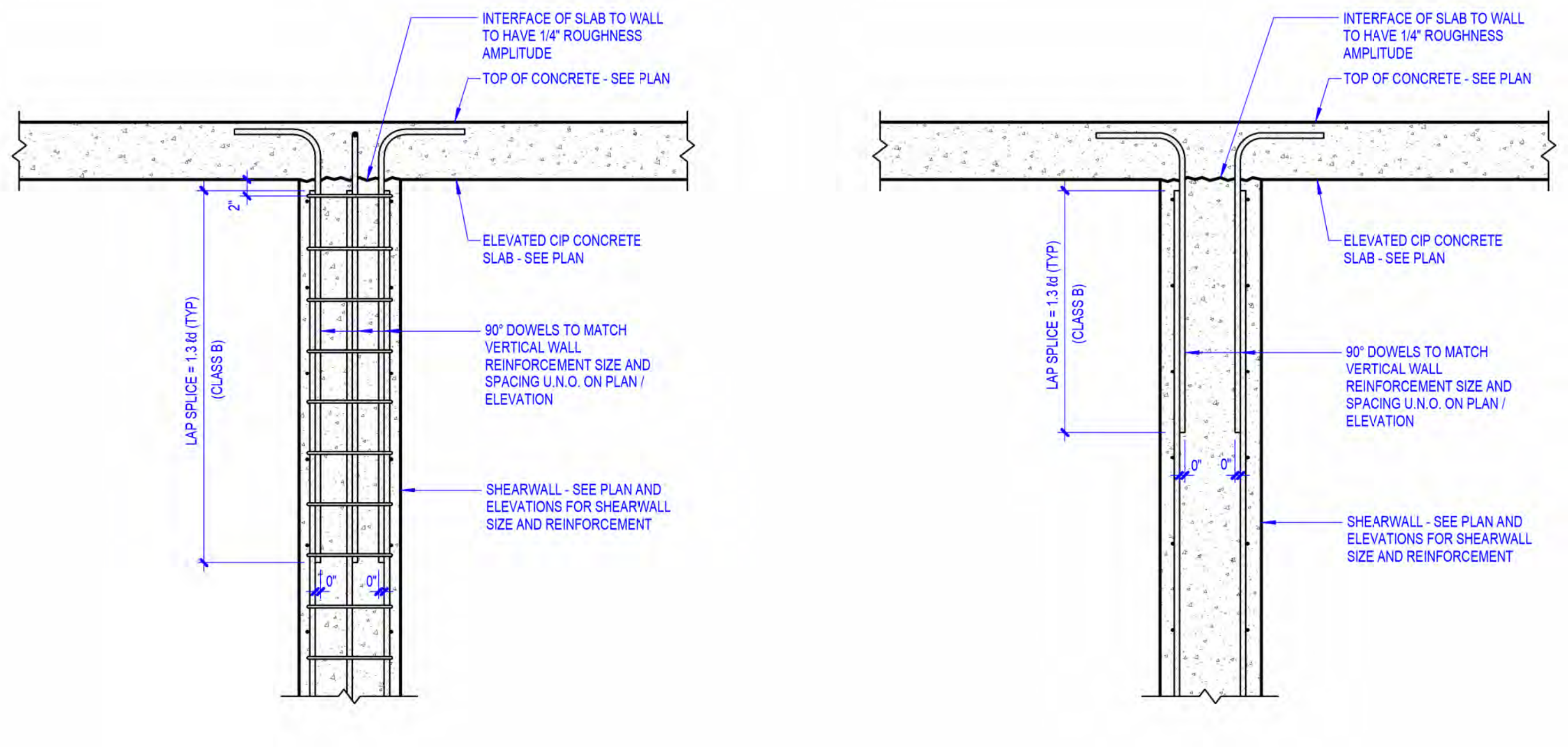

SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
0"
0"
90° DOWELS TO MATCH VERTICAL WALL REINFORCEMENT SIZE AND SPACING U.N.O. ON PLAN / ELEVATION
LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
INTERFACE OF SLAB TO WALL TO HAVE 1/4" ROUGHNESS AMPLITUDE
TOP OF CONCRETE - SEE PLAN
ELEVATED CIP CONCRETE SLAB - SEE PLAN
LINK BEAM - SEE PLAN AND ELEVATION FOR LINK BEAM DESIGNATION AND DESIGNATED DETAIL FOR REINFORCEMENT AND ADDITIONAL INFORMATION NOT SHOWN

8 Detail at Punched Opening (LB-A)
3/4" = 1'-0"


SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
0"
0"
LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
90° DOWELS TO MATCH VERTICAL WALL REINFORCEMENT SIZE AND SPACING U.N.O. ON PLAN / ELEVATION
TOP OF CONCRETE - SEE PLAN
ELEVATED CIP CONCRETE SLAB - SEE PLAN
INTERFACE OF SLAB TO WALL TO HAVE 1/4" ROUGHNESS AMPLITUDE

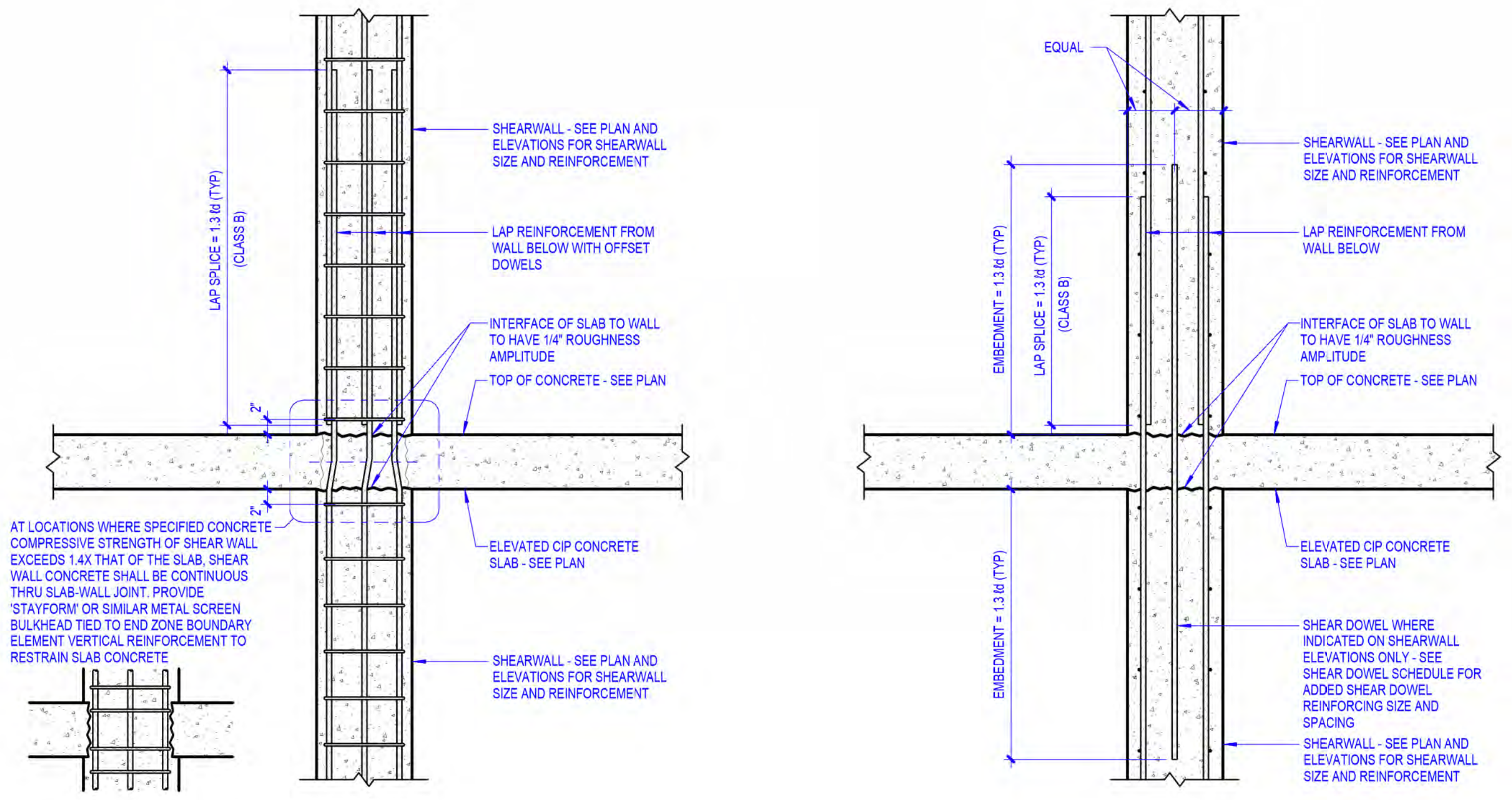
7 Detail at Shear Wall Over Opening
3/4" = 1'-0"


INTERFACE OF SLAB TO WALL TO HAVE 1/4" ROUGHNESS AMPLITUDE
TOP OF CONCRETE - SEE PLAN
2"
ELEVATED CIP CONCRETE SLAB - SEE PLAN
LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
90° DOWELS TO MATCH VERTICAL WALL REINFORCEMENT SIZE AND SPACING U.N.O. ON PLAN / ELEVATION
SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
0"
0"

END ZONE CONDITION

PANEL CONDITION

6 Detail at Top of Shear Wall
3/4" = 1'-0"

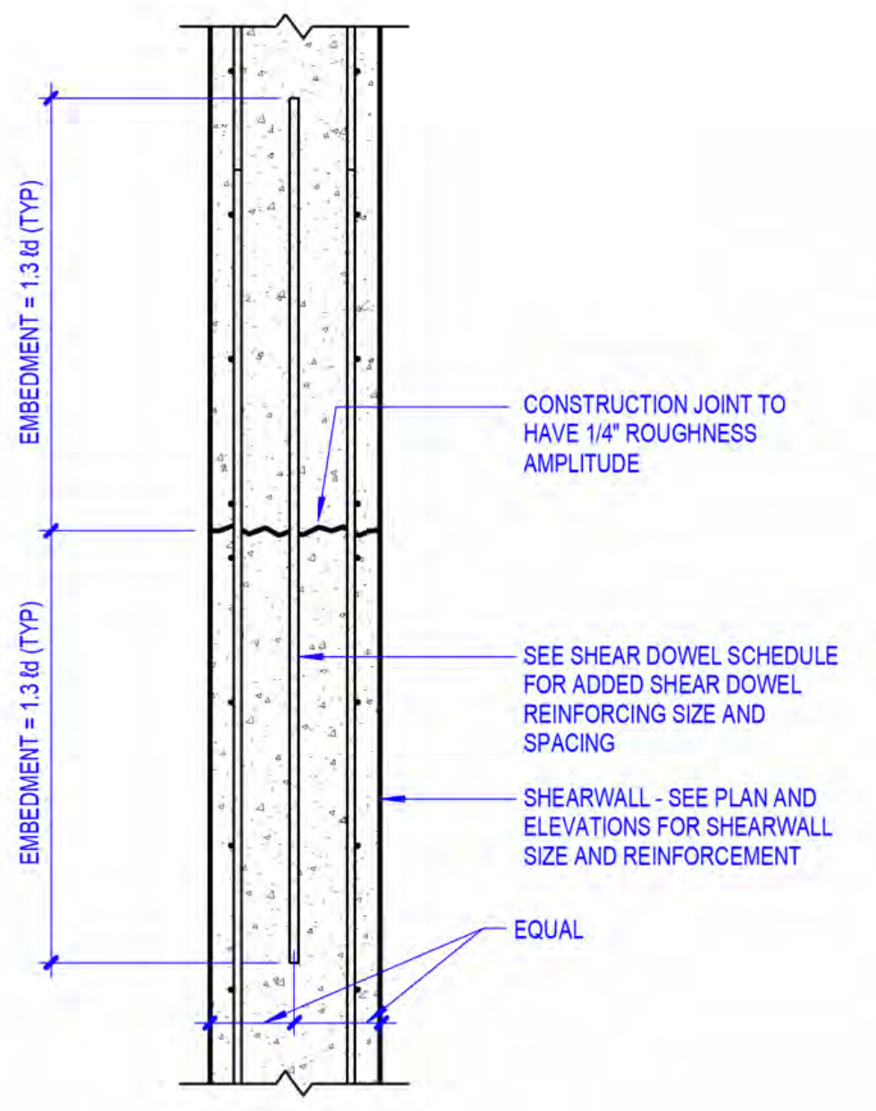

SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
LAP REINFORCEMENT FROM WALL BELOW WITH OFFSET DOWELS
LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
INTERFACE OF SLAB TO WALL TO HAVE 1/4" ROUGHNESS AMPLITUDE
TOP OF CONCRETE - SEE PLAN
2"
2"
ELEVATED CIP CONCRETE SLAB - SEE PLAN
SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
EQUAL
SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
EMBEDMENT = 1.3 ℓd (TYP)
LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
LAP REINFORCEMENT FROM WALL BELOW
INTERFACE OF SLAB TO WALL TO HAVE 1/4" ROUGHNESS AMPLITUDE
TOP OF CONCRETE - SEE PLAN
ELEVATED CIP CONCRETE SLAB - SEE PLAN
EMBEDMENT = 1.3 ℓd (TYP)
SHEAR DOWEL WHERE INDICATED ON SHEARWALL ELEVATIONS ONLY - SEE SHEAR DOWEL SCHEDULE FOR ADDED SHEAR DOWEL REINFORCING SIZE AND SPACING
SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT

AT LOCATIONS WHERE SPECIFIED CONCRETE COMPRESSIVE STRENGTH OF SHEAR WALL EXCEEDS 1.4X THAT OF THE SLAB, SHEAR WALL CONCRETE SHALL BE CONTINUOUS THRU SLAB-WALL JOINT. PROVIDE 'STAYFORM' OR SIMILAR METAL SCREEN BULKHEAD TIED TO END ZONE BOUNDARY ELEMENT VERTICAL REINFORCEMENT TO RESTRAIN SLAB CONCRETE

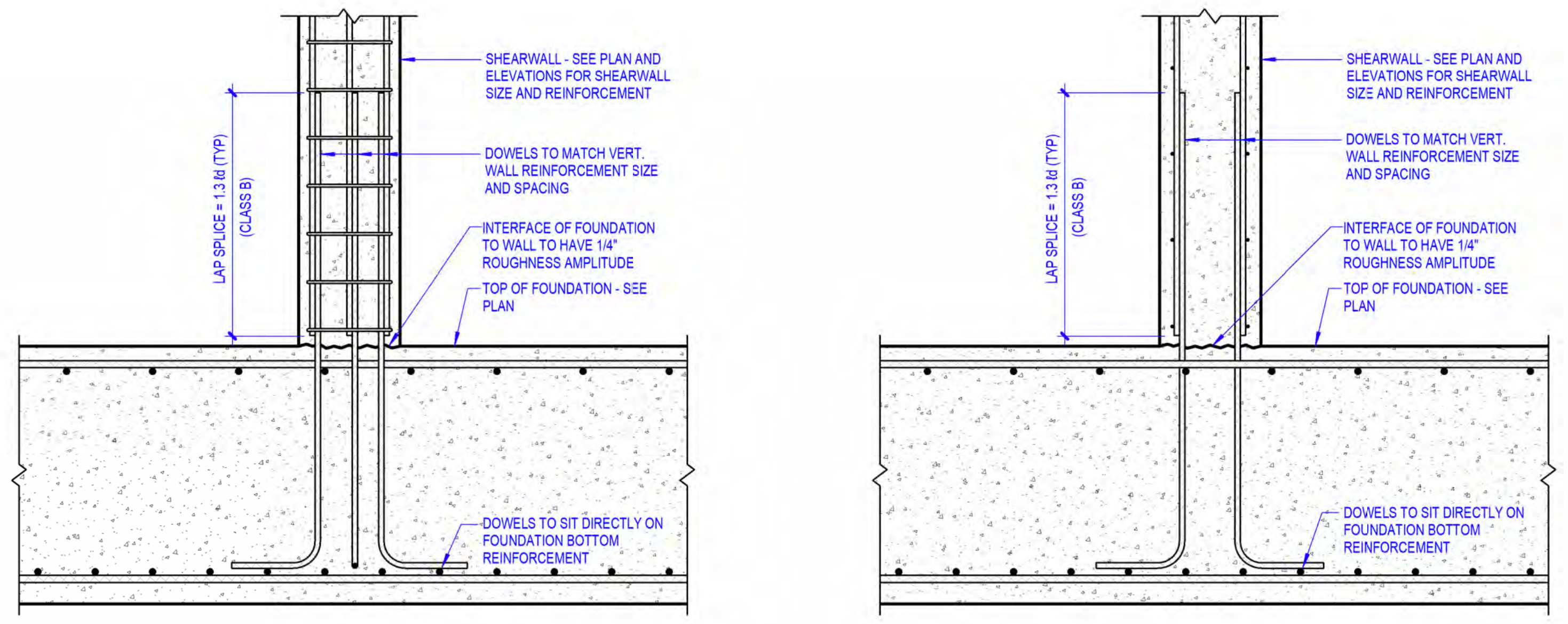
END ZONE CONDITION

PANEL CONDITION

5 Detail at Shear Wall at Elevated Concrete Slab
3/4" = 1'-0"


EMBEDMENT = 1.3 ℓd (TYP)
CONSTRUCTION JOINT TO HAVE 1/4" ROUGHNESS AMPLITUDE
EMBEDMENT = 1.3 ℓd (TYP)
SEE SHEAR DOWEL SCHEDULE FOR ADDED SHEAR DOWEL REINFORCING SIZE AND SPACING
SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
EQUAL

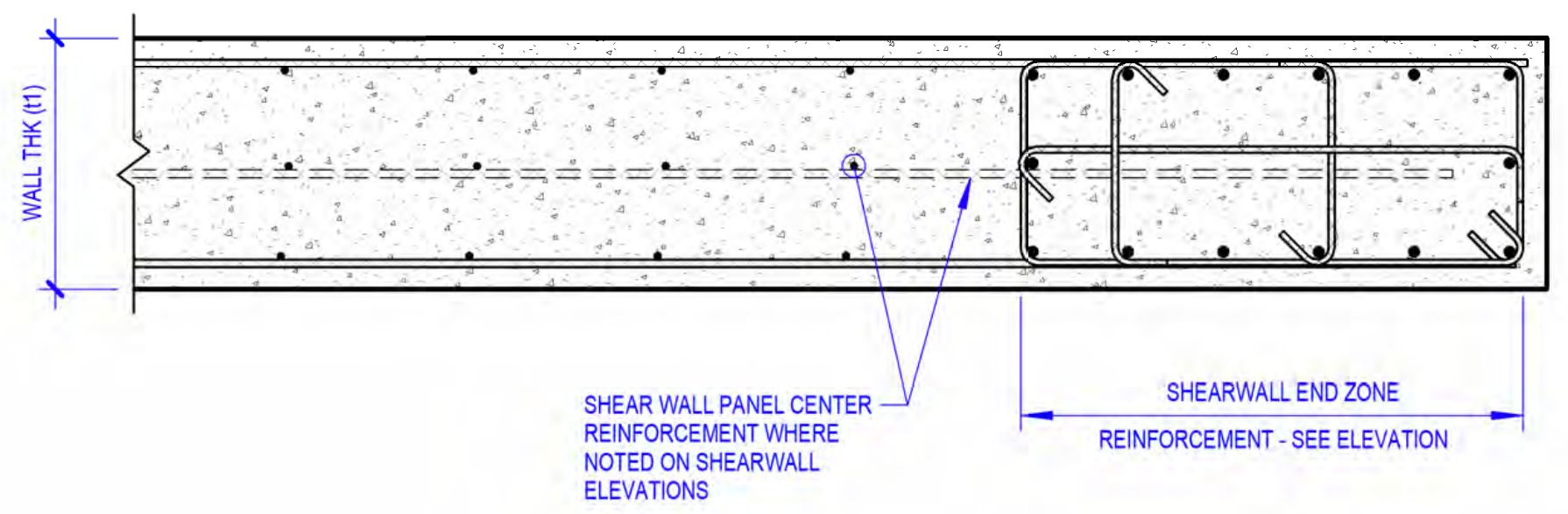
9 Detail at Construction Joint In Shear Wall
3/4" = 1'-0"


SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
DOWELS TO MATCH VERT. WALL REINFORCEMENT SIZE AND SPACING
LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
INTERFACE OF FOUNDATION TO WALL TO HAVE 1/4" ROUGHNESS AMPLITUDE
TOP OF FOUNDATION - SEE PLAN
DOWELS TO SIT DIRECTLY ON FOUNDATION BOTTOM REINFORCEMENT
SHEARWALL - SEE PLAN AND ELEVATIONS FOR SHEARWALL SIZE AND REINFORCEMENT
DOWELS TO MATCH VERT. WALL REINFORCEMENT SIZE AND SPACING
LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
INTERFACE OF FOUNDATION TO WALL TO HAVE 1/4" ROUGHNESS AMPLITUDE
TOP OF FOUNDATION - SEE PLAN
DOWELS TO SIT DIRECTLY ON FOUNDATION BOTTOM REINFORCEMENT

END ZONE CONDITION

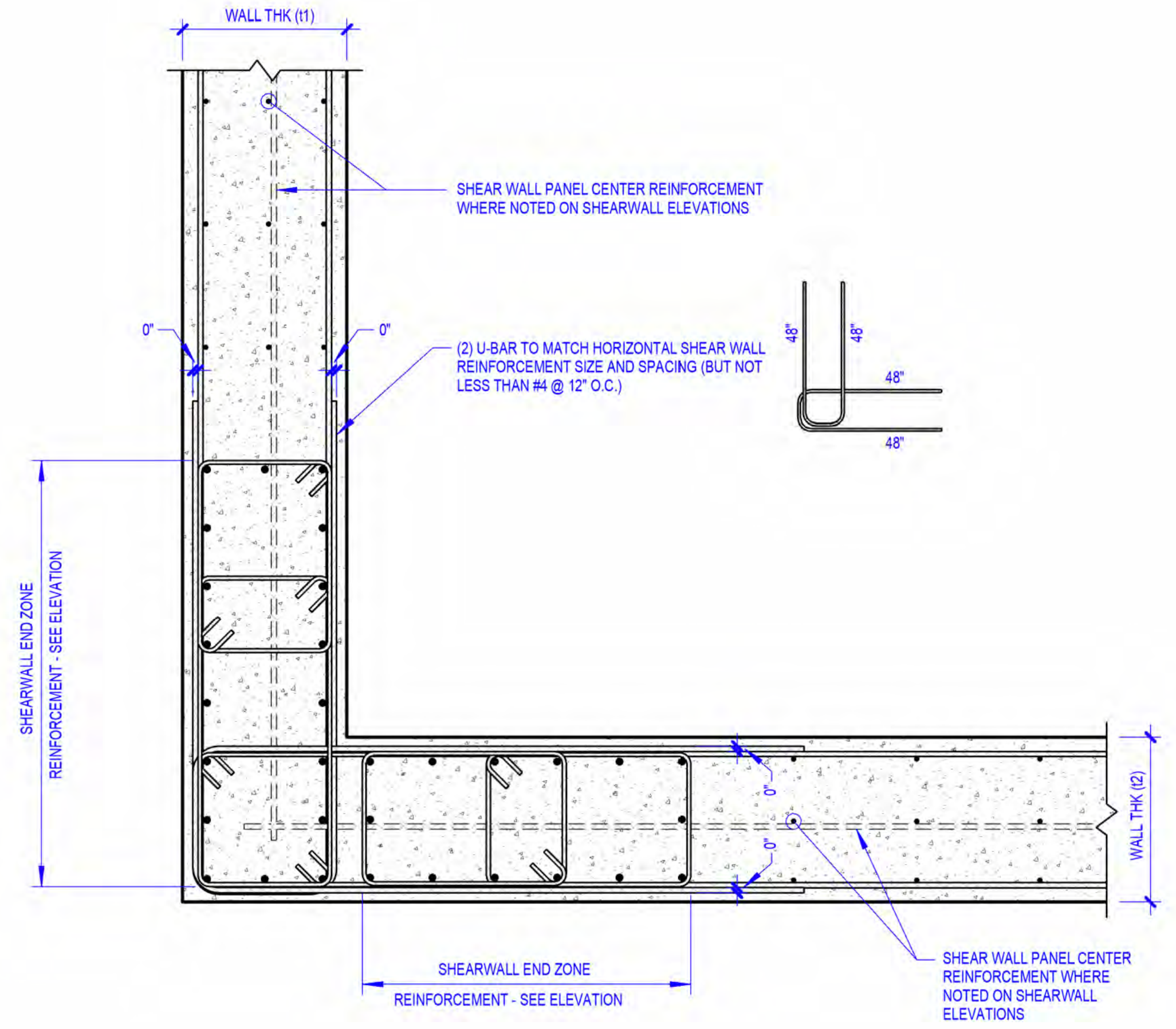
PANEL CONDITION

4 Detail at Shear Wall at Foundation
3/4" = 1'-0"


WALL THK (t1)
SHEAR WALL PANEL CENTER REINFORCEMENT WHERE NOTED ON SHEARWALL ELEVATIONS
SHEARWALL END ZONE
REINFORCEMENT - SEE ELEVATION

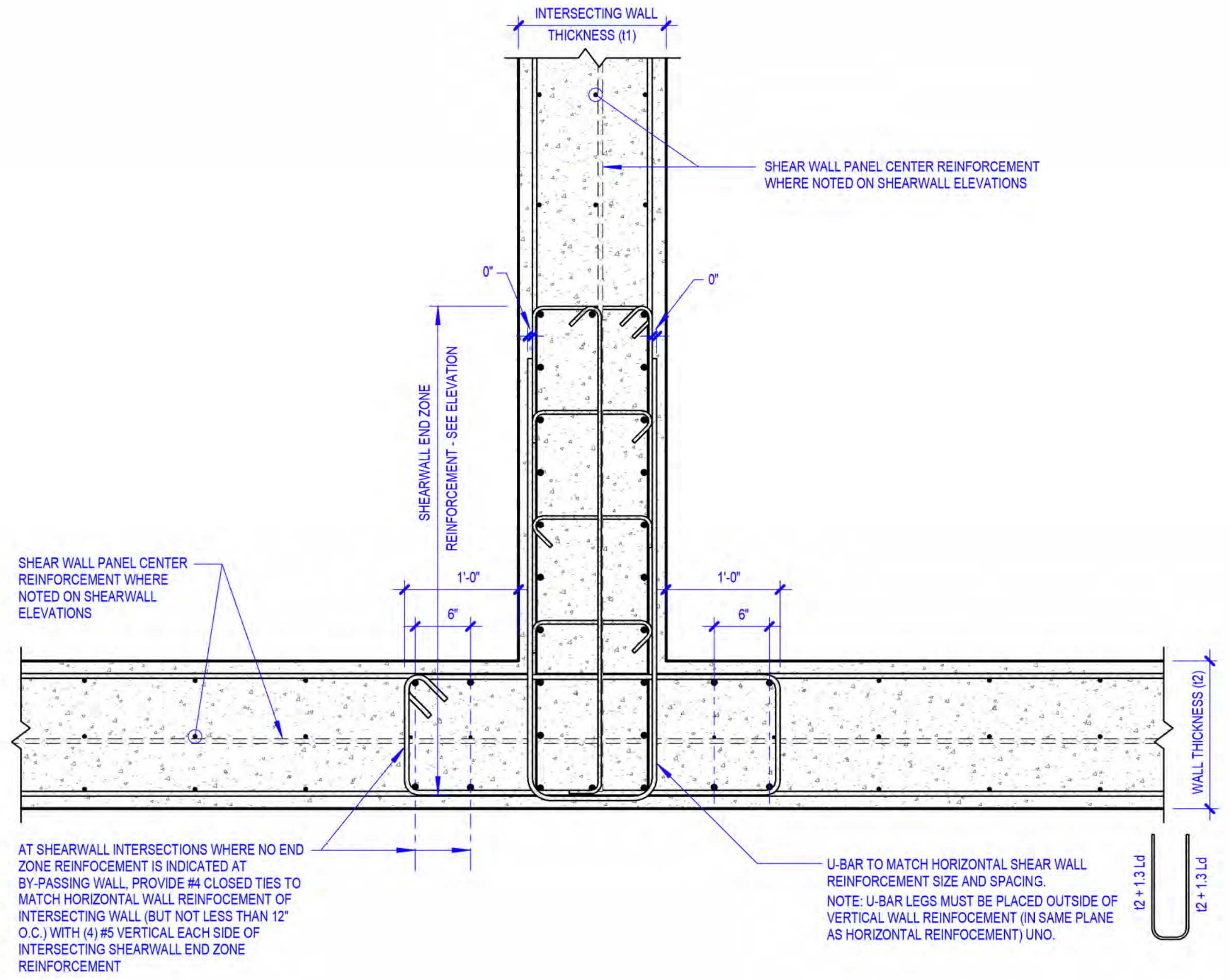
NOTE:
1. SEE SHEARWALL ELEVATIONS AND SHEARWALL END ZONE REINFORCEMENT DIAGRAM 4 / S5.200

3 Detail at Shear Wall End Zone
1" = 1'-0"


WALL THK (t1)
SHEAR WALL PANEL CENTER REINFORCEMENT WHERE NOTED ON SHEARWALL ELEVATIONS
0"
0"
(2) U-BAR TO MATCH HORIZONTAL SHEAR WALL REINFORCEMENT SIZE AND SPACING (BUT NOT LESS THAN #4 @ 12" O.C.)
48"
48"
48"
48"
SHEARWALL END ZONE
REINFORCEMENT - SEE ELEVATION
0"
0"
WALL THK (t2)
SHEARWALL END ZONE
REINFORCEMENT - SEE ELEVATION
SHEAR WALL PANEL CENTER REINFORCEMENT WHERE NOTED ON SHEARWALL ELEVATIONS

NOTE:
1. SEE SHEARWALL ELEVATIONS AND SHEARWALL END ZONE REINFORCEMENT DIAGRAM 4 / S5.200

2 Detail at Shearwall Corner End Zone
1" = 1'-0"


INTERSECTING WALL
THICKNESS (t1)
SHEAR WALL PANEL CENTER REINFORCEMENT WHERE NOTED ON SHEARWALL ELEVATIONS
0"
0"
SHEARWALL END ZONE
REINFORCEMENT - SEE ELEVATION
SHEAR WALL PANEL CENTER REINFORCEMENT WHERE NOTED ON SHEARWALL ELEVATIONS
1'-0"
6"
1'-0"
6"
WALL THICKNESS (t2)
AT SHEARWALL INTERSECTIONS WHERE NO END ZONE REINFORCEMENT IS INDICATED AT BY-PASSING WALL, PROVIDE #4 CLOSED TIES TO MATCH HORIZONTAL WALL REINFOCEMENT OF INTERSECTING WALL (BUT NOT LESS THAN 12" O.C.) WITH (4) #5 VERTICAL EACH SIDE OF INTERSECTING SHEARWALL END ZONE REINFORCEMENT
U-BAR TO MATCH HORIZONTAL SHEAR WALL REINFORCEMENT SIZE AND SPACING.
NOTE: U-BAR LEGS MUST BE PLACED OUTSIDE OF VERTICAL WALL REINFORCEMENT (IN SAME PLANE AS HORIZONTAL REINFOCEMENT) UNO.
t2 + 1.3 Ld
t2 + 1.3 Ld

NOTE:
1. SEE SHEARWALL ELEVATIONS AND SHEARWALL END ZONE REINFORCEMENT DIAGRAM 4 / S5.200

1 Detail at Shear Wall Intersection End Zone
1" = 1'-0"

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PROJECT # : 26154
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SHEET TITLE:
CIP Concrete Shearwall Typical Details

SHEET NUMBER:
S5.201

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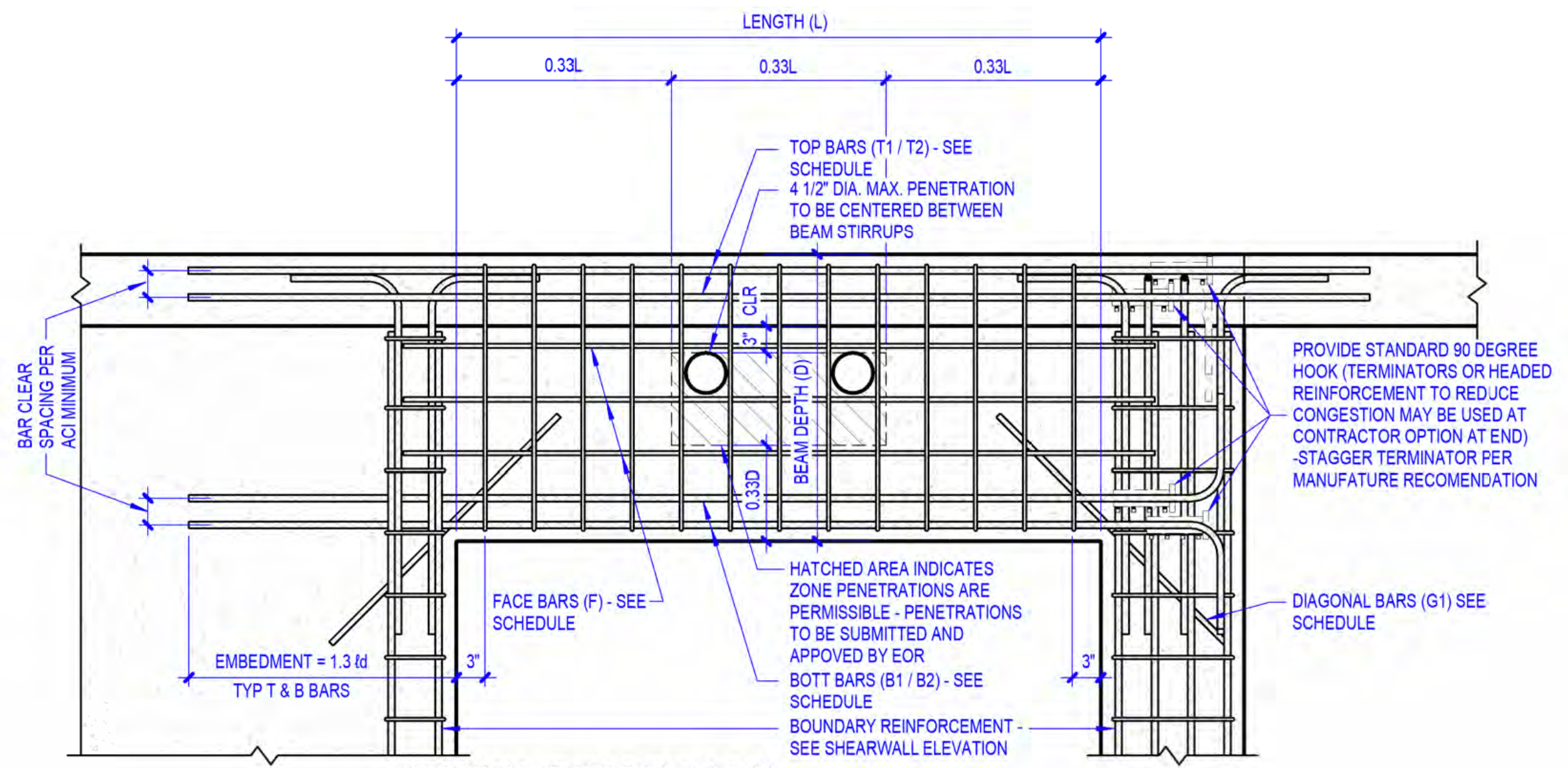
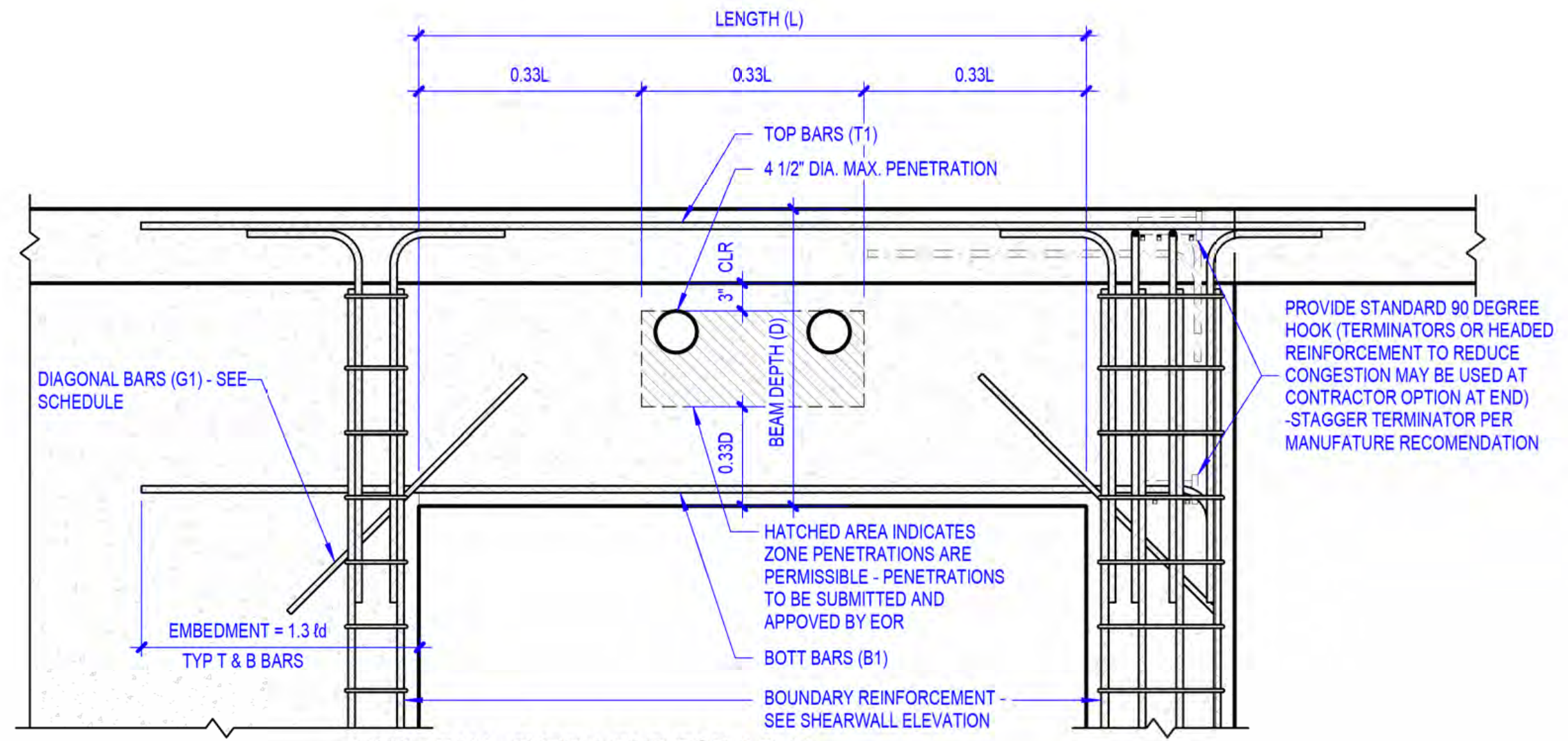
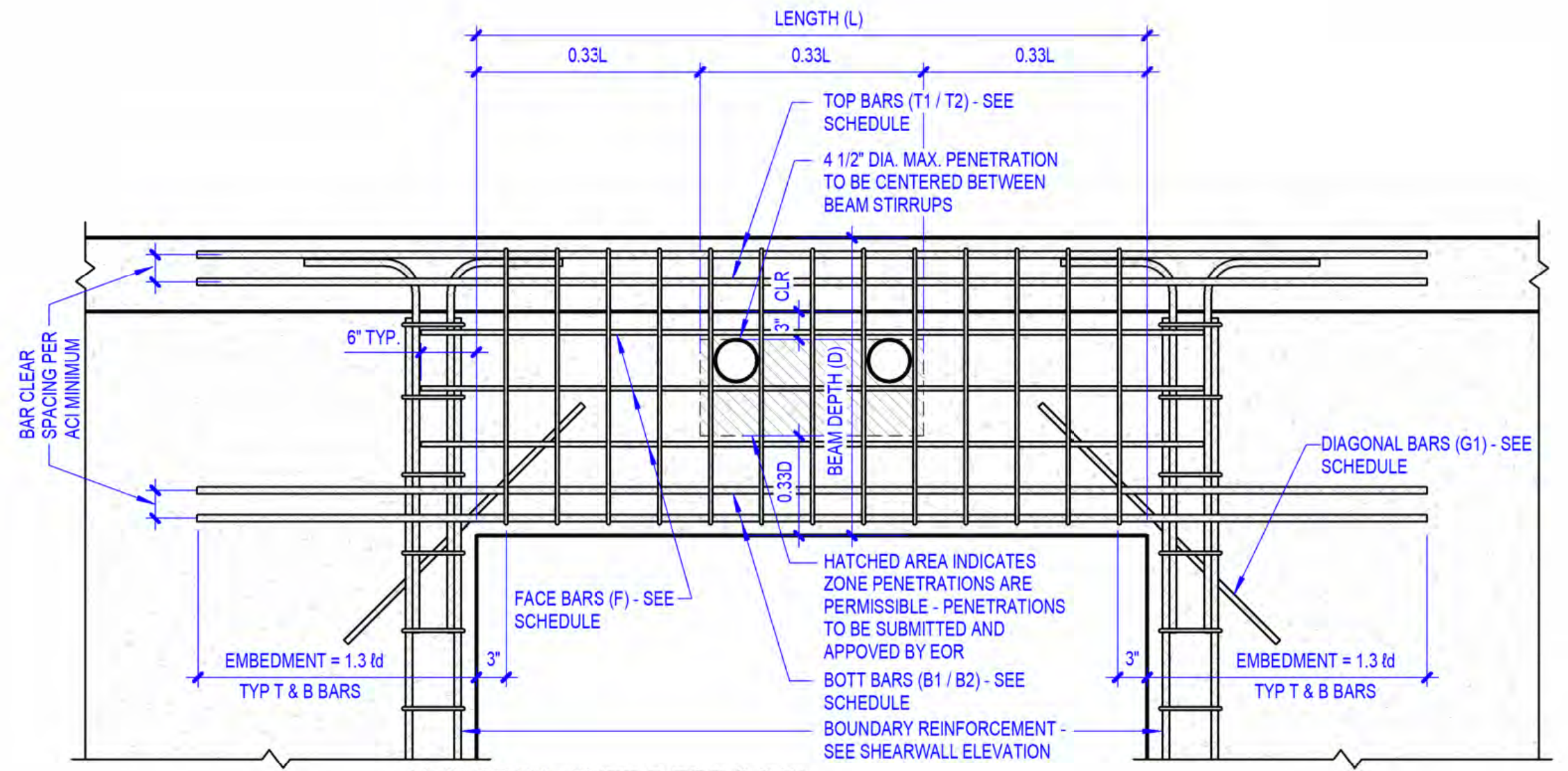

LENGTH (L)
0.33L
0.33L
0.33L
TOP BARS (T1 / T2) - SEE SCHEDULE
4 1/2" DIA. MAX. PENETRATION TO BE CENTERED BETWEEN BEAM STIRRUPS
BAR CLEAR SPACING PER ACI MINIMUM
CLR
3"
BEAM DEPTH (D)
0.33D
PROVIDE STANDARD 90 DEGREE HOOK (TERMINATORS OR HEADED REINFORCEMENT TO REDUCE CONGESTION MAY BE USED AT CONTRACTOR OPTION AT END) -STAGGER TERMINATOR PER MANUFATURE RECOMENDATION
HATCHED AREA INDICATES ZONE PENETRATIONS ARE PERMISSIBLE - PENETRATIONS TO BE SUBMITTED AND APPOVED BY EOR
FACE BARS (F) - SEE SCHEDULE
DIAGONAL BARS (G1) SEE SCHEDULE
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
3"
3"
BOTT BARS (B1 / B2) - SEE SCHEDULE
BOUNDARY REINFORCEMENT - SEE SHEARWALL ELEVATION

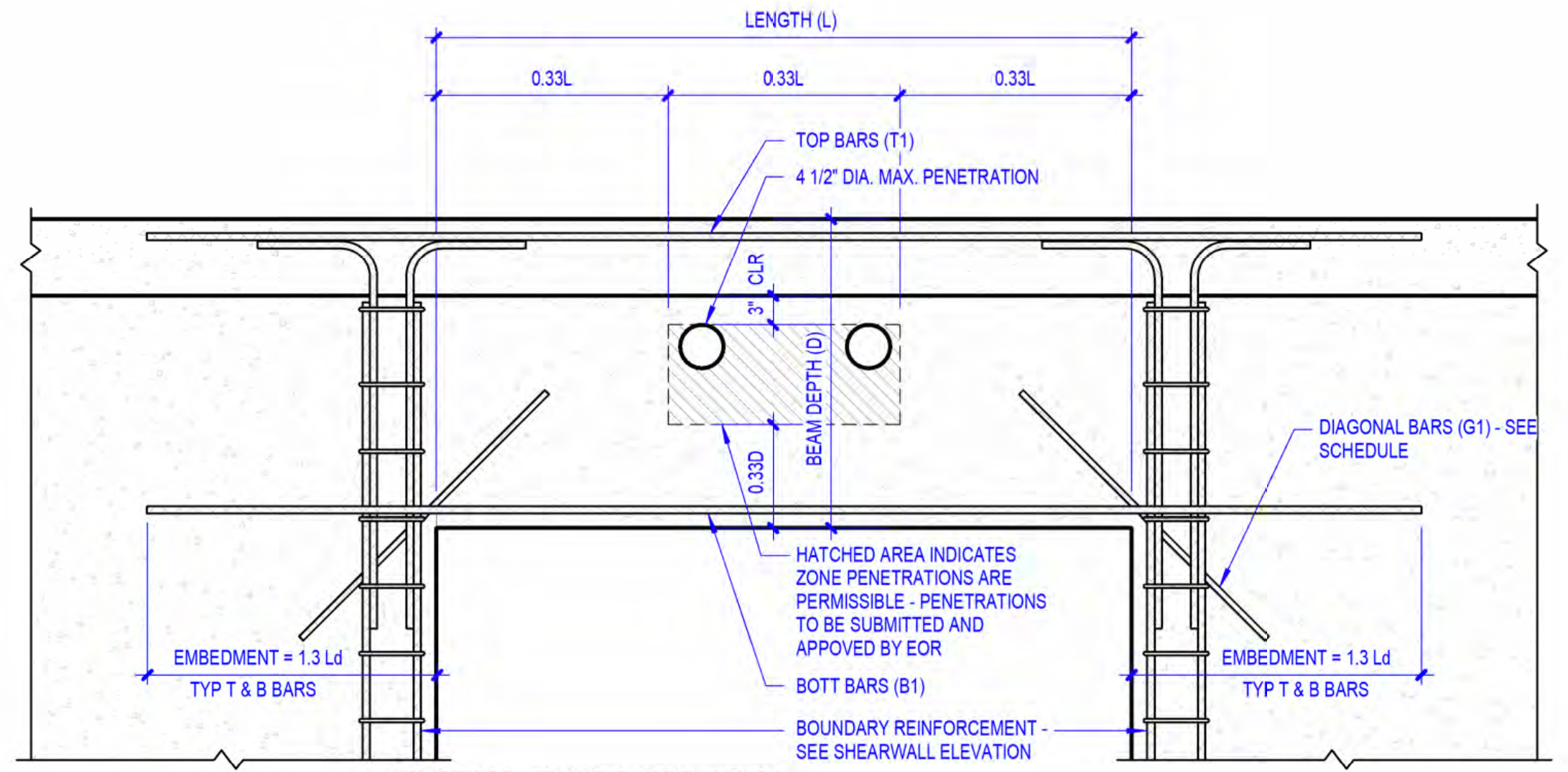
LINK BEAM - TYPE B END (LB-B)


LENGTH (L)
0.33L
0.33L
0.33L
TOP BARS (T1)
4 1/2" DIA. MAX. PENETRATION
CLR
3"
BEAM DEPTH (D)
0.33D
DIAGONAL BARS (G1) - SEE SCHEDULE
PROVIDE STANDARD 90 DEGREE HOOK (TERMINATORS OR HEADED REINFORCEMENT TO REDUCE CONGESTION MAY BE USED AT CONTRACTOR OPTION AT END) -STAGGER TERMINATOR PER MANUFATURE RECOMENDATION
HATCHED AREA INDICATES ZONE PENETRATIONS ARE PERMISSIBLE - PENETRATIONS TO BE SUBMITTED AND APPOVED BY EOR
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
BOTT BARS (B1)
BOUNDARY REINFORCEMENT - SEE SHEARWALL ELEVATION

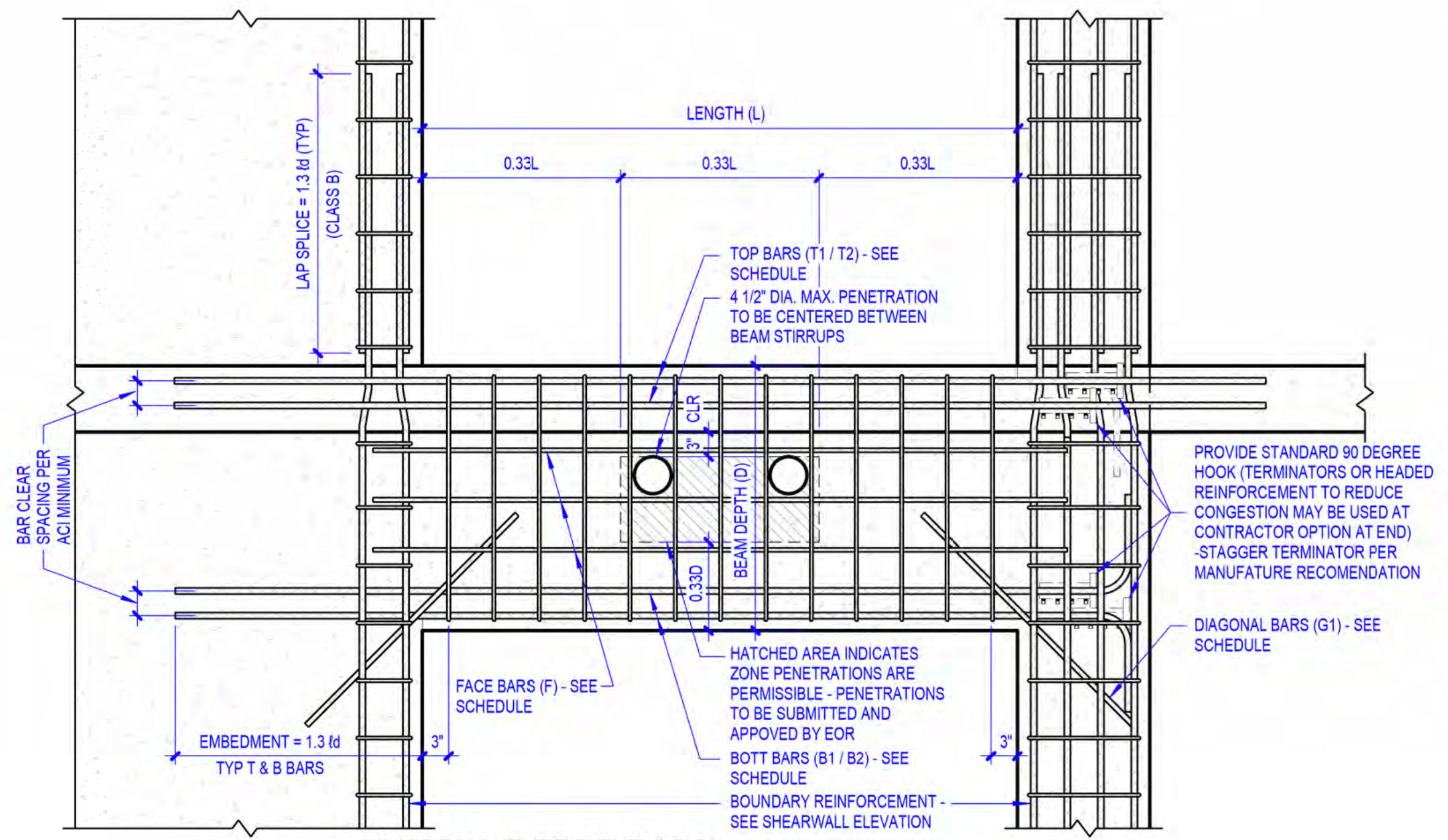
LINK BEAM - TYPE A END TOP (LB-A)


LENGTH (L)
0.33L
0.33L
0.33L
TOP BARS (T1 / T2) - SEE SCHEDULE
4 1/2" DIA. MAX. PENETRATION TO BE CENTERED BETWEEN BEAM STIRRUPS
BAR CLEAR SPACING PER ACI MINIMUM
6" TYP.
CLR
3"
BEAM DEPTH (D)
0.33D
DIAGONAL BARS (G1) - SEE SCHEDULE
FACE BARS (F) - SEE SCHEDULE
HATCHED AREA INDICATES ZONE PENETRATIONS ARE PERMISSIBLE - PENETRATIONS TO BE SUBMITTED AND APPOVED BY EOR
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
3"
3"
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
BOTT BARS (B1 / B2) - SEE SCHEDULE
BOUNDARY REINFORCEMENT - SEE SHEARWALL ELEVATION

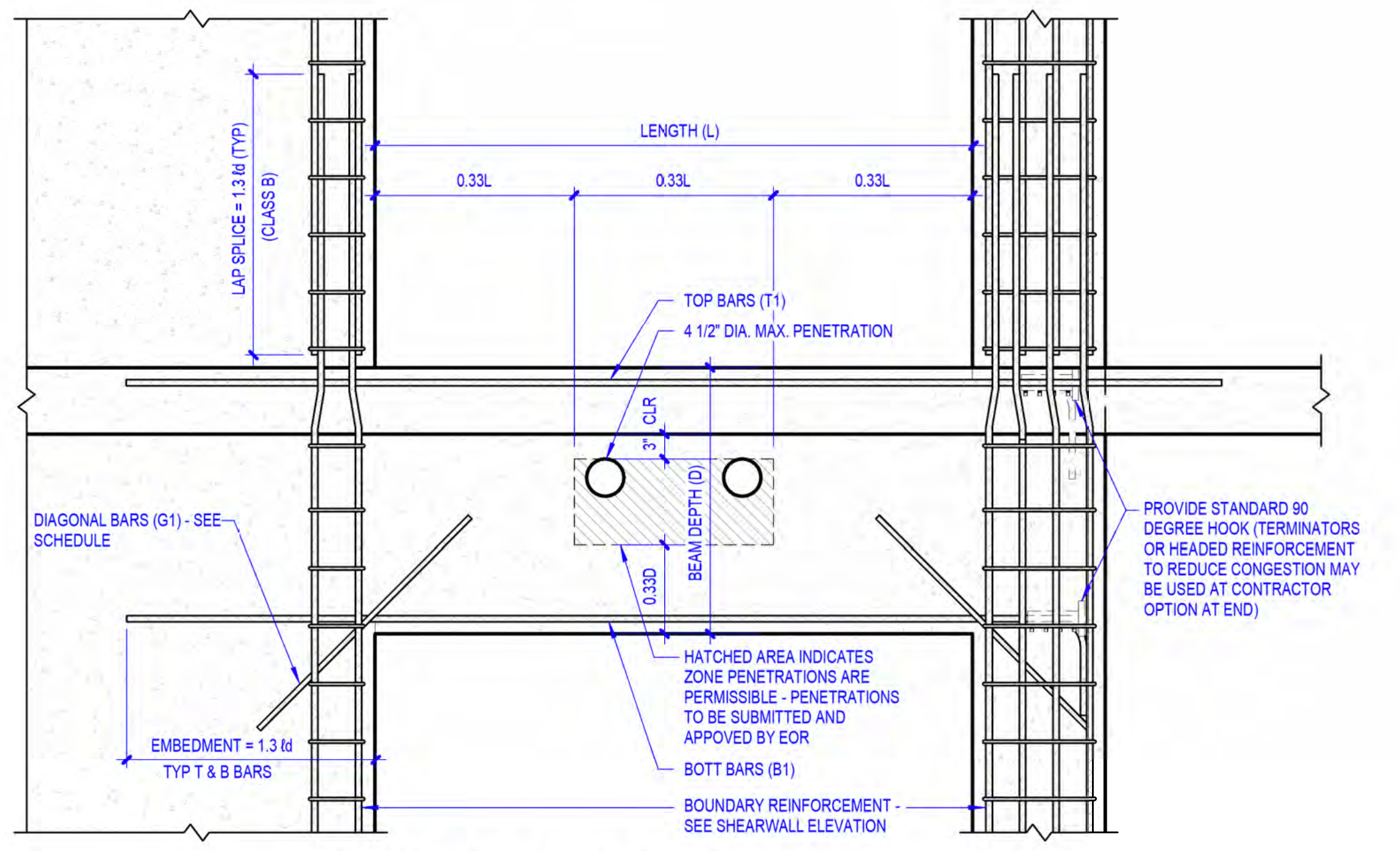
LINK BEAM - TYPE B TOP (LB-B)


LENGTH (L)
0.33L
0.33L
0.33L
TOP BARS (T1)
4 1/2" DIA. MAX. PENETRATION
CLR
3"
BEAM DEPTH (D)
0.33D
DIAGONAL BARS (G1) - SEE SCHEDULE
HATCHED AREA INDICATES ZONE PENETRATIONS ARE PERMISSIBLE - PENETRATIONS TO BE SUBMITTED AND APPOVED BY EOR
EMBEDMENT = 1.3 Ld
TYP T & B BARS
EMBEDMENT = 1.3 Ld
TYP T & B BARS
BOTT BARS (B1)
BOUNDARY REINFORCEMENT - SEE SHEARWALL ELEVATION

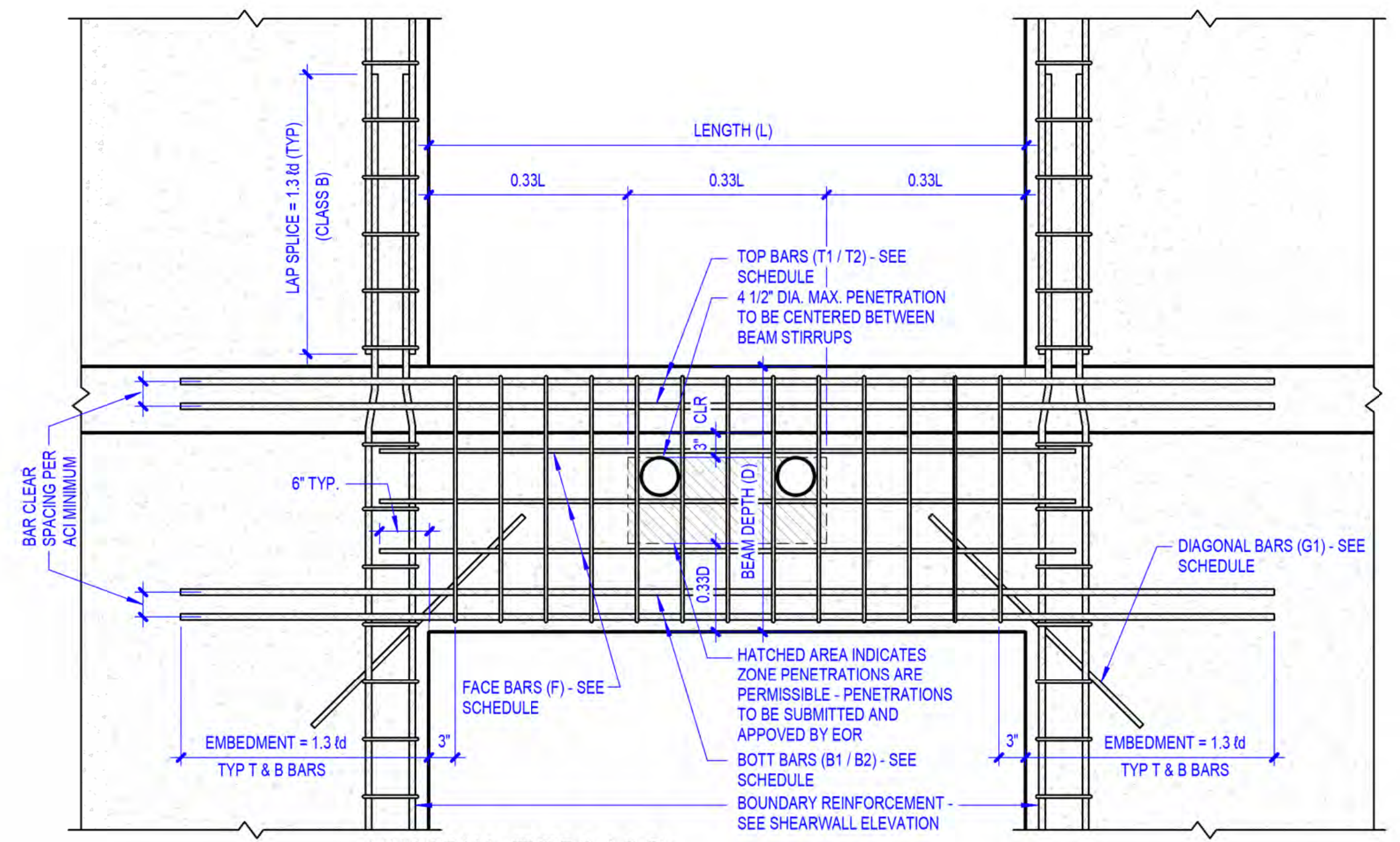
LINK BEAM - TYPE A TOP (LB-A)


LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
LENGTH (L)
0.33L
0.33L
0.33L
TOP BARS (T1 / T2) - SEE SCHEDULE
4 1/2" DIA. MAX. PENETRATION TO BE CENTERED BETWEEN BEAM STIRRUPS
BAR CLEAR SPACING PER ACI MINIMUM
CLR
3"
BEAM DEPTH (D)
0.33D
PROVIDE STANDARD 90 DEGREE HOOK (TERMINATORS OR HEADED REINFORCEMENT TO REDUCE CONGESTION MAY BE USED AT CONTRACTOR OPTION AT END) -STAGGER TERMINATOR PER MANUFATURE RECOMENDATION
DIAGONAL BARS (G1) - SEE SCHEDULE
HATCHED AREA INDICATES ZONE PENETRATIONS ARE PERMISSIBLE - PENETRATIONS TO BE SUBMITTED AND APPOVED BY EOR
FACE BARS (F) - SEE SCHEDULE
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
3"
3"
BOTT BARS (B1 / B2) - SEE SCHEDULE
BOUNDARY REINFORCEMENT - SEE SHEARWALL ELEVATION

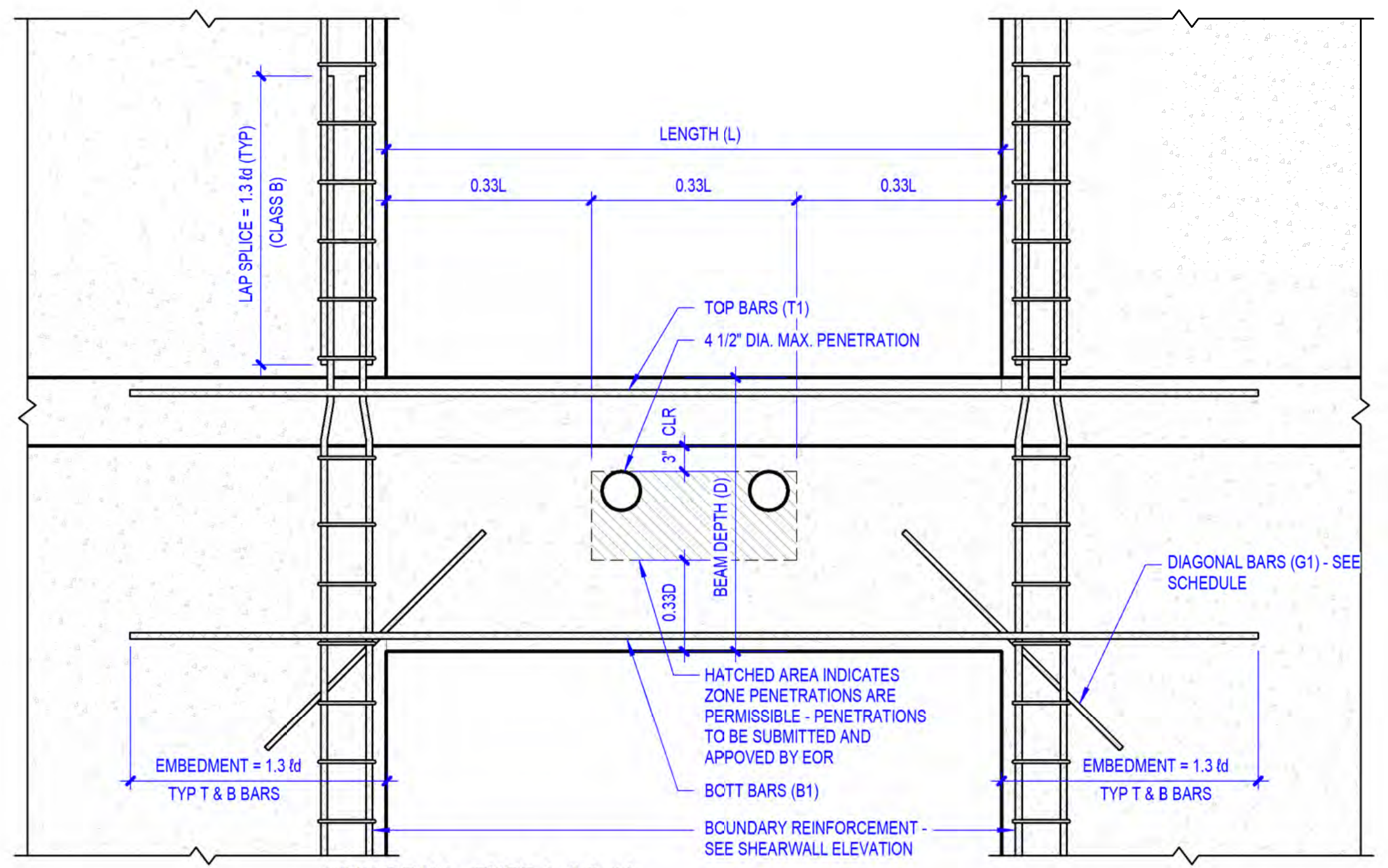
LINK BEAM - TYPE B END (LB-B)


LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
LENGTH (L)
0.33L
0.33L
0.33L
TOP BARS (T1)
4 1/2" DIA. MAX. PENETRATION
CLR
3"
BEAM DEPTH (D)
0.33D
DIAGONAL BARS (G1) - SEE SCHEDULE
PROVIDE STANDARD 90 DEGREE HOOK (TERMINATORS OR HEADED REINFORCEMENT TO REDUCE CONGESTION MAY BE USED AT CONTRACTOR OPTION AT END)
HATCHED AREA INDICATES ZONE PENETRATIONS ARE PERMISSIBLE - PENETRATIONS TO BE SUBMITTED AND APPOVED BY EOR
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
BOTT BARS (B1)
BOUNDARY REINFORCEMENT - SEE SHEARWALL ELEVATION

LINK BEAM - TYPE A END (LB-A)


LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
LENGTH (L)
0.33L
0.33L
0.33L
TOP BARS (T1 / T2) - SEE SCHEDULE
4 1/2" DIA. MAX. PENETRATION TO BE CENTERED BETWEEN BEAM STIRRUPS
BAR CLEAR SPACING PER ACI MINIMUM
6" TYP.
CLR
3"
BEAM DEPTH (D)
0.33D
DIAGONAL BARS (G1) - SEE SCHEDULE
FACE BARS (F) - SEE SCHEDULE
HATCHED AREA INDICATES ZONE PENETRATIONS ARE PERMISSIBLE - PENETRATIONS TO BE SUBMITTED AND APPOVED BY EOR
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
3"
3"
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
BOTT BARS (B1 / B2) - SEE SCHEDULE
BOUNDARY REINFORCEMENT - SEE SHEARWALL ELEVATION

LINK BEAM - TYPE B (LB-B)


LAP SPLICE = 1.3 ℓd (TYP) (CLASS B)
LENGTH (L)
0.33L
0.33L
0.33L
TOP BARS (T1)
4 1/2" DIA. MAX. PENETRATION
CLR
3"
BEAM DEPTH (D)
0.33D
DIAGONAL BARS (G1) - SEE SCHEDULE
HATCHED AREA INDICATES ZONE PENETRATIONS ARE PERMISSIBLE - PENETRATIONS TO BE SUBMITTED AND APPOVED BY EOR
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
EMBEDMENT = 1.3 ℓd
TYP T & B BARS
BOTT BARS (B1)
BOUNDARY REINFORCEMENT - SEE SHEARWALL ELEVATION

LINK BEAM - TYPE A (LB-A)

① Link Beam Diagram
3/4" = 1'-0"

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Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
CIP Concrete Shearwall Typical Details

SHEET NUMBER:
S5.202

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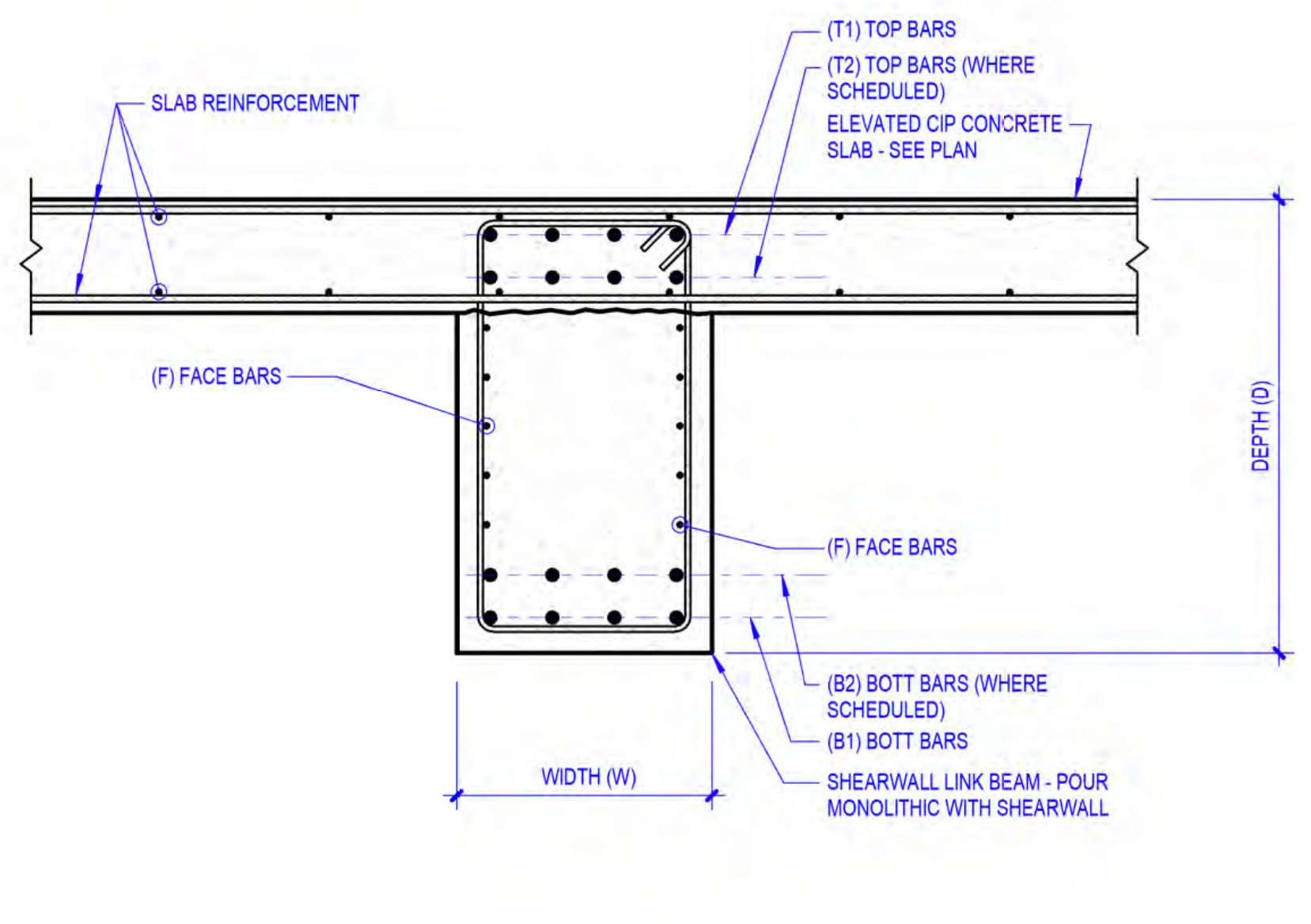
NOTES:

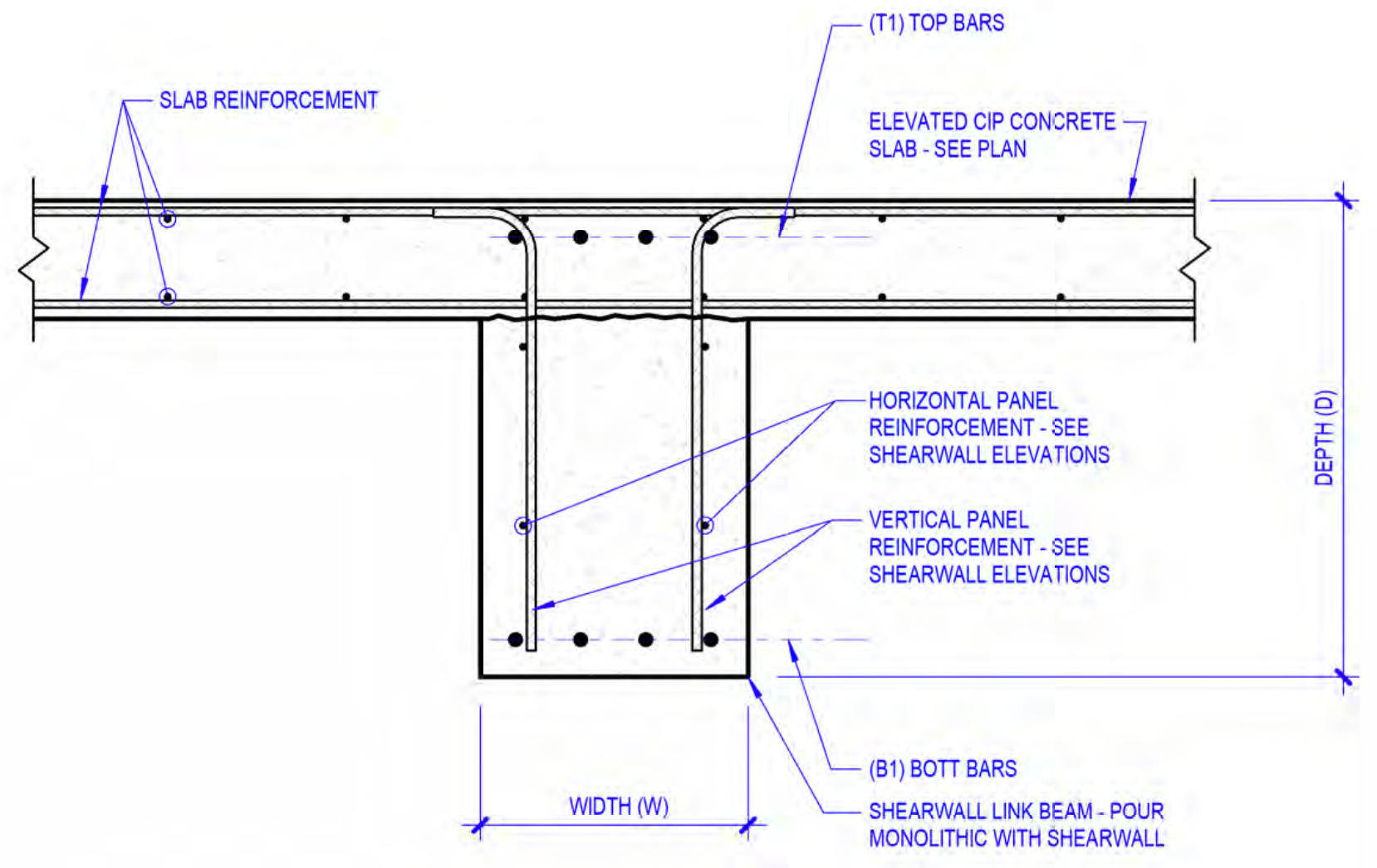
1. SEE SHEAR WALL ELEVATIONS FOR LINK BEAM ID.
2. VERTICAL PENETRATIONS ARE NOT PERMITTED THROUGH LINK BEAMS AT ANY LOCATION. ANY HORIZONTAL PENETRATIONS (INCLUDING IN SLAB CONDUIT PASSING THOUGH OPENING) SHALL BE COORDINATED WITH EOR PRIOR TO SUBMITTING SHOP DRAWINGS FOR LINK BEAM REINFORCEMENT.
3. LINK BEAM SHALL HAVE FULLY CLOSED OUTER STIRRUP W/ 135 DEGREE HOOKS (WHERE SCHEDULED).
4. IN LINK BEAMS WITH MORE THAN TWO LEG STIRRUPS SCHEDULED, ADDITIONAL TIES MAY BE SINGLE LEG TIES W/ 90 DEGREE HOOK ON ONE END AND 135 DEGREE HOOK ON OPPOSITE END.

