- 1. All bracina shown and provided by the Metal Buildina Provider (MBP) for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303-16: Section 7.10).
- 2. Temporary supports, such as quys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10.3).
- Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.14).
- Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
- 4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
- 5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met;
- welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code Steel or AWS D1.3 Structural Welding Code — Sheet as applicable, for the processes, positions, and materials involved.
- All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
- All documentation and records shall be the responsibility of the customer.
- Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
- Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
- Neither the Metal Building Provider nor the customer will cut, drill or otherwise after their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16. Section 7.15).
- 10. The Metal Building Provider Field Modifications Policy:
- 10.1. The Metal Building Provider will only be responsible for the field-modified parts designed and approved by the Metal Building Provider's Customer Service Department
- Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and
- The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
- 11. WARNING SOME PANELS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL—OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

#### RESPONSIBILITIES

- 1. The Metal Building Provider Customer, hereafter referred to as the "customer," obtains and pays for all building permits, licenses, public assessments, paying or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
- 2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal
- 3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
- 4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
- The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing. it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
- 6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of
- Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3). 7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
- 8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional
- accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with
- approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements. 10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

#### GENERAL SPECIFICATIONS

1.1" of upward deflection.

- Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
- Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and
- The Metal Building Provider's red-oxide and gray-oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions.
- 4. All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections. 5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation Inspection requirements for Snug-Tight Bolts
- (Specification for Structural joints, Section 9.1) is suggested.
- Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane. 7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

The Fnaineer whose seal and signature appear on these documents represents the metal building manufacturer and is not the Fnaineer of Record for the overall project. The Engineer's responsibility is limited to material furnished by the metal building manufacturer and excludes foundation design, erection of the building, and any accessories not furnished by the metal building manufacturer.

-Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition and/or relocation of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

-The framing at line 12 is designed to receive a future addition with a maximum bay spacing of 30'-0" as measured between centerline of the existing endwall frame to the centerline of the future frame.

-The rigid frames at lines 5.5, 1, B1, and B4 are designed as non-expandable rigid frames. Corresponding frame reactions are calculated based upon actual tributary area.

-The Building B vertical deflection limit for purlins and rafters has been limited to L/240 but not greater than 2.5" combined total gravity plus uplift due to the attachment of a stud track by others. The stud track by others should allow for 1.4" of downward deflection and

-Per AISI S-100-16 Appendix A Section 16.3.1a, the wind uplift loading in edge and corner zones is permitted to be multiplied by a factor of 0.67 for the standing seam roof panel used on this building. This reduction factor is not expressed in the ultimate wind uplift values tabulated on this

-The support member provided by the metal building manufacturer on Building B in the curtain wall areas has been designed to support the wall system not by metal building manufacturer. The support member has been designed to deflect horizontally less than L/240 under wind or seismic loading and deflect vertically less than L/600 under gravity loading. The wall system shall be attached to the support members at a maximum spacing of 4'-0" on center. The maximum weight of wall material considered is 15 psf. A pipe column by others that also provides support to the materials by others must be installed at 1'-6" from the steel line at each frame line.

#### BUILDING DESIGN CODES APPROVAL SPECIFICATIONS

10.00 in/hr

IBC 21 Building Code: Steel Specification: AISC 360-16 Cold-Formed Specification: AISI S100-16

GENERAL LOADS 3.00 psf (A), 2.65 psf (B), 2.50 psf (C) Roof Dead Load: Roof Collateral Load: 5.00 psf (A), 6.00 psf (B), 3.00 psf (C) Roof Live Load: 20.00 psf Tributary Live Load Reduction: Yes

Wind Speed (3-sec gust) Vult: 127 mph 98 mph 77 mph V service:

Rainfall Intensity (5-minute duration 5-year recurrence):

Wind Exposure Category: Wind Condition: Enclosed (A & B), Partially Enclosed (C) Internal Pressure Coefficient: 0.18, -0.18 (A & B), 0.55, -0.55 (C)

#### SNOW LOAD

Ground Snow Load (Pg) 0.00 psf Roof Snow Load (Pf): 0.00 psf Snow Exposure Factor (Ce): 1.00 1.00 Snow Load Importance Factor (Is): Thermal Factor (Ct): 1.00

DEFLECTION CRITERIA **Endwall Columns:** H/120 H/240 H/120 Endwall Rafters (Live): L/180 L/240 L/180 Endwall Rafters (Wind): L/180 L/240 L/180 L/90 L/240 L/90 Wall Girts:

Roof Purlins (Live): L/150 L/240 L/150 Roof Purlins (Wind): L/180 L/240 L/180 L/60 L/60 Wall Panels: L/60 Roof Panels (Live) L/60 L/60 L/60 Roof Panels (Wind): L/60 L/60 L/60 H/130

Rigid Frames (Horz): H/60 H/100 L/180 \_L/240 Rigid Frames (Live): L/180 Rigid Frames (Wind): L/180 L/240 L/180 Rigid Frames (Seismic): L/50 L/100 L/130

L/100 L/100 L/100 Rigid Frames (Crane): SEISMIC LOAD

Seismic Importance Factor (le): Spectral Response Acceleration (Ss): Spectral Response Acceleration (S1): Site Class:

Spectral Response Coefficients (Sds): Spectral Response Coefficients (Sd1): Seismic Design Category:

Basic Seismic Force Resisting Systems\*: Framing Direction: Longitudinal Structural Syst: Design Base Shear (A):

5.93 Kips 13.54 Kips Design Base Shear (B): 7.24 Kips 7.09 Kips 0.89 Kips 0.94 Kips Design Base Shear (C): 0.0314 0.0314 Sesimic Response Coefficient(s) (Cs) Response Modification Factor(s) (R): 3.0 3.0 Deflection Amplification Factor(s): 3.0

Analysis Procedure: Equivalent Lateral Force

H: Steel Systems Not Specially Detailed for Seismic Resistance

Others loads: (2) 10 Tons TRSG Cranes & (1) 20 Ton TRDG Crane

\* Ordinary Steel Concentrically Braced Frame(s) and/or Ordinary Steel Moment Frame(s)

#### ROOF PANEL (A & B)

Profile: Super Seam—Plus Gauge: 24 Color: Cool White

UL580 Class 90: Yes Clip Type if Standing Seam: High Floating

ROOF PANEL (C) Profile: Super Span X

WALL PANEL (B)

UL580 Class 90: Yes WALL PANEL (A & C)

Purlins, Eave Struts:

Profile: Super Span X Gauge: <u>26</u> Color: <u>NEED COLOR</u>

Gauge: <u>26</u> Color: <u>Galvalume Plus</u>

II - Normal

1.00

0.0884

0.0565

0.0943

0.0904

Profile: Berridge Deep Deck Gauge: 24 Color: NEED COLOR

Pre-Galvanized

PRIMARY FRAMING Built-Up & Hot-Rolled: Gray Oxide Primer SECONDARY FRAMING

Girts, Light Gage Columns: Pre-Galvanized Light Gage Jambs & Headers: Pre-Galvanized

Hot-Dip Galvanizing conforms to the ASTM A123 specification Pre-Galvanized members conform to the ASTM A653, Grade 50, Coating G-90 specification.

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

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Gauge: <u>26</u> Color: <u>NEED COLOR</u>

Gauge: 24 Color: NEED COLOR

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LINER PANEL (A. B & C)

Profile: FLAT 12-R

SOFFIT PANEL (B)

Super Span X

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DRAWING STATUS

FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final.
Only drawings issued "For Erector Installation" can be consid FINAL DESCRIPTION:
Final drawings for construction

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BY CHK SHEET DESCRIPTION: DATE CUSTOMER LOCATION: 08.15.25 FOR APPROVAL Waukesha-Pearce Industries, LL 12320 S. Main Stree PROJECT REFERENCE: WPI Baton Rouge JORSITE LOCATION: Airline Highway, Baton Rogue LA 70817 East Baton Rouge 8/ 7/25 DMH 14554-37882

P1-P8 A1 08.15.25 RIGID FRAME FLEVATION A1 08.15.25 PORTAL FRAME ELEVATION W1-W3 F3 A1 08.15.25 ROOF SHEETING PLAN E4-E18 A1 08.15.25 FRAME & SHEETING ELEVATION E19 A1 08.15.25 LINER SHEETING PLAN A1 08.15.25 CRANE PLAN & SECTIONS A1 08.15.25 BUILDING SECTIONS D1-D6 A1 08.15.25 STANDARD DETAILS PAGE Drawings listed under Drawing Schedule are submitted for Approval. These drawings represent

DRAWING SCHEDULE

A1 08.15.25 ANCHOR BOLT PLAN

A1 08.15.25 ANCHOR BOLT DETAILS

A1 08.15.25 ANCHOR BOLT REACTIONS

A1 08.15.25 COVER SHEET

DESCRIPTION

the Metal Building Provider (MBP) interpretation of our scope of work. You are reviewing these drawings to confirm that the MBP has correctly interpreted the project requirements. All dimensions, sections, details and notes require your review. All "clouded areas" must be reviewed and addressed/answered before your project is placed within the schedule. Please complete all field verifications prior to returning your approval drawings. The MBP is not responsible for checking the material and/or design compatibility of components not supplied by the MBP. MBP drawings may not match architectural/structural drawings and specifications.

Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly

3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings

6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the

Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal

Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and

DWG NO. ISSUE DATE

constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

C1

F2

F3-F5

Building Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to

The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid

4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.

contract or a person authorized, in writing, by the Metal Building Provider customer.

Building Provider will not be responsible

Be made in contrasting ink.

Be legible and unambiguous.

5. It is imperative that any changes to these drawings:

Have all instances of changes clearly indicated.

misfabrication. This may impact the delivery schedule.

in writing by change order or separate documentation.

the shipping schedule.

L/130

L/130

L/120

L/120

L/60

L/120

L/120

<u>L/</u>120

GAUGE:

GAUGE:

GAUGE:

GAUGE:

GAUGE:

GAUGE:

Wind Framing (Wind):

Roof Purlins (Total):

Endwall Rafters (Total):

GUTTER: Need Color

RAKE: Need Color

CORNER: Need Color

BASE: Need Color

ACCESSORY: Need Color

DOWNSPOUT: Need Color

Rigid Frames (Total):

TRIM COLOR:

Lateral

Wind Framing (Seismic): L/50

L/100

L/100

L/180

L/180

L/180

26 26

interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design,

2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal

and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including

material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or

"Approved as Submitted" (no changes or corrections and project is released for fabrication)

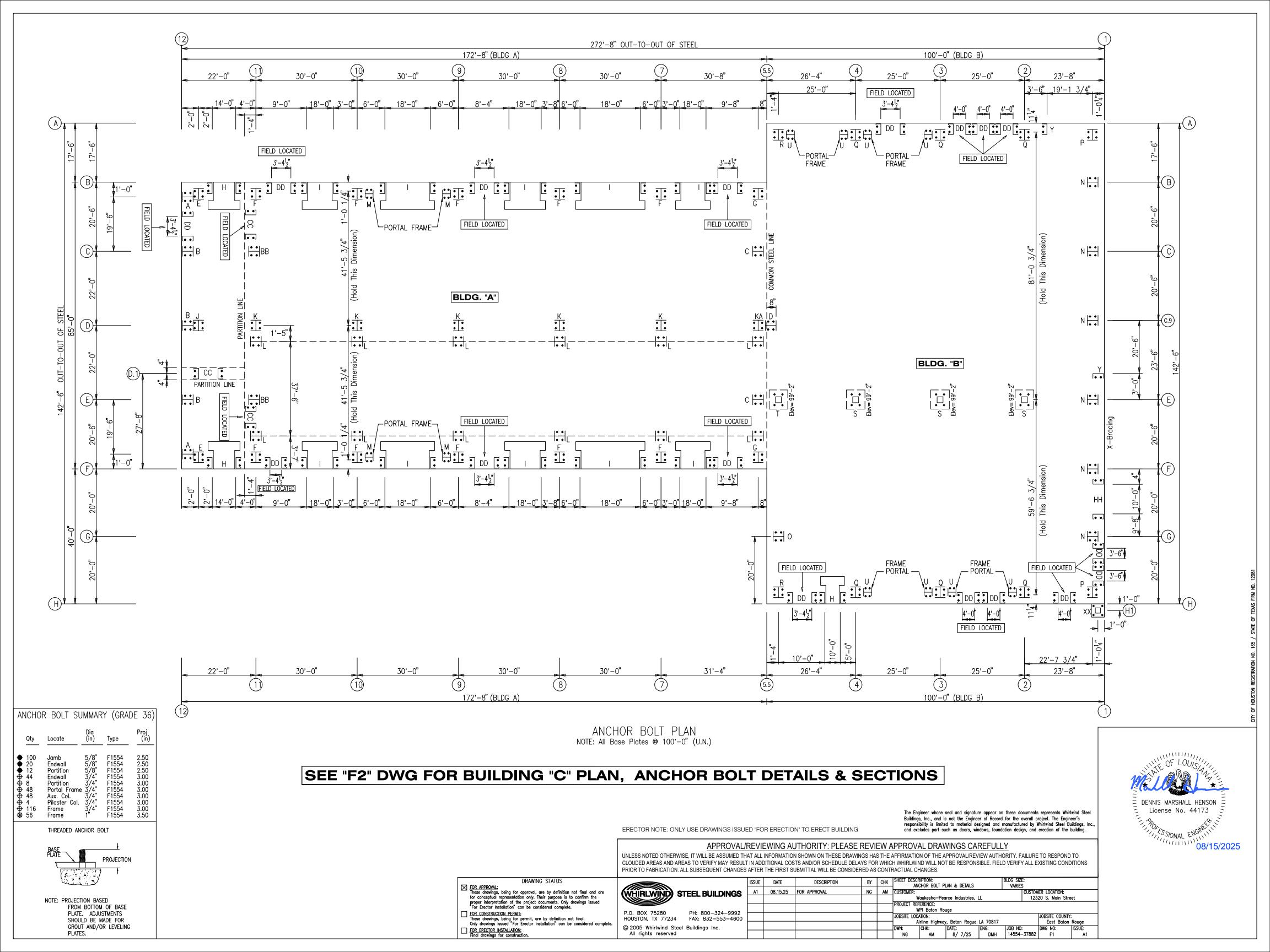
"Approved as Noted" (with corrections and/or changes clearly noted and project is released

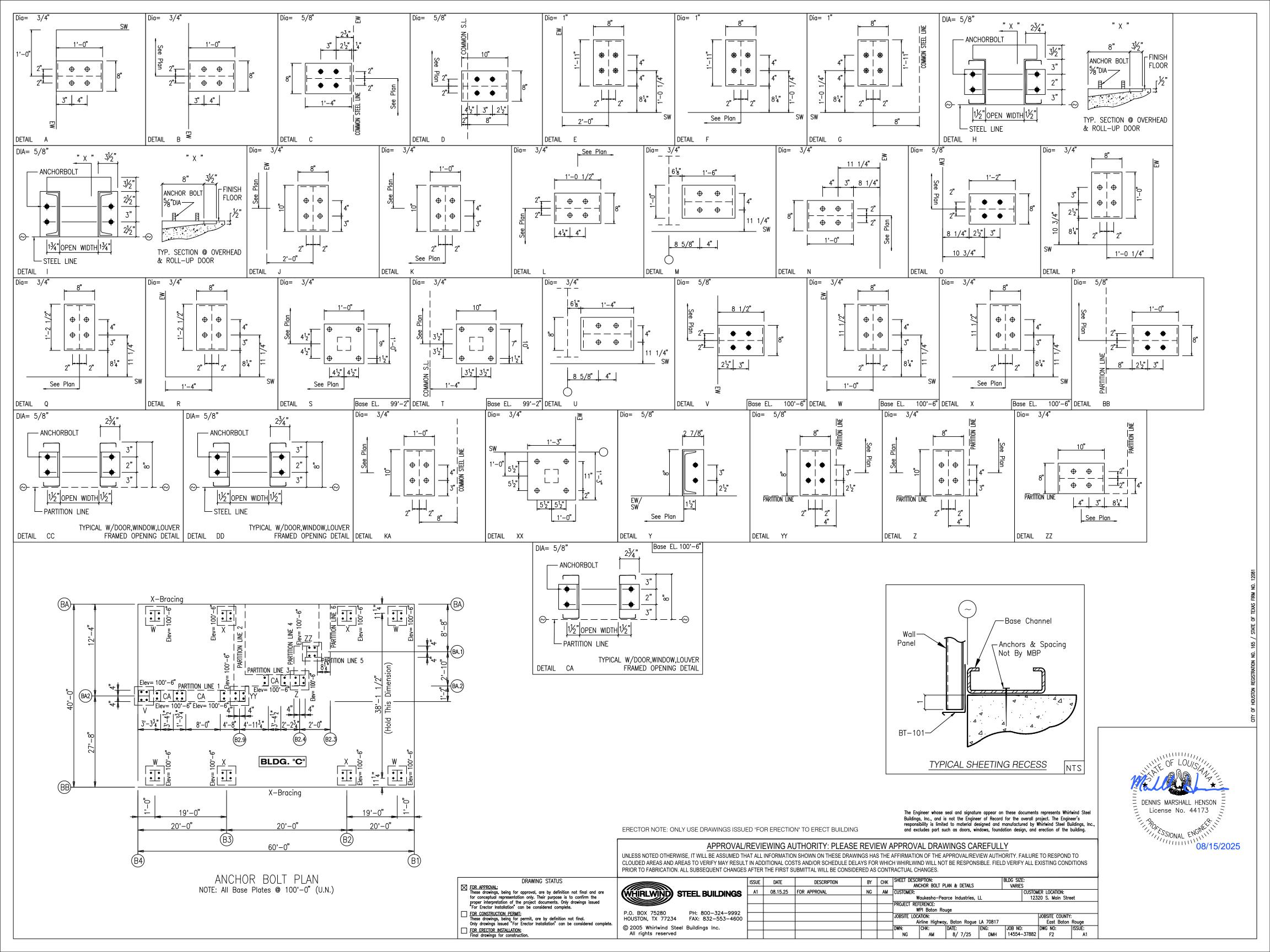
"Revise and Resubmit" (with corrections and/or changes clearly noted and revised drawings will be resubmitted for approval)

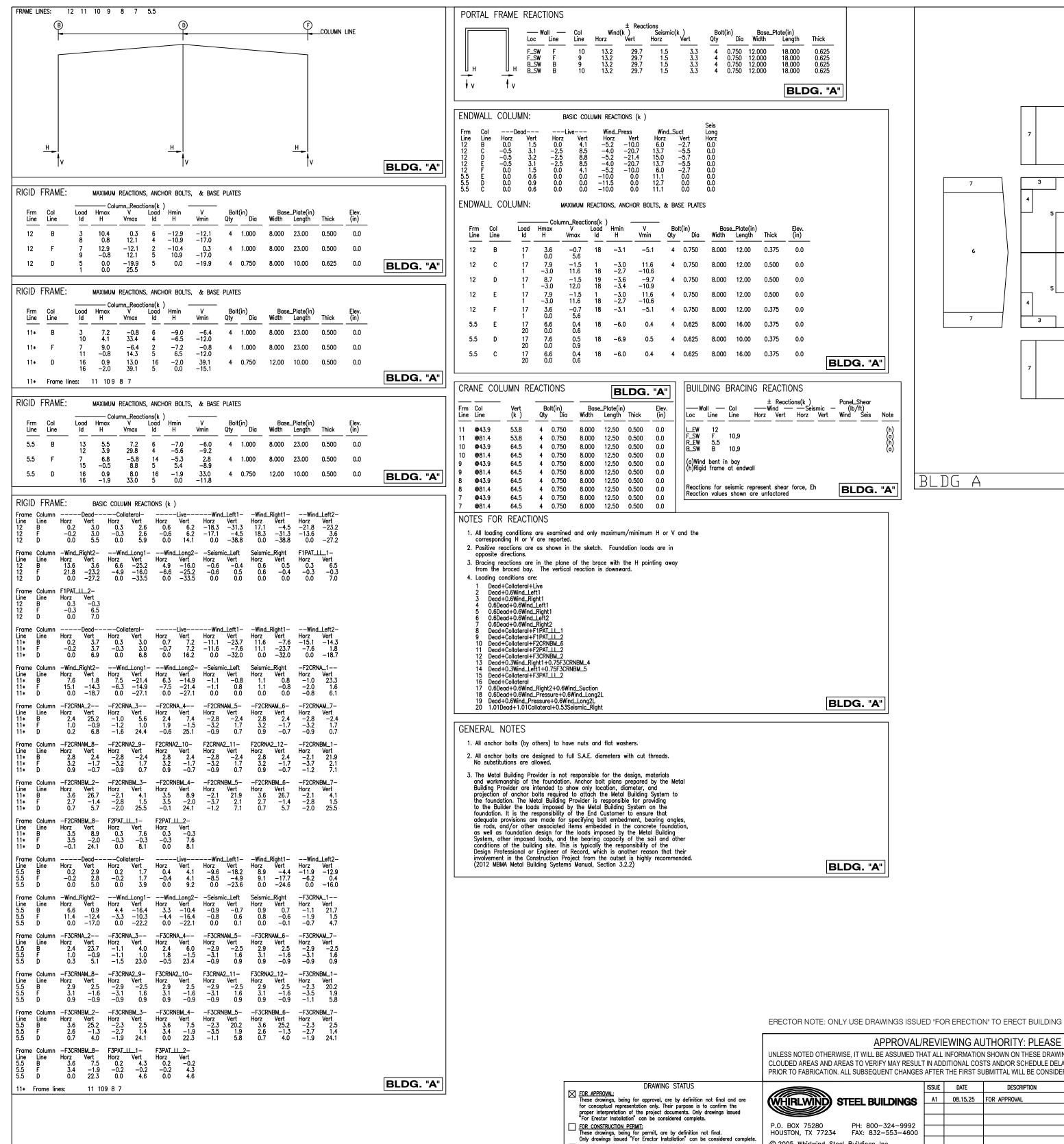
Approval Signature: ☐ Approved As Submitted ☐ Approved As Noted ☐ Revised & Resubmit Print Name: Desired Delivery Date:

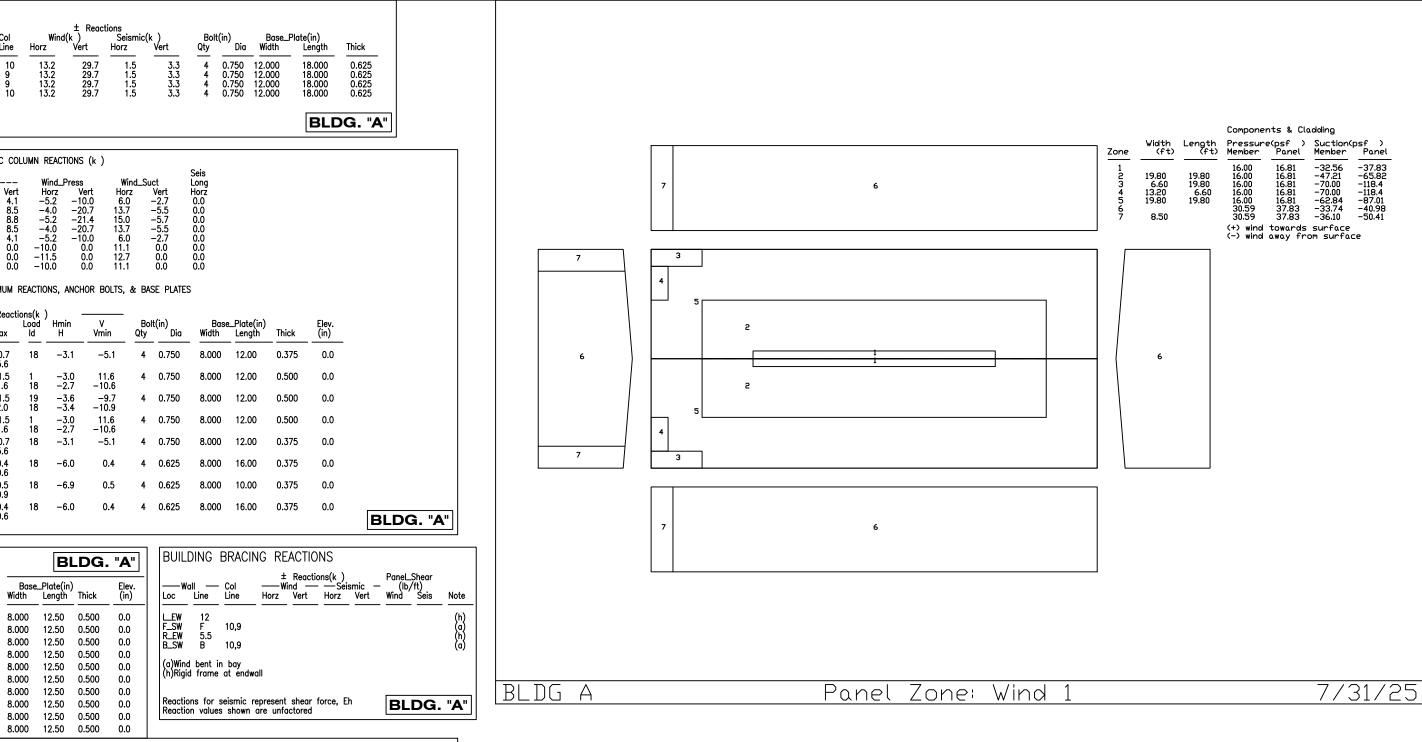
> DENNIS MARSHALL HENSON License No. 44173 SONAL ENGINE

08/15/2025









BLDG. "A"

BLDG. "A"

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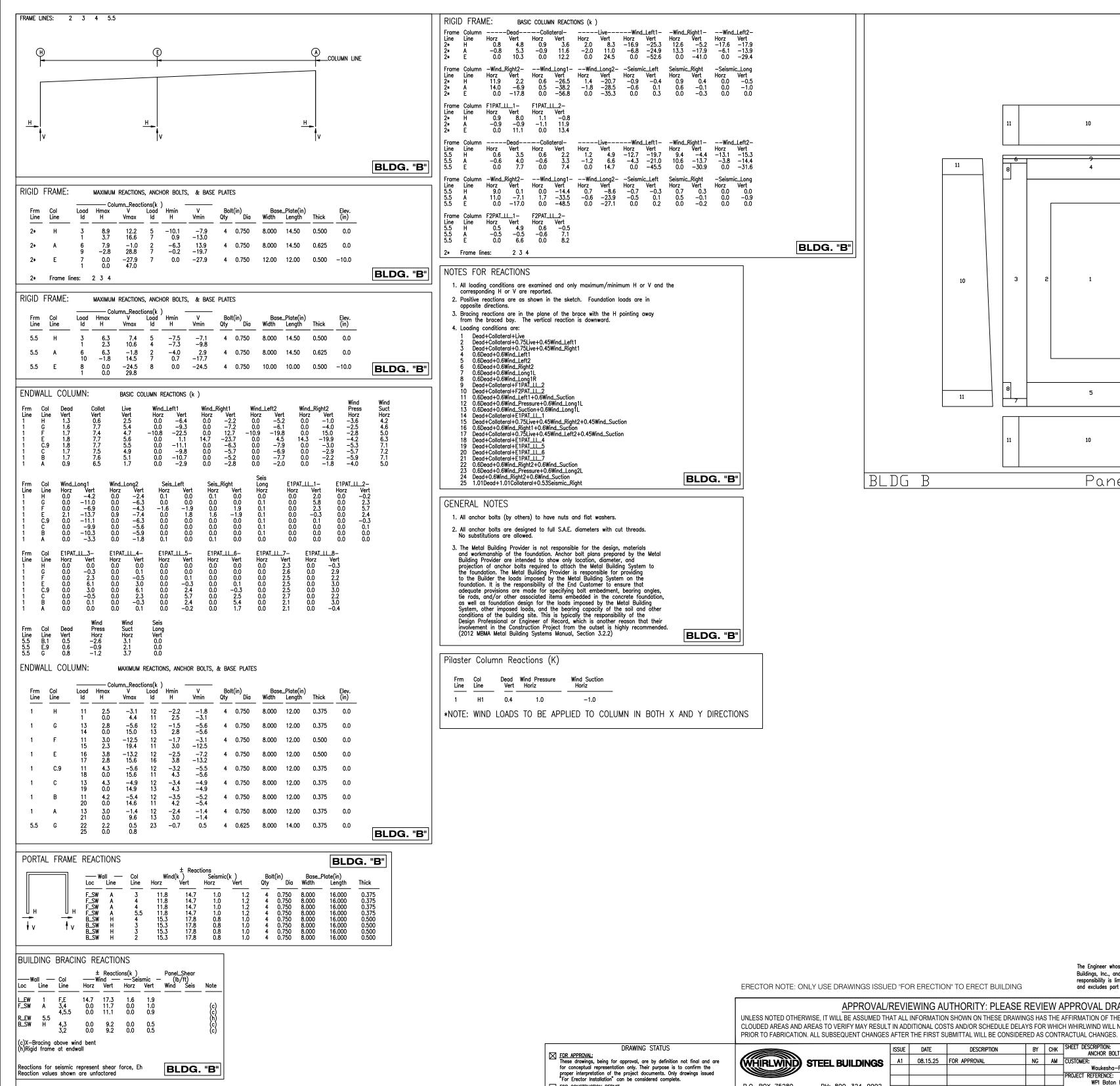
or approval, are by definition not final and are	
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the project documents. Only drawings issued can be considered complete.	
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	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCR AN	RIPTION: CHOR BOLT RI	EACTIONS	E	BLDG SIZE: VARIES				
NGS	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:	CUSTOMER:			CUSTOM	CUSTOMER LOCATION:			
100						] w	/aukesha-Pea	ırce Industries, l	123	12320 S. Main Street				
						PROJECT REFERENCE:								
9992						WPI Baton Rouge								
-4600						JOBSITE LOCA	ATION:			JOBSITE COUNTY:				
-4600						A	Airline Highway, Baton Rogue LA 70817				East Baton	Rouge		
						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:		
						NG	AM	8/ 7/25	DMH	14554-37882	F3	<b>A1</b>		





Reactions for seismic represent shear force, Eh

Reaction values shown are unfactored

BLDG. "B"

Components & Cladding Width (ft) Pressure(psf ) Suction(psf ) Member Panel Member Panel Zone -30.14 -43.70 -58.17 -58.17 -58.17 -64.80 -64.80 -64.80 -44.79 -31.24 16.00 16.00 16.00 16.00 16.00 16.00 16.00 35.02 35.02 -35.02 -60.93 -80.55 -80.55 -109.6 -109.6 -74.59 -37.94 13.80 16.00 16.00 16.00 16.00 16.00 16.00 16.00 28.32 28.32 13.80 2.60 4.60 9.20 2.00 13.80 13.80 4.60 10 9.20 -33.43 (+) wind towards surface (-) wind away from surface 11 3 10 11 7/31/25 Panel Zone: Wind

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Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings and excludes part such as doors, windows, foundation design, and erection of the building.