CIP-Link Beam Schedule - VSMsq

Link Beam ID	Shearwall ID	Width	D (Depth)	T1 (Top Bars)	T2 (Top Bars)	B1 (Bottom Bars)	B2 (Bottom Bars)	F (Face Bars)	S (Stirrups)	G1 (Diagonal Bars)	G2 (Diagonal Bars)	G3 (Diagonal Bars)


(T1) TOP BARS
(T2) TOP BARS (WHERE SCHEDULED)
SLAB REINFORCEMENT
ELEVATED CIP CONCRETE SLAB - SEE PLAN
(F) FACE BARS
DEPTH (D)
(F) FACE BARS
(B2) BOTT BARS (WHERE SCHEDULED)
(B1) BOTT BARS
WIDTH (W)
SHEARWALL LINK BEAM - POUR MONOLITHIC WITH SHEARWALL

LINK BEAM TYPE B


(T1) TOP BARS
SLAB REINFORCEMENT
ELEVATED CIP CONCRETE SLAB - SEE PLAN
HORIZONTAL PANEL REINFORCEMENT - SEE SHEARWALL ELEVATIONS
DEPTH (D)
VERTICAL PANEL REINFORCEMENT - SEE SHEARWALL ELEVATIONS
(B1) BOTT BARS
WIDTH (W)
SHEARWALL LINK BEAM - POUR MONOLITHIC WITH SHEARWALL

LINK BEAM TYPE A

① Section at Link Beam
1" = 1'-0"

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SHEET TITLE:
CIP Concrete Shearwall Typical Details

SHEET NUMBER:
S5.203

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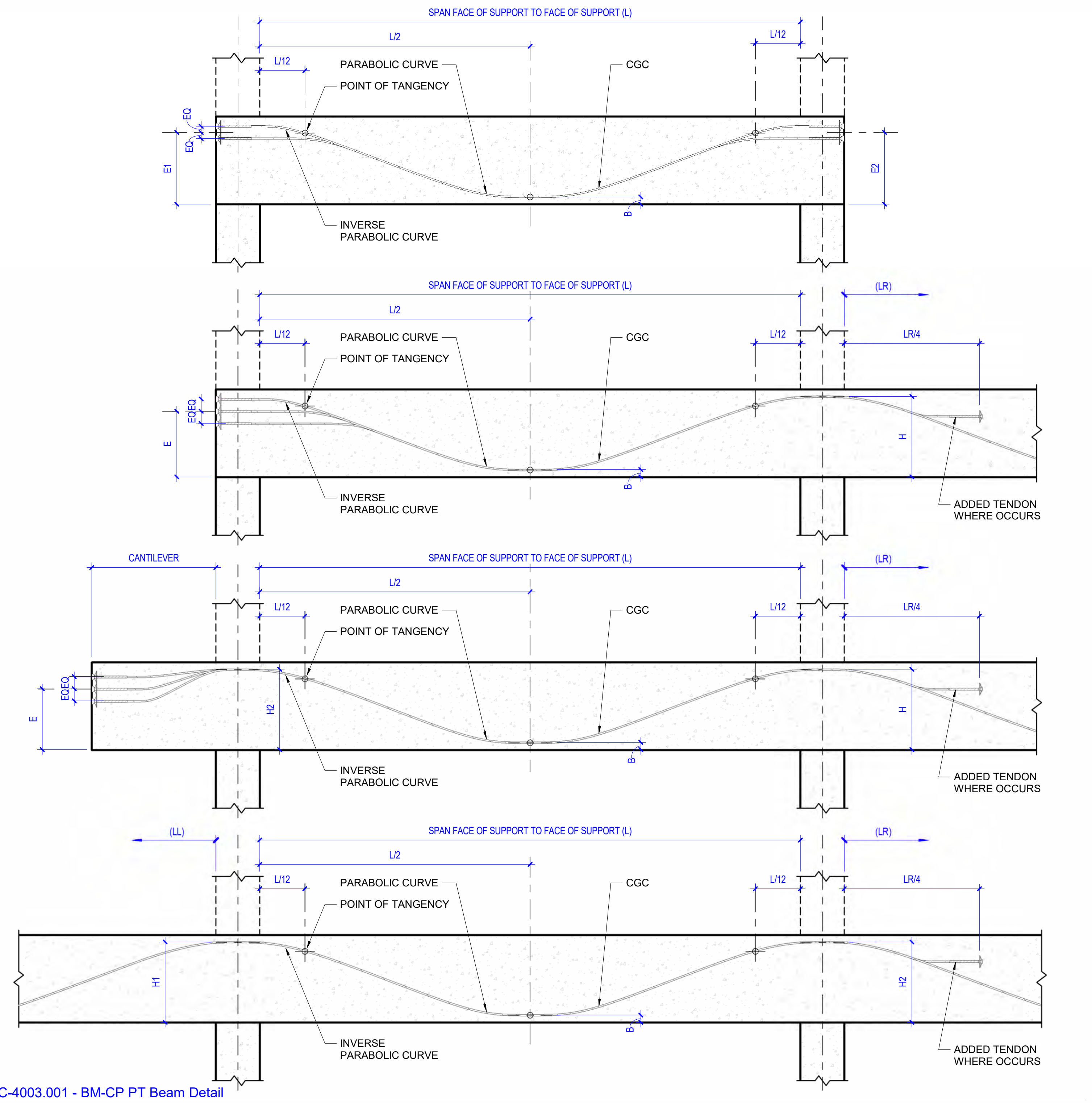
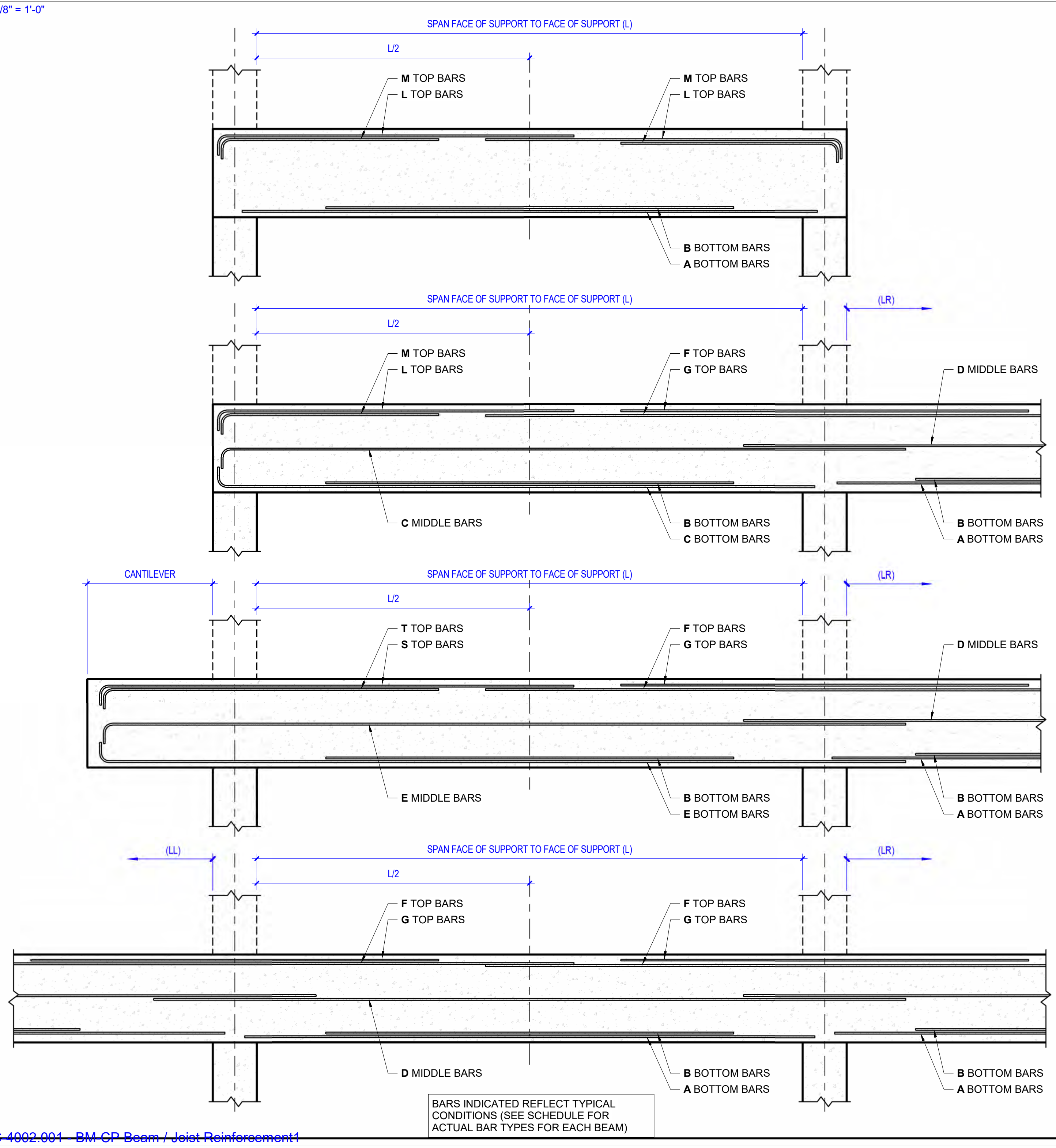


C-4003.001 - BM-CP PT Beam Detail
3/8" = 1'-0"



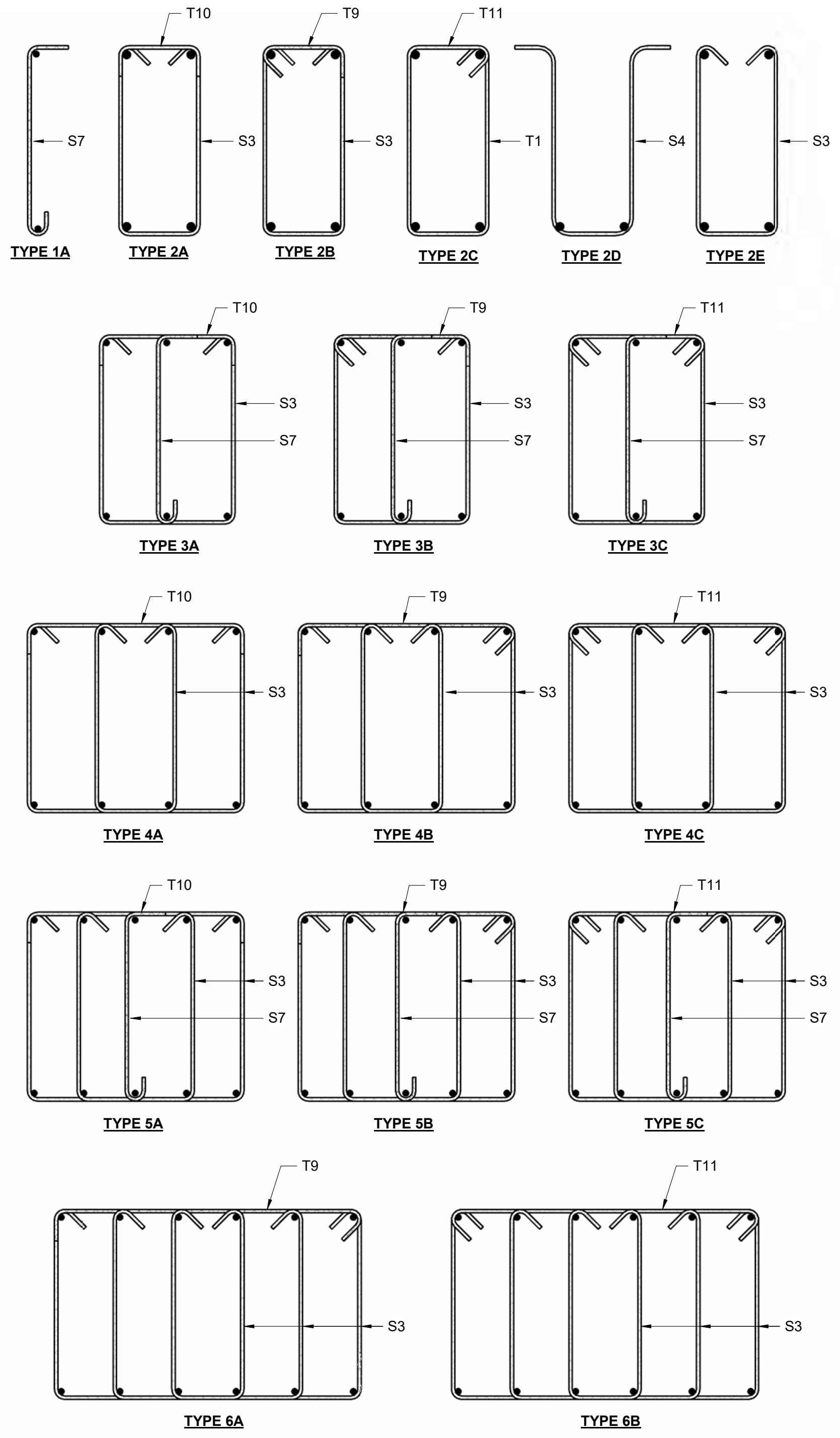
BARS INDICATED REFLECT TYPICAL CONDITIONS (SEE SCHEDULE FOR ACTUAL BAR TYPES FOR EACH BEAM)

C-4002.001 - BM-CP Beam / Joist Reinforcement1
3/8" = 1'-0"

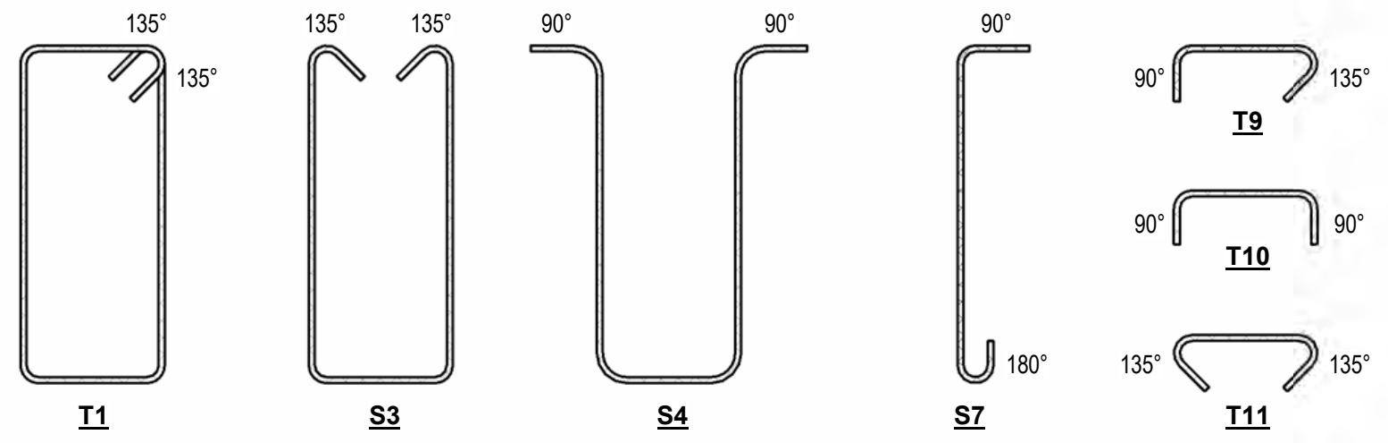
TYPICAL STIRRUP NOTES:

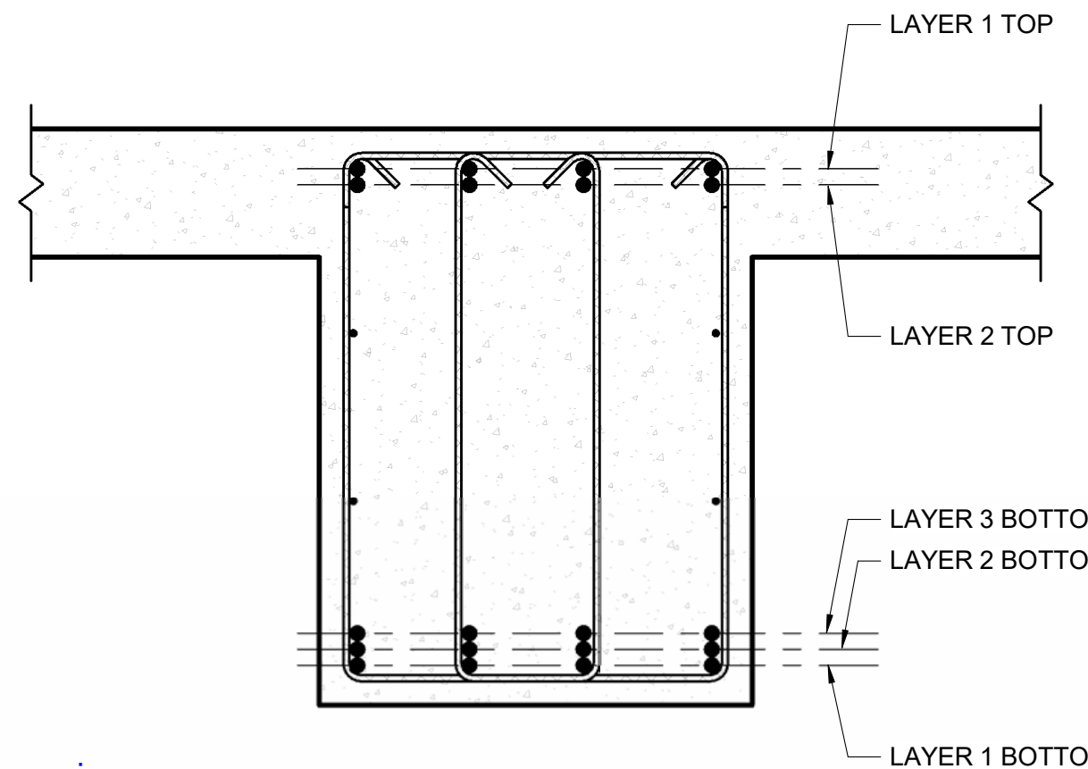
1. ALL BENDS AND HOOKS TO BE FABRICATED PER ACI GUIDELINES.
2. STEEL FABRICATOR SHALL DENOTE BAR TYPES ON SHOP DRAWINGS USING THE SAME BAR DESIGNATIONS SHOWN ON STRUCTURAL DRAWINGS.
3. ALL STIRRUP LEGS MUST BE HOOKED AROUND A LONGITUDINAL BEAM BAR FOR SUPPORT. IF THE NUMBER OF SCHEDULED TOP OR BOTTOM BARS AT ANY POINT ALONG THE BEAM IS LESS THAN THE NUMBER OF STIRRUP LEGS, PROVIDE ADDITIONAL LONGITUDINAL #5 BARS AS NEEDED SO EACH STIRRUP IS HOOKED AROUND A LONGITUDINAL BEAM BAR. LAP ADDITIONAL SUPPORT BAR 2'-0" WITH SCHEDULED LONGITUDINAL BARS OR MAKE CONTINUOUS IF NECESSARY.
4. TOP CLOSURE BAR SHALL BE PLACED SO THAT 135° HOOK IS NOT ON THE SIDE CONFINED BY THE SLAB. IN THE CASE WHERE THERE IS NO ADJACENT SLAB, STIRRUP TYPE 2C, 4C, OR 6B SHALL BE PROVIDED WITH TWO 135° HOOKS ON TOP CLOSURE BAR.
5. BEAM STIRRUP SPACING IS MEASURED FROM EACH END STARTING AT SUPPORT FACE. FOR STIRRUP SPACING, R = REMAINDER.
6. LONGITUDINAL REINFORCEMENT SHOWN IS FOR GRAPHICAL PURPOSES ONLY. SIZE AND QUANTITY OF BARS TO BE PER SCHEDULES.

BEAM STIRRUP TYPES

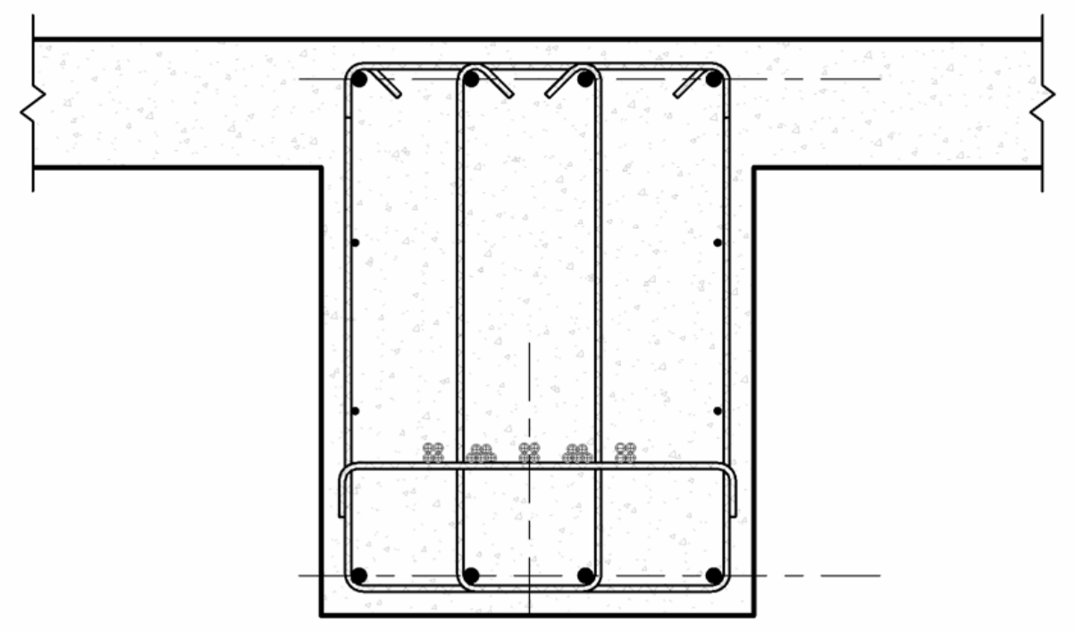


STIRRUP BENDING DIAGRAMS



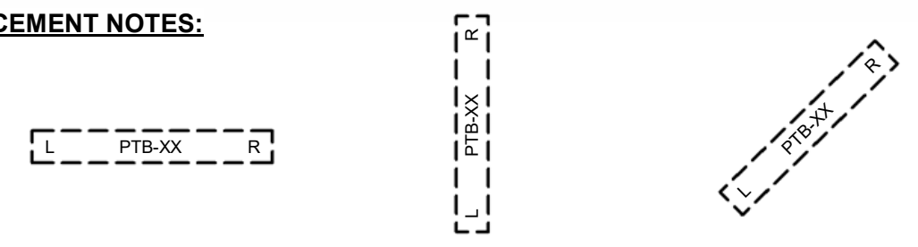


Beam Bar Layering
1" = 1'-0"

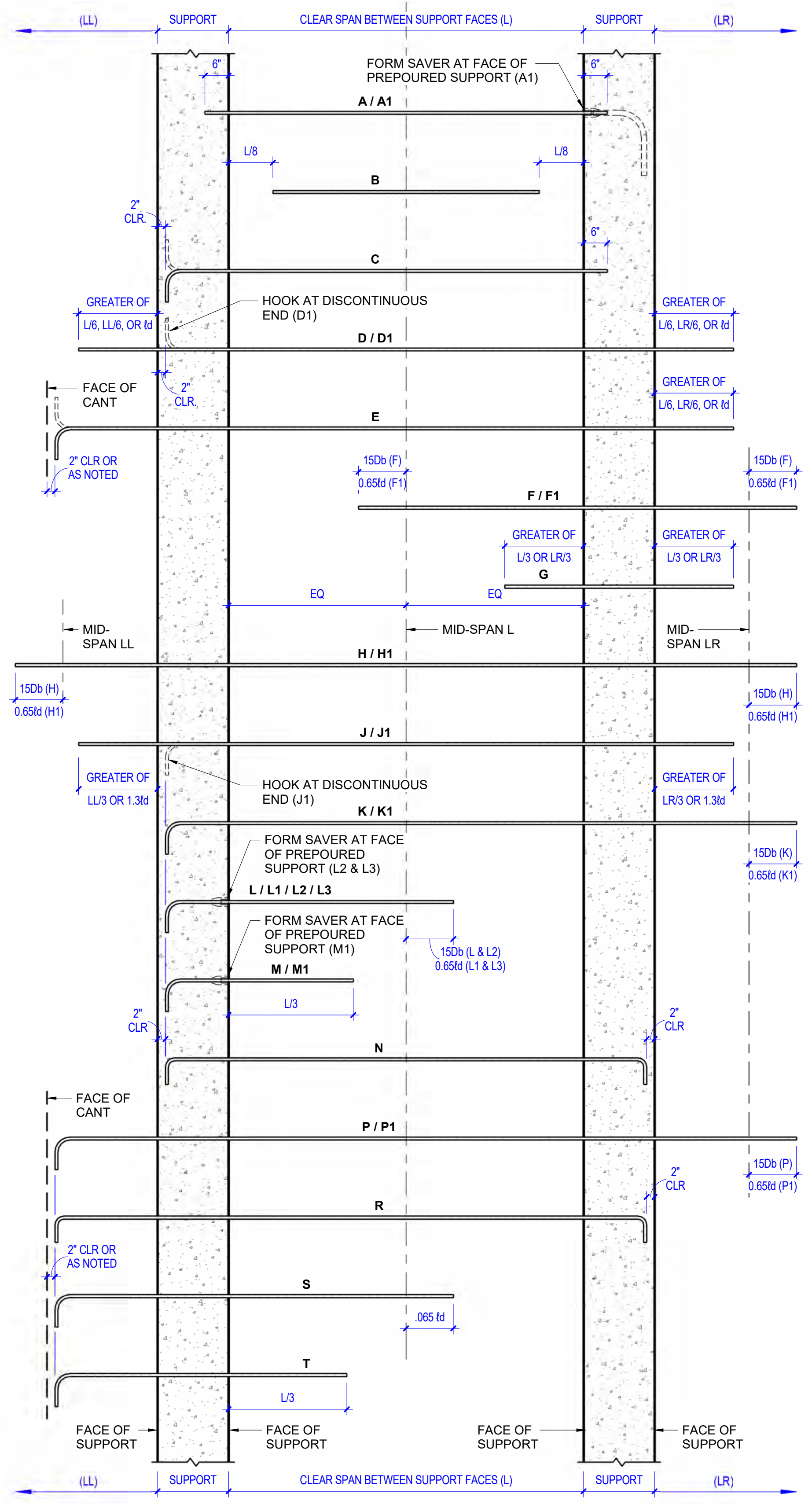


Beam Bar Layering
1" = 1'-0"

TYPICAL BEAM BAR PLACEMENT NOTES:



1. SCHEDULED BEAM LEFT AND RIGHT ENDS ARE RELATIVE TO PLAN LEFT AND RIGHT OR BOTTOM AND TOP AS SHOWN ABOVE. DIAGONAL BEAM LEFT AND RIGHT ENDS ARE LOCATED AS SHOWN AS WELL.
2. STEEL FABRICATOR SHALL DENOTE BAR TYPES ON SHOP DRAWINGS USING THE SAME BAR DESIGNATIONS SHOWN ON STRUCTURAL DRAWINGS.
3. ALL HOOKS SHOWN ARE STANDARD 90° HOOKS. SUBSTITUTE 180° END HOOKS FOR BARS WHERE 90 DEGREE HOOK WILL NOT FIT IN THE FORMS.
4. UNLESS NOTED OTHERWISE, TOP BEAM REINFORCEMENT HOOKS SHALL BE ORIENTED DOWNWARD AND BOTTOM BEAM REINFORCEMENT HOOKS SHALL BE ORIENTED UPWARD.
5. ℓd = TENSION DEVELOPMENT LENGTH.
6. SPLICES IN SCHEDULED BARS LESS THAN 60'-0" IN LENGTH ARE NOT PERMITTED. SPLICES IN SCHEDULED BARS MORE THAN 60'-0" IN LENGTH SHALL BE SHOWN CLEARLY ON SHOPS DRAWINGS FOR EOR REVIEW AND APPROVAL.
7. ALL BEAM FACE REINFORCEMENT (WHEN NOTED IN SCHEDULE) SHALL EXTEND ℓd BEYOND CENTER OF SUPPORT. AT DISCONTINUOUS ENDS, PROVIDE 90 DEGREE HOOK ON ALL FACE REINFORCEMENT.
8. INDIVIDUAL BEAM REMARKS AS REQUIRED (REFERENCE SCHEDULE NOTE COLUMN):



BEAM BAR BENDING DIAGRAM

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Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
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SHEET TITLE:
CIP Concrete Beams

SHEET NUMBER:
S5.300

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CIP-PTB Beam Schedule Schedule - VSMsq

Type	Width (W)	Depth (D)	Effective Post Tension Force	PT Drape - Left (A)	PT Drape - Center (B)	PT Drape - Right (C)	Reinforcement (Top Left)	Reinforcement (Bottom Center)	Reinforcement (Top Center)	Reinforcement (Top Right)	Reinforcement (Face)	Stirrup - Size	Stirrup - No Legs	Stirrup - Spacing	Beam Notes
22 X 18	1' - 10"	1' - 6"	0.00 kip	0"	0"	0"								0"	
24 X 18	2' - 0"	1' - 6"	0.00 kip	0"	0"	0"								0"	
24 X 30	2' - 0"	2' - 6"	0.00 kip	0"	0"	0"								0"	
36 X 18	3' - 0"	1' - 6"	0.00 kip	0"	0"	0"								0"	

CIP-RCB Beam Schedule Schedule - VSMsq

Beam ID	Width (W)	Depth (D)	Reinforcement (Top Left)	Reinforcement (Bottom Center)	Reinforcement (Top Right)	Reinforcement (Face)	Stirrup - Size	Stirrup - No Legs	Stirrup - Spacing	Beam Notes

FOR BAR TYPICAL BEAM DETAILS AND BENDING DIAGRAM SEE SHEET S5.300

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SHEET TITLE:
CIP Concrete Beam Schedule

SHEET NUMBER:
S5.310

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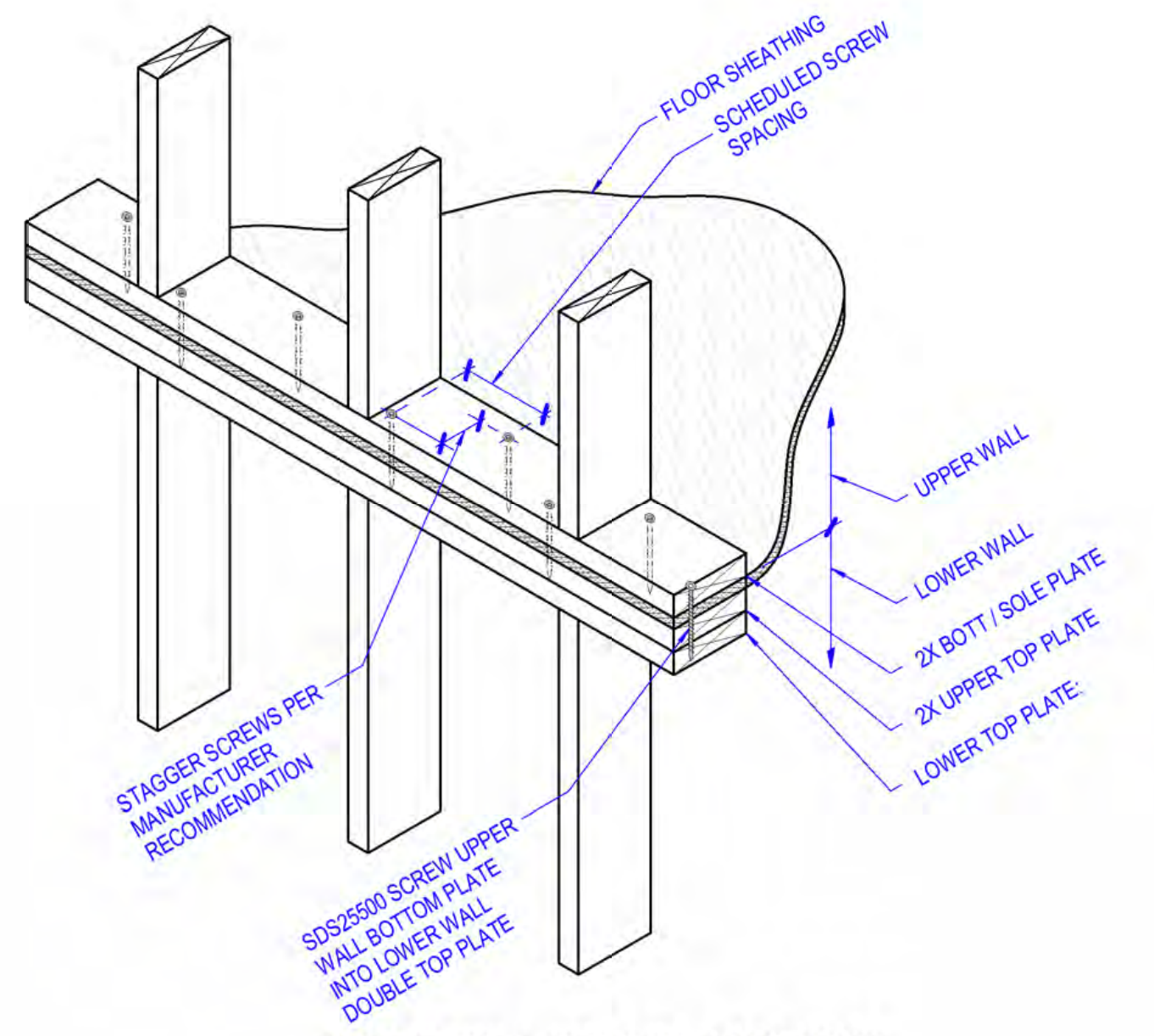
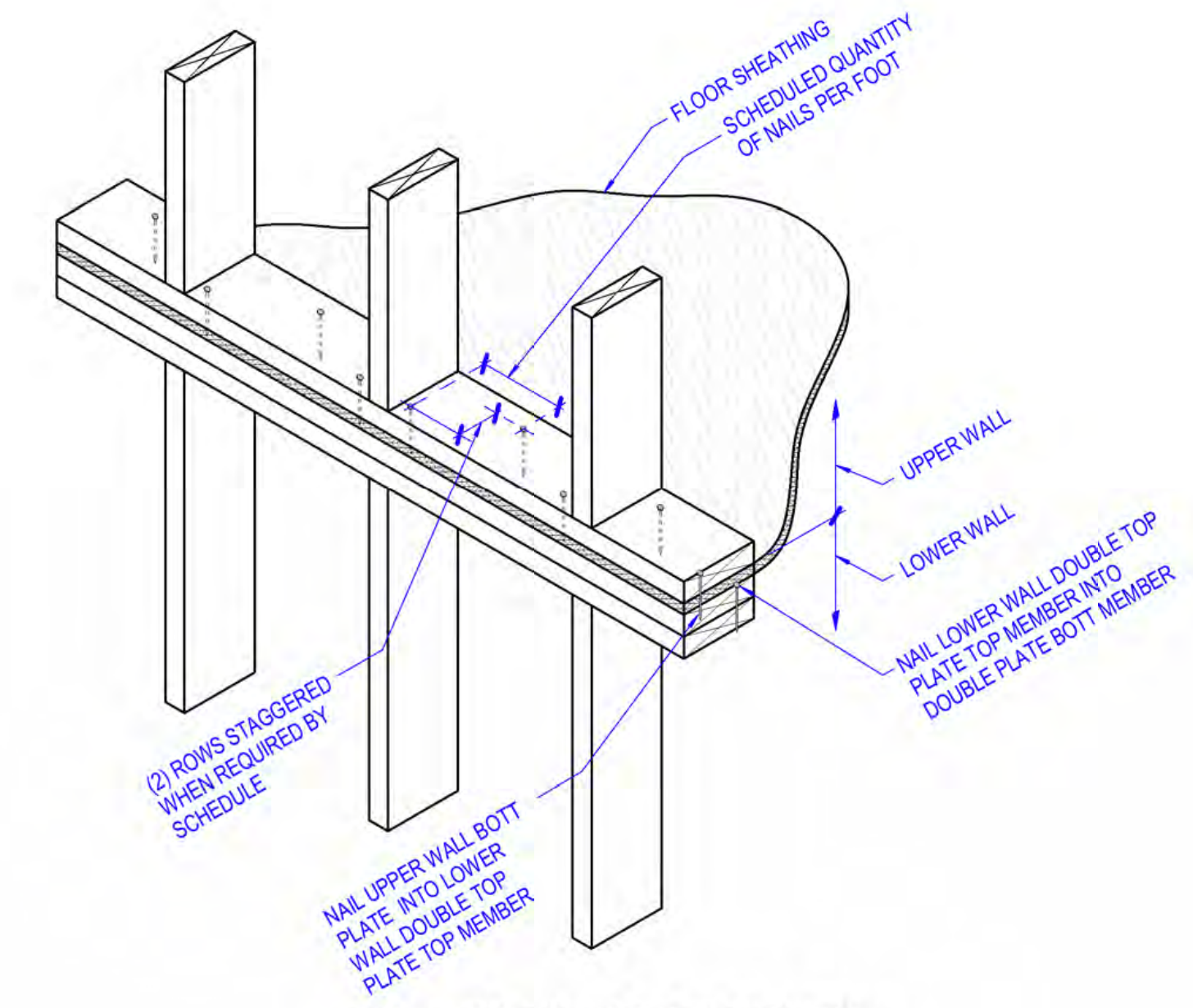
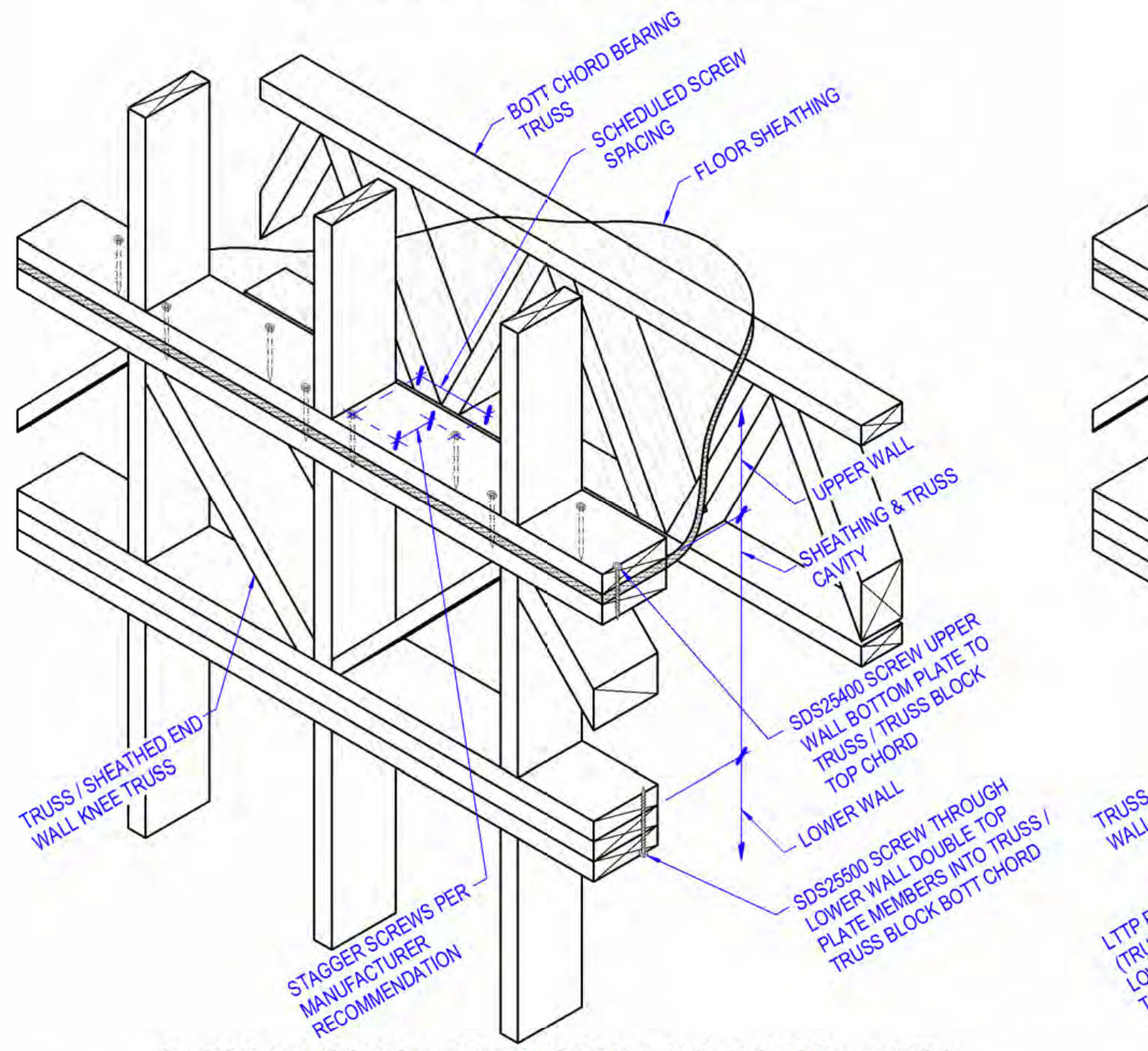

FLOOR SHEATHING
SCHEDULED SCREW SPACING
UPPER WALL
LOWER WALL
2X BOTT / SOLE PLATE
2X UPPER TOP PLATE
LOWER TOP PLATE
STAGGER SCREWS PER MANUFACTURER RECOMMENDATION
SDS25500 SCREW UPPER WALL BOTTOM PLATE INTO LOWER WALL DOUBLE TOP PLATE

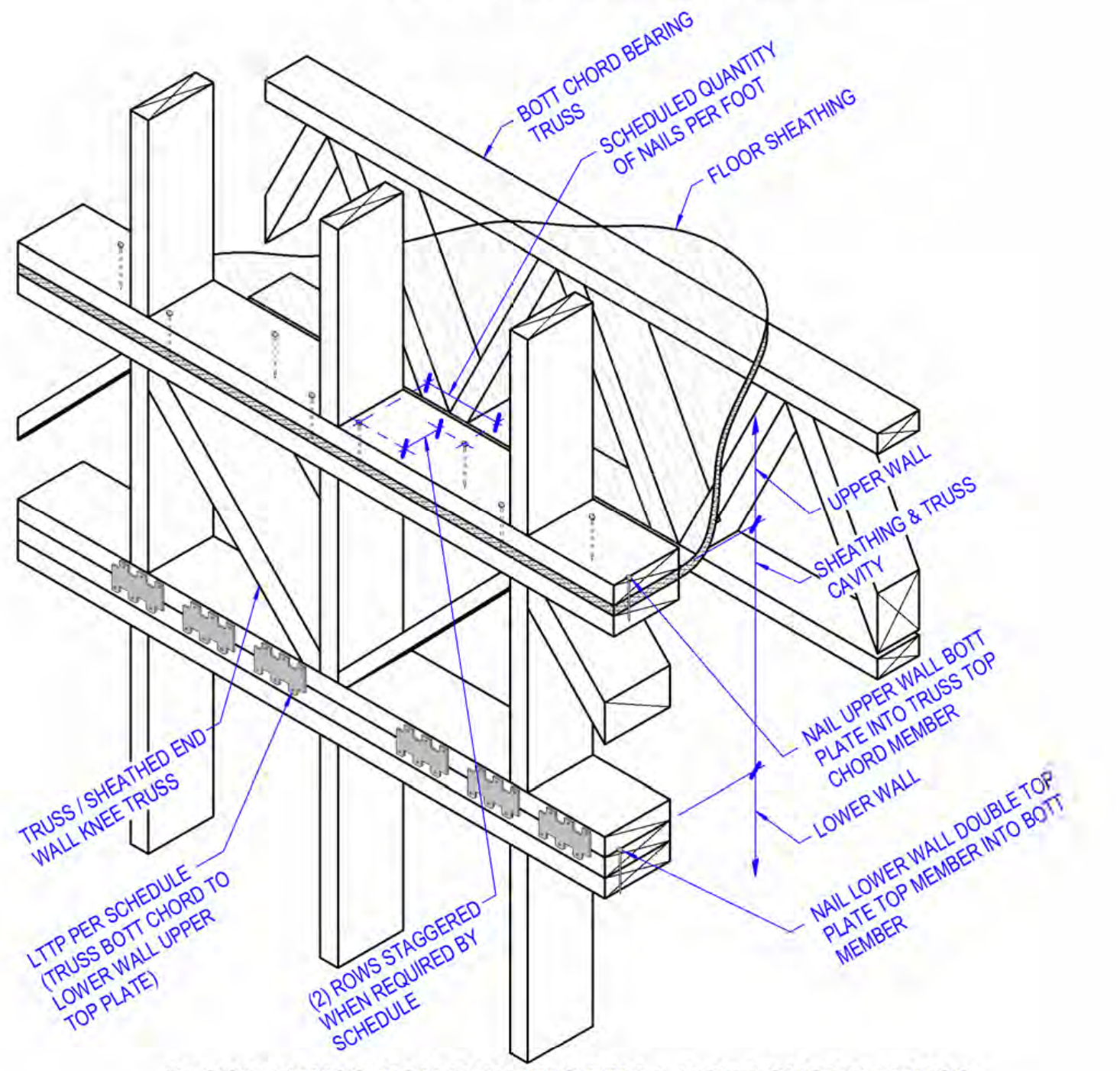
2D NON-FLOOR TRUSS BEARING WALL SCREW SHEAR FASTENER ASSEMBLY


FLOOR SHEATHING
SCHEDULED QUANTITY OF NAILS PER FOOT
UPPER WALL
LOWER WALL
NAIL LOWER WALL DOUBLE TOP PLATE TOP MEMBER INTO DOUBLE PLATE BOTT MEMBER
(2) ROWS STAGGERED WHEN REQUIRED BY SCHEDULE
NAIL UPPER WALL BOTT PLATE INTO LOWER WALL DOUBLE TOP PLATE TOP MEMBER

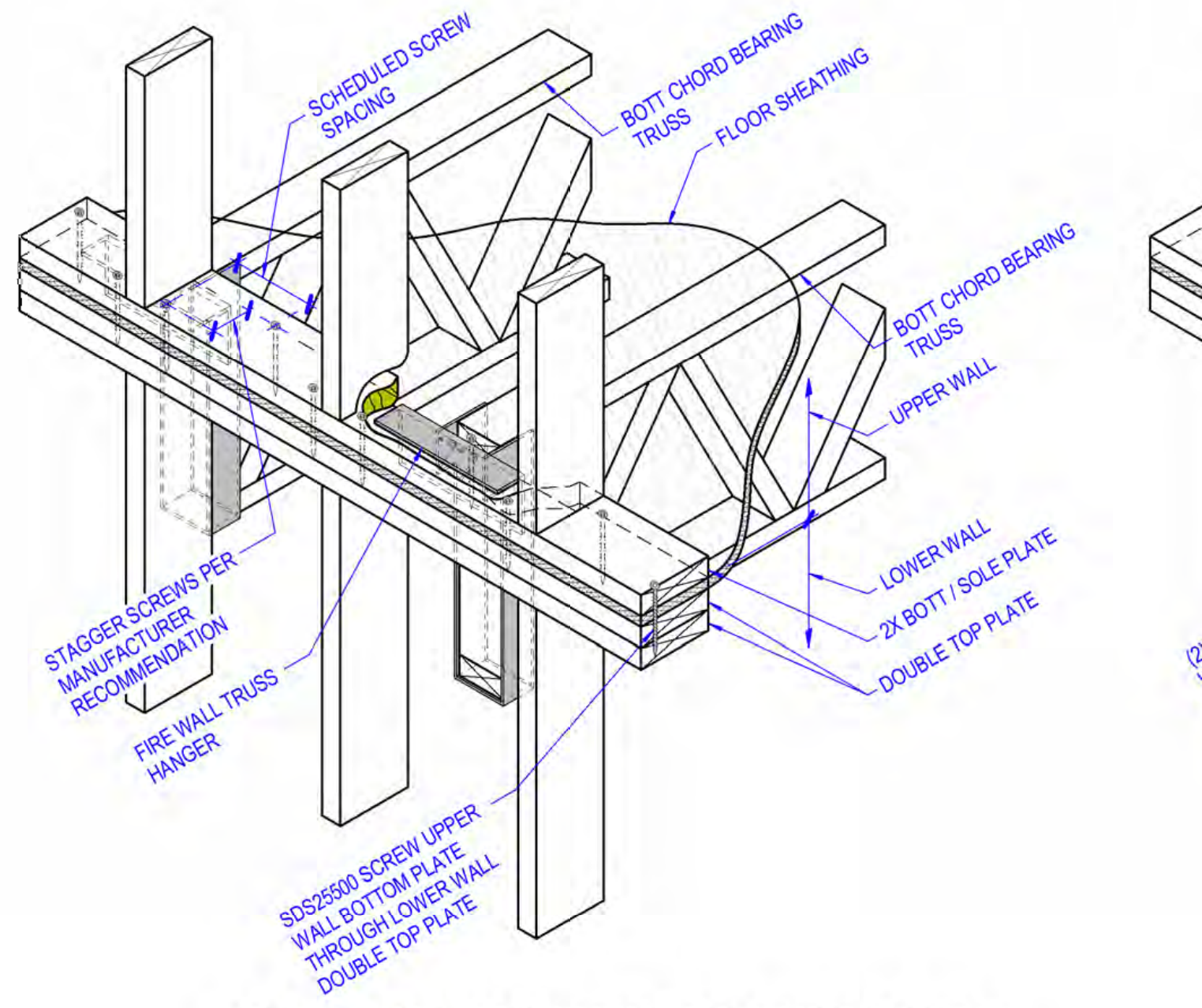
1D NON-FLOOR TRUSS BEARING WALL NAIL SHEAR FASTENER ASSEMBLY


BOTT CHORD BEARING TRUSS
SCHEDULED SCREW SPACING
FLOOR SHEATHING
UPPER WALL
SHEATHING & TRUSS CAVITY
SDS25400 SCREW UPPER WALL BOTTOM PLATE TO TRUSS / TRUSS BLOCK TOP CHORD
LOWER WALL
SDS25500 SCREW THROUGH LOWER WALL DOUBLE TOP PLATE MEMBERS INTO TRUSS / TRUSS BLOCK BOTT CHORD
TRUSS / SHEATHED END WALL KNEE TRUSS
STAGGER SCREWS PER MANUFACTURER RECOMMENDATION

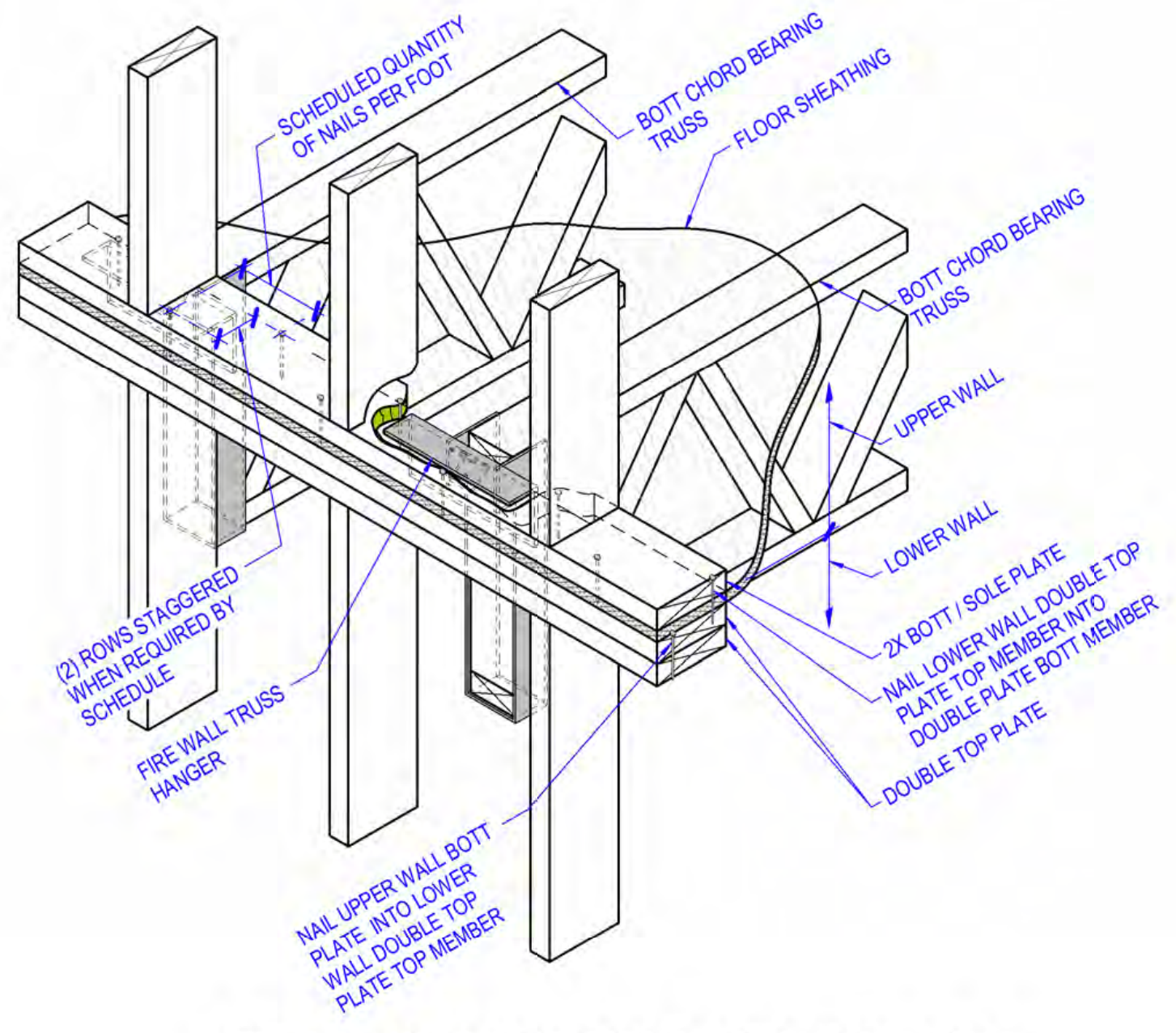
2C FLOOR TRUSS NON-BEARING WALL BOTT CHORD TRUSS NAIL SHEAR FASTENER ASSEMBLY


BOTT CHORD BEARING TRUSS
SCHEDULED QUANTITY OF NAILS PER FOOT
FLOOR SHEATHING
UPPER WALL
SHEATHING & TRUSS CAVITY
NAIL UPPER WALL BOTT PLATE INTO TRUSS TOP CHORD MEMBER
LOWER WALL
NAIL LOWER WALL DOUBLE TOP PLATE TOP MEMBER INTO BOTT MEMBER
TRUSS / SHEATHED END WALL KNEE TRUSS
LTTP PER SCHEDULE (TRUSS BOTT CHORD TO LOWER WALL UPPER TOP PLATE)
(2) ROWS STAGGERED WHEN REQUIRED BY SCHEDULE

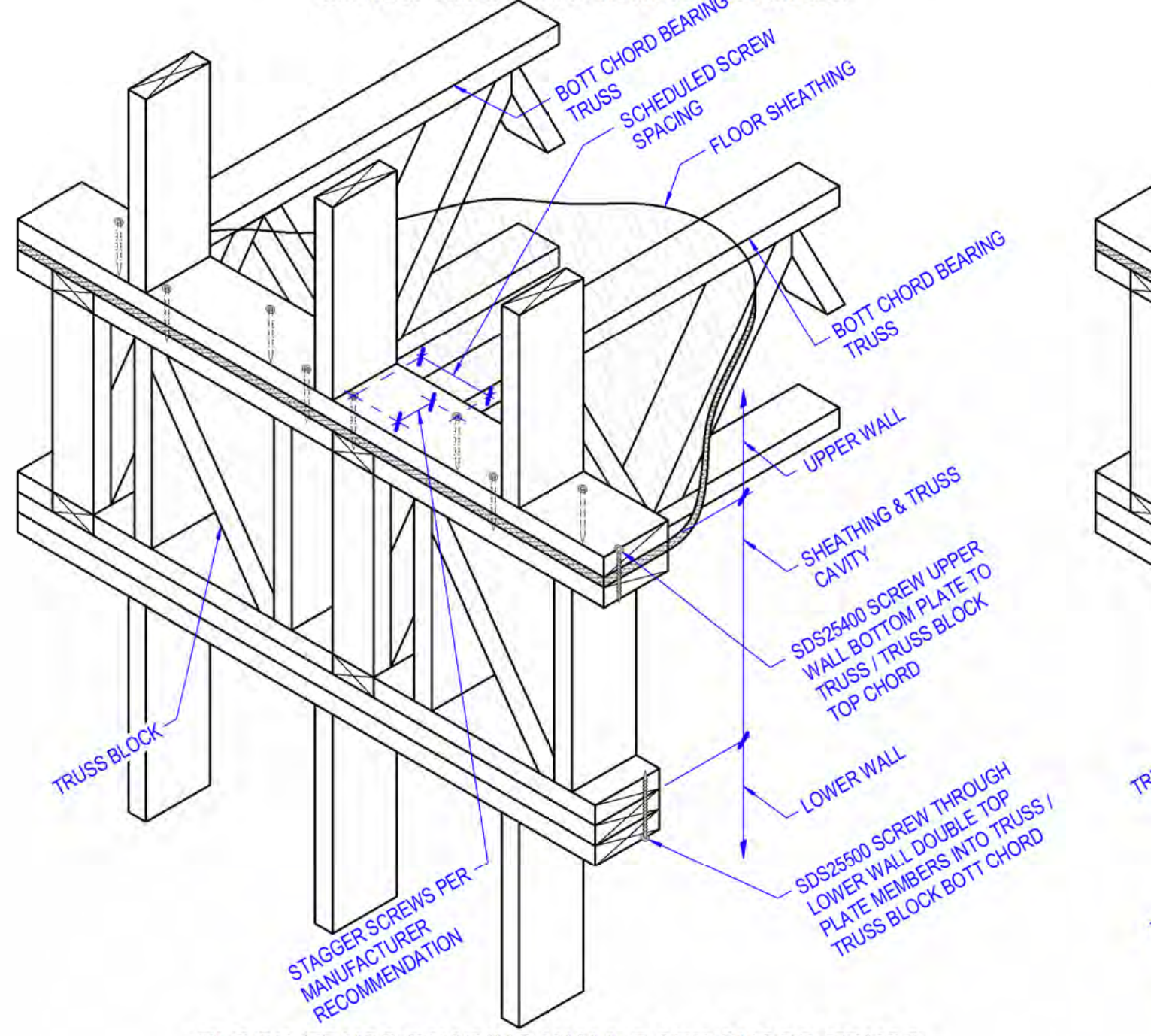
1C FLOOR TRUSS NON-BEARING WALL BOTT CHORD TRUSS NAIL SHEAR FASTENER ASSEMBLY



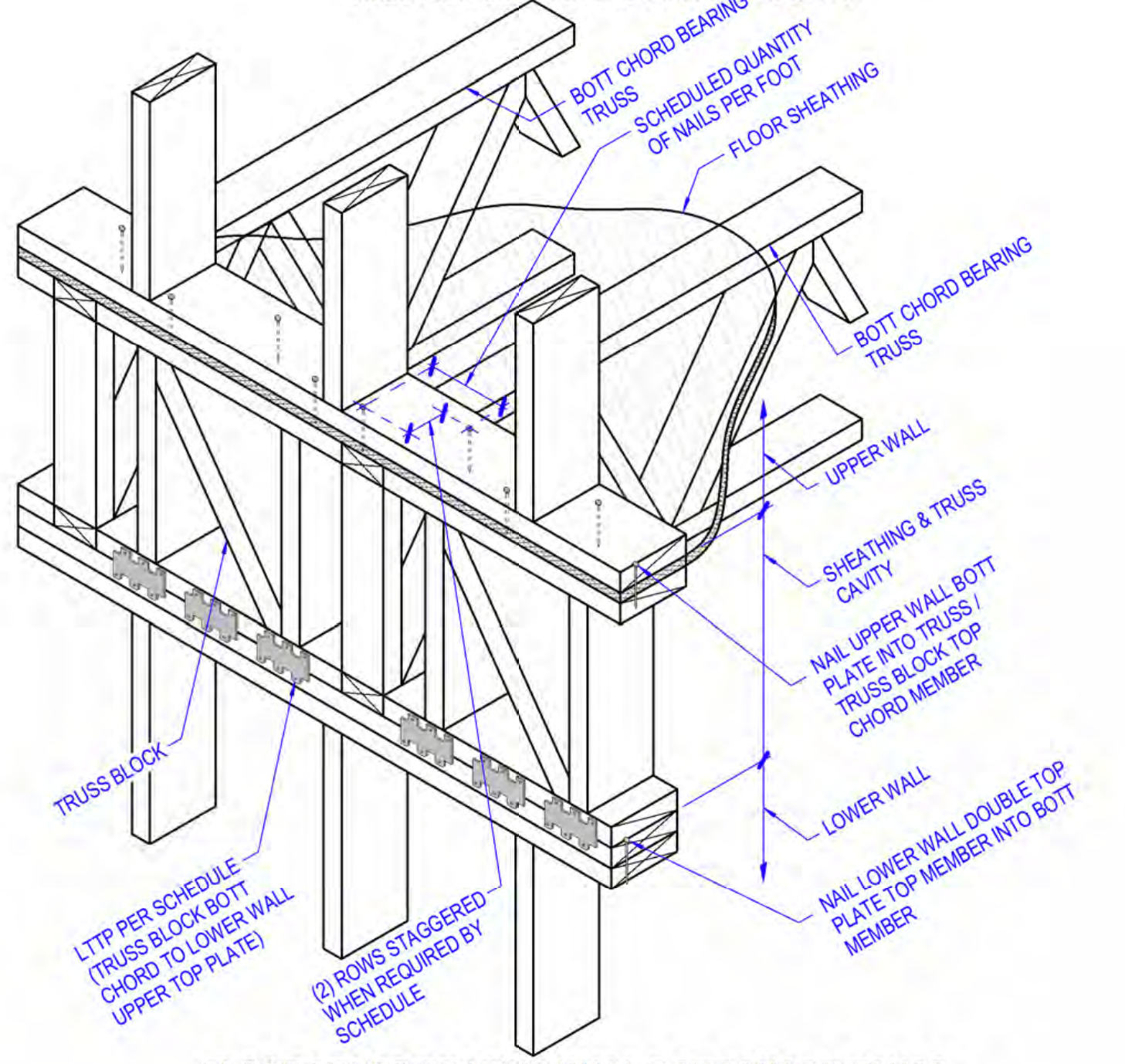
2B FLOOR TRUSS BEARING WALL BOTT CHORD TRUSS (FIRE WALL HANGER) SCREW SHEAR FASTENER ASSEMBLY


SCHEDULED QUANTITY OF NAILS PER FOOT
BOTT CHORD BEARING TRUSS
FLOOR SHEATHING
BOTT CHORD BEARING TRUSS
UPPER WALL
LOWER WALL
2X BOTT / SOLE PLATE
NAIL LOWER WALL DOUBLE TOP PLATE TOP MEMBER INTO DOUBLE PLATE BOTT MEMBER
DOUBLE TOP PLATE
(2) ROWS STAGGERED WHEN REQUIRED BY SCHEDULE
FIRE WALL TRUSS HANGER
NAIL UPPER WALL BOTT PLATE INTO LOWER WALL DOUBLE TOP PLATE TOP MEMBER

1B FLOOR TRUSS BEARING WALL BOTT CHORD TRUSS (FIRE WALL HANGER) NAIL SHEAR FASTENER ASSEMBLY


BOTT CHORD BEARING TRUSS
SCHEDULED SCREW SPACING
FLOOR SHEATHING
BOTT CHORD BEARING TRUSS
UPPER WALL
SHEATHING & TRUSS CAVITY
SDS25400 SCREW UPPER WALL BOTTOM PLATE TO TRUSS / TRUSS BLOCK TOP CHORD
LOWER WALL
SDS25500 SCREW THROUGH LOWER WALL DOUBLE TOP PLATE MEMBERS INTO TRUSS / TRUSS BLOCK BOTT CHORD
TRUSS BLOCK
STAGGER SCREWS PER MANUFACTURER RECOMMENDATION

2A FLOOR TRUSS BEARING WALL BOTT CHORD TRUSS SCREW SHEAR FASTENER ASSEMBLY


BOTT CHORD BEARING TRUSS
SCHEDULED QUANTITY OF NAILS PER FOOT
FLOOR SHEATHING
BOTT CHORD BEARING TRUSS
UPPER WALL
SHEATHING & TRUSS CAVITY
NAIL UPPER WALL BOTT PLATE INTO TRUSS / TRUSS BLOCK TOP CHORD MEMBER
LOWER WALL
NAIL LOWER WALL DOUBLE TOP PLATE TOP MEMBER INTO BOTT MEMBER
TRUSS BLOCK
LTTP PER SCHEDULE (TRUSS BLOCK BOTT CHORD TO LOWER WALL UPPER TOP PLATE)
(2) ROWS STAGGERED WHEN REQUIRED BY SCHEDULE

1A FLOOR TRUSS BEARING WALL BOTT CHORD TRUSS NAIL SHEAR FASTENER ASSEMBLY

WD10.0 WOOD SHEAR WALL SCHEDULE (WSP) FOR WIND AND SEISMIC DESIGN CATEGORY A-C:

WOOD FRAME SHEAR WALL SCHEDULE - WOOD STRUCTURAL PANEL (WSP) [1,8,16,17,18,19]

SHEAR WALL TYPE [2,3]	SHEATHED FACES	BOUNDARY / EDGE NAILING [4,5,9,10]	BLOCKED EDGES [6,8]	SILL PLATE [7,22] ANCHORAGE TO CONCRETE [11,12,23]			BOTTOM PLATE TO TOP PLATE CONNECTION [23]			ALLOWABLE SHEAR CAPACITY (PLF) SEISMIC [13]	ALLOWABLE SHEAR CAPACITY (PLF) WIND [13]
				CAST-IN ANCHOR BOLTS (ASTM F1554 GR. 36)	POST-INSTALLED MECH. ANCHORS [14,15] (ICC ESR-2713)	SIMPSON MASA MUD SILL ANCHORS (ICC ESR-2555)	NAILS NAILS / FT (0.148"Ø X 3 1/4")	SCREWS SIMPSON SDS25 SCREWS STAGGERED	SIDE CONNECTIONS SIMPSON LTP4		
A	1	2" O.C.	(2) 2X	5/8"Ø @ 18" O.C.	5/8"Ø @ 18" O.C.	12" O.C.	5 NAILS/FT - 2 ROWS	6" O.C.	10" O.C.	588	823
B	1	3" O.C.	2X	5/8"Ø @ 24" O.C.	5/8"Ø @ 24" O.C.	16" O.C.	4 NAILS/FT - 1 ROW	8" O.C.	13" O.C.	450	630
C	1	4" O.C.	2X	5/8"Ø @ 33" O.C.	5/8"Ø @ 33" O.C.	21" O.C.	3 NAILS/FT - 1 ROW	10" O.C.	17" O.C.	349	489
CC	2	4" O.C.	2X	5/8"Ø @ 16" O.C.	5/8"Ø @ 16" O.C.	10" O.C.	6 NAILS/FT - 1 ROW	5" O.C.	8" O.C.	698	978
D	1	6" O.C.	2X	5/8"Ø @ 48" O.C.	5/8"Ø @ 48" O.C.	31" O.C.	2 NAILS/FT - 1 ROW	16" O.C.	25" O.C.	239	335
S	1	2" O.C.	(2) 2X	5/8"Ø @ 18" O.C.	5/8"Ø @ 18" O.C.	11" O.C.	6 NAILS/FT - 1 ROW	5" O.C.	8" O.C.	708	901

SHEAR WALL SCHEDULE NOTES:

1. WSP SHALL BE 7/16" THICK 3-PLY MIN. PLYWOOD OR OSB SHALL HAVE A MAXIMUM MOISTURE CONTENT OF 19% AT TIME OF FABRICATION. SEE GENERAL NOTES FOR WSP SPECIFICATIONS.
2. DOUBLE SIDED SHEAR WALLS LABELED AS CC SHALL BE INSTALLED WITH WSP BOTH SIDES OF WALL SHEATHED AND FASTENED PER SCHEDULE.
3. S WALLS SHALL BE SHEATHED WITH 7/16" STRUCTURAL 1 WSP OR 15/32" WSP W/ 0.148" DIA x 3" LONG NAILS SPACED PER SCHEDULE.
4. FIELD NAILING (NAILS @ ALL OTHER SUPPORTS) SHALL BE 12" O.C. TYPICAL WITH THE SAME NAIL SIZE AS SPECIFIED AT BOUNDARY / EDGE NAILING CONDITIONS.
5. ALL BOUNDARY / EDGE AND FIELD NAILING SHALL BE WITH 0.131"Ø x 2 1/2" LONG NAILS SPACED PER SCHEDULE.
6. ALL VERTICAL AND HORIZONTAL JOINTS SHALL BE BLOCKED AS SCHEDULED WITH BLOCKS TO MATCH WALL THICKNESS (TYPICAL).
7. ALL SHEAR WALL SILL PLATES SHALL BE 2X MIN.
8. PROVIDE (2)2X OR 3X STUDS AT ABUTTING WALL PANELS PER SCHEDULE. DOUBLE 2X STUDS SHALL BE FACE NAILED TOGETHER WITH (2) STAGGERED ROWS OF 0.148"Ø x 3" LONG NAILS SPACED @ 4" O.C. (TYPICAL).
9. EDGE DISTANCE FROM PANEL EDGES SHALL BE 1/2" MIN. AND EDGE DISTANCE FROM CONNECTING MEMBERS SHALL BE 3/8" MIN.
10. STAGGER NAILING AT ALL ADJOINING PANEL EDGES WHERE NAIL SPACING IS LESS THAN 6" O.C.
11. POST-INSTALLED MECHANICAL ANCHORS SHALL BE SIMPSON TITAN-HD (ICC ESR-2713) x 5" MIN. EMBEDMENT INTO CONCRETE. CAST-IN-PLACE ANCHORS SHALL BE EMBEDDED 7" MIN. INTO CONCRETE.
12. SIMPSON MASA ANCHORS SHALL BE INSTALLED WITH STANDARD INSTALLATION AND 20% MAX. ONE-LEG UP INSTALLATION ALONG A WALL LINE (1 OF EVERY 5 MAX.) WITH ALL NAIL HOLES FILLED (TYPICAL).
13. ALLOWABLE SHEAR CAPACITY VALUES ARE BASED ON SPF STUD FRAMING. FOR DFL STUD FRAMING, THE ALLOWABLE SHEAR VALUES SHALL BE DIVIDED BY 0.92
14. CONTRACTOR SHALL APPROPRIATELY SURVEY STRUCTURAL SLABS PRIOR TO POST-INSTALLED ANCHOR INSTALLATION TO ENSURE EXISTING POST-TENSIONING TENDONS ARE NOT DAMAGED DURING ANCHOR INSTALLATION (TYPICAL). IN NO INSTANCE SHALL EXISTING POST-TENSIONING TENDONS BE DAMAGED.
15. CONTRACTOR SHALL FOLLOW ALL POST-INSTALLED ANCHOR MANUFACTURER INSTALLATION INSTRUCTIONS, RECOMMENDATIONS AND PROTOCOLS IN ADDITION TO BUILDING CODE SPECIAL INSPECTION REQUIREMENTS.
16. REFER TO SHEAR WALL END ANCHOR ELEVATIONS AND SCHEDULE FOR REQUIRED SHEAR WALL END POST / STUDS. IF NO ANCHOR IS SPECIFIED, PROVIDE (2)2X FULL HEIGHT STUDS EACH SIDE OF HOLDOWN ROD AND AT EACH END OF SHEAR WALL. FACE NAIL (2)2X STUDS TOGETHER WITH (2) ROWS OF 0.148"Ø x 3" NAILS @ 4" O.C. STAGGERED ON ALTERNATING SIDES (TYPICAL).
17. WHERE SHEAR WALLS ARE SUPPORTED BY STEEL BEAMS, PROVIDE 2X OR 3X CONTINUOUS NAILER RIPPED TO MATCH STEEL BEAM FLANGE WIDTH AND ANCHOR WITH 5/8"Ø THREADED STUDS SHOP WELDED TO BEAM TOP FLANGE @ SCHEDULED ANCHOR BOLT SPACING.
18. SHEAR FASTENER INSTALLATION SHALL MEET ONE OF THE PROVIDED ASSEMBLY DETAILS PROVIDED, SEE ACCOMPANYING DETAILS.
19. SHEAR WALL FRAMING SHALL BE 2X STUDS @ 16" O.C. MAX. REFER TO WOOD SHEAR WALL PLAN(S) FOR SHEAR WALL TYPES AND LOCATIONS.
20. NO SUBSTITUTIONS FOR SCHEDULED SHEAR WALL FRAMING OR HARDWARE WITHOUT FORMAL RFI REQUEST AND APPROVAL BY SEOR PRIOR TO CONSTRUCTION.
21. AT WALLS WITH PANELS APPLIED TO ONE FACE ONLY, PANELS MAY BE PLACED ON EITHER FACE YET MUST ATTACH DIRECTLY TO STUDS (TYPICAL).
22. SILL PLATE ANCHORAGE SHALL BE CAST-IN ANCHORS. POST-INSTALLED OR MASA ANCHORS AT CONTRACTOR'S OPTION.
23. FASTENER SPACING SPECIFIED SHALL BE BASED ON AN AVERAGE SPACING PROVIDED ALONG THE SHEARWALL LENGTH. WHERE STUDS AND OTHER OBSTRUCTIONS PREVENT THE SPECIFIED UNIFORM SPACING TO BE INSTALLED, A TIGHTER SPACING MAY BE USED PROVIDED THAT THE MINIMUM SPACING IS MAINTAINED PER TABLE BELOW.

FASTNER TYPE	MIN. SPACING (IN ANY SINGLE ROW)	MIN. SPACING BETWEEN ROWS	MIN. WOOD EDGE / END DISTANCES
• CAST-IN ANCHOR BOLTS	3"	N/A	1 1/2" EDGE / 5 1/2" END
• POST-INSTALLED MECH. ANCHORS	5"	N/A	1 1/2" EDGE / 5 1/2" END
• SIMPSON MASA MUD SILL ANCHORS	4"	N/A	2" END
• NAILS	2 1/4"	3/8"	3/8" EDGE / 2 1/2" END
• SIMPSON SDS25 SCREWS	3"	5/8"	5/8" EDGE / 3" END
• SIDE CONNECTORS (LTP4)	6"	N/A	N/A

WD10.1 GYPSUM SHEAR WALL SCHEDULE (GWB) FOR WIND AND SEISMIC DESIGN CATEGORY A-C:

WOOD FRAME SHEAR WALL SCHEDULE - GYPSUM WALLBOARD (GWB) [1,3,8,10,16,17,18,19]

SHEAR WALL TYPE [2,3,19]	SHEATHED FACES	BOUNDARY / EDGE & FIELD NAILING [5,9]	BLOCKED EDGES [6]	SILL PLATE [7,23] ANCHORAGE TO CONCRETE [11,12,20]			BOTTOM PLATE TO TOP PLATE CONNECTION [20]			ALLOWABLE SHEAR CAPACITY (PLF) SEISMIC	ALLOWABLE SHEAR CAPACITY (PLF) WIND
				CAST-IN ANCHOR BOLTS (ASTM F1554 GR. 36)	POST-INSTALLED MECH. ANCHORS [14,15] (ICC ESR-2713)	SIMPSON MASA MUD SILL ANCHORS (ICC ESR-2555)	NAILS NAILS / FT (0.148"Ø X 3 1/4")	SCREWS SIMPSON SDS25 SCREWS STAGGERED	SIDE CONNECTIONS SIMPSON LTP4		
E	1	7" O.C.	NA	5/8"Ø @ 48" O.C.	5/8"Ø @ 48" O.C.	48" O.C.	2 NAILS/FT	48" O.C.	48" O.C.	80	115
EE	2	7" O.C.	NA	5/8"Ø @ 48" O.C.	5/8"Ø @ 48" O.C.	48" O.C.	2 NAILS/FT	24" O.C.	24" O.C.	160	230
G	1	4" O.C.	NA	5/8"Ø @ 48" O.C.	5/8"Ø @ 48" O.C.	48" O.C.	2 NAILS/FT	45" O.C.	42" O.C.	100	145
GG	2	4" O.C.	NA	5/8"Ø @ 48" O.C.	5/8"Ø @ 42" O.C.	39" O.C.	2 NAILS/FT	22" O.C.	21" O.C.	200	290
H	1	4" O.C.	2X	5/8"Ø @ 48" O.C.	5/8"Ø @ 48" O.C.	48" O.C.	2 NAILS/FT	36" O.C.	36" O.C.	125	175
HH	2	4" O.C.	2X	5/8"Ø @ 48" O.C.	5/8"Ø @ 40" O.C.	32" O.C.	2 NAILS/FT	18" O.C.	18" O.C.	250	350
N	1	9" O.C. / 7"O.C.	2X	5/8"Ø @ 48" O.C.	5/8"Ø @ 48" O.C.	45" O.C.	2 NAILS/FT	36" O.C.	24" O.C.	175	250
NN	2	9" O.C. / 7"O.C.	2X	5/8"Ø @ 32" O.C.	5/8"Ø @ 24" O.C.	22" O.C.	3 NAILS/FT	18" O.C.	24" O.C.	350	500

SHEAR WALL SCHEDULE NOTES:

1. GWB SHALL BE 5/8" THICK. GWB SHALL CONFORM TO ASTM C 1396 AND SHALL BE INSTALLED IN ACCORDANCE WITH ASTM C 840.
2. DOUBLE SIDED SHEAR WALLS LABELED AS EE, GG, HH OR NN SHALL BE INSTALLED WITH GWB BOTH SIDES OF WALL SHEATHED AND FASTENED PER SCHEDULE. GWB ON BOTH SIDES OF WALL SHALL ATTACH DIRECTLY TO STUDS.
3. AT WALLS WITH PANELS APPLIED TO ONE FACE ONLY, PANELS MAY BE PLACED ON EITHER FACE YET MUST ATTACH DIRECTLY TO STUDS (TYPICAL).
4. END JOINTS OF ADJACENT COURSES OF GWB SHALL NOT OCCUR OVER THE SAME STUD.
5. OUTER PLY OF DOUBLE LAYER GYP SHEAR WALL TYPES 'N' AND 'NN' BOUNDARY / EDGE AND FIELD NAILING SHALL BE 8d (0.113"Ø x 2 3/8" LONG x 0.281" HEAD), WALLBOARD NAILS (0.113"Ø x 2 3/8" LONG x 3/8" HEAD), OR 0.120"Ø NAIL x 2 3/8" LONG WITH 3/8" HEAD. WHERE TWO NAILING SPACINGS ARE INDICATED, FIRST NUMBER INDICATES BASE PLY NAILING SPACING, SECOND NUMBER INDICATES OUTER PLY NAILING SPACING. ALL OTHER GYP SHEAR WALL BOUNDARY / EDGE AND FIELD NAILING SHALL BE 6d COOLER (0.092"Ø x 1 7/8" LONG, 1/4" HEAD), WALLBOARD NAILS (0.0915"Ø x 1 7/8" LONG, 19/64" HEAD), OR 0.120"Ø x 1 3/4" LONG NAILS WITH 3/8" MIN. HEAD.
6. ALL VERTICAL AND HORIZONTAL JOINTS SHALL BE BLOCKED AS SCHEDULED WITH BLOCKS TO MATCH WALL THICKNESS (TYPICAL).
7. ALL SHEAR WALL SILL PLATES SHALL BE 2X MIN.
8. NO SUBSTITUTIONS FOR SCHEDULED SHEAR WALL FRAMING OR HARDWARE WITHOUT FORMAL RFI REQUEST AND APPROVAL BY SEOR PRIOR TO CONSTRUCTION.
9. EDGE DISTANCE FROM PANEL EDGES AND EDGE DISTANCE FROM CONNECTING MEMBERS SHALL BE 3/8" MIN.
10. SHEAR WALL FRAMING SHALL BE 2X STUDS @ 16" O.C. MAX. REFER TO WOOD SHEAR WALL PLAN(S) FOR SHEAR WALL TYPES AND LOCATIONS.
11. POST-INSTALLED MECHANICAL ANCHORS SHALL BE SIMPSON TITAN-HD (ICC ESR-2713) x 5" MIN. EMBEDMENT INTO CONCRETE. CAST-IN-PLACE ANCHORS SHALL BE EMBEDDED 7" MIN. INTO CONCRETE.
12. SIMPSON MASA ANCHORS SHALL BE INSTALLED WITH STANDARD INSTALLATION AND 30% MAX. ONE-LEG UP INSTALLATION ALONG A WALL LINE (1 OF EVERY 3 MAX.) WITH ALL NAIL HOLES FILLED (TYPICAL).
13. SILL PLATE ANCHORAGE SHALL BE CAST-IN ANCHORS. POST-INSTALLED OR MASA ANCHORS AT CONTRACTOR'S OPTION.
14. CONTRACTOR SHALL APPROPRIATELY SURVEY STRUCTURAL SLABS PRIOR TO POST-INSTALLED ANCHOR INSTALLATION TO ENSURE EXISTING POST-TENSIONING TENDONS ARE NOT DAMAGED DURING ANCHOR INSTALLATION (TYPICAL). IN NO INSTANCE SHALL EXISTING POST-TENSIONING TENDONS BE DAMAGED.
15. CONTRACTOR SHALL FOLLOW ALL POST-INSTALLED ANCHOR MANUFACTURER INSTALLATION INSTRUCTIONS, RECOMMENDATIONS AND PROTOCOLS IN ADDITION TO BUILDING CODE SPECIAL INSPECTION REQUIREMENTS.
16. REFER TO SHEAR WALL END ANCHOR ELEVATIONS AND SCHEDULE FOR REQUIRED SHEAR WALL END POST / STUDS. IF NO ANCHOR IS SPECIFIED, PROVIDE (2)2X FULL HEIGHT STUDS EACH SIDE OF HOLD-DOWN ROD AND AT EACH END OF SHEAR WALL. FACE NAIL (2)2X STUDS TOGETHER TO MATCH NAIL SIZE AND SPACING OF SCHEDULED BOTTOM PLATE NAILING.
17. WHERE SHEAR WALLS ARE SUPPORTED BY STEEL BEAMS, PROVIDE 2X OR 3X CONTINUOUS NAILER RIPPED TO MATCH STEEL BEAM FLANGE WIDTH AND ANCHOR WITH 5/8"Ø THREADED STUDS SHOP WELDED TO BEAM TOP FLANGE @ SCHEDULED ANCHOR BOLT SPACING.
18. SHEAR FASTENER INSTALLATION SHALL MEET ONE OF THE PROVIDED ASSEMBLY DETAILS PROVIDED, SEE ACCOMPANYING DETAILS.
19. SHEAR WALL TYPES 'N' SHALL BE SHEATHED AS (2)PLY OF 5/8" (1 1/4" TOTAL) APPLIED TO ONE SIDE OF STUD WALL. SHEAR WALL TYPE 'NN' SHALL BE SHEATHED AS (2)PLY OF 5/8" (1 1/4" TOTAL) APPLED TO BOTH SIDES OF STUD WALL.
20. FASTENER SPACING SPECIFIED SHALL BE BASED ON AN AVERAGE SPACING PROVIDED ALONG THE SHEAR WALL LENGTH. WHERE STUDS AND OTHER OBSTRUCTIONS PREVENT THE SPECIFIED UNIFORM SPACING TO BE INSTALLED, A TIGHTER SPACING MAY BE USED PROVIDED THAT THE MINIMUM SPACING IS MAINTAINED PER TABLE BELOW.

FASTENER TYPE	MIN. SPACING (IN ANY SINGLE ROW)	MIN. SPACING BETWEEN ROWS	MIN. WOOD EDGE / END DISTANCES
• CAST-IN ANCHOR BOLTS	3"	N/A	1 1/2" EDGE / 5 1/2" END
• POST-INSTALLED MECH. ANCHORS	5"	N/A	1 1/2" EDGE / 5 1/2" END
• SIMPSON MASA MUD SILL ANCHORS	4"	N/A	2" END
• NAILS	2 1/4"	3/8"	3/8" EDGE / 2 1/2" END
• SIMPSON SDS25 SCREWS	3"	5/8"	5/8" EDGE / 3" END
• SIDE CONNECTORS (LTP4)	6"	N/A	N/A

FLOOR TRUSS FASTENER ASSEMBLY DETAILS

	FLOOR TRUSS BEARING (INTERIOR)	FLOOR TRUSS BEARING (EXTERIOR)	FLOOR TRUSS NON BEARING	FLOOR TRUSS NON BEARING (ALTERNATE)
NAILING	1A / S5.410	1B / S5.410	1C / S5.410	1D / S5.410
SCREWS	2A / S5.410	2B / S5.410	2C / S5.410	2D / S5.410

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers
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Houston, TX 77077
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PROJECT # : 26154
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CHECKED BY: Checker

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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH
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SHEET TITLE:
Wood Shear Wall Schedule

SHEET NUMBER:
S5.410

NOT RELEASED FOR CONSTRUCTION

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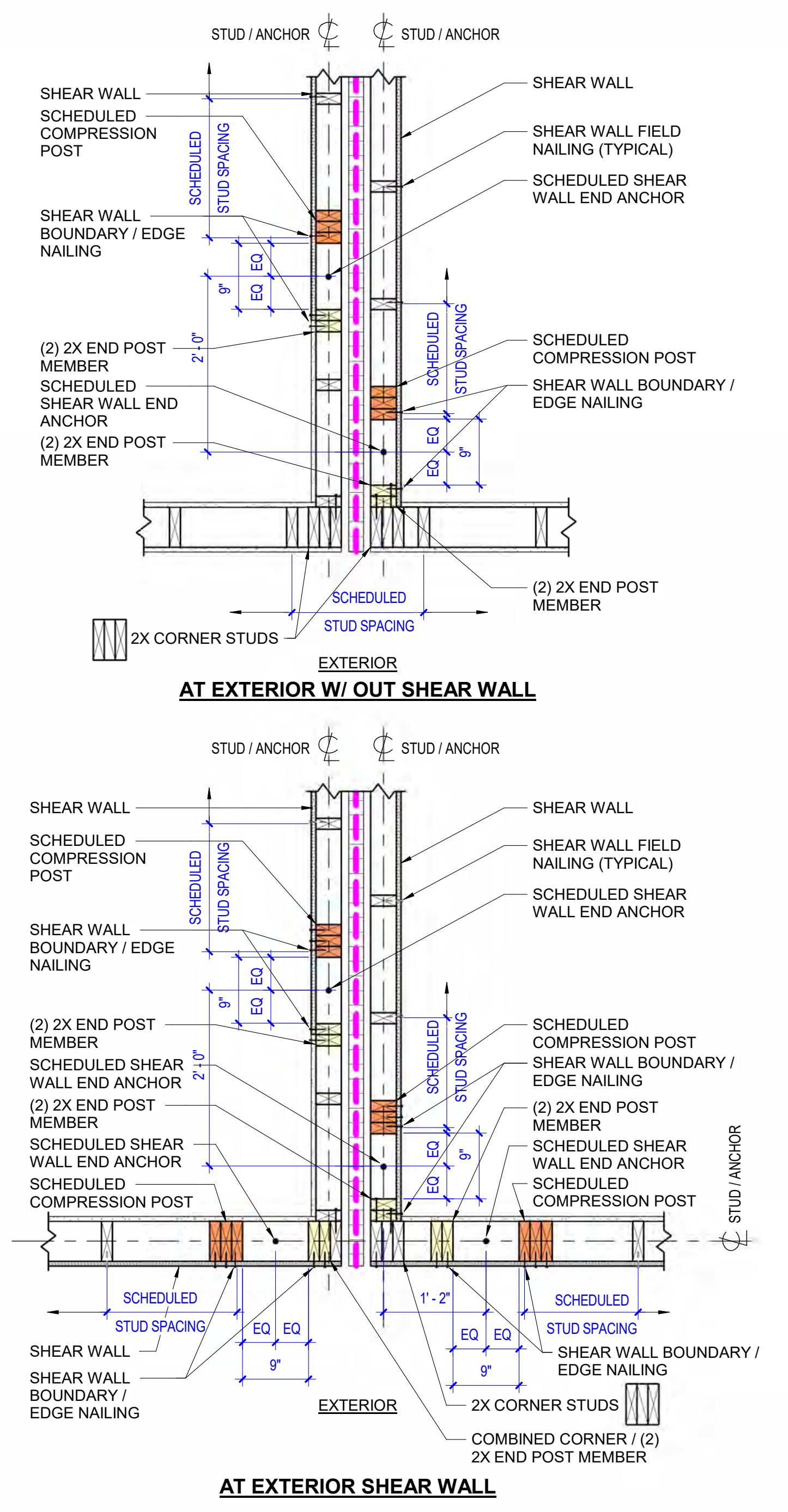
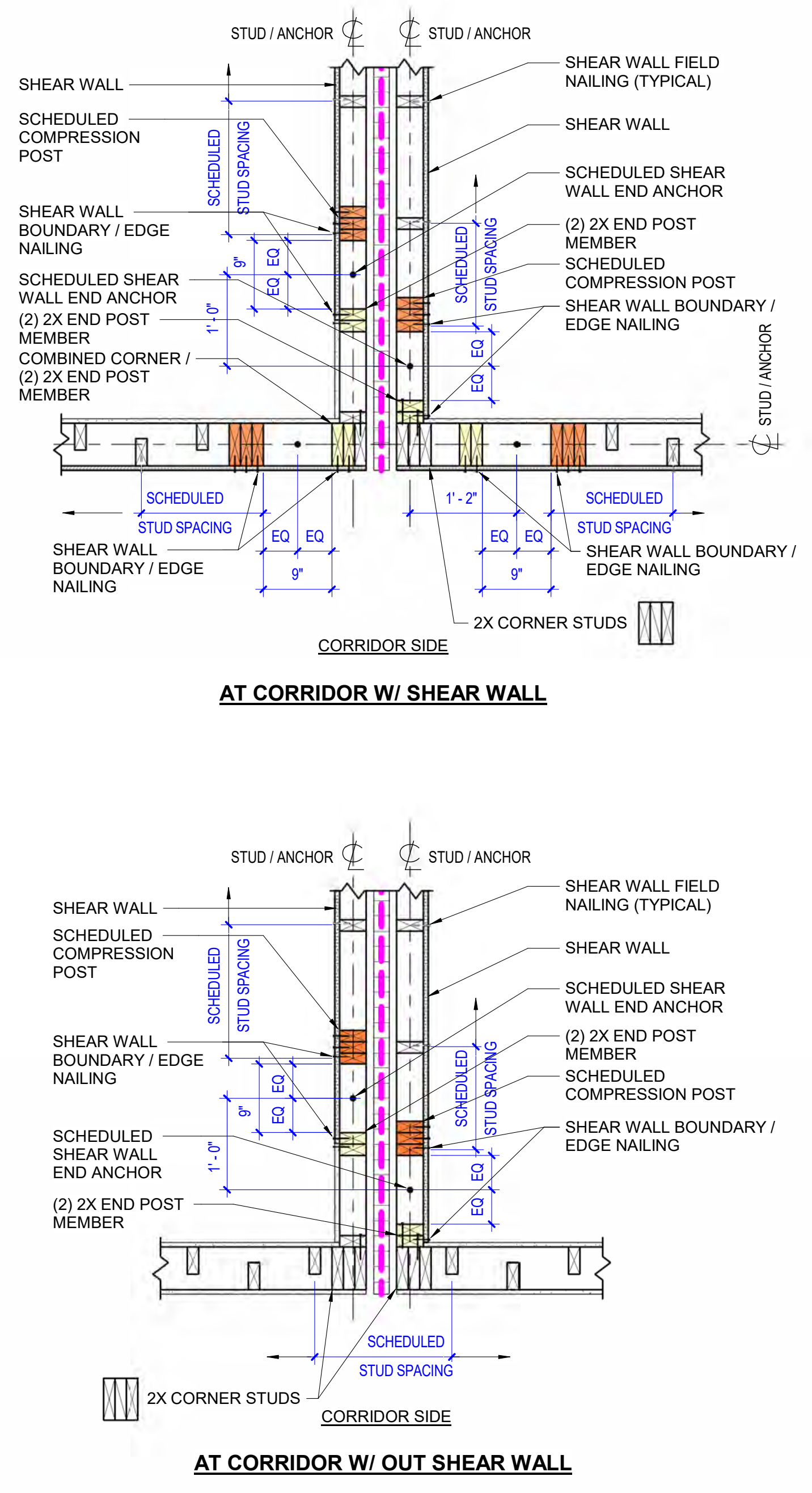
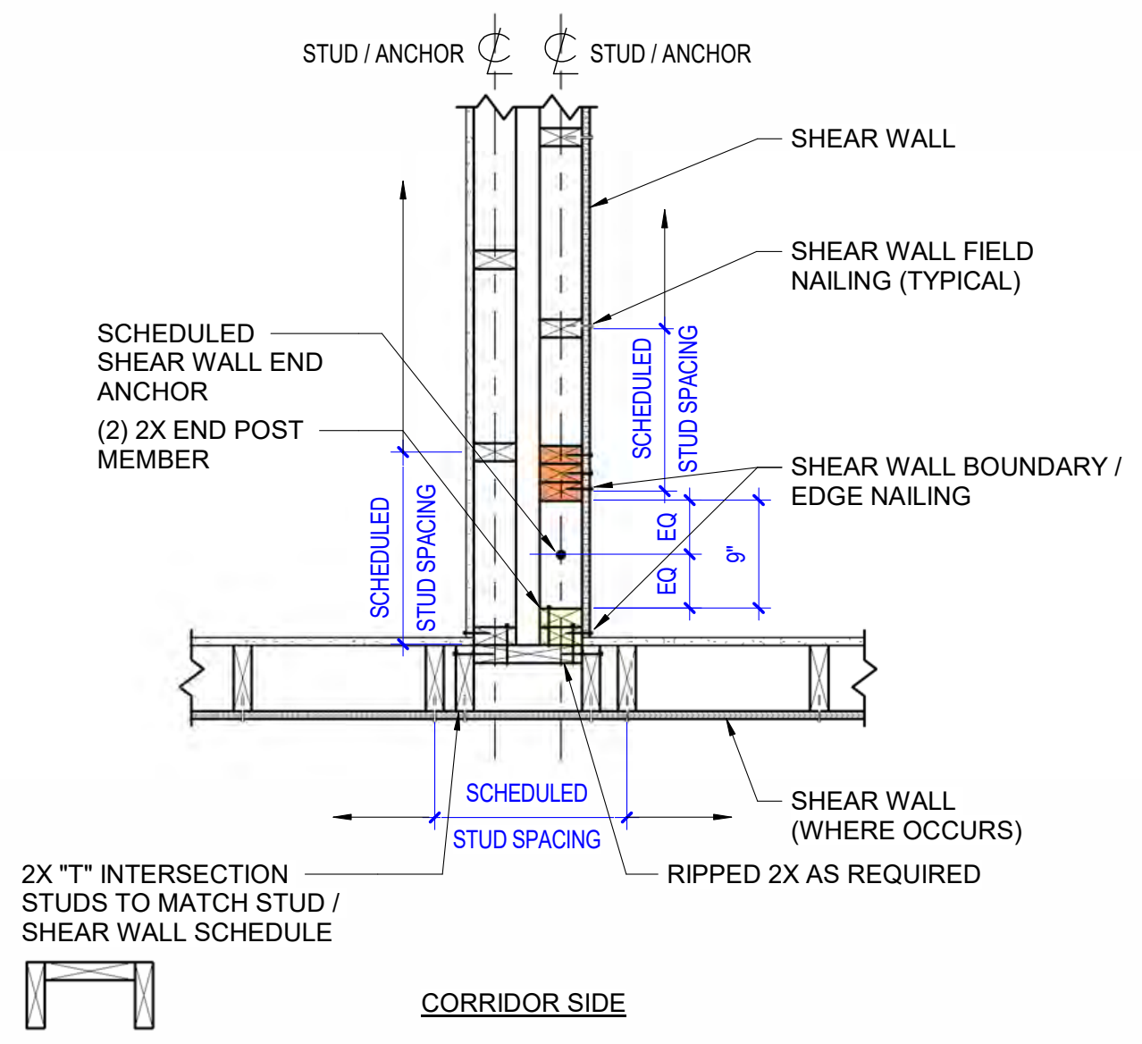

STUD / ANCHOR
STUD / ANCHOR
SHEAR WALL
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
(2) 2X END POST MEMBER
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
SHEAR WALL
SHEAR WALL FIELD NAILING (TYPICAL)
SCHEDULED SHEAR WALL END ANCHOR
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
(2) 2X END POST MEMBER
SCHEDULED STUD SPACING
EQ
EQ
9"
2' - 0"
2X CORNER STUDS
EXTERIOR
AT EXTERIOR W/ OUT SHEAR WALL
STUD / ANCHOR
STUD / ANCHOR
SHEAR WALL
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
(2) 2X END POST MEMBER
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
SCHEDULED SHEAR WALL END ANCHOR
SCHEDULED COMPRESSION POST
SHEAR WALL
SHEAR WALL FIELD NAILING (TYPICAL)
SCHEDULED SHEAR WALL END ANCHOR
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
(2) 2X END POST MEMBER
SCHEDULED SHEAR WALL END ANCHOR
SCHEDULED COMPRESSION POST
SCHEDULED STUD SPACING
EQ
EQ
9"
1' - 2"
2' - 0"
SHEAR WALL
SHEAR WALL BOUNDARY / EDGE NAILING
SHEAR WALL BOUNDARY / EDGE NAILING
EXTERIOR
2X CORNER STUDS
COMBINED CORNER / (2) 2X END POST MEMBER
AT EXTERIOR SHEAR WALL

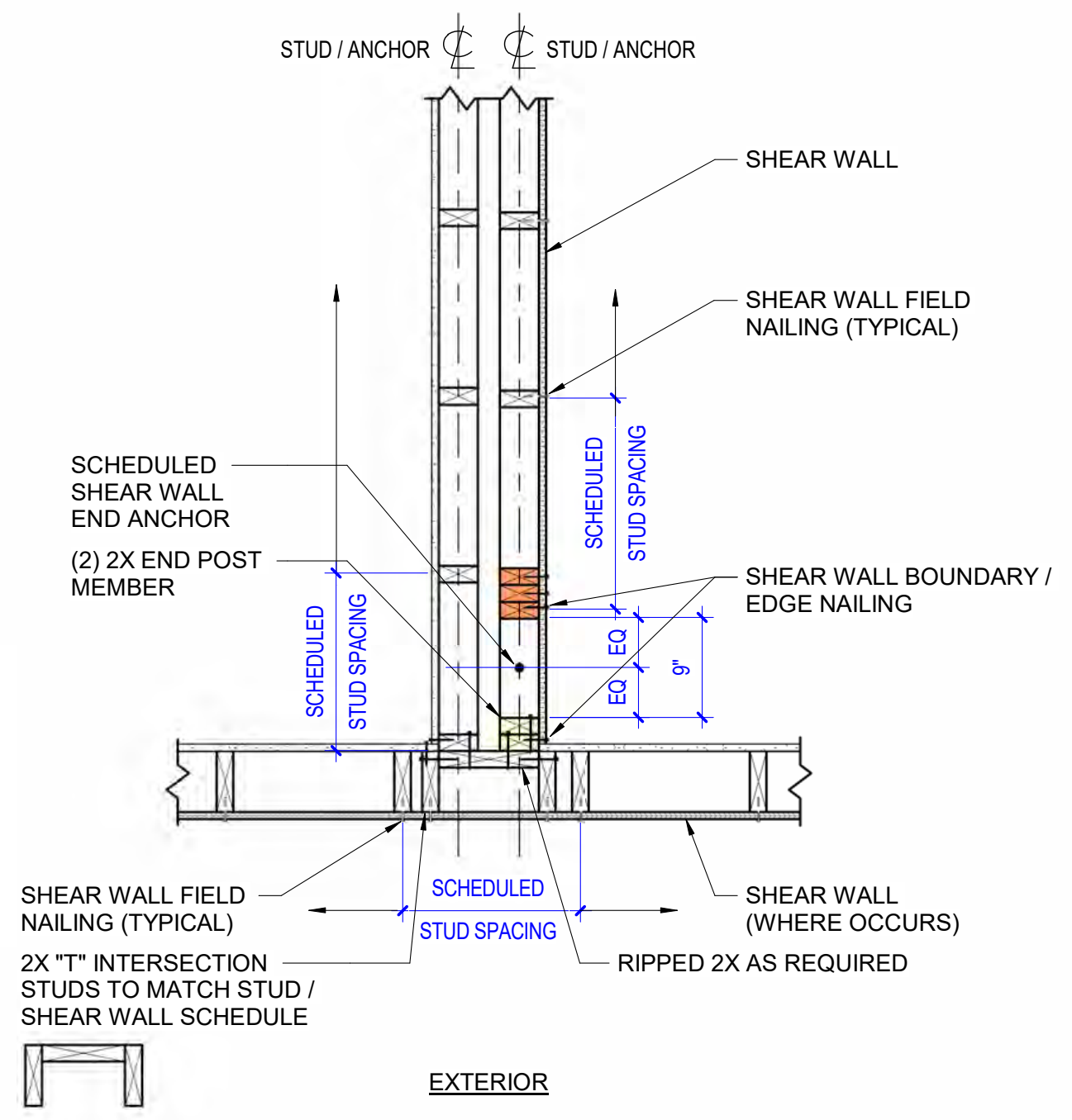
⑬ Exterior Exterior Shear Wall Anchorage - Fire Wall
3/4" = 1'-0"


STUD / ANCHOR
STUD / ANCHOR
SHEAR WALL
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
COMBINED CORNER / (2) 2X END POST MEMBER
SHEAR WALL FIELD NAILING (TYPICAL)
SHEAR WALL
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED STUD SPACING
EQ
EQ
9"
1' - 0"
1' - 2"
SHEAR WALL BOUNDARY / EDGE NAILING
SHEAR WALL BOUNDARY / EDGE NAILING
2X CORNER STUDS
CORRIDOR SIDE
AT CORRIDOR W/ SHEAR WALL
STUD / ANCHOR
STUD / ANCHOR
SHEAR WALL
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
SHEAR WALL FIELD NAILING (TYPICAL)
SHEAR WALL
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED STUD SPACING
EQ
EQ
9"
1' - 0"
2X CORNER STUDS
CORRIDOR SIDE
AT CORRIDOR W/ OUT SHEAR WALL

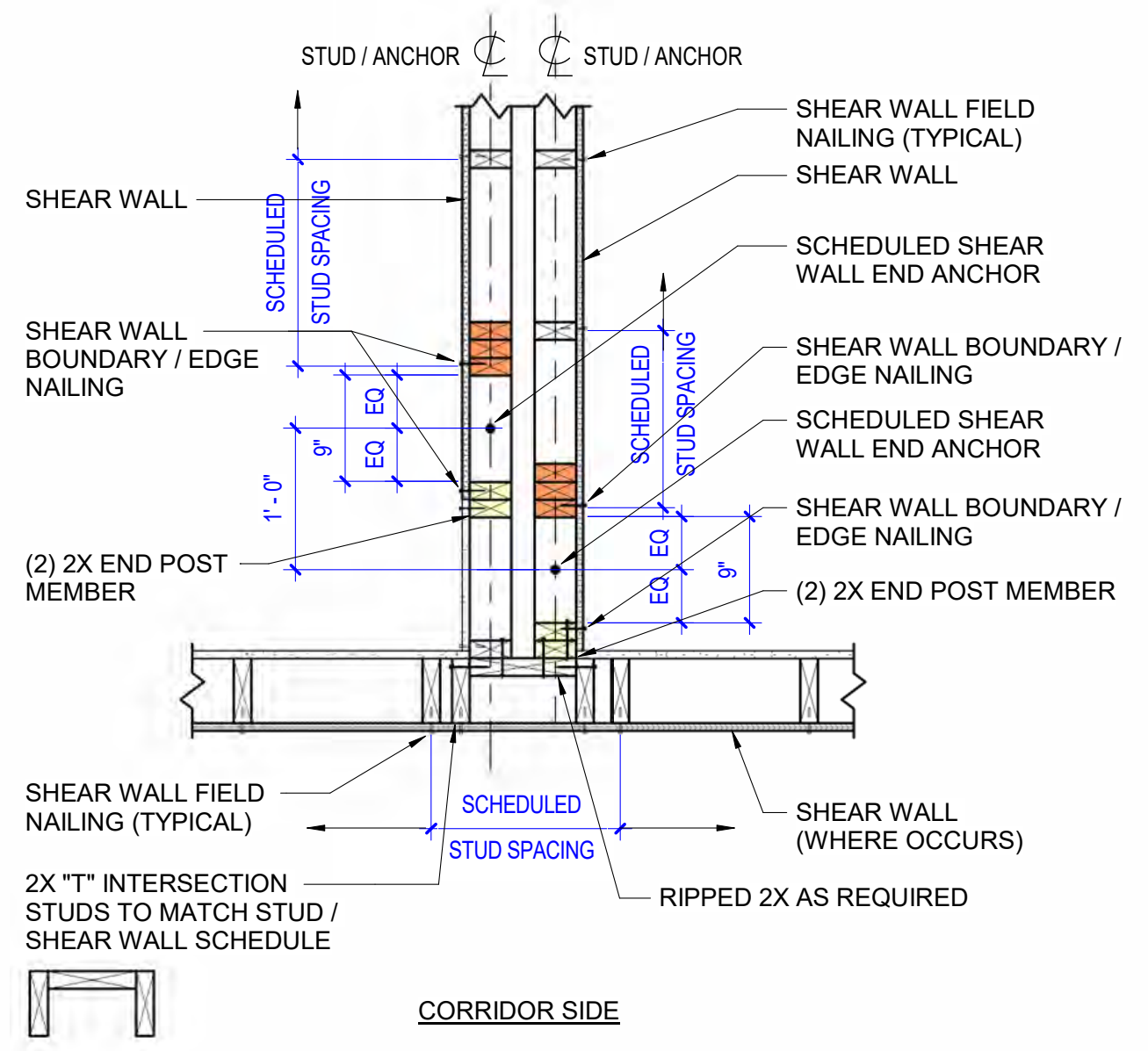
⑫ Interior Shear Wall Anchorage - Fire Wall
3/4" = 1'-0"


STUD / ANCHOR
STUD / ANCHOR
SHEAR WALL
SHEAR WALL FIELD NAILING (TYPICAL)
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED STUD SPACING
EQ
EQ
9"
SHEAR WALL (WHERE OCCURS)
RIPPED 2X AS REQUIRED
2X "T" INTERSECTION STUDS TO MATCH STUD / SHEAR WALL SCHEDULE
CORRIDOR SIDE

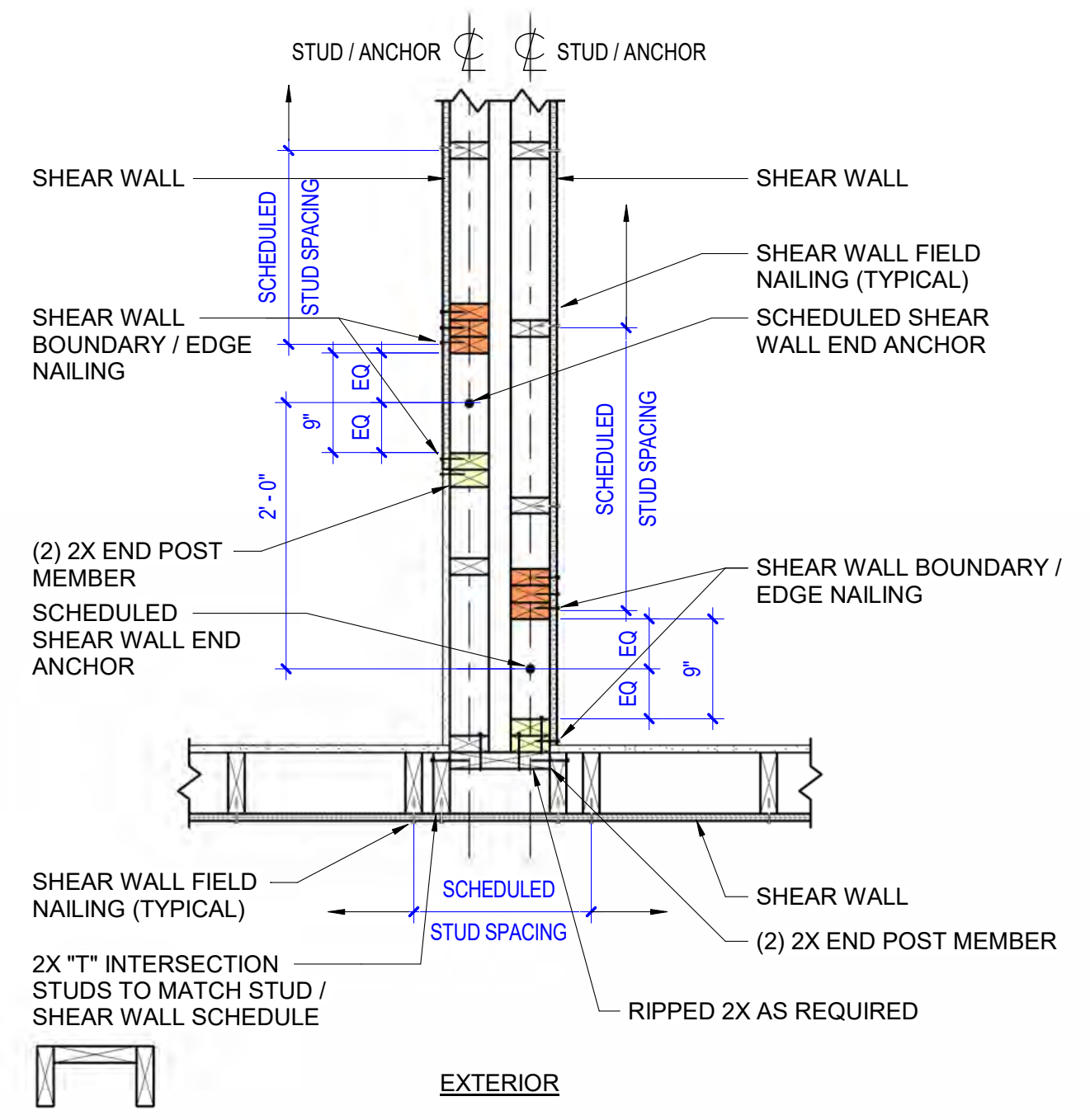
⑪ Interior Shear Wall Anchorage - Demising Wall 1 Side
3/4" = 1'-0"


STUD / ANCHOR
STUD / ANCHOR
SHEAR WALL
SHEAR WALL FIELD NAILING (TYPICAL)
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED STUD SPACING
EQ
EQ
9"
SHEAR WALL FIELD NAILING (TYPICAL)
SHEAR WALL (WHERE OCCURS)
2X "T" INTERSECTION STUDS TO MATCH STUD / SHEAR WALL SCHEDULE
RIPPED 2X AS REQUIRED
EXTERIOR

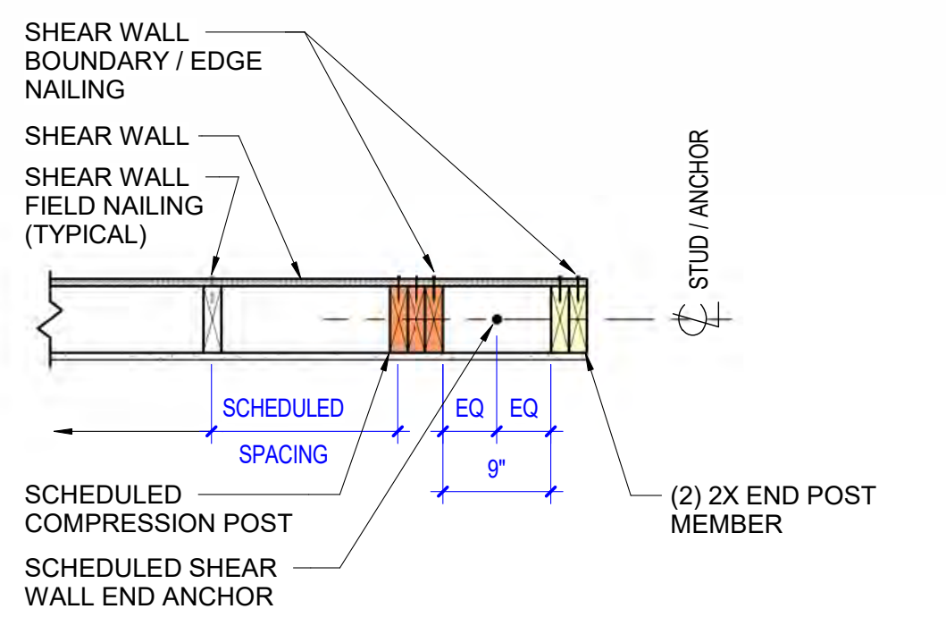
⑩ Exterior Shear Wall Anchorage - Demising Wall 1 Side
3/4" = 1'-0"


STUD / ANCHOR
STUD / ANCHOR
SHEAR WALL FIELD NAILING (TYPICAL)
SHEAR WALL
SHEAR WALL
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED SHEAR WALL END ANCHOR
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED SHEAR WALL END ANCHOR
SHEAR WALL BOUNDARY / EDGE NAILING
(2) 2X END POST MEMBER
(2) 2X END POST MEMBER
SCHEDULED STUD SPACING
EQ
EQ
9"
1' - 0"
SHEAR WALL FIELD NAILING (TYPICAL)
SHEAR WALL (WHERE OCCURS)
2X "T" INTERSECTION STUDS TO MATCH STUD / SHEAR WALL SCHEDULE
RIPPED 2X AS REQUIRED
CORRIDOR SIDE

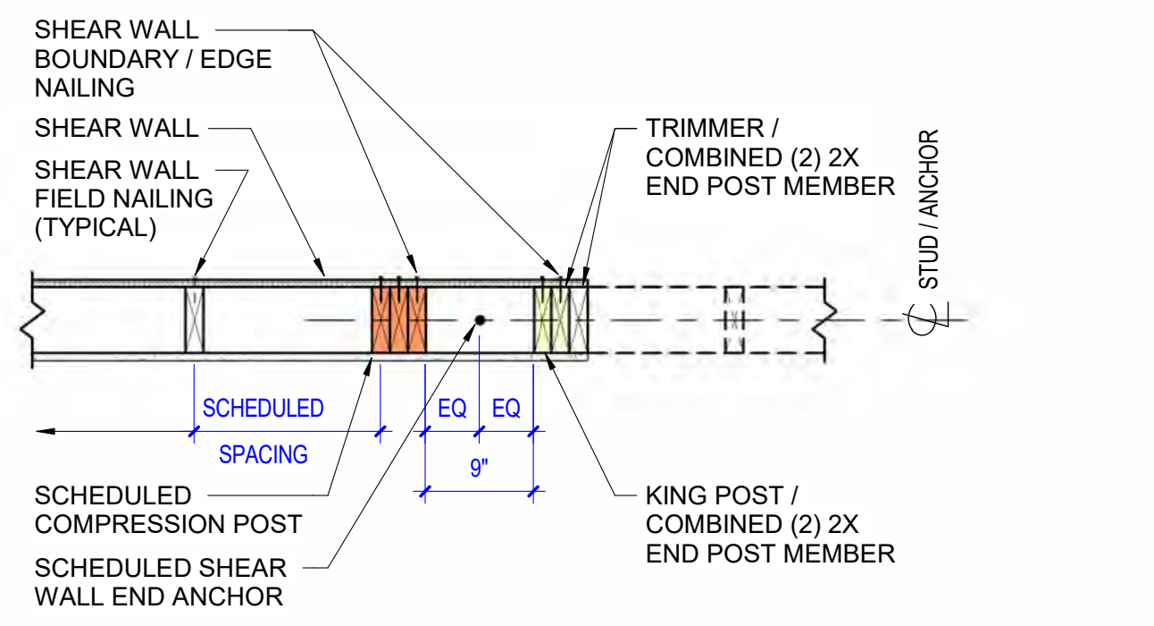
⑨ Interior Shear Wall Anchorage - Demising Wall 2 Sides
3/4" = 1'-0"


STUD / ANCHOR
STUD / ANCHOR
SHEAR WALL
SHEAR WALL
SHEAR WALL FIELD NAILING (TYPICAL)
SCHEDULED SHEAR WALL END ANCHOR
SHEAR WALL BOUNDARY / EDGE NAILING
(2) 2X END POST MEMBER
SCHEDULED SHEAR WALL END ANCHOR
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED STUD SPACING
EQ
EQ
9"
2' - 0"
SHEAR WALL FIELD NAILING (TYPICAL)
SHEAR WALL
(2) 2X END POST MEMBER
2X "T" INTERSECTION STUDS TO MATCH STUD / SHEAR WALL SCHEDULE
RIPPED 2X AS REQUIRED
EXTERIOR

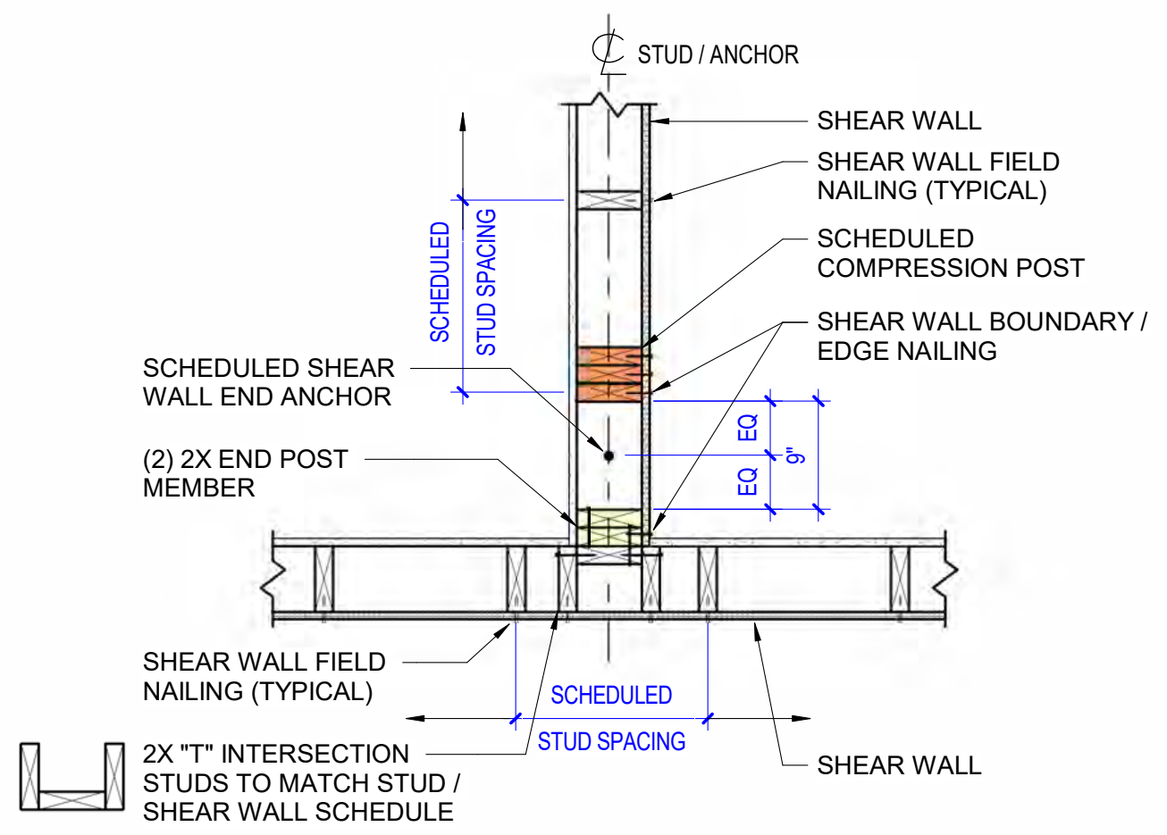
⑧ Exterior Shear Wall Anchorage - Demising Wall 2 Sides
3/4" = 1'-0"


SHEAR WALL BOUNDARY / EDGE NAILING
SHEAR WALL
SHEAR WALL FIELD NAILING (TYPICAL)
STUD / ANCHOR
SCHEDULED SPACING
EQ
EQ
9"
SCHEDULED COMPRESSION POST
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER

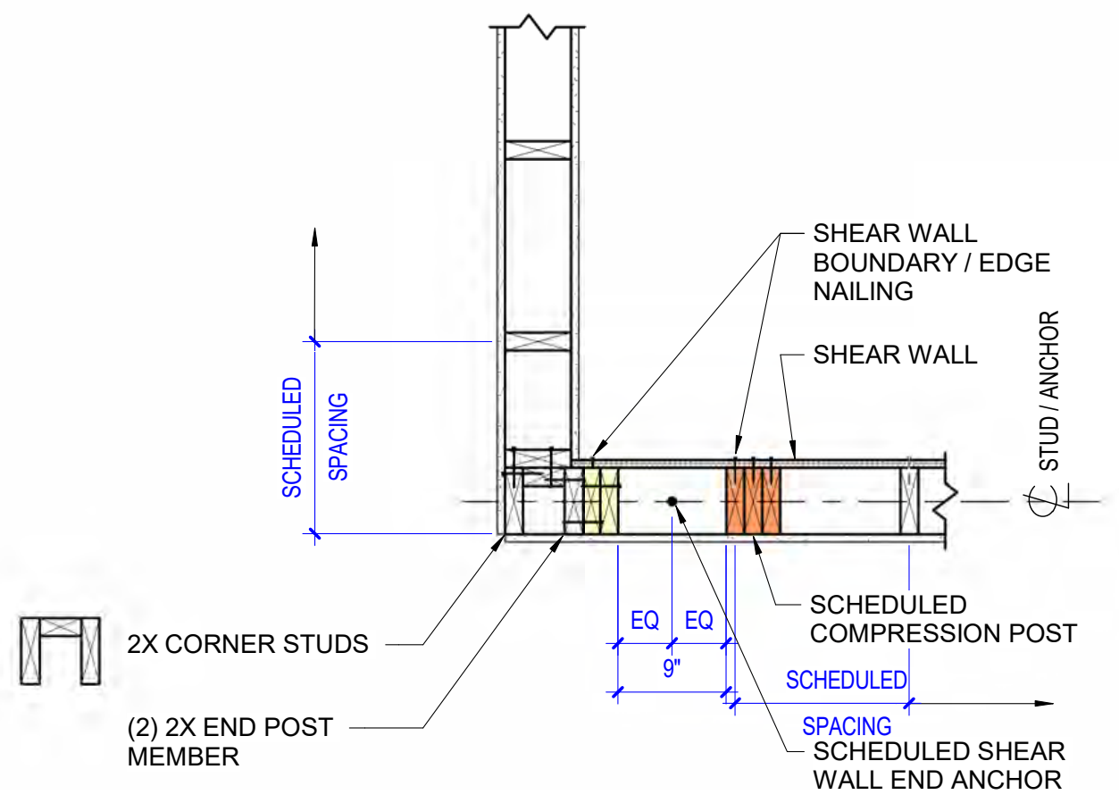
⑦ nterior Shear Wall Anchorage - End of Wall
3/4" = 1'-0"


SHEAR WALL BOUNDARY / EDGE NAILING
SHEAR WALL
SHEAR WALL FIELD NAILING (TYPICAL)
TRIMMER / COMBINED (2) 2X END POST MEMBER
STUD / ANCHOR
SCHEDULED SPACING
EQ
EQ
9"
SCHEDULED COMPRESSION POST
SCHEDULED SHEAR WALL END ANCHOR
KING POST / COMBINED (2) 2X END POST MEMBER

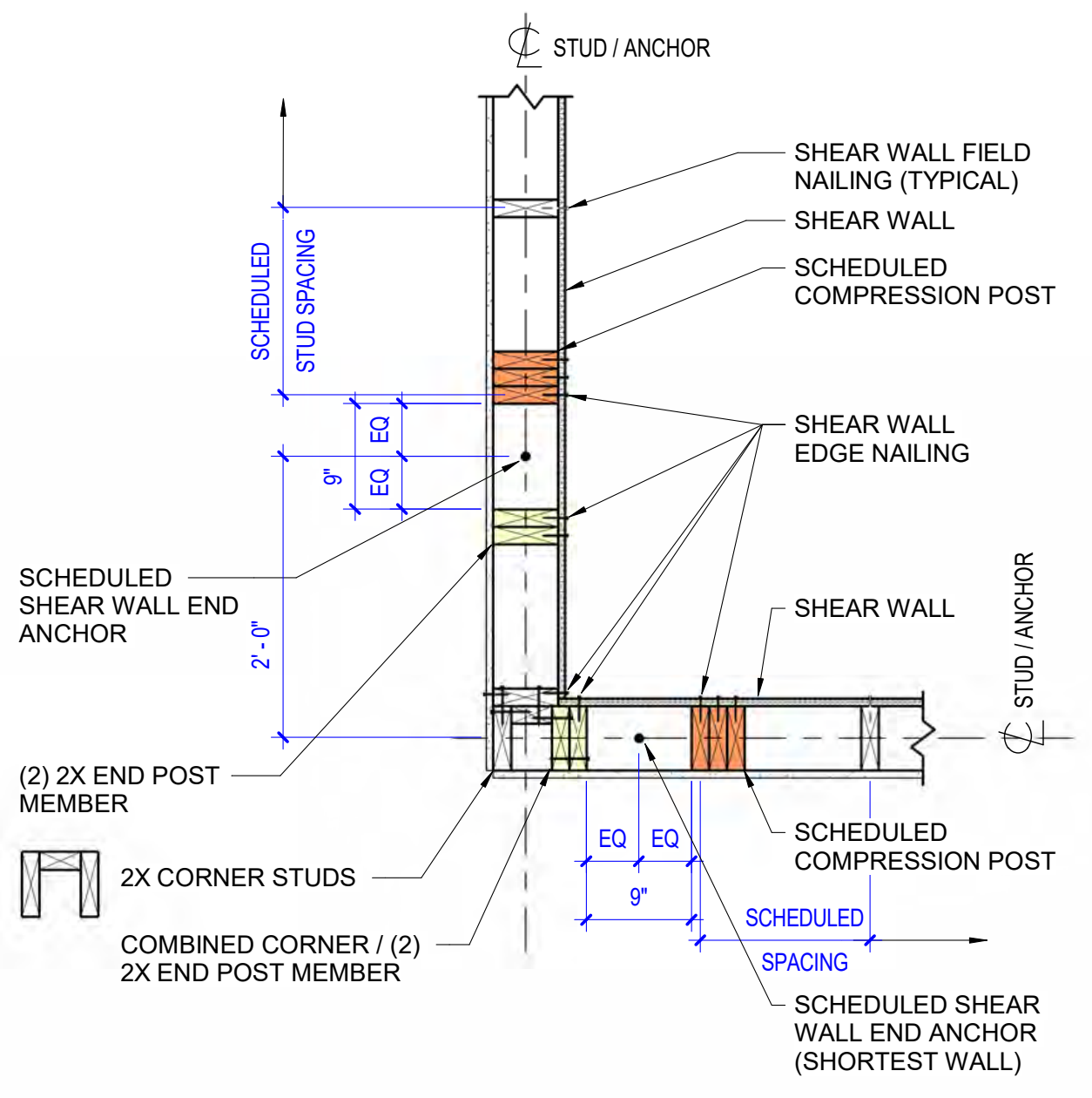
⑥ Exterior Shear Wall Anchorage - Wall Opening
3/4" = 1'-0"