NG AM 8/ 7/25 DMH 14554-37882 F4

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DRAWING STATUS	
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for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete.	
FOR CONSTRUCTION PERMIT:	P.O. BOX 7
These drawings, being for permit, are by definition not final.	HOUSTON, T
Only drawings issued "For Erector Installation" can be considered complete.	@ 2005 Whi

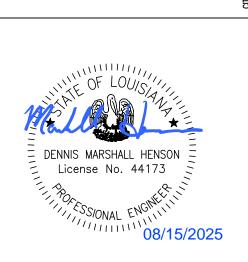
Final drawings for construction

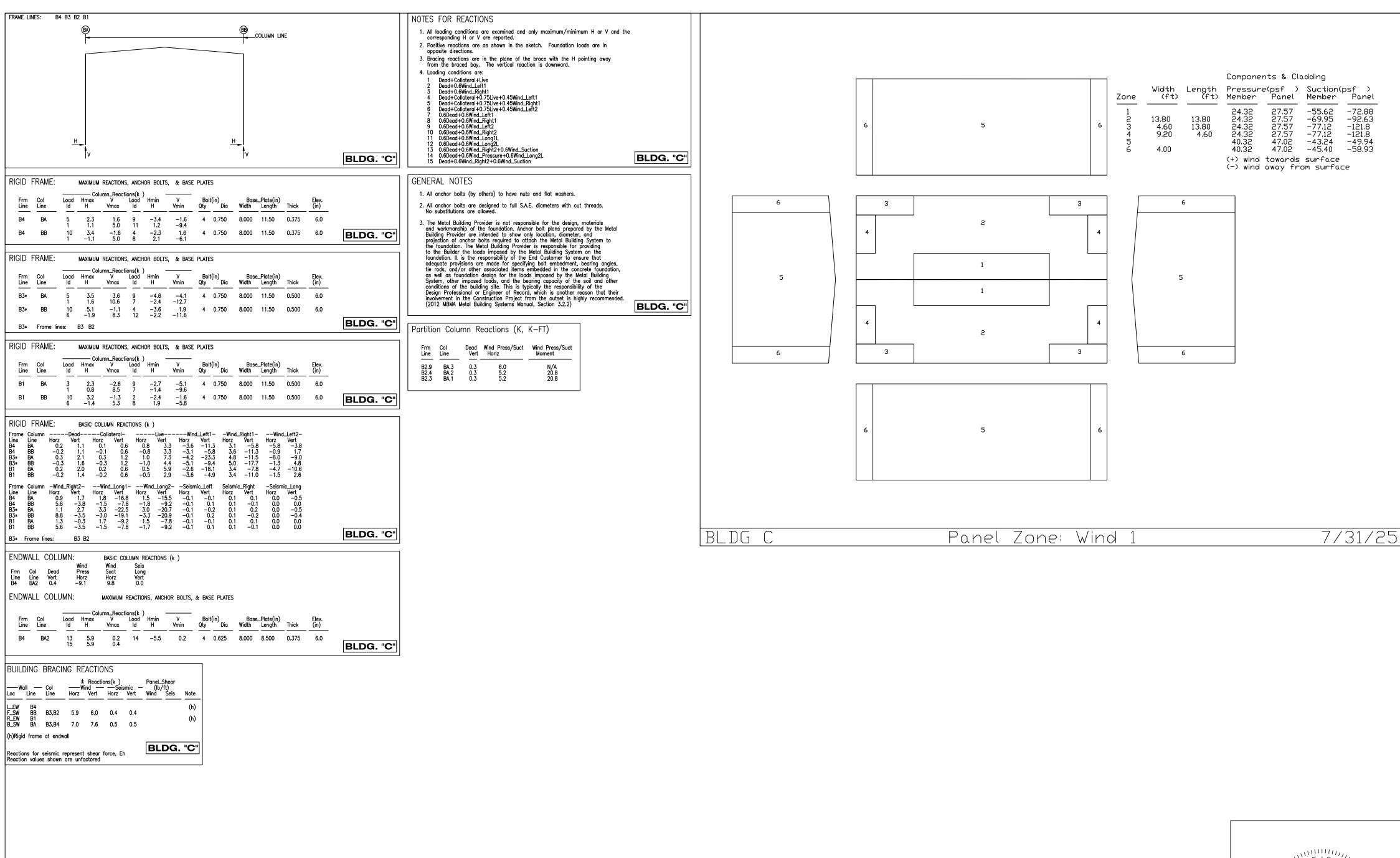
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	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCRIPTION: ANCHOR BOLT REACTIONS	BLDG SIZE VARIES	
3S	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:		CUSTOMER LOCATION:
<b></b>						Waukesha-Pearce Industries, LL		12320 S. Main Street
92						PROJECT REFERENCE: WPI Baton Rouge		
600						JOBSITE LOCATION:		JOBSITE COUNTY:
						Airline Highway, Baton Rogue LA 70817		East Baton Rouge
						DWN: CHK: DATE: ENG:	JOB NO:	DWG NO: ISSUE:





ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

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Baton Rouge

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VAL:						
inge	heina	for	approval	are	hv	de

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FOR CONSTRUCTION PERMIT:
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5280	PH: 800-324-9992
X 77234	FAX: 832-553-4600

	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCR	RIPTION: CHOR BOLT RI	EACTIONS		VARIES				
NGS	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:				1	CUSTOMER	R LOCATION:		
100							Waukesha-Pearce Industries, LL					12320 S. Main Street		
						PROJECT REF								
9992				_			PI Baton Ro	uge						
-4600						JOBSITE LOCA					J	obsite coun		
							Airline Highway, Baton Rogue LA 70817					East Bate		
						DWN:	CHK:	DATE:	ENG:	JOB NO:	D	WG NO:	ISSU	
						NG	AM	8/ 7/25	DMH	14554-	37882	F5		



SPLICE PLATE & BOLT TABLE Bot Int Type Mark Thick 2 A325 2 A325 5/8" 3/8" 3/4" 3/4" 2'-8 3/4"

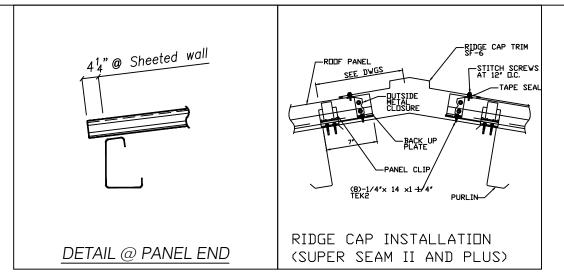
CAP PLATES

CAF FLATES	)							
Col			Bolt		Plate Size			
ld	Qnt	Туре	Dia	Len	Width	Thick	Length	
RI1-4	4	A325	0.625	1.750	8.000	0.625	10.000	

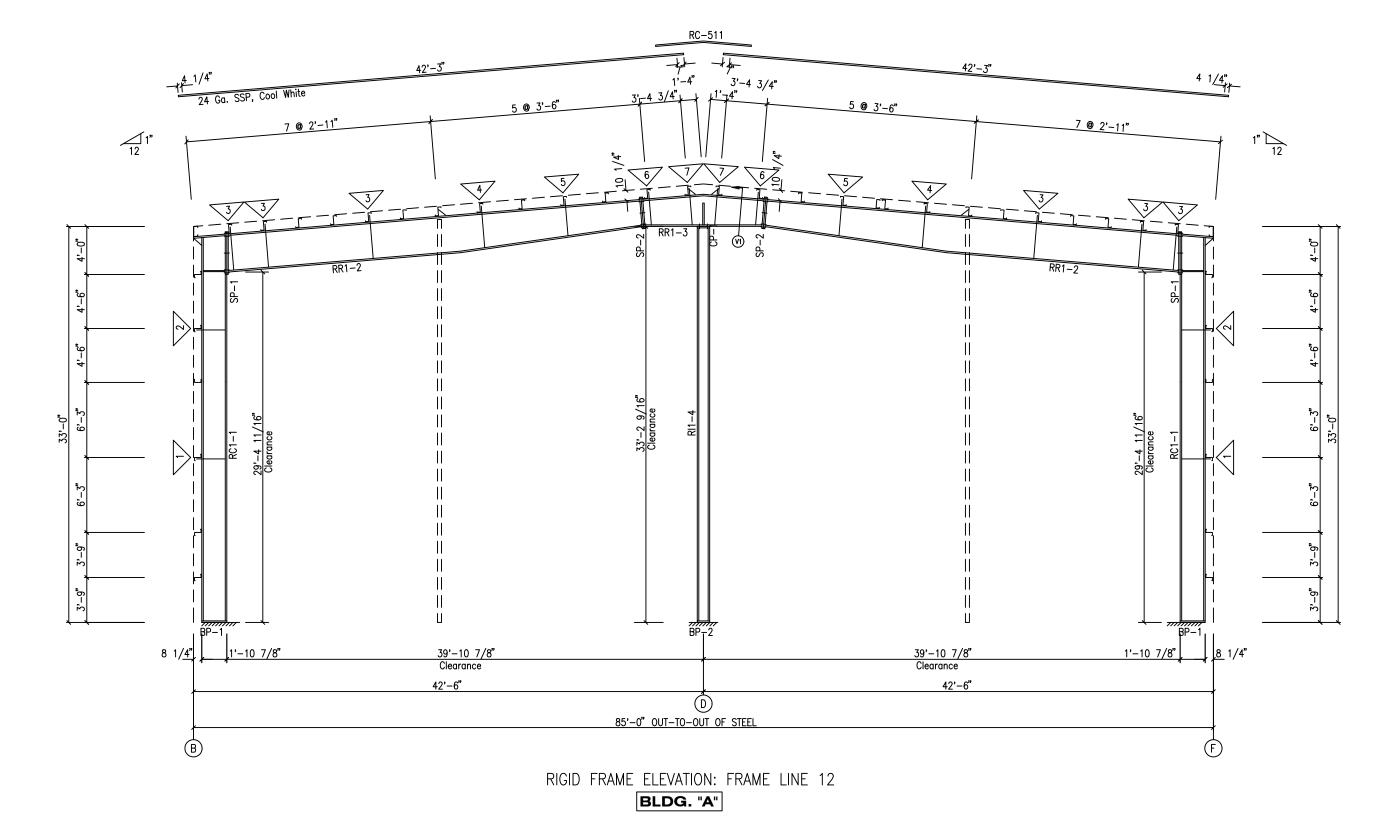
A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16

FRAME LINE: 12									
	∇ID	# SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP		
	1 2 3 4 5 6 7	1 1 1 1 1	FB3A FB4C FB17A FB13A FB10A FB5A FB9A	2'-7 1/2" 2'-7 1/2" 3'-9 7/8" 3'-3 3/8" 2'-11 3/4" 2'-9" 2'-11 1/4"	2'-4" 2'-4" 3'-0" 2'-4" 2'-4" 2'-4"	G26 G26 G26 G26 G26 G26 G26	AK226 AK227 AK227 AK227 AK227 AK227 AK227		

BASE PLATE TABLE							
Col	Plo	ate Size	Length				
Mark	Width	Thick					
BP-1	8"	1/2"	1'-11"				
BP-2	8"	5/8"	10"				



MEMBER	TABLE			
Manali	Web Depth	Web Plate	Outside Flange	Inside_Flange
Mark	Start/End	Thick Length	W x Thk x Length	W x Thk x Length
RC1-1	22.0/22.0	0.133   240.0	6 x 3/8" x 240.0	6 x 3/8" x 240.0
	22.0/22.0 22.0/22.0 22.0/22.0 35.0/35.0 35.0/24.0	0.133 108.0 0.250 39.5	6 x 1/2" x 145.6	6 x 1/2" x 108.0
	22.0/22.0	0.250 39.5	$6 \times 5/16$ ° × 30.7	, in the second
RR1-2	35.0/35.0	0.184 235.8	6 x 5/16" x 232.9	6 x 1/4" x 235.8
	35.0/24.0	0.184 184.1	6 x 1/4" x 184.1	6 x 1/4" x 184.4 6 x 1/4" x 119.6
RR1-3	24.0/29.1	0.161   123.7	6 x 1/4" x 62.0	6 x 1/4" x 119.6
			6 x 1/4" x 62.0	
RI1-4	W10851		,	
·				



#### BOLT TIGHTENING (Pretensioned)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as pretensioned joints in accordance with the Specification for Structural Joints Using High—Strength Bolts, June 11, 2020. The specification recognizes five pretensioning methods without preference to any one; (1) Turn-of-Nut, (2) Calibrated Wrench, (3) Twist—Off Tension Control, (4) Direct tension Indicator, and (5) Combined Method.

The metal building manufacturer recommends the use of Turn-of-Nut method as it requires no special bolts, washers, or installation tools. When standard size holes are used with Turn-of-Nut method, and the nut is the part rotated, no washers are required per Section 6.2 of the Specification. The requirements of the Specification, Section 7 shall be completed before joint assembly.

Excerpts from the Specification, Section 8.2.1:

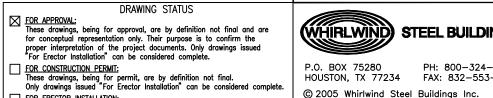
After the Snug-Tight operation has been performed, the nut rotation as specified in Table 8.1 shall be applied to all bolts in the joint, progressing systematically from the most rigid part of the joint in a manner that will minimize relaxation of previously pretensioned bolts.

Installation inspection requirements for Pretensioned joints using Turn-of-Nut method can be found in Section 9.2.1 of the Specification. ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc. and excludes part such as doors, windows, foundation design, and erection of the building.

#### APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY

UNLESS NOTED OTHERWISE, IT WILL BE ASSUMED THAT ALL INFORMATION SHOWN ON THESE DRAWINGS HAS THE AFFIRMATION OF THE APPROVAL/REVIEW AUTHORITY. FAILURE TO RESPOND TO CLOUDED AREAS AND AREAS TO VERIFY MAY RESULT IN ADDITIONAL COSTS AND/OR SCHEDULE DELAYS FOR WHICH WHIRLWIND WILL NOT BE RESPONSIBLE. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION. ALL SUBSEQUENT CHANGES AFTER THE FIRST SUBMITTAL WILL BE CONSIDERED AS CONTRACTUAL CHANGES.



DRAWING STATUS

WHIRLWIND S	TEEL BUILDING
P.O. BOX 75280 HOUSTON, TX 77234	PH: 800-324-99 FAX: 832-553-46
	P.O. BOX 75280

	ISSUE	DATE	DESCRIPTION		CHK	SHEET DESCRIPTION: 				BLDG SIZE: VARIES		
STEEL BUILDINGS	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:					MER LOCATION:	
WILLIAM OF LEE BOILDINGS						Waukesha-Pearce Industries, LL			12	12320 S. Main Street		
						PROJECT REF						
BOX 75280 PH: 800-324-9992						W	'PI Baton Ro	uge				
STON, TX 77234 FAX: 832-553-4600						JOBSITE LOCA					JOBSITE COUNTY	
•						A A	irline Highway	, Baton Rogue	LA 70817		East Baton	Rouge
005 Whirlwind Steel Buildings Inc.						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
l rights reserved						NG	AM	8/ 7/25	DMH	14554-37882	P1	A1



SPLICE PLATE & BOLT TABLE Qty Top Bot Int Type Dia Mark Thick 4 2 A325 4 2 A325 3'-8" 2'-9"

CAP PLATES

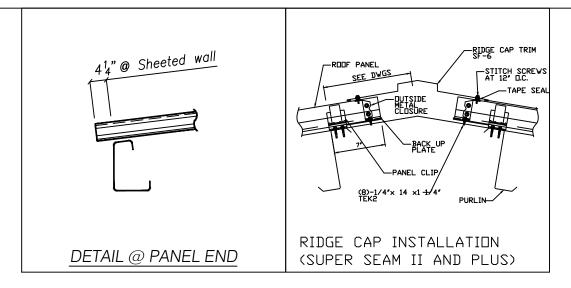
CAF FLATES											
Col			Bolt		Plate Size						
ld	Qnt	Туре	Dia	Len	Width	Thick	Length				
RI2-6	4	A325	0.750	1.750	12.000	0.500	10.000				

FLANGE BRACE TABLE

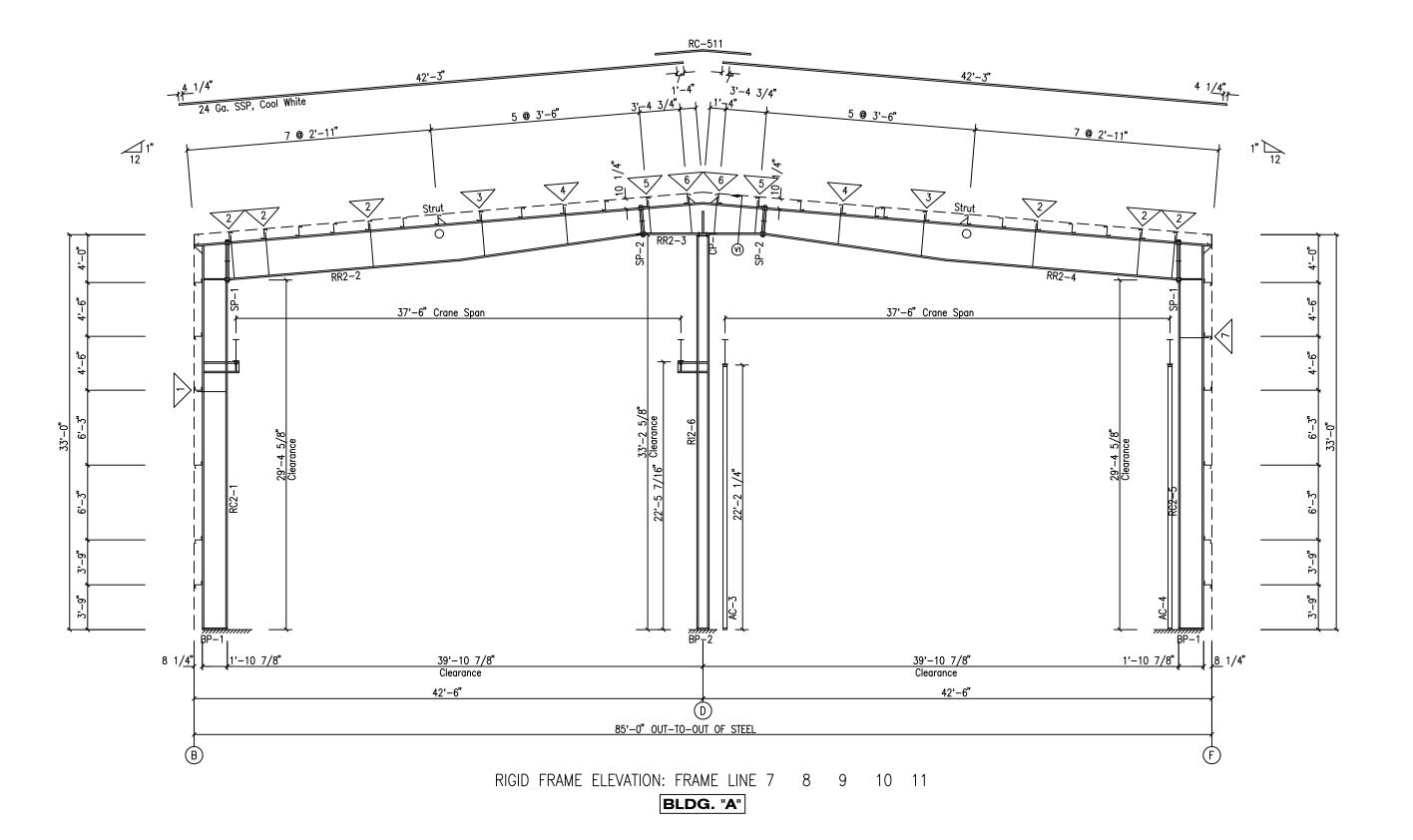
A-12-2-14-04 P-12-2-12-04 C-12-2-14-18 D-13-3-3-14-18

A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16 FRAME LINE: 11 10 9 8 7										
∇ID	# SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP				
1 2 3 4	1 1 1	FB3A FB17A FB16A FB10A	2'-7 1/2" 3'-9 7/8" 3'-9" 2'-11 3/4"	2'-4" 3'-0" 3'-0"	G26 G26 G26 G26	AK226 AK227 AK227 AK227				
5 6 7	1 1 1	FB5A FB9A FB4C	2'-9" 2'-11 1/4" 2'-7 1/2"	2'-4" 2'-4" 2'-4"	G26 G26 G26	AK227 AK227 AK227 AK226				

BASE PLATE	TABLE			
Col		ate Size		
Mark	Width	Thick	Length	
BP-1	8"	1/2"	1'-11"	
BP-2	1'-0"	1/2"	10"	



MEMBER	TABLE Twob Donth	Web P	lata	Outside Flance	Incide Flance
Mark	Web Depth			Outside Flange	Inside_Flange
	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length
RC2-1	22.0/22.0 22.0/22.0	0.188	240.0	8 x 3/8" x 240.0	8 x 1/2" x 240.0
	22.0/22.0	0.188	147.5	8 x 3 <sup>′</sup> /8" x 145.7	8 x 1/2" x 108.0
				8 x 1/4" x 30.7	
RR2-2	35 0 / 35 0	0.188	236.1	8 x 5/16" x 240.0	8 x 3/8" x 236.1
11112 2	35.0/35.0 35.0/24.0 24.0/29.1	0.188	184.0	8 x 5/16" x 177.1	8 \ 3/8" \ 18/3
DDO 7	33.0/24.0	0.100		0 x 3/10 x 1//.1	8 x 3/8" x 184.3 8 x 1/4" x 119.4
RR2-3	24.0/29.1	0.164	123.4	8 x 1/4" x 61.9	0 X 1/4 X 119.4
	/			8 x 1/4" x 61.9	
RR2-4	24.0/35.0	0.188	184.0	8 x 5/16" x 177.1	8 x 3/8" x 184.3
	24.0/35.0 35.0/35.0 22.0/22.0 22.0/22.0	0.188	236.1	8 x 5/16" x 240.0	8 x 3/8" x 236.1
RC2-5	22.0/22.0	0.188	147.5	8 x 1/4" x 30.7	8 x 1/2" x 108.0 8 x 1/2" x 240.0
	22 0/22 0	0.188	240.0	8 x 3/8" x 145.7	$\begin{bmatrix} 8 & 1/2 & 240.0 \end{bmatrix}$
	22.07 22.0	0.100	210.0	8 x 3/8" x 240.0	0 x 1/2 x 210.0
RI2-6	W10263			0 x 3/0 x 2+0.0	
AC-3	W12X26				
AC-4	W12X26				



#### **BOLT TIGHTENING (Pretensioned)**

All bolted joints with ASTM F3125 Grade A325 bolts are specified as pretensioned joints in accordance with the Specification for Structural Joints Using High—Strength Bolts, June 11, 2020. The specification recognizes five pretensioning methods without preference to any one; (1) Turn-of-Nut, (2) Calibrated Wrench, (3) Twist-Off Tension Control, (4) Direct tension Indicator, and (5) Combined Method.

The metal building manufacturer recommends the use of Turn-of-Nut method as it requires no special bolts, washers, or installation tools. When standard size holes are used with Turn-of-Nut method, and the nut is the part rotated, no washers are required per Section 6.2 of the Specification. The requirements of the Specification, Section 7 shall be completed before joint assembly.

Excerpts from the Specification, Section 8.2.1:

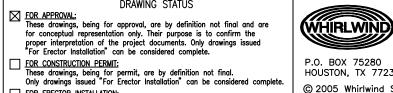
After the Snug-Tight operation has been performed, the nut rotation as specified in Table 8.1 shall be applied to all bolts in the joint, progressing systematically from the most rigid part of the joint in a manner that will minimize relaxation of previously pretensioned bolts.

Installation inspection requirements for Pretensioned joints using Turn-of-Nut method can be found in Section 9.2.1 of the Specification. ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

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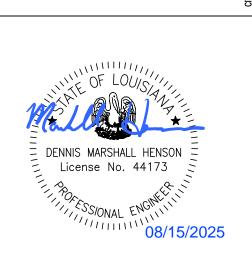


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DRAWING STATUS

WHIRLWIND ST	TEEL BUILDING
P.O. BOX 75280 HOUSTON, TX 77234	PH: 800-324-999 FAX: 832-553-46
© 2005 Whirlwind Steel	Buildings Inc.

	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCR RIG	ription: ID Frame Ele	VATION	E	BLDG SIZE: VARIES		
GS	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:			L	CUSTOM	IER LOCATION:	
as						W	aukesha-Pea	ırce Industries,	LL	123	20 S. Main Stre	et
						PROJECT REF	ERENCE:			-		
992						ļ w	'PI Baton Roi	uge				
4600						JOBSITE LOCA	ATION:				JOBSITE COUNTY:	:
+000						A	irline Highway	, Baton Rogue	LA 70817		East Baton	Rouge
						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
						NG	AM	8/ 7/25	DMH	14554-37882	P2	A1



SPLICE PLATE & BOLT TABLE Mark Top Bot Int Type Dia Width Thick Length SP-1 2 A325 4 2 A325

AK227

CAP PLATES

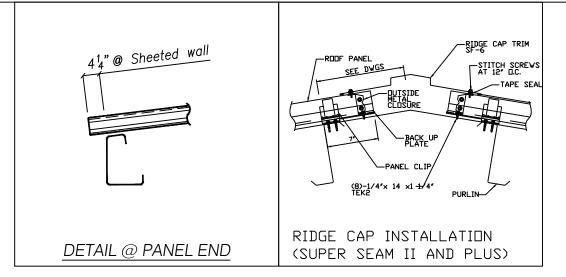
CAP PLATES	)							
Col			Bolt		Plate Size			
ld	Qnt	Туре	Dia	Len	Width	Thick	Length	
RI3-5	4	A325	0.750	1.750	12.000	0.500	10.000	

FLANGE BRACE TABLE A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16FRAME LINE: 5.5 ▽ ID SIDES MARK OFFSET DETAIL LENGTH FB3A 2'-7 1/2" FB17A 3'-9 7/8" FB12A 3'-1 1/2" FB8A 2'-10" 2'-4" 3'-0" 2'-4" 2'-4" 2'-4" AK226 AK227 AK227 AK227 G26 G26 G26 G26

2'-11 1/4"

BASE PLATE TABLE							
	Col Mark	Plo Width	Length				
	BP-1 BP-2	8" 1'-0"	1/2" 1/2"	1'-11" 10"			

FB9A



	MEMBER TA	BLE				
	Mandi	Web Depth	Web F		Outside Flange	Inside Flange
L	Mark	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length
	RC3-1	22.0/22.0 22.0/22.0	0.188	240.0	8 x 3/8" x 240.0	8 x 1/2" x 240.0
		22.0/22.0	0.188	147.5	8 x 3/8" x 145.7	8 x 1/2" x 108.0
					l 8 x 1/4" x 30.7	·
	RR3-2	35.0/35.0	0.188 0.188 0.164	236.1	8 x 1/4" x 240.0	8 x 5/16" x 236.1
		35.0/24.0	0.188	184.0	8 x 1/4" x 177.1	8 x 5/16" x 184.3
	RR3-3	35.0/35.0 35.0/24.0 24.0/29.1	0.164	123.4	8 x 1/4" x 61.9 8 x 1/4" x 61.9	8 x 5/16" x 236.1 8 x 5/16" x 184.3 8 x 1/4" x 119.4
		·			8 x 1/4" x 61.9	·
	RC3-4	22.0/22.0	0.188 0.188	147.5	8 x 1/4" x 30.7	8 x 3/8" x 108.0 8 x 3/8" x 240.0
		22.0/22.0 22.0/22.0	0.188	240.0	8 x 3/8" x 145.7	8 x 3/8" x 240.0
					8 x 3/8" x 240.0	·
L	RI3-5	W10263			,	

33'-0" 3'-9" 3'-9" 6'-3" 6'-3" 4'-6" 4'-0"	24 Ga. SSP, Cool White  7	7 © 2'-11"  7 © 2'-11"  7 © 2'-11"  RR3-2  37'-6" Crane Span	2 4 1/4" 1" 12 12 12 17 17 12 12 13 1/4" 33/4" 33/4" 8 1/4"
			3/4" 8 1/4"
	B RIGID FRAME ELEVAT	OUT OF STEEL  TION: FRAME LINE 5.5	E

BLDG. "A"

#### BOLT TIGHTENING (Pretensioned)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as pretensioned joints in accordance with the Specification for Structural Joints Using High-Strength Bolts, June 11, 2020. The specification recognizes five pretensioning methods without preference to any one; (1) Turn-of-Nut, (2) Calibrated Wrench, (3) Twist-Off Tension Control, (4) Direct tension Indicator, and (5) Combined Method.

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Excerpts from the Specification, Section 8.2.1:

After the Snug-Tight operation has been performed, the nut rotation as specified in Table 8.1 shall be applied to all bolts in the joint, progressing systematically from the most rigid part of the joint in a manner that will minimize relaxation of previously pretensioned bolts.

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| DATE: | ENG: | JOB NO: | DWG NO: | 8 / 7 / 25 | DMH | 14554 – 37882 | P3

.. Waukesha-Pearce Industries, LL

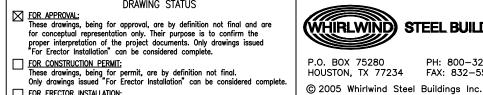
Airline Highway, Baton Rogue LA 70817

CUSTOMER LOCATION: 12320 S. Main Street

East Baton Rouge
DWG NO: ISSUE:

#### APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY

UNLESS NOTED OTHERWISE, IT WILL BE ASSUMED THAT ALL INFORMATION SHOWN ON THESE DRAWINGS HAS THE AFFIRMATION OF THE APPROVAL/REVIEW AUTHORITY. FAILURE TO RESPOND TO CLOUDED AREAS AND AREAS TO VERIFY MAY RESULT IN ADDITIONAL COSTS AND/OR SCHEDULE DELAYS FOR WHICH WHIRLWIND WILL NOT BE RESPONSIBLE. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION. ALL SUBSEQUENT CHANGES AFTER THE FIRST SUBMITTAL WILL BE CONSIDERED AS CONTRACTUAL CHANGES.