STUD / ANCHOR
SHEAR WALL
SHEAR WALL FIELD NAILING (TYPICAL)
SCHEDULED COMPRESSION POST
SHEAR WALL BOUNDARY / EDGE NAILING
SCHEDULED SHEAR WALL END ANCHOR
(2) 2X END POST MEMBER
SCHEDULED STUD SPACING
EQ
EQ
9"
SHEAR WALL FIELD NAILING (TYPICAL)
2X "T" INTERSECTION STUDS TO MATCH STUD / SHEAR WALL SCHEDULE
SHEAR WALL

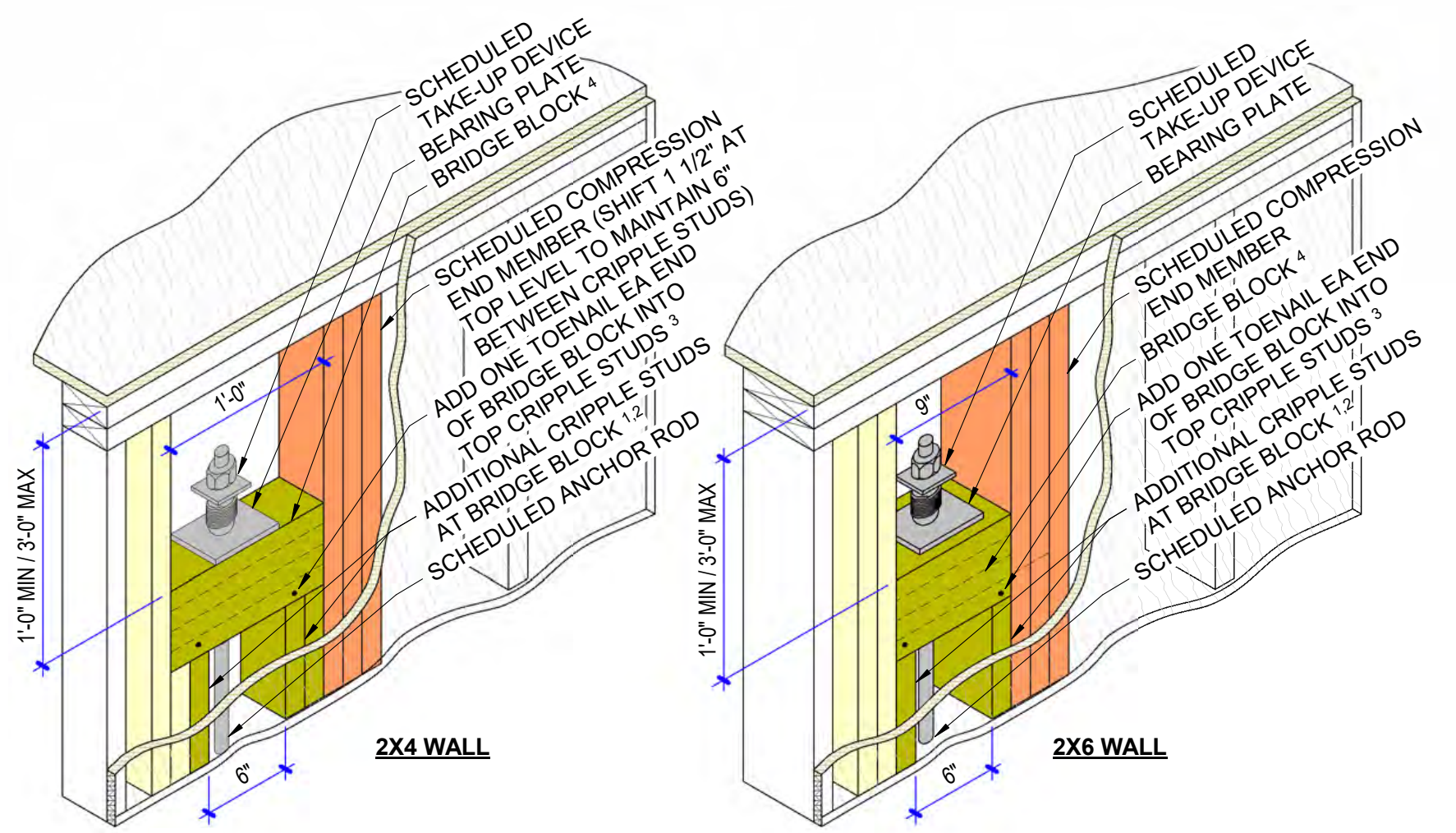
⑤ Shear Wall Anchorage - Tee
3/4" = 1'-0"



④ Exterior Shear Wall Anchorage - Corner
3/4" = 1'-0"

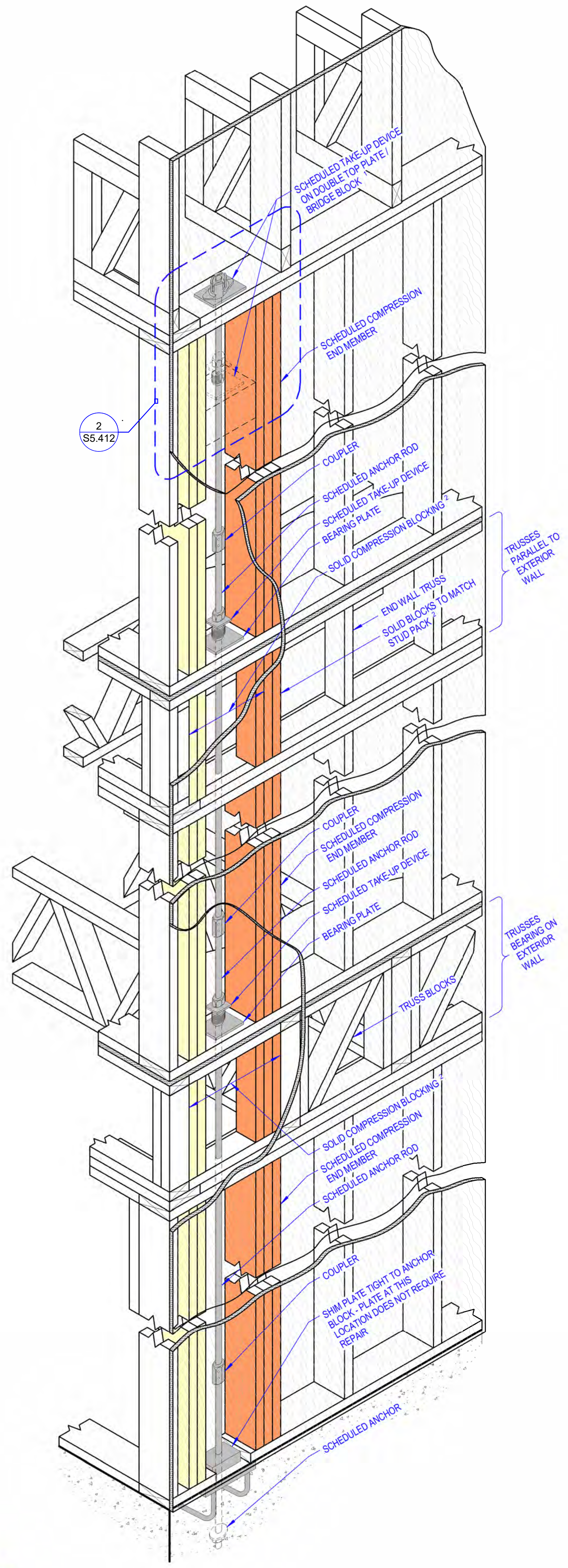

STUD / ANCHOR
SHEAR WALL FIELD NAILING (TYPICAL)
SHEAR WALL
SCHEDULED COMPRESSION POST
SHEAR WALL EDGE NAILING
SCHEDULED STUD SPACING
EQ
EQ
9"
2' - 0"
SCHEDULED SHEAR WALL END ANCHOR
SHEAR WALL
STUD / ANCHOR
(2) 2X END POST MEMBER
2X CORNER STUDS
COMBINED CORNER / (2) 2X END POST MEMBER
EQ
EQ
9"
SCHEDULED COMPRESSION POST
SCHEDULED SPACING
SCHEDULED SHEAR WALL END ANCHOR (SHORTEST WALL)

③ Shear Wall Anchorage - Corner
3/4" = 1'-0"


SCHEDULED TAKE-UP DEVICE
BEARING PLATE
BRIDGE BLOCK 4
SCHEDULED COMPRESSION END MEMBER (SHIFT 1 1/2" AT TOP LEVEL TO MAINTAIN 6" BETWEEN CRIPPLE STUDS)
ADD ONE TOENAIL EA END OF BRIDGE BLOCK INTO TOP CRIPPLE STUDS 3
ADDITIONAL CRIPPLE STUDS AT BRIDGE BLOCK 1,2
SCHEDULED ANCHOR ROD
1'-0"
1'-0" MIN / 3'-0" MAX
6"
2X4 WALL
SCHEDULED TAKE-UP DEVICE
BEARING PLATE
SCHEDULED COMPRESSION END MEMBER
BRIDGE BLOCK 4
ADD ONE TOENAIL EA END OF BRIDGE BLOCK INTO TOP CRIPPLE STUDS 3
ADDITIONAL CRIPPLE STUDS AT BRIDGE BLOCK 1,2
SCHEDULED ANCHOR ROD
9"
1'-0" MIN / 3'-0" MAX
6"
2X6 WALL

NOTE:
1. CRIPPLE STUDS SHALL BE 2X MEMBERS STICHED NAILED PER FASTENING SCHEDULE WITH A MAX NAIL SPACING OF 6" O.C. (CRIPPLE STUDS MAY BE NAILED TO FULL HEIGHT STUDS FROM EITHER SIDE.)
2. CRIPPLE STUDS TO BE (TOP LEVEL ONLY):
   - 2X4 WALL = (2) 2X4 EA SIDE (ONE END MEMBER STUD TO BE USED AS CRIPPLE STUD)
   - 2X6 WALL = (1) 2X6 EA SIDE
3. DO NOT NAIL BRIDGE BLOCK TO FULL HEIGHT END / COMRESSION MEMBER STUDS OR TO SHEATHING.
4. BRIDGE BLOCK TO BE:
   - 2X4 WALL = 4X6 #1 OR (4) 2X4 #1 STACKED FLATWISE
   - 2X6 WALL = 6X6 #1 OR (4) 2X6 #1 STACKED FLATWISE

② Shear Wall Anchor Anchorage
1" = 1'-0"


SCHEDULED TAKE-UP DEVICE ON DOUBLE TOP PLATE / BRIDGE BLOCK 1
SCHEDULED COMPRESSION END MEMBER
2
S5.412
COUPLER
SCHEDULED ANCHOR ROD
SCHEDULED TAKE-UP DEVICE
BEARING PLATE
SOLID COMPRESSION BLOCKING 2
END WALL TRUSS
SOLID BLOCKS TO MATCH STUD PACK 2
TRUSSES PARALLEL TO EXTERIOR WALL
COUPLER
SCHEDULED COMPRESSION END MEMBER
SCHEDULED ANCHOR ROD
SCHEDULED TAKE-UP DEVICE
BEARING PLATE
TRUSSES BEARING ON EXTERIOR WALL
TRUSS BLOCKS
SOLID COMPRESSION BLOCKING 2
SCHEDULED COMPRESSION END MEMBER
SCHEDULED ANCHOR ROD
COUPLER
SHIM PLATE TIGHT TO ANCHOR BLOCK - PLATE AT THIS LOCATION DOES NOT REQUIRE REPAIR
SCHEDULED ANCHOR

NOTE:
1. BRIDGE BLOCK TO BE USED CONTRACTORS OPTION AND AT CONDITIONS WHERE TRUSS BEARING INTERFERES WITH SCHEDULED ANCHOR ROD TAKE-UP DEVICE.
2. SOLID COMPRESSION BLOCKING IN TRUSS CAVITY TO MATCH SCHEDULED COMPRESSION POST.

① Shear Wall Hold-Down Schematic
1" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

NILES BOLTON ASSOCIATES

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No.	Description	Date
	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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- MARTIN R. MAINGOT

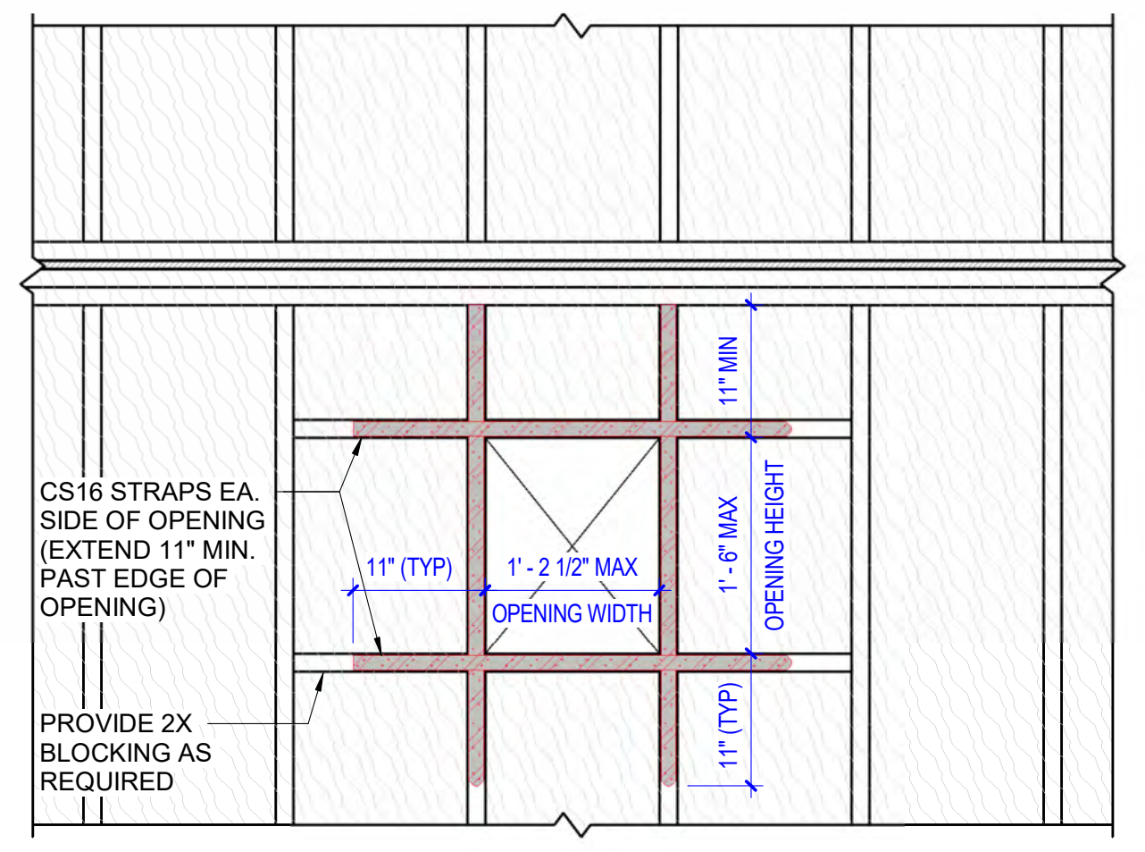
LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Wood Shear Wall Hold-Down Details - ATR / Rod System

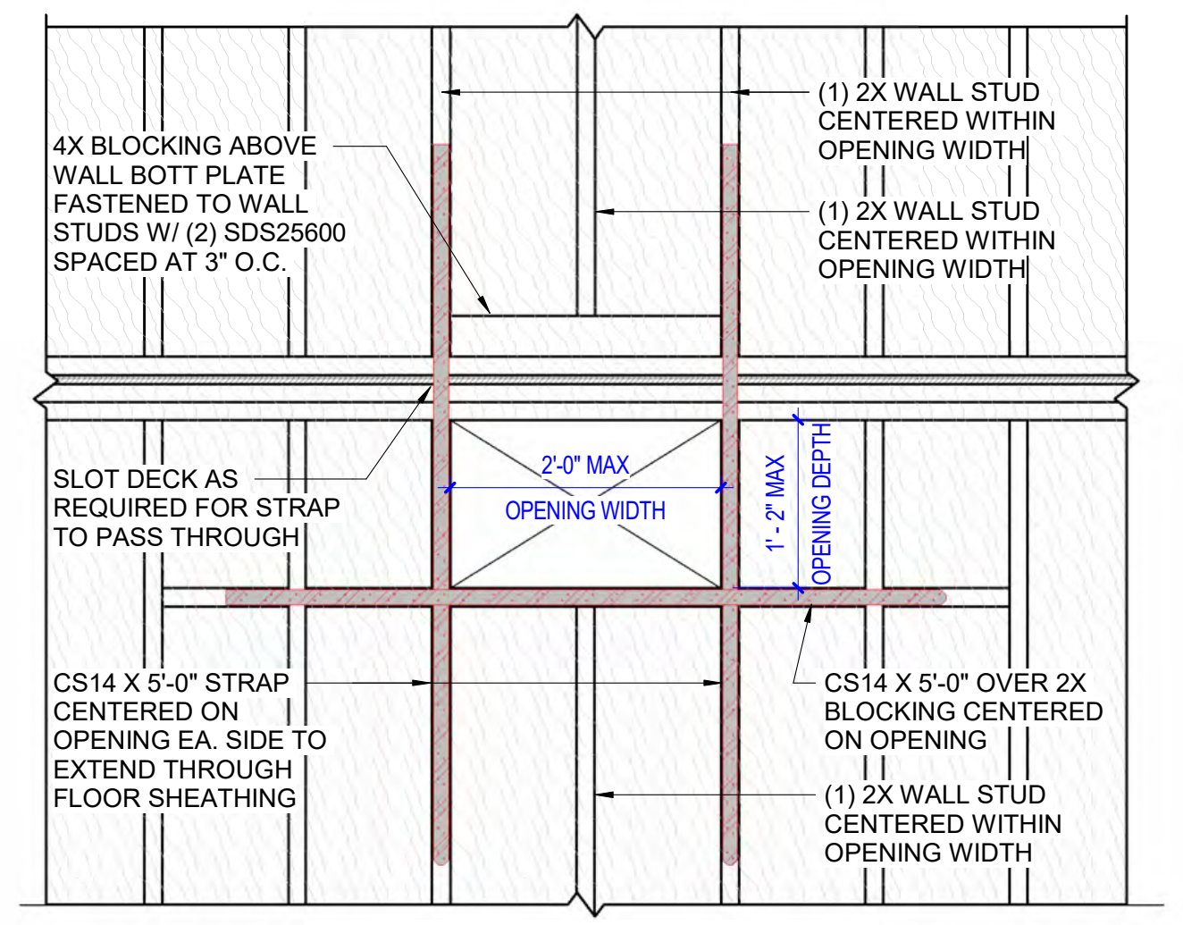
SHEET NUMBER:
S5.412

NOT RELEASED FOR CONSTRUCTION

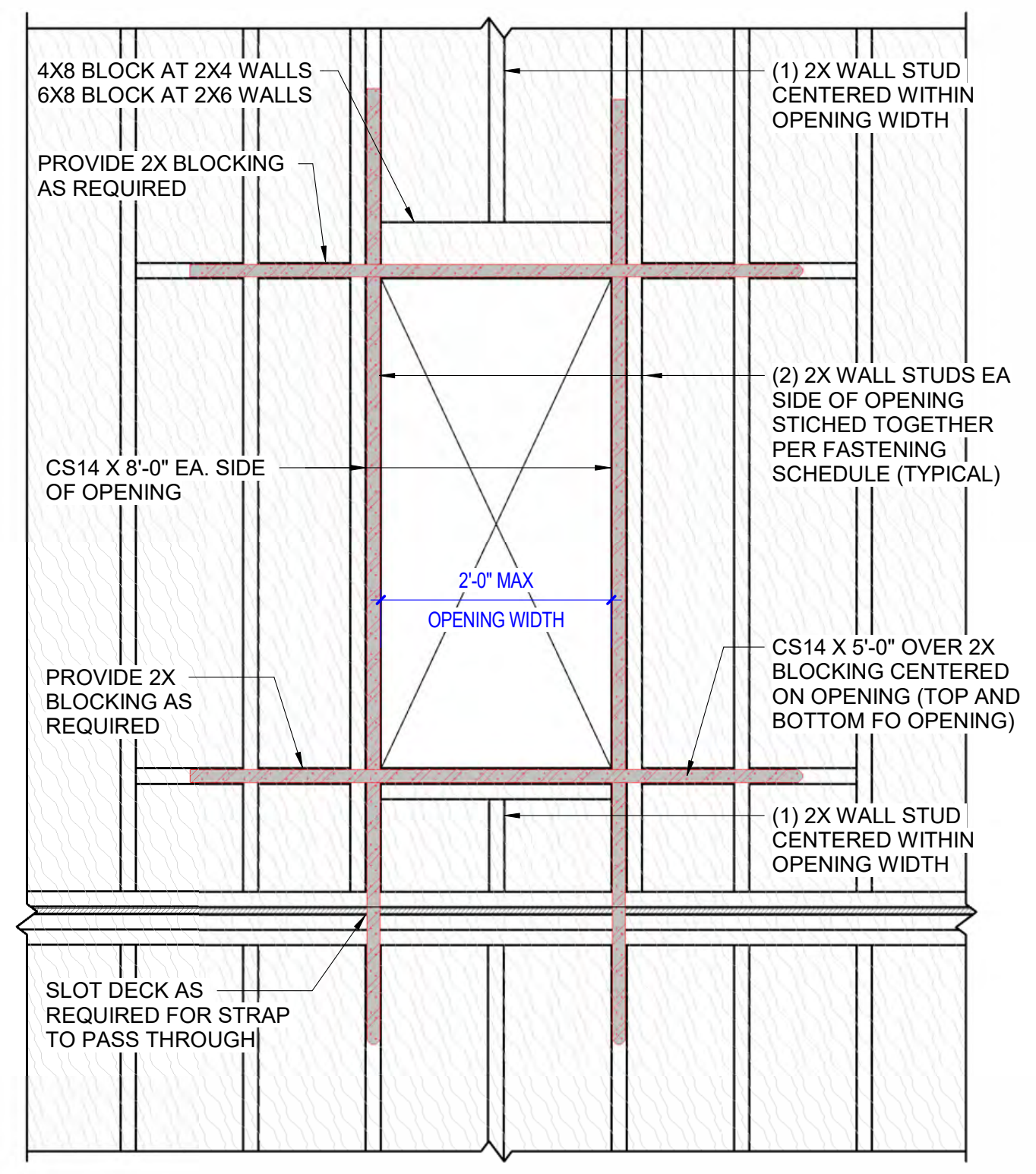
9/30/2026 7:35:17 PM


CS16 STRAPS EA. SIDE OF OPENING (EXTEND 11" MIN. PAST EDGE OF OPENING)
PROVIDE 2X BLOCKING AS REQUIRED
11" MIN
11" (TYP)
1' - 2 1/2" MAX
OPENING WIDTH
1' - 6" MAX
OPENING HEIGHT
11" (TYP)

1'-6" HIGH MAX OPENING IN SHEAR WALL


4X BLOCKING ABOVE WALL BOTT PLATE FASTENED TO WALL STUDS W/ (2) SDS25600 SPACED AT 3" O.C.
(1) 2X WALL STUD CENTERED WITHIN OPENING WIDTH
(1) 2X WALL STUD CENTERED WITHIN OPENING WIDTH
SLOT DECK AS REQUIRED FOR STRAP TO PASS THROUGH
2'-0" MAX
OPENING WIDTH
1' - 2" MAX
OPENING DEPTH
CS14 X 5'-0" STRAP CENTERED ON OPENING EA. SIDE TO EXTEND THROUGH FLOOR SHEATHING
CS14 X 5'-0" OVER 2X BLOCKING CENTERED ON OPENING
(1) 2X WALL STUD CENTERED WITHIN OPENING WIDTH

OPENING AT TOP OF SHEAR WALL


4X8 BLOCK AT 2X4 WALLS
6X8 BLOCK AT 2X6 WALLS
PROVIDE 2X BLOCKING AS REQUIRED
(1) 2X WALL STUD CENTERED WITHIN OPENING WIDTH
(2) 2X WALL STUDS EA SIDE OF OPENING STICHED TOGETHER PER FASTENING SCHEDULE (TYPICAL)
CS14 X 8'-0" EA. SIDE OF OPENING
2'-0" MAX
OPENING WIDTH
PROVIDE 2X BLOCKING AS REQUIRED
CS14 X 5'-0" OVER 2X BLOCKING CENTERED ON OPENING (TOP AND BOTTOM FO OPENING)
(1) 2X WALL STUD CENTERED WITHIN OPENING WIDTH
SLOT DECK AS REQUIRED FOR STRAP TO PASS THROUGH

4'-0" HIGH HEIGHT OPENING IN SHEAR WALL

NOTE:
1. ALL PANEL EDGES AT PERIMETER OF OPENINGS TO BE NAILED PER EDGE NAILING IN SHEAR WALL SCHEDULE

① Temporary and Permanent Openings In Wood Shear Walls
3/4" = 1'-0"

VSM2
Voss | Starling | Maingot | Martin
Structural Engineers

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Wood Shear Wall Typical Details

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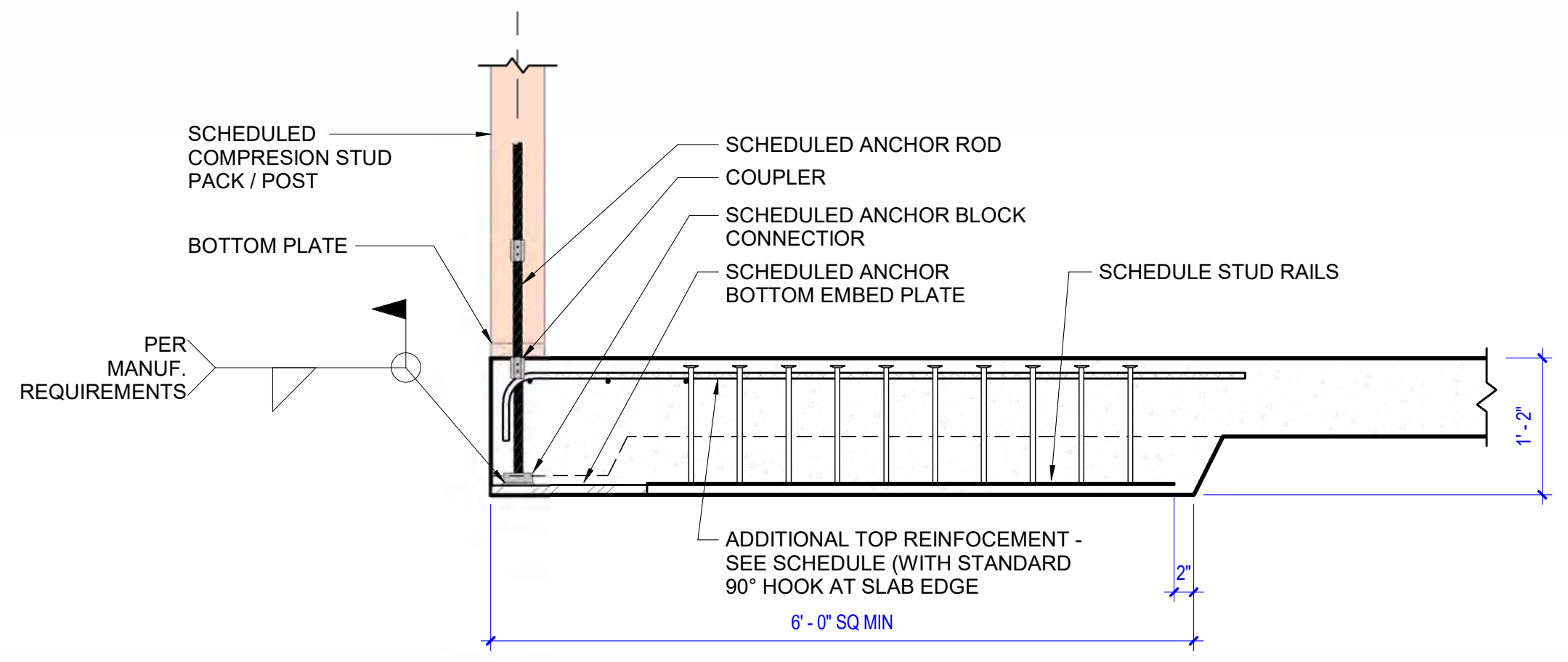
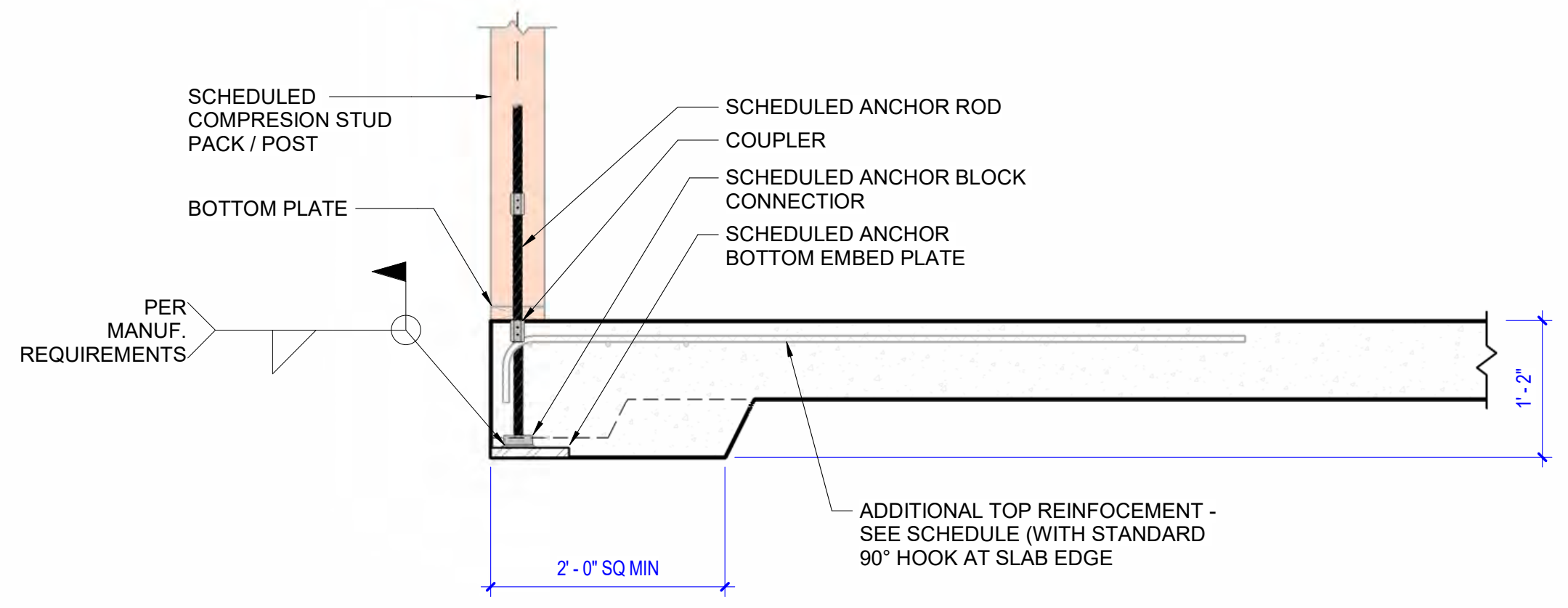
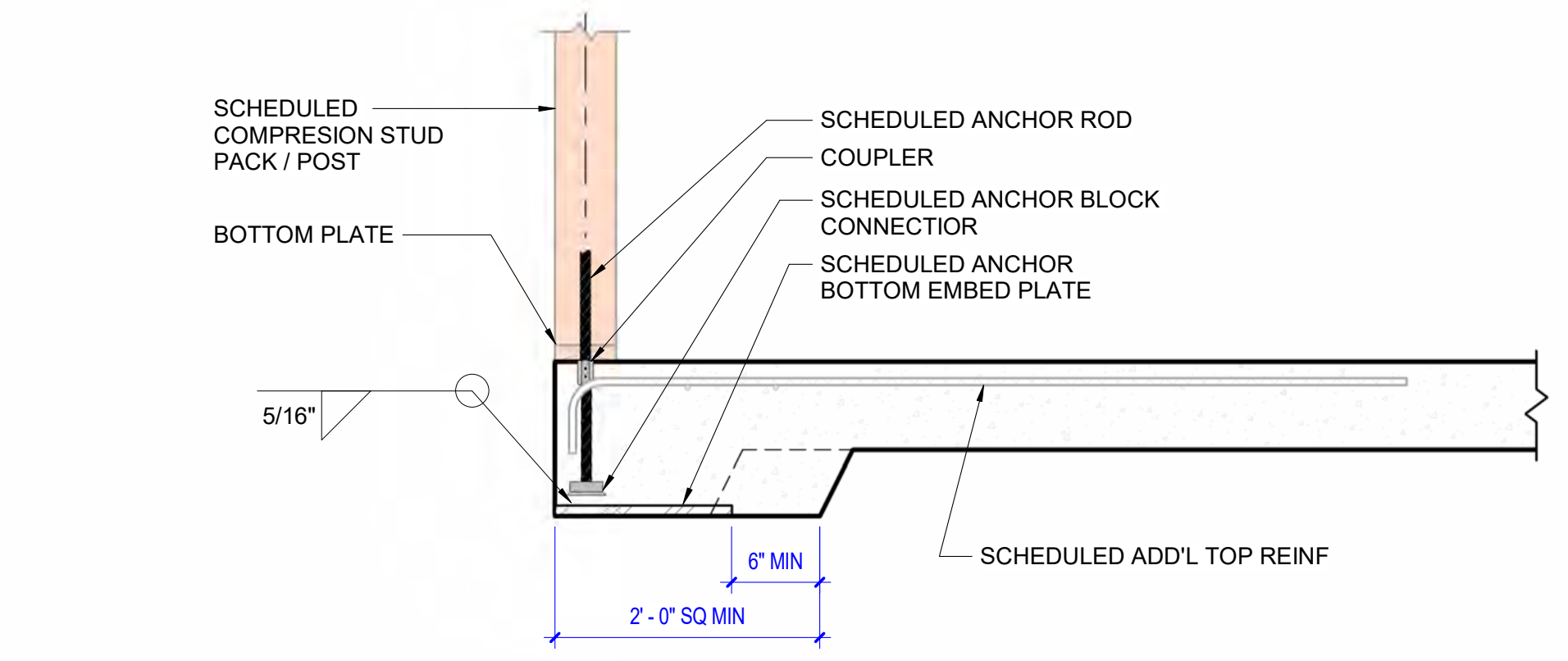

SCHEDULED COMPRESION STUD PACK / POST
BOTTOM PLATE
PER MANUF. REQUIREMENTS
SCHEDULED ANCHOR ROD
COUPLER
SCHEDULED ANCHOR BLOCK CONNECTIOR
SCHEDULED ANCHOR BOTTOM EMBED PLATE
SCHEDULE STUD RAILS
ADDITIONAL TOP REINFOCEMENT - SEE SCHEDULE (WITH STANDARD 90° HOOK AT SLAB EDGE
1' - 2"
2"
6' - 0" SQ MIN

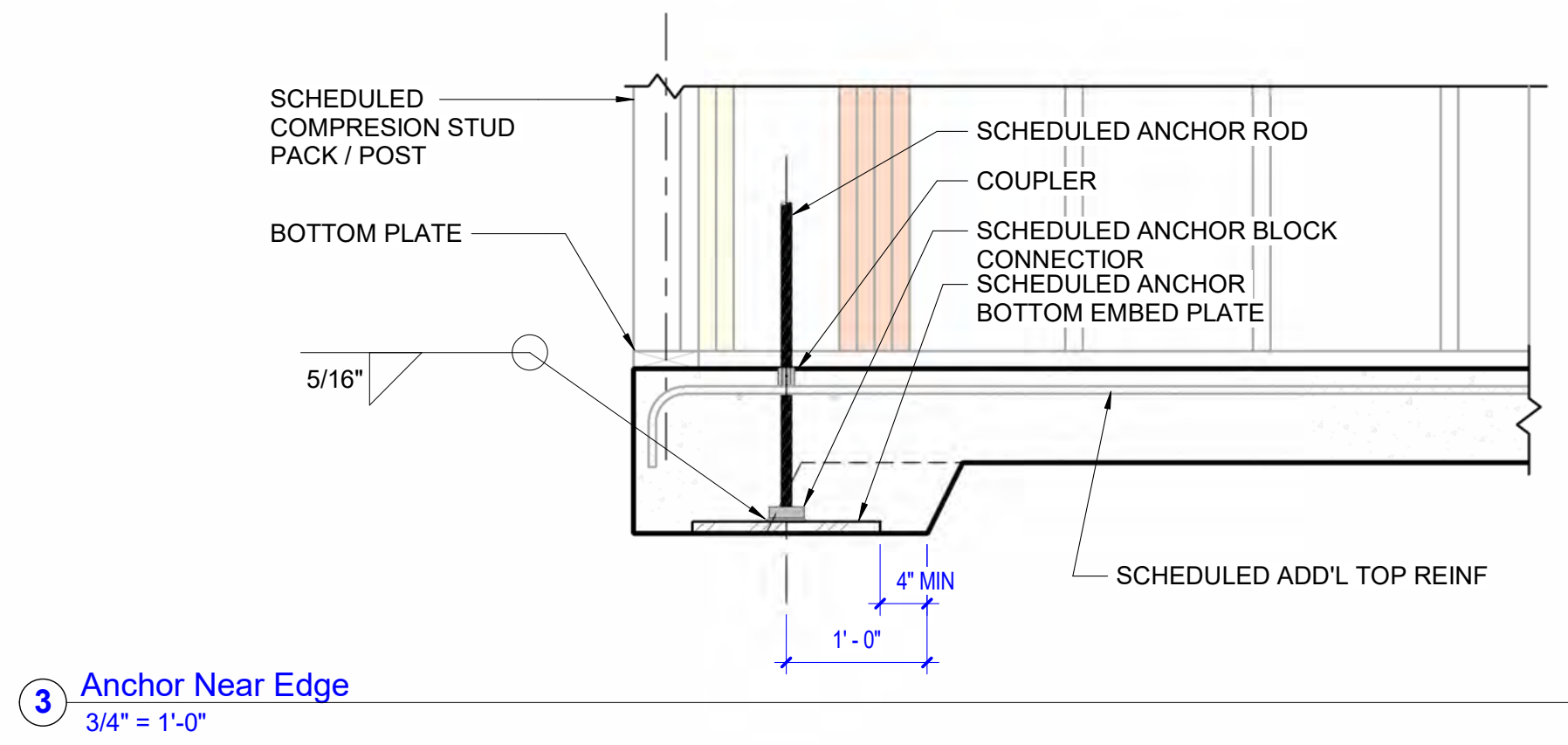
6 Anchor at Corner
3/4" = 1'-0"


SCHEDULED COMPRESION STUD PACK / POST
BOTTOM PLATE
PER MANUF. REQUIREMENTS
SCHEDULED ANCHOR ROD
COUPLER
SCHEDULED ANCHOR BLOCK CONNECTIOR
SCHEDULED ANCHOR BOTTOM EMBED PLATE
ADDITIONAL TOP REINFOCEMENT - SEE SCHEDULE (WITH STANDARD 90° HOOK AT SLAB EDGE
1' - 2"
2' - 0" SQ MIN

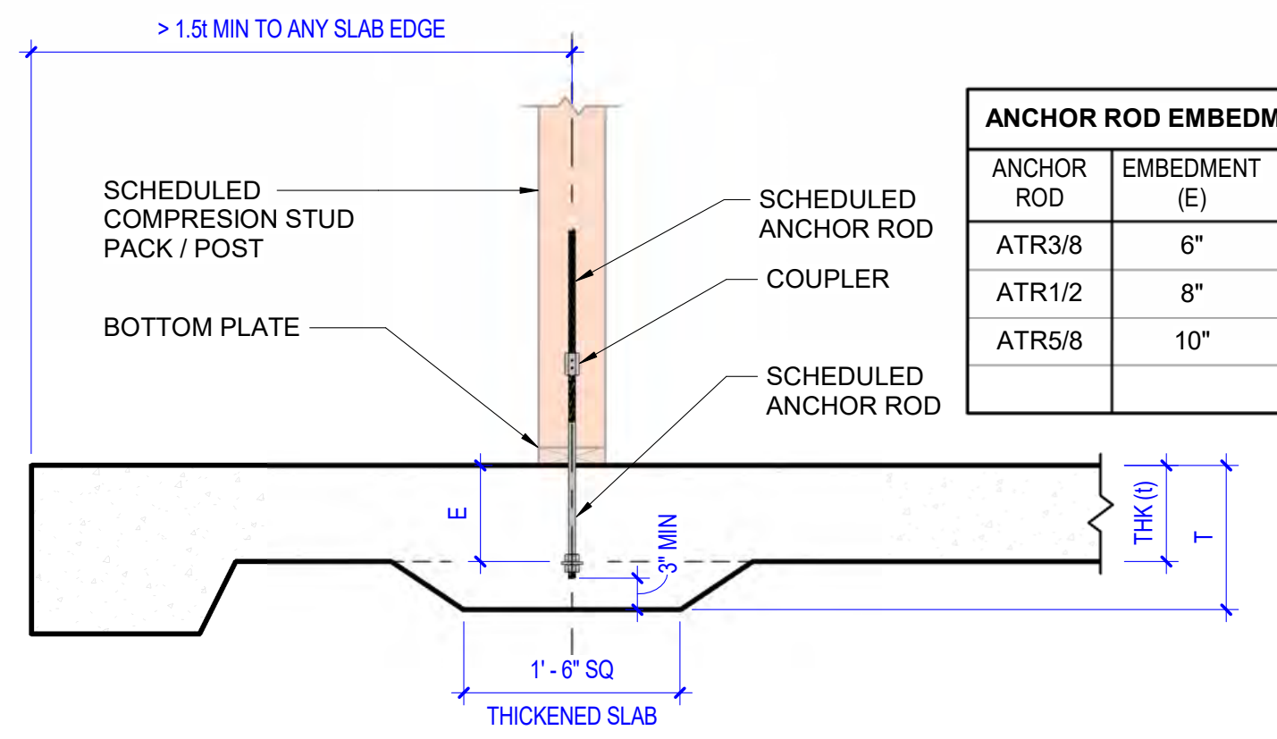
5 Anchor at Corner
3/4" = 1'-0"


SCHEDULED COMPRESION STUD PACK / POST
BOTTOM PLATE
5/16"
SCHEDULED ANCHOR ROD
COUPLER
SCHEDULED ANCHOR BLOCK CONNECTIOR
SCHEDULED ANCHOR BOTTOM EMBED PLATE
SCHEDULED ADD'L TOP REINF
6" MIN
2' - 0" SQ MIN

4 Anchor at Slab Edge
3/4" = 1'-0"

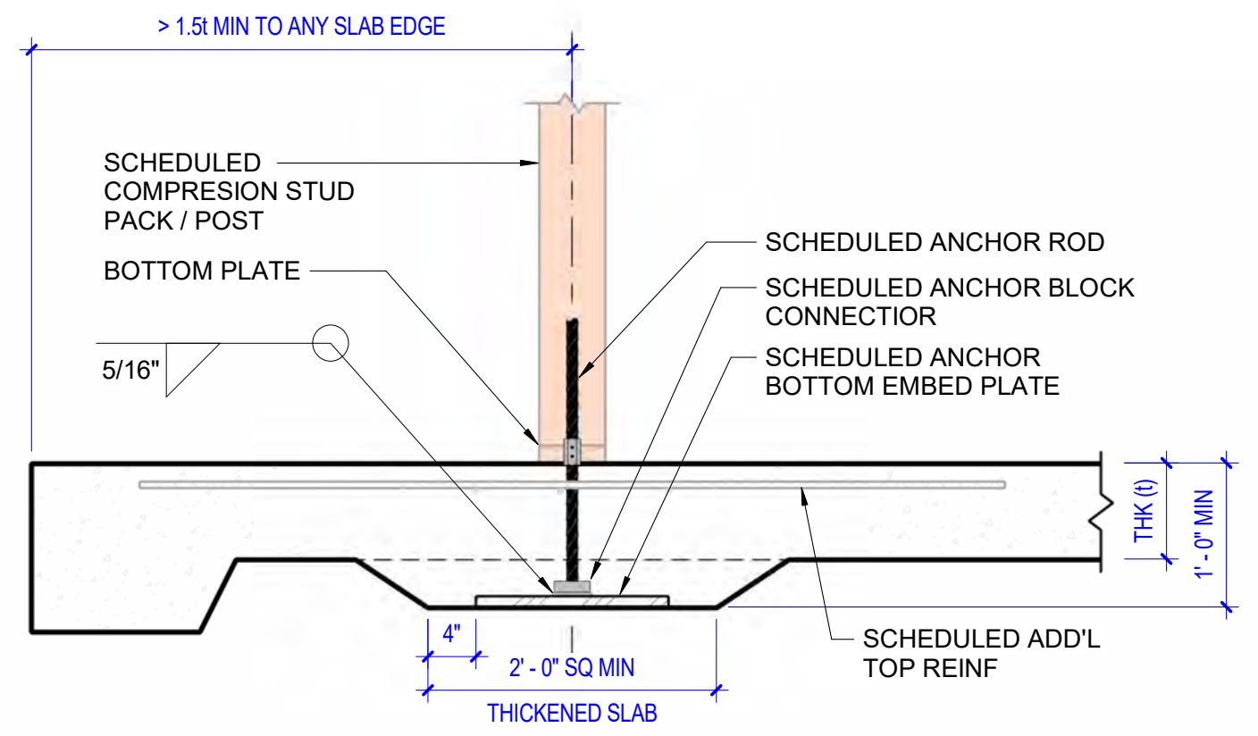

SCHEDULED COMPRESION STUD PACK / POST
BOTTOM PLATE
5/16"
SCHEDULED ANCHOR ROD
COUPLER
SCHEDULED ANCHOR BLOCK CONNECTIOR
SCHEDULED ANCHOR BOTTOM EMBED PLATE
SCHEDULED ADD'L TOP REINF
4" MIN
1' - 0"

3 Anchor Near Edge
3/4" = 1'-0"

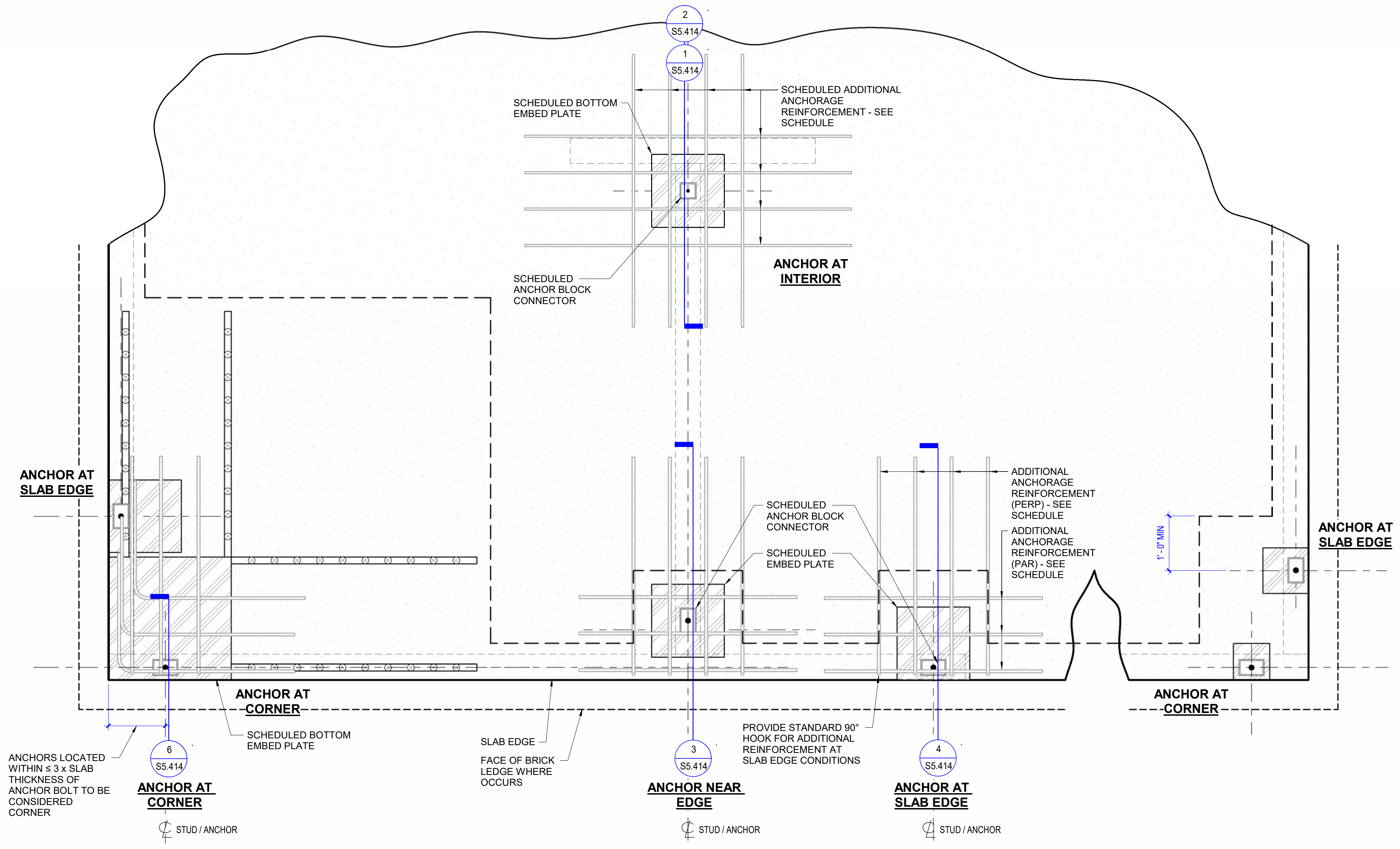


ANCHOR ROD EMBEDMENT TABLE		
ANCHOR ROD	EMBEDMENT (E)	THICKNESS (T)
ATR3/8	6"	1'-0"
ATR1/2	8"	1'-2"
ATR5/8	10"	1'-4"

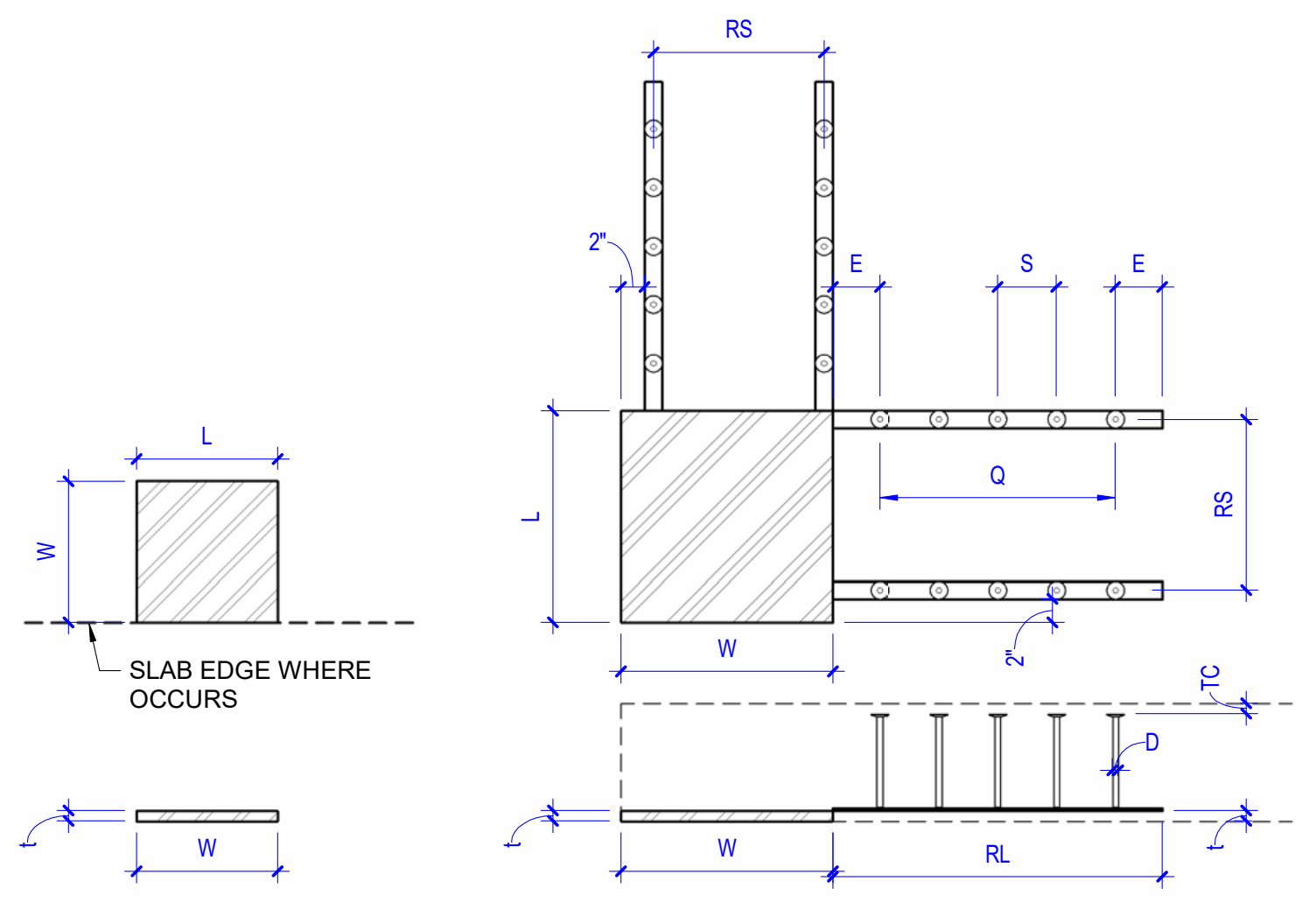
2 Anchor at Interior
3/4" = 1'-0"


> 1.5t MIN TO ANY SLAB EDGE
SCHEDULED COMPRESION STUD PACK / POST
BOTTOM PLATE
5/16"
SCHEDULED ANCHOR ROD
SCHEDULED ANCHOR BLOCK CONNECTIOR
SCHEDULED ANCHOR BOTTOM EMBED PLATE
SCHEDULED ADD'L TOP REINF
THK (t)
1' - 0" MIN
4"
2' - 0" SQ MIN
THICKENED SLAB

1 Anchor at Interior
3/4" = 1'-0"


2 S5.414
1 S5.414
SCHEDULED BOTTOM EMBED PLATE
SCHEDULED ADDITIONAL ANCHORAGE REINFORCEMENT - SEE SCHEDULE
SCHEDULED ANCHOR BLOCK CONNECTOR
ANCHOR AT INTERIOR
ANCHOR AT SLAB EDGE
SCHEDULED ANCHOR BLOCK CONNECTOR
SCHEDULED EMBED PLATE
ADDITIONAL ANCHORAGE REINFORCEMENT (PERP) - SEE SCHEDULE
ADDITIONAL ANCHORAGE REINFORCEMENT (PAR) - SEE SCHEDULE
1' - 0" MIN
ANCHOR AT SLAB EDGE
ANCHOR AT CORNER
ANCHOR AT CORNER
ANCHORS LOCATED WITHIN ≤ 3 x SLAB THICKNESS OF ANCHOR BOLT TO BE CONSIDERED CORNER
SCHEDULED BOTTOM EMBED PLATE
SLAB EDGE
FACE OF BRICK LEDGE WHERE OCCURS
PROVIDE STANDARD 90° HOOK FOR ADDITIONAL REINFORCEMENT AT SLAB EDGE CONDITIONS
6 S5.414
ANCHOR AT CORNER
3 S5.414
ANCHOR NEAR EDGE
4 S5.414
ANCHOR AT SLAB EDGE
℄ STUD / ANCHOR
℄ STUD / ANCHOR
℄ STUD / ANCHOR

ANCHOR LOCATION DIAGRAM



TYPE AP-B1
(B.O. SLAB)

TYPE AP-B2
(B.O. SLAB)

ANCHOR BOTTOM EMBED PLATE TYPES

d = SLAB THK - (PLATE THK + COVER) / d=T - (t + C) - TYPICAL UNO

SHEAR WALL END ANCHOR EMBED PLATE SCHEDULE

ANCHOR BOTTOM EMBED PLATE TYPE	PLATE SIZE			CONFINING REINFORCEMENT
	W	L	t	
AP-B1.3-3	3"	3"	3/8"	(1) TYPE E T&B
AP-B1.512-512	5 1/2"	5 1/2"	5/8"	(1) TYPE E T&B
AP-B1.8-8	8"	8"	7/8"	(1) TYPE E T&B
AP-B1.10-10	10"	10"	1"	(1) TYPE E T&B
AP-B1.10-14	10"	14"	1"	(1) TYPE E T&B
AP-B1.12-12	12"	12"	1 1/4"	(1) TYPE E T&B
AP-B1.12-16	12"	16"	1 1/4"	(1) TYPE E T&B
AP-B1.14-14	14"	14"	1 1/2"	(1) TYPE E T&B
AP-B1.14-18	14"	18"	1 1/2"	(1) TYPE E T&B
AP-B1.15-15	15"	15"	1 1/4"	(1) TYPE E T&B
AP-B1.16-16	16"	16"	1 3/4"	(1) TYPE E T&B
AP-B1.16-20	16"	20"	1 3/4"	(1) TYPE E T&B
AP-B1.18-18	18"	18"	1 3/8"	(1) TYPE E T&B
AP-B1.22-22	22"	22"	1 1/2"	(1) TYPE E T&B

ANCHOR BOTTOM EMBED PLATE TYPE	W	L	t	RAILS PER SIDE (RS)	OVERALL LENGTH OF RAIL (RL)	NUMBER OF STUDS PER RAIL (Q)	DISTANCE TO FIRST STUD (E)	STUD SPACING (S)	STUD DIA. (D)	TOP COVER (TC)
	PLATE SIZE			STUD RAIL REINFORCEMENT						
AP-B2.18-18	18"	18"	1 3/8"	2	24"	5	4"	4"	3/4"	1"
AP-B2.27-27	27"	27"	1 1/2"	2	62"	10	4"	6"	3/4"	1"

BOTTOM EMBED PLATE NOTES:
1. AT LOCATIONS WHERE EMBED PLATE IS EXPOSED TO THE ELEMENTS, PLATE SHALL BE HOT DIP GALVANIZED.
2. ALTERNATIVELY, PROVIDE CJP SHOP WELD BETWEEN SCHEDULED ROD AND PLATE.
3. BOTTOM PLATE SCHEDULE BASED ON 5,000 PSI MIN. SPECIFIED CONCRETE COMPRESSIVE STRENGTH. CONTRACTOR TO COORDINATE INCREASE IN STUD RAIL REINFORCEMENT FOR ALL LOCATIONS WITH LESS THAN 5,000 PSI MIN. SPECIFIED CONCRETE COMPRESSIVE STRENGTH

SHEAR WALL END ANCHOR ROD ANCHORAGE SCHEDULE

ANCHOR ROD ^{1}	LOCATION ^{4}	EMBEDDED ROD ANCHORAGE				LOCATION ^{4}	ANCHOR ROD ^{1}
		SIMPOSON BLOCK ANCHOR	EMBED PLATE	ANCHORAGE DETAIL	ADDITIONAL REINFORCEMENT		
ATR3/8	INTERIOR	SBC6H (W/ REDUCING CPLR)	AP-B1.3-3	2 / S5.414 / 1 / S5.414	NA	INTERIOR	ATR3/8
	NEAR EDGE			3 / S5.414	NA	NEAR EDGE	
	SLAB EDGE			4 / S5.414	NA	SLAB EDGE	
	CORNER		AP-B1.3-3	5 / S5.414	NA	CORNER	
ATR1/2	INTERIOR	SBC6H (W/ REDUCING CPLR)	AP-B1.3-3	2 / S5.414 / 1 / S5.414	NA	INTERIOR	ATR1/2
	NEAR EDGE			3 / S5.414	NA	NEAR EDGE	
	SLAB EDGE			4 / S5.414	NA	SLAB EDGE	
	CORNER		AP-B1.3-3	5 / S5.414	NA	CORNER	
ATR5/8	INTERIOR	SBC6H (W/ REDUCING CPLR)	AP-B1.3-3	2 / S5.414 / 1 / S5.414	NA	INTERIOR	ATR5/8
	NEAR EDGE			3 / S5.414	NA	NEAR EDGE	
	SLAB EDGE			4 / S5.414	NA	SLAB EDGE	
	CORNER		AP-B1.3-3	5 / S5.414	NA	CORNER	
ATR3/4	INTERIOR	SBC6H	AP-B1.512-512	1 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	INTERIOR	ATR3/4
	NEAR EDGE			3 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	SLAB EDGE			4 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	CORNER		AP-B1.512-512	5 / S5.414	(2) #5X6'-6" EW @ 8"O.C. (T)	CORNER	
ATR7/8	INTERIOR	SBC8H (W/ REDUCING CPLR)	AP-B1.8-8	1 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	INTERIOR	ATR7/8
	NEAR EDGE			3 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	SLAB EDGE			4 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	CORNER		AP-B1.8-8	5 / S5.414	(2) #5X6'-6" EW @ 8"O.C. (T)	CORNER	
ATR1	INTERIOR	SBC8H	AP-B1.8-8	1 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	INTERIOR	ATR1
	NEAR EDGE			3 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	SLAB EDGE			4 / S5.414	(2) #4X10'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	CORNER		AP-B1.8-8	5 / S5.414	(2) #5X6'-6" EW @ 8"O.C. (T)	CORNER	
ATR1-1/8	INTERIOR	SBC10H (W/ REDUCING CPLR)	AP-B1.10-10	1 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	INTERIOR	ATR1-1/8
	NEAR EDGE		AP-B1.10-14	3 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	SLAB EDGE		AP-B1.10-14	4 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	CORNER		AP-B1.15-15	5 / S5.414	(3) #5X6'-6" EW @ 4"O.C. (T)	CORNER	
ATR1-1/4	INTERIOR	SBC10H	AP-B1.10-10	1 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	INTERIOR	ATR1-1/4
	NEAR EDGE		AP-B1.10-14	3 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	SLAB EDGE		AP-B1.10-14	4 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	CORNER		AP-B1.15-15	5 / S5.414	(3) #5X6'-6" EW @ 4"O.C. (T)	CORNER	
ATS-HSR10	INTERIOR	SBC10H	AP-B1.14X14	1 / S5.414	(4) #5X11'-10" EW @ 4"O.C. (T)	INTERIOR	ATS-HSR10
	NEAR EDGE		AP-B1.14X18	3 / S5.414	(4) #5X11'-10" EW @ 4"O.C. (T)	NEAR EDGE	
	SLAB EDGE		AP-B1.14X18	4 / S5.414	(4) #5X11'-10" EW @ 4"O.C. (T)	SLAB EDGE	
	CORNER		AP-B2.1	6 / S5.414	(6) #5X6'-6" EW @ 4"O.C. (T)	CORNER	
ATR1-3/8	INTERIOR	SBC11H	AP-B1.12-12	1 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	INTERIOR	ATR1-3/8
	NEAR EDGE		AP-B1.12-16	3 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	SLAB EDGE		AP-B1.12-16	4 / S5.414	(2) #5X11'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	CORNER		AP-B1.18-18	5 / S5.414	(3) #5X6'-6" EW @ 4"O.C. (T)	CORNER	
ATR1-1/2	INTERIOR	SBC12H	AP-B1.12-12	1 / S5.414	(3) #5X11'-6" EW @ 4"O.C. (T)	INTERIOR	ATR1-1/2
	NEAR EDGE		AP-B1.12-16	3 / S5.414	(3) #5X11'-6" EW @ 4"O.C. (T)	NEAR EDGE	
	SLAB EDGE		AP-B1.12-16	4 / S5.414	(3) #5X11'-6" EW @ 4"O.C. (T)	SLAB EDGE	
	CORNER		AP-B1.22-22	6 / S5.414	(3) #6X8'-0" EW @ 4"O.C. (T)	CORNER	
ATS-HSR12	INTERIOR	SBC12H	AP-B1.16-16	1 / S5.414	(6) #5X12'-0" EW @ 4"O.C. (T)	INTERIOR	ATS-HSR12
	NEAR EDGE		AP-B1.16-20	3 / S5.414	(6) #5X12'-0" EW @ 4"O.C. (T)	NEAR EDGE	
	SLAB EDGE		AP-B1.16-20	4 / S5.414	(6) #5X12'-0" EW @ 4"O.C. (T)	SLAB EDGE	
	CORNER		AP-B2.2	6 / S5.414	(3) #6X8'-0" EW @ 4"O.C. (T)	CORNER	

NOTE:
1. SEE SHEAR WALL BRACING PLAN FOR SHEAR WALL END ANCHORAGE ROD
2. SEE ANCHOR ROD / ANCHOR DETAILS FOR ANCHOR REINFORCEMENT
3. WHEN SCHEDULED ANCHOR IS LARGER THAN SCHEDULED ROD, USE SIMPSON REDUCING COUPLER
4. SEE ANCHORAGE DIAGRAM TO DETERMINE ANCHOR LOCATION FOR EDGE, NEAR EDGE, INTERIOR, AND CORNER CONDITIONS.
5. ALL BOTTOM EMBED PLATES TO BE EPOXY COATED.

9/30/2026 7:35:22 PM

VSM²
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SHEET TITLE:
Wood Shear Wall Anchorage ATR / Rod System - Uniform Thickness Slab on Grade

SHEET NUMBER:
S5.414

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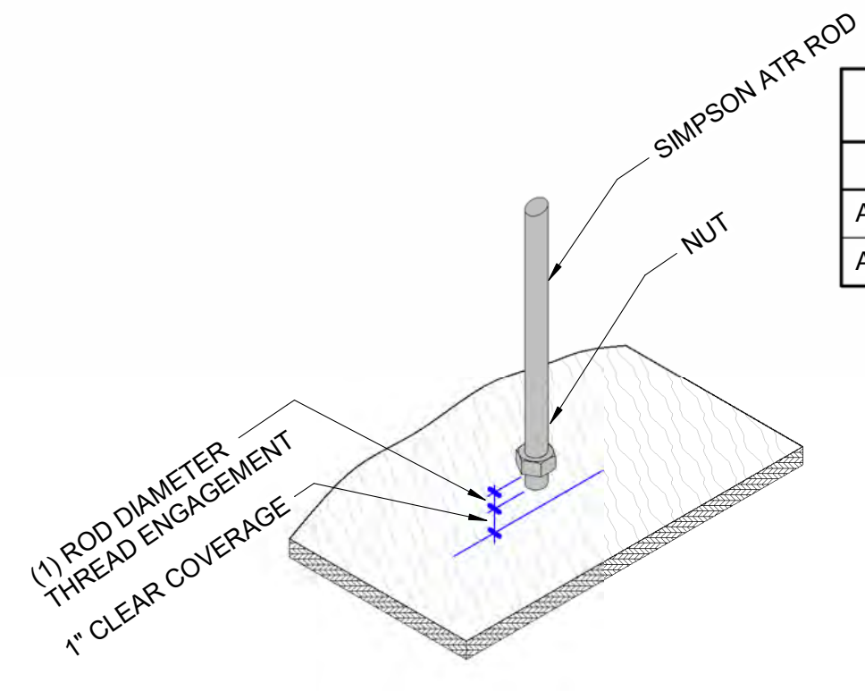
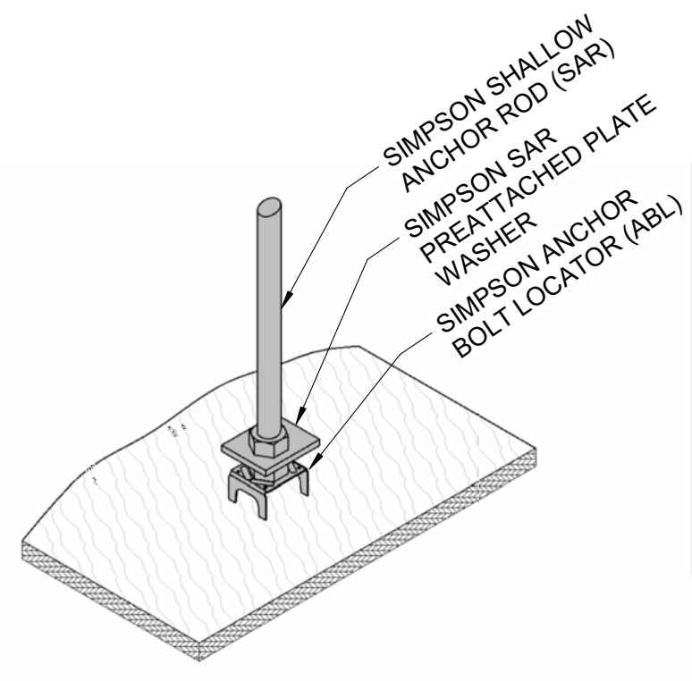


ANCHOR BOLT (AB)

ANCHOR PLATE TABLE	
ROD	ANCHOR NUT
ATR3/8	3/8"Ø HVY HEX NUT
ATR1/2	1/2"Ø HVY HEX NUT

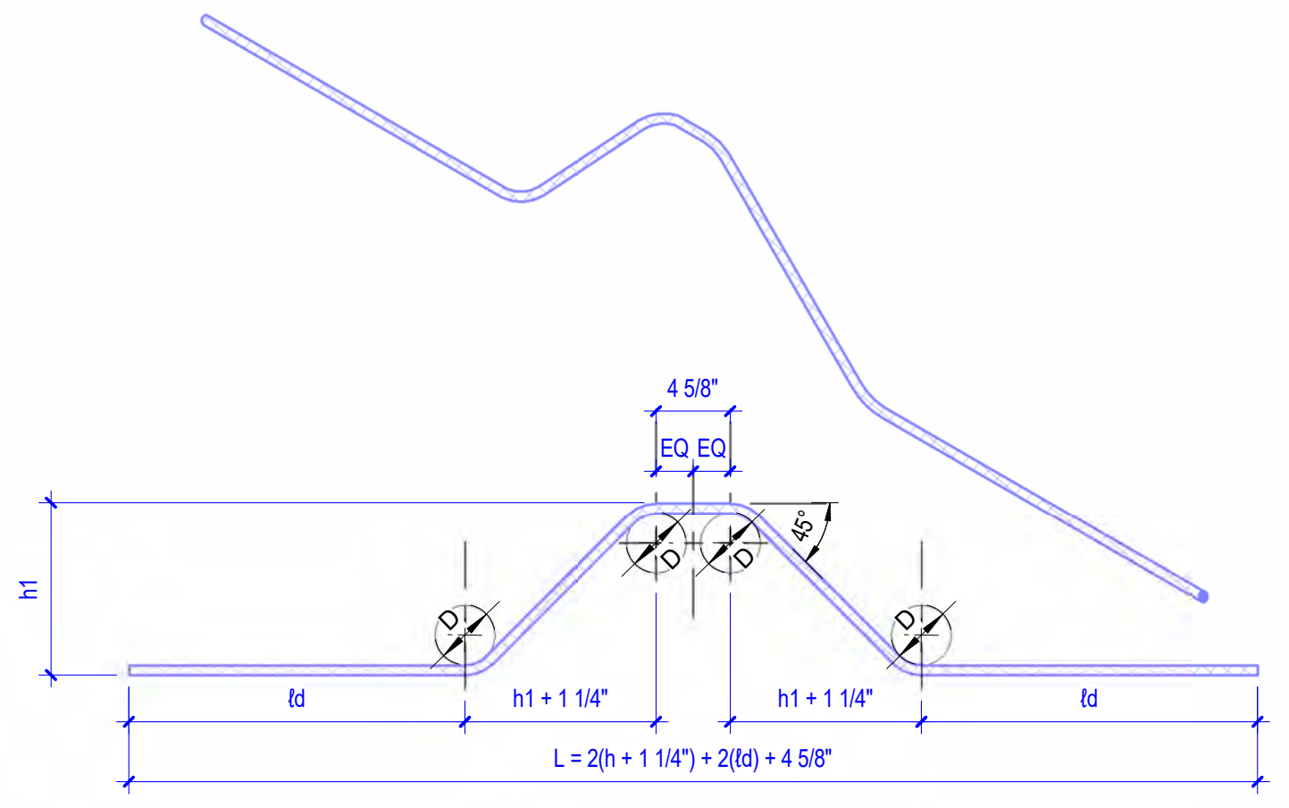

SIMPSON SHALLOW ANCHOR ROD (SAR)
SIMPSON SAR PREATTACHED PLATE WASHER
SIMPSON ANCHOR BOLT LOCATOR (ABL)

ANCHOR BOLT KIT

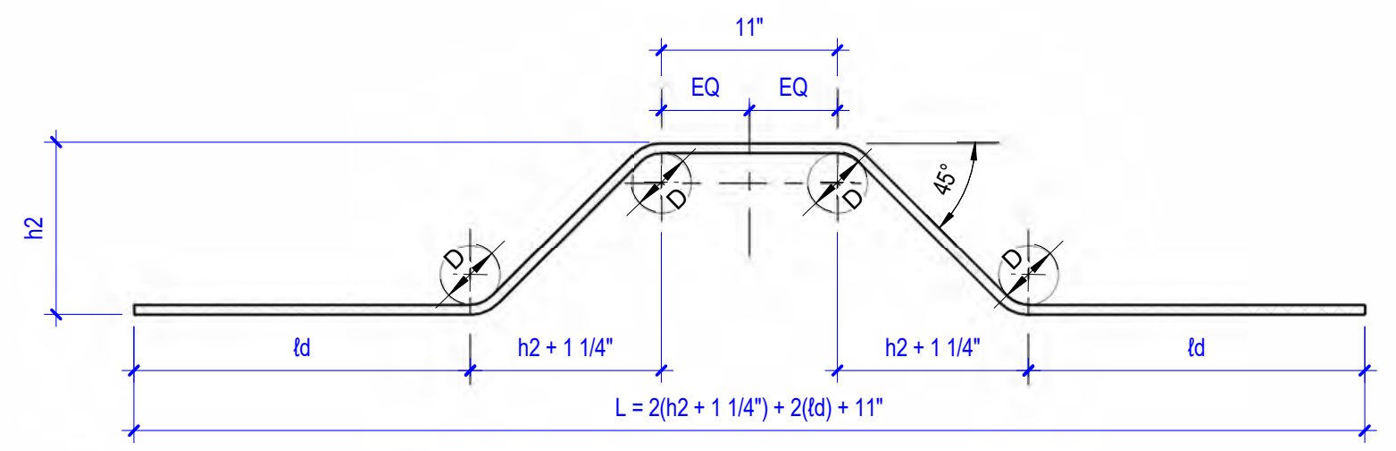
ANCHOR BOLT KIT	
ROD	ANCHOR KIT
ATR1/2	SA1-4-xxKT (ABL4-1 / SAR-4-xx)
ATR5/8	SA1-5-xxKT (ABL5-1 / SAR-5-xx)
ATR3/4	SA1-6-xxKT (ABL6-1 / SAR-6-xx)
ATR7/8	SA1-7-xxKT (ABL7-1 / SAR-7-xx)
ATR1	SA1-8-xxKT (ABL8-1 / SAR-8-xx)
ATR1-1/8	SA1-9-xxKT (ABL9-1 / SAR-9-xx)
ATR1-1/4	SA1-10-xxKT (ABL10-1 / SAR-10-xx)

② Anchor Rod Anchor Bolt Locator

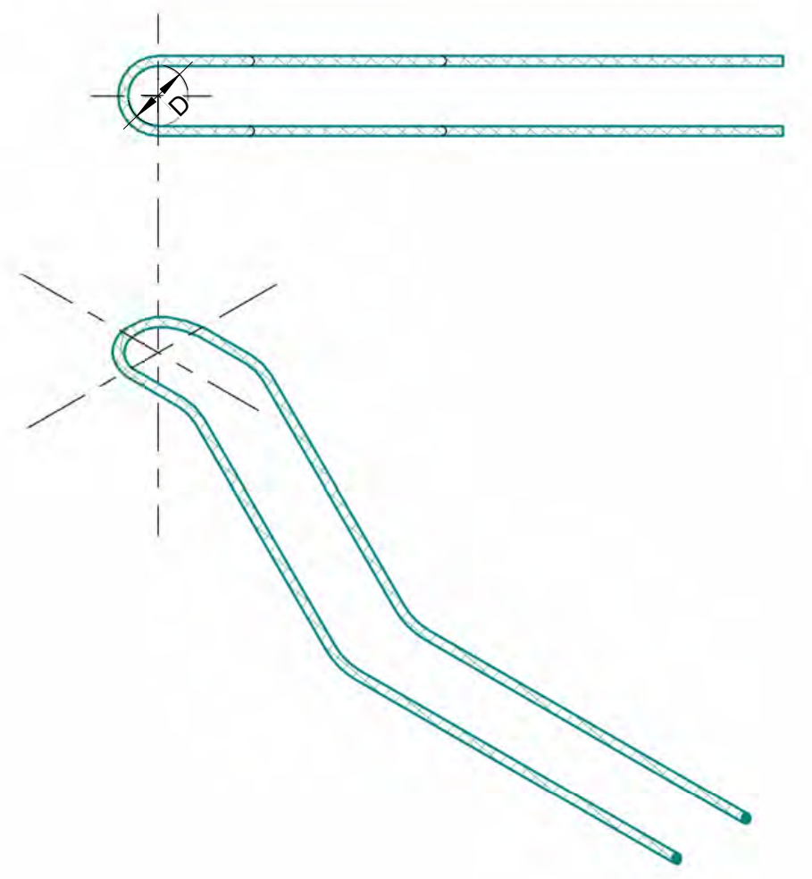
1 1/2" = 1'-0"



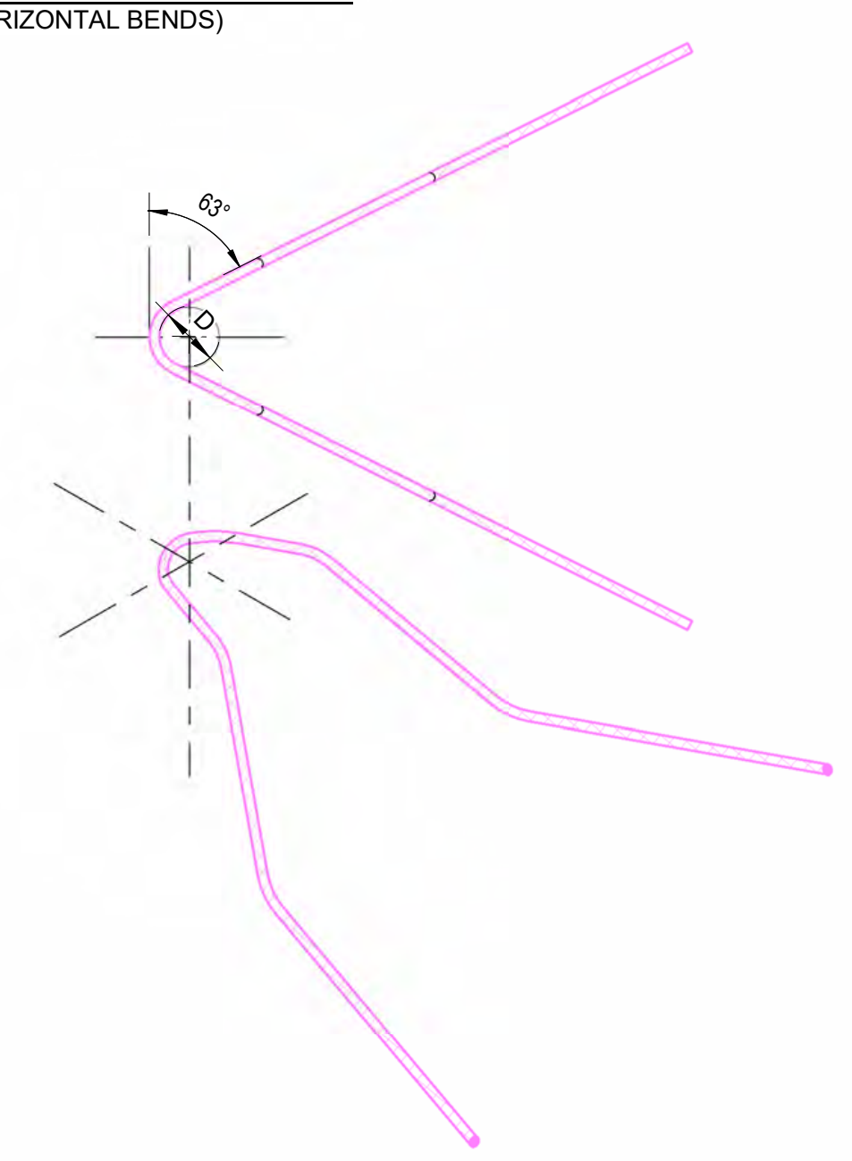
REINFORCEMENT TYPE A (#5 BAR)



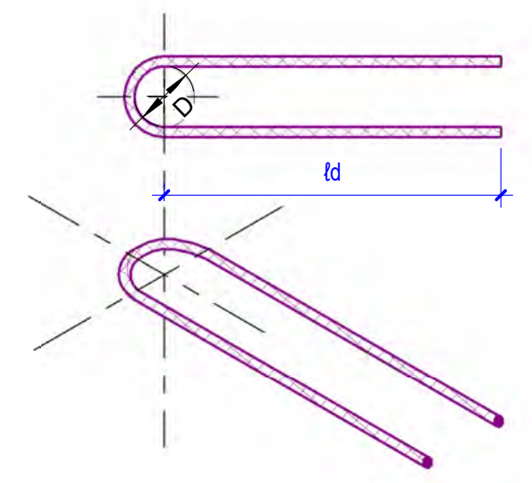
VERTICAL BENDS FOR REINFORCEMENT TYPE B & C
(PRIOR TO HORIZONTAL BENDS)



REINFORCEMENT TYPE B (#5 BAR)



REINFORCEMENT TYPE C (#5 BAR)

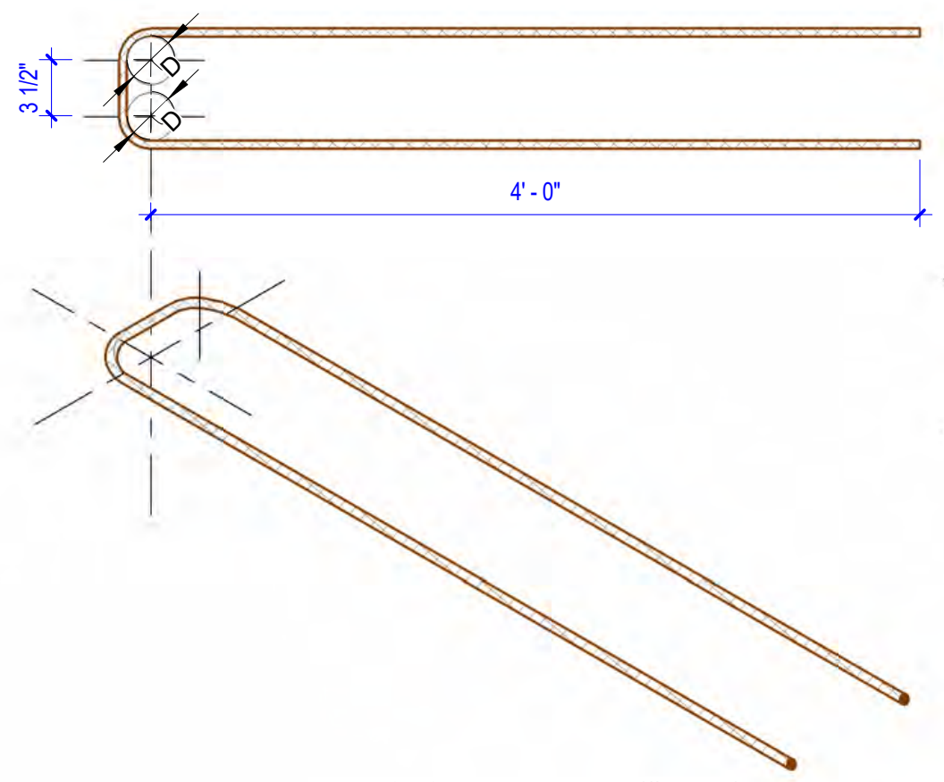


REINFORCEMENT TYPE D (#4 BAR)

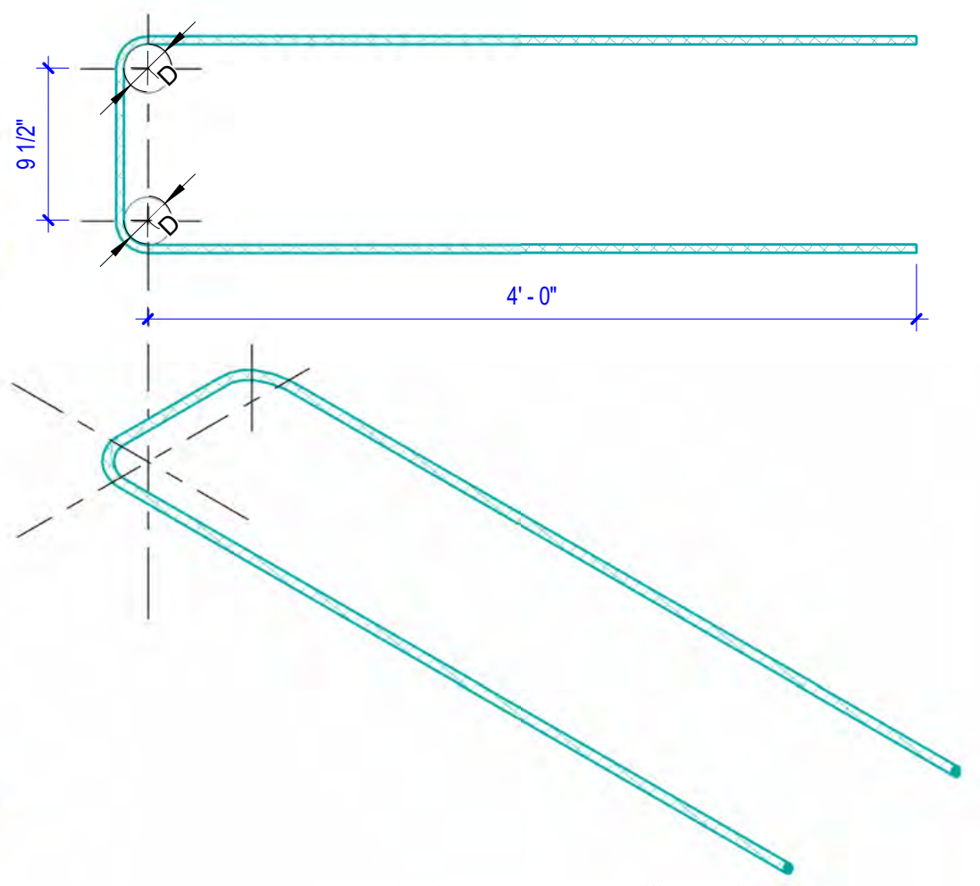
DEVELOPMENT LENGTH (ℓd)		
BAR SIZE	CONCRETE STRENGTH (f'c = PSI)	
	4,000	5,000
#4	21"	19"
#5	19"	

DEVELOPEMENT LENGTH AND VARIABLE DEFINITION FOR REINFORCEMENT BAR BENDS

t = SLAB THICKNESS
D = 6 d_b
h1 = t - (2 1/4" + BOTT COVER)
h1 (AT EDGE) = t - (1 1/2" + BOTT COVER)
h2 = t - (3" + BOTT COVER)



REINFORCEMENT TYPE E (#4 BAR)



REINFORCEMENT TYPE F (#4 BAR)

① EMBED ROD ANCHORAGE REINFORCEMENT

1" = 1'-0"

SHEAR WALL END ANCHOR ROD ANCHORAGE SCHEDULE (MINIMUM CONCRETE COMPRESSIVE STRENGTH = 5000 PSI)

ANCHOR ROD	LOCATION	EMBEDDED ROD ANCHORAGE			EMBED PLATE ANCHORAGE				LOCATION	ANCHOR ROD
		SIMPSON SHALLOW ANCHOR KIT / AB 2 / S5.414A	ANCHORAGE TYPE / DETAIL	ADDITIONAL SLAB REINFORCEMENT	SIMPSON BLOCK CONNECTOR	EMBED PLATE	ANCHORAGE DETAIL	ADDITIONAL SLAB REINFORCEMENT		
ATR3/8	INTERIOR	AB	TYPE 1 - 1 / S5.415	NA	NA	NA	NA	NA	INTERIOR	ATR3/8
	SLAB EDGE	AB	TYPE 1 - 1 / S5.415	NA					SLAB EDGE	
	NEAR EDGE	AB	TYPE 1 - 1 / S5.415	NA					NEAR EDGE	
	CORNER	NA	NA	NA	SBC6H	AP-B1.3-3	2 / S5.416	NA	CORNER	
ATR1/2	INTERIOR	SA1-4-xxKT / AB	TYPE 1 - 1 / S5.415	NA	NA	NA	NA	NA	INTERIOR	ATR1/2
	SLAB EDGE	SA1-4-xxKT/ AB	TYPE 1 - 1 / S5.415	NA					SLAB EDGE	
	NEAR EDGE	SA1-4-xxKT / AB	TYPE 1 - 1 / S5.415	NA					NEAR EDGE	
	CORNER	NA	NA	NA	SBC6H (W/ REDUCING CPLR)	AP-B1.3-3	2 / S5.416		CORNER	
ATR5/8	INTERIOR	SA1-5-xxKT	TYPE 1 - 1 / S5.415	NA	NA	NA	NA	NA	INTERIOR	ATR5/8
	SLAB EDGE	SA1-5-xxKT	TYPE 1 - 1 / S5.415	NA					SLAB EDGE	
	NEAR EDGE	SA1-5-xxKT	TYPE 1 - 1 / S5.415	NA					NEAR EDGE	
	CORNER	NA	NA	NA	SBC6H (W/ REDUCING CPLR)	AP-B1.3-3	2 / S5.416	NA	CORNER	
ATR3/4	INTERIOR	SA1-6-xxKT	TYPE 1 - 1 / S5.415	NA	SBC6H	AP-A2.6-8.1	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	INTERIOR	ATR3/4
	SLAB EDGE	SA1-6-xxKT	TYPE 2 - 2 / S5.415	(4) #5X8'-0" EW (T)		AP-S2.6-8.1	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	NEAR EDGE	SA1-6-xxKT	TYPE 1 - 1 / S5.415	NA		AP-S2.6-8.2	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	CORNER	NA	NA	NA		AP-B1.512-512	2 / S5.416	(2) #5X6'-6" EW @ 8"O.C. (T)	CORNER	
ATR7/8	INTERIOR	SA1-7-xxKT	TYPE 1 - 1 / S5.415	NA	SBC8H (W/ REDUCING CPLR)	AP-A2.6-8.2	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	INTERIOR	ATR7/8
	SLAB EDGE	SA1-7-xxKT	TYPE 2 - 2 / S5.415	(4) #5X8'-0" EW (T)		AP-S2.6-8.3	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	NEAR EDGE	SA1-7-xxKT	TYPE 1 - 1 / S5.415	NA		AP-S2.6-8.4	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	CORNER	NA	NA	NA		AP-B1.8-8	2 / S5.416	(2) #5X6'-6" EW @ 8"O.C. (T)	CORNER	
ATR1	INTERIOR	SA1-8-xxKT	TYPE 1 - 1 / S5.415	NA	SBC8H	AP-A2.6-8.2	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	INTERIOR	ATR1
	SLAB EDGE	SA1-8-xxKT	TYPE 2 - 2 / S5.415	(4) #5X8'-0" EW (T)		AP-S2.6-8.3	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	NEAR EDGE	SA1-8-xxKT	TYPE 1 - 1 / S5.415	NA		AP-S2.6-8.4	1 / S5.416	(2) #4X10'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	CORNER	NA	NA	NA		AP-B1.8-8	2 / S5.416	(2) #5X6'-6" EW @ 8"O.C. (T)	CORNER	
ATR1-1/8	INTERIOR	SA1-9-xxKT	TYPE 2 - 2 / S5.415	(4) #5X12'-0" EW (T)	SBC10H (W/ REDUCING CPLR)	AP-A4.8-8.1	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	INTERIOR	ATR1-1/8
	SLAB EDGE	SA1-9-xxKT	TYPE 2 - 2 / S5.415	(4) #5X8'-0" PAR (T) (4) #5X9'-6" PERP (T)		AP-S4.8-10.1	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	NEAR EDGE	SA1-9-xxKT	TYPE 2 - 2 / S5.415	(4) #5X8'-0" PAR (T) (4) #5X9'-6" PERP (T)		AP-S4.8-8.1	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	CORNER	NA	NA	NA		AP-B1.15-15	2 / S5.416	(3) #5X6'-6" EW @ 4"O.C. (T)	CORNER	
ATR1-1/4	INTERIOR	SA1-10-xxKT	TYPE 2 - 2 / S5.415	(8) #5X12'-0" EW (T)	SBC10H	AP-A4.8-8.1	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	INTERIOR	ATR1-1/4
	SLAB EDGE	NA	NA			AP-S4.8-10.1	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	NEAR EDGE	SA1-10-xxKT	TYPE 2 - 2 / S5.415	(4) #5X8'-0" PAR (T) (4) #5X9'-6" PERP (T)		AP-S4.8-8.1	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	CORNER	NA	NA	NA		AP-B1.15-15	2 / S5.416	(3) #5X6'-6" EW @ 4"O.C. (T)	CORNER	
ATS-HSR10	INTERIOR	NA	NA	NA	SBC10H	AP-A6.8-12.1	1 / S5.416	(4) #5X11'-10" EW @ 4"O.C. (T)	INTERIOR	ATS-HSR10
	SLAB EDGE					AP-S6.10-12.1	1 / S5.416	(4) #5X11'-10" EW @ 4"O.C. (T)	SLAB EDGE	
	NEAR EDGE					AP-S6.8-12.1	1 / S5.416	(4) #5X11'-10" EW @ 4"O.C. (T)	NEAR EDGE	
	CORNER					AP-B2.18-18	2 / S5.416	(6) #5X6'-6" EW @ 4"O.C. (T)	CORNER	
ATR1-3/8	INTERIOR	NA	NA	NA	SBC11H	AP-A4.8-8.2	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	INTERIOR	ATR1-3/8
	SLAB EDGE					AP-S4.8-10.1	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	SLAB EDGE	
	NEAR EDGE					AP-S8-8.2	1 / S5.416	(2) #5X11'-6" EW @ 8"O.C. (T)	NEAR EDGE	
	CORNER					AP-B1.18-18	2 / S5.416	(3) #5X6'-6" EW @ 4"O.C. (T)	CORNER	
ATR1-1/2	INTERIOR	NA	NA	NA	SBC12H	AP-A4.8-8.2	1 / S5.416	(3) #5X11'-6" EW @ 4"O.C. (T)	INTERIOR	ATR1-1/2
	SLAB EDGE					AP-S4.8-10.1	1 / S5.416	(3) #5X11'-6" EW @ 4"O.C. (T)	SLAB EDGE	
	NEAR EDGE					AP-S4.8-8.2	1 / S5.416	(3) #5X11'-6" EW @ 4"O.C. (T)	NEAR EDGE	
	CORNER					AP-B1.22-22	3 / S5.416	(3) #6X8'-0" EW @ 4"O.C. (T)	CORNER	
ATS-HSR12	INTERIOR	NA	NA	NA	SBC12H	AP-A8.12-12.1	1 / S5.416	(6) #5X12'-0" EW @ 4"O.C. (T)	INTERIOR	ATS-HSR12
	SLAB EDGE					AP-S8.10-16.1	1 / S5.416	(6) #5X12'-0" EW @ 4"O.C. (T)	SLAB EDGE	
	NEAR EDGE					AP-S8.8-16.1	1 / S5.416	(6) #5X12'-0" EW @ 4"O.C. (T)	NEAR EDGE	
	CORNER					AP-B2.27-27	3 / S5.416	(3) #6X8'-0" EW @ 4"O.C. (T)	CORNER	

NOTE:
1. SEE SHEAR WALL BRACING PLAN FOR SHEAR WALL END ANCHORAGE ROD
2. SEE ANCHOR ROD / ANCHOR DETAILS FOR ANCHOR REINFORCEMENT
3. WHEN BOTH EMBEDDED ROD ANCHORAGE AND EMBED PLATE ANCHORAGE OPTIONS ARE INDICATED, EITHER ANCHORAGE MAY BE USED AT CONTRACTORS OPTION
4. SEE ANCHORAGE DETAILS TO DETERMINE ANCHOR LOCATION FOR SLAB FACE, NEAR EDGE, AND INTERIOR / AWAY FROM EDGE. ALL ANCHORS AT SLAB FACE OR NEAR EDGE WITHIN NEAR EDGE DISTANCE OF CORNER SHALL BE CONSIDERED CORNER
5. FOR ADDITIONAL ANCHORAGE INFORMATION AT SLAB STEPS, SEE DETAIL 1 / S5.417
6. FOR EMBED ROD ANCHORAGE REINFORCEMENT TYPES SEE 1 / S5.414A

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Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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- NO. 85617

SHEET TITLE:
Wood Shear Wall Anchorage ATR / Rod System - Uniform Thickness Podium Slab

SHEET NUMBER:
S5.414A

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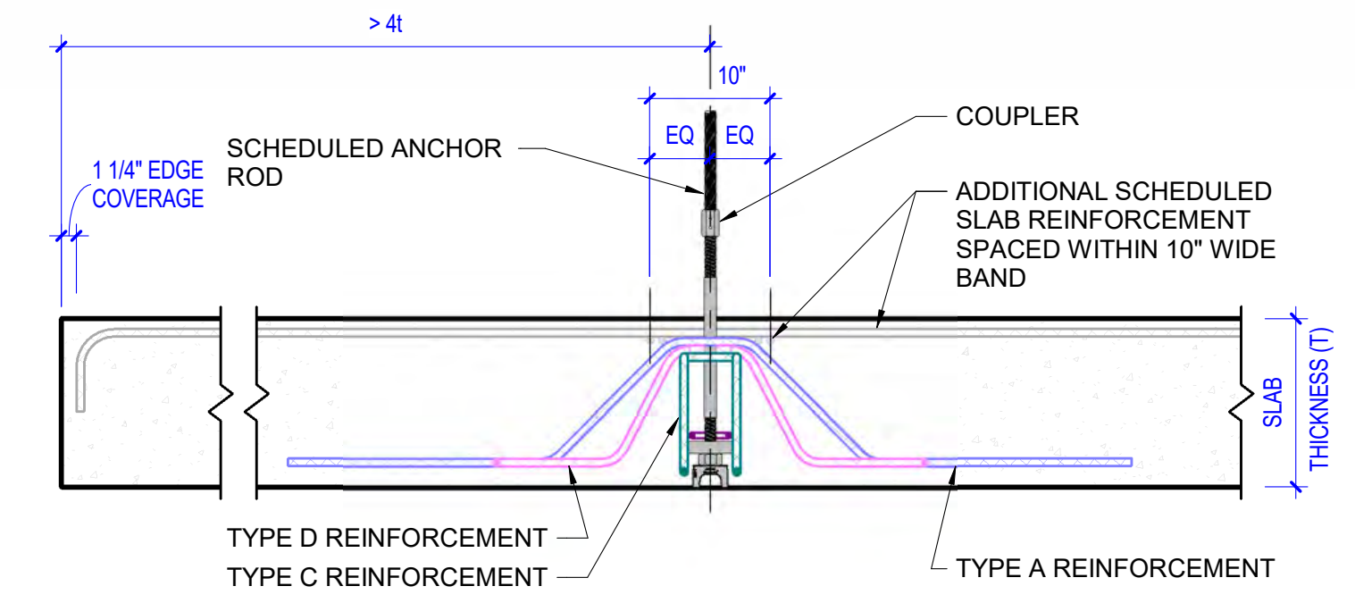
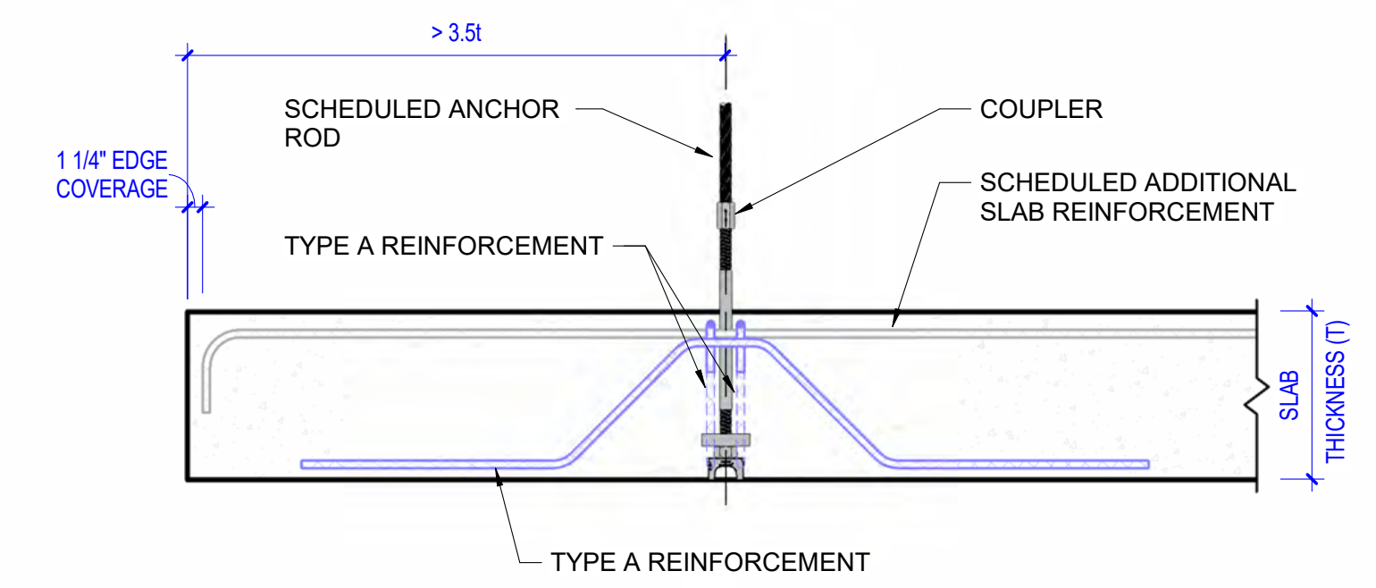
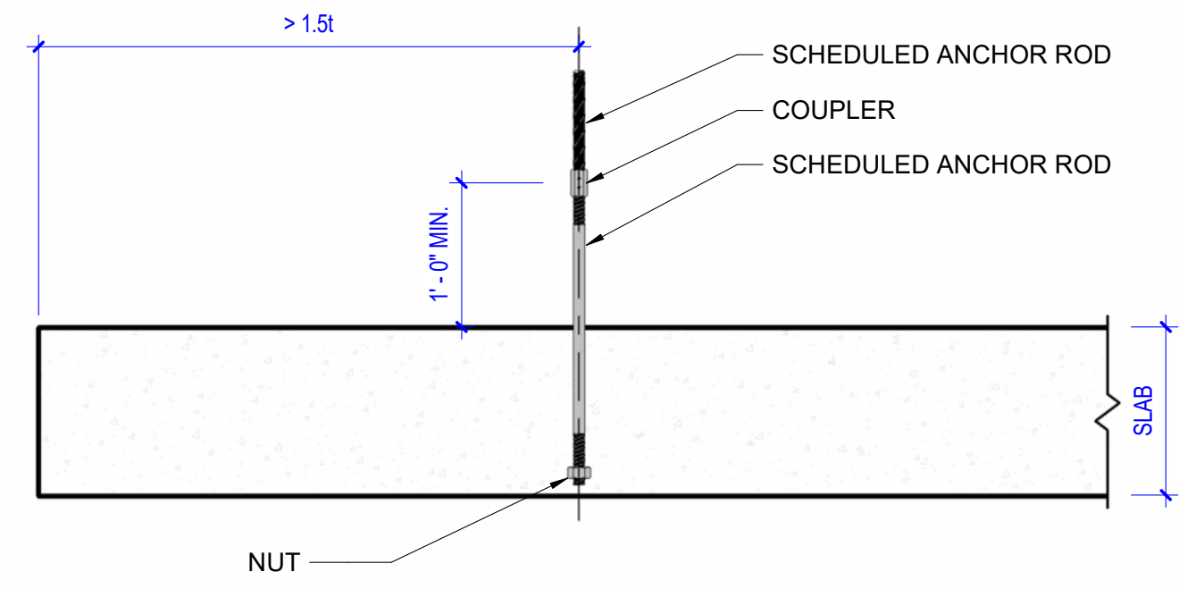


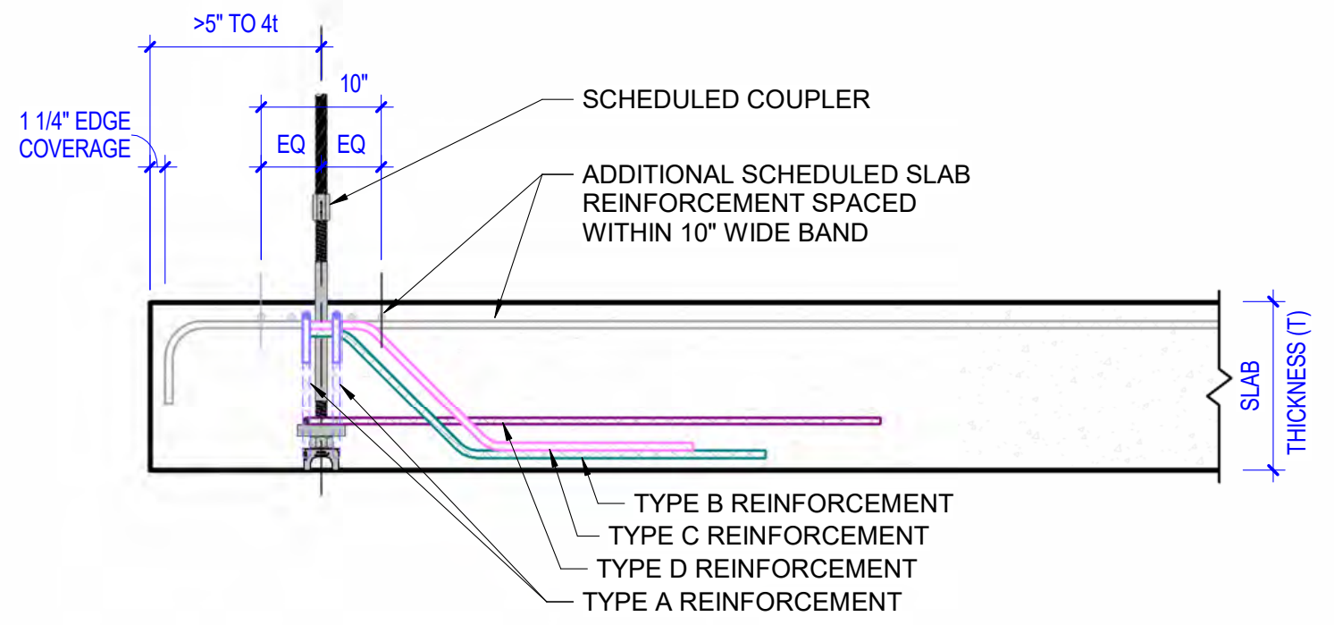
2D Type 2 - Anchor Slab Edge / Near Edge Cross Section
3/4" = 1'-0"



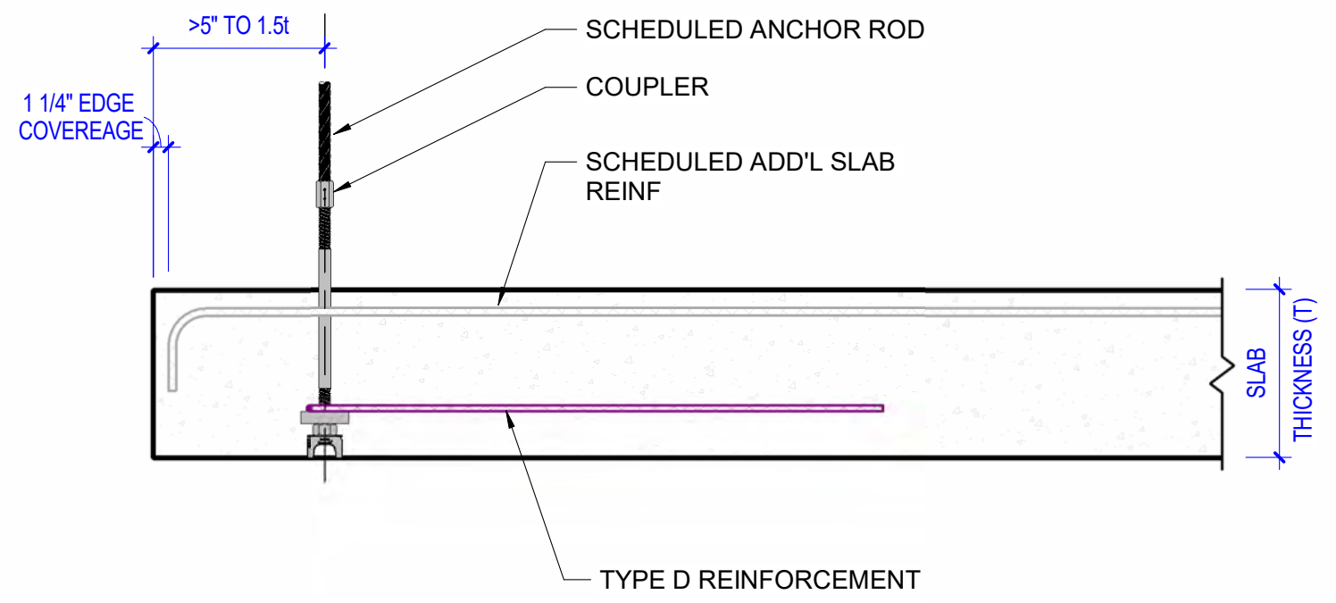
2C Type 2 - Anchor at Interior
3/4" = 1'-0"



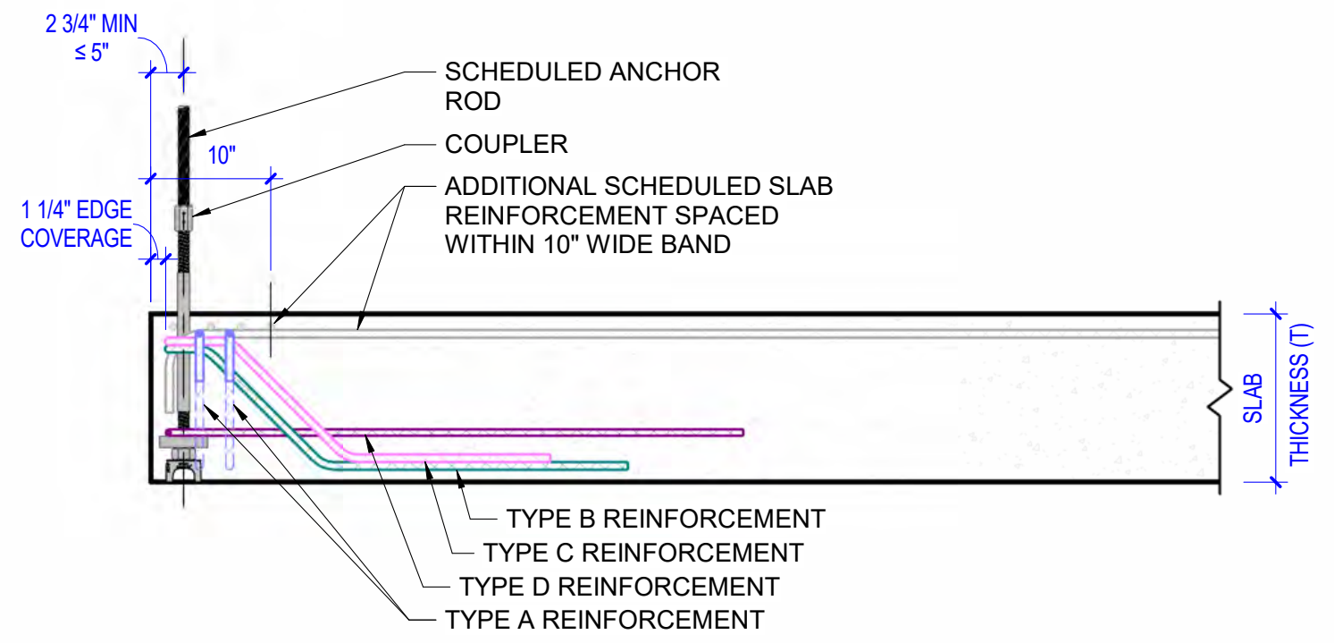
1C Type 1 - Anchor at Interior
3/4" = 1'-0"



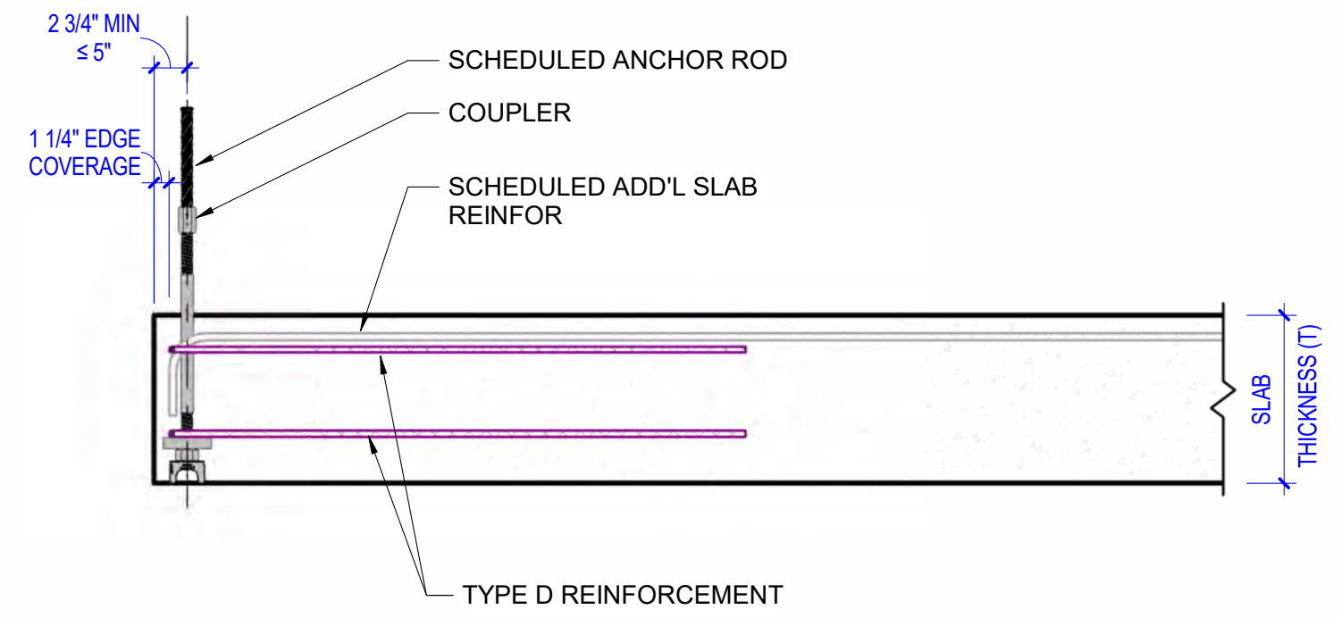
2B Type 2 - Anchor Near Edge
3/4" = 1'-0"



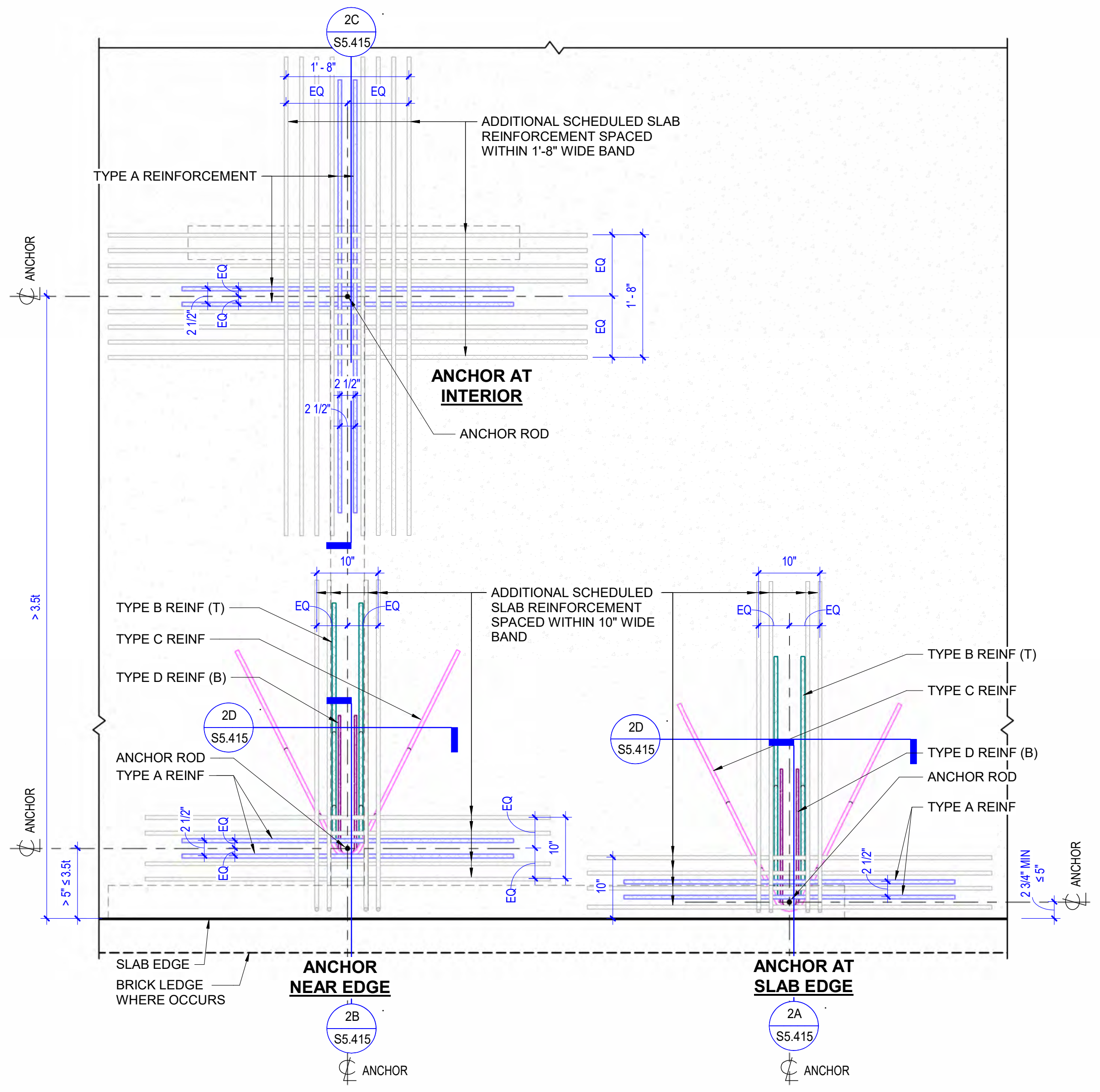
1B Type 1 - Anchor Near Edge
3/4" = 1'-0"



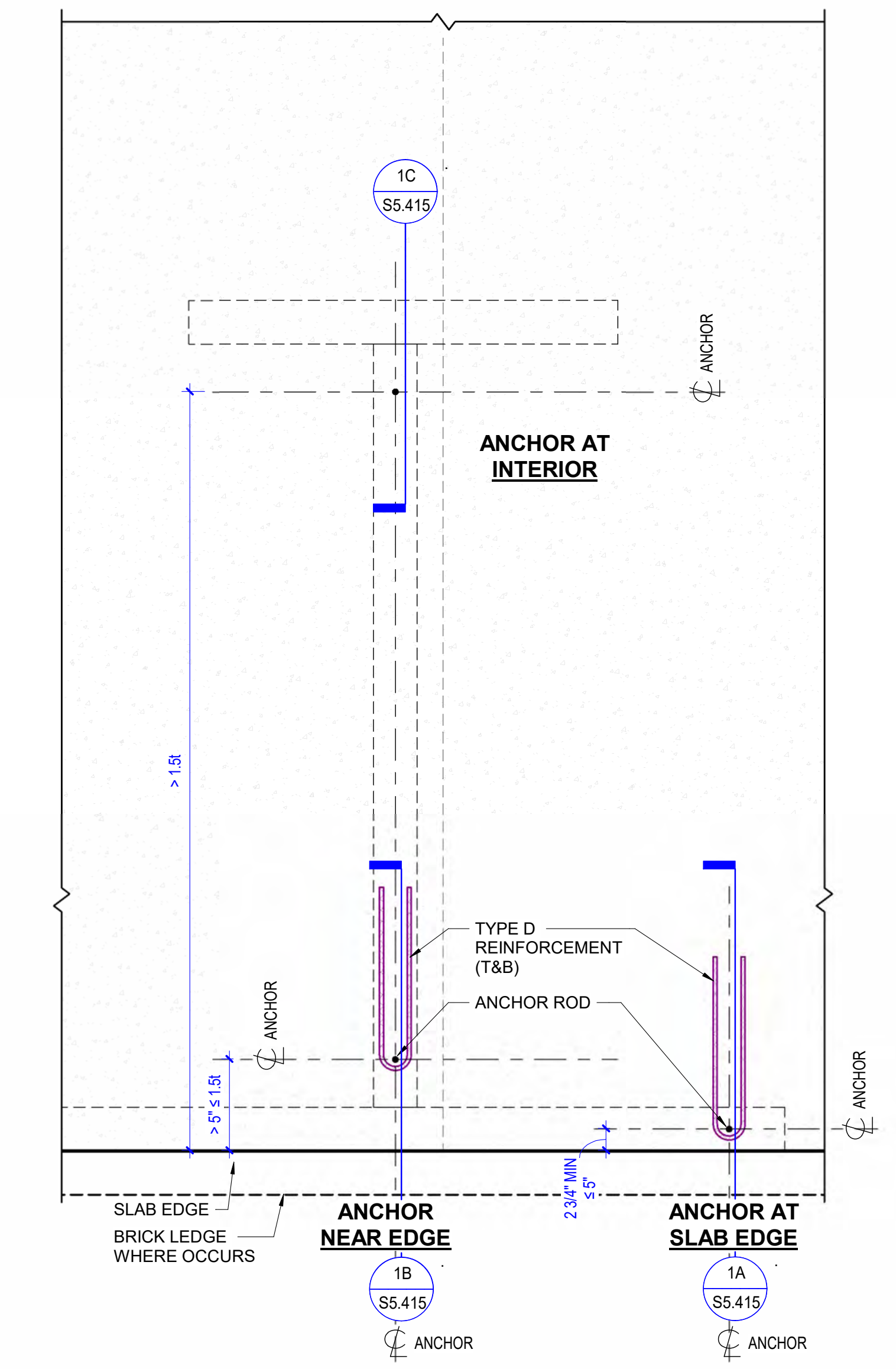
2A Type 2 - Anchor at Slab Edge
3/4" = 1'-0"



1A Type 1 - Anchor at Slab Edge
3/4" = 1'-0"



2 Anchor Rod Embedded Rod Type 2
3/4" = 1'-0"



1 Shear Wall Anchor Rod Embedded Rod Type 1
3/4" = 1'-0"

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Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
Embedded Rod Anchorage Option - Uniform Thickness Slab

SHEET NUMBER:
S5.415

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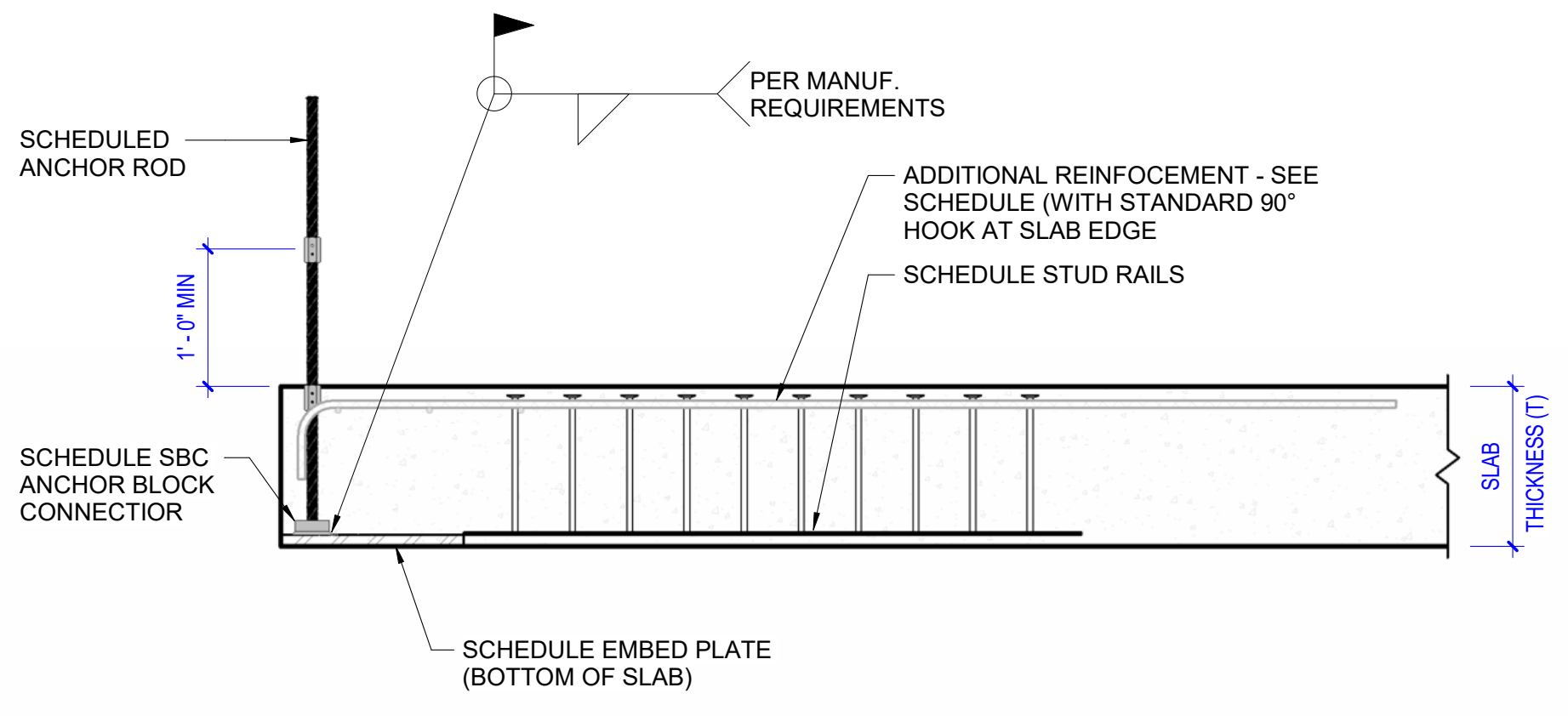
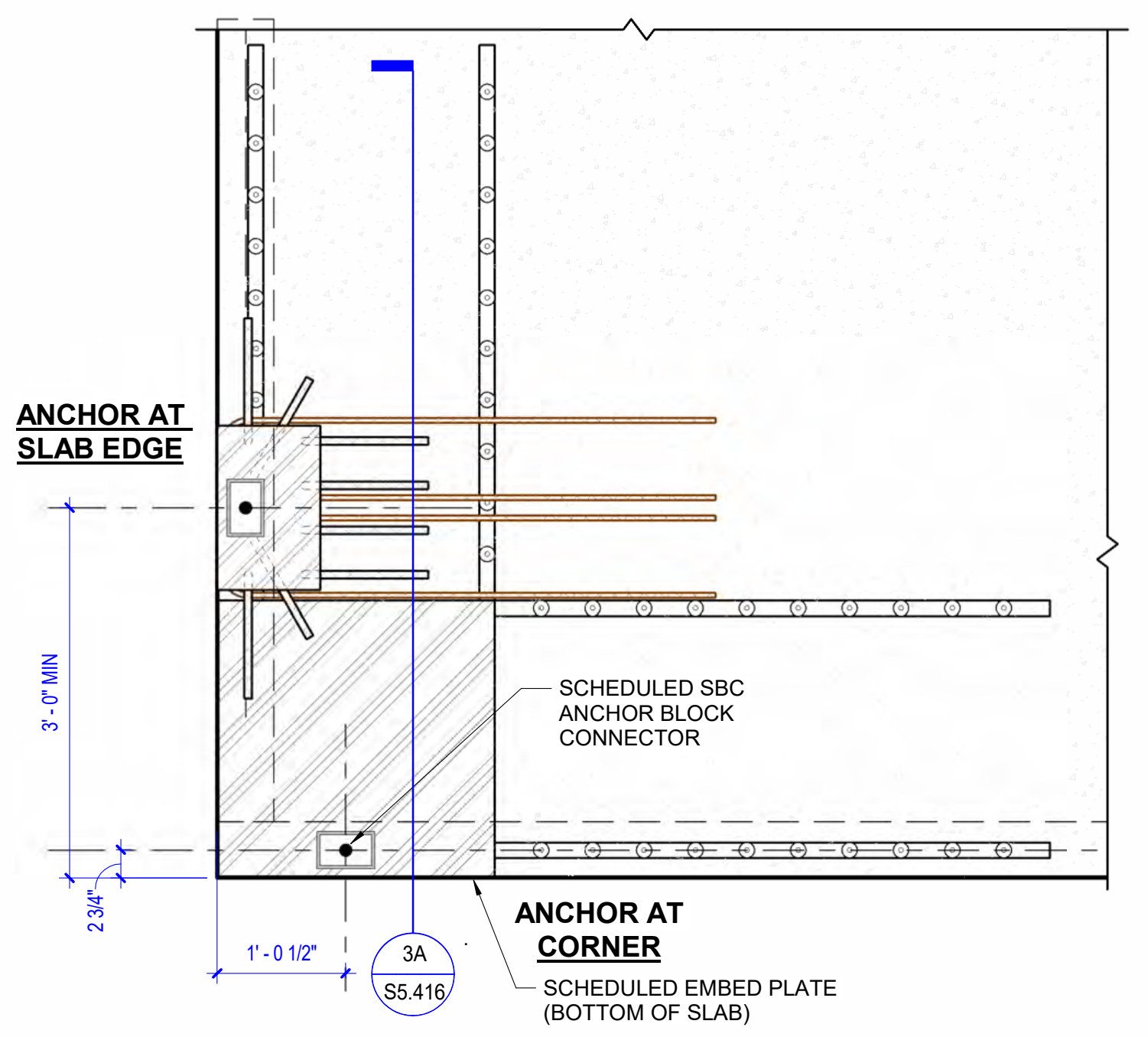


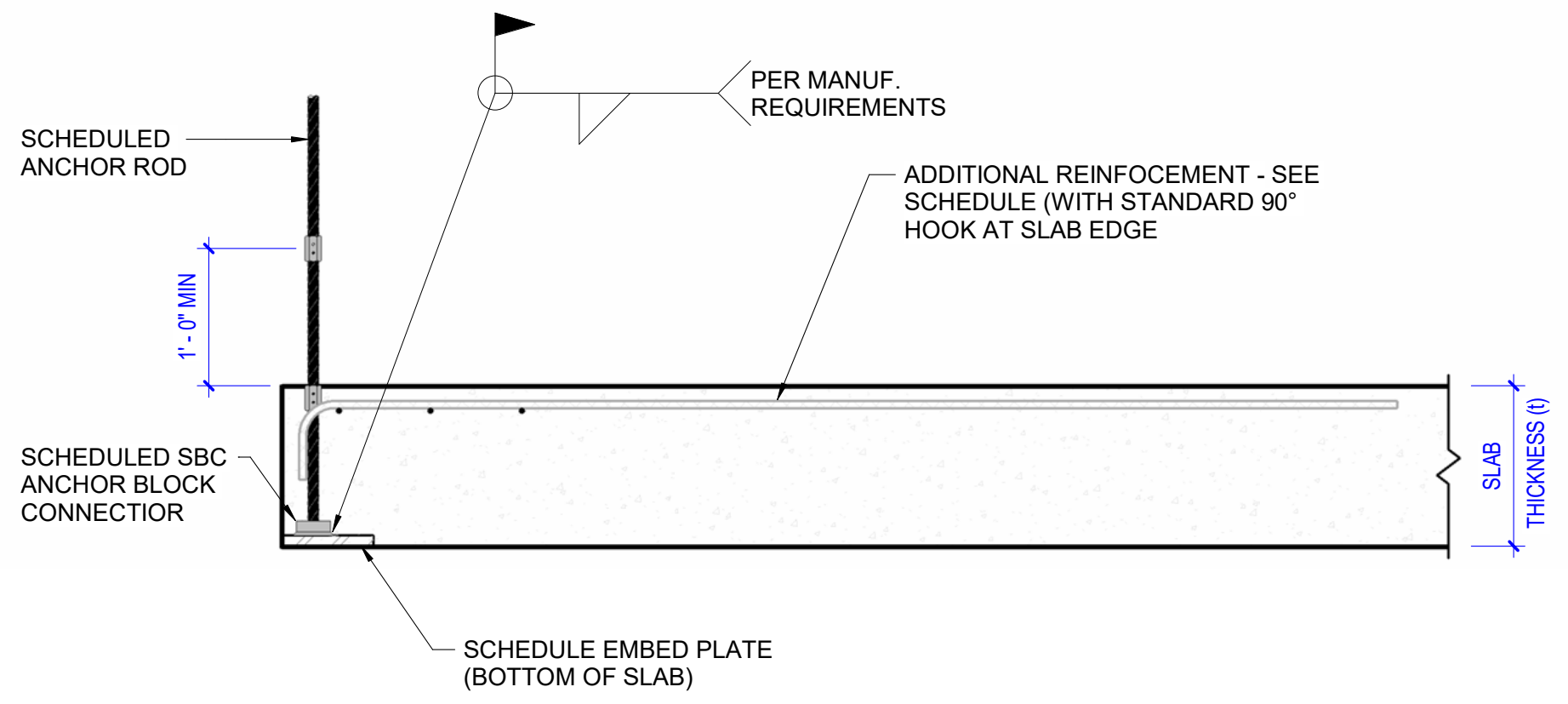
PER MANUF. REQUIREMENTS
SCHEDULED ANCHOR ROD
ADDITIONAL REINFOCEMENT - SEE SCHEDULE (WITH STANDARD 90° HOOK AT SLAB EDGE
SCHEDULE STUD RAILS
1'-0" MIN
SCHEDULE SBC ANCHOR BLOCK CONNECTIOR
SLAB THICKNESS (T)
SCHEDULE EMBED PLATE (BOTTOM OF SLAB)

3A Shear Wall Embed Plate Anchorage - Corner
3/4" = 1'-0"