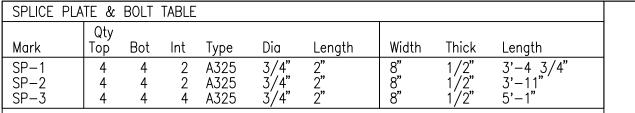
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DRAWING STATUS

WHIRLWIND	STEEL BUILDING
P.O. BOX 75280 HOUSTON, TX 772	PH: 800-324-99 PAX: 832-553-46
a 2005 Whirlwind	Steel Buildings Inc

	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCR	RIPTION: ID FRAME ELE	VATION
NGS	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:	ID THUMBE ELL	
100						W	aukesha-Pec	ırce Indi
						PROJECT REF	ERENCE:	
9992						] W	'PI Baton Ro	uge
-4600						JOBSITE LOCA	ATION:	
-+000						Ai	irline Highway	, Baton
						DWN:	CHK:	DATE:



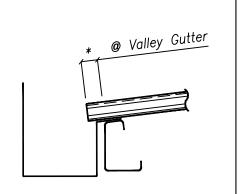


CAP PLATES Plate Size Thick Length ----Bolt----Dia Type A325 0.625 2.000 6.000 0.625 13.005

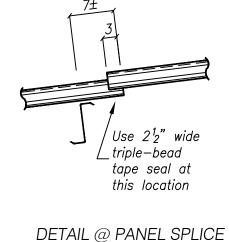
FLANGE BRACE TABLE

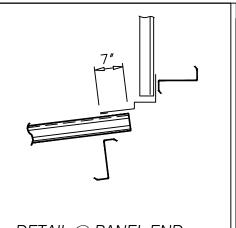
A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16

FRAM	IE LINE:	5.5				
	#					
$\triangle$ ID	SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP
1	1	FB104A	2'-11"	2'-4"	G26	AK226
2	1	FB106A	3'-2 1/2"	2'-4"	G26	AK227
3	1	FB108A	3'-10 3/8"	3'-0"	G26	AK227
4	1	FB109A	3'-11 1 <i>/</i> 2"	3'-0"	G26	AK227
5	1	FB110A		3'-0"	G26	AK227
6	1	FB111A	4'-3 1/2"	3'-0"	G26	AK227
7	1	FB112A	5'-3 1/4"	4'-0"	G26	AK227
8	1	FB114A		4'-0"	G26	AK227
9	l 1	FB103A	2'-10 1/8"	2'-4"	G26	AK226



DETAIL @ PANEL END

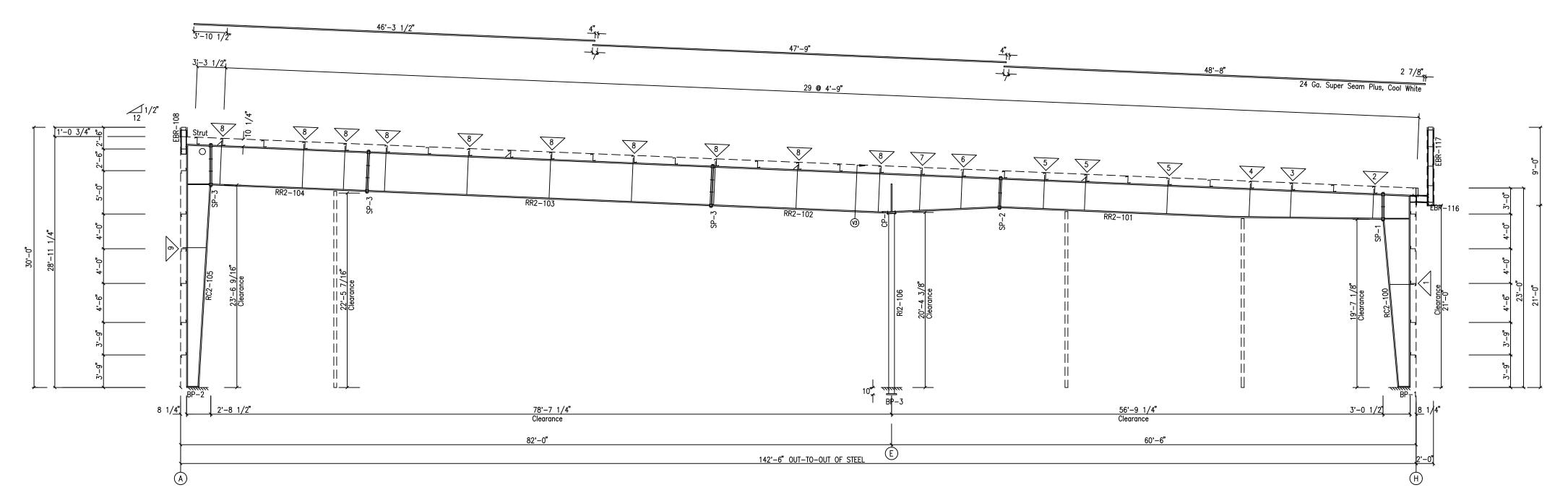




DETAIL @ PANEL END
HIGH SIDE

<u>  Member</u> T <i>e</i>	<u>ABLE</u>				
H Manala	Web Depth	Web		」Outside Flange	Inside Flange
Mark	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length
RC2-100	14.0/25.5	0.133	120.0	6 x 1/4" x 240.0	6 x 1/4" x 231.5
	25.5/36.0	0.184	146.8	6 x 1/4" x 25.3	·
				6 x 1/4" x 43.9	
RR2-101	32.0/38.0	0.184	208.2	8 x 1/4" x 240.0	8 x 1/4" x 208.3
	38.0/38.0	0.184	240.0	8 x 1/4" x 240.0	8 x 1/4" x 240.0
	38.0/38.0	0.184	82.5	8 x 1/4" x 49.4	8 x 1/4" x 82.5
RR2-102	38.0/52.0	0.250	160.5	8 x 3/8" x 240.0	8 x 3/8" x 161.1
	52.0/52.0	0.250	237.5	8 x 3/8" x 158.0	8 x 3/8" x 237.5
RR2-103	52.0/52.0	0.250	240.0	8 x 3/8" x 240.0	8 x 3/8" x 240.0
	52.0/52.0	0.250	235.6	8 x 3/8" x 235.6	8 x 3/8" x 235.6
RR2-104	52.0/52.0	0.250	218.4	8 x 3/8" x 218.4	8 x 3/8" x 216.2
RC2-105	32.0/29.5	0.161	95.8	6 x 1/4" x 32.3	6 x 1/4" x 38.5
	29.5/14.0	0.161	240.0	6 x 1/4" x 95.8	6 x 1/4" x 240.0
				6 x 1/4" x 240.0	
' RI2-106	T5X188				
EBR-108	W8X10				
EBR-116	W10X12				
EBR-117	W8X13				

BASE PLATE TABLE						
Col	Pl	ate Size	Length			
Mark	Width	Thick				
BP-1	8"	1/2"	1'-2 1/2"			
BP-2	8"	5/8"	1'-2 1/2"			
BP-3	10"	1/2"	10"			



RIGID FRAME ELEVATION: FRAME LINE 5.5

BLDG. "B"

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASIM F3120 Grade 7020 2000 accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts Using High—Strength Bolts Using High—Structural June 11, 2020, accordance with the Specification of Structural June 11, 2020, accordance with the Specification of Struct All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug—Tightened Joints in

direct tension indicator are not required. Installation inspection requirements for Snug—Tight Bolt is found in Section 9.1 of the Specification.

FOR CONSTRUCTION PERMIT:
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IIRLWIND	STEEL BUILDING
BOX 75280	PH: 800-324-99
STON, TX 77234	FAX: 832-553-4

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

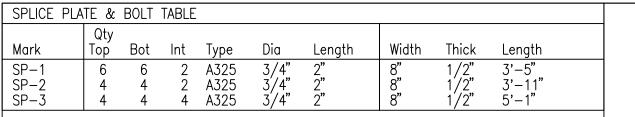
DENNIS MARSHALL HENSON License No. 44173 170/ESS/ONAL ENGINEER 1111 08/15/2025

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

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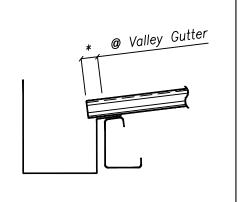
		DATE	DESCRIPTION	BY	СНК	SHEET DESCRIPTION: RIGID FRAME ELEVATION				BLDG SIZE: VARIES		
WHIRI WIND STEEL BUILDINGS	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:				CUSTO	IER LOCATION:	
WHIRLWIND STEEL BUILDINGS							Waukesha-Pearce Industries, LL			123	12320 S. Main Street	
						PROJECT REF						
P.O. BOX 75280 PH: 800-324-9992							PI Baton Ro	uge				
HOUSTON, TX 77234 FAX: 832-553-4600						JOBSITE LOCA					JOBSITE COUNTY	
						Airline Highway, Baton Rogue LA 70817 E					East Baton	
© 2005 Whirlwind Steel Buildings Inc.						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
All rights reserved						NG	AM	8/ 7/25	DMH	14554-37882	P4	A1



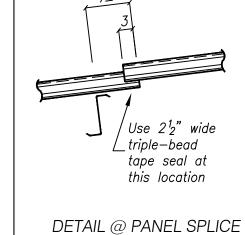
CAP PLATES ----Bolt----Plate Size Thick Length Type Dia A325 0.625 2.000 0.625 8.000 14.006

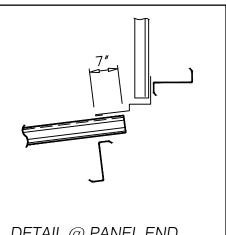
FLANGE BRACE TABLE A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16 FRAME LINE: 2 3 4

1 1///1/	L LIINL.		<u> </u>			
	#					
$\triangle$ ID	SIDES	MARK	LENGTH	OFFSET	DETAIL	CLIP
1	1		3'-2 3/8"	2'-4"	G26	AK226
2	1	FB1070		2'-4"	G26	AK227
3	1	FB108A	3'-10 3/8"	3'-0"	G26	AK227
4	1	FB110A	4'-0"	3'-0"	G26	AK227
5	2	FB111A	4'-3 1/2"	3'–0 <b>"</b>	G26	AK227
6	2	FB113A		4'-0"	G26	AK227
7	2	FB115A		4'-0"	G26	AK227
8	1	FB103A	2'-10 1/8"	2'-4"	G26	AK226



DETAIL @ PANEL END

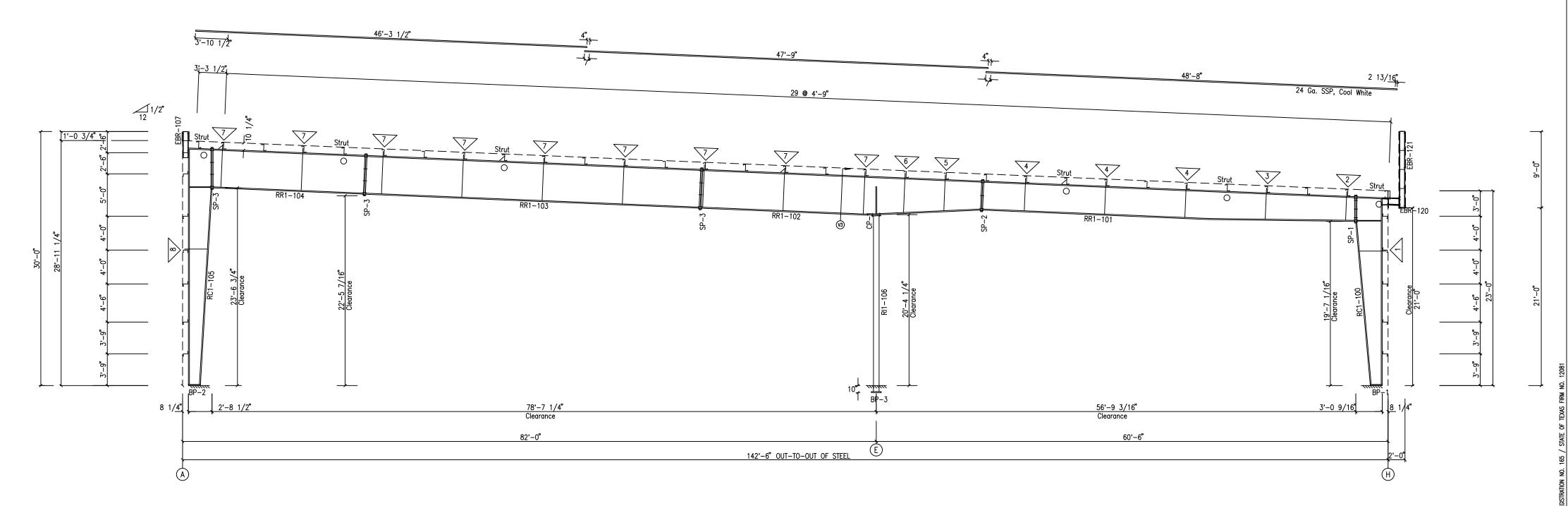




DETAIL @ PANEL END
HIGH SIDE

MEMBER TA	MEMBER TABLE									
14	Web Depth	Web F	Plate	Outside Flange	Inside Flange					
Mark	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length					
RC1-100	14.0/25.5	0.133	120.0	8 x 1/4" x 240.0	8 x 1/4" x 120.5					
	25.5/36.0	0.184	146.8	8 x 1/4" x 25.3	8 x 5/16" x 110.9					
	,			8 x 1/4" x 43.9	, ,					
RR1-101	32.0/38.0	0.184	208.2	8 x 5/16" x 206.9	8 x 5/16" x 208.3					
	38.0/38.0	0.184	240.0	8 x 1/4" x 240.0	8 x 1/4" x 240.0					
	38.0/38.0	0.184	82.5	8 x 1/4" x 82.5	8 x 1/4" x 82.5					
RR1-102	38.0/52.0	0.250	160.5	8 x 1/2" x 240.0	8 x 1/2" x 161.1					
	52.0/52.0	0.250	237.5	8 x 1/2" x 158.0	8 x 1/2" x 237.5					
RR1-103	52.0/52.0	0.250	240.0	8 x 1/2" x 240.0	8 x 1/2" x 240.0					
554 464	52.0/52.0	0.250	235.6	8 x 1/2" x 235.6	8 x 1/2" x 235.6					
RR1-104	52.0/52.0	0.250	218.4	8 x 1/2" x 218.4	8 x 1/4" x 216.2					
RC1-105	32.0/29.5	0.161	95.8	8 x 1/4" x 32.3	8 x 1/4" x 38.5					
	29.5/14.0	0.161	240.0	8 x 1/4" x 95.8	8 x 1/4" x 240.0					
DI4 100	TCVOFO			8 x 1/4" x 240.0						
RI1-106	T6X250									
EBR-107	W8X10									
EBR-120	W10X22									
EBR-121	W8X24									

BASE PLATE	TABLE		
Col	PI-	ate Size	
Mark	Width	Thick	Length
BP-1	8"	1/2"	1'-2 1/2" 1'-2 1/2"
BP-2	8"	5′/8"	1'-2 1/2"
BP-3	l 1'-0"	1/2"	1'-0"



RIGID FRAME ELEVATION: FRAME LINE 2 3 4 BLDG. "B"

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASIM F3120 Grade 7020 2000 accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts Using High—Strength Bolts Using High—Structural June 11, 2020, accordance with the Specification of Structural June 11, 2020, accordance with the Specification of Struct All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in

direct tension indicator are not required. Installation inspection requirements for Snug—Tight Bolt is found in Section 9.1 of the Specification.

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ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

ISSUE DATE

APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY UNLESS NOTED OTHERWISE, IT WILL BE ASSUMED THAT ALL INFORMATION SHOWN ON THESE DRAWINGS HAS THE AFFIRMATION OF THE APPROVAL/REVIEW AUTHORITY. FAILURE TO RESPOND TO CLOUDED AREAS AND AREAS TO VERIFY MAY RESULT IN ADDITIONAL COSTS AND/OR SCHEDULE DELAYS FOR WHICH WHIRLWIND WILL NOT BE RESPONSIBLE. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION. ALL SUBSEQUENT CHANGES AFTER THE FIRST SUBMITTAL WILL BE CONSIDERED AS CONTRACTUAL CHANGES.

BY CHK SHEET DESCRIPTION:

RIGID FRAME ELEVATION

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VARIES

CUSTOMER LOCATION: 12320 S. Main Street

East Baton Rouge
DWG NO: ISSUE:

P.O.

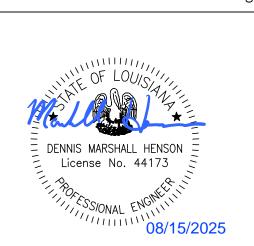
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IRL	WIND	STEEL	. BÜILDING
	75280 TX 77234		800-324-99 832-553-4

S S	TEEL BUILDINGS
280	PH: 800-324-9992
77234	FAX: 832-553-4600

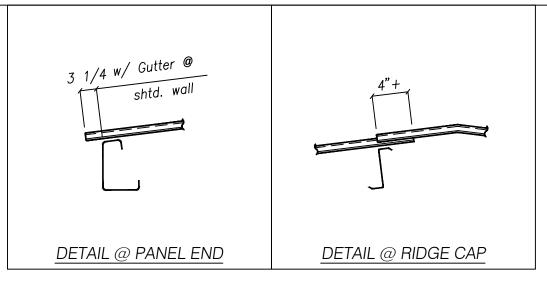
TEEL BUILDINGS	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:				CUSTON	IER LOCATION
TELE DOILDINGS						W	aukesha-Pea	rce Industries, I	1	123	20 S. Main
D						PROJECT REF	ERENCE: Pl Baton Rou	ıge			
PH: 800-324-9992 FAX: 832-553-4600						JOBSITE LOCA	ATION:		14 70017		JOBSITE COU
el Buildings Inc.								, Baton Rogue DATE:		JOB NO:	East Bat DWG NO:
•						NG	AM	8/ 7/25	DMH	14554-37882	



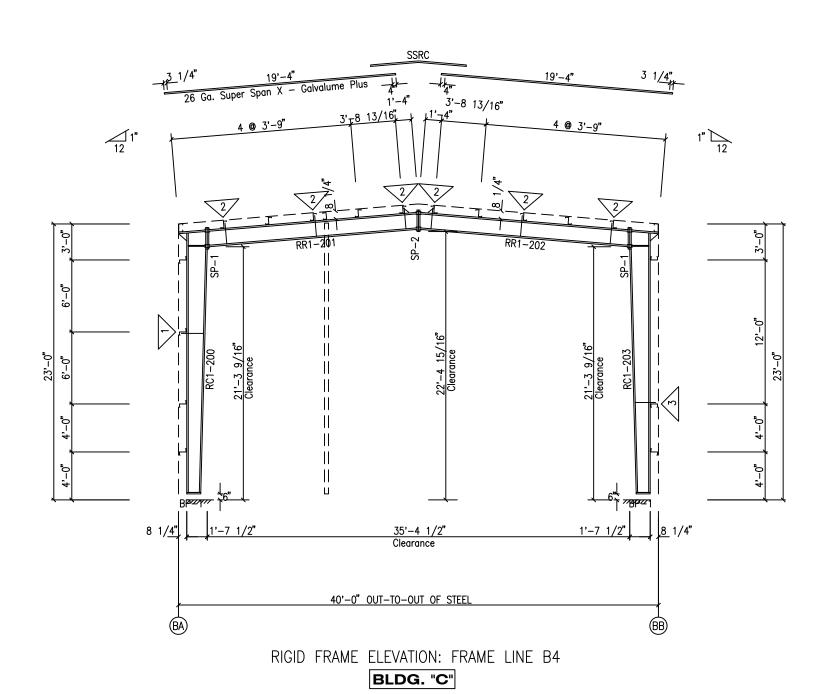
SPLICE PLATE & BOLT TABLE Mark Dia Width Thick 0 A325 0 A325 3/4" 2" 3/4" 1 3/4"

FLANGE BRACE TABLE
A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16
FRAME LINE: B4 OFFSET DETAIL CLIP ✓ ID SIDES LENGTH FB204A 2'-8 5/8" FB202A 2'-7 1/2" FB200A 2'-5 3/8" 2'-4" 2'-4" 2'-4" G25 AK222

BASE PLATE TABLE Plate Size Width Thick Length Mark BP-1 3/8" 11 1/2"



MEMBER TA	BLE				
Manual.	Web Depth	Web	Plate	Outside Flange	Inside Flange
Mark	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length
RC1-200	11.0/18.8	0.135	239.4	5 x 1/4" x 237.8	5 x 1/4" x 221.0
	18.8/19.0	0.135	24.0	5 x 1/4" x 24.0	5 x 1/4" x 24.0
	,			6 x 1/4" x 27.6	
RR1-201	14.0/14.0	0.135	213.0	5 x 1/4" x 211.8	5 x 1/4" x 211.8
RR1-202	14.0/14.0	0.135	213.0	5 x 1/4" x 211.8	5 x 1/4" x 211.8
RC1-203	14.0/14.0   19.0/18.8	0.135	24.0	6 x 1′/4" x 27.6	5 x 1/4" x 24.0
	18.8/11.0	0.135	239.4	5 x 1/4" x 24.0	5 x 1 <sup>′</sup> /4" x 221.0
				5 x 1/4" x 237.8	,



### BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug—Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Drawing Sialus

For approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete. Inseed drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete. Only drawings issued "For Erector Installation" can be considered complete. These drawings, being for approval, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete. These drawings, being for approval, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete. These drawings, being for approval, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete. These drawings, being for approval, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete. These drawings being for approval, are by definition not final and are for Erector Installation" can be considered complete. These drawings being for permit, are by definition on the considered complete. These drawings being for permit, are by definition on the considered complete. These drawings being for permit, are by definition on the considered complete. These drawings being for permit, are by definition on the considered complete. The control of the control of

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WHIRLWIND S	TEEL BUILD
P.O. BOX 75280 HOUSTON, TX 77234	PH: 800-324- FAX: 832-553

WHIR		9 81	EEL BU	JILDING
	X 75280 N, TX 772	234		-324-999 2-553-460
© 2005	Whirlwind	Steel	Buildinas	Inc.

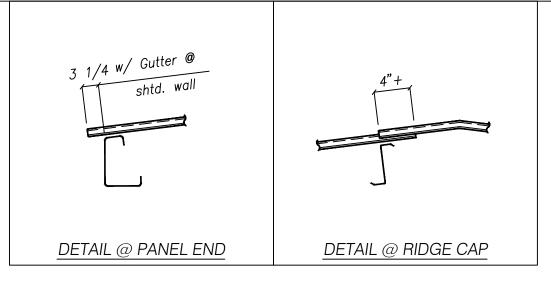
	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCR	ription: Id frame ele	VATION	1	BLDG SIZE: VARIES		
WHIRLWIND STEEL BUILDINGS		08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:	ID THUME ELL				IER LOCATION:	
						] w	Waukesha-Pearce Industries, LL			123	20 S. Main Stre	eet
					_	PROJECT REF	ERENCE:					
P.O. BOX 75280 PH: 800-324-9992						w	PI Baton Rou	ıge				
HOUSTON, TX 77234 FAX: 832-553-4600						JOBSITE LOCA		-			JOBSITE COUNTY	
1100310N, 1X 77254 1AX. 652-555-4000						Airline Highway, Baton Rogue LA 70817					East Baton	Rouge
© 2005 Whirlwind Steel Buildings Inc.						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
All rights reserved						NG	AM	8/ 7/25	DMH	14554-37882	P6	A1



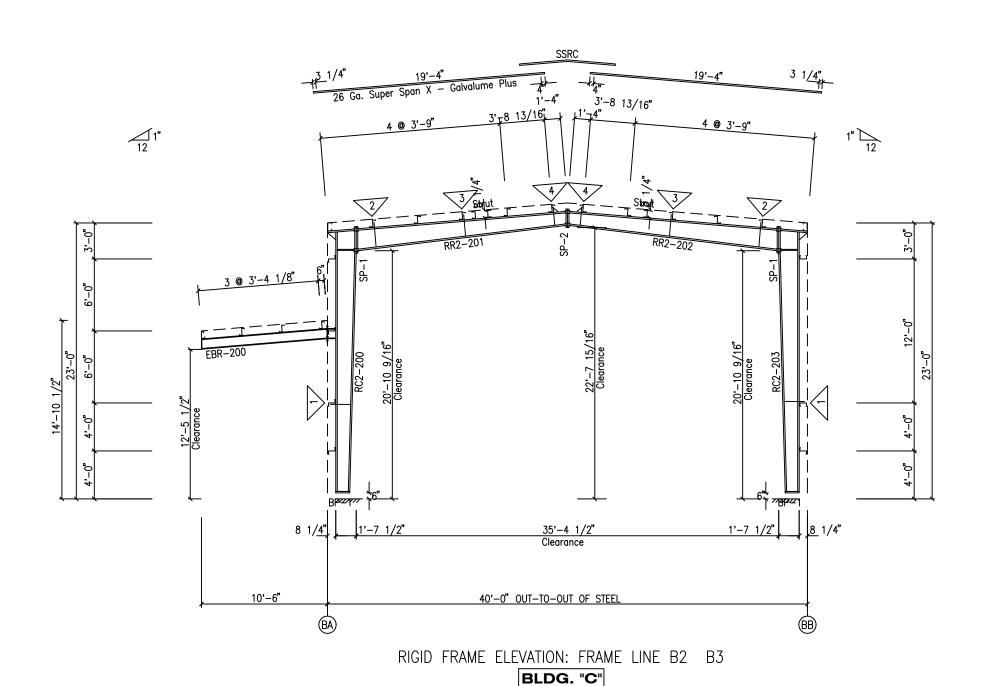
SPLICE PLATE & BOLT TABLE Mark Dia Thick 0 A325 0 A325 3/4" 2" 3/4" 1 3/4"

FLANGE BRACE TABLE
A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16 FRAME LINE: B3 B2 CLIP ✓ ID SIDES OFFSET DETAIL LENGTH FB200A 2'-5 3/8" FB205A 2'-9 3/4" FB203A 2'-7 7/8" FB201A 2'-6 3/8" 2'-4" 2'-4" 2'-4" 2'-4" G25 AK222

BASE PLATE TABLE Plate Size Width Thick Length Mark BP-1 1/2" 11 1/2"



MEMBER TA	BLE				
	Web Depth	Web F	Plate	Outside Flange	Inside Flange
Mark	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length
RC2-200	11.0/19.0	0.135	239.8	6 x 1/4" x 237.7	6 x 1/4" x 239.9
	19.0/19.0	0.161	23.5	6 x 1/4" x 24.0	,
	,			6 x 1/4" x 27.6	
RR2-201	19.0/11.0	0.135	213.4	5 x 1/4" x 211.8	5 x 1/4" x 212.6
RR2-202	11.0//19.0	0.135	213.4	5 x 1/4" x 211.8	5 x 1/4" x 212.6
RC2-203	19.0/19.0	0.161	23.5	6 x 1/4" x 27.6	6 x 1/4" x 239.9
	19.0/11.0	0.135	239.8	6 x 1/4" x 24.0	,
	,			6 x 1/4" x 237.7	
EBR-200	W10X22			,	
•		•			



### BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASIM F3123 Grade 7023 2000 accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts, June 11, 2020, accordance with the Specification of Structural Joints Using High—Strength Bolts Using All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in

direct tension indicator are not required. Installation inspection requirements for Snug—Tight Bolt is found in Section 9.1 of the Specification.

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ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

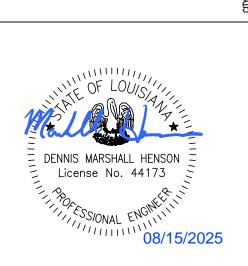
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William of ELL Boildings	
D.O. DOV. 75000 DIL. 800 704 0000	
P.O. BOX 75280 PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600	
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BY CHK SHEET DESCRIPTION:
RIGID FRAME ELEVATION ISSUE DATE WHIRLWIND STEEL BUILDINGS A1 08.15.25 FOR APPROVAL NG AM CUSTOMER: CUSTOMER LOCATION: 12320 S. Main Street PROJECT REFERENCE: JOBSITE LOCATION: East Baton Rouge
DWG NO: ISSUE: Airline Highway, Baton Rogue LA 70817 CHK: DATE: ENG: JOB NO: DWG NO: AM 8/ 7/25 DMH 14554-37882 P7



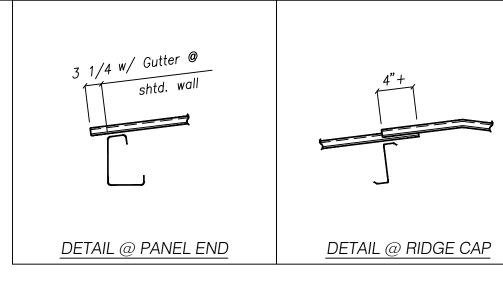
SPLICE PLATE & BOLT TABLE Qty Top Bot Int Type Mark Dia Length Thick 0 A325 0 A325 3/4" 2" 3/4" 1 3/4"

G25

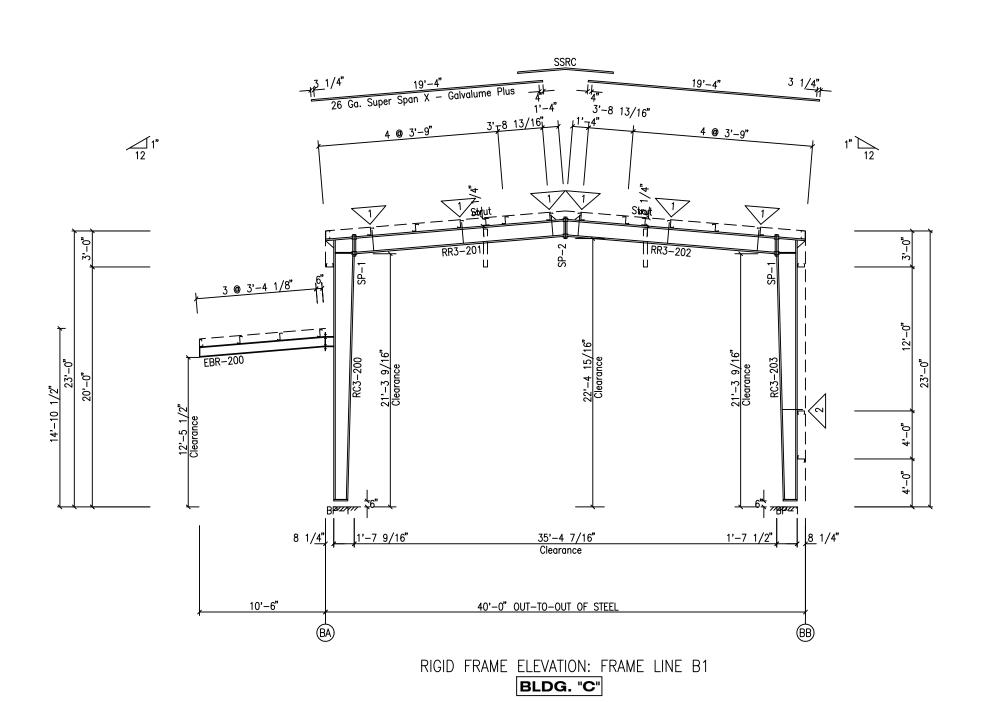
AK222

FLANGE BRACE TABLE
A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16
FRAME LINE: B1 MARK OFFSET DETAIL CLIP ✓ ID SIDES LENGTH 2'-4" 2'-4" FB202A 2'-7 1/2" FB200A 2'-5 3/8"

BASE PLATE TABLE Plate Size Width Thick Length Mark 1/2" 11 1/2"



MEMBER TA	BLE				
Manali	Web Depth Web Plate Ou			Outside Flange	Inside_Flange
Mark	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length
RC3-200	11.0/14.9	0.135	120.0	6 x 1/4" x 120.0	6 x 1/4" x 120.1
	14.9/19.0	0.164	143.3	6 x 5/16" x 141.7	6 x 5/16" x 124.8
				6 x 1/4" x 27.6	,
RR3-201	14.0/14.0	0.135	213.0	5 x 1/4" x 211.8	5 x 1/4" x 211.8
RR3-202	14.0/14.0	0.135	213.0	5 x 1/4" x 211.8	5 x 1/4" x 211.8 5 x 1/4" x 24.0
RC3-203	19.0/19.0	0.164	18.5	6 x 1/4" x 27.6	5 x 1 <sup>′</sup> /4" x 24.0
	19.0/18.2	0.135	24.0	5 x 1/4" x 24.0	5 x 1/4" x 220.9
	18.2/11.0	0.135	220.8	5 x 1/4" x 237.7	
EBR-200	W10X22			,	



## BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specification of Structural Joints Using High—Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug—Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Destancioning methods, including Turn—of—Nut, calibrated wrench, twist—off tension control bolts or inspection requirements for Snug—Tight Bolt is

Destancioning methods, including Turn—of—Nut, calibrated wrench, twist—off tension control bolts or inspection requirements for Snug—Tight Bolt is

Destancioning methods, including Turn—of—Nut, calibrated wrench, twist—off tension control bolts or inspection requirements for Snug—Tight Bolt is

Destancioning methods, including Turn—of—Nut, calibrated wrench, twist—off tension control bolts or inspection requirements for Snug—Tight Bolt is

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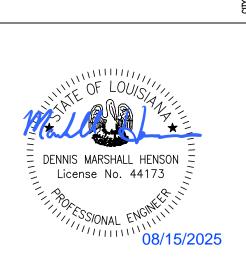
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MHIRL	WIND	STEEL BUILD
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HOUSTON,	TX 77234	FAX: 832-55

D. BOX 75280 JUSTON, TX 77234	PH: 800-324-9992 FAX: 832-553-460
2005 Whirlwind Steel	Buildings Inc.

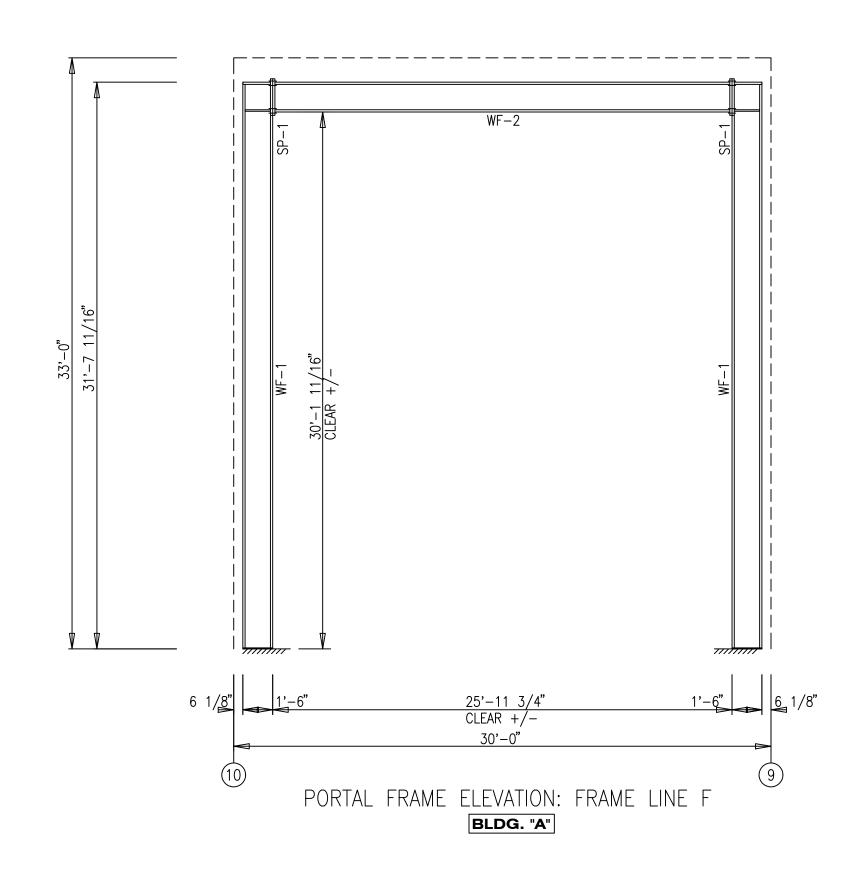
	ISSUE	DATE	DESCRIPTION	BY	СНК		SHEET DESCRIPTION: RIGID FRAME ELEVATION  CUSTOMER:							
<b>is</b>	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:					CUSTOMER LOCATION:			
						] w	aukesha-Pea	ırce Industries, l	T	123	20 S. Main Stre	et		
						PROJECT REF	ERENCE:							
92						W	WPI Baton Rouge							
32 300						JOBSITE LOCA	ATION:		JOBSITE COUNTY:					
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						NG	AM	8/ 7/25	DMH	14554-37882	P8	A1		

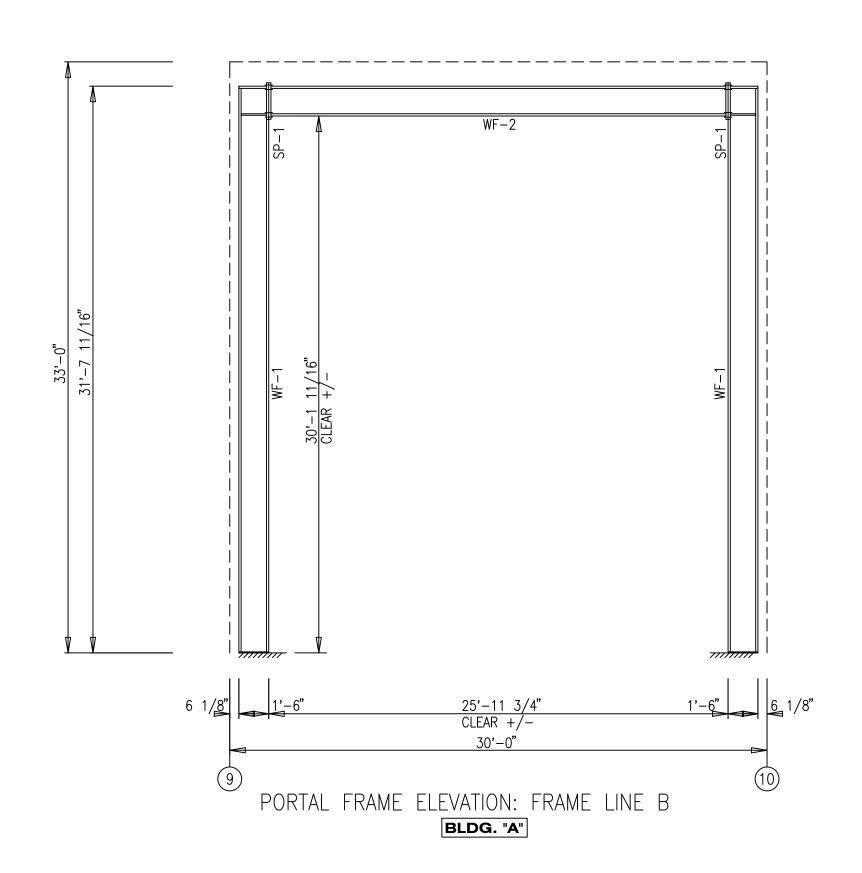


SPLICE BOI	LTS						
Splice Quan Mark Top/ Bo			Bolt				
Mark	Top/	Bot	Туре	Dia	Length		
SP- 1	4	4	A325	1.250	3.00		

<u>MEMBER SIZE</u>	TABLE	SPLICE BOLTS						
/ARK	MEMBER	LENGTH	Splice Mark	Quan	Bot		-Bolt Dia	
VF-2	W18083	25'-11 1/4"	Mark	Top/ I	DUL	lype	Dia	Length
VF-1	W18283	31'-7 11/16"	SP- 1	4	4	A325	1.250	3.00

MEMBER SIZE TABLE									
MARK	MEMBER	LENGTH							
WF-2 WF-1	W18083 W18283	25'-11 1/4" 31'-7 11/16"							





#### BOLT TIGHTENING (Pretensioned)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as pretensioned joints in accordance with the Specification for Structural Joints Using High-Strength Bolts, June 11, 2020. The specification recognizes five pretensioning methods without preference to any one; (1) Turn-of-Nut, (2) Calibrated Wrench, (3) Twist-Off Tension Control, (4) Direct tension Indicator, and (5) Combined Method.

The metal building manufacturer recommends the use of Turn-of-Nut method as it requires no special bolts, washers, or installation tools. When standard size holes are used with Turn-of-Nut method, and the nut is the part rotated, no washers are required per Section 6.2 of the Specification. The requirements of the Specification, Section 7 shall be completed before joint assembly.

Excerpts from the Specification, Section 8.2.1:

After the Snug-Tight operation has been performed, the nut rotation as specified in Table 8.1 shall be applied to all bolts in the joint, progressing systematically from the most rigid part of the joint in a manner that will minimize relaxation of previously pretensioned bolts.

Installation inspection requirements for Pretensioned joints using Turn-of-Nut method can be found in Section 9.2.1 of the Specification.

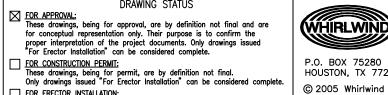
USE AK508 CLIP BUT CHANGE BOLTS TO 3/4" DIA. AND ADD 3/8" STIFFENERS TO CLIP. CLIP TO BE LOCATED AT THE TOP OF THE PORTAL FRAME AND AT THE CRANE LONGITUDINAL ANGLE BRACE ELEVATION.

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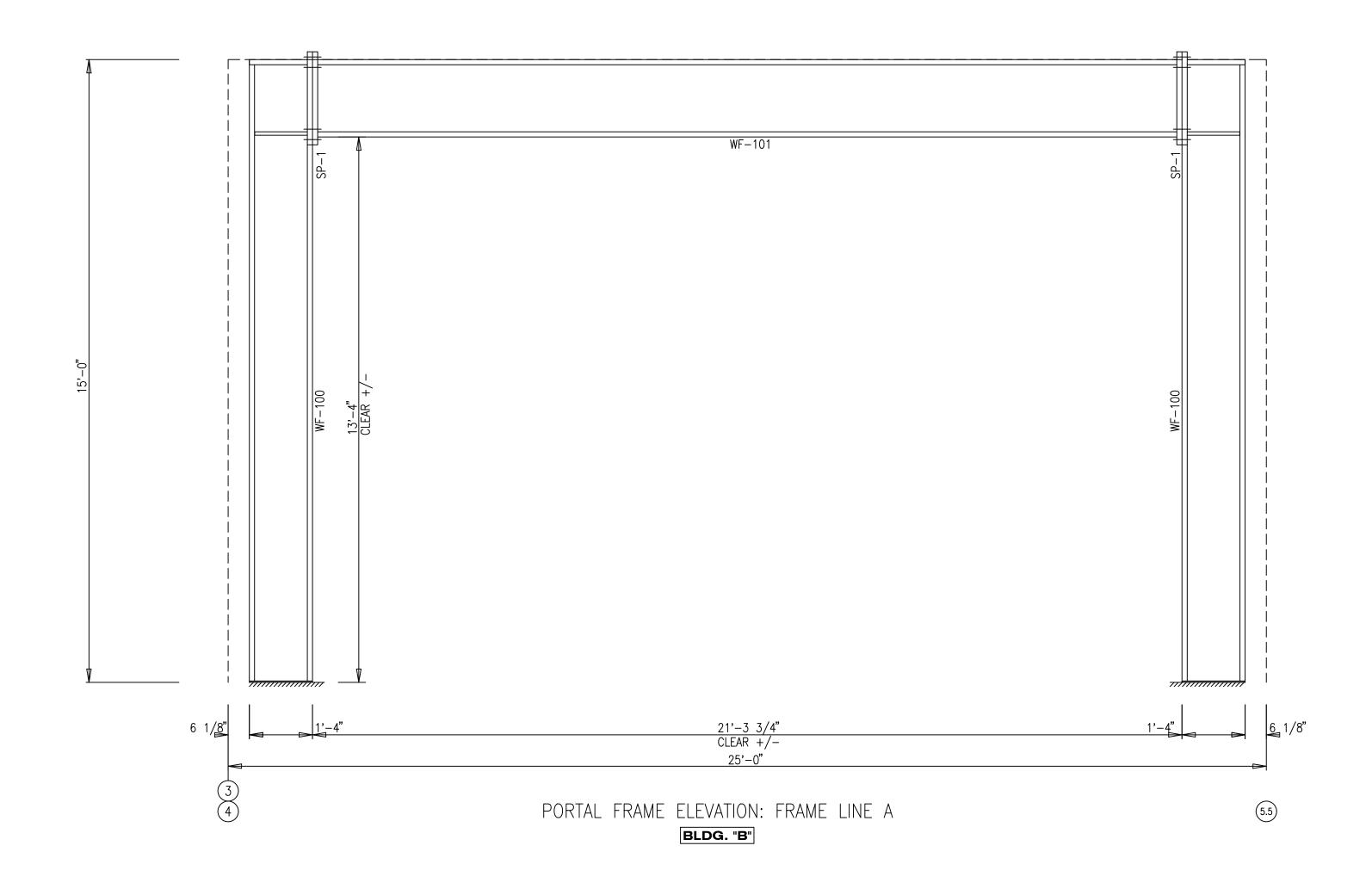
WHI	RLV	VIND	STEEL	. BUILDIN
P.O. B				800-324-9
HOUST	ON ⁻	TY 77234	. <b>ΓΔΥ</b> ·	832-553-

	ISSUE	DATE	DESCRIPTION	BY	CHK	PO	RTAL FRAME E	FI FVATION		VARIES		
WHIRI WIND STEEL BUILDINGS	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:					MER LOCATION:	
WHIRLWIND STEEL BUILDINGS								arce Industries,	LL	123	320 S. Main Stre	:et
	$\vdash$					PROJECT REI						
P.O. BOX 75280 PH: 800-324-9992						, v	VPI Baton Ro	uge				
HOUSTON, TX 77234 FAX: 832-553-4600						JOBSITE LOC					JOBSITE COUNTY:	
						1 4	Airline Highwa	y, Baton Rogue	LA 70817		East Baton	
© 2005 Whirlwind Steel Buildings Inc.	$\perp$					DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE
All rights reserved	1 1					NG	AM	8/ 7/25	DMH	14554-37882	W1	



SPLICE BOLTS Splice Mark Quan ----Bolt----Top/ Bot Type Dia Length ----Bolt----4 4 A325 0.750 2.00

MEMBER SIZE TABLE MARK MEMBER LENGTH W20851 W16861 21'-3 1/2" 15'-0"



USE AK508 CLIP BUT CHANGE BOLTS TO 3/4" DIA. AND ADD 3/8" STIFFENERS TO CLIP. CLIP TO BE LOCATED NEAR THE TOP OF THE PORTAL FRAME.

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Description of the Specification methods, including Turn—of—Nut, calibrated wrench, twist—off tension control bolts or the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Construction of the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integration of the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integration of the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integrated in the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integrated in the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integrated in the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integrated in the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integrated in the project documents are proper integrated in the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integrated in the project documents are proper integrated in the project documents. Only drawings issued "For Erector Installation" can be considered complete. For Erector Installation are proper integrated in the project documents are proper integrated i

Final drawings for construction



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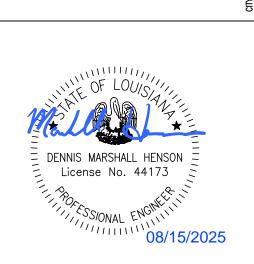
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	ISSUE	DATE	DESCRIPTION	BY	CHK	POI	RIPTION: RTAL FRAME E	LEVATION		VARIES		
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						PROJECT REF	ERENCE:					
992						] w	'PI Baton Roi					
992 4600						JOBSITE LOCA	ATION:	JOBSITE COUNTY:				
+000						Ai	irline Highway	, Baton Rogue	LA 70817		East Baton	Rouge
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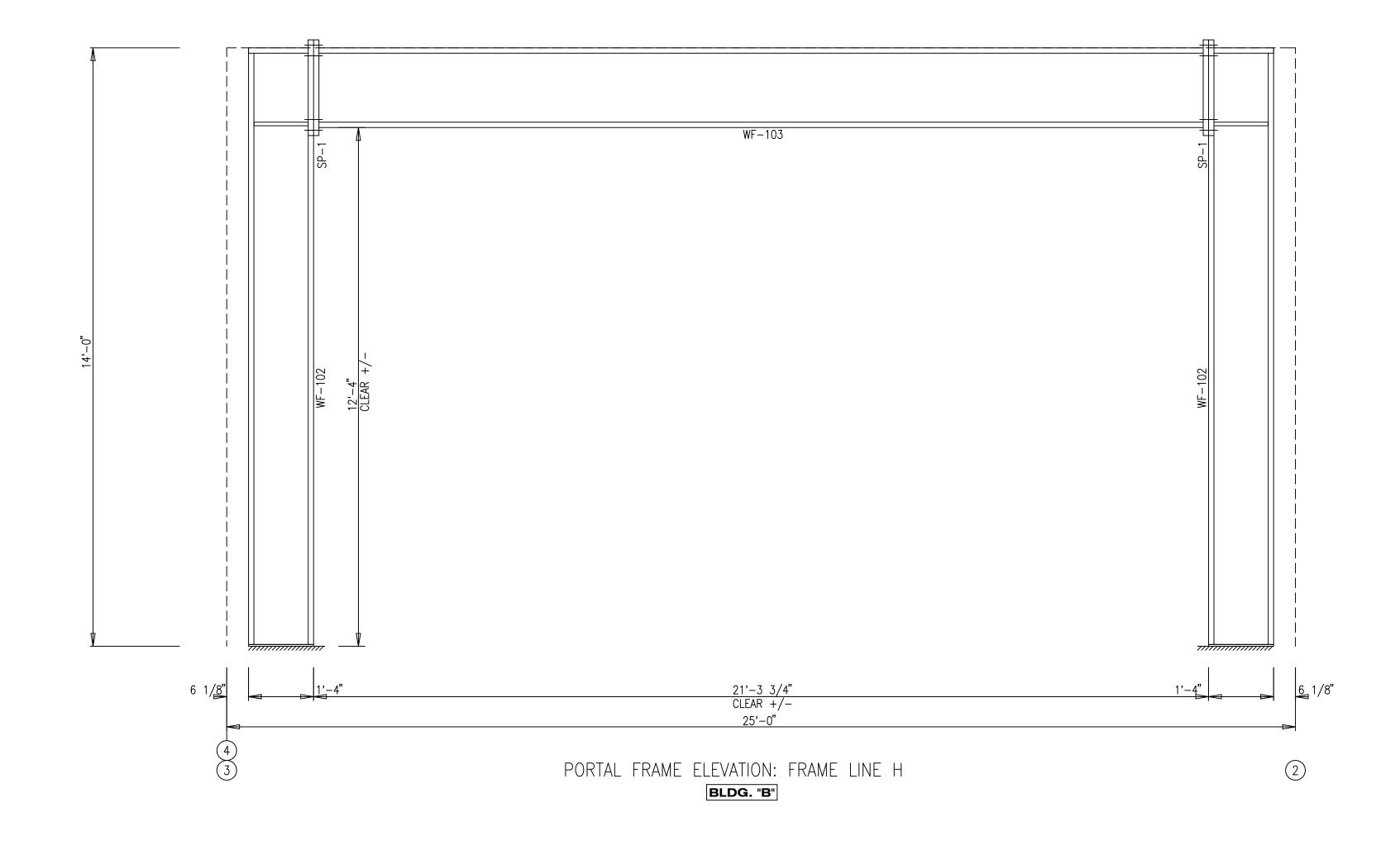
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SPLICE BOLTS Quan ----Bolt----Top/Bot Type Dia Length Splice Mark 4 4 A325 0.750 2.25

MEMBER SIZE TABLE MARK MEMBER LENGTH W20851 W16861 21'-3 1/4" 14'-0"



USE AK508 CLIP BUT CHANGE BOLTS TO 3/4" DIA. AND ADD 3/8" STIFFENERS TO CLIP. CLIP TO BE LOCATED NEAR THE TOP OF THE PORTAL FRAME.

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Final drawings for construction.



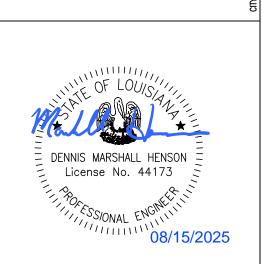
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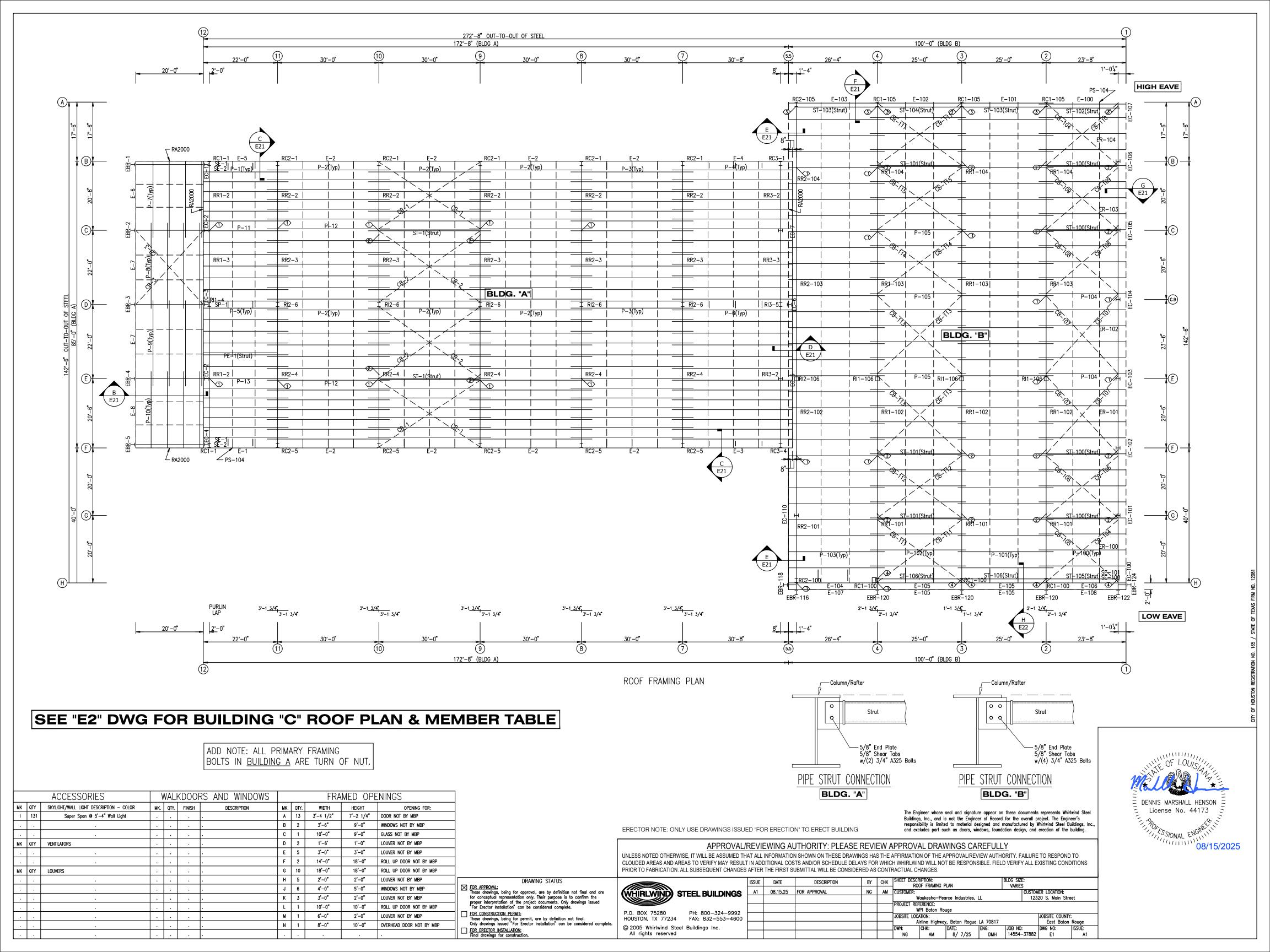
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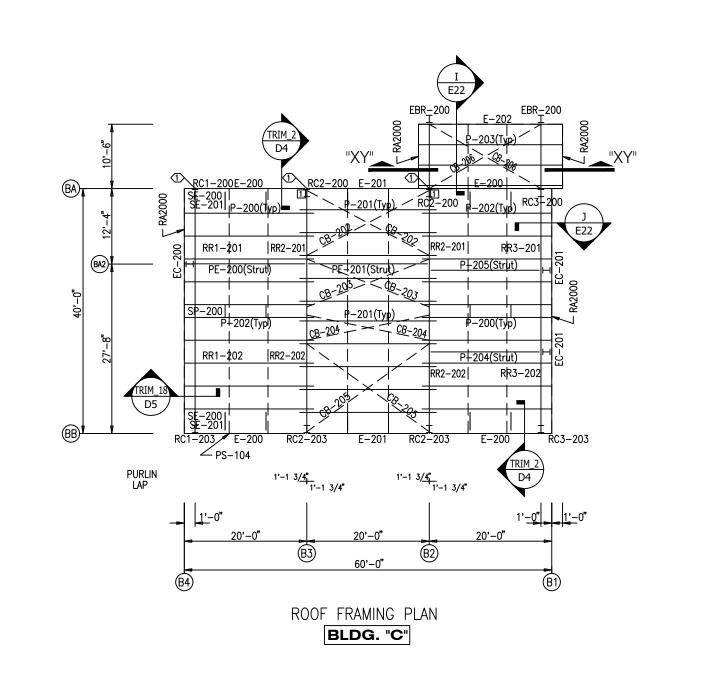
	ISSUE	DATE	DESCRIPTION	BY	СНК		HEET DESCRIPTION: BLDG S PORTAL FRAME ELEVATION VAF							
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92						W	WPI Baton Rouge							
600						JOBSITE LOCA	ATION:		JOBSITE COUNTY:	:				
600						A	irline Highway	, Baton Rogue		East Baton	Rouge			
						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:		
						NG	AM	8/ 7/25	DMH	14554-37882	W3	A1		

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#12x1 self-drilling pancake

Roof Purlin

Phillips Head Screw -Zinc

(Typ.)

Peak Spacer

(See Detail)

(5) SPACES MAX)

2"x26 ga. Strapping

(5) SPACES MAX)

-CA005 w/ (3) roof member

— SEE DETAIL"B"

(3) Pancake Phillips—

Screw

-(See Erection Drawings

for Strapping locations)

DETAIL"A"

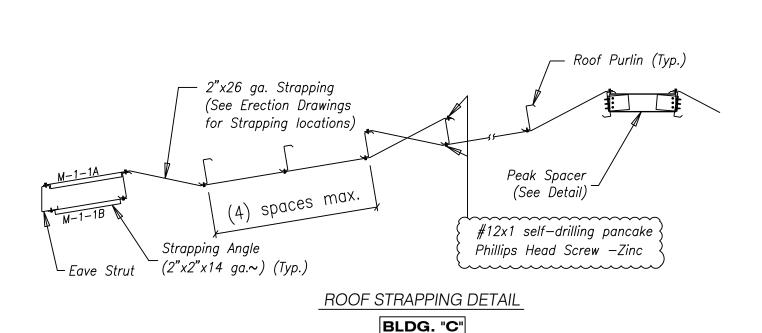
ROOF STRAPPING DETAIL

BLDG. "A" & "B"

*\\_\_ SEE DETAIL"A"* 

└─SA005 w/ (3x) Roof Member

screws each leg (Typ.)



P-100 10X25Z14 P-101 10X25Z14 P-102 10X25Z14 P-103 10X25Z12 P-104 10X25Z12 P-105 10X25Z12 E-100 10X25Z16 E-101 10X25Z16 E-102 10X25Z16 E-103 E-104 E-105 10X25Z16 10ES140 10ES140 E-106 10ES140 E-107 10ES140 E-108 10ES140 ST-100 ST-101 P0450237 P0450237 ST-102 P0450337 ST-103 P0556258 ST-104 ST-105 ST-106 CB-104 CB-105 P0556375 P0556258 P0556258 1.00\_R0D 1.00\_ROD CB-106 1.00\_ROD CB-107 0.50\_ROD CB-108 CB-109 CB-110 0.63\_ROD 0.75\_ROD 1.00\_ROD CB-111 1.00\_ROD CB-112 1.00\_ROD CB-113 0.50\_ROD CB-114 0.63\_ROD CB-115 SE-100 0.75\_ROD M - 1 - 1

SE-101

M-1-1

3/8" BACK UP PLATES REQUIRED AT RODS 3/4" OR GREATER ATTACHING TO WEBS 3/16" THICK OR LESS

MEMBER TABLE ROOF PLAN
MARK PART

BEAM

BEAM

BEAM

BEAM

10HES141

10X25Z16

10X25Z16

10X25Z12

10X25Z12 10X25Z16

10X25Z12 8X25Z16

8X25Z16

8X25Z16

8X25Z16

10X25Z14

10X25Z14

10X25Z14

10ES141

10ES141

10ES141

10ES141

10ES141

8ES141

8ES141

8ES141

P0556258

1.00\_ROD 0.50\_ROD 0.63\_ROD

6X25C14 M-1-1

M - 1 - 1

8ES141

8X25Z14

8X25Z14

8X25Z14

8X35Z16

W8X18

W8X18

8ES141

8ES141

8ES141

0.50\_ROD

0.50\_ROD

0.50\_ROD

0.50\_ROD

0.50\_ROD

6X25C14

BUILDING C EBR-200 W10X22 PE-200 8ES141

BUILDING A

EBR-1

EBR-2

EBR-3

EBR-4

EBR-5

PE-1

P-3

P-8

P-9 P-10

P-12

ST-1

CB-1 CB-2 CB-3

SP-1

SE-1

SE-2

PE-201

P-200

P-201

P-202 P-203 P-204 P-205

E-200

E-201

E-202

CB-202

CB-203

CB-204

CB-205

CB-206 0.50\_RC SP-200 6X25C1 SE-200 M-1-1 SE-201 M-1-1

CONNECTION PLATES

ROOF PLAN

DID MARK/PART

1 AK106

2 1/4"

1 3/4" 1 3/4"

Peak Spacer (14 ga. "Cee" Channel) 2"x26 ga. Strapping (See Erection Drawings Peak Purlin for Strapping locations) ¯(Тур.) CA005 w/ (3x) Roof Member screws each leg (Typ.)