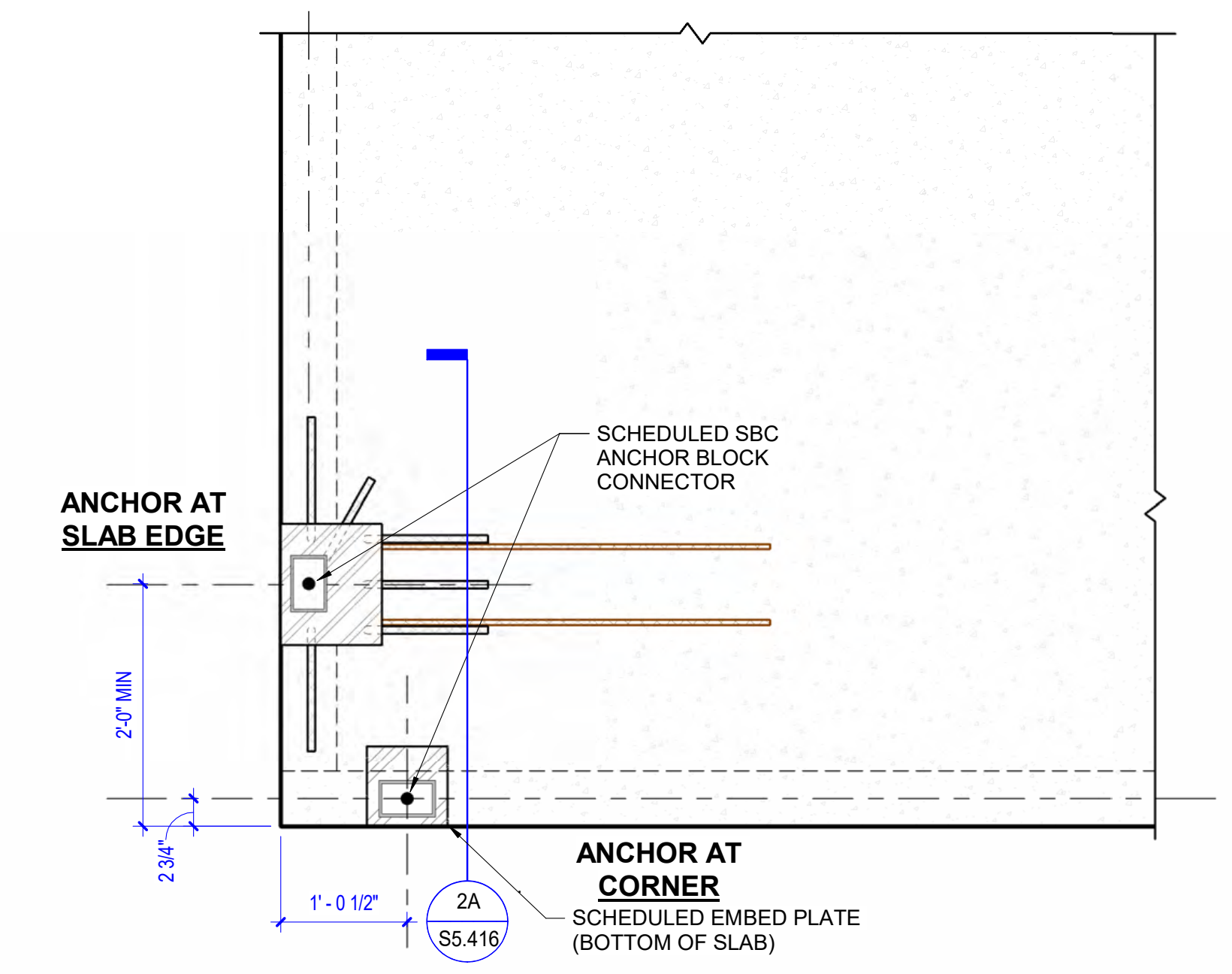
ANCHOR AT SLAB EDGE
3'-0" MIN
2 3/4"
1'-0 1/2"
3A
S5.416
SCHEDULED SBC ANCHOR BLOCK CONNECTOR
ANCHOR AT CORNER
SCHEDULED EMBED PLATE (BOTTOM OF SLAB)

3 Embed Plate Anchorage - Corner
3/4" = 1'-0"



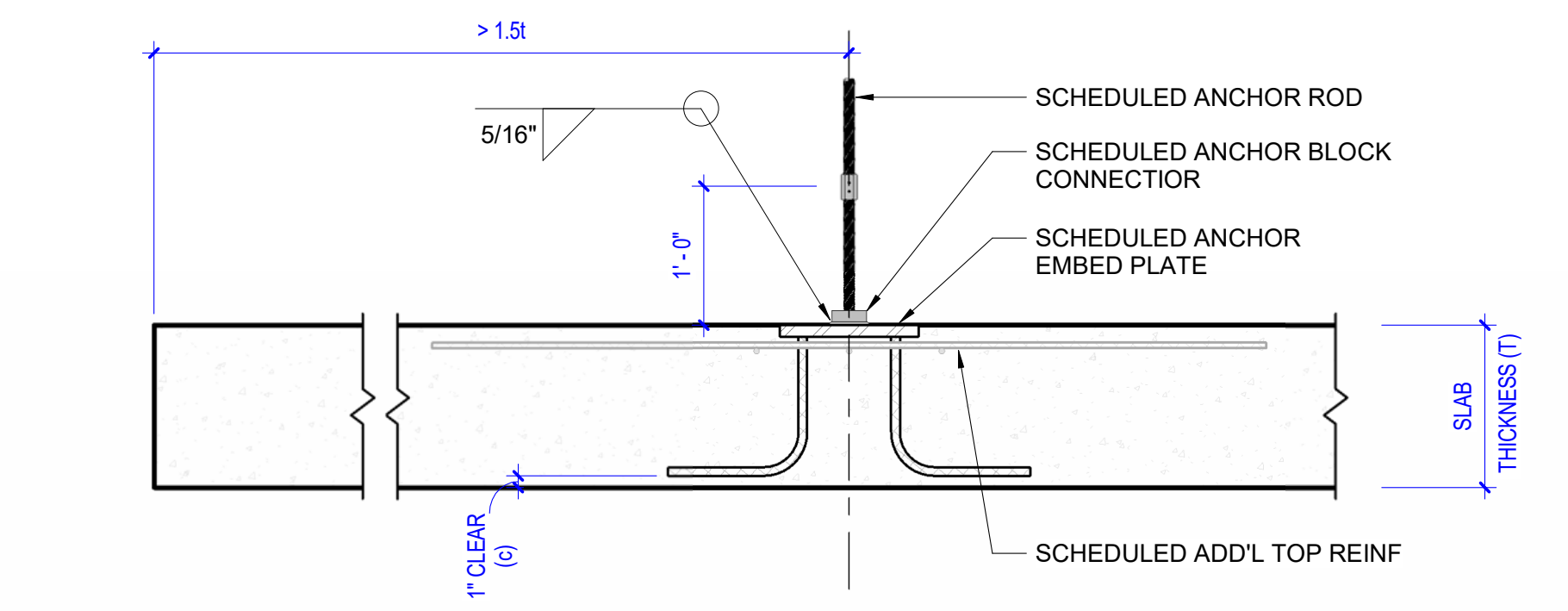
PER MANUF. REQUIREMENTS
SCHEDULED ANCHOR ROD
ADDITIONAL REINFOCEMENT - SEE SCHEDULE (WITH STANDARD 90° HOOK AT SLAB EDGE
1'-0" MIN
SCHEDULED SBC ANCHOR BLOCK CONNECTIOR
SLAB THICKNESS (t)
SCHEDULE EMBED PLATE (BOTTOM OF SLAB)

2A Embed Plate Anchorage - Corner
3/4" = 1'-0"



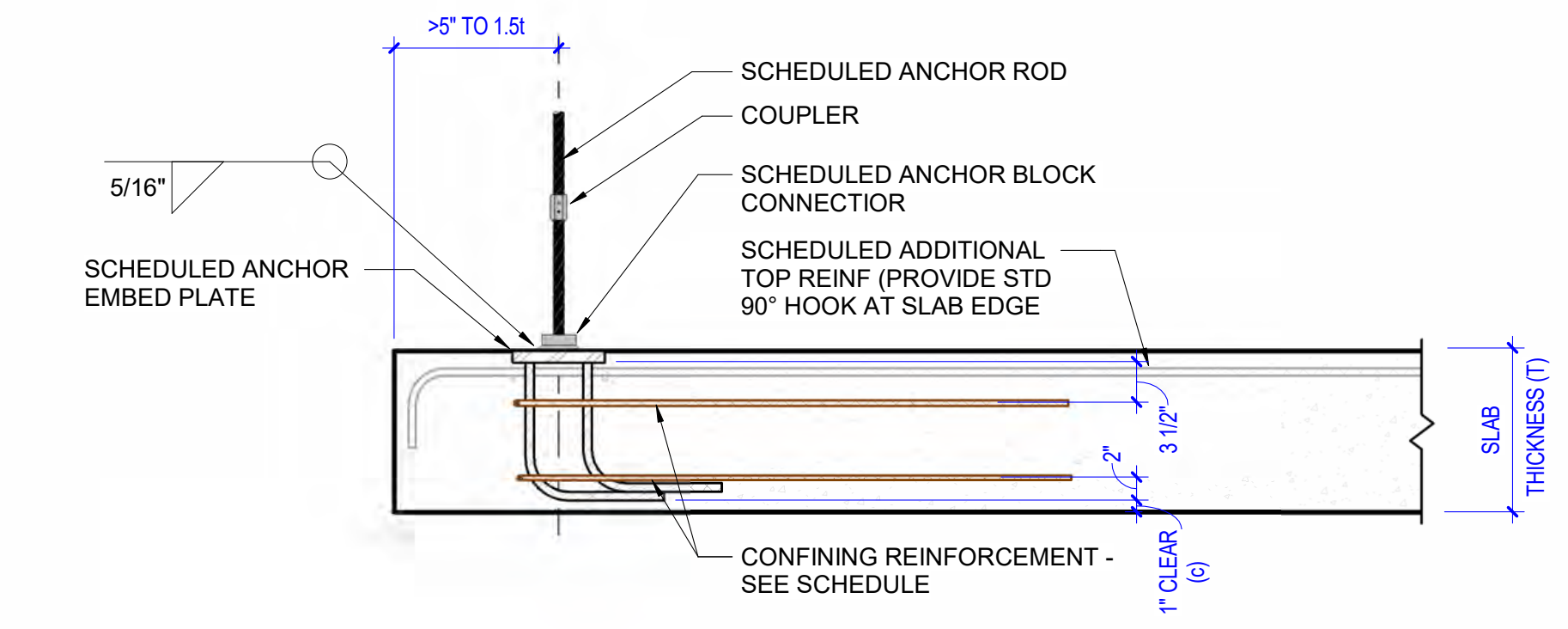
SCHEDULED SBC ANCHOR BLOCK CONNECTOR
ANCHOR AT SLAB EDGE
2'-0" MIN
2 3/4"
1'-0 1/2"
2A
S5.416
ANCHOR AT CORNER
SCHEDULED EMBED PLATE (BOTTOM OF SLAB)

2 Shear Wall Anchor Rod Corner Plate Anchorage
3/4" = 1'-0"



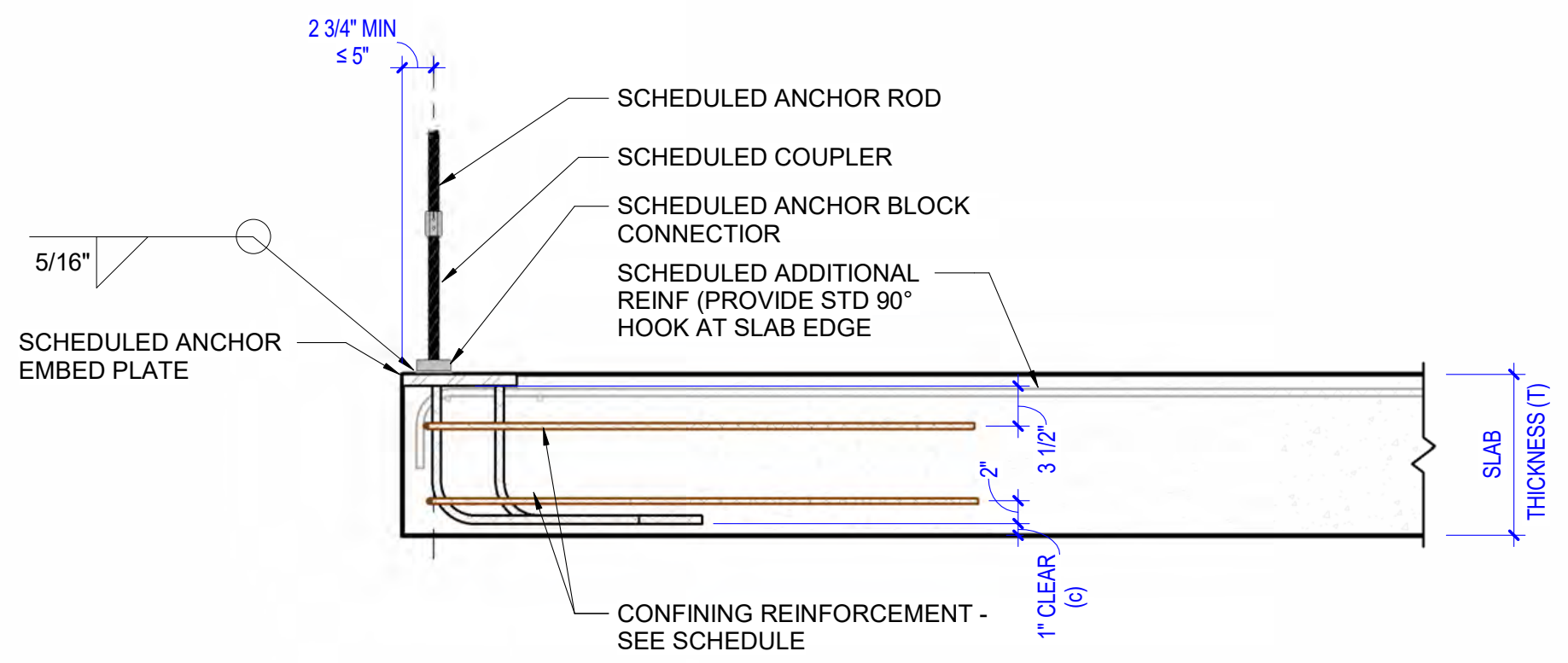
> 1.5t
5/16"
1'-0"
SCHEDULED ANCHOR ROD
SCHEDULED ANCHOR BLOCK CONNECTIOR
SCHEDULED ANCHOR EMBED PLATE
SLAB THICKNESS (T)
1" CLEAR (c)
SCHEDULED ADD'L TOP REINF

1C Shear Wall Embed Plate Anchorage - Interior
3/4" = 1'-0"



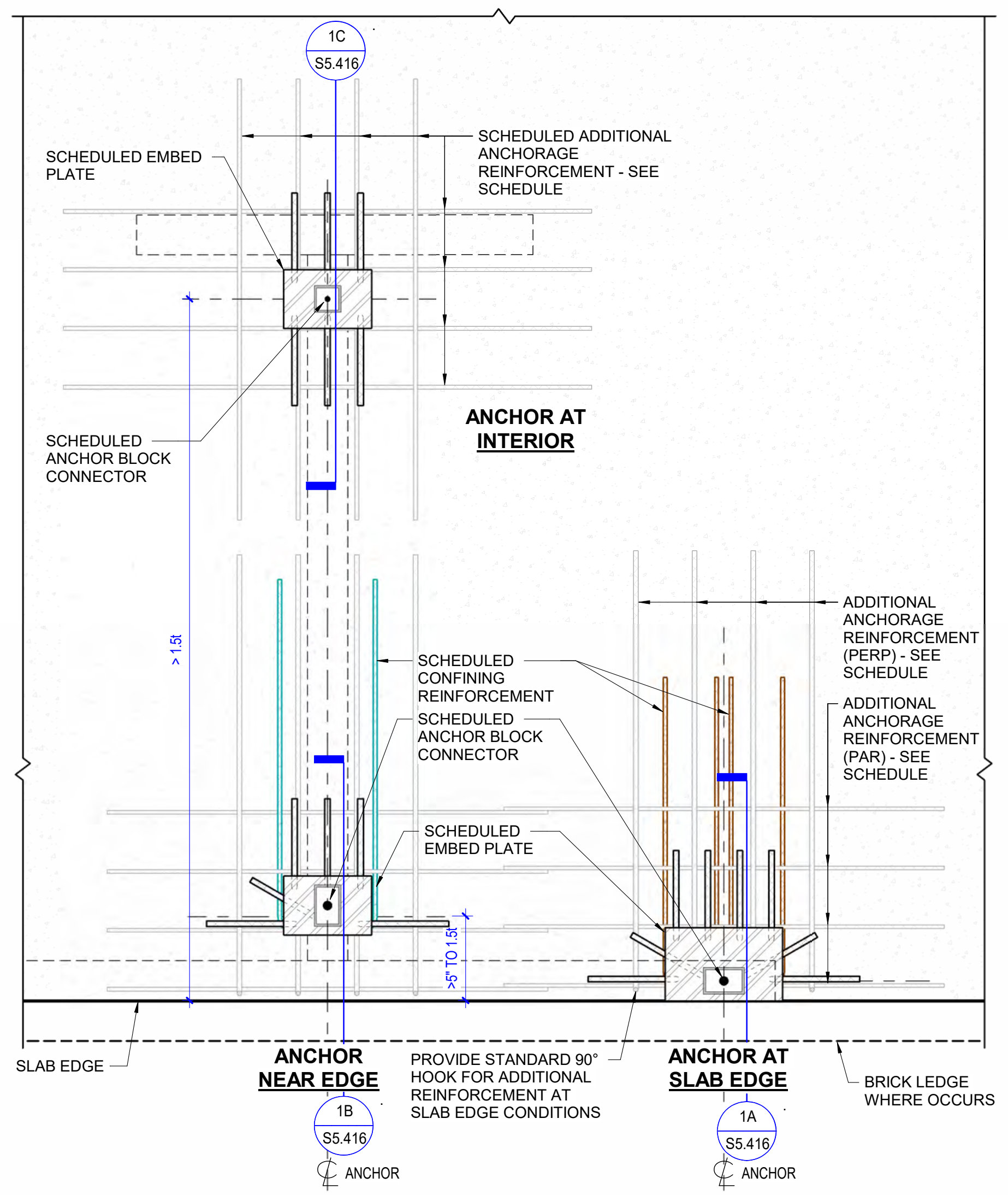
>5" TO 1.5t
5/16"
SCHEDULED ANCHOR ROD
COUPLER
SCHEDULED ANCHOR BLOCK CONNECTIOR
SCHEDULED ANCHOR EMBED PLATE
SCHEDULED ADDITIONAL TOP REINF (PROVIDE STD 90° HOOK AT SLAB EDGE
2"
3 1/2"
SLAB THICKNESS (T)
CONFINING REINFORCEMENT - SEE SCHEDULE
1" CLEAR (c)

1B Shear Wall Embed Plate Anchorage - Near Edge
3/4" = 1'-0"



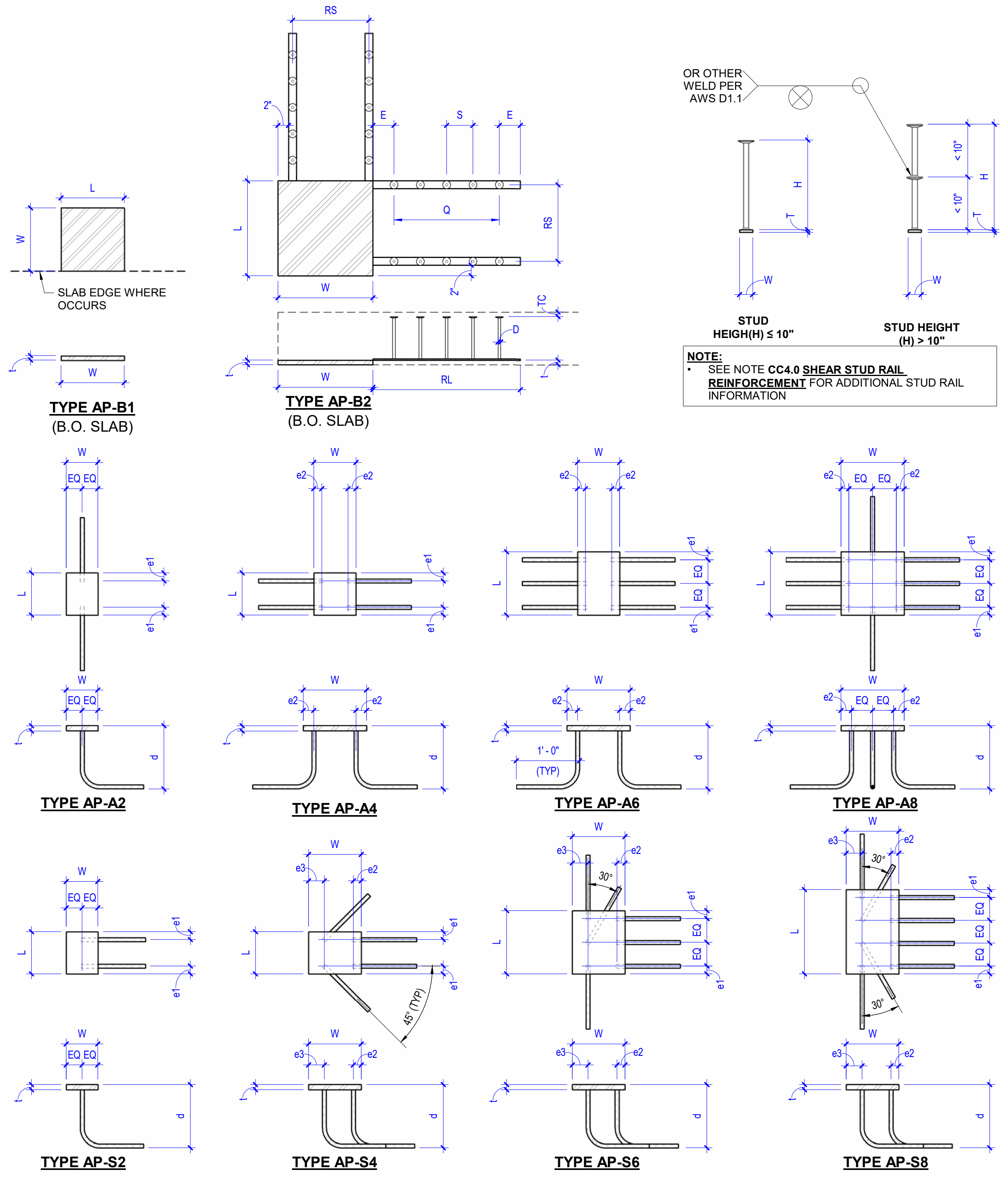
2 3/4" MIN
≤ 5"
5/16"
SCHEDULED ANCHOR ROD
SCHEDULED COUPLER
SCHEDULED ANCHOR BLOCK CONNECTION
SCHEDULED ADDITIONAL REINF (PROVIDE STD 90° HOOK AT SLAB EDGE
SCHEDULED ANCHOR EMBED PLATE
2"
3 1/2"
SLAB THICKNESS (T)
CONFINING REINFORCEMENT - SEE SCHEDULE
1" CLEAR (c)

1A Shear Wall Embed Plate Anchorage - At Edge Section
3/4" = 1'-0"



1C
S5.416
SCHEDULED EMBED PLATE
SCHEDULED ADDITIONAL ANCHORAGE REINFORCEMENT - SEE SCHEDULE
ANCHOR AT INTERIOR
SCHEDULED ANCHOR BLOCK CONNECTOR
> 1.5t
SCHEDULED CONFINING REINFORCEMENT
SCHEDULED ANCHOR BLOCK CONNECTOR
SCHEDULED EMBED PLATE
ADDITIONAL ANCHORAGE REINFORCEMENT (PERP) - SEE SCHEDULE
ADDITIONAL ANCHORAGE REINFORCEMENT (PAR) - SEE SCHEDULE
>5" TO 1.5t
SLAB EDGE
ANCHOR NEAR EDGE
1B
S5.416
ANCHOR
PROVIDE STANDARD 90° HOOK FOR ADDITIONAL REINFORCEMENT AT SLAB EDGE CONDITIONS
ANCHOR AT SLAB EDGE
1A
S5.416
ANCHOR
BRICK LEDGE WHERE OCCURS

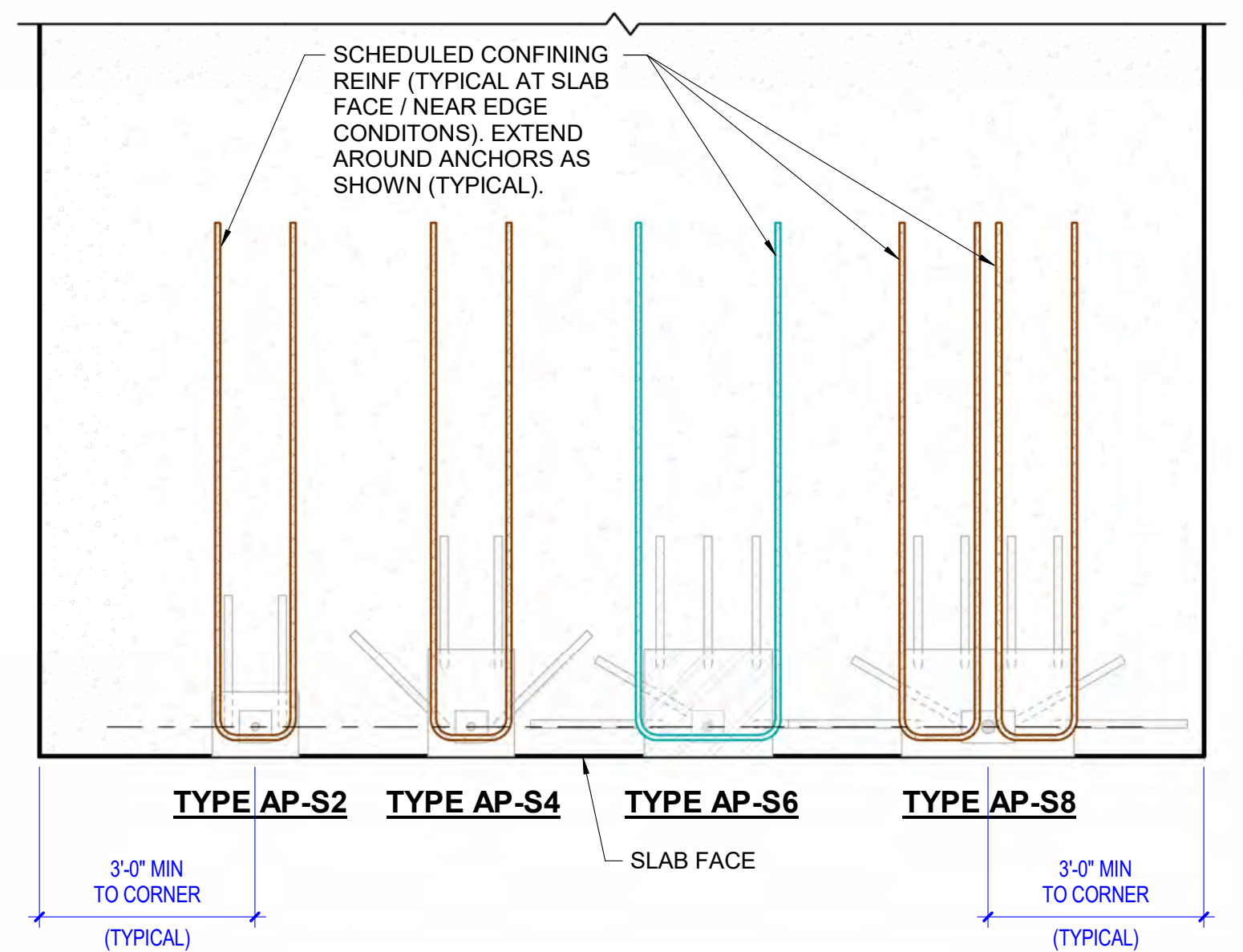
1 Anchor Rod Embed Plate Anchorage
3/4" = 1'-0"



NOTE:
- SEE NOTE **CC4.0 SHEAR STUD RAIL REINFORCEMENT** FOR ADDITIONAL STUD RAIL INFORMATION

ANCHOR EMBED PLATE TYPES

d = SLAB THK - (PLATE THK + COVER) / d=T - (t + C) - TYPICAL UNO



SCHEDULED CONFINING REINF (TYPICAL AT SLAB FACE / NEAR EDGE CONDITONS). EXTEND AROUND ANCHORS AS SHOWN (TYPICAL).
TYPE AP-S2
TYPE AP-S4
TYPE AP-S6
TYPE AP-S8
SLAB FACE
3'-0" MIN TO CORNER
(TYPICAL)

CONFINING REINFORCEMENT

EMBED PLATE NOTES:

1. AT LOCATIONS WHERE EMBED PLATE IS EXPOSED TO THE ELEMENTS, PLATE SHALL BE HOT DIP GALVANIZED.
2. ALTERNATIVELY, PROVIDE CJP SHOP WELD BETWEEN SCHEDULED ROD AND PLATE.
3. BOTTOM PLATE SCHEDULE BASED ON 5,000 PSI MIN. SPECIFIED CONCRETE COMPRESSIVE STRENGTH. CONTRACTOR TO COORDINATE INCREASE IN STUD RAIL REINFORCEMENT FOR ALL LOCATIONS WITH LESS THAN 5,000 PSI MIN. SPECIFIED CONCRETE COMPRESSIVE STRENGTH

SHEAR WALL END ANCHOR EMBED PLATE SCHEDULE

ANCHOR EMBED PLATE TYPE	LOCATION	PLATE SIZE						A706 DBA		CONFINING REINFORCEMENT
		W	L	t	e1	e2	e3	QTY	SIZE	
AP-S2.6-8.1	SLAB EDGE	6"	8"	3/4"	1 1/2"	1 1/2"	3"	2	#4	(1) TYPE E T&B
AP-S2.6-8.2	NEAR EDGE	6"	8"	3/4"	1 1/2"	1 1/2"	1 1/2"	2	#4	(1) TYPE E T&B
AP-S2.6-8.3	SLAB EDGE	6"	8"	3/4"	1 1/2"	1 1/2"	3"	2	#5	(1) TYPE E T&B
AP-S2.6-8.4	NEAR EDGE	6"	8"	3/4"	1 1/2"	1 1/2"	1 1/2"	2	#5	(2) TYPE E T&B
AP-S4.8-8.1	NEAR EDGE	8"	8"	1"	1 1/2"	1 1/2"	1 1/2"	4	#4	(1) TYPE E T&B
AP-S4.8-8.2	NEAR EDGE	8"	8"	1 1/8"	1 1/2"	1 1/2"	1 1/2"	4	#5	(1) TYPE E T&B
AP-S4.8-10.1	SLAB EDGE	8"	10"	1"	1 1/2"	1 1/2"	3"	4	#4	(1) TYPE E T&B
AP-S4.8-10.2	SLAB EDGE	8"	10"	1 1/8"	1 1/2"	1 1/2"	3"	4	#5	(1) TYPE E T&B
AP-S6.8-12.1	NEAR EDGE	8"	12"	1 3/8"	1 1/2"	1 1/2"	1 1/2"	6	#5	(1) TYPE F T&B
AP-S6.10-12.1	SLAB EDGE	10"	12"	1 3/8"	1 1/2"	1 1/2"	3"	6	#5	(1) TYPE F T&B
AP-S8.16-10.1	SLAB EDGE	10"	16"	1 5/8"	1 1/2"	1 1/2"	3"	8	#5	(2) TYPE E T&B
AP-S8.16-8.1	NEAR EDGE	8"	16"	1 5/8"	1 1/2"	1 1/2"	1 1/2"	8	#5	(2) TYPE E T&B
AP-A2.6-8.1	INTERIOR	6"	8"	3/4"	1 1/2"	1 1/2"	-	2	#4	-
AP-A2.6-8.2	INTERIOR	6"	8"	3/4"	1 1/2"	1 1/2"	-	2	#5	-
AP-A4.8-8.1	INTERIOR	8"	8"	1"	1 1/2"	1 1/2"	-	4	#4	-
AP-A4.8-8.2	INTERIOR	8"	8"	1 1/8"	1 1/2"	1 1/2"	-	4	#5	-
AP-A6.8-12.1	INTERIOR	8"	12"	1 3/8"	1 1/2"	1 1/2"	-	6	#5	-
AP-A8.12-12.1	INTERIOR	12"	12"	1 5/8"	1 1/2"	1 1/2"	-	8	#5	-

ANCHOR EMBED PLATE TYPE	LOCATION	W	L	t	RAILS PER SIDE (RS)	OVERALL LENGTH OF RAIL (RL)	NUMBER OF STUDS PER RAIL (Q)	DISTANCE TO FIRST STUD (E)	STUD SPACING (S)	STUD DIA. (D)	TOP COVER (TC)
AP-B1.3-3	CORNER	3"	3"	3/8"	-	-	-	-	-	-	-
AP-B1.512-512	CORNER	5 1/2"	5 1/2"	5/8"	-	-	-	-	-	-	-
AP-B1.8-8	CORNER	8"	8"	7/8"	-	-	-	-	-	-	-
AP-B1.15-15	CORNER	15"	15"	1 1/4"	-	-	-	-	-	-	-
AP-B1.18-18	CORNER	18"	18"	1 3/8"	-	-	-	-	-	-	-
AP-B1.22-22	CORNER	22"	22"	1 1/2"	-	-	-	-	-	-	-
AP-B2.18-18	CORNER	18"	18"	1 3/8"	2	24"	5	4"	4"	3/4"	1"
AP-B2.27-27	CORNER	27"	27"	1 1/2"	2	62"	10	4"	6"	3/4"	1"
		PLATE SIZE			STUD RAIL REINFORCEMENT						

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SHEET TITLE:
Embed Plate Anchorage Option - Uniform Thickness Slab

SHEET NUMBER:
S5.416

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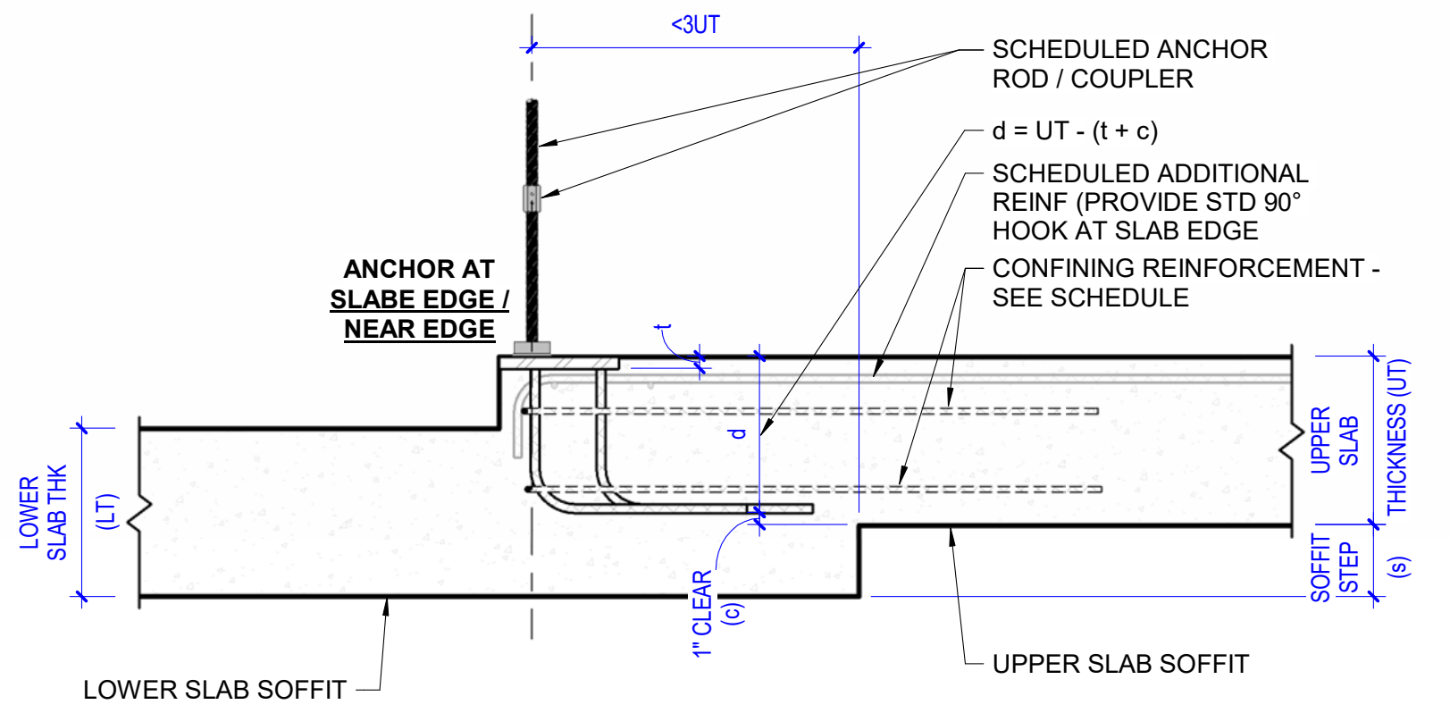
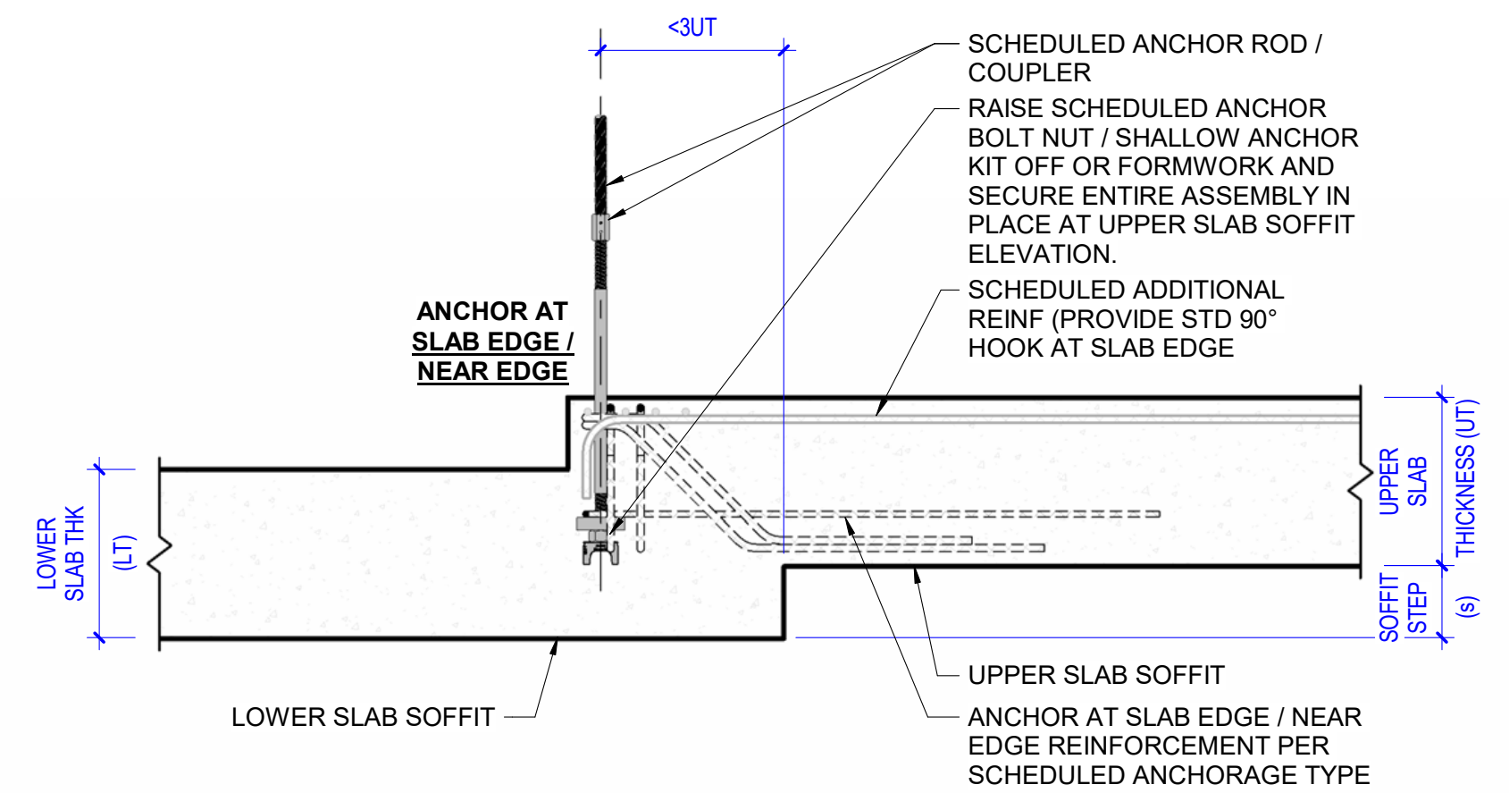


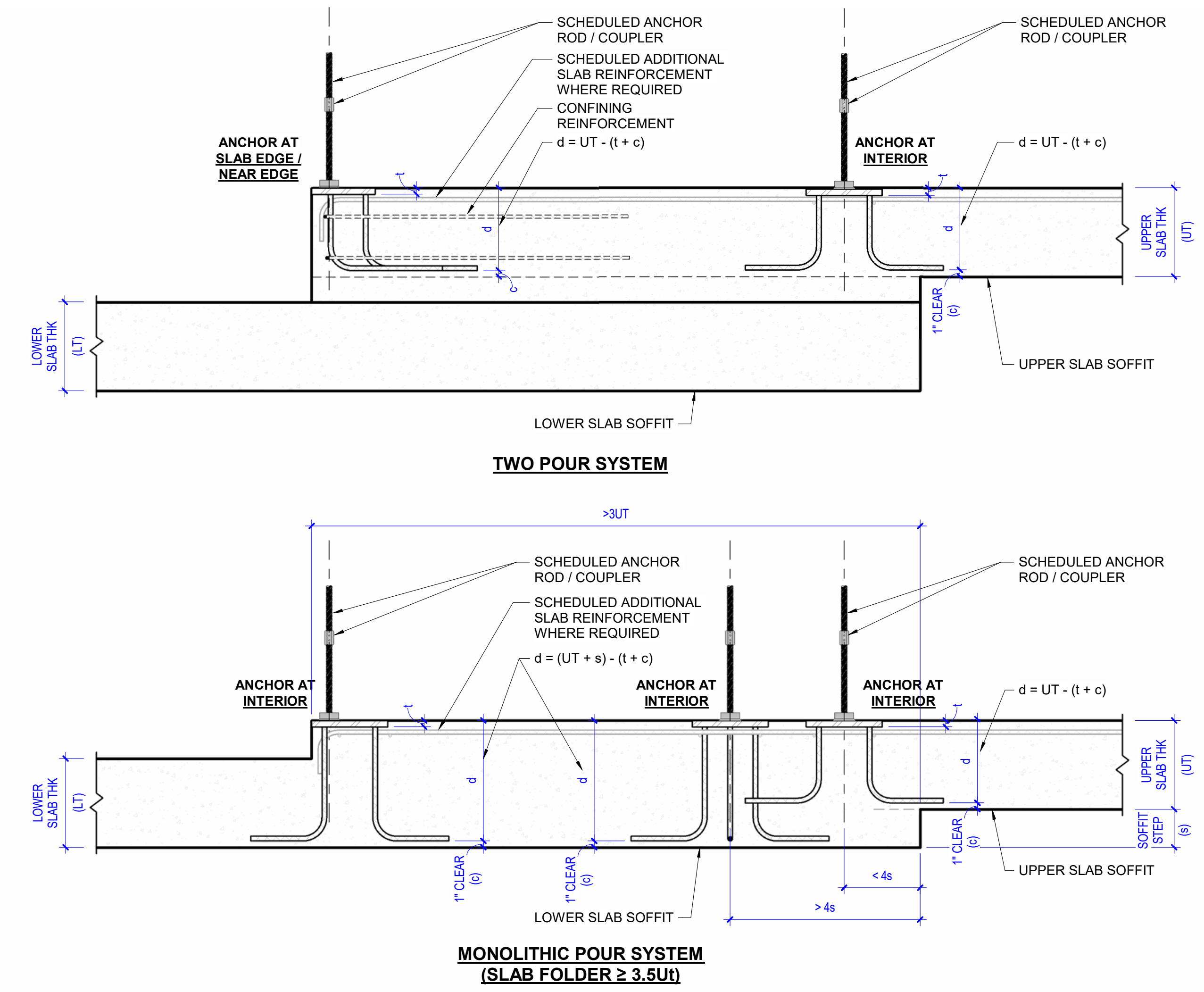
<3UT
SCHEDULED ANCHOR ROD / COUPLER
d = UT - (t + c)
SCHEDULED ADDITIONAL REINF (PROVIDE STD 90° HOOK AT SLAB EDGE
CONFINING REINFORCEMENT - SEE SCHEDULE
ANCHOR AT SLABE EDGE / NEAR EDGE
t
d
LOWER SLAB THK (LT)
UPPER SLAB THICKNESS (UT)
SOFFIT STEP (s)
1" CLEAR (c)
LOWER SLAB SOFFIT
UPPER SLAB SOFFIT

5 Shear Wall Anchor Rod Anchorage at Step - Embed Plate Anchorage
3/4" = 1'-0"

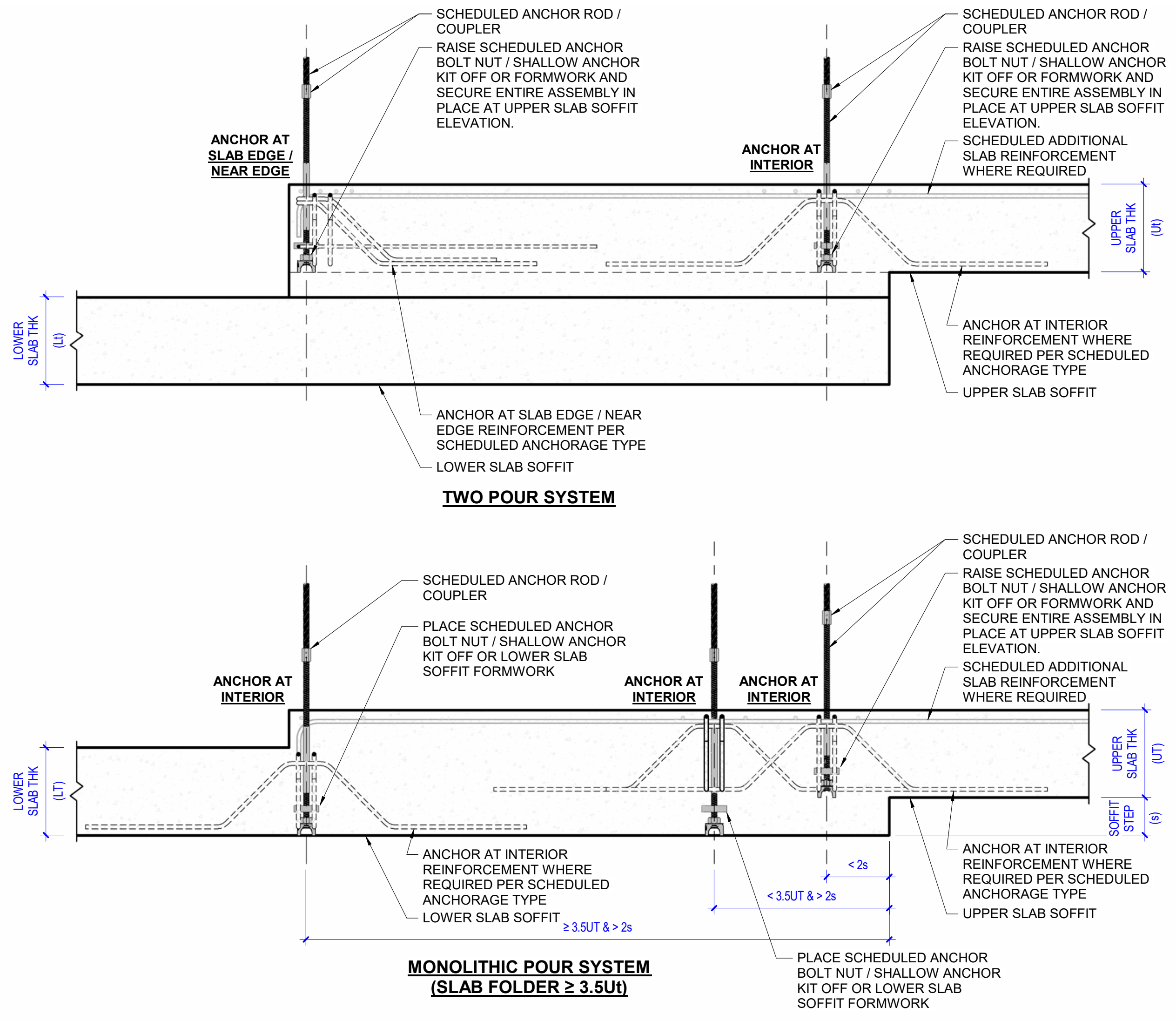


<3UT
SCHEDULED ANCHOR ROD / COUPLER
RAISE SCHEDULED ANCHOR BOLT NUT / SHALLOW ANCHOR KIT OFF OR FORMWORK AND SECURE ENTIRE ASSEMBLY IN PLACE AT UPPER SLAB SOFFIT ELEVATION.
SCHEDULED ADDITIONAL REINF (PROVIDE STD 90° HOOK AT SLAB EDGE
ANCHOR AT SLAB EDGE / NEAR EDGE
LOWER SLAB THK (LT)
UPPER SLAB THICKNESS (UT)
SOFFIT STEP (s)
LOWER SLAB SOFFIT
UPPER SLAB SOFFIT
ANCHOR AT SLAB EDGE / NEAR EDGE REINFORCEMENT PER SCHEDULED ANCHORAGE TYPE

4 Shear Wall Anchor Rod Anchorage at Step - Embedded Rod Anchorage
3/4" = 1'-0"

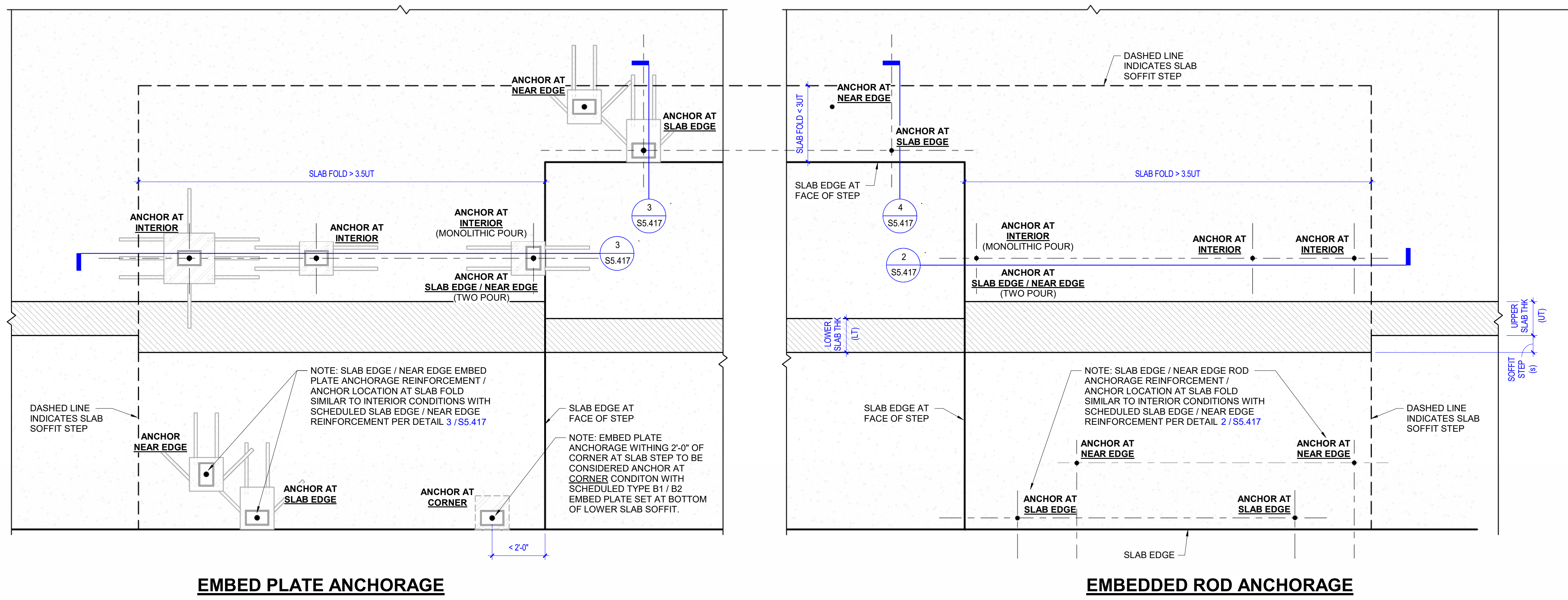


3 Shear Wall Anchor Rod Anchorage at Step - Embed Plate Anchorage
3/4" = 1'-0"



SCHEDULED ANCHOR ROD / COUPLER
RAISE SCHEDULED ANCHOR BOLT NUT / SHALLOW ANCHOR KIT OFF OR FORMWORK AND SECURE ENTIRE ASSEMBLY IN PLACE AT UPPER SLAB SOFFIT ELEVATION.
ANCHOR AT SLAB EDGE / NEAR EDGE
ANCHOR AT INTERIOR
SCHEDULED ANCHOR ROD / COUPLER
RAISE SCHEDULED ANCHOR BOLT NUT / SHALLOW ANCHOR KIT OFF OR FORMWORK AND SECURE ENTIRE ASSEMBLY IN PLACE AT UPPER SLAB SOFFIT ELEVATION.
SCHEDULED ADDITIONAL SLAB REINFORCEMENT WHERE REQUIRED
LOWER SLAB THK (Lt)
UPPER SLAB THK (Ut)
ANCHOR AT INTERIOR REINFORCEMENT WHERE REQUIRED PER SCHEDULED ANCHORAGE TYPE
UPPER SLAB SOFFIT
ANCHOR AT SLAB EDGE / NEAR EDGE REINFORCEMENT PER SCHEDULED ANCHORAGE TYPE
LOWER SLAB SOFFIT
TWO POUR SYSTEM
SCHEDULED ANCHOR ROD / COUPLER
PLACE SCHEDULED ANCHOR BOLT NUT / SHALLOW ANCHOR KIT OFF OR LOWER SLAB SOFFIT FORMWORK
ANCHOR AT INTERIOR
ANCHOR AT INTERIOR
ANCHOR AT INTERIOR
SCHEDULED ANCHOR ROD / COUPLER
RAISE SCHEDULED ANCHOR BOLT NUT / SHALLOW ANCHOR KIT OFF OR FORMWORK AND SECURE ENTIRE ASSEMBLY IN PLACE AT UPPER SLAB SOFFIT ELEVATION.
SCHEDULED ADDITIONAL SLAB REINFORCEMENT WHERE REQUIRED
LOWER SLAB THK (LT)
UPPER SLAB THK (UT)
SOFFIT STEP (s)
ANCHOR AT INTERIOR REINFORCEMENT WHERE REQUIRED PER SCHEDULED ANCHORAGE TYPE
LOWER SLAB SOFFIT
< 2s
< 3.5UT & > 2s
≥ 3.5UT & > 2s
ANCHOR AT INTERIOR REINFORCEMENT WHERE REQUIRED PER SCHEDULED ANCHORAGE TYPE
UPPER SLAB SOFFIT
PLACE SCHEDULED ANCHOR BOLT NUT / SHALLOW ANCHOR KIT OFF OR LOWER SLAB SOFFIT FORMWORK
MONOLITHIC POUR SYSTEM (SLAB FOLDER ≥ 3.5Ut)

2 Shear Wall Anchor Rod Anchorage at Step - Embedded Rod Anchorage
3/4" = 1'-0"



ANCHOR AT NEAR EDGE
ANCHOR AT SLAB EDGE
SLAB FOLD > 3.5UT
ANCHOR AT INTERIOR
ANCHOR AT INTERIOR
ANCHOR AT INTERIOR (MONOLITHIC POUR)
ANCHOR AT SLAB EDGE / NEAR EDGE (TWO POUR)
3 S5.417
3 S5.417
DASHED LINE INDICATES SLAB SOFFIT STEP
NOTE: SLAB EDGE / NEAR EDGE EMBED PLATE ANCHORAGE REINFORCEMENT / ANCHOR LOCATION AT SLAB FOLD SIMILAR TO INTERIOR CONDITIONS WITH SCHEDULED SLAB EDGE / NEAR EDGE REINFORCEMENT PER DETAIL 3 / S5.417
ANCHOR NEAR EDGE
ANCHOR AT SLAB EDGE
ANCHOR AT CORNER
SLAB EDGE AT FACE OF STEP
NOTE: EMBED PLATE ANCHORAGE WITHING 2'-0" OF CORNER AT SLAB STEP TO BE CONSIDERED ANCHOR AT CORNER CONDITON WITH SCHEDULED TYPE B1 / B2 EMBED PLATE SET AT BOTTOM OF LOWER SLAB SOFFIT.
< 2'-0"
EMBED PLATE ANCHORAGE
SLAB FOLD < 3UT
DASHED LINE INDICATES SLAB SOFFIT STEP
ANCHOR AT NEAR EDGE
ANCHOR AT SLAB EDGE
SLAB EDGE AT FACE OF STEP
4 S5.417
2 S5.417
SLAB FOLD > 3.5UT
ANCHOR AT INTERIOR (MONOLITHIC POUR)
ANCHOR AT INTERIOR
ANCHOR AT INTERIOR
ANCHOR AT SLAB EDGE / NEAR EDGE (TWO POUR)
LOWER SLAB THK (LT)
UPPER SLAB THK (UT)
SOFFIT STEP (s)
SLAB EDGE AT FACE OF STEP
NOTE: SLAB EDGE / NEAR EDGE ROD ANCHORAGE REINFORCEMENT / ANCHOR LOCATION AT SLAB FOLD SIMILAR TO INTERIOR CONDITIONS WITH SCHEDULED SLAB EDGE / NEAR EDGE REINFORCEMENT PER DETAIL 2 / S5.417
DASHED LINE INDICATES SLAB SOFFIT STEP
ANCHOR AT NEAR EDGE
ANCHOR AT NEAR EDGE
ANCHOR AT SLAB EDGE
ANCHOR AT SLAB EDGE
SLAB EDGE
EMBEDDED ROD ANCHORAGE

1 Shear Wall Anchorage at Slab Step
3/4" = 1'-0"

VSM2
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Structural Engineers

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SHEET TITLE:
Shear Wall Anchorage at Slab Steps

SHEET NUMBER:
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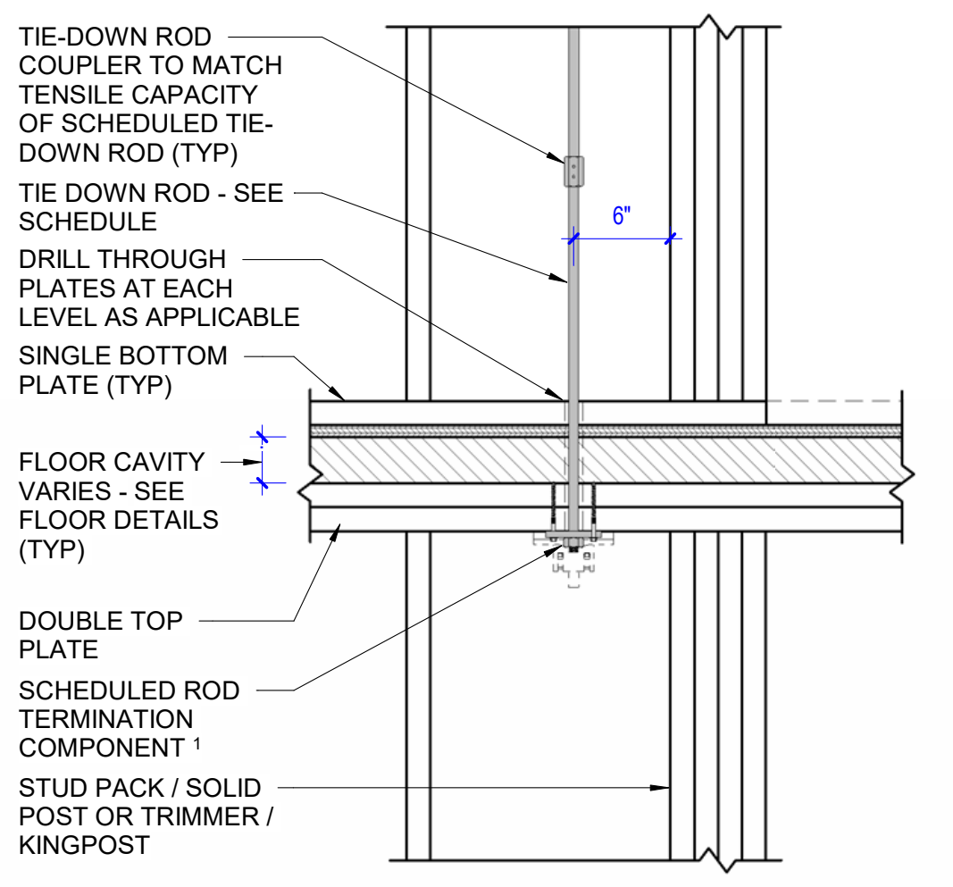
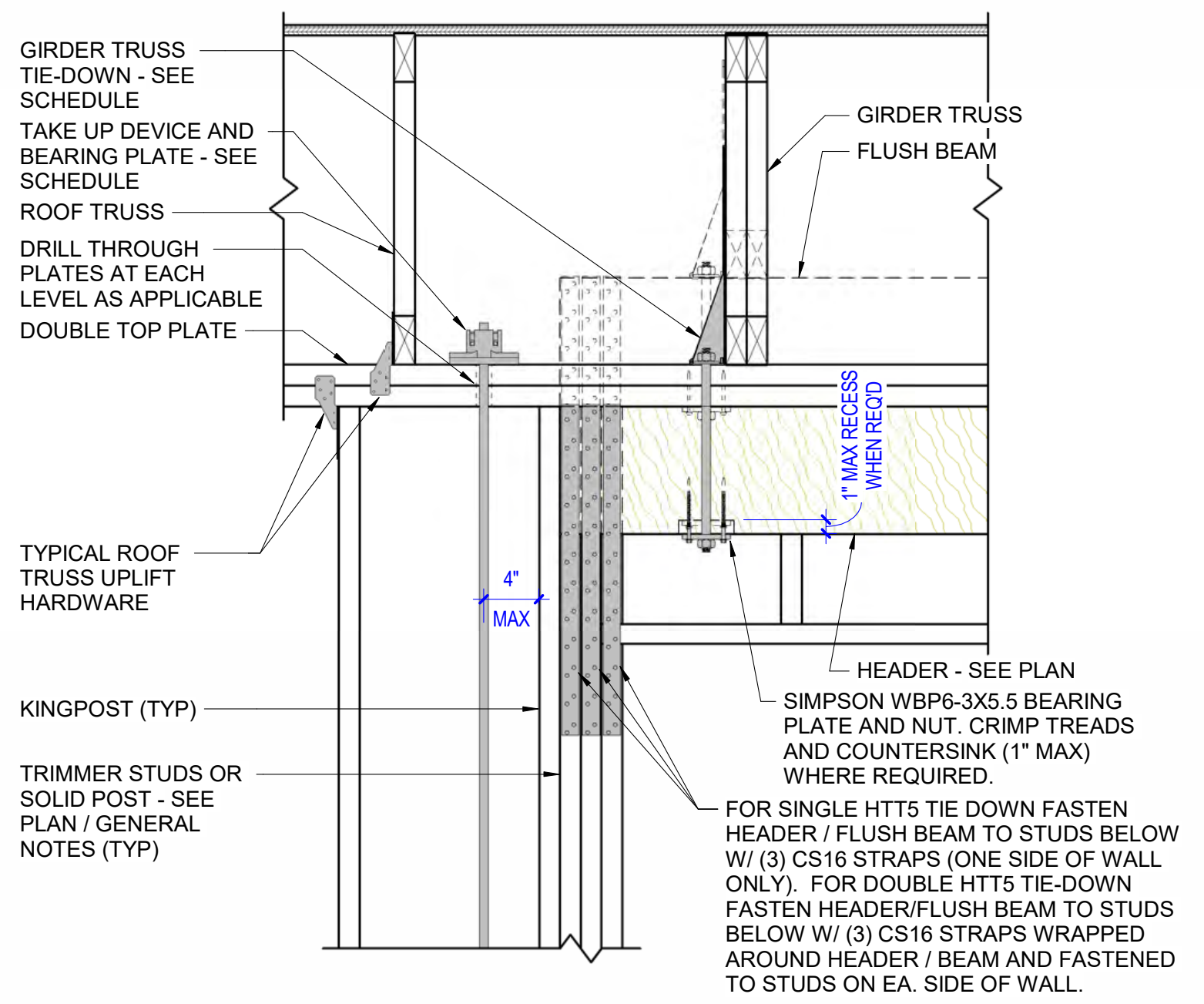

TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
TIE DOWN ROD - SEE SCHEDULE
6"
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE
SCHEDULED ROD TERMINATION COMPONENT 1
STUD PACK / SOLID POST OR TRIMMER / KINGPOST

SCHEDULED ROD TERMINATION COMPONENT

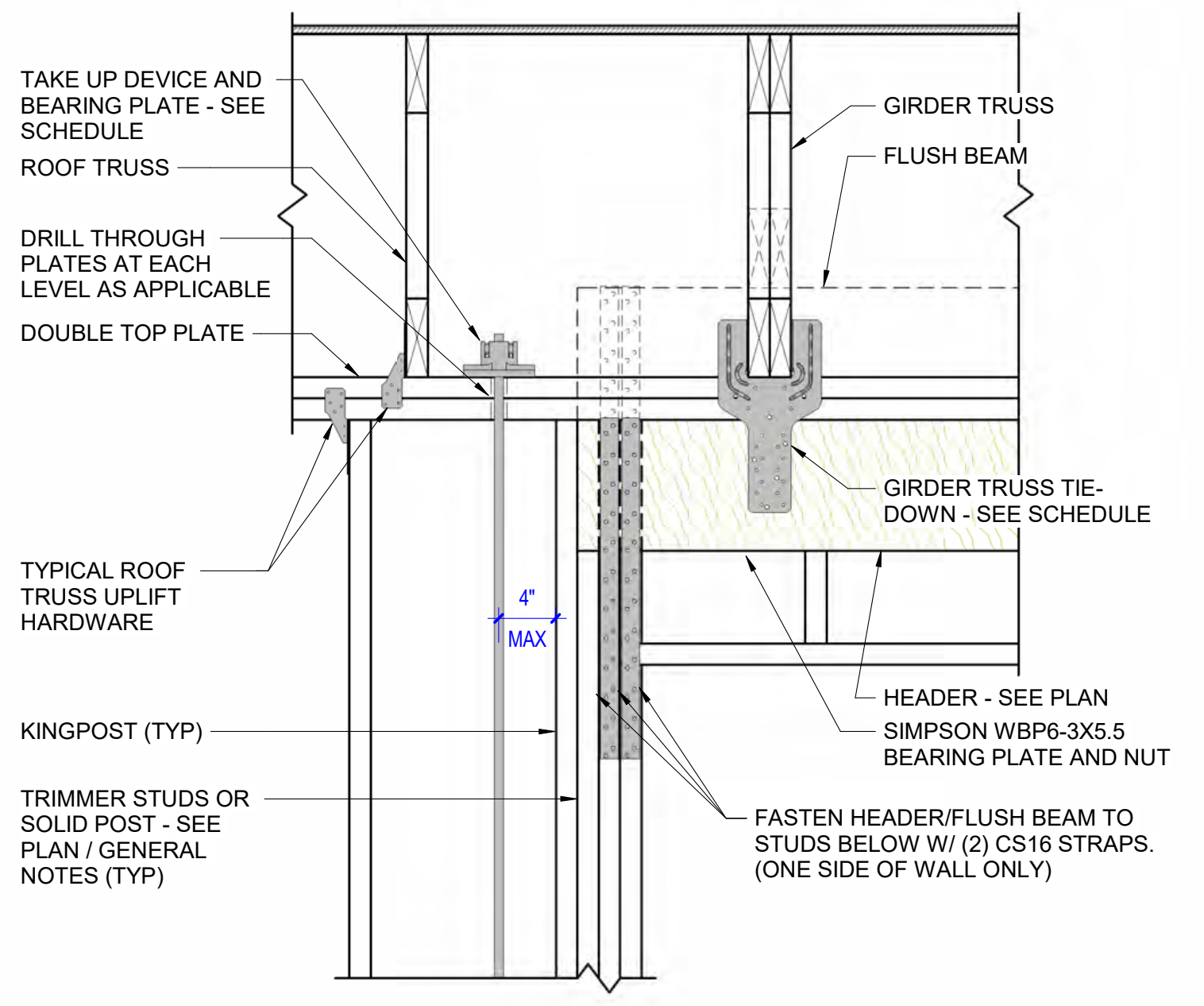
TIE-DOWN ROD SIZE	TERMINATION COMPONENT
ATR3/8	PL1/4x3"x0'-3" W/ 3/8"⌀ HEAVY HEX NUT
ATR1/2	SIMPSON WBPWBP4-3X3.5 W/ 1/2"⌀ HEAVY HEX NUT

NOTE:
1. ALTERNATIVELY, TERMINATION COMPONENTS MAY BE INSTALLED ON TOP OF ROD WITH RTUD INSTALLED AT BOTTOM OF ROD.

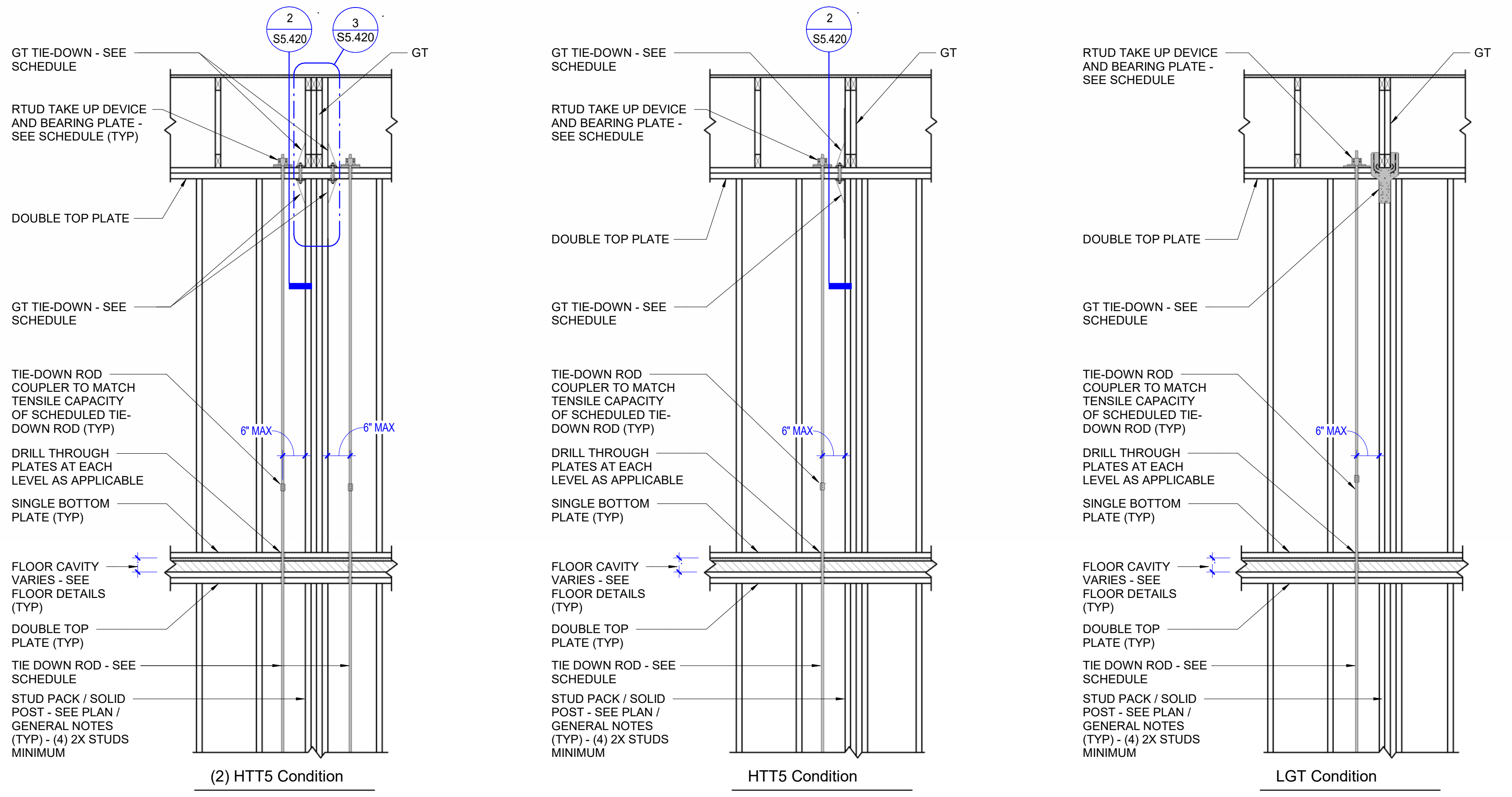
6 Roof Tie-Down Rod Termination at Plate
1" = 1'-0"


GIRDER TRUSS TIE-DOWN - SEE SCHEDULE
TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
ROOF TRUSS
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
DOUBLE TOP PLATE
GIRDER TRUSS
FLUSH BEAM
1" MAX RECESS WHEN REQ'D
TYPICAL ROOF TRUSS UPLIFT HARDWARE
4" MAX
KINGPOST (TYP)
TRIMMER STUDS OR SOLID POST - SEE PLAN / GENERAL NOTES (TYP)
HEADER - SEE PLAN
SIMPSON WBP6-3X5.5 BEARING PLATE AND NUT. CRIMP TREADS AND COUNTERSINK (1" MAX) WHERE REQUIRED.
FOR SINGLE HTT5 TIE DOWN FASTEN HEADER / FLUSH BEAM TO STUDS BELOW W/ (3) CS16 STRAPS (ONE SIDE OF WALL ONLY). FOR DOUBLE HTT5 TIE-DOWN FASTEN HEADER/FLUSH BEAM TO STUDS BELOW W/ (3) CS16 STRAPS WRAPPED AROUND HEADER / BEAM AND FASTENED TO STUDS ON EA. SIDE OF WALL.

5 HTT5 Tie-Down Over Header / Flush Beam
1" = 1'-0"


TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
ROOF TRUSS
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
DOUBLE TOP PLATE
GIRDER TRUSS
FLUSH BEAM
GIRDER TRUSS TIE-DOWN - SEE SCHEDULE
TYPICAL ROOF TRUSS UPLIFT HARDWARE
4" MAX
KINGPOST (TYP)
TRIMMER STUDS OR SOLID POST - SEE PLAN / GENERAL NOTES (TYP)
HEADER - SEE PLAN
SIMPSON WBP6-3X5.5 BEARING PLATE AND NUT
FASTEN HEADER/FLUSH BEAM TO STUDS BELOW W/ (2) CS16 STRAPS. (ONE SIDE OF WALL ONLY)

4 LGT Tie-Down Over Header / Flush Beam
1" = 1'-0"


2 S5.420
3 S5.420
GT TIE-DOWN - SEE SCHEDULE
RTUD TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE (TYP)
GT
DOUBLE TOP PLATE
GT TIE-DOWN - SEE SCHEDULE
TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
6" MAX
6" MAX
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE (TYP)
TIE DOWN ROD - SEE SCHEDULE
STUD PACK / SOLID POST - SEE PLAN / GENERAL NOTES (TYP) - (4) 2X STUDS MINIMUM
2 S5.420
GT TIE-DOWN - SEE SCHEDULE
RTUD TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
GT
DOUBLE TOP PLATE
GT TIE-DOWN - SEE SCHEDULE
TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
6" MAX
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE (TYP)
TIE DOWN ROD - SEE SCHEDULE
STUD PACK / SOLID POST - SEE PLAN / GENERAL NOTES (TYP) - (4) 2X STUDS MINIMUM
RTUD TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
GT
DOUBLE TOP PLATE
GT TIE-DOWN - SEE SCHEDULE
TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
6" MAX
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE (TYP)
TIE DOWN ROD - SEE SCHEDULE
STUD PACK / SOLID POST - SEE PLAN / GENERAL NOTES (TYP) - (4) 2X STUDS MINIMUM

(2) HTT5 Condition

HTT5 Condition

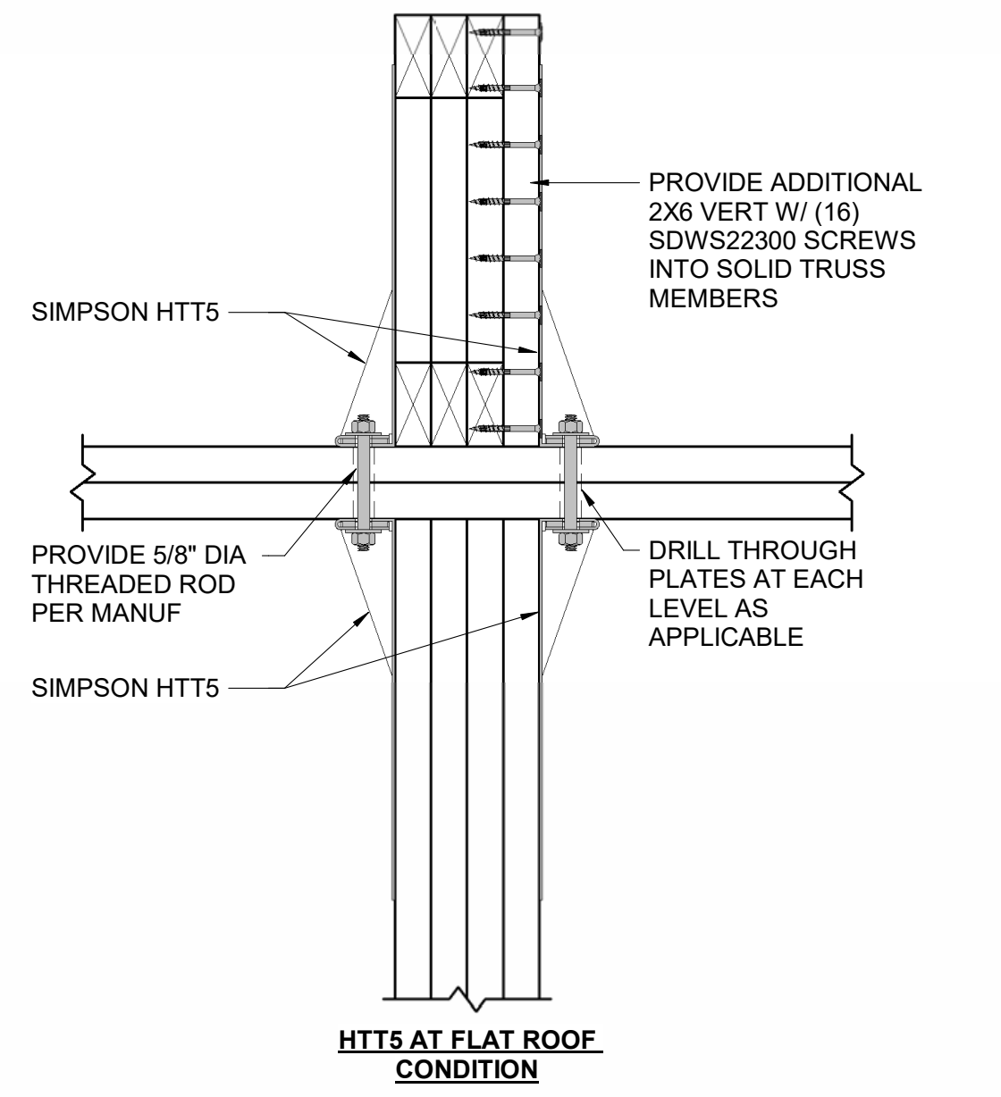
LGT Condition

GIRDER TRUSS (GT) UPLIFT TIE-DOWN SCHEDULE[1] AT FLAT ROOF

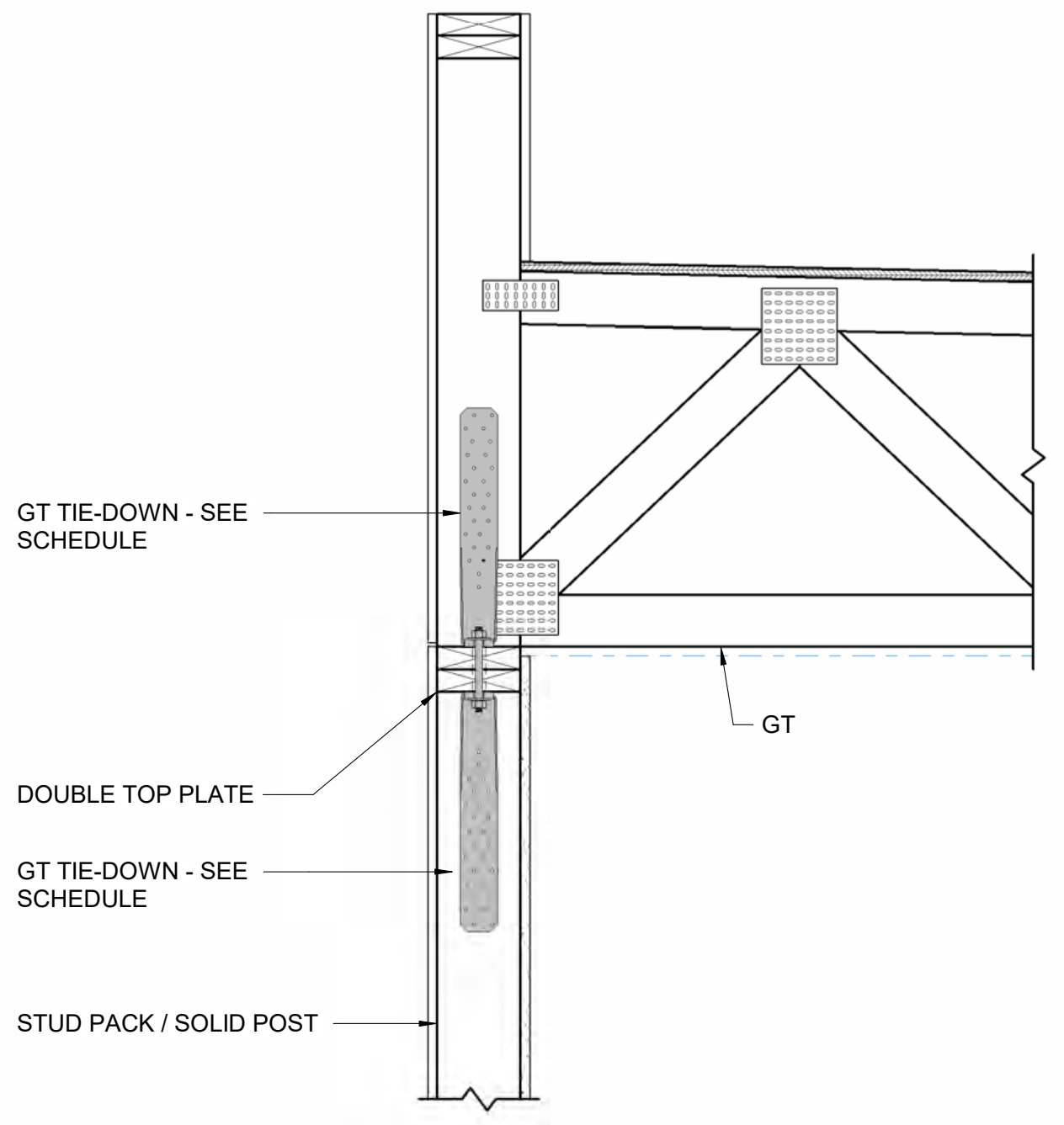
G.T. UPLIFT REACTION [10]	NUMBER OF TRUSS PLIES	TIE-DOWN EACH BEARING POINT [7, 8, 9]	TIE-DOWN ROD SIZE [3]	TAKE-UP DEVICE [6]	BEARING PLATE
0-700 LBS	1 OR MORE	H2.5A	-	-	-
701 - 2,124 LBS	2	LGT2 - #9	ATR3/8[4]	RTUD3B	BPRTUD3-4B
2,124 - 2,505 LBS	3 OR 4	LGT3-SDS2.5 / LGT4-SDS3	ATR1/2[5]	RTUD4B	BPRTUD3-4B
2,506 - 4,375 LBS	2 OR MORE	HTT5 @ GT & STUD PACK / POST	ATR5/8	RTUD5	BPRTUD5-6B
4,376 - 8,680 LBS	3 OR MORE	(2) HTT5 @ GT & STUD PACK / POST	(2) ATR5/8	(2) RTUD5	(2) BPRTUD5-6B

GT UPLIFT TIE-DOWN SCHEDULE NOTES:
1. UPLIFT RESTRAINT IS PROVIDED BY SIMPSON STRONG-TIE "URS" SYSTEM AND SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS (TYPICAL). ROOF ANCHOR TYPES ARE MANUFACTURED BY SIMPSON STRONG-TIE AND SHALL BE INSTALLED PER MANUFACTURER REQUIREMENTS, TYP.
2. TIE-DOWN RODS, TAKE-UP SHRINKAGE COMPENSATING DEVICES AND BEARING PLATE MODELS BY SIMPSON STRONG-TIE AS NOTED.
3. ATR RODS SHALL BE CAST IN PLACE INTO FOUNDATION OR PODIUM WITH 7" MINIMUM EMBEDMENT WITH HEAVY HEX NUTS FITTED AT EMBEDDED END AND TACK WELDED TO ROD. A MINIMUM 2 3/4" CONCRETE EDGE DISTANCE FROM CENTER OF ROD. CORNER CONDITIONS ARE NOT PERMITTED WITH EDGE DISTANCES SHOWN. (AT CONTRACTORS OPTION, AT NON-PT LOCATIONS ATR RODS MAY BE DRILL AND EPOXY THREADED RODS IN HILTI HY-200 V3 ADHESIVE INSTALLED PER MANUFACTURER REQUIREMENTS.)
4. FOR BUILDINGS WITH WOOD FRAMING MORE THAN 3 STORIES IN HEIGHT, ATR3/8 TIE-DOWN RODS NEED ONLY EXTEND DOWN 3 LEVELS BELOW ROOF LEVEL WITH SCHEDULED SIMPSON WBP AND NUT BELOW DOUBLE TOP PLATE. SEE DETAIL 6 / S5.420
5. FOR BUILDINGS WITH WOOD FRAMING MORE THAN 4 STORIES IN HEIGHT, ATR1/2 TIE-DOWN RODS NEED ONLY EXTEND DOWN 4 LEVELS BELOW ROOF LEVEL WITH SCHEDULED SIMPSON WBP AND NUT BELOW DOUBLE TOP PLATE. SEE DETAIL 6 / S5.420
6. DO NOT PROVIDE FIRE CAULKING IN 2X PLATES AT TAKE-UP SHRINKAGE COMPENSATING DEVICES (TYPICAL).
7. LGT2 AND LGT3 TIE-DOWNS TO BE INSTALLED WITH #9 SD / SDS SCREWS PER MANUFACTURER. HTT5 TIES TO BE INSTALLED WITH 0.148"⌀ x 3" LONG NAILS OR 0.162"⌀ x 2 1/2" NAILS, U.N.O.
8. WHERE LGT TIE-DOWN OCCURS OVER HEADER OR FLUSH BEAM, SEE DETAIL 4 / S5.420
9. WHERE HTT5 OCCURS OVER HEADER OR FLUSH BEAM, SEE DETAIL 5 / S5.420
10. G.T. UPLIFT REACTIONS ARE INDICATED ON TRUSS DESIGN DRAWINGS PROVIDED BY THE WOOD TRUSS MANUFACTURER IN ACCORDANCE WITH THE WOOD GENERAL NOTES.

1 Girder Truss Uplift Detail - Flat Roof
1/2" = 1'-0"


PROVIDE ADDITIONAL 2X6 VERT W/ (16) SDWS22300 SCREWS INTO SOLID TRUSS MEMBERS
SIMPSON HTT5
PROVIDE 5/8" DIA THREADED ROD PER MANUF
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SIMPSON HTT5
HTT5 AT FLAT ROOF CONDITION

3 Girder Truss Uplift Detail - Double Tie-Down
1 1/2" = 1'-0"



2 Girder Truss Uplift Detail - Flat Roof
1" = 1'-0"

VSM²
Voss | Starling | Maingot | Martin
Structural Engineers

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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No.	Description	Date
	100% DD	09/30/2026

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BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

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LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
Roof Girder Truss Uplift Details

SHEET NUMBER:
S5.420

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ROOF TRUSS UPLIFT AT EXTERIOR BEARING WALL SCHEDULE [1]

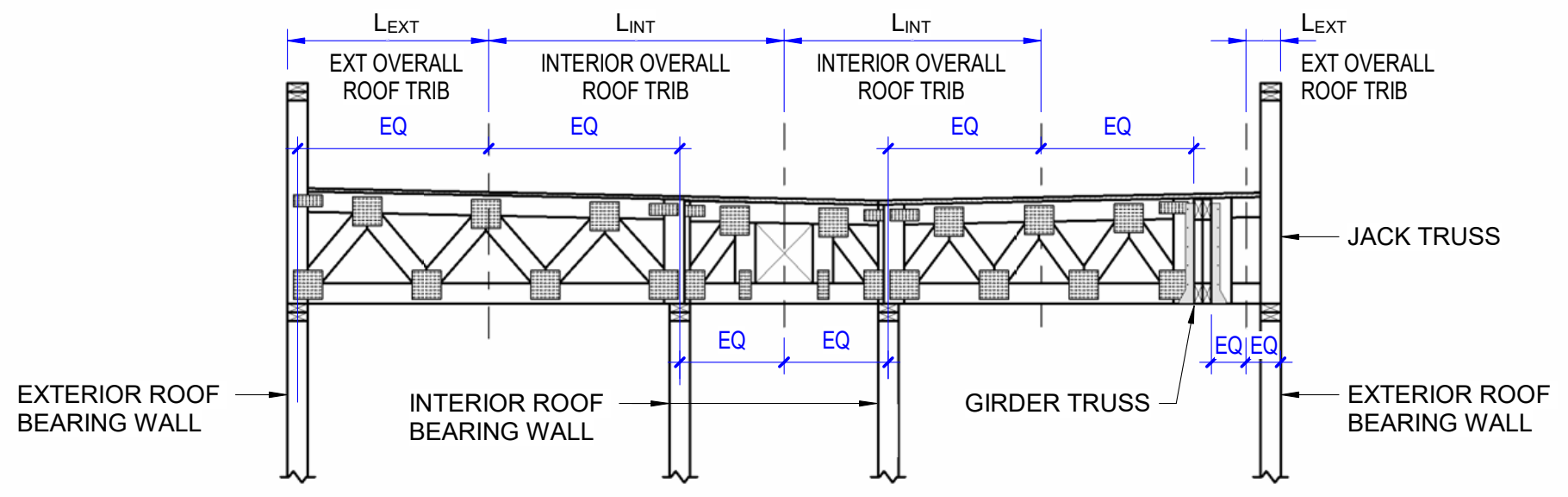
	L_{EXT} OVERALL ROOF TRUSS BEARING TRIB [4]	3'-11" OR LESS	4'-0" TO 10'-0"	10'-1" TO 18'-0"
ROOF TRUSS BEARING POINTS	DOUBLE TOP PLATE TO ROOF TRUSS CONNECTION W/ CLIPS OR SCREWS [2]	(1) @ EA. TRUSS	(1) @ EA. TRUSS	(1) @ EA. TRUSS
TOP LEVEL	DOUBLE TOP PLATE TO STUD CONNECTION W/ CLIPS OR SCREWS [2]	-	(1) @ 16" O.C.	(1) @ 16" O.C.
CONTINUOUS ROD	TOP 2 LEVELS [3]	-	(1) @ 48" O.C.	(1) @ 48" O.C.

ROOF TRUSS UPLIFT AT INTERIOR BEARING WALL SCHEDULE [1]

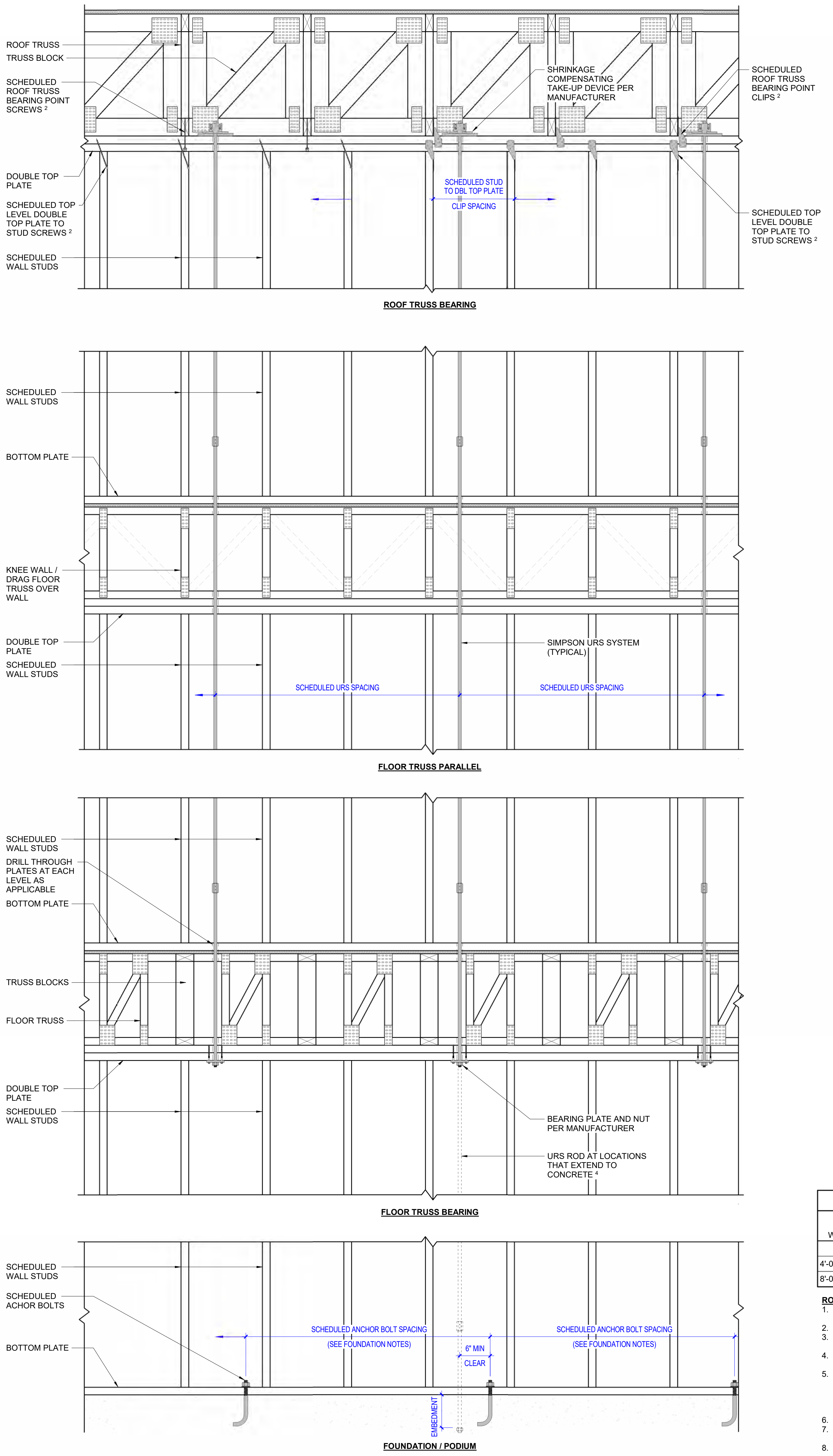
	L_{EXT} OVERALL ROOF TRUSS BEARING TRIB [4]	3'-11" OR LESS	4'-0" TO 10'-0"	10'-1" TO 18'-0"
ROOF TRUSS BEARING POINTS	DOUBLE TOP PLATE TO ROOF TRUSS CONNECTION W/ CLIPS OR SCREWS [2]	-	(1) @ EA. TRUSS	(1) @ EA. TRUSS
TOP LEVEL	DOUBLE TOP PLATE TO STUD CONNECTION W/ CLIPS OR SCREWS [2]	-	-	(1) @ 16" O.C.
CONTINUOUS ROD	TOP 2 LEVELS [3]	-	-	(1) @ 48" O.C.

NOTES:

1. SEE PROVIDED DETAIL FOR MORE INFORMATION.
2. CLIPS TO DOUBLE TOP PLATES SHALL BE SIMPSON H2.5A OR APPROVED EQUAL. CLIPS TO SINGLE BOTTOM PLATE TO BE SIMPSON H3 CLIP OR APPROVED EQUAL. ALTERNATE SCREW ATTACHMENT TO BE SIMPSON SDWC15600 OR APPROVED EQUAL INSTALLED PER MANUF REQUIREMENTS.
3. CONTINUOUS ROD IS TO BE 3/8" DIA SIMPSON UPLIFT RESTRAINT PROVIDED BY SIMPSON STRONG-TIE "URS" SYSTEM AND SHALL BE INSTALLED PER MANUF REQUIREMENTS. ROD SYSTEM SHALL EXTEND FROM UPPER MOST TOP PLATE DOWN THROUGH 2 FLOOR CAVITIES MINIMUM. ONE AND TWO LEVEL BUILDINGS SHALL HAVE ROD SYSTEM CAST INTO FOUNDATION OR PODIUM W/ 7" EMBEDMENT W/ HEAVY HEX NUTS FITTED TO END AND TACK WELDED IN PLACE. (AT CONTRACTORS OPTION, AT NON-PT LOCATIONS ATR RODS MAY BE DRILL AND EPOXY THREADED RODS IN HILTI HY-200 V3 ADHESIVE INSTALLED PER MANUFACTURER REQUIREMENTS.)
4. OVERALL ROOF TRUSS TRIB LENGTH IS DEFINED HEREIN:

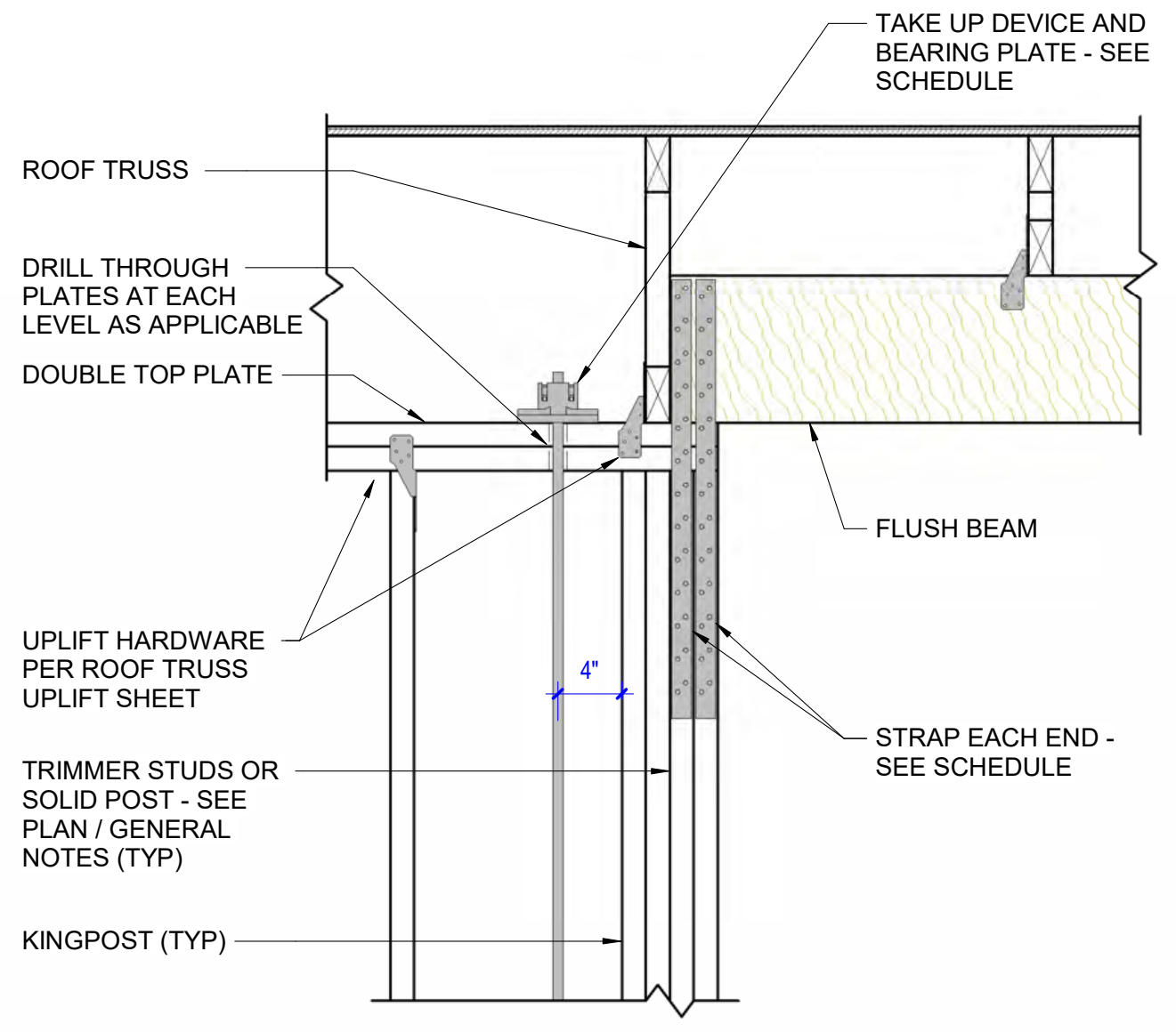


L_{EXT}
EXT OVERALL ROOF TRIB
L_{INT}
INTERIOR OVERALL ROOF TRIB
L_{INT}
INTERIOR OVERALL ROOF TRIB
L_{EXT}
EXT OVERALL ROOF TRIB
EQ
EQ
EQ
EQ
JACK TRUSS
EQ
EQ
EQ
EQ
EXTERIOR ROOF BEARING WALL
INTERIOR ROOF BEARING WALL
GIRDER TRUSS
EXTERIOR ROOF BEARING WALL



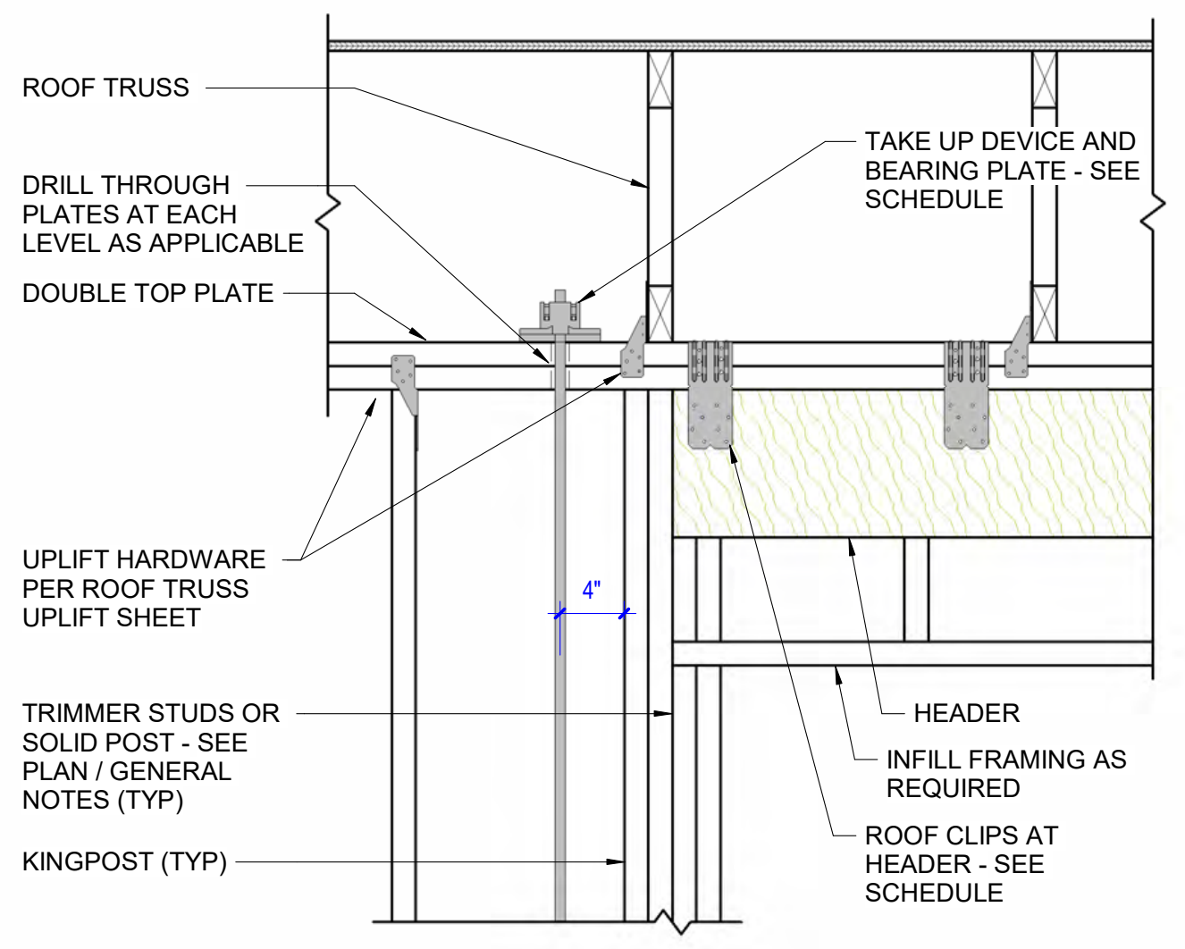
ROOF TRUSS
TRUSS BLOCK
SCHEDULED ROOF TRUSS BEARING POINT SCREWS ²
SHRINKAGE COMPENSATING TAKE-UP DEVICE PER MANUFACTURER
SCHEDULED ROOF TRUSS BEARING POINT CLIPS ²
DOUBLE TOP PLATE
SCHEDULED TOP LEVEL DOUBLE TOP PLATE TO STUD SCREWS ²
SCHEDULED STUD TO DBL TOP PLATE
CLIP SPACING
SCHEDULED TOP LEVEL DOUBLE TOP PLATE TO STUD SCREWS ²
SCHEDULED WALL STUDS
ROOF TRUSS BEARING
SCHEDULED WALL STUDS
BOTTOM PLATE
KNEE WALL / DRAG FLOOR TRUSS OVER WALL
DOUBLE TOP PLATE
SCHEDULED WALL STUDS
SIMPSON URS SYSTEM (TYPICAL)
SCHEDULED URS SPACING
SCHEDULED URS SPACING
FLOOR TRUSS PARALLEL
SCHEDULED WALL STUDS
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
BOTTOM PLATE
TRUSS BLOCKS
FLOOR TRUSS
DOUBLE TOP PLATE
SCHEDULED WALL STUDS
BEARING PLATE AND NUT PER MANUFACTURER
URS ROD AT LOCATIONS THAT EXTEND TO CONCRETE ⁴
FLOOR TRUSS BEARING
SCHEDULED WALL STUDS
SCHEDULED ACHOR BOLTS
BOTTOM PLATE
SCHEDULED ANCHOR BOLT SPACING
(SEE FOUNDATION NOTES)
6" MIN CLEAR
SCHEDULED ANCHOR BOLT SPACING
(SEE FOUNDATION NOTES)
EMBEDMENT
FOUNDATION / PODIUM

4 Uplift at Floor Trusses
1" = 1'-0"



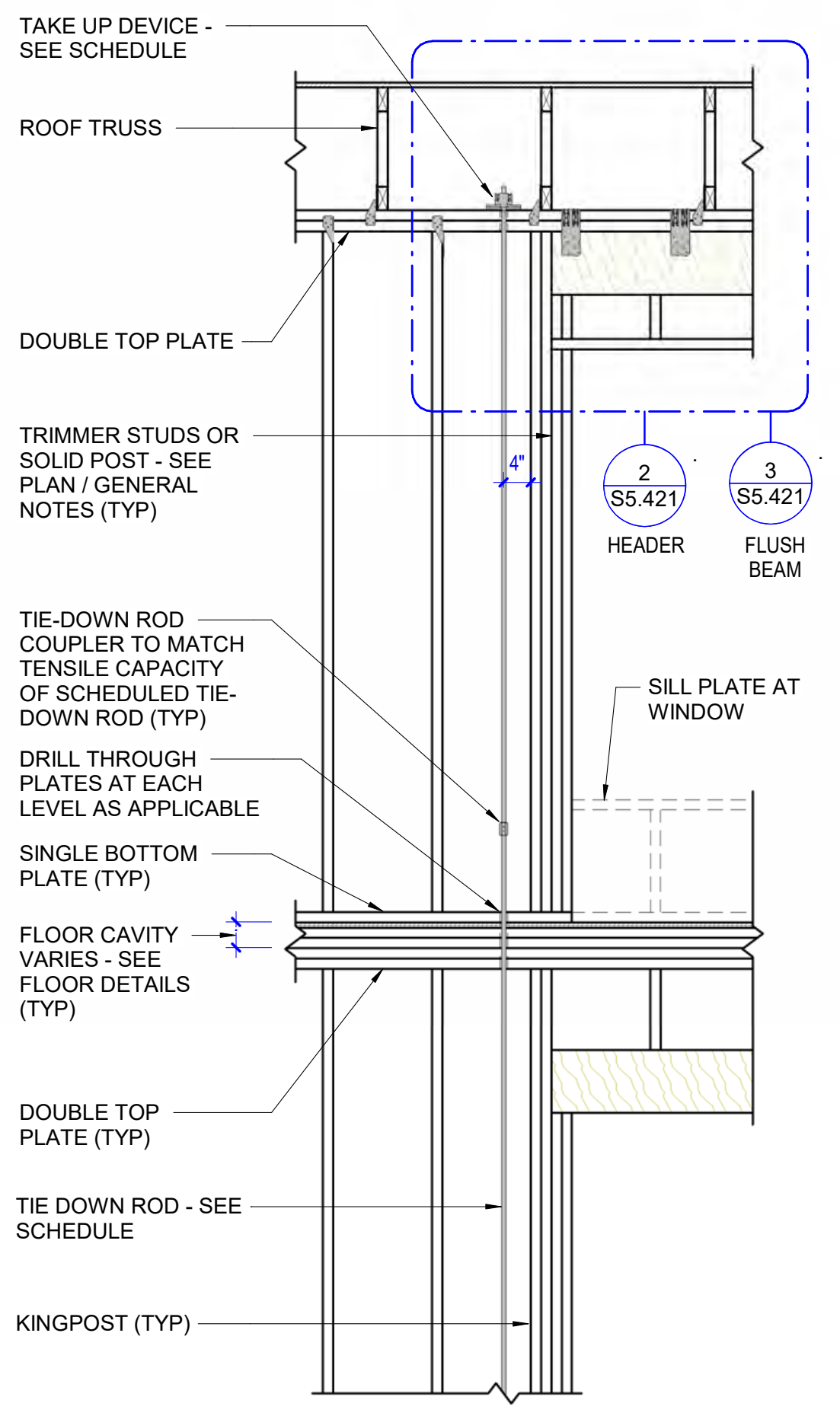
TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
ROOF TRUSS
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
DOUBLE TOP PLATE
FLUSH BEAM
UPLIFT HARDWARE PER ROOF TRUSS UPLIFT SHEET
4"
STRAP EACH END - SEE SCHEDULE
TRIMMER STUDS OR SOLID POST - SEE PLAN / GENERAL NOTES (TYP)
KINGPOST (TYP)

3 Roof Flush Beam Tie-Down Enlarged Detail
1" = 1'-0"



ROOF TRUSS
TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
DOUBLE TOP PLATE
UPLIFT HARDWARE PER ROOF TRUSS UPLIFT SHEET
4"
TRIMMER STUDS OR SOLID POST - SEE PLAN / GENERAL NOTES (TYP)
HEADER
INFILL FRAMING AS REQUIRED
ROOF CLIPS AT HEADER - SEE SCHEDULE
KINGPOST (TYP)

2 Roof Deep Header Tie-Down Enlarged Detail
1" = 1'-0"



TAKE UP DEVICE - SEE SCHEDULE
ROOF TRUSS
DOUBLE TOP PLATE
TRIMMER STUDS OR SOLID POST - SEE PLAN / GENERAL NOTES (TYP)
4"
2 S5.421 HEADER
3 S5.421 FLUSH BEAM
TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
SILL PLATE AT WINDOW
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE (TYP)
TIE DOWN ROD - SEE SCHEDULE
KINGPOST (TYP)

ROOF BEAM / DEEP HEADER UPLIFT TIE-DOWN SCHEDULE

ROUGH OPENING WIDTH [2] (R.O.)	ROOF CLIPS AT HEADER [3]	ROOF STRAPS AT FLUSH BEAM ENDS [8]	TIE-DOWN ROD SIZE [4]	EMBEDMENT INTO CONCRETE [5, 7]	MIN EDGE DISTANCE [6]	TAKE-UP DEVICE [4]	BEARING PLATE
R.O. < 3'-11"	DSP @ 16" O.C.	(1) CS16 STRAP					
4'-0" < R.O. < 7'-11"	DSP @ 16" O.C.	(2) CS16 STRAPS	ATR3/8	-	-	RTUD3B	BPRTUD3-4B
8'-0" < R.O. < 12'-0"	DSP @ 16" O.C.	(3) CS16 STRAPS	ATR1/2	6" MIN.	2 3/4"	RTUD4B	BPRTUD3-4B

ROOF HEADER UPLIFT TIE-DOWN SCHEDULE NOTES:

1. UPLIFT RESTRAINT IS PROVIDED BY SIMPSON STRONG-TIE "URS" SYSTEM AND SHALL BE INSTALLED PER MANUFACTURER REQUIREMENTS (TYPICAL).
2. R.O. OPENING WIDTH IS MEASURED AS CLEAR SPAN / ROUGHT OPENING WIDTH (TYPICAL).
3. ROOF HEADER CLIPS ARE MANUFACTURED BY SIMPSON STRONG-TIE AND SHALL BE INSTALLED PER MANUFACTURER REQUIREMENTS. FOR DFL LUMBER, SPACING MAY BE INCREASED TO 22" OC MAX.
4. TIE-DOWN RODS AND TAKE UP SHRINKAGE COMPENSATING DEVICES TO BE SIMPSON STRONG ROD MODEL AS NOTED. DO NOT PROVIDE FIRE CAULKING IN 2X PLATES AT TAKE UP SHRINKAGE COMPENSATING DEVICES.
5. ANCHORAGE TO CONCRETE SHALL BE CAST IN PLACE INTO FOUNDATION OR PODIUM WITH 7" MINIMUM EMBEDMENT WITH HEAVY HEX NUTS FITTED AT EMBEDDED END AND TACK WELDED TO ROD. A MINIMUM 2 3/4" CONCRETE EDGE DISTANCE FROM CENTER OF ROD. CORNER CONDITIONS ARE NOT PERMITTED WITH EDGE DISTANCES SHOWN. (AT CONTRACTORS OPTION, AT NON-PT LOCATIONS ATR RODS MAY BE DRILL AND EPOXY THREADED RODS IN HILTI HY-200 V3 ADHESIVE INSTALLED PER MANUFACTURER REQUIREMENTS.)
6. MIN EDGE DISTANCE SHALL BE MEASURED FROM CENTER OF ROD ANCHOR TO NEAREST CONCRETE EDGE (TYPICAL).
7. FOR BUILDINGS WITH WOOD FRAMING MORE THAN 4 STORIES IN HEIGHT; OPENINGS LESS THAN 8'-0" IN WIDTH, ATR TIE-DOWN RODS NEED ONLY EXTEND DOWN 3 LEVELS BELOW ROOF LEVEL WITH SCHEDULED SIMPSON WBP AND NUT BELOW DOUBLE TOP PLATE.
8. STRAPPING MAY BE PROVIDED ON EITHER SIDE OF WALL BUT IS ONLY REQUIRED TO BE PROVIDED ON ONE SIDE OF WALL.

1 Roof Header Uplift Tie-Down
1/2" = 1'-0"

PROJECT # : 26154
DRAWN BY: Author
CHECKED BY: Checker

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SHEET TITLE:
Roof Truss Uplift Details

SHEET NUMBER:
S5.421

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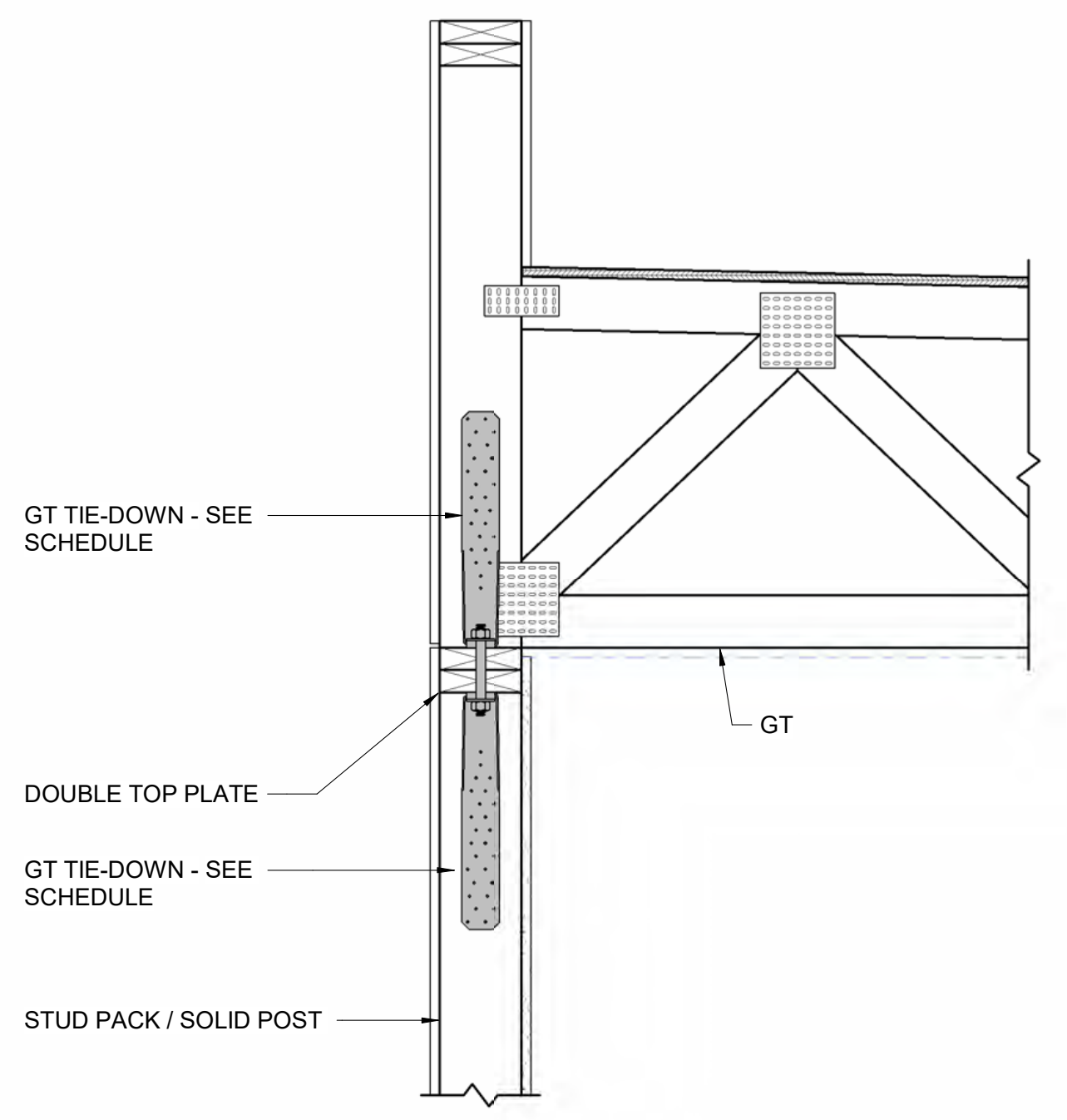
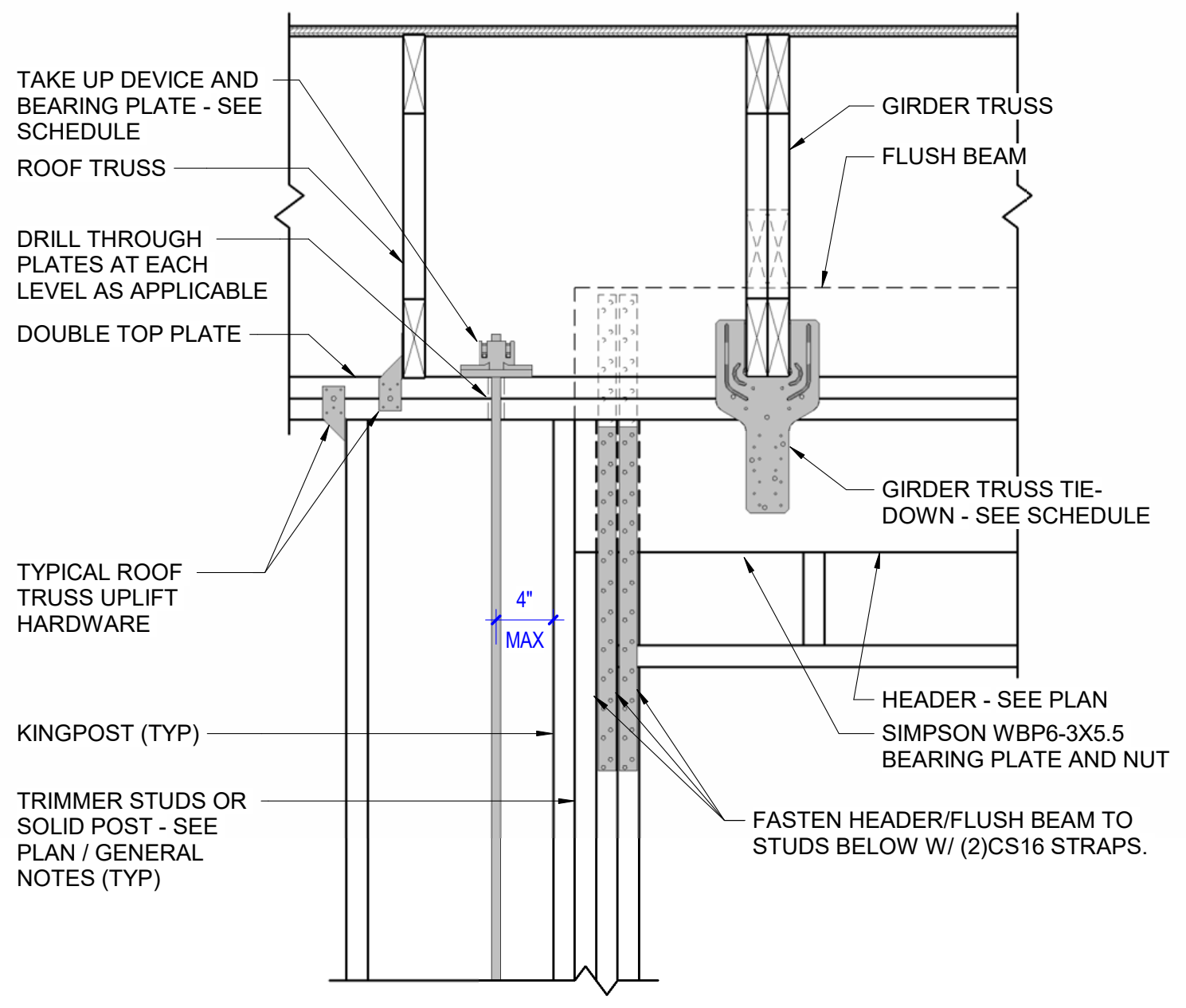
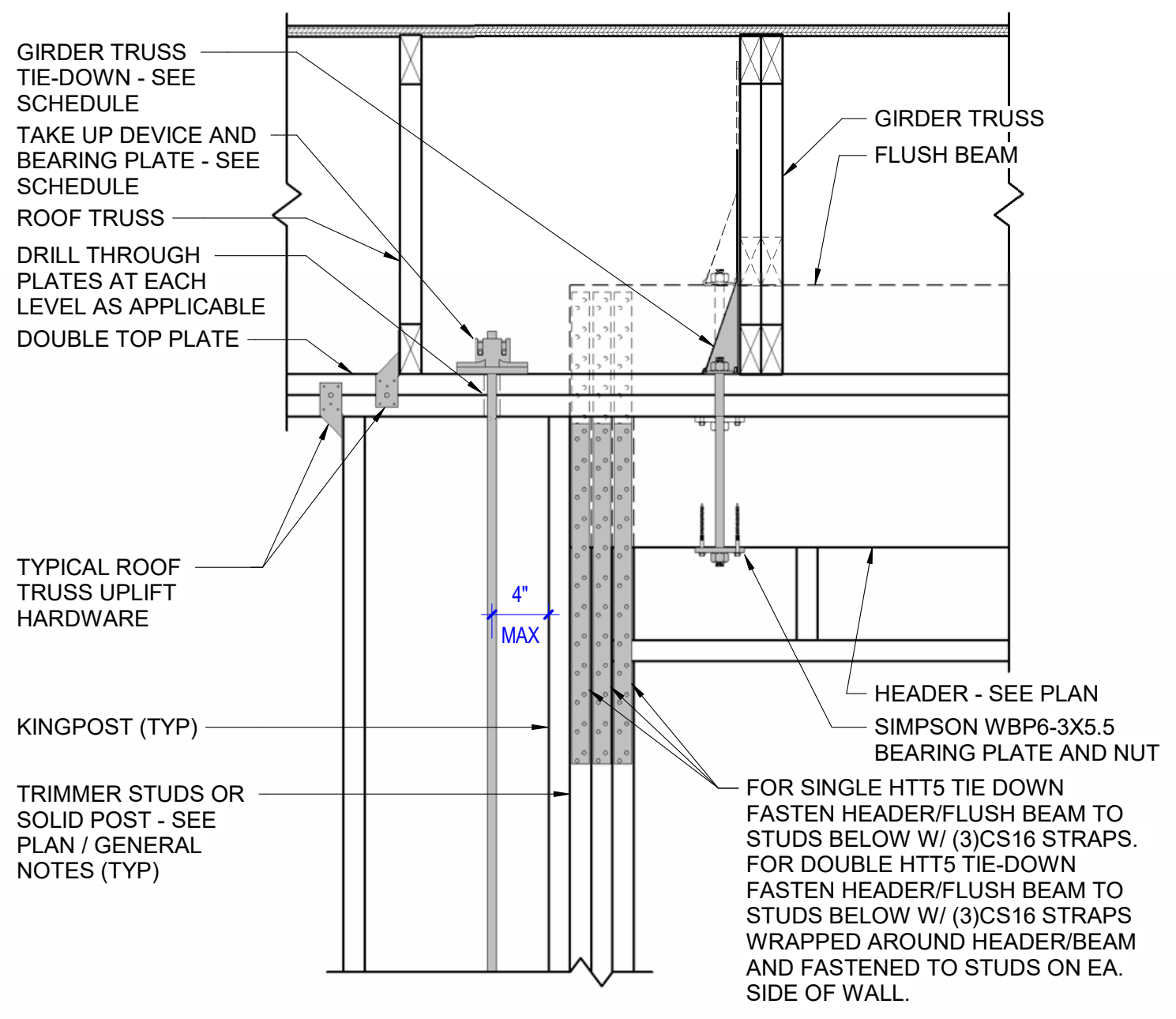


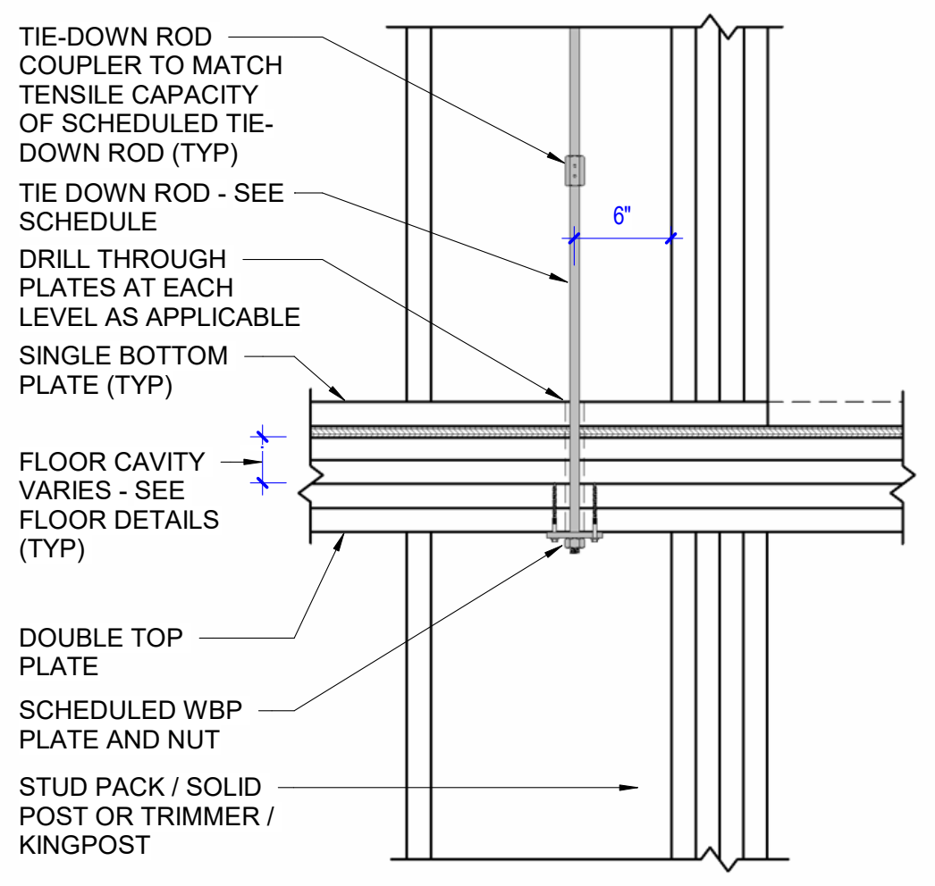
6 Girder Truss Uplift Detail - Flat Roof
1" = 1'-0"


TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
ROOF TRUSS
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
DOUBLE TOP PLATE
TYPICAL ROOF TRUSS UPLIFT HARDWARE
4" MAX
KINGPOST (TYP)
TRIMMER STUDS OR SOLID POST - SEE PLAN / GENERAL NOTES (TYP)
GIRDER TRUSS
FLUSH BEAM
GIRDER TRUSS TIE-DOWN - SEE SCHEDULE
HEADER - SEE PLAN
SIMPSON WBP6-3X5.5 BEARING PLATE AND NUT
FASTEN HEADER/FLUSH BEAM TO STUDS BELOW W/ (2)CS16 STRAPS.

5 LGT Tie-Down Over Header / Flush Beam
1" = 1'-0"

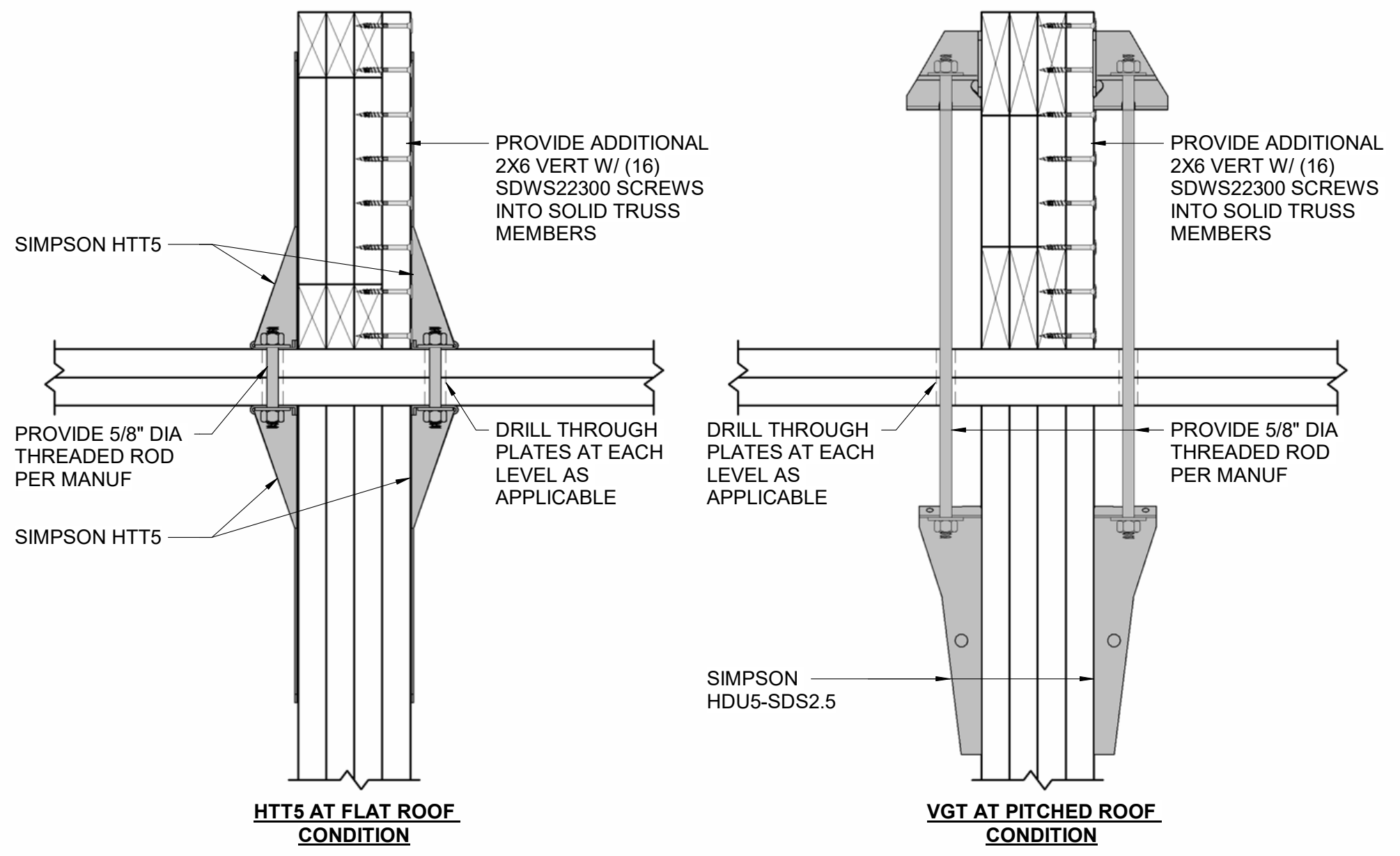

GIRDER TRUSS TIE-DOWN - SEE SCHEDULE
TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
ROOF TRUSS
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
DOUBLE TOP PLATE
TYPICAL ROOF TRUSS UPLIFT HARDWARE
4" MAX
KINGPOST (TYP)
TRIMMER STUDS OR SOLID POST - SEE PLAN / GENERAL NOTES (TYP)
GIRDER TRUSS
FLUSH BEAM
HEADER - SEE PLAN
SIMPSON WBP6-3X5.5 BEARING PLATE AND NUT
FOR SINGLE HTT5 TIE DOWN FASTEN HEADER/FLUSH BEAM TO STUDS BELOW W/ (3)CS16 STRAPS. FOR DOUBLE HTT5 TIE-DOWN FASTEN HEADER/FLUSH BEAM TO STUDS BELOW W/ (3)CS16 STRAPS WRAPPED AROUND HEADER/BEAM AND FASTENED TO STUDS ON EA. SIDE OF WALL.