PEAK SPACER ATTACHMENT DETAIL

screws

DETAIL"B"

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EXTENSION/CANOPY BOLTS ROOF PLAN

A325 A325 A325

A325

A325 A325 A325 A325 A325

A325

1/2" 5/8" 5/8" 3/4"

MEMBER TABLE

MARK PART

BUILDING B EBR-116 W10X12

EBR-118 W10X12

EBR-120 W10X22

EBR-122 | W10X12

EBR-124 W10X12

ROOF PLAN

A307 A325 A325 A325 A325

MARK EBR-

EBR-2

EBR-3

EBR-4

EBR-5

EBR-11

EBR-11

EBR-12

EBR-12

EBR-12

EBR-20

SPECIAL BOLTS
ROOF PLAN
D QUAN

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DRAWING STATUS	
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FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.	P.O. BOX 75280 HOUSTON, TX 77234

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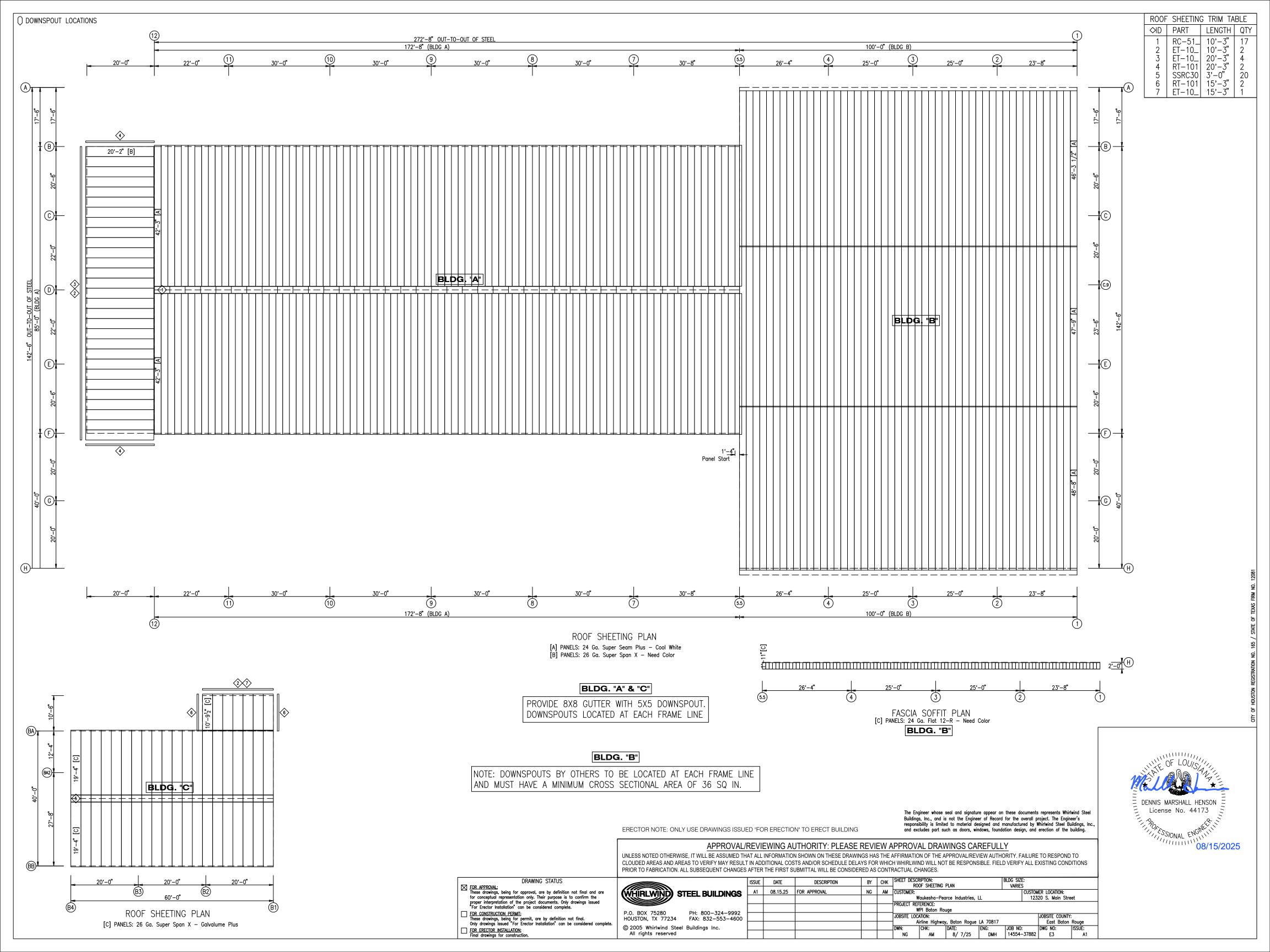
DRAWING STATUS

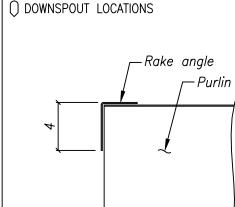
WIND) S	TEEL BUILDING
75280	PH: 800-324-999

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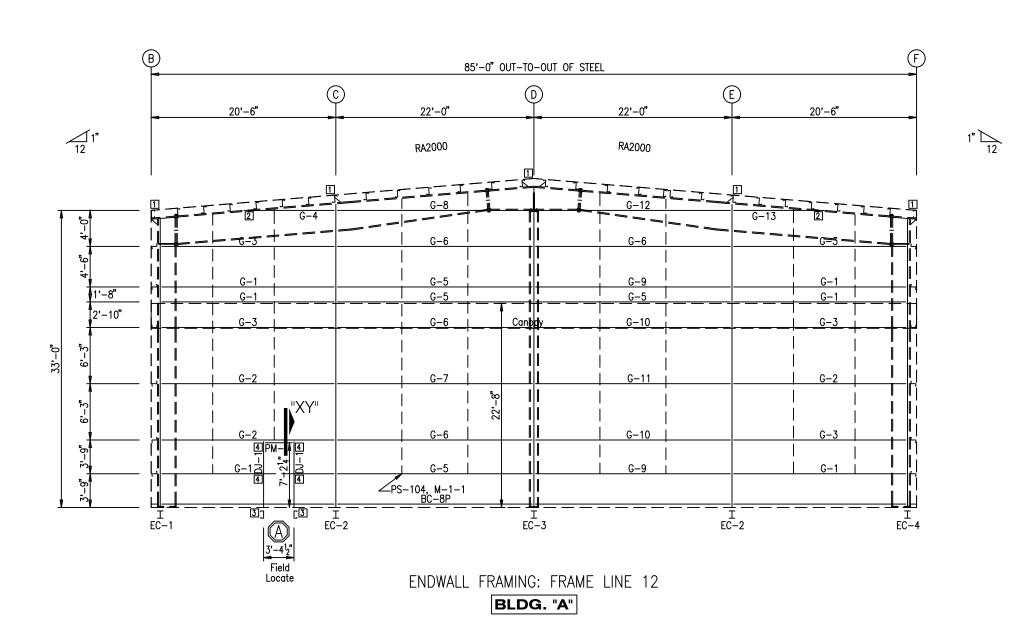
ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCR ROO	IPTION: OF FRAMING P	LAN		BLDG SIZE: VARIES			
A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:	CUSTOMER: CUSTOMER LOCATION:						
					w	aukesha-Pea	rce Industries, I	L	123	320 S. Main Stree	et	
					PROJECT REF	PROJECT REFERENCE:						
				l								
					WPI Baton Rouge							
					JOBSITE LOCATION: JOBSITE COUNTY:							
					Ai	rline Highway	, Baton Rogue	LA 70817		East Baton	Rouge	
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					NG	AM	8/ 7/25	DMH	14554-37882	E2	A1	

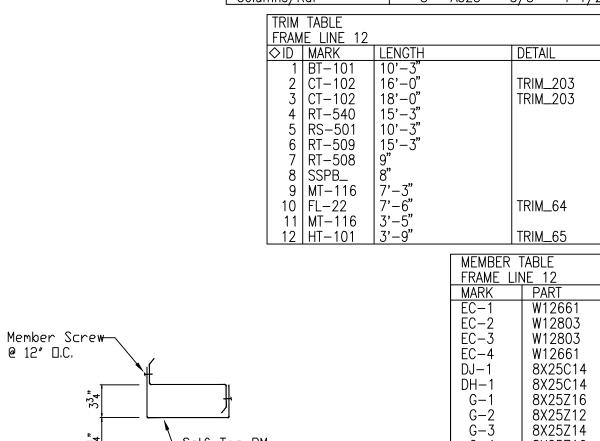






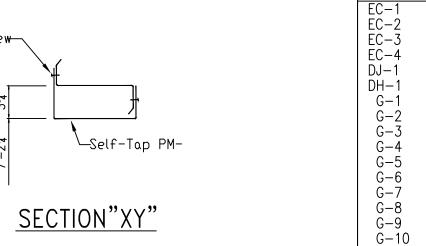
Detail at Rake Angle





BOLT TABLE FRAME LINE 12

LOCATION Columns/Raf



		0/100212
G-	12	8X25Z14
G-	13	8X25Z16
CON	<b>IECTIO</b>	N PLATES
FRAM	E LIN	E 12
	MARK	(/PART
1	n1	•
2	d1	
3	AK40	1
4	AK20	0
5	AB20	1

G-11

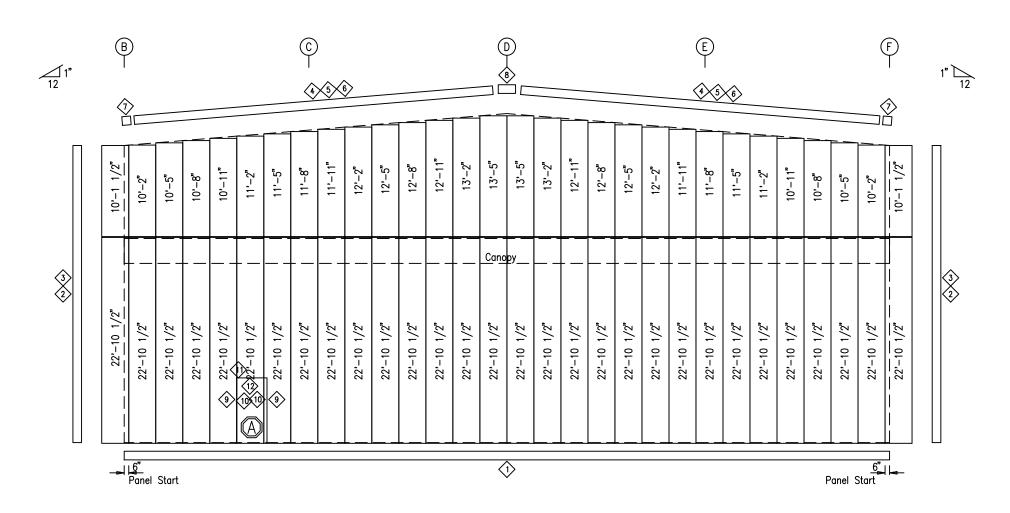
8X25Z16 8X25Z16

8X35Z14 8X35Z12 8X25Z14

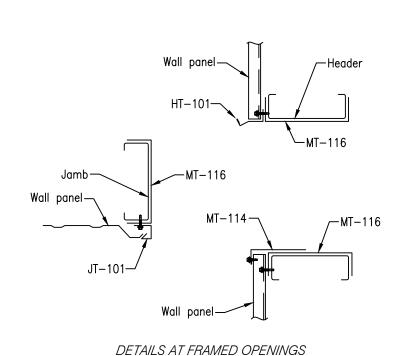
8X25Z16

8X35Z14

8X35Z12



ENDWALL SHEETING & TRIM: FRAME LINE 12 PANELS: 26 Ga. Super Span X - NEED COLOR BLDG. "A"



@ 12″ □.C.

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc. and excludes part such as doors, windows, foundation design, and erection of the building.

#### ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY

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DRAWING STATUS

FOR APPROVAL:

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FOR CONSTRUCTION PERMIT:
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BOX 75280 TON, TX 77234	PH: 800-324-9992 FAX: 832-553-460

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ISSUE	DATE	DESCRIPTION	BY	CHK	FR/		NG ELEVATION		VARIES			
A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:		arce Industries.	<u>_</u> 	CUSTON	MER LOCATION: 20 S. Main Stre		
					PROJECT REF		irce industries,		120	20 3. Maii 300	-	
					w	WPI Baton Rouge						
					JOBSITE LOCA			14 70047		JOBSITE COUNTY		
					DWN:	irline Highway ICHK:	y, Baton Rogue    DATE:	LA 70817 TENG:	JOB NO:	East Baton	ISSUE:	
					NG	AM	8/ 7/25	DMH	14554-37882		A1	

## DENNIS MARSHALL HENSON License No. 44173 PORESSIONAL ENGINEE 08/15/2025

#### GENERAL SHEETING & TRIM NOTES:

1. Refer to erection drawings for rake angle locations. 2. Roof member screws are on 12" centers at the intermediate purlins. The spacing at the eave, end lap, and peak purlins are

3. Wall member screws are on 6" centers at the base member and 12" centers for all remaining members.

4. Roof stitch screws are located (1) at each member and (2) between members spaced evenly apart (20" maximum spacing). 5. Wall stitch screws are located (1) at each member then spaced evenly apart between members with the spacing not to

**GENERAL FRAMING NOTES:** 

Angles are marked by their length in feet and inches. Field cut or lap angles as required to fit. Flange braces are marked by their length in decimal inches. Outside flange of girt turns down unless noted.

Endwall girts and eave struts do not lap.
Field cut and self—tap girts at walk doors.

Field slot girts for brace rods or cables.

(2) 5/8" x 1-1/2" A325 bolts if (1) AK400 required. (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 required.

11. Locate top of roof framed openings flush with the pan of the roof

panel. 12. Some field drilling at framed openings may be required. Field drill

Sub-jambs for overhead or roll-up doors, if required, are not furnished by Metal Building Provider.

Field locate windows and walk doors. 9. Field weld all splices at 14 gauge valley gutters.10. Field Bolt Ak400 base clip to endwall columns:

9/16" diameter holes.

exceed 20".

6. Skylight stitch screws are at 6" o.c.

7. Start endwall panels at centerline of bldg. unless noted.

8. Gutter, rake, & eave trim lap 2". All other trims lap 1".

Field cut or lap panels as required to fit.
 Field cut panels for all openings.
 Pop rivet gutter counterflashing to wall panel on 3'-0" centers and caulk all laps.

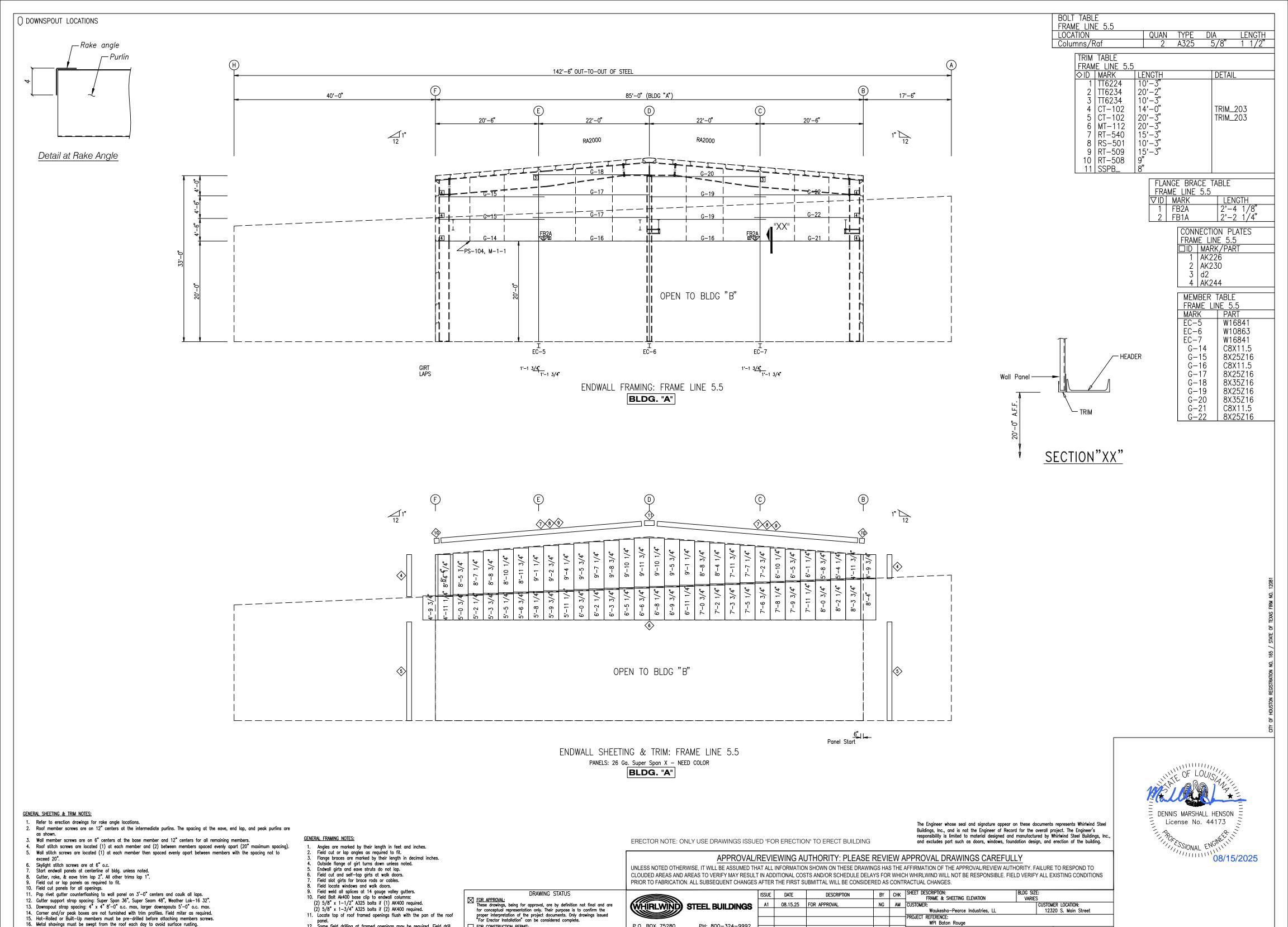
12. Gutter support strap spacing: Super Span 36", Super Seam 48", Weather Lok-16 32".

13. Downspout strap spacing: 4" x 4" 8'-0" o.c. max, larger downspouts 5'-0" o.c. max.

14. Corner and/or peak boxes are not furnished with trim profiles. Field miter as required. 15. Hot-Rolled or Built-Up members must be pre-drilled before attaching members screws.

16. Metal shavings must be swept from the roof each day to avoid surface rusting. Windows and louvers must be installed before sheeting the walls.

18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the appropriate standing seam technical/erection manual or standard pull outs for through-fastened (screw-down) type roof systems for additional installation instructions.



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(2) 5/8" x 1-3/4" A325 bolts if (2) AK400 required.

Hot-Rolled or Built-Up members must be pre-drilled before attaching members screws.

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9/16" diameter holes.

13. Sub-jambs for overhead or roll-up doors, if required, are not furnished by Metal Building Provider.

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Final drawings for construction.

12320 S. Main Street JOBSITE COUNTY: East Baton Rouge | DATE: | ENG: | JOB NO: | DWG NU: | 8/ 7/25 | DMH | 14554-37882 | E5

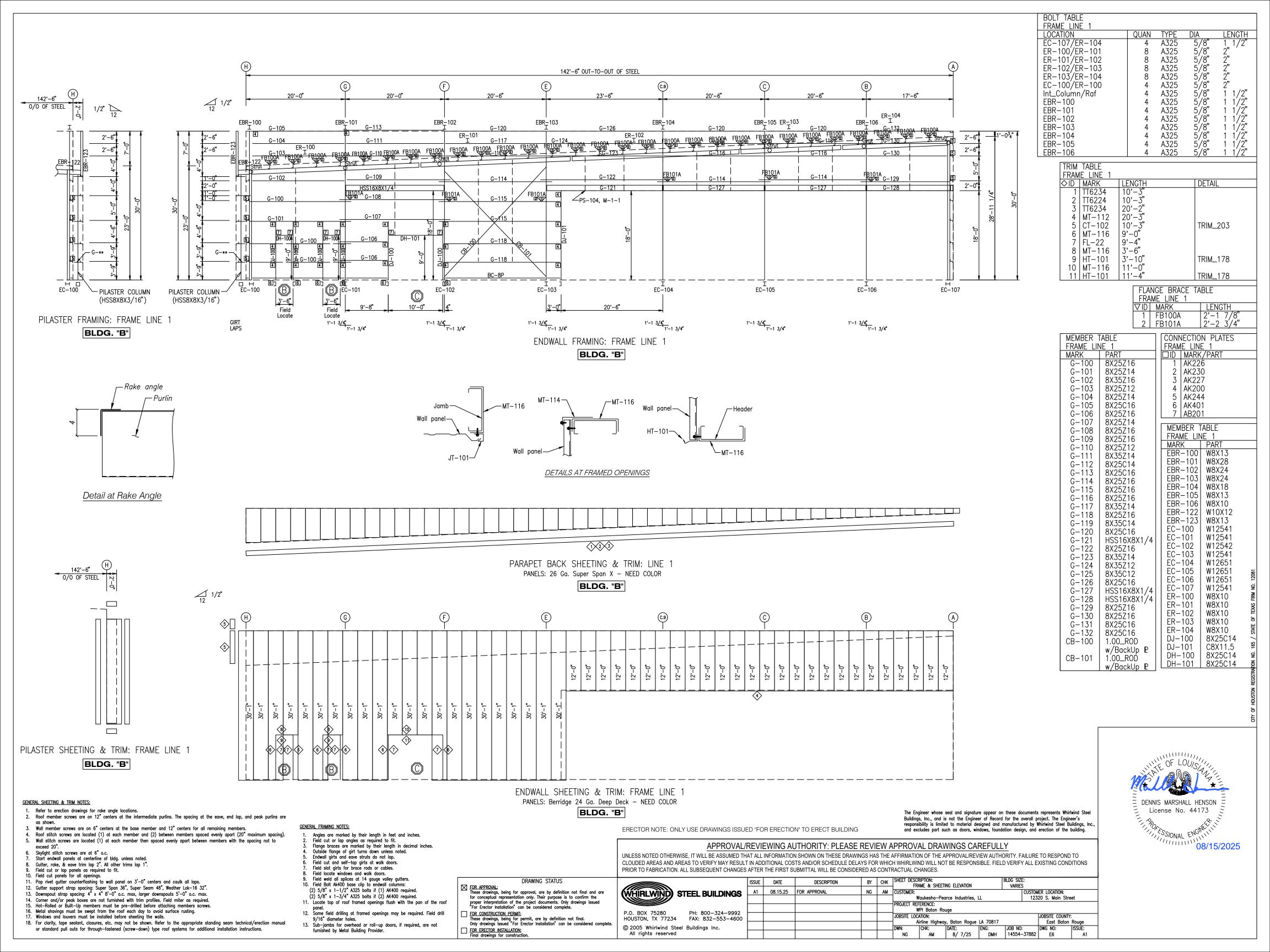
Waukesha-Pearce Industries, LL

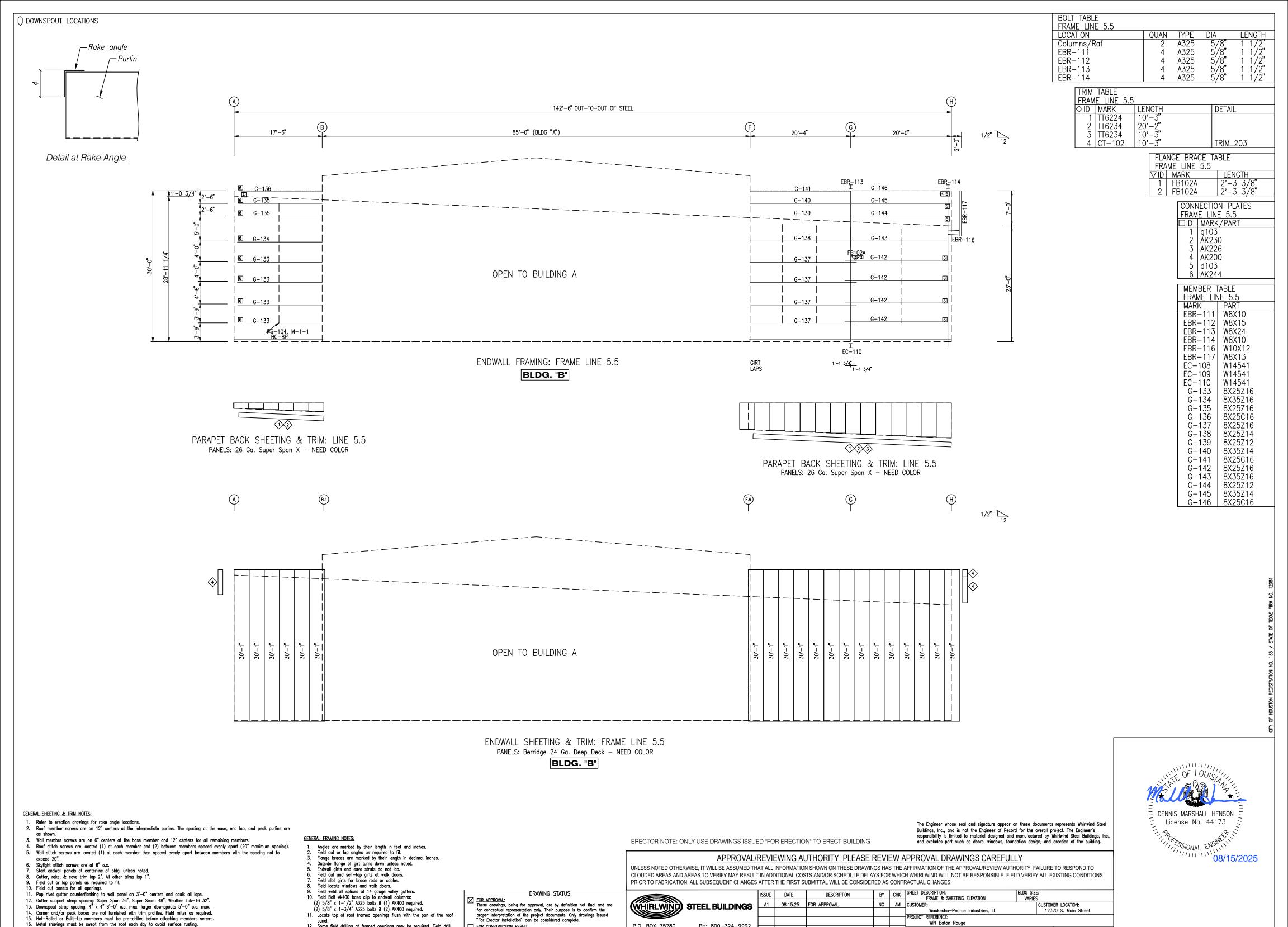
Airline Highway, Baton Rogue LA 70817

WPI Baton Rouge

PROJECT REFERENCE:

JOBSITE LOCATION:





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Final drawings for construction.

PROJECT REFERENCE:

JOBSITE LOCATION:

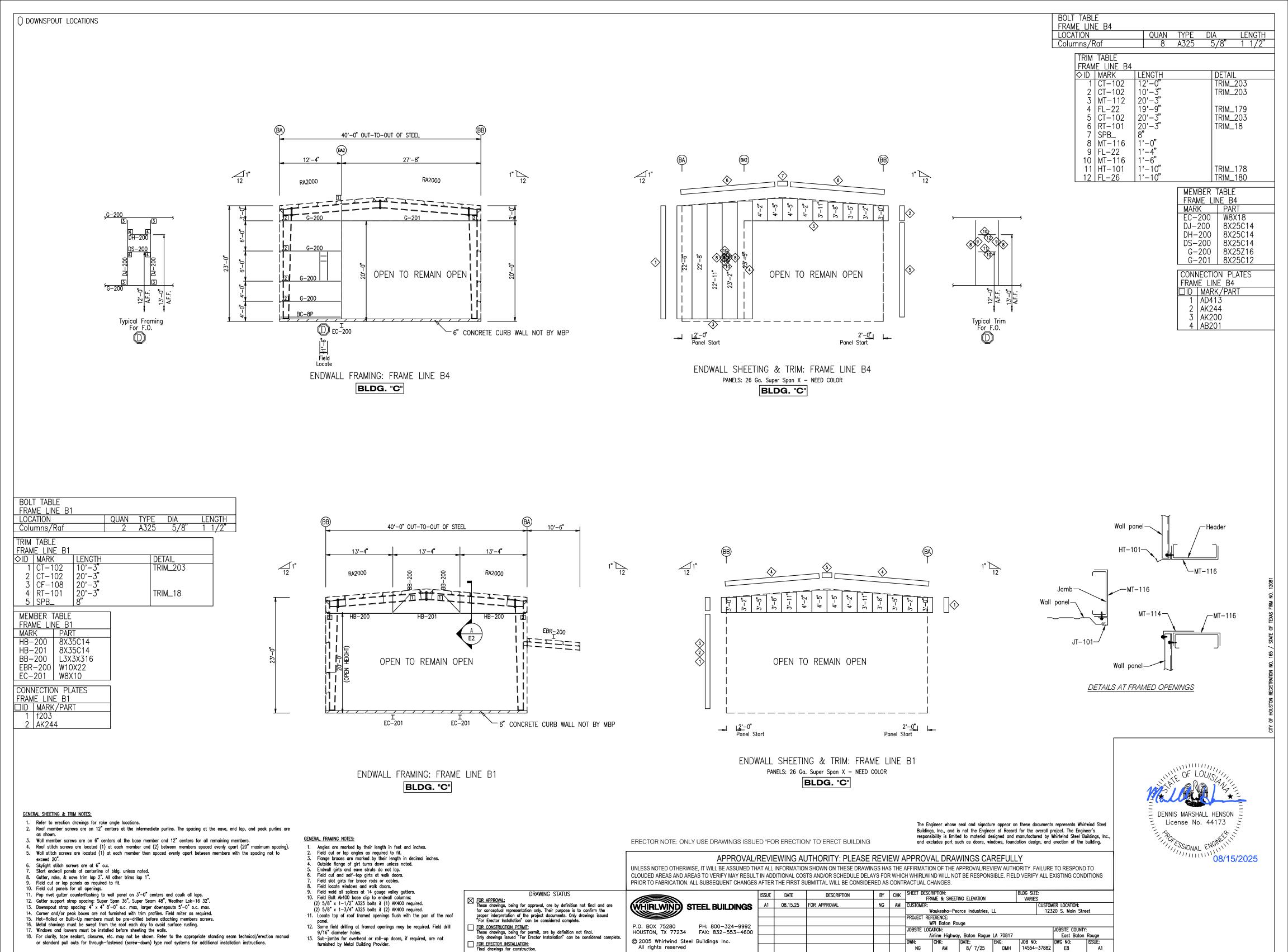
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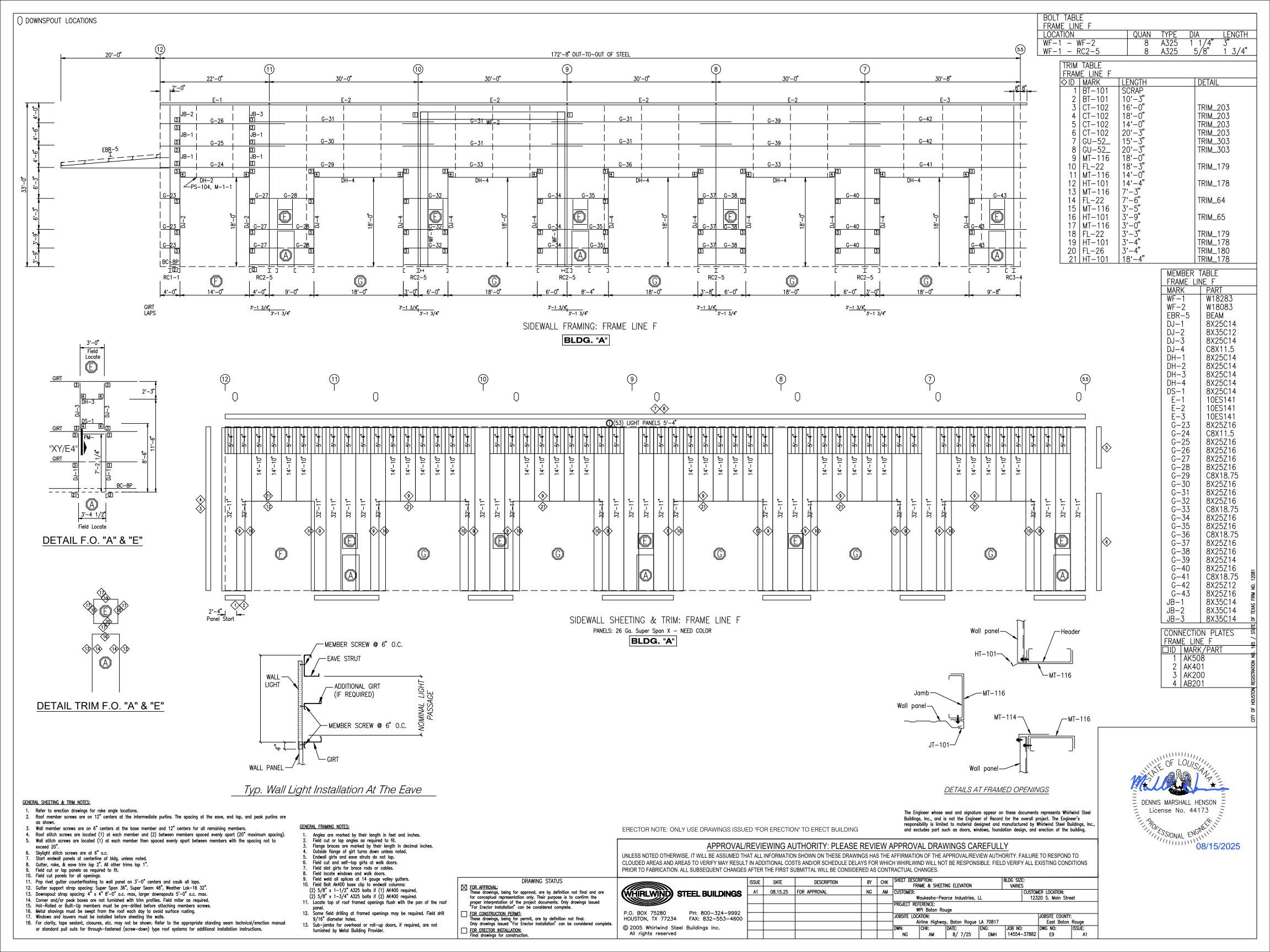
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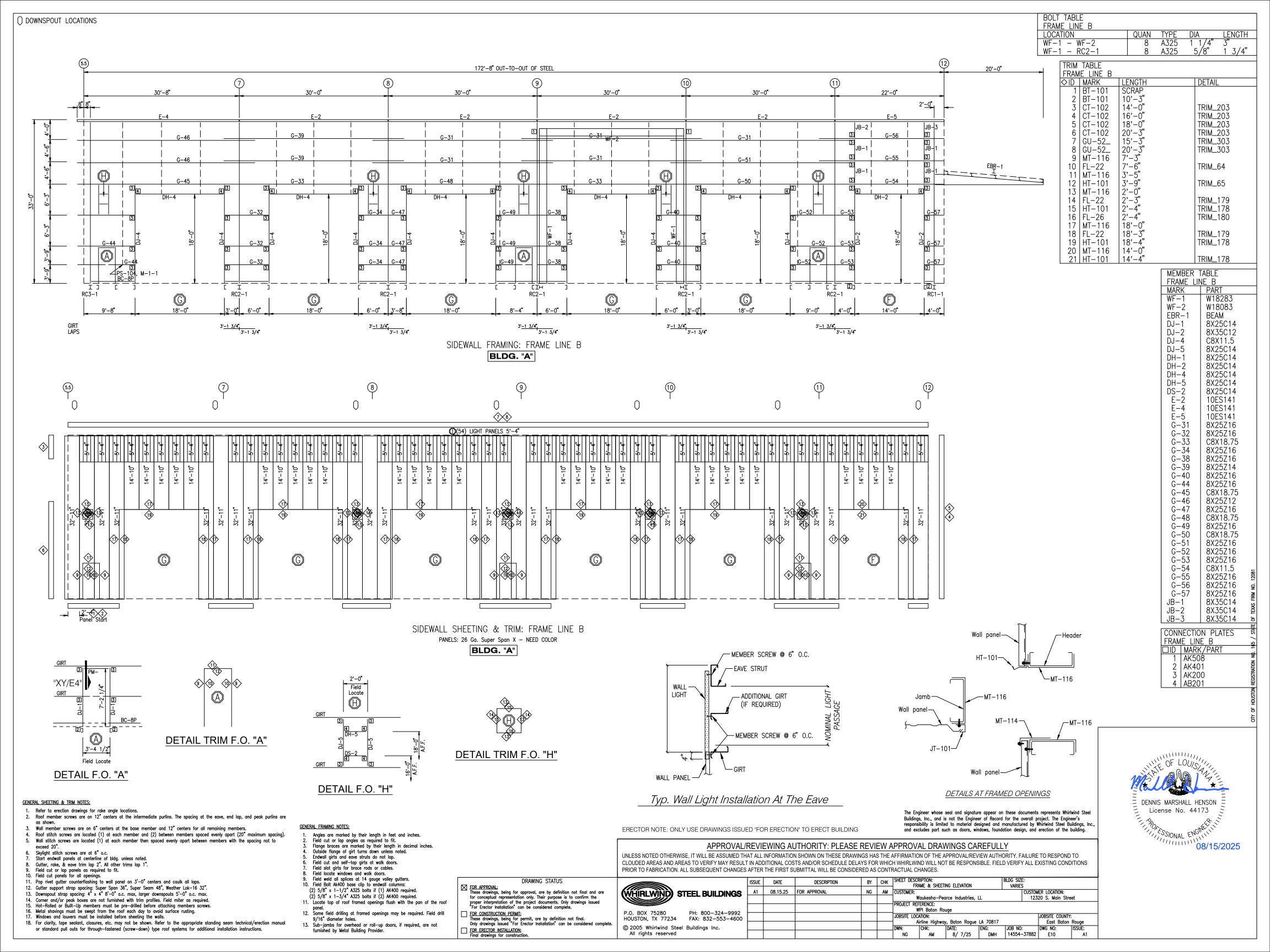
| DATE: | ENG: | JOB NO: | DWG NU: | 8/ 7/25 | DMH | 14554-37882 | E7

JOBSITE COUNTY:

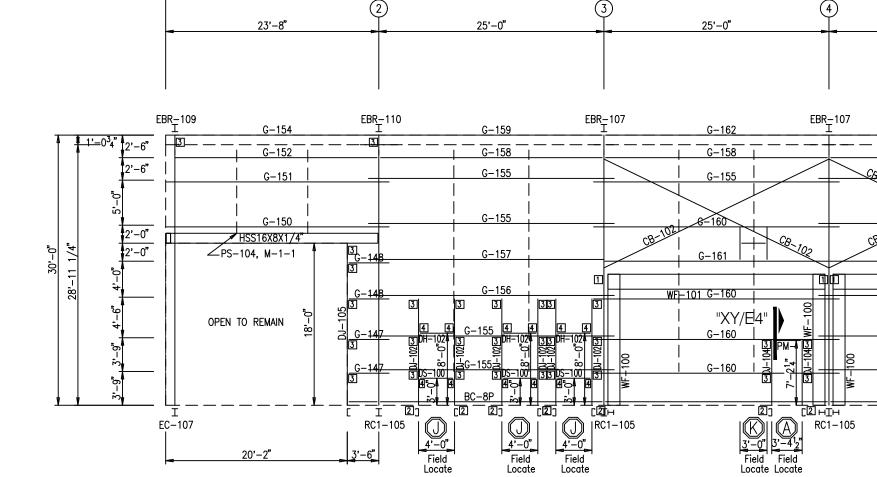
East Baton Rouge









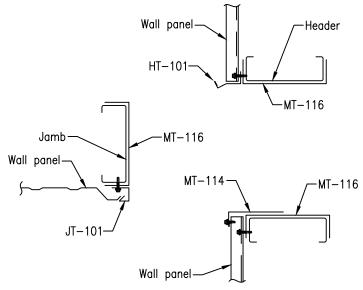


1'-1 3/4"
1'-1 3/4"

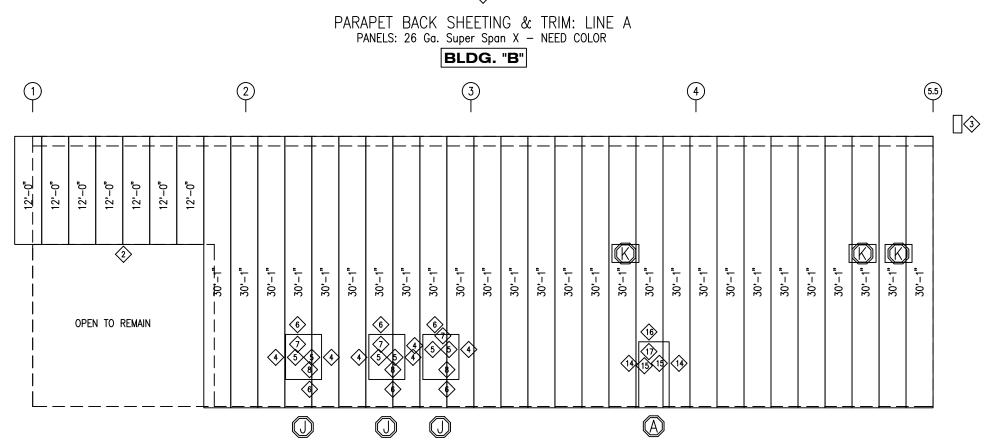
3'-0"

DETAIL F.O. "K"

DETAIL TRIM F.O. "K"



DETAILS AT FRAMED OPENINGS



100'-0" OUT-TO-OUT OF STEEL

Field Locate

SIDEWALL FRAMING: FRAME LINE A BLDG. "B"

Field Field Locate Locate

SIDEWALL SHEETING & TRIM: FRAME LINE A PANELS: Berridge 24 Ga. Deep Deck - NEED COLOR BLDG. "B"

#### GENERAL SHEETING & TRIM NOTES:

- Refer to erection drawings for rake angle locations. 2. Roof member screws are on 12" centers at the intermediate purlins. The spacing at the eave, end lap, and peak purlins are
- 3. Wall member screws are on 6" centers at the base member and 12" centers for all remaining members.
- 4. Roof stitch screws are located (1) at each member and (2) between members spaced evenly apart (20" maximum spacing). 5. Wall stitch screws are located (1) at each member then spaced evenly apart between members with the spacing not to
- wall statch screws are located (1) at each member then sexceed 20".
  Skylight stitch screws are at 6" o.c.
  Start endwall panels at centerline of bldg. unless noted.
  Gutter, rake, & eave trim lap 2". All other trims lap 1".
- Field cut or lap panels as required to fit.
   Field cut panels for all openings.
   Pop rivet gutter counterflashing to wall panel on 3'-0" centers and caulk all laps.
- 12. Gutter support strap spacing: Super Span 36", Super Seam 48", Weather Lok-16 32".

  13. Downspout strap spacing: 4" x 4" 8'-0" o.c. max, larger downspouts 5'-0" o.c. max.
- Corner and/or peak boxes are not furnished with trim profiles. Field miter as required.
   Hot-Rolled or Built-Up members must be pre-drilled before attaching members screws.
- 16. Metal shavings must be swept from the roof each day to avoid surface rusting. Windows and louvers must be installed before sheeting the walls.

18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the appropriate standing seam technical/erection manual

or standard pull outs for through—fastened (screw—down) type roof systems for additional installation instructions.

**GENERAL FRAMING NOTES:** 

GIRT LAPS

- Angles are marked by their length in feet and inches. Field cut or lap angles as required to fit. Flange braces are marked by their length in decimal inches. Outside flange of girt turns down unless noted.
- Endwall girts and eave struts do not lap.
  Field cut and self—tap girts at walk doors.
- Field slot girts for brace rods or cables. Field locate windows and walk doors.
- Field weld all splices at 14 gauge valley gutters.
   Field Bolt Ak400 base clip to endwall columns: (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 required. (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 required.
- 11. Locate top of roof framed openings flush with the pan of the roof 12. Some field drilling at framed openings may be required. Field drill
- 9/16" diameter holes. Sub-jambs for overhead or roll-up doors, if required, are not furnished by Metal Building Provider.
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1'-4"

EBR<u>-</u>108

Field Field Locate

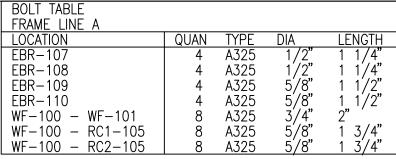
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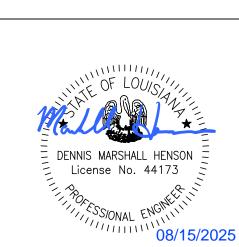
	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCR		NG ELEVATION		BLDG SIZE: VARIES		
	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:						
•						W	aukesha-Pea	ırce Industries, l	L	123	20 S. Main Stre	et
						PROJECT REF	ERENCE: 'PI Baton Roi	uge				
0						learner earnings				JOBSITE COUNTY East Baton		
									ENG:	JOB NO:	DWG NO:	ISSUE
						NG	AM	8/ 7/25	DMH	14554-37882	E11	

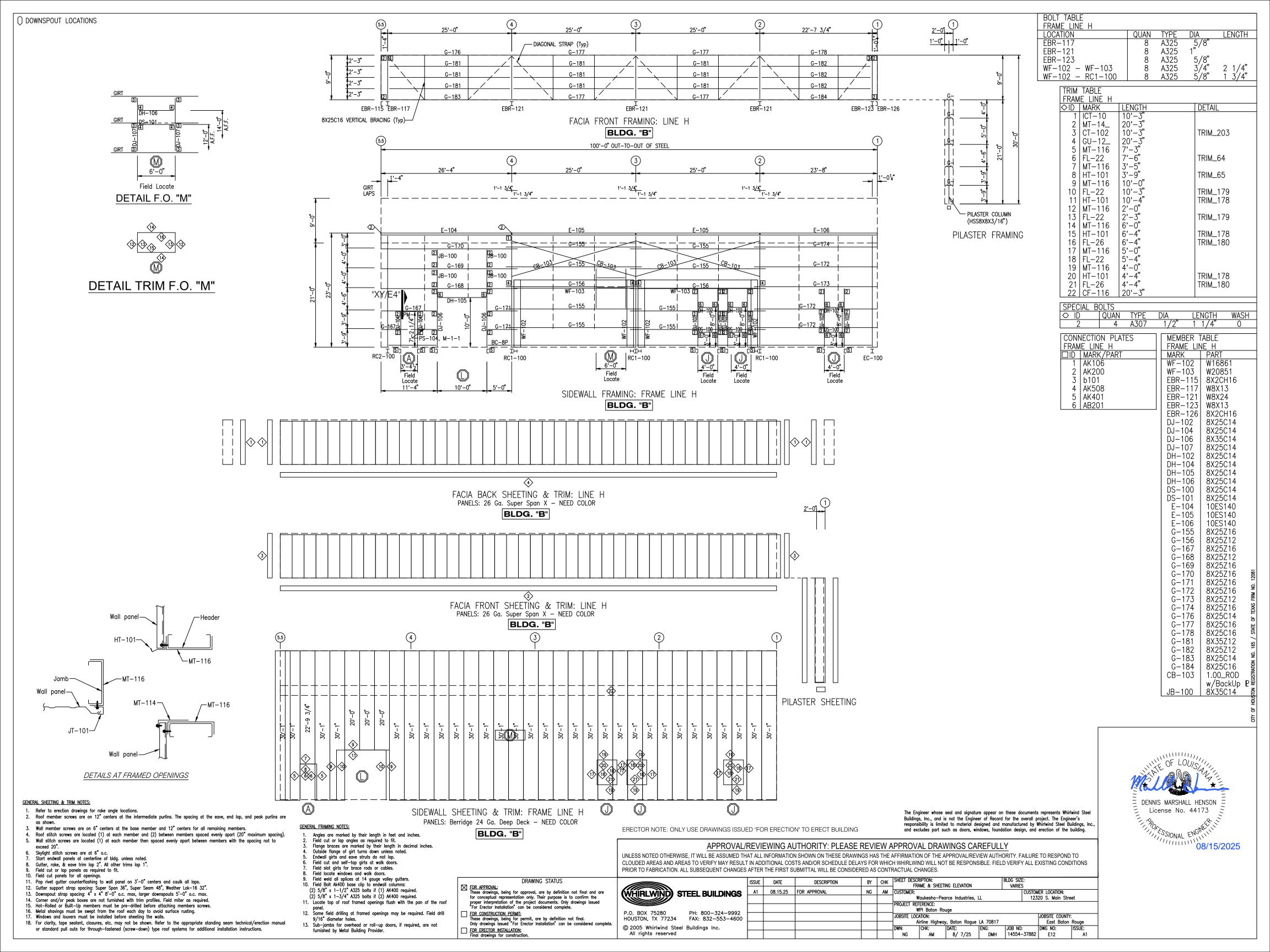


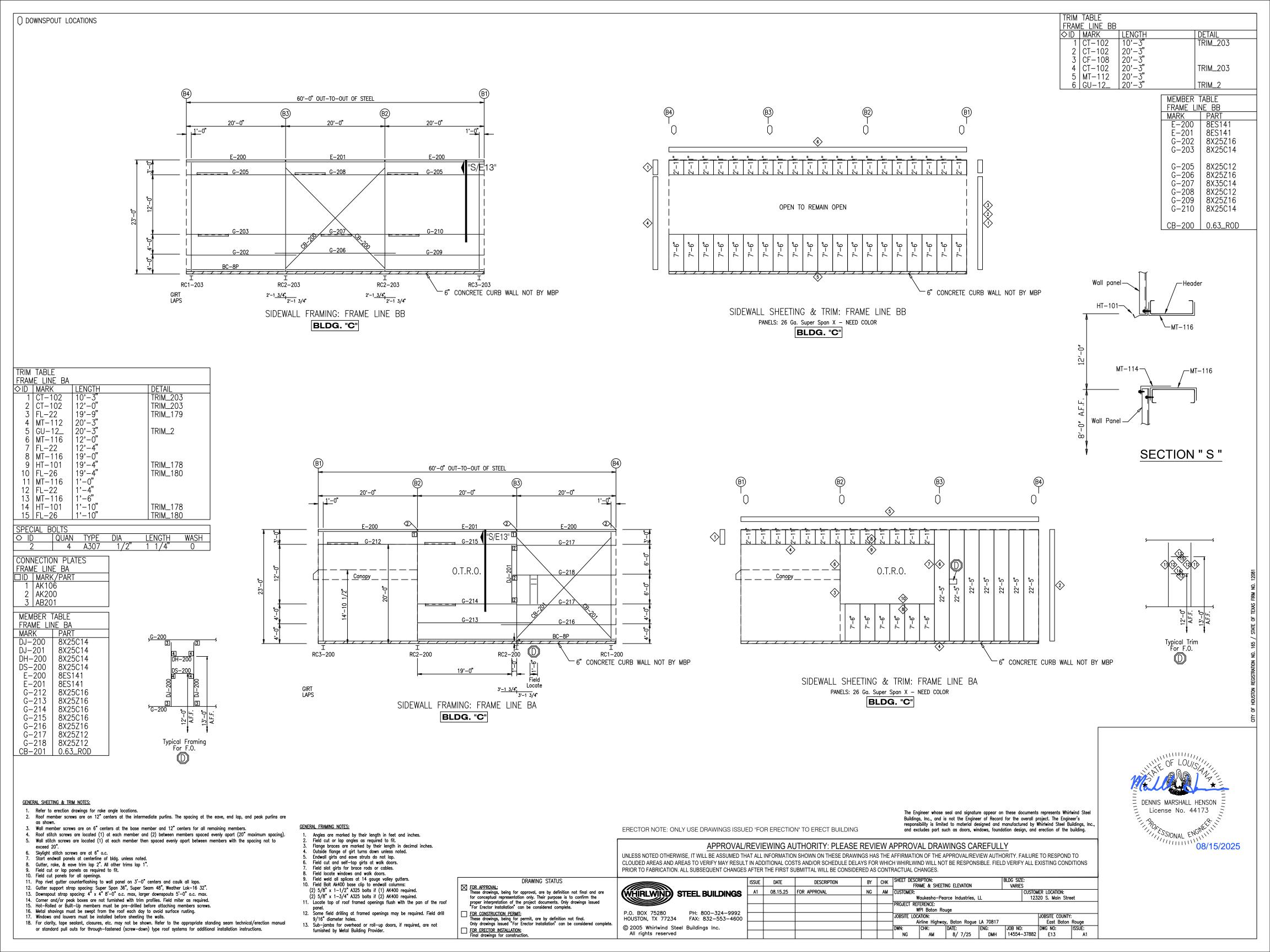
100	1102 100		7020	<del>5/ 0</del>	1 5/ 1
			·		
	TABLE				
FRAM	E LINE A				
♦D	MARK	LENGTH		DETAIL	
1	MT1024	20'-2"			
2	MT-112	20'-3"			
2 3	CT-102	10'-3"		TRIM_20	3
4 5	MT-116	5'-0"			
5	FL-22	5'-4"			
6	MT-116	4'-0"			
6 7	HT-101	4'-4"		TRIM_17	'8
8	FL-26	4'-4"		TRIM_18	
9	MT-116	2',-0"			,0
10	FL-22	2'-3"		TRIM_17	'a
11	MT-116	3'-0"		''\''\'_' /	3
12		3'-4"		TRIM_17	'Q
13	FL-26	3'-4"		TRIM_18	50
14	MT-116	7'-3"			
15	FL-22	7'-6"		TRIM_64	-
16	MT-116	3'-5"			
17	HT-101	3'-9"		TRIM_65	· •

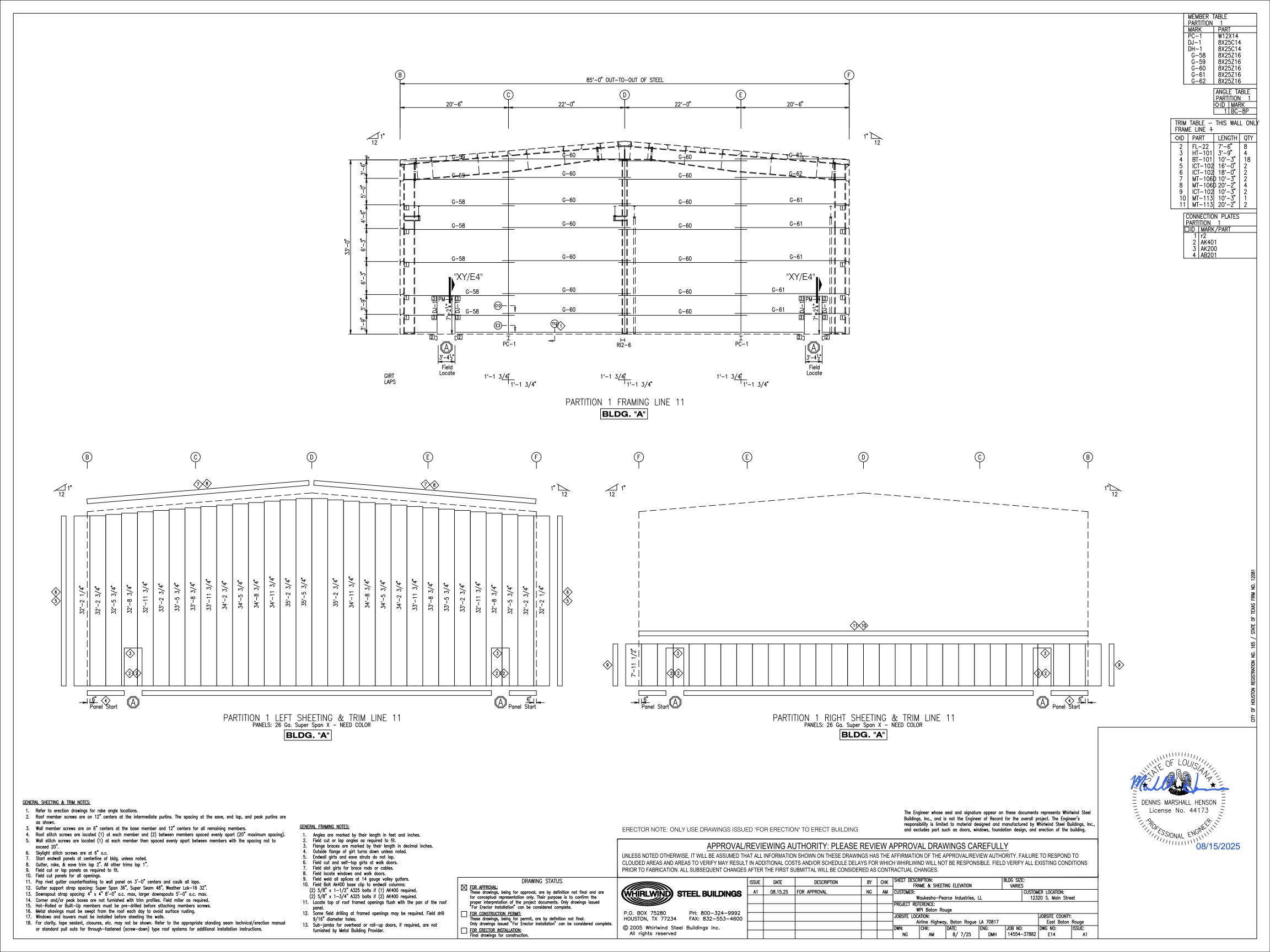
MEMBER 1	[ABLE	
FRAME LIN		
MARK	PART	
WF-100 WF-101	W16861 W20851	
EBR-107		
EBR-108	W8X10	
EBR-109	W8X10	
EBR-110	W8X10	
DJ-102	8X25C14	
DJ-103	8X25C14	
DJ-102 DJ-103 DJ-104	8X25C14 8X25C14 8X25C14	
DJ-105	L C8X11.5	
DH-102	8X25C14	
DH-103	8X25C14	
DH-104	8X25C14	
DS-100	8X25C14	
G-147	8X25Z16	
G-148	8X25Z12	
G-150	8X25Z16	
G-151 G-152	8X25716	
G-152	8X25Z16	
G-153	8X35C14	
G-154	8X25C16	
G-155	8X25Z16	
G-156	8X25Z12	
G-157	8X25Z12	
G-158	8X35Z16	
G-159	8X35C16	
G-160 G-161 G-162 G-163	8X25Z14 8X25C12 8X35C16 8X25Z14	
G-161	8X25U12	
G-162	0000010	
G-163 G-164	8X25C12	
G-165	8X25Z16	
G-166	8X35C16	
CB-102	1.00_ROD	
	w/BackUp	₽

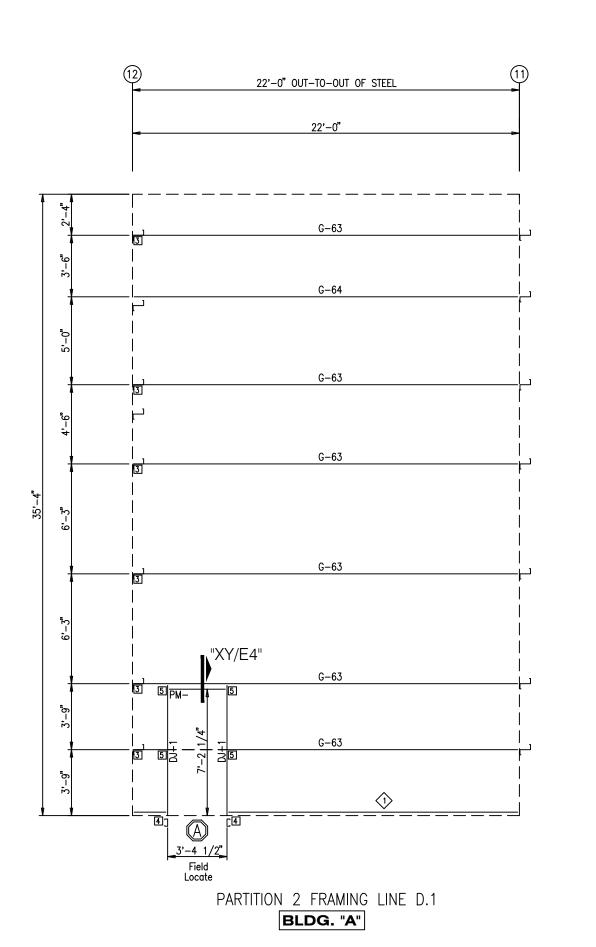
w/BackUp CONNECTION PLATES FRAME LINE A □ID | MARK/PART 1 AK508 2 AK401 3 AK200 4 AB201

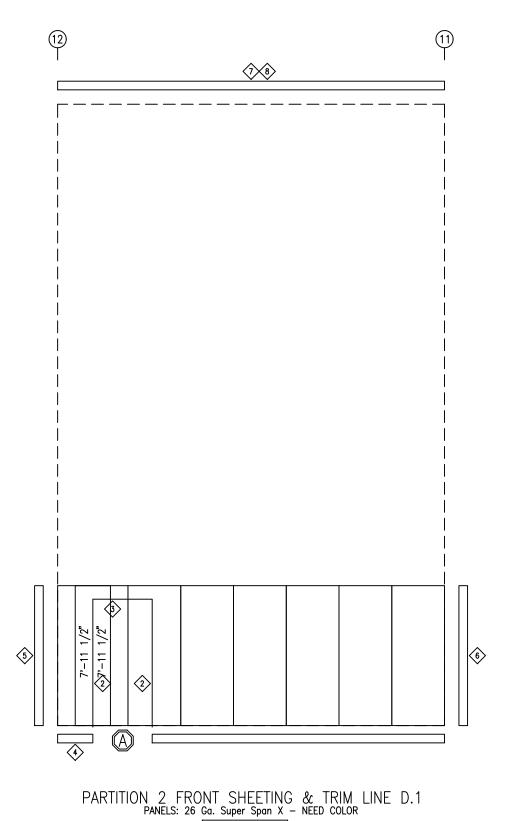




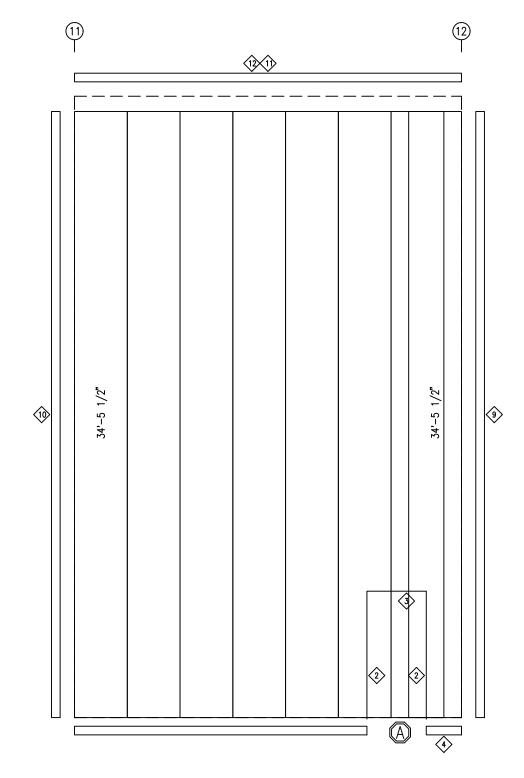








BLDG. "A"



PARTITION 2 BACK SHEETING & TRIM LINE D.1 PANELS: 26 Ga. Super Span X - NEED COLOR BLDG. "A"

GENERAL SHEETING & TRIM NOTES:

1. Refer to erection drawings for rake angle locations. 2. Roof member screws are on 12" centers at the intermediate purlins. The spacing at the eave, end lap, and peak purlins are

3. Wall member screws are on 6" centers at the base member and 12" centers for all remaining members.

4. Roof stitch screws are located (1) at each member and (2) between members spaced evenly apart (20" maximum spacing). Wall stitch screws are located (1) at each member then spaced evenly apart between members with the spacing not to exceed 20".
 Skylight stitch screws are at 6" o.c.
 Start endwall panels at centerline of bldg. unless noted.
 Gutter, rake, & eave trim lap 2". All other trims lap 1".