4 HTT5 Tie-Down Over Header / Flush Beam
1" = 1'-0"


TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
TIE DOWN ROD - SEE SCHEDULE
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE
SCHEDULED WBP PLATE AND NUT
STUD PACK / SOLID POST OR TRIMMER / KINGPOST
6"

WBP SCHEDULE		
TIE-DOWN ROD SIZE	SIMPSON WOOD-BEAM PLATE (WBP)	NUT
ATR3/8	WBP4-3X3.5	3/8"Ø HEAVY HEX
ATR1/2	WBP5-3X3.5	1/2"Ø HEAVY HEX

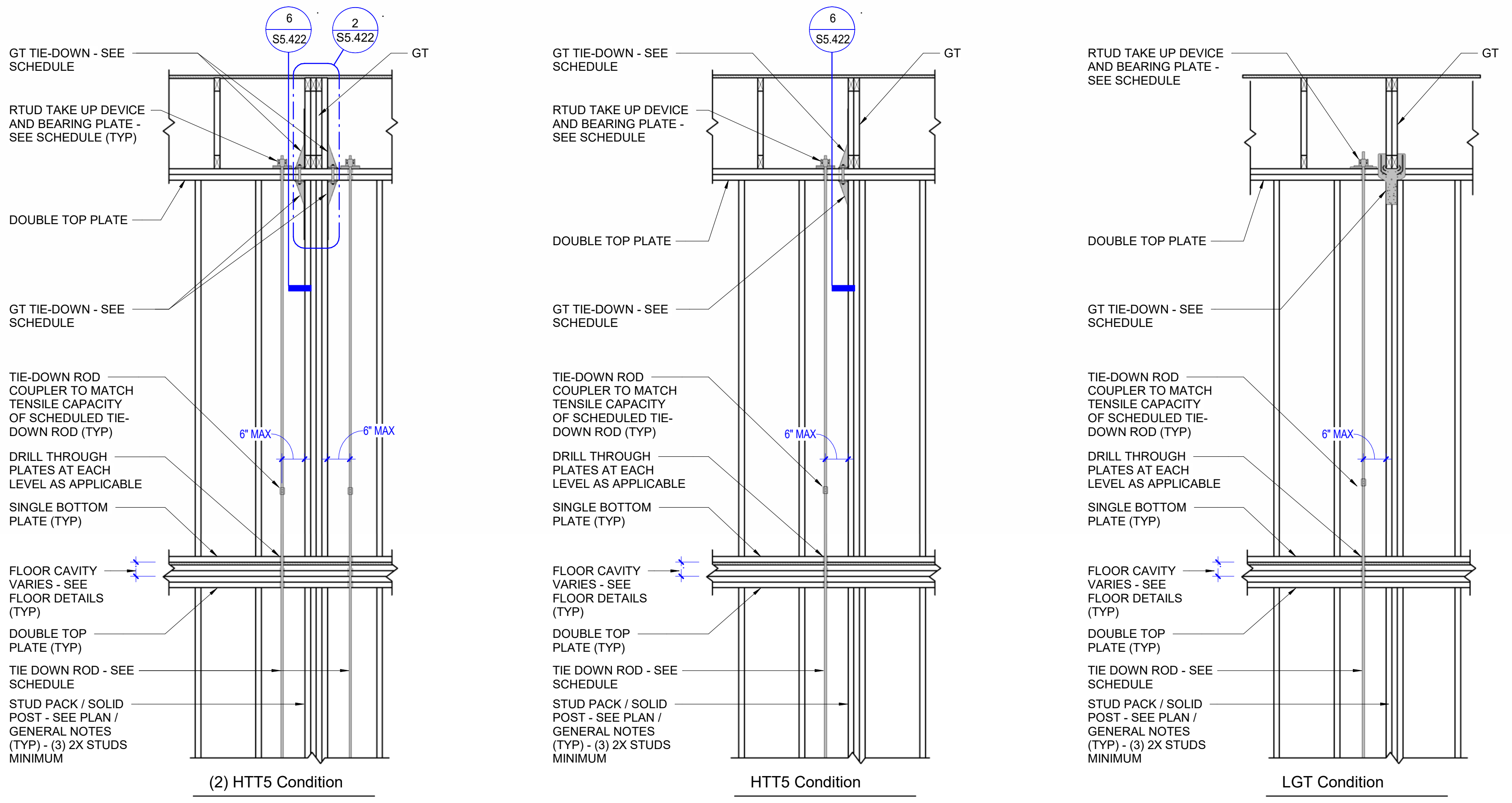
3 Roof Tie-Down Rod Termination at Plate
1" = 1'-0"


SIMPSON HTT5
PROVIDE ADDITIONAL 2X6 VERT W/ (16) SDWS22300 SCREWS INTO SOLID TRUSS MEMBERS
PROVIDE 5/8" DIA THREADED ROD PER MANUF
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SIMPSON HTT5
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
PROVIDE ADDITIONAL 2X6 VERT W/ (16) SDWS22300 SCREWS INTO SOLID TRUSS MEMBERS
PROVIDE 5/8" DIA THREADED ROD PER MANUF
SIMPSON HDU5-SDS2.5

HTT5 AT FLAT ROOF CONDITION

VGT AT PITCHED ROOF CONDITION

2 Girder Truss Uplift Detail - Double Tie-Down
1 1/2" = 1'-0"


6 S5.422
2 S5.422
GT TIE-DOWN - SEE SCHEDULE
RTUD TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE (TYP)
DOUBLE TOP PLATE
GT TIE-DOWN - SEE SCHEDULE
TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE (TYP)
TIE DOWN ROD - SEE SCHEDULE
STUD PACK / SOLID POST - SEE PLAN / GENERAL NOTES (TYP) - (3) 2X STUDS MINIMUM
GT
6" MAX
6" MAX
6 S5.422
GT TIE-DOWN - SEE SCHEDULE
RTUD TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
DOUBLE TOP PLATE
GT TIE-DOWN - SEE SCHEDULE
TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE (TYP)
TIE DOWN ROD - SEE SCHEDULE
STUD PACK / SOLID POST - SEE PLAN / GENERAL NOTES (TYP) - (3) 2X STUDS MINIMUM
GT
6" MAX
RTUD TAKE UP DEVICE AND BEARING PLATE - SEE SCHEDULE
DOUBLE TOP PLATE
GT TIE-DOWN - SEE SCHEDULE
TIE-DOWN ROD COUPLER TO MATCH TENSILE CAPACITY OF SCHEDULED TIE-DOWN ROD (TYP)
DRILL THROUGH PLATES AT EACH LEVEL AS APPLICABLE
SINGLE BOTTOM PLATE (TYP)
FLOOR CAVITY VARIES - SEE FLOOR DETAILS (TYP)
DOUBLE TOP PLATE (TYP)
TIE DOWN ROD - SEE SCHEDULE
STUD PACK / SOLID POST - SEE PLAN / GENERAL NOTES (TYP) - (3) 2X STUDS MINIMUM
GT
6" MAX

(2) HTT5 Condition

HTT5 Condition

LGT Condition

GIRDER TRUSS (GT) UPLIFT TIE-DOWN SCHEDULE[1] AT FLAT ROOF					
G.T. UPLIFT REACTION [10]	NUMBER OF TRUSS PLIES	TIE-DOWN EACH BEARING POINT [7, 8, 9]	TIE-DOWN ROD SIZE [3]	TAKE-UP DEVICE [6]	BEARING PLATE
0-700 LBS	1 OR MORE	H2.5A	-	-	-
701 - 2,124 LBS	2	LGT2 - #9	ATR3/8[4]	RTUD3B	BPRTUD3-4B
2,124 - 2,505 LBS	3 OR 4	LGT3-SDS2.5 / LGT4-SDS3	ATR1/2[5]	RTUD4B	BPRTUD3-4B
2,506 - 4,375 LBS	2 OR MORE	HTT5 @ GT & STUD PACK / POST	ATR5/8	RTUD5	BPRTUD5-6B
4,376 - 8,680 LBS	3 OR MORE	(2) HTT5 @ GT & STUD PACK / POST	(2) ATR5/8	(2) RTUD5	(2) BPRTUD5-6B

GT UPLIFT TIE-DOWN SCHEDULE NOTES:

1. UPLIFT RESTRAINT IS PROVIDED BY SIMPSON STRONG-TIE "URS" SYSTEM AND SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS (TYPICAL). ROOF ANCHOR TYPES ARE MANUFACTURED BY SIMPSON STRONG-TIE AND SHALL BE INSTALLED PER MANUFACTURER REQUIREMENTS, TYP.
2. TIE-DOWN RODS, TAKE-UP SHRINKAGE COMPENSATING DEVICES AND BEARING PLATE MODELS BY SIMPSON STRONG-TIE AS NOTED.
3. ATR RODS SHALL BE CAST IN PLACE INTO FOUNDATION OR PODIUM WITH 7" MINIMUM EMBEDMENT WITH HEAVY HEX NUTS FITTED AT EMBEDDED END AND TACK WELDED TO ROD. A MINIMUM 2 3/4" CONCRETE EDGE DISTANCE FROM CENTER OF ROD. CORNER CONDITIONS ARE NOT PERMITTED WITH EDGE DISTANCES SHOWN. (AT CONTRACTORS OPTION, AT NON-PT LOCATIONS ATR RODS MAY BE DRILL AND EPOXY THREADED RODS IN HILTI HY-200 V3 ADHESIVE INSTALLED PER MANUFACTURER REQUIREMENTS.)
4. FOR BUILDINGS WITH WOOD FRAMING MORE THAN 3 STORIES IN HEIGHT, ATR3/8 TIE-DOWN RODS NEED ONLY EXTEND DOWN 3 LEVELS BELOW ROOF LEVEL WITH SCHEDULED SIMPSON WBP AND NUT BELOW DOUBLE TOP PLATE. SEE DETAIL 3 / S5.422
5. FOR BUILDINGS WITH WOOD FRAMING MORE THAN 4 STORIES IN HEIGHT, ATR1/2 TIE-DOWN RODS NEED ONLY EXTEND DOWN 4 LEVELS BELOW ROOF LEVEL WITH SCHEDULED SIMPSON WBP AND NUT BELOW DOUBLE TOP PLATE. SEE DETAIL 3 / S5.422
6. DO NOT PROVIDE FIRE CAULKING IN 2X PLATES AT TAKE-UP SHRINKAGE COMPENSATING DEVICES (TYPICAL).
7. LGT2 AND LGT3 TIE-DOWNS TO BE INSTALLED WITH #9 SD / SDS SCREWS PER MANUFACTURER. HTT5 TIES TO BE INSTALLED WITH 0.148"Ø x 3" LONG NAILS OR 0.162"Ø x 2 1/2" NAILS, U.N.O.
8. WHERE LGT TIE-DOWN OCCURS OVER HEADER OR FLUSH BEAM, SEE DETAIL 5 / S5.422
9. WHERE HTT5 OCCURS OVER HEADER OR FLUSH BEAM, SEE DETAIL 4 / S5.422
10. G.T. UPLIFT REACTIONS ARE INDICATED ON TRUSS DESIGN DRAWINGS PROVIDED BY THE WOOD TRUSS MANUFACTURER IN ACCORDANCE WITH THE WOOD GENERAL NOTES.

1 Girder Truss Uplift Detail - Flat Roof
1/2" = 1'-0"

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Structural Engineers
VSMsq Structural Engineers, LLC
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PROJECT # : 26154
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No.	Description	Date
	100% DD	09/30/2026

BURBANK DR - BATON ROUGE, LA

BATON ROUGE, LOUISIANA 70808
EAST BATON ROUGE PARISH

LV COLLECTIVE

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- MARTIN R. MAINGOT

LICENSE NO STATE OF LOUISIANA:
- NO. 85617

SHEET TITLE:
ROOF GIRDER TRUSS UPLIFT DETAILS - FLAT ROOF

SHEET NUMBER:
S5.422

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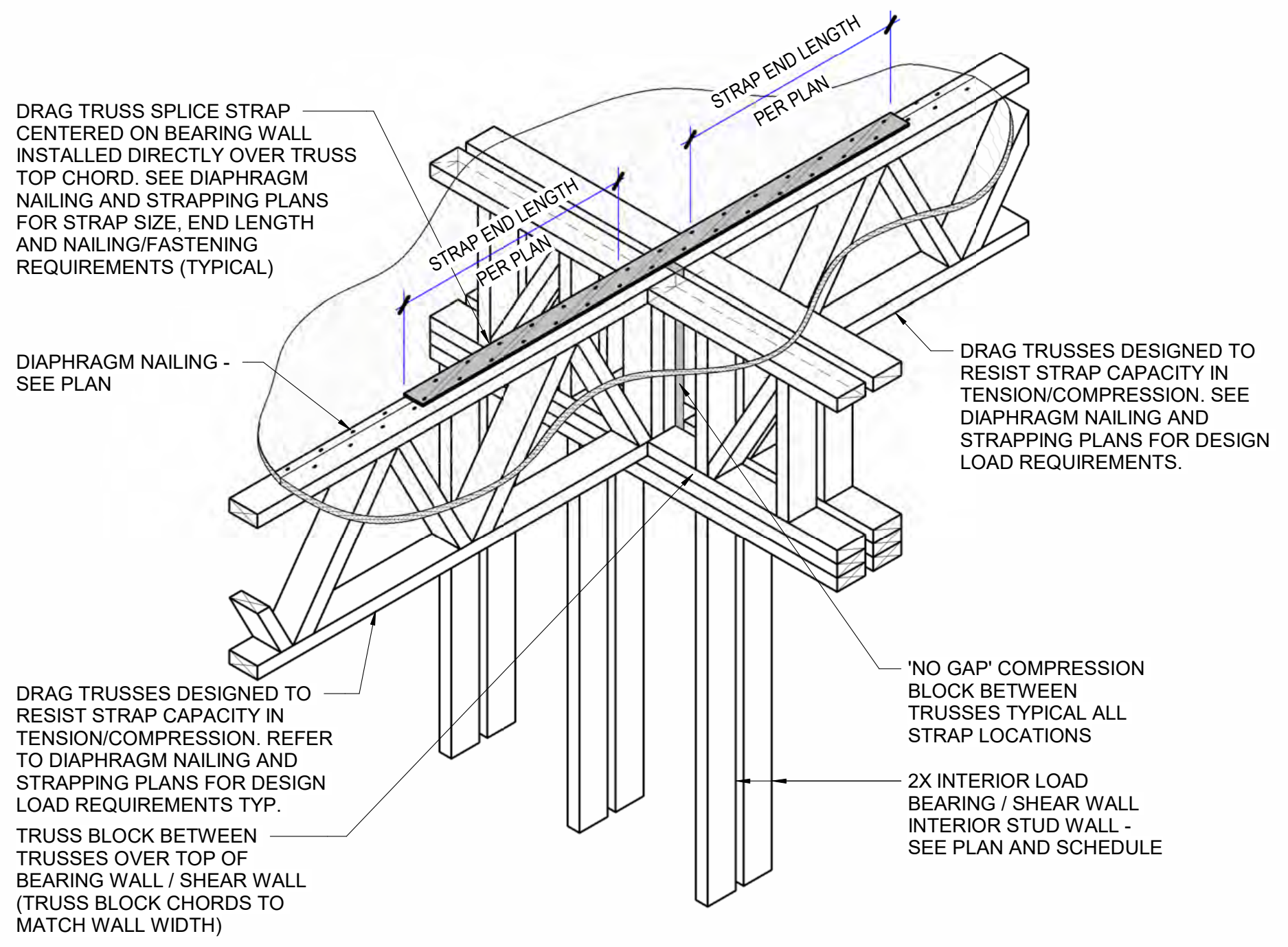
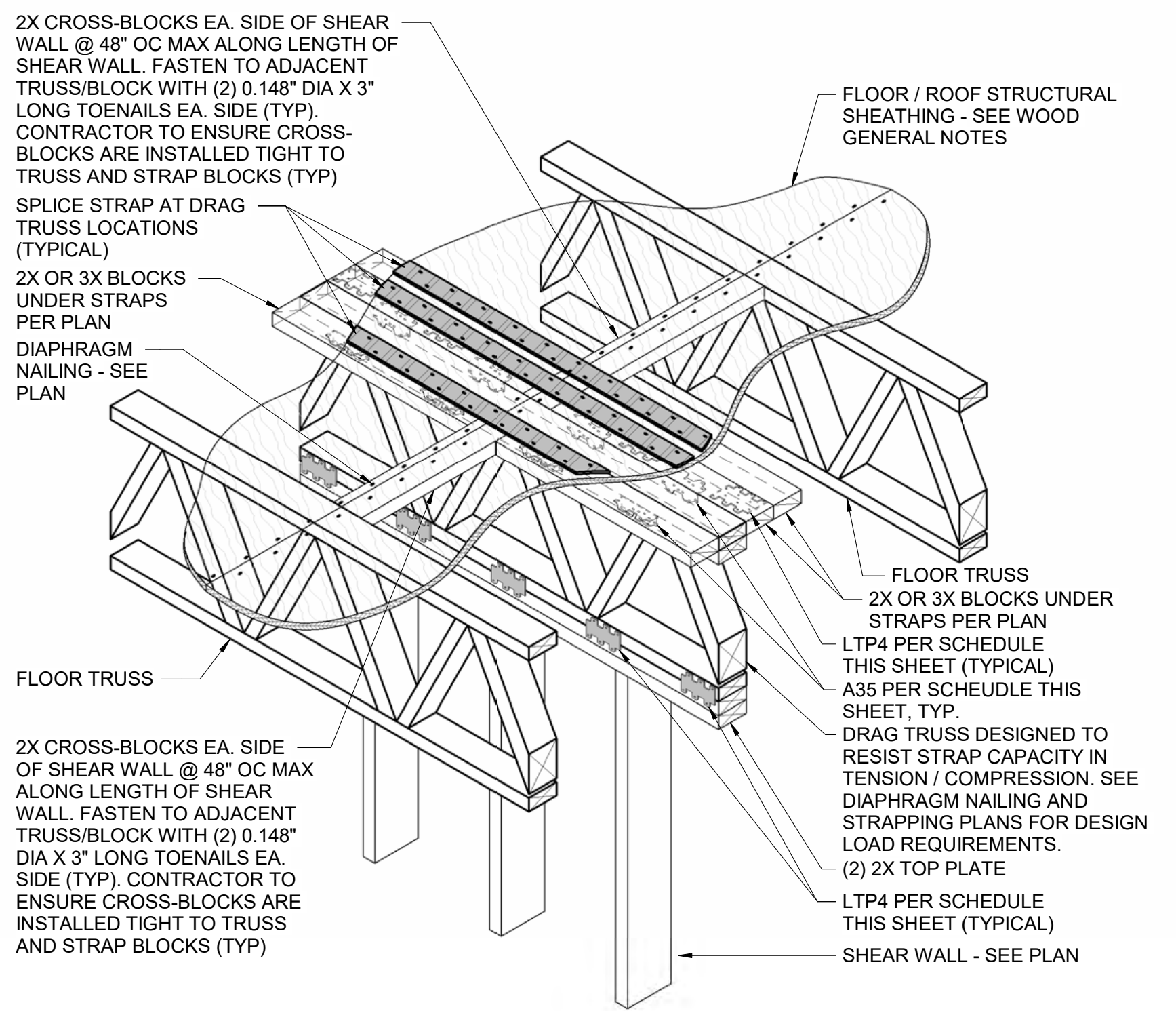
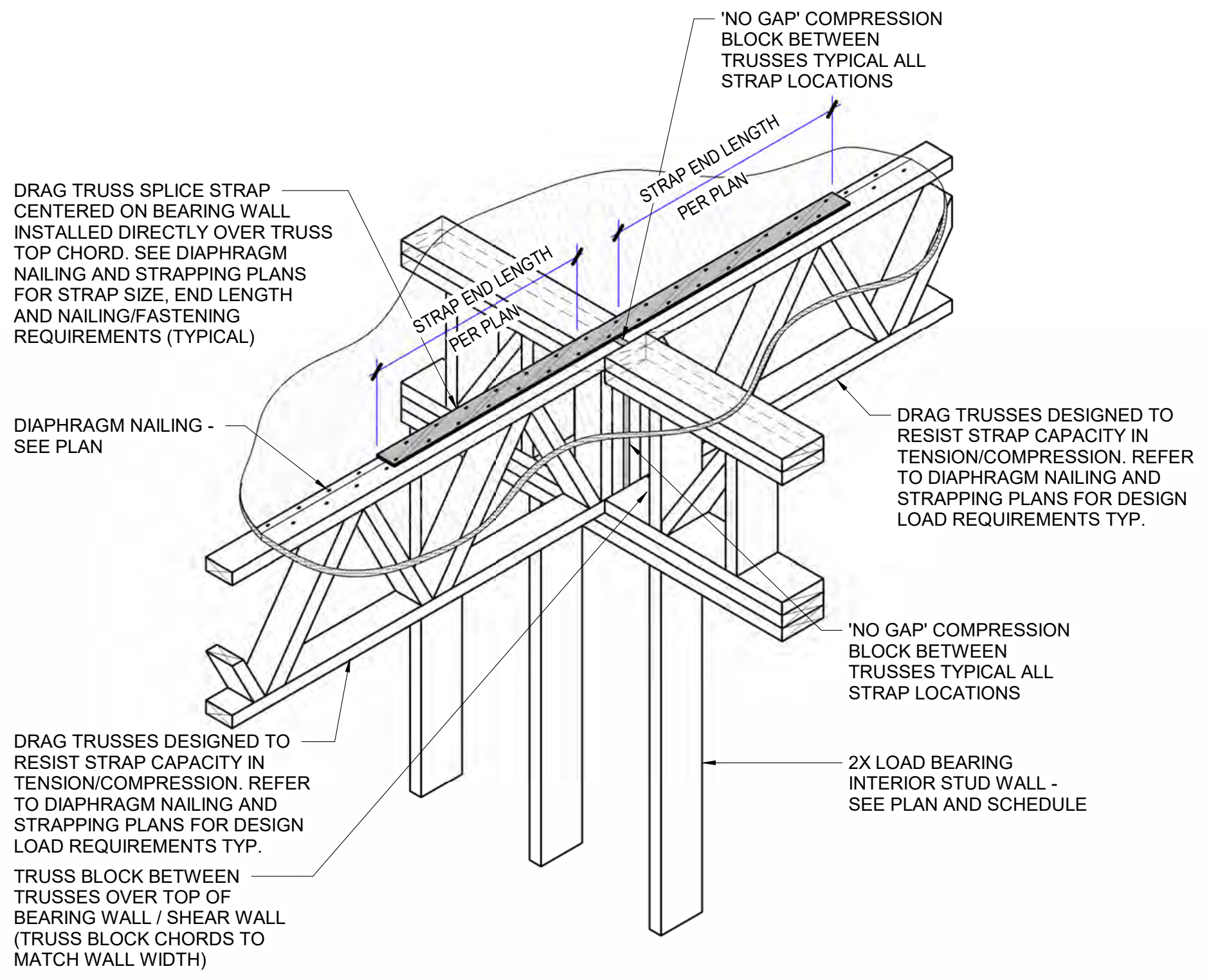

DRAG TRUSS SPLICE STRAP CENTERED ON BEARING WALL INSTALLED DIRECTLY OVER TRUSS TOP CHORD. SEE DIAPHRAGM NAILING AND STRAPPING PLANS FOR STRAP SIZE, END LENGTH AND NAILING/FASTENING REQUIREMENTS (TYPICAL)
STRAP END LENGTH PER PLAN
STRAP END LENGTH PER PLAN
DIAPHRAGM NAILING - SEE PLAN
DRAG TRUSSES DESIGNED TO RESIST STRAP CAPACITY IN TENSION/COMPRESSION. SEE DIAPHRAGM NAILING AND STRAPPING PLANS FOR DESIGN LOAD REQUIREMENTS.
'NO GAP' COMPRESSION BLOCK BETWEEN TRUSSES TYPICAL ALL STRAP LOCATIONS
DRAG TRUSSES DESIGNED TO RESIST STRAP CAPACITY IN TENSION/COMPRESSION. REFER TO DIAPHRAGM NAILING AND STRAPPING PLANS FOR DESIGN LOAD REQUIREMENTS TYP.
2X INTERIOR LOAD BEARING / SHEAR WALL INTERIOR STUD WALL - SEE PLAN AND SCHEDULE
TRUSS BLOCK BETWEEN TRUSSES OVER TOP OF BEARING WALL / SHEAR WALL (TRUSS BLOCK CHORDS TO MATCH WALL WIDTH)

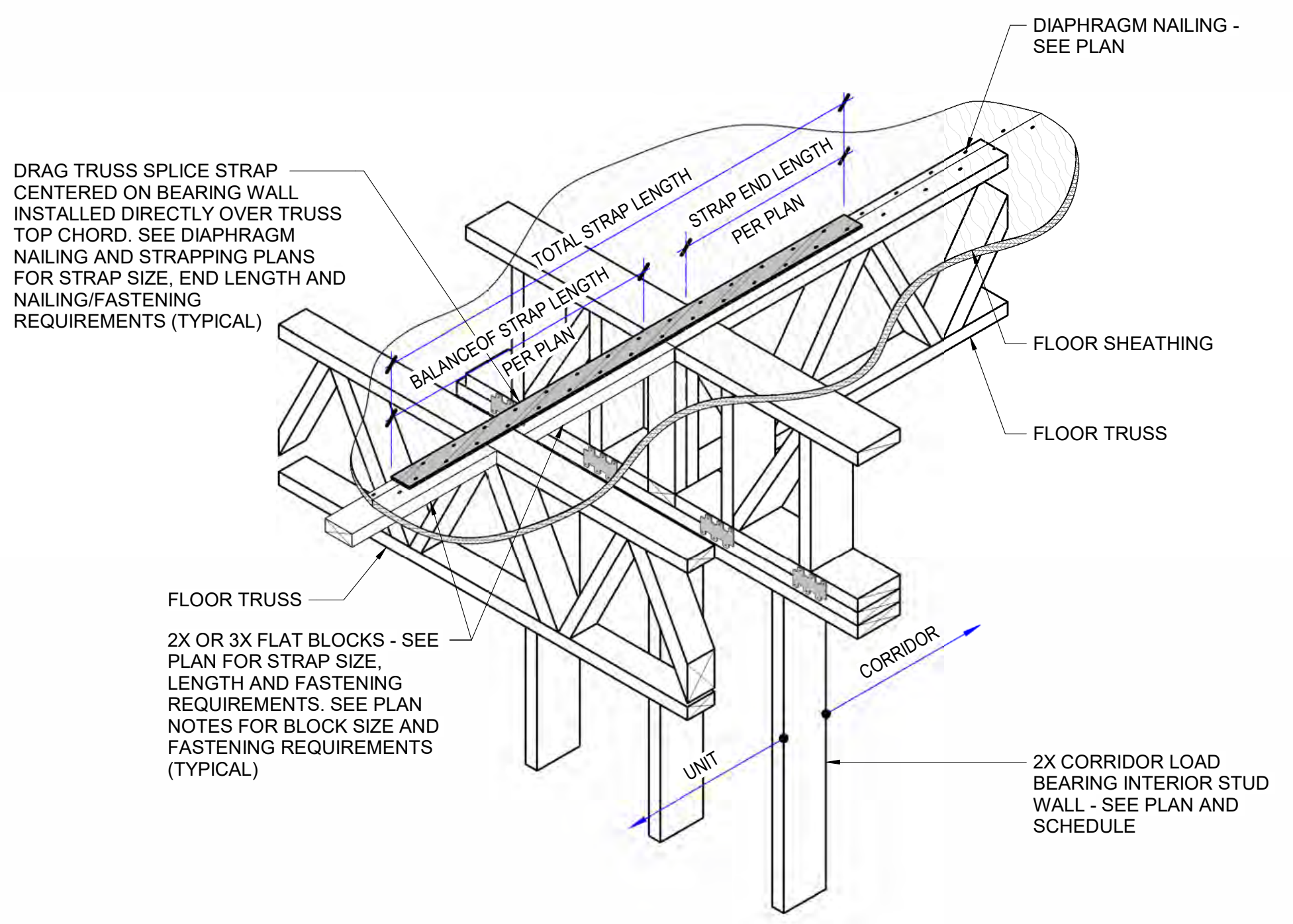
6 Drag Truss Splice Strap at Interior Bearing / Shear Wall
3/4" = 1'-0"


2X CROSS-BLOCKS EA. SIDE OF SHEAR WALL @ 48" OC MAX ALONG LENGTH OF SHEAR WALL. FASTEN TO ADJACENT TRUSS/BLOCK WITH (2) 0.148" DIA X 3" LONG TOENAILS EA. SIDE (TYP). CONTRACTOR TO ENSURE CROSS-BLOCKS ARE INSTALLED TIGHT TO TRUSS AND STRAP BLOCKS (TYP)
FLOOR / ROOF STRUCTURAL SHEATHING - SEE WOOD GENERAL NOTES
SPLICE STRAP AT DRAG TRUSS LOCATIONS (TYPICAL)
2X OR 3X BLOCKS UNDER STRAPS PER PLAN
DIAPHRAGM NAILING - SEE PLAN
FLOOR TRUSS
2X OR 3X BLOCKS UNDER STRAPS PER PLAN
LTP4 PER SCHEDULE THIS SHEET (TYPICAL)
A35 PER SCHEUDLE THIS SHEET, TYP.
FLOOR TRUSS
DRAG TRUSS DESIGNED TO RESIST STRAP CAPACITY IN TENSION / COMPRESSION. SEE DIAPHRAGM NAILING AND STRAPPING PLANS FOR DESIGN LOAD REQUIREMENTS.
(2) 2X TOP PLATE
2X CROSS-BLOCKS EA. SIDE OF SHEAR WALL @ 48" OC MAX ALONG LENGTH OF SHEAR WALL. FASTEN TO ADJACENT TRUSS/BLOCK WITH (2) 0.148" DIA X 3" LONG TOENAILS EA. SIDE (TYP). CONTRACTOR TO ENSURE CROSS-BLOCKS ARE INSTALLED TIGHT TO TRUSS AND STRAP BLOCKS (TYP)
LTP4 PER SCHEDULE THIS SHEET (TYPICAL)
SHEAR WALL - SEE PLAN

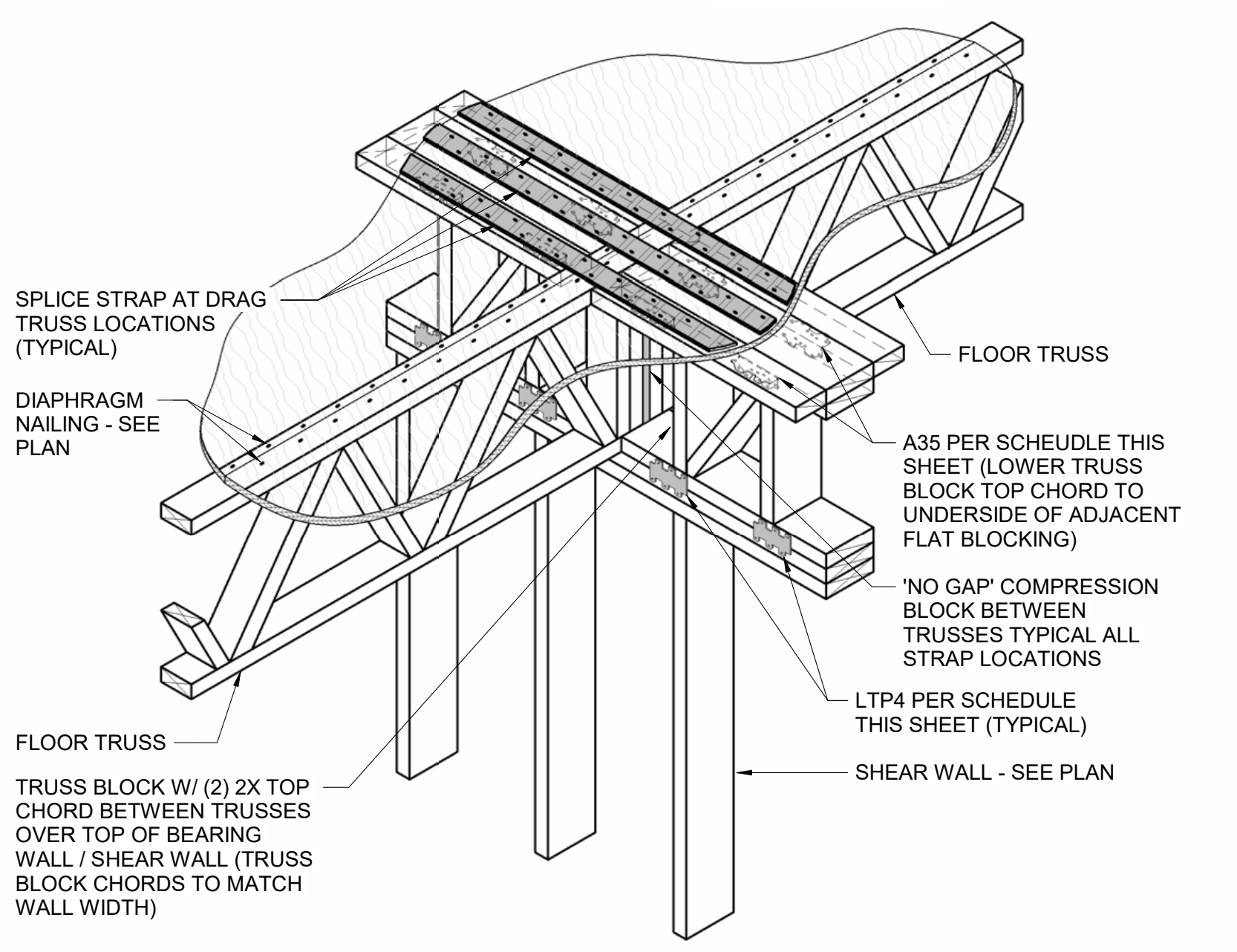
5 Strapping Detail at Interior Wall Parallel to Truss
3/4" = 1'-0"


'NO GAP' COMPRESSION BLOCK BETWEEN TRUSSES TYPICAL ALL STRAP LOCATIONS
STRAP END LENGTH PER PLAN
DRAG TRUSS SPLICE STRAP CENTERED ON BEARING WALL INSTALLED DIRECTLY OVER TRUSS TOP CHORD. SEE DIAPHRAGM NAILING AND STRAPPING PLANS FOR STRAP SIZE, END LENGTH AND NAILING/FASTENING REQUIREMENTS (TYPICAL)
STRAP END LENGTH PER PLAN
DIAPHRAGM NAILING - SEE PLAN
DRAG TRUSSES DESIGNED TO RESIST STRAP CAPACITY IN TENSION/COMPRESSION. REFER TO DIAPHRAGM NAILING AND STRAPPING PLANS FOR DESIGN LOAD REQUIREMENTS TYP.
'NO GAP' COMPRESSION BLOCK BETWEEN TRUSSES TYPICAL ALL STRAP LOCATIONS
DRAG TRUSSES DESIGNED TO RESIST STRAP CAPACITY IN TENSION/COMPRESSION. REFER TO DIAPHRAGM NAILING AND STRAPPING PLANS FOR DESIGN LOAD REQUIREMENTS TYP.
2X LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE
TRUSS BLOCK BETWEEN TRUSSES OVER TOP OF BEARING WALL / SHEAR WALL (TRUSS BLOCK CHORDS TO MATCH WALL WIDTH)

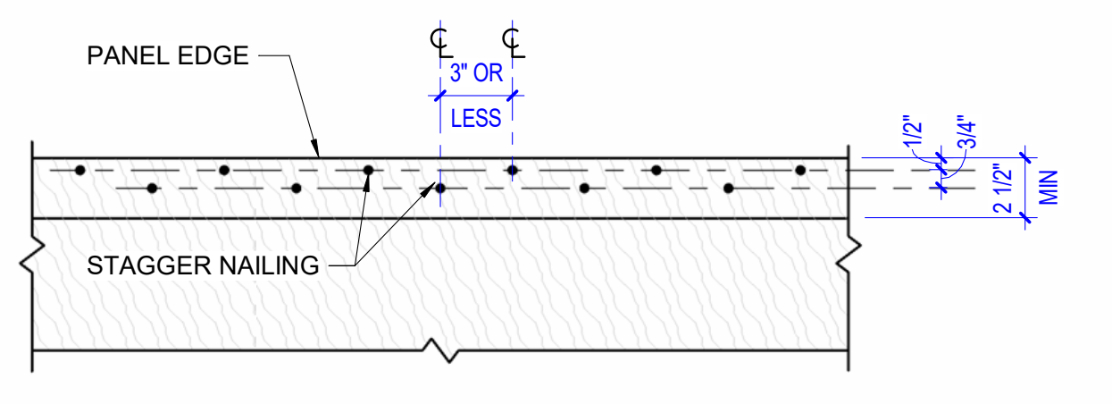
4 Drag Truss Splice Strap at Interior Bearing / Shear Wall
3/4" = 1'-0"


DIAPHRAGM NAILING - SEE PLAN
DRAG TRUSS SPLICE STRAP CENTERED ON BEARING WALL INSTALLED DIRECTLY OVER TRUSS TOP CHORD. SEE DIAPHRAGM NAILING AND STRAPPING PLANS FOR STRAP SIZE, END LENGTH AND NAILING/FASTENING REQUIREMENTS (TYPICAL)
TOTAL STRAP LENGTH
STRAP END LENGTH PER PLAN
BALANCEOF STRAP LENGTH PER PLAN
FLOOR SHEATHING
FLOOR TRUSS
FLOOR TRUSS
2X OR 3X FLAT BLOCKS - SEE PLAN FOR STRAP SIZE, LENGTH AND FASTENING REQUIREMENTS. SEE PLAN NOTES FOR BLOCK SIZE AND FASTENING REQUIREMENTS (TYPICAL)
CORRIDOR
UNIT
2X CORRIDOR LOAD BEARING INTERIOR STUD WALL - SEE PLAN AND SCHEDULE

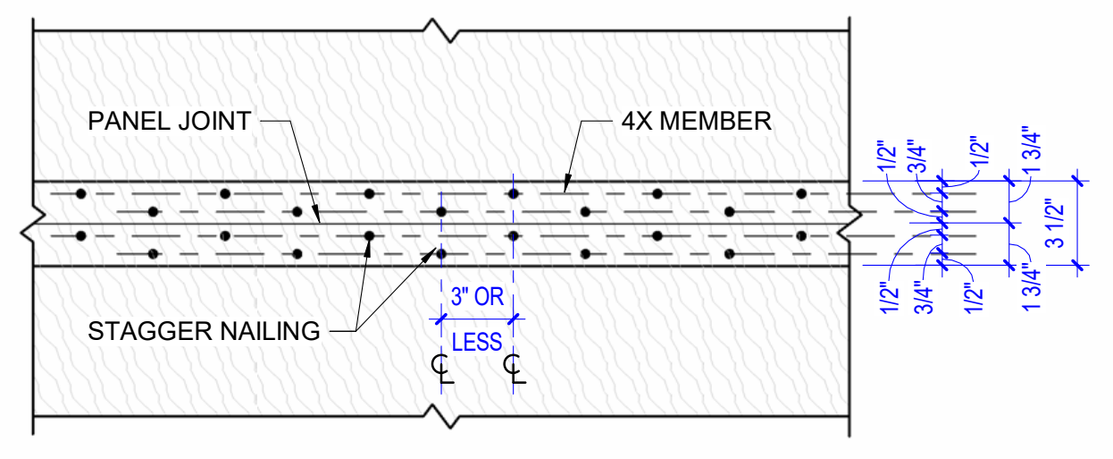
3 Discontinuous Strapping at Corridor
3/4" = 1'-0"


SPLICE STRAP AT DRAG TRUSS LOCATIONS (TYPICAL)
DIAPHRAGM NAILING - SEE PLAN
FLOOR TRUSS
A35 PER SCHEUDLE THIS SHEET (LOWER TRUSS BLOCK TOP CHORD TO UNDERSIDE OF ADJACENT FLAT BLOCKING)
'NO GAP' COMPRESSION BLOCK BETWEEN TRUSSES TYPICAL ALL STRAP LOCATIONS
LTP4 PER SCHEDULE THIS SHEET (TYPICAL)
FLOOR TRUSS
TRUSS BLOCK W/ (2) 2X TOP CHORD BETWEEN TRUSSES OVER TOP OF BEARING WALL / SHEAR WALL (TRUSS BLOCK CHORDS TO MATCH WALL WIDTH)
SHEAR WALL - SEE PLAN

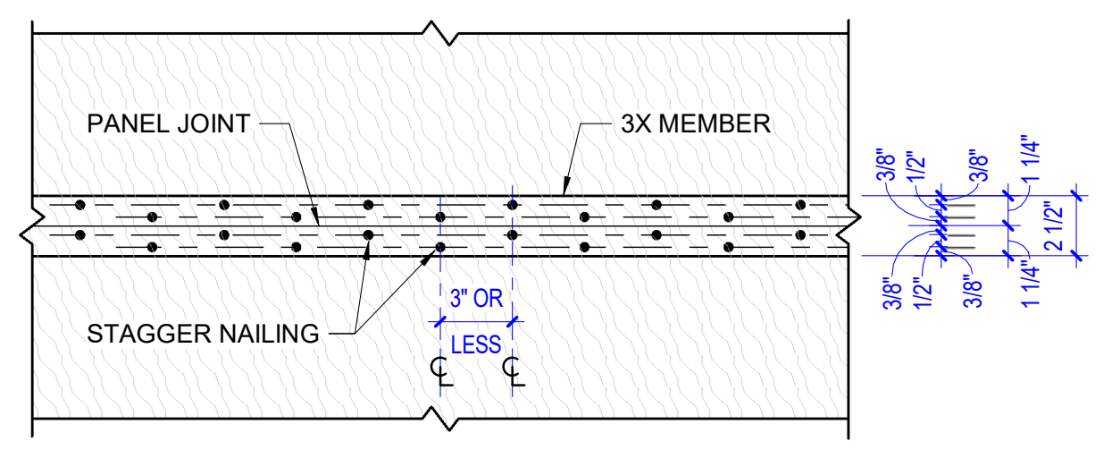
2 Strapping Detail at Interior Wall Perpendicular to Truss
3/4" = 1'-0"



NAILING AT BOUNDARY EDGE



NAILING AT 4" NOMINAL FRAMING MEMBER



NAILING AT 3" NOMINAL FRAMING MEMBER

DRAG STRUT AND DIAPHRAGM STRAP BLOCKING SCHEDULE

STRAP SIZE	A35 CLIPS TO SHEAR WALL (5 INSTALLATION)	LTP4 CLIPS BLOCK TO BLOCK (G INSTALLATION)	REQUIRED CONNECTION LENGTH	MAX DRAG FORCE (ASD) (LBS)
(1) CS14	4	-	1'-10"	2,490
(1) CMST14	10	-	4'-0"	6,475
(1) CMST12	15	-	6'-0"	9,215
(2) CMST14	(10) EA SIDE OF WALL	-	4'-0"	12,950
	(20) ONE SIDE OF WALL	10	8'-0"	12,950
(3) CMST14	(10) ONE SIDE +	-	-	19,425
	(20) OPPOSITE SIDE	10 AT DOUBLE BLOCK	8'-0"	19,425
(2) CMST12	(15) EA SIDE OF WALL	-	6'-0"	18,430
	(30) ONE SIDE OF WALL	13	12'-0"	18,430
(3) CMST12	(15) ONE SIDE +	-	-	27,645
	(30) OPPOSITE SIDE	13	12'-0"	27,645

1 Drag Strut-Diaphragm Schedule / Plywood Nailing Diagrams
3/4" = 1'-0"

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- NO. 85617

SHEET TITLE:
Wood Strapping Details

SHEET NUMBER:
S5.430

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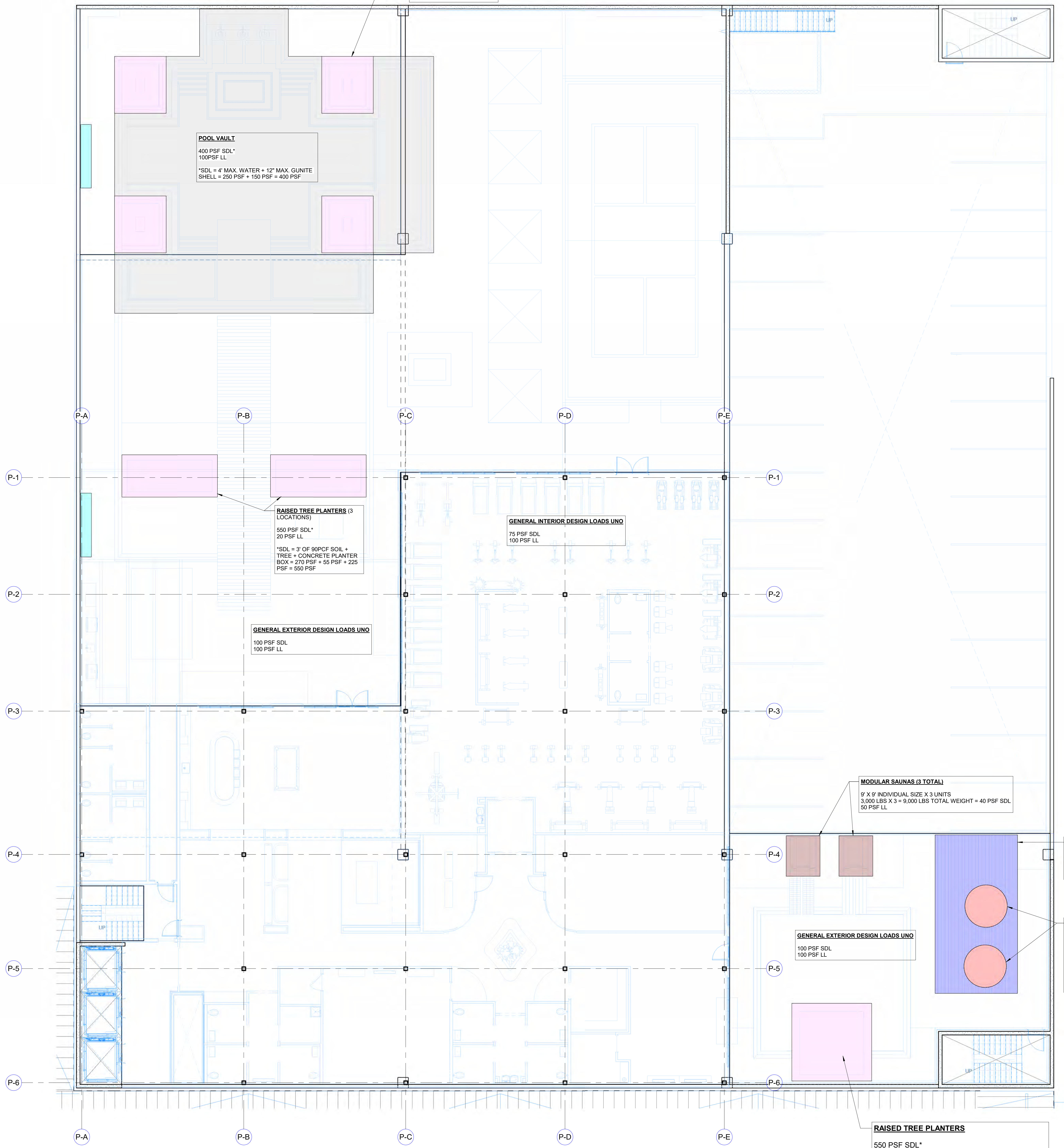


RAISED TREE PLANTERS (3 LOCATIONS)

550 PSF SDL*
20 PSF LL

*SDL = 3' OF 90PCF SOIL + TREE + CONCRETE PLANTER BOX = 270 PSF + 55 PSF + 225 PSF = 550 PSF

POOL VAULT

400 PSF SDL*
100PSF LL

*SDL = 4' MAX. WATER + 12" MAX. GUNITE SHELL = 250 PSF + 150 PSF = 400 PSF

RAISED TREE PLANTERS (3 LOCATIONS)

550 PSF SDL*
20 PSF LL

*SDL = 3' OF 90PCF SOIL + TREE + CONCRETE PLANTER BOX = 270 PSF + 55 PSF + 225 PSF = 550 PSF

GENERAL INTERIOR DESIGN LOADS UNO

75 PSF SDL
100 PSF LL

GENERAL EXTERIOR DESIGN LOADS UNO

100 PSF SDL
100 PSF LL

MODULAR SAUNAS (3 TOTAL)

9' X 9' INDIVIDUAL SIZE X 3 UNITS
3,000 LBS X 3 = 9,000 LBS TOTAL WEIGHT = 40 PSF SDL
50 PSF LL

COLD PLUNGE PLATFORM

50 PSF SDL*
50 PSF LL

*SDL = WOOD FRAMED PLATFORM

COLD PLUNGE (2 TOTAL)

225 PSF SDL*
50 PSF LL

*SDL = 9.5' DIAMETER TANK WITH 1500 GAL CAPACITY (3' MAX WATER DEPTH) + 2,500 LBS TANK WEIGHT + 500 MISC. EQUIPMENT = 180 PSF + 45 PSF = 225 PSF

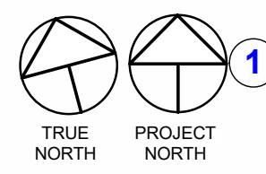
GENERAL EXTERIOR DESIGN LOADS UNO

100 PSF SDL
100 PSF LL

RAISED TREE PLANTERS

550 PSF SDL*
20 PSF LL

*SDL = 3' OF 90PCF SOIL + TREE + CONCRETE PLANTER BOX = 270 PSF + 55 PSF + 225 PSF = 550 PSF



① Loading Plan - Parking Level 07.5
1/8" = 1'-0"

9/30/2026 7:29:42 PM

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Structural Engineers

PROJECT # : 26154
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SHEET TITLE:
Loading Plan - Parking Level 07.5

SHEET NUMBER:
PS0.107

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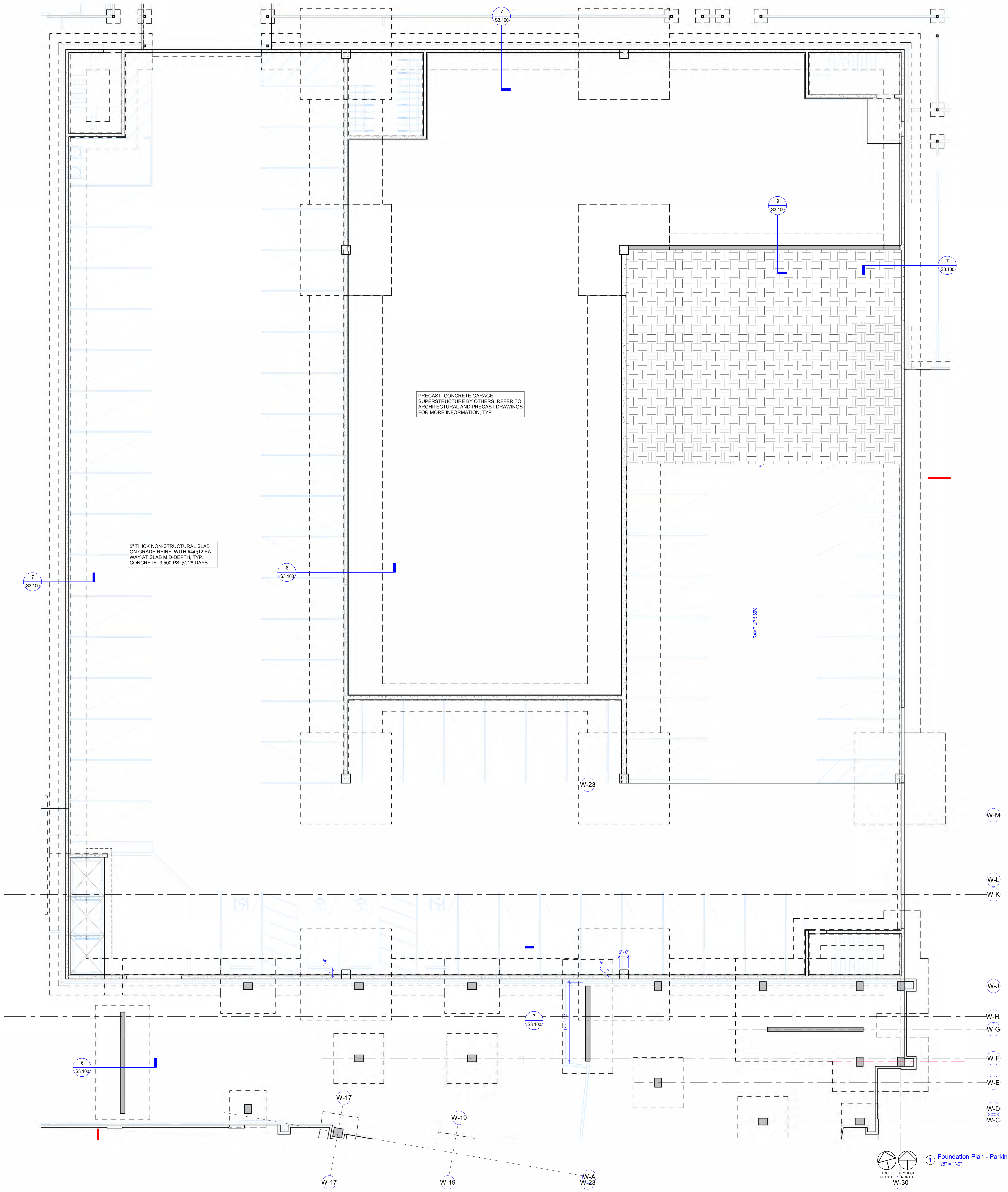

PRECAST CONCRETE GARAGE SUPERSTRUCTURE BY OTHERS, REFER TO ARCHITECTURAL AND PRECAST DRAWINGS FOR MORE INFORMATION, TYP.

5" THICK NON-STRUCTURAL SLAB ON GRADE REINF. WITH #4@12 EA. WAY AT SLAB MID-DEPTH, TYP. CONCRETE: 3,500 PSI @ 28 DAYS

1 Foundation Plan - Parking Building - Level 01
1/8" = 1'-0"

FOUNDATION NOTES:

1. CONTRACTOR TO CROSS REFERENCE AND/OR VERIFY ALL DIMENSIONS, ELEVATIONS, RECESSES DEPRESSIONS, CURB SIZES/LOCATIONS, WALL ELEVATIONS, GUARDRAILS, FINISHES, OPENINGS AND SLAB PENETRATION SIZES/LOCATIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ACHITECT AND ENGINEER OF ANY DISCREPANCIES PIOR TO CONSTRUCTION.
2. REFER TO ARCHITECTURAL SLAB DIMENSIONAL CONTROL PLAN FOR ALL SLAB EDGE AND OPENING DIMENSIONS NOT SHOWN, TYPICAL.
3. TOP OF CONCRETE (TOC) = REF PLAN
4. SLAB THICKNESS = SEE PLAN
5. CONCRETE STRENGTH = REFER TO GENERAL NOTES
6. **C-X** DENOTES CIP CONCRETE COLUMN. FOR COLUMN SCHEDULE SEE SHEET SERIES S5.100
7. **SW-X** DENOTES CIP CONCRETE SHEAR WALL. FOR SHEAR WALL ELEVATIONS SEE SHEET SERIES S5.200
8. **SR-X** DENOTES STUD RAILS. FOR STUD RAIL SCHEDULE AND DETAILS SEE SHEET S5.130

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SHEET TITLE:
Parking Level 01

SHEET NUMBER:
PS1.101

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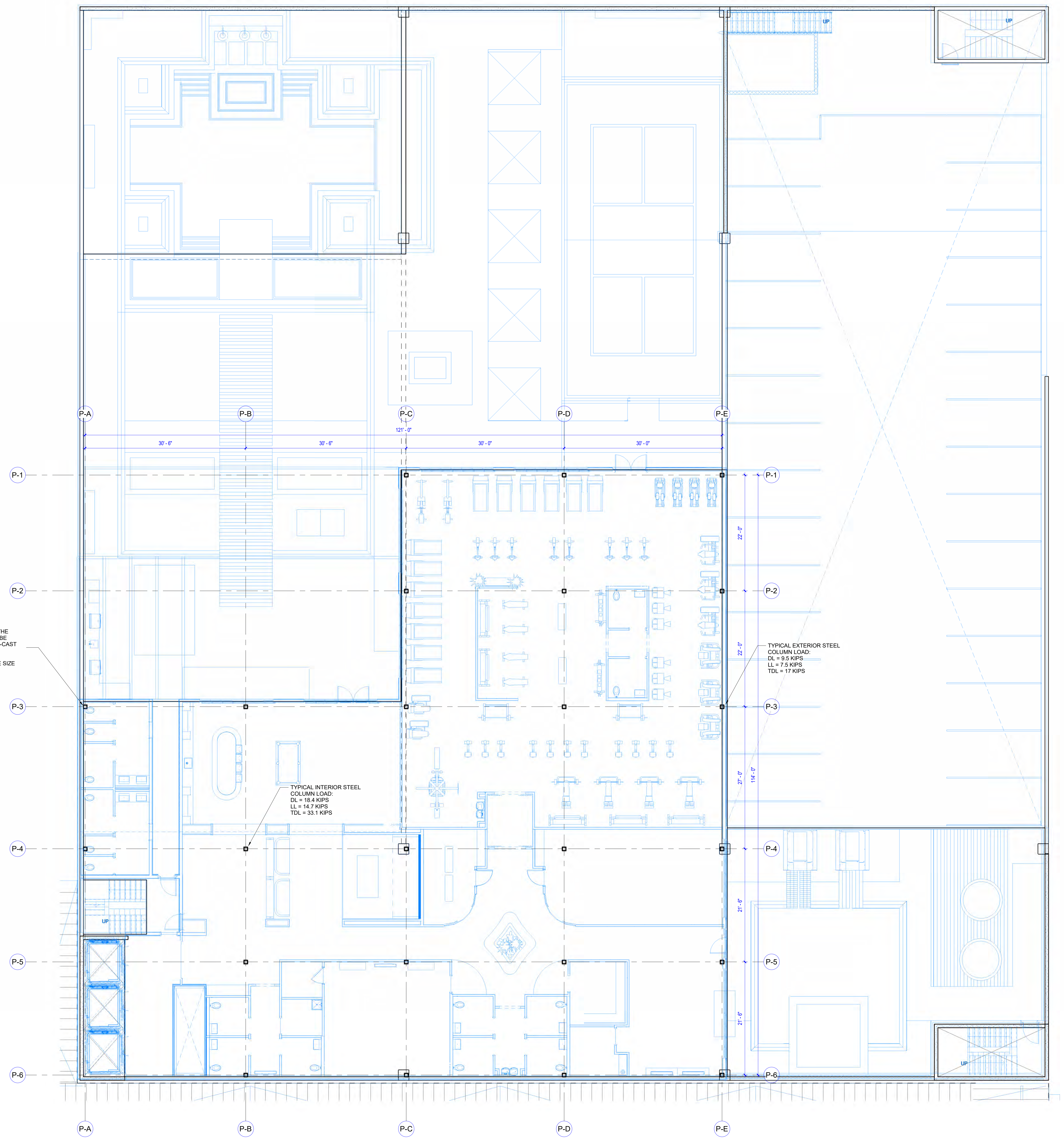

EMBED PLATE CONNECTION TO THE GARAGE SLAB TO BE DESIGNED BY PRE-CAST GARAGE MFR.

MIN. EMBED PLATE SIZE SHALL BE 12"X12"

TYPICAL EXTERIOR STEEL
COLUMN LOAD:
DL = 9.5 KIPS
LL = 7.5 KIPS
TDL = 17 KIPS

TYPICAL INTERIOR STEEL
COLUMN LOAD:
DL = 18.4 KIPS
LL = 14.7 KIPS
TDL = 33.1 KIPS

TRUE NORTH

PROJECT NORTH

1 Steel Framing Plan - Parking Level 07.5

1/8" = 1'-0"

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Structural Engineers

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SHEET TITLE:
Parking Level 07.5

SHEET NUMBER:
PS2.107

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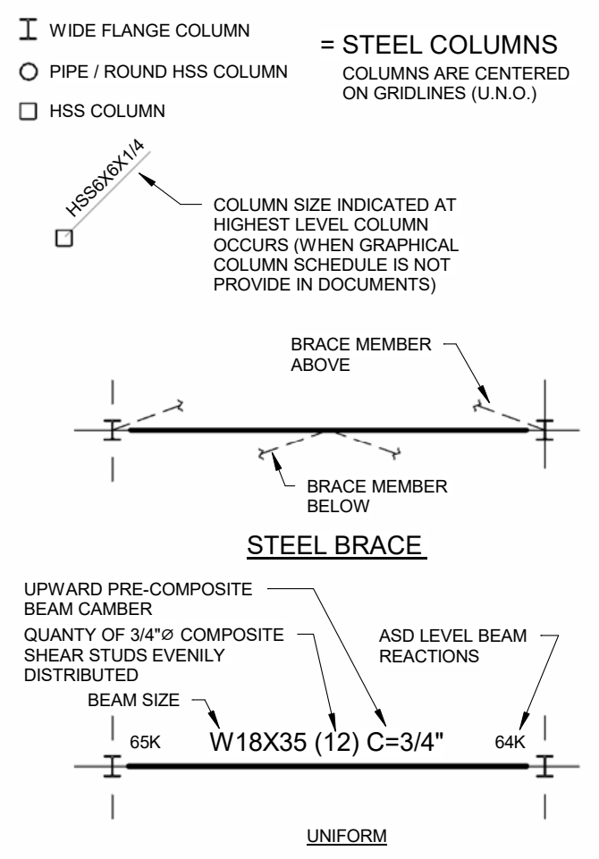
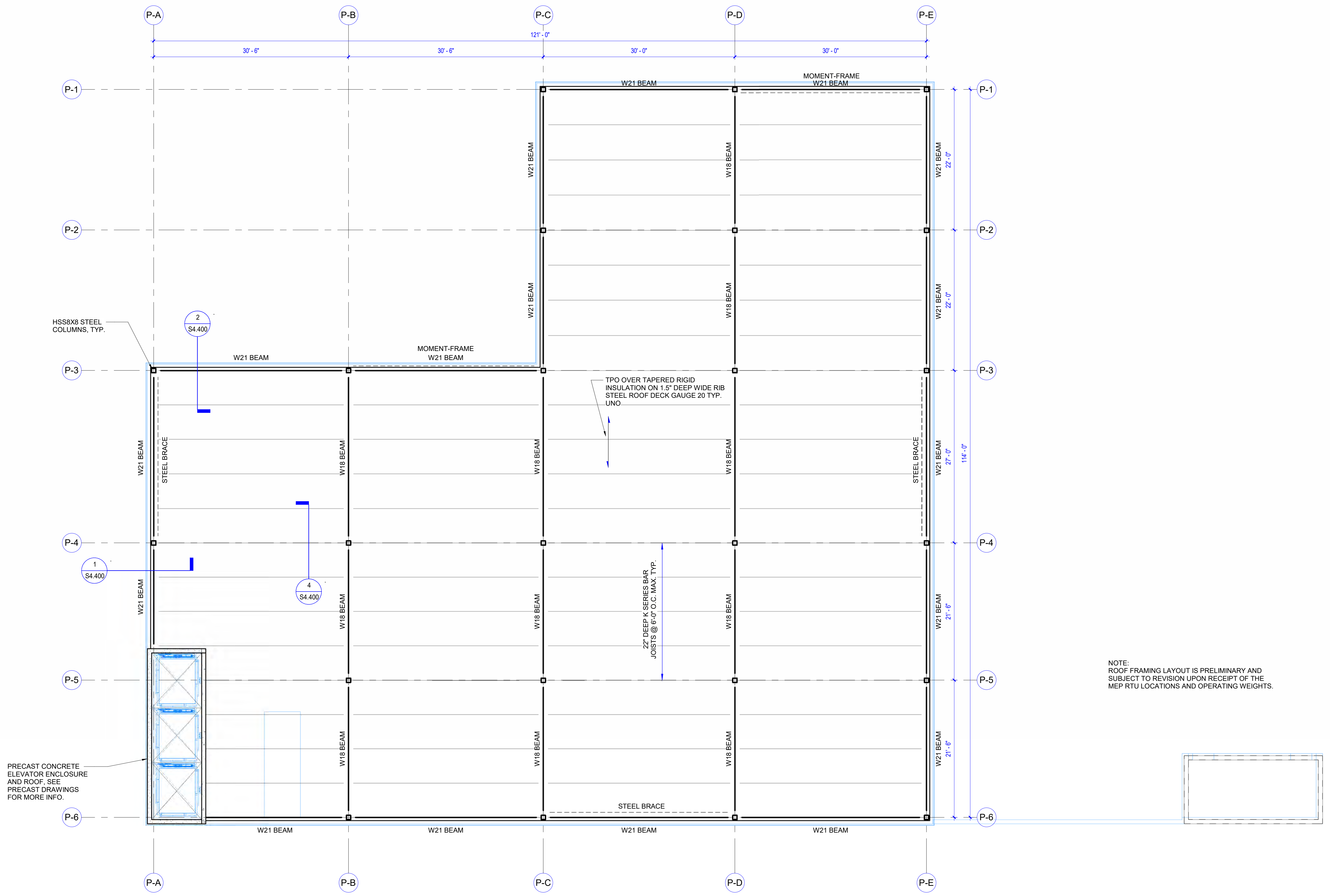
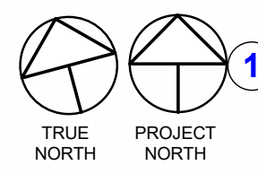
FLOOR FRAMING NOTES: (COMPOSITE)

1. CONTRACTOR TO CROSS REFERENCE AND / OR VERIFY ALL DIMENSIONS, ELEVATIONS, RECESSES DEPRESSIONS, CURB SIZES / LOCATIONS, WALL ELEVATIONS, GUARDRAILS, FINISHES, OPENINGS AND SLAB PENETRATION SIZES / LOCATIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ACHITECT AND ENGINEER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.
2. REFER TO ARCHITECTURAL SLAB DIMENSIONAL CONTROL PLAN FOR ALL SLAB EDGE AND OPENING DIMENSIONS NOT SHOWN (TYPICAL).
3. TOP OF STEEL (T.O.S.) = SEE PLAN
4. SLAB THICKNESS = SEE PLAN
5. FOR TYPICAL COMPOSITE FRAMING DETAILS, INCLUDING COMPOSITE SHEAR CONNECTOR PLACEMENT SEE COMPOSITE STEEL FRAMING GENERAL NOTES / TYPICAL DETAIL SHEETS


WIDE FLANGE COLUMN
PIPE / ROUND HSS COLUMN
HSS COLUMN
= STEEL COLUMNS
COLUMNS ARE CENTERED ON GRIDLINES (U.N.O.)
HSS6X6X1/4
COLUMN SIZE INDICATED AT HIGHEST LEVEL COLUMN OCCURS (WHEN GRAPHICAL COLUMN SCHEDULE IS NOT PROVIDE IN DOCUMENTS)
BRACE MEMBER ABOVE
BRACE MEMBER BELOW
STEEL BRACE
UPWARD PRE-COMPOSITE BEAM CAMBER
QUANTY OF 3/4"Ø COMPOSITE SHEAR STUDS EVENILY DISTRIBUTED
ASD LEVEL BEAM REACTIONS
BEAM SIZE
65K
W18X35 (12) C=3/4"
64K
UNIFORM


P-A
P-B
P-C
P-D
P-E
121' - 0"
30' - 6"
30' - 6"
30' - 0"
30' - 0"
P-1
P-2
P-3
P-4
P-5
P-6
W21 BEAM
MOMENT-FRAME W21 BEAM
W21 BEAM
W18 BEAM
22' - 0"
22' - 0"
27' - 0"
21' - 6"
21' - 6"
114' - 0"
HSS8X8 STEEL COLUMNS, TYP.
2
S4.400
1
S4.400
4
S4.400
MOMENT-FRAME W21 BEAM
STEEL BRACE
TPO OVER TAPERED RIGID INSULATION ON 1.5" DEEP WIDE RIB STEEL ROOF DECK GAUGE 20 TYP. UNO
22" DEEP K SERIES BAR JOISTS @ 6'-0" O.C. MAX. TYP.
PRECAST CONCRETE ELEVATOR ENCLOSURE AND ROOF. SEE PRECAST DRAWINGS FOR MORE INFO.

NOTE:
ROOF FRAMING LAYOUT IS PRELIMINARY AND SUBJECT TO REVISION UPON RECEIPT OF THE MEP RTU LOCATIONS AND OPERATING WEIGHTS.



1 Steel Framing Plan - Parking Roof
1/8" = 1'-0"

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SHEET TITLE:
Parking Level Roof

SHEET NUMBER:
PS2.108

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