Field cut or lap panels as required to fit.
 Field cut panels for all openings.
 Pop rivet gutter counterflashing to wall panel on 3'-0" centers and caulk all laps.

Pop rivet gutter counterflashing to wall panel on 3-U centers and caulk all laps.
 Gutter support strap spacing: Super Span 36", Super Seam 48", Weather Lok-16 32".
 Downspout strap spacing: 4" x 4" 8'-0" o.c. max, larger downspouts 5'-0" o.c. max.
 Corner and/or peak boxes are not furnished with trim profiles. Field miter as required.
 Hot-Rolled or Built-Up members must be pre-drilled before attaching members screws.
 Metal shavings must be swept from the roof each day to avoid surface rusting.
 Windows and lawer must be installed before sheeting the walls.

Windows and louvers must be installed before sheeting the walls. 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the appropriate standing seam technical/erection manual

or standard pull outs for through-fastened (screw-down) type roof systems for additional installation instructions.

**GENERAL FRAMING NOTES:** 

Angles are marked by their length in feet and inches.
Field cut or lap angles as required to fit.
Flange braces are marked by their length in decimal inches.
Outside flange of girt turns down unless noted.
Endwall girts and eave struts do not lap.
Field cut and self-tap girts at walk doors.
Field self eight for brace rade or cables. Field slot girts for brace rods or cables.

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(2) 5/8" x 1-3/4" A325 bolts if (2) AK400 required. 11. Locate top of roof framed openings flush with the pan of the roof panel. 12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.

13. Sub-jambs for overhead or roll-up doors, if required, are not furnished by Metal Building Provider.

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ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

BY CHK SHEET DESCRIPTION:
FRAME & SHEETING ELEVATION ISSUE DATE DESCRIPTION VARIES NG AM CUSTOMER: CUSTOMER LOCATION: 12320 S. Main Street A1 08.15.25 FOR APPROVAL Waukesha-Pearce Industries, LL PROJECT REFERENCE: WPI Baton Rouge JOBSITE LOCATION: East Baton Rouge

APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY UNLESS NOTED OTHERWISE, IT WILL BE ASSUMED THAT ALL INFORMATION SHOWN ON THESE DRAWINGS HAS THE AFFIRMATION OF THE APPROVAL/REVIEW AUTHORITY. FAILURE TO RESPOND TO

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and excludes part such as doors, windows, foundation design, and erection of the building.

DENNIS MARSHALL HENSON License No. 44173 08/15/2025

2 FL-22 7'-6"
3 HT-101 3'-9"
4 BT-101 10'-3"
5 ICT-102 10'-3"
6 JT-101 8'-2"
7 MT-113 10'-3"
8 MT-113 20'-2"
9 ICT-102 18'-0"
10 JT-101 18'-2"
11 MT-106B 10'-3"
12 MT-106B 20'-2" CONNECTION PLATES 4 AK401 5 AK200

MEMBER TABLE PARTITION 2

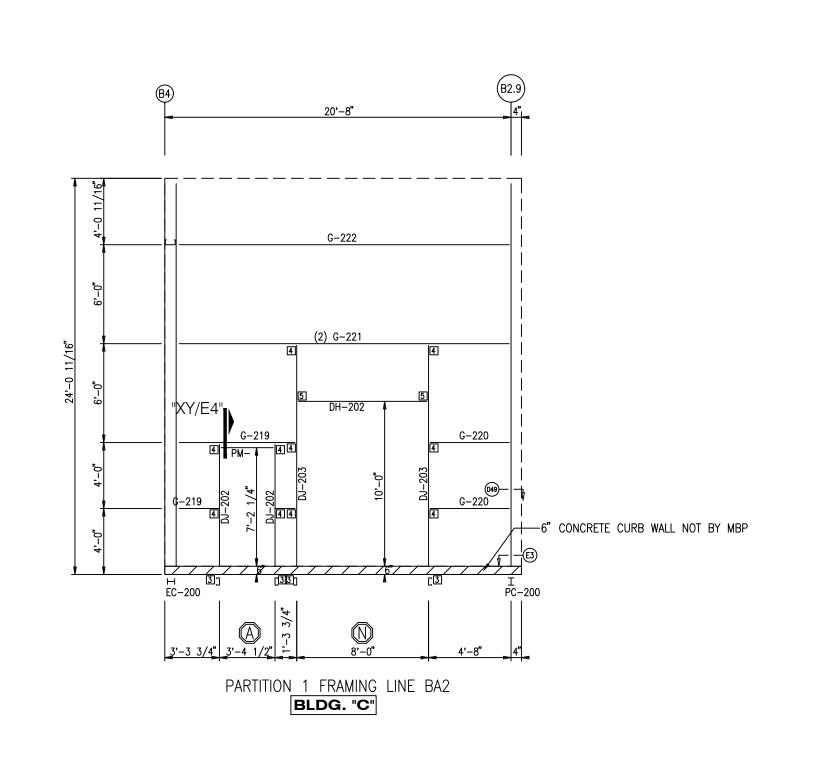
G-63 8X25Z16 G-64 8X25Z16

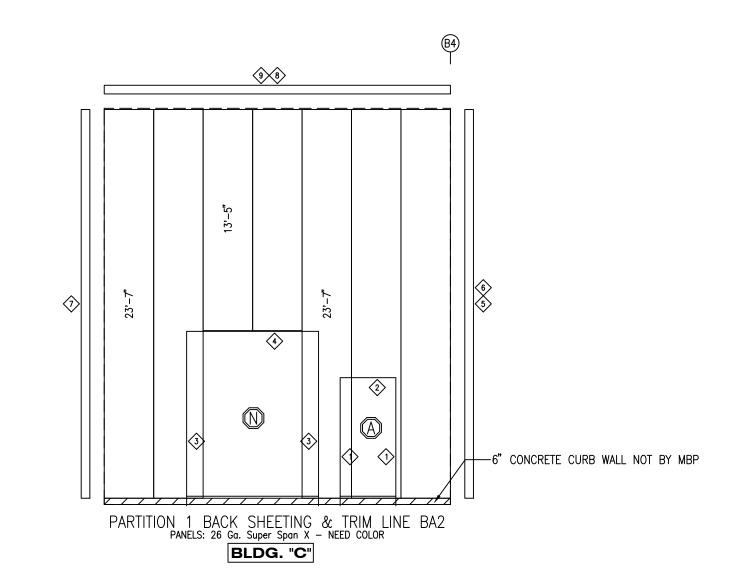
TRIM TABLE — THIS WALL ONL FRAME LINE 2 ◇ID PART LENGTH QTY

10HES141 8X25C14 8X25C14

ANGLE TABLE

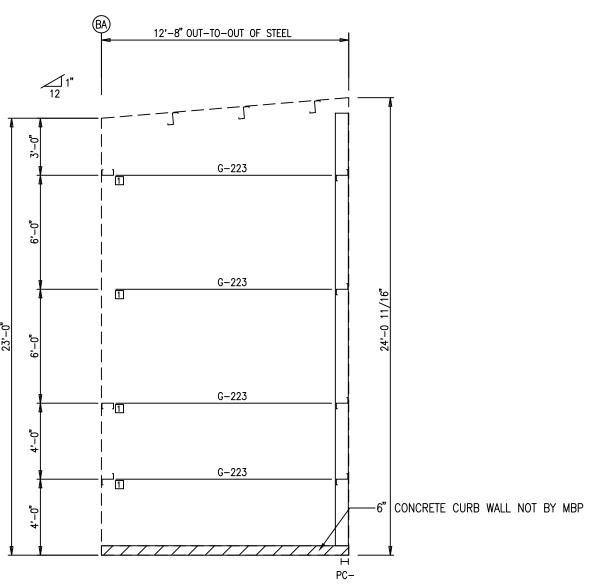
MARK
PE-1
DJ-1
DH-1
G-63





PARTITION 2
MARK PART
G-223 8X25Z16 TRIM TABLE - THIS WALL ONLY FRAME LINE 2 ◇ID PART LENGTH QTY 1 ICT-102 12'-0" 2 2 JT-101 12'-2" 2 3 MT-106C 20'-2" 1 CONNECTION PLATES

MEMBER TABLE



PARTITION 2 FRAMING LINE B2.9

BLDG. "C"

BA 1" 12 3/4" 22'-7 9-72, CONCRETE CURB WALL NOT BY MBP

Panel Start PARTITION 2 RIGHT SHEETING & TRIM LINE B2.9 PANELS: 26 Ga. Super Span X - NEED COLOR

BLDG. "C"

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CUSTOMER LOCATION:

12320 S. Main Street

JOBSITE COUNTY:

East Baton Rouge

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

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WHIRLWIND P.O. BOX 75280

)	STEEL BUILDINGS
71	PH: 800-324-9992

	ISSUE	DATE	DESCRIPTION	BY	CHK	FRAME & SHEETING ELEVATION	DCDC
S	A1	08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:	
<i>_</i>						Waukesha—Pearce Industries, LL	
00						PROJECT REFERENCE: WPI Baton Rouge	
92 600						JOBSITE LOCATION:	
						Airline Highway, Baton Rogue LA 70817	

DENNIS MARSHALL HENSON License No. 44173 PORSSIONAL ENGINE 08/15/2025

MEMBER TABLE
PARTITION 1

MARK PART
PC-200 W8X10
PE-200 8ES141
PE-201 8ES141
DJ-202 8X25C14
DJ-203 8X35C14
DH-201 8X25C14
DH-202 8X25C14
DS-201 8X25C14
DS-202 8X25C14
G-219 8X25C14
G-220 8X25C16
G-221 8X25Z12
G-222 8X35Z12

TRIM TABLE — THIS WALL ONL' FRAME LINE + ◇ID PART LENGTH QTY

1 FL-22 7'-6"
2 HT-101 3'-9"
3 FL-22 10'-3"
4 HT-101 8'-4"
5 ICT-102 12'-0"
6 ICT-102 14'-0"
7 JT-101 12'-2"
8 MT-106B 10'-3"
9 MT-106B 20'-2"

CONNECTION PLATES

2 e200 3 AK401

4 AK200 5 AB201

#### GENERAL SHEETING & TRIM NOTES:

1. Refer to erection drawings for rake angle locations. 2. Roof member screws are on 12" centers at the intermediate purlins. The spacing at the eave, end lap, and peak purlins are

3. Wall member screws are on 6" centers at the base member and 12" centers for all remaining members.

4. Roof stitch screws are located (1) at each member and (2) between members spaced evenly apart (20" maximum spacing). 5. Wall stitch screws are located (1) at each member then spaced evenly apart between members with the spacing not to

wall statch screws are located (1) at each member then exceed 20".
Skylight stitch screws are at 6" o.c.
Start endwall panels at centerline of bldg. unless noted.
Gutter, rake, & eave trim lap 2". All other trims lap 1".

Field cut or lap panels as required to fit.
 Field cut panels for all openings.
 Pop rivet gutter counterflashing to wall panel on 3'-0" centers and caulk all laps.

Gutter support strap spacing: Super Span 36", Super Seam 48", Weather Lok-16 32".
 Downspout strap spacing: 4" x 4" 8'-0" o.c. max, larger downspouts 5'-0" o.c. max.
 Corner and/or peak boxes are not furnished with trim profiles. Field miter as required.
 Hot-Rolled or Built-Up members must be pre-drilled before attaching members screws.

16. Metal shavings must be swept from the roof each day to avoid surface rusting.

Windows and louvers must be installed before sheeting the walls. 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the appropriate standing seam technical/erection manual or standard pull outs for through-fastened (screw-down) type roof systems for additional installation instructions.

**GENERAL FRAMING NOTES:** 

Angles are marked by their length in feet and inches. Field cut or lap angles as required to fit. Flange braces are marked by their length in decimal inches. Outside flange of girt turns down unless noted. Endwall girts and eave struts do not lap.
Field cut and self—tap girts at walk doors.

Field slot girts for brace rods or cables. Field locate windows and walk doors.

 Field weld all splices at 14 gauge valley gutters.
 Field Bolt Ak400 base clip to endwall columns: (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 required. (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 required.

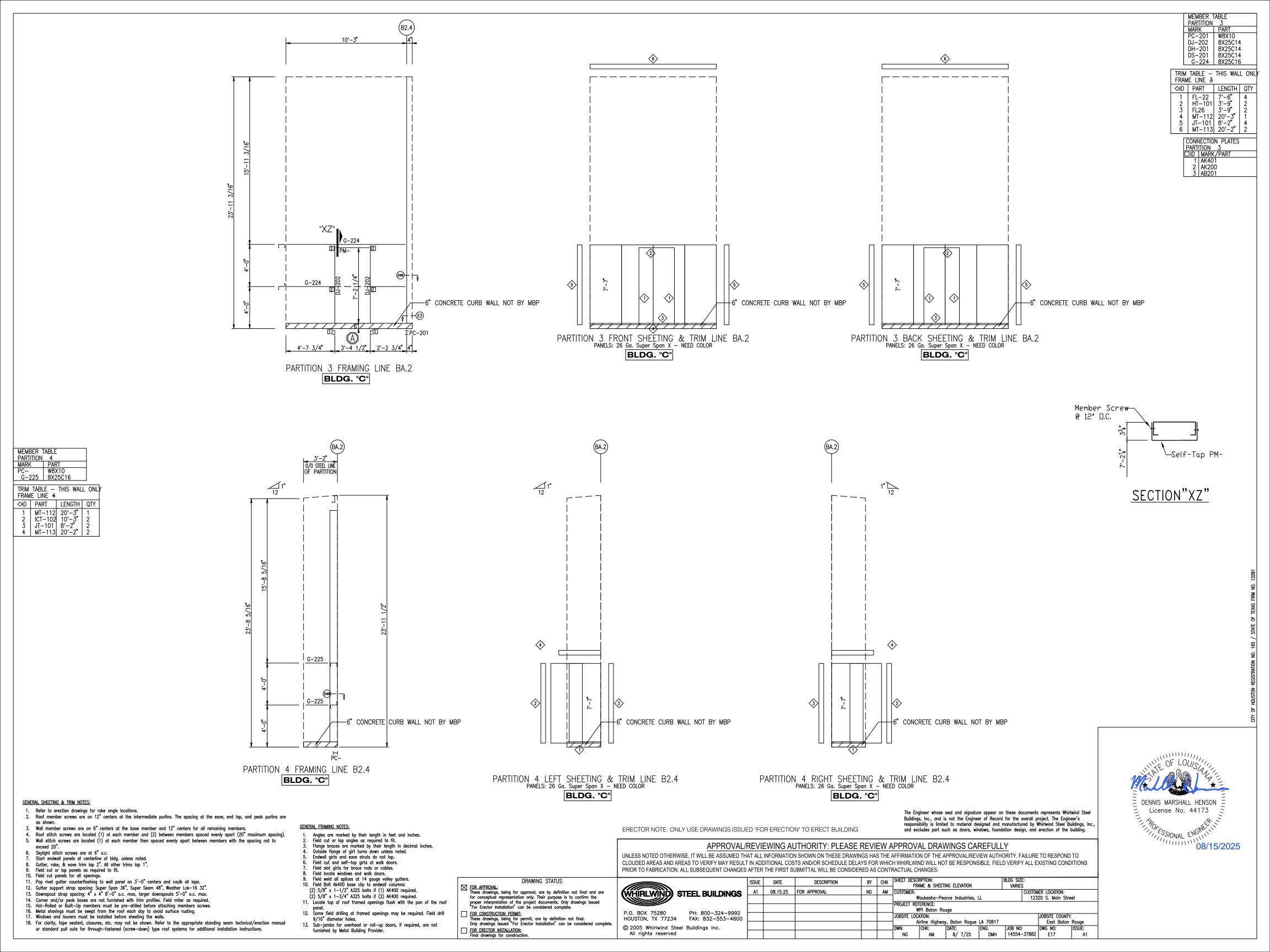
panel. 12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes. Sub-jambs for overhead or roll-up doors, if required, are not furnished by Metal Building Provider.

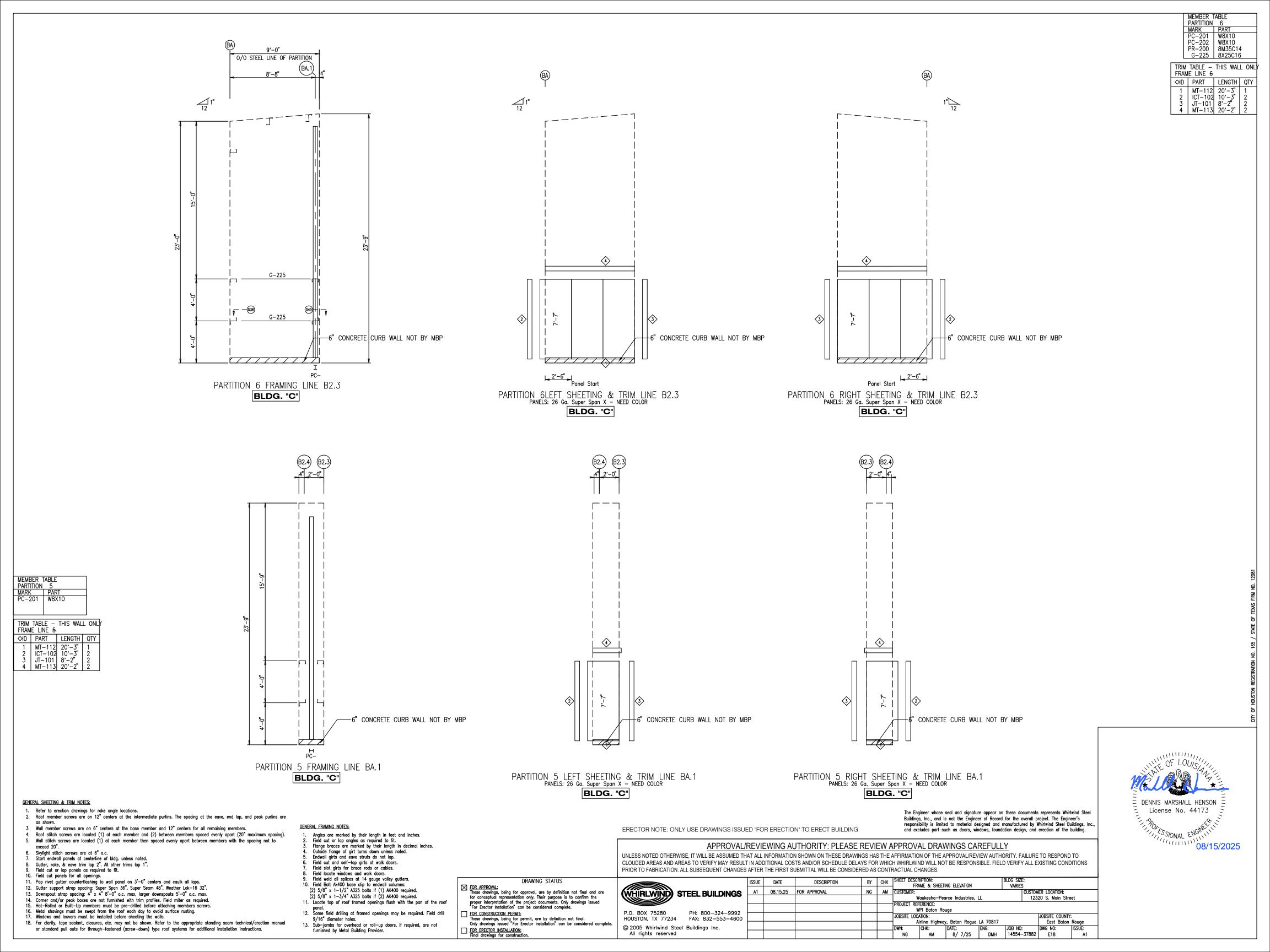
11. Locate top of roof framed openings flush with the pan of the roof

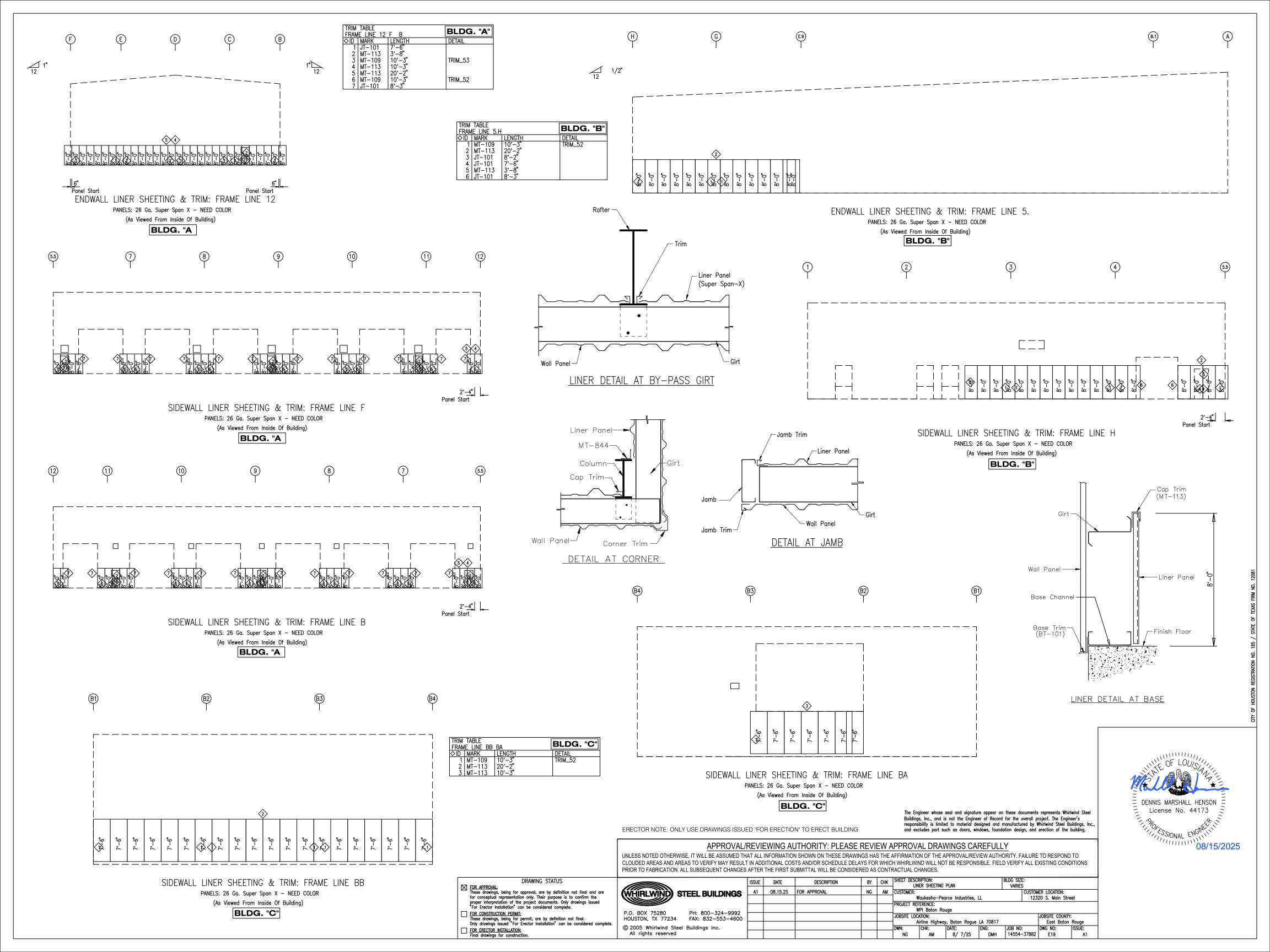
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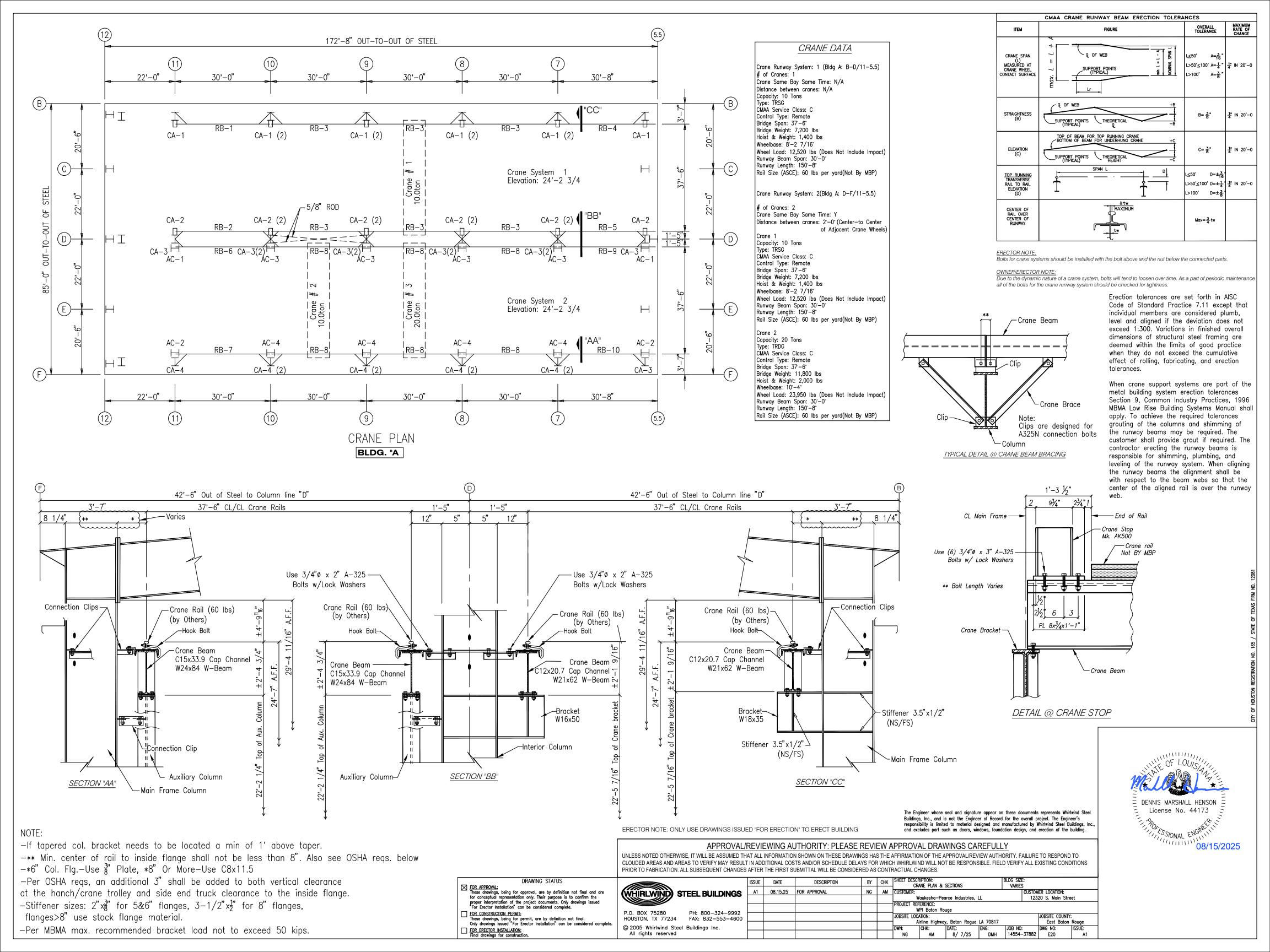
HOUSTON, TX 77234 FAX: 832-553-460 © 2005 Whirlwind Steel Buildings Inc.

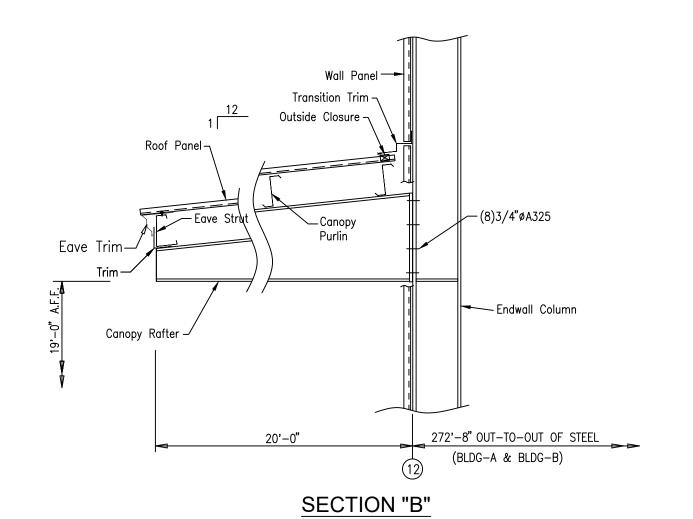
\_\_\_\_DWN: CHK: DATE: ENG: JOB NO: DWG NO: NG NG AM 8/ 7/25 DMH 14554-37882 E16











100'-0" OUT-TO-OUT OF STEEL (BUILDING-B)

Cap Trim

Back Panel

- Rake Angle

(RC2000)

SECTION "E"

Transition Trim

➤ EW Rafter

Rake Trim Closure

Purlin

Roof Panel S.S. Plus

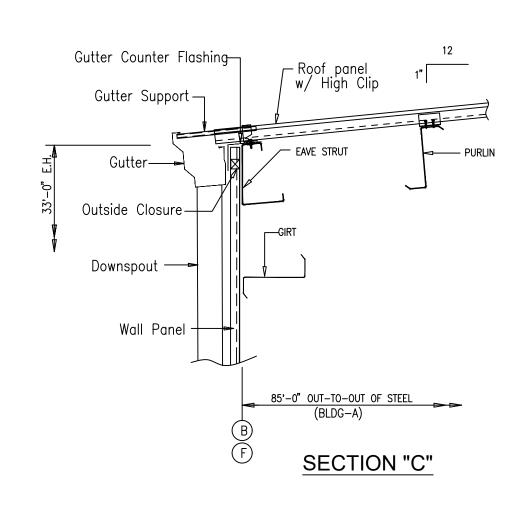
- GIRT

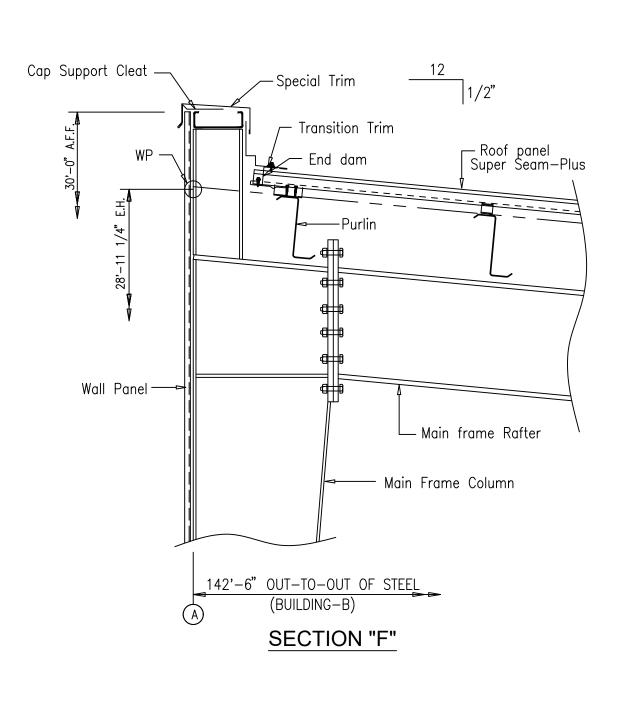
1'-4"

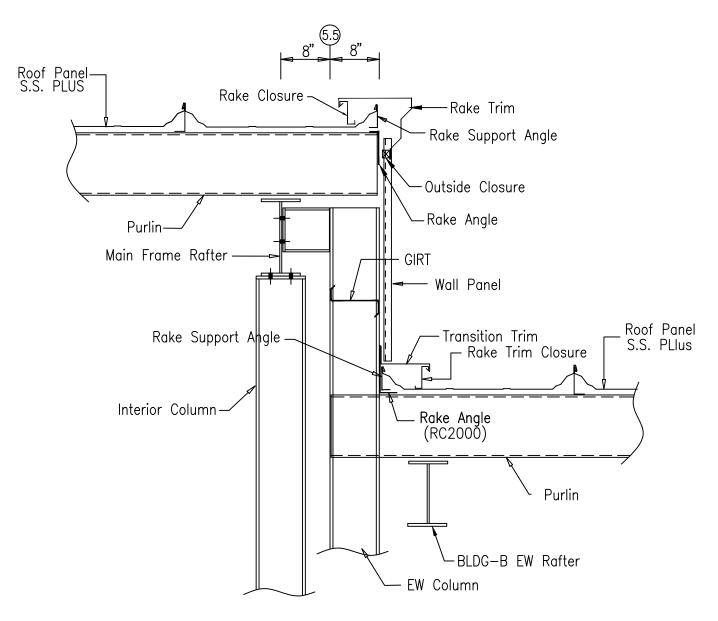
Cap Support Cleat— (MT−133)

Wall Panel

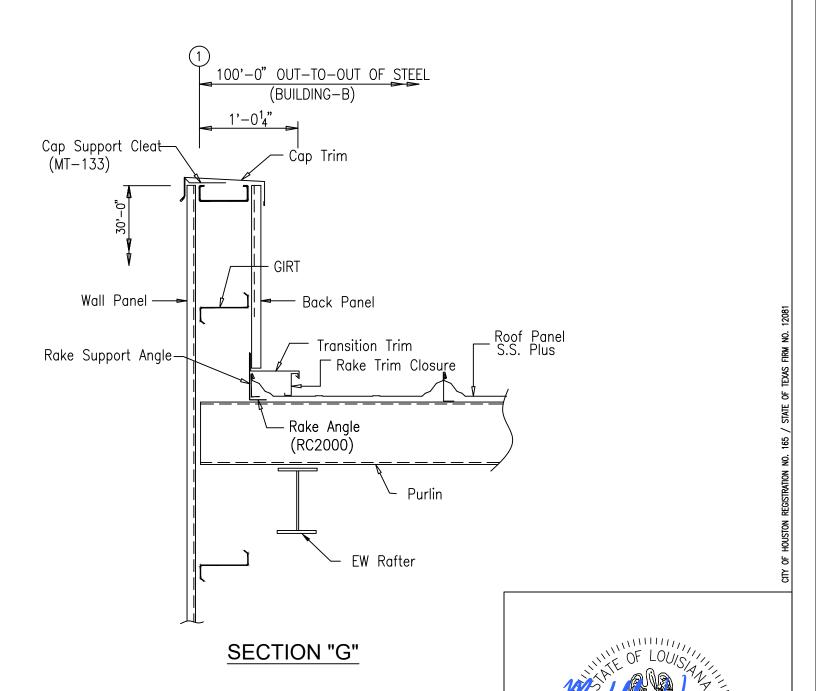
Rake Support Angle-







## SECTION "D"



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08/15/2025

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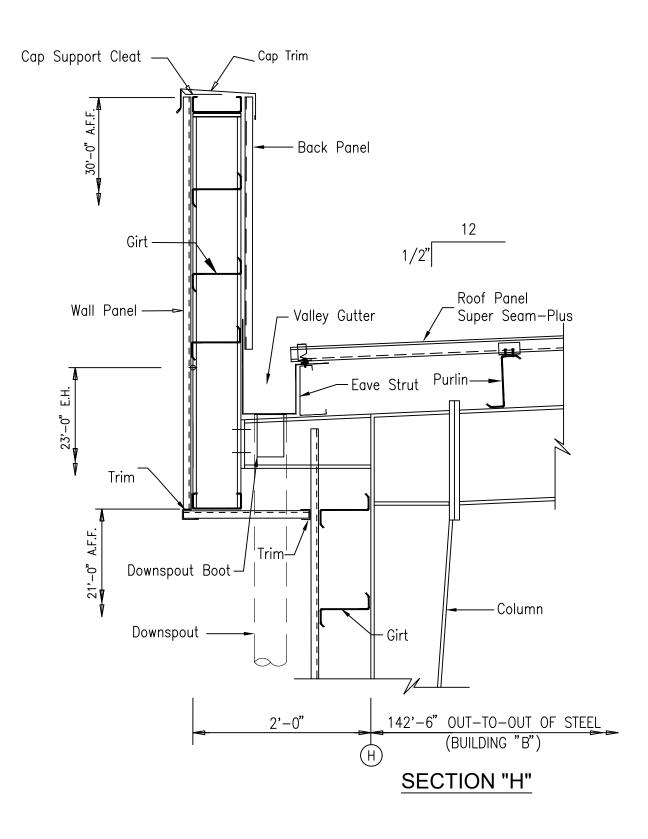
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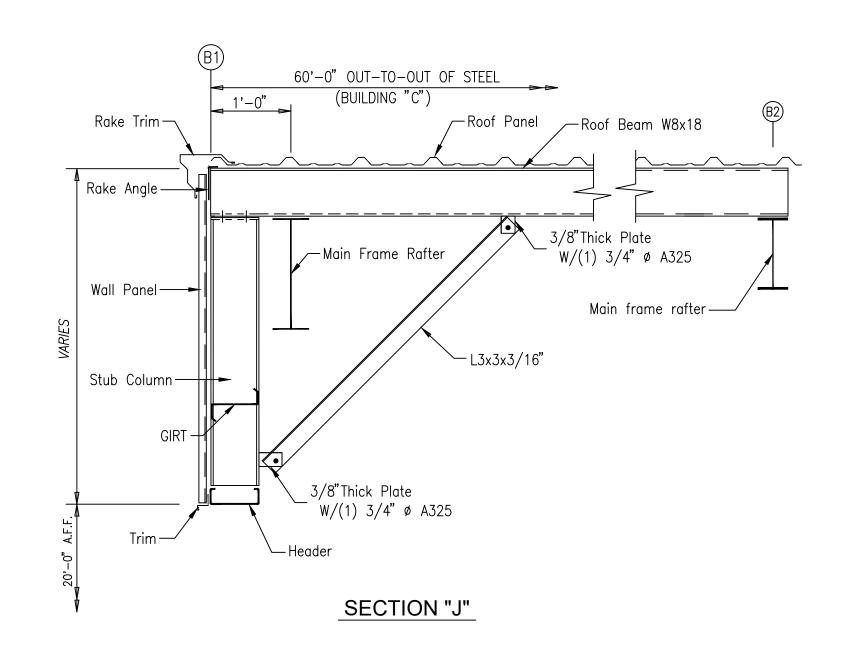
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FOR ERECTOR INSTALLATION: Final drawings for construction.

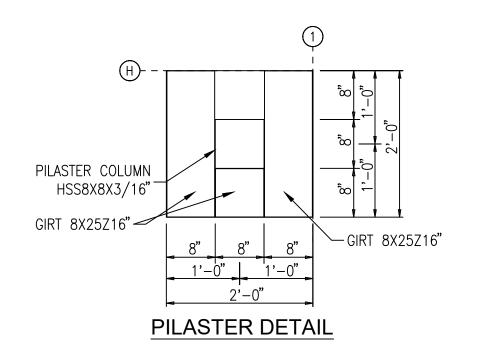
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	P.O. BOX 75280 HOUSTON, TX 77234	PH: 800-324-9992 FAX: 832-553-4600
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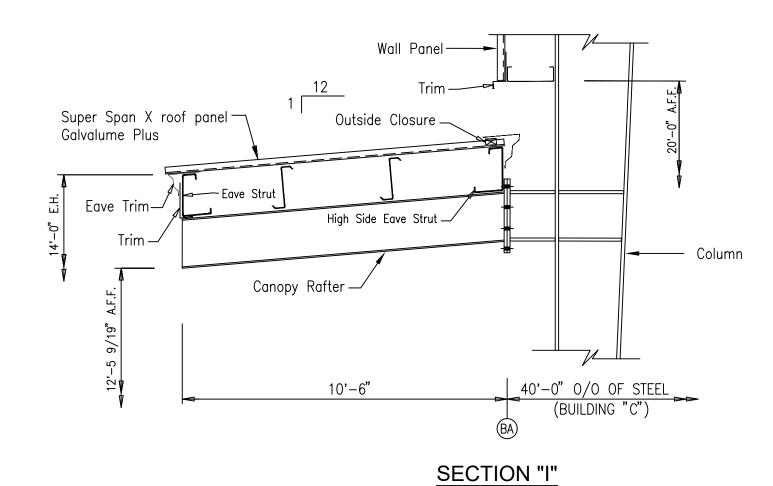
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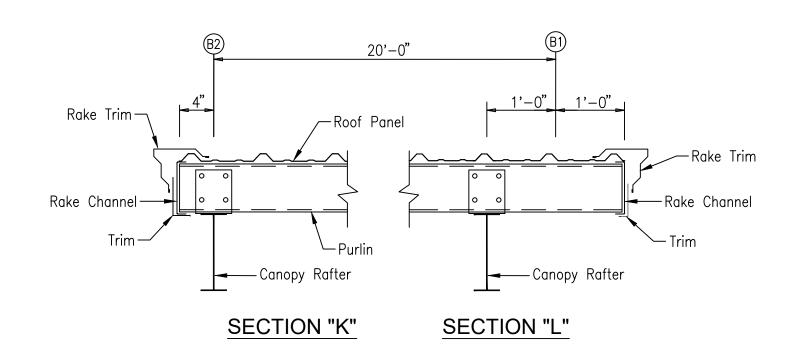
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, ,							Airline Highway, Baton Rogue LA 70817					East Baton		
						DWN:	CHK:	DATE:		ENG:	JOB NO:	DWG NO:	ISSUE:	
						NG	AM	8/	7/25	DMH	14554-37882	! E21	A1	











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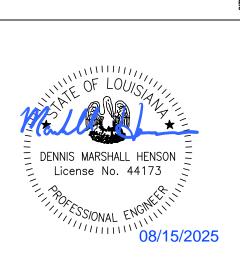
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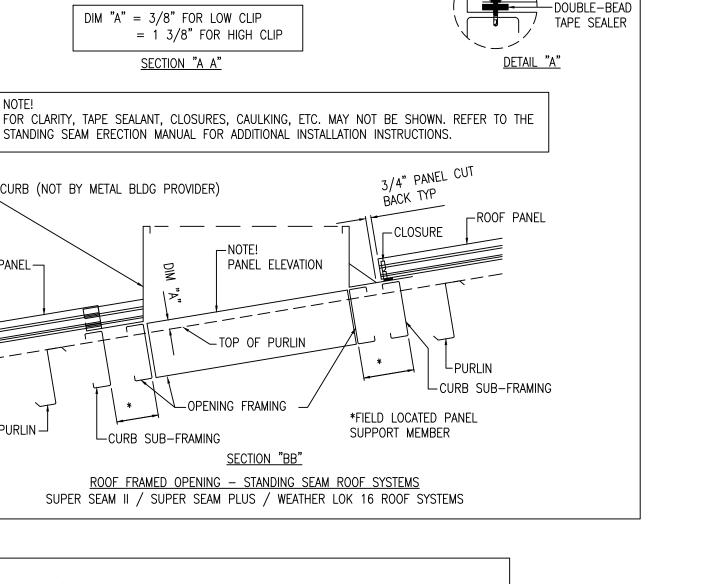
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Final drawings for construction.

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	ISSUE	DATE	DESCRIPTION	BY	CHK		SHEET DESCRIPTION: BUILDING SECTIONS BUILDING SECTIONS BUILDING SECTIONS					
	A1	08.15.25	FOR APPROVAL	NG	AM							
•						] w	aukesha-Pea	20 S. Main Stre	et			
						PROJECT REF	ERENCE:			•		
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n						JOBSITE LOCATION: JOE						
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						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
						NG	AM	8/ 7/25	DMH	14554-37882	E22	A1





FASTENER #1E

@ 2" O.C.

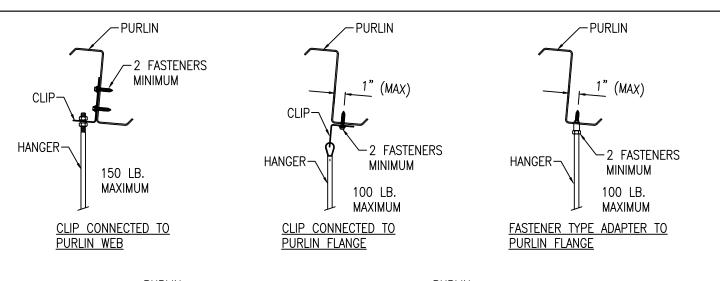
3/4" PANEL CUT BACK TYP

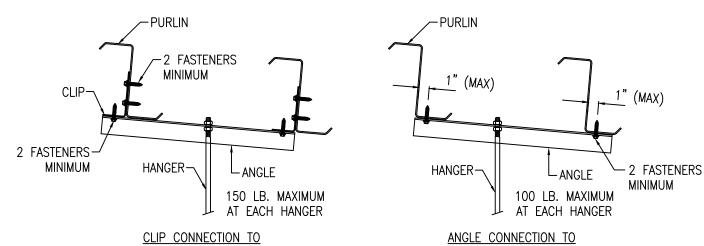
TOP OF PURLIN BEYOND

BACK TYP

-CLOSURE

SUPPORT MEMBER





Metal Building Provider is providing the Purlin Only.All other Hardware and Accessories are not by Metal Building Provider.

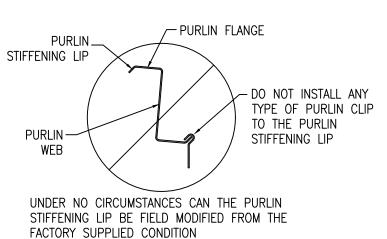
PURLIN WEB

The total hanger load shall not exceed the Design Collateral Load for the Building. Example:

5'-0" Purlin Spacing X 5'-0" hanger spacing X 6 Psf Collateral Load = 150 Lbs. Hanger Load

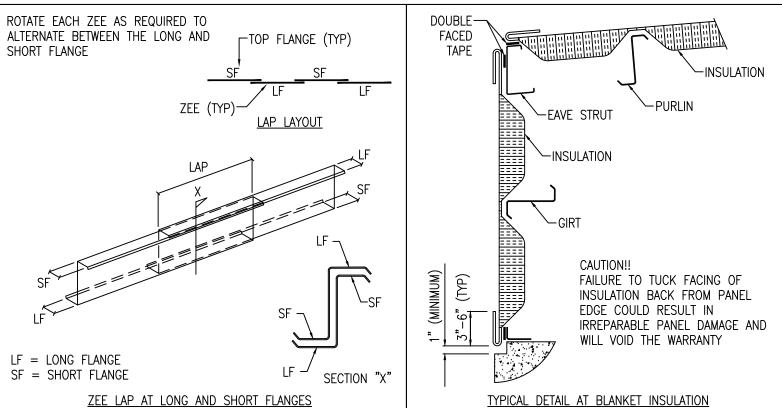
See C1 for the Design Collateral Loads for this Building.

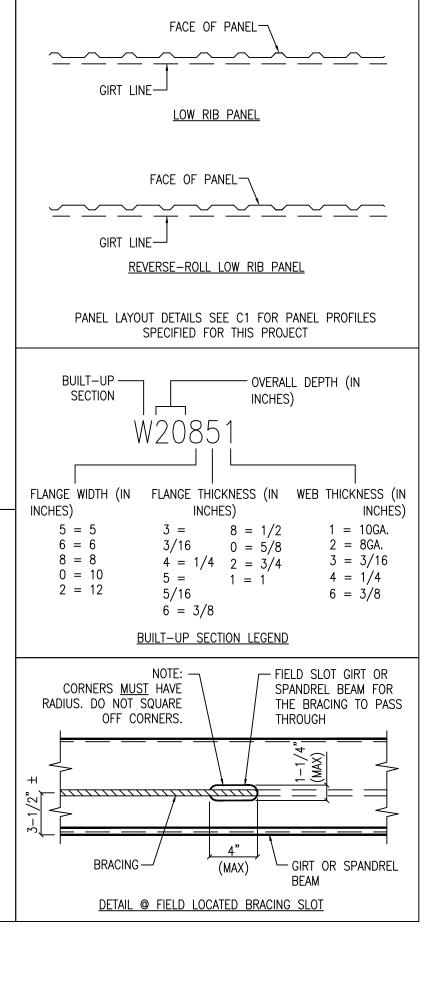
Note: If the building is designed for 0 Psf Collateral then adding any suspended system (i.e. Duct Work, Piping, Lights, Ceiling, Etc..) will correspondingly reduce the Design Live Load.



PURLIN FLANGE

#### ACCEPTABLE CONNECTIONS FOR BUILDING ACCESSORIES TO PURLIN ATTACHMENT





FACE OF PANEL-

SUPERM SPAN PANEL

REVERSE-ROLL SUPER SPAN PANEL

FACE OF PANEL-

GIRT LINE-

GIRT LINE-

CURB (NOT BY METAL-

– NOTE!

PANEL ELVATION

OPENING WIDTH

DIM "A" = 3/8" FOR LOW CLIP

SECTION "A A"

CURB (NOT BY METAL BLDG PROVIDER)

= 1 3/8" FOR HIGH CLIP

STANDING SEAM ERECTION MANUAL FOR ADDITIONAL INSTALLATION INSTRUCTIONS.

PANEL ELEVATION

-TOP OF PURLIN

SECTION "BB"

ROOF FRAMED OPENING - STANDING SEAM ROOF SYSTEMS

-OPENING FRAMING

LCURB SUB-FRAMING

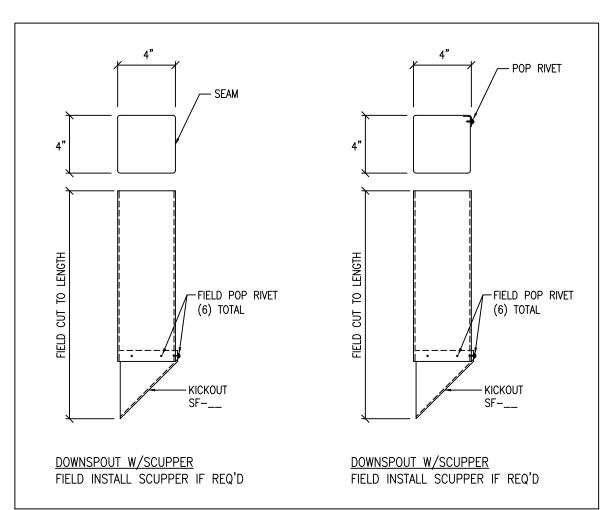
BLDG PROVIDER)

PANEL CLIP-

OPENING FRAMING

ROOF PANEL-

PURLIN -



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CUSTOMER LOCATION:

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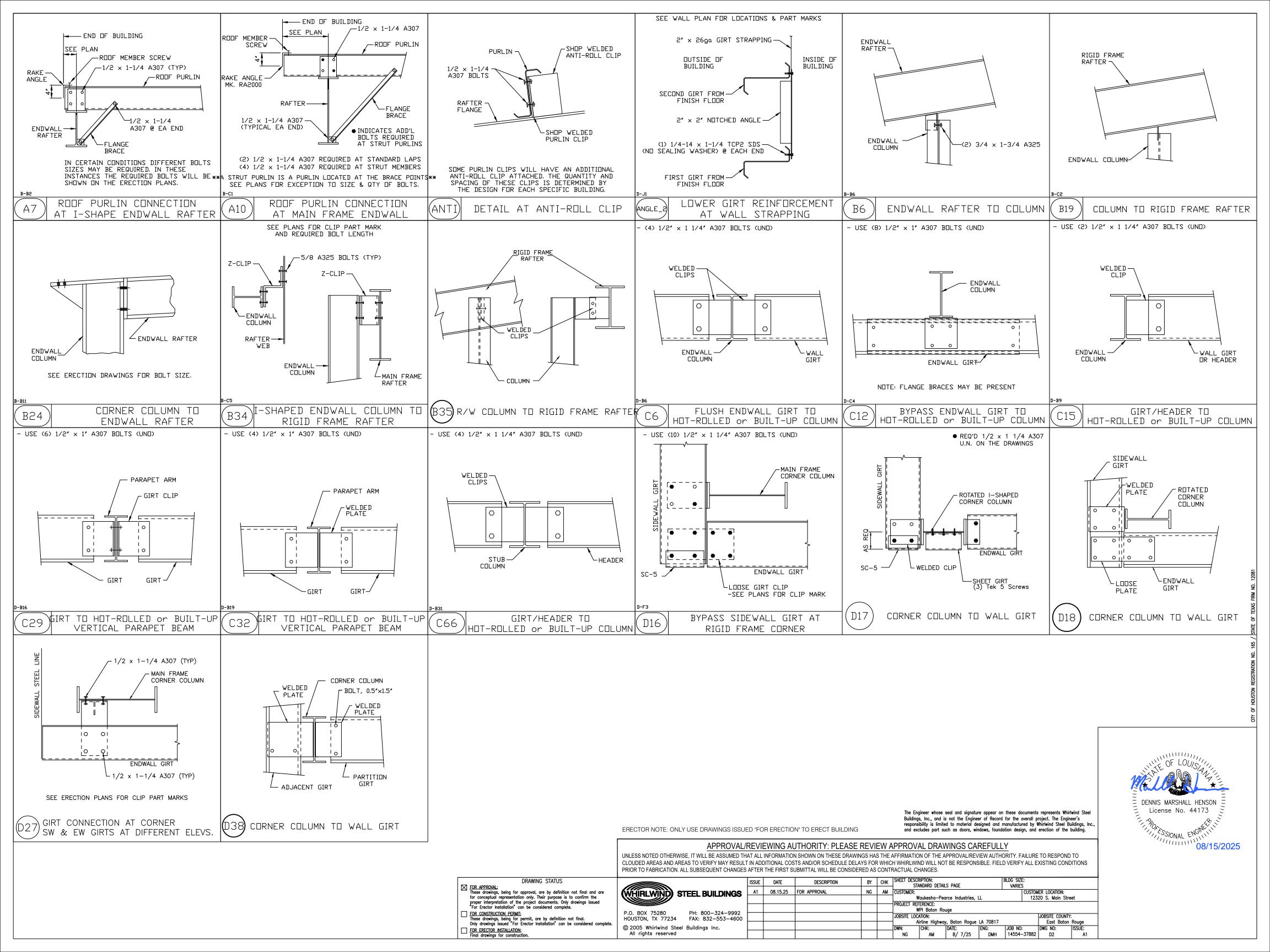
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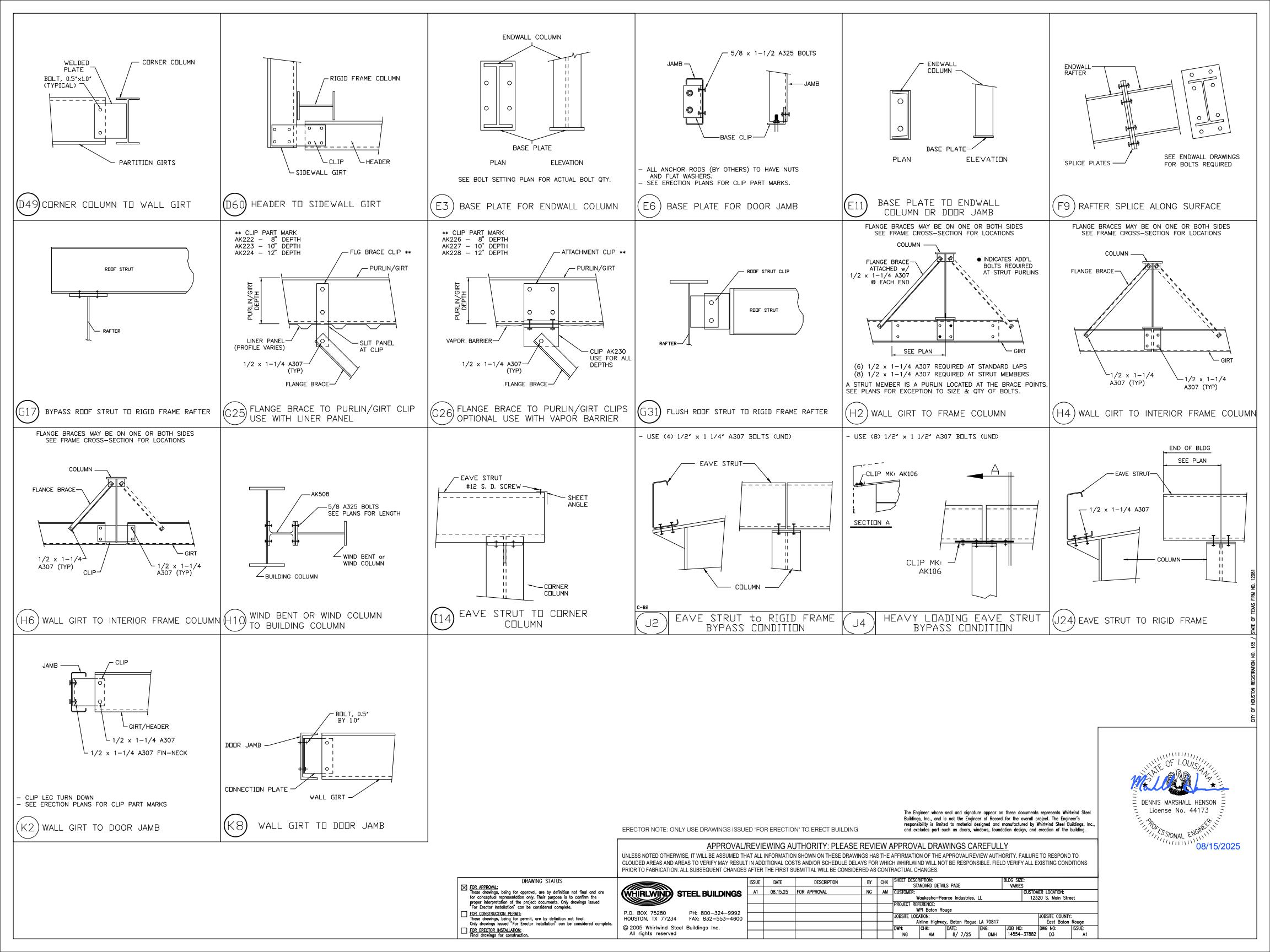
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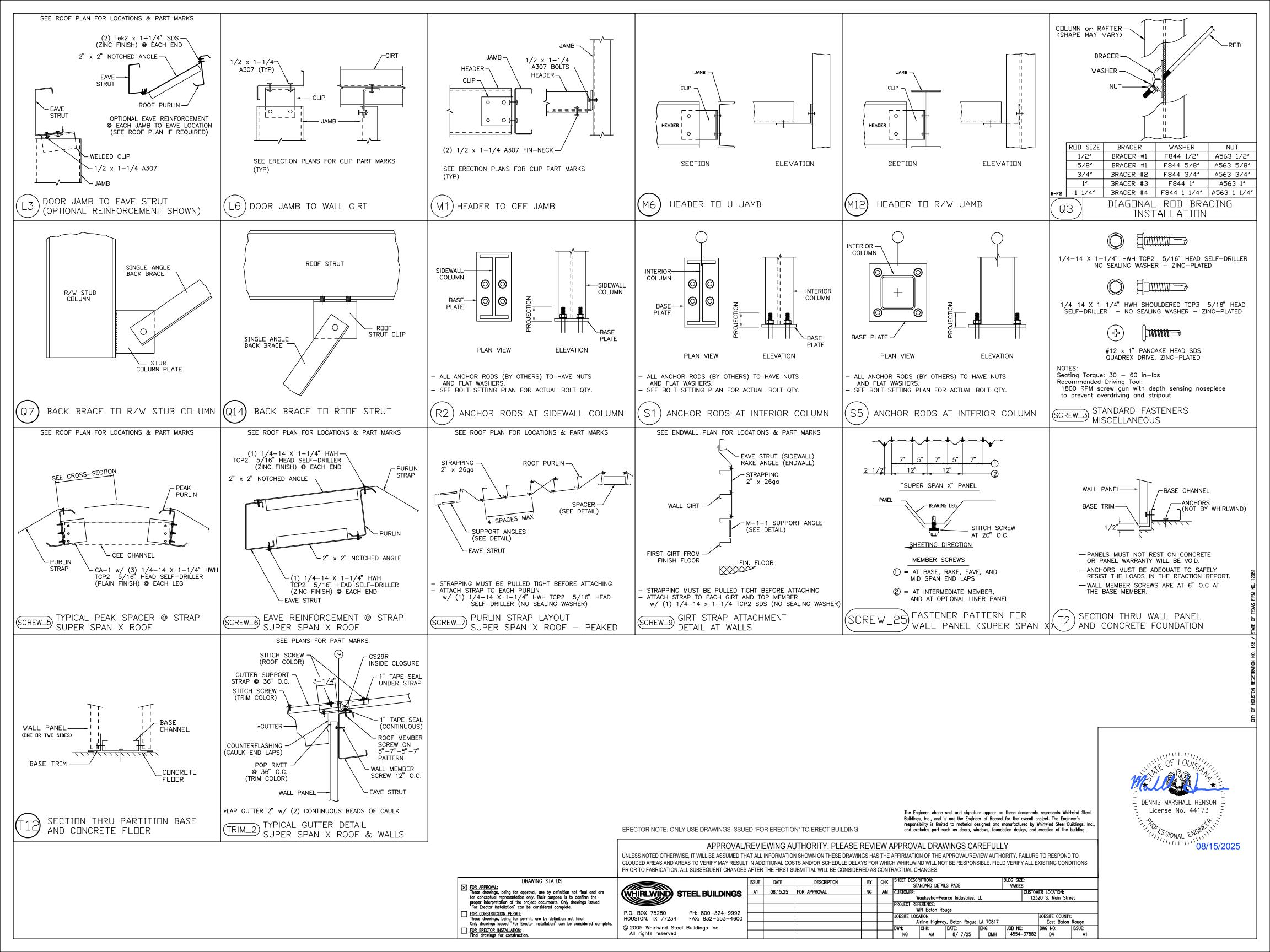
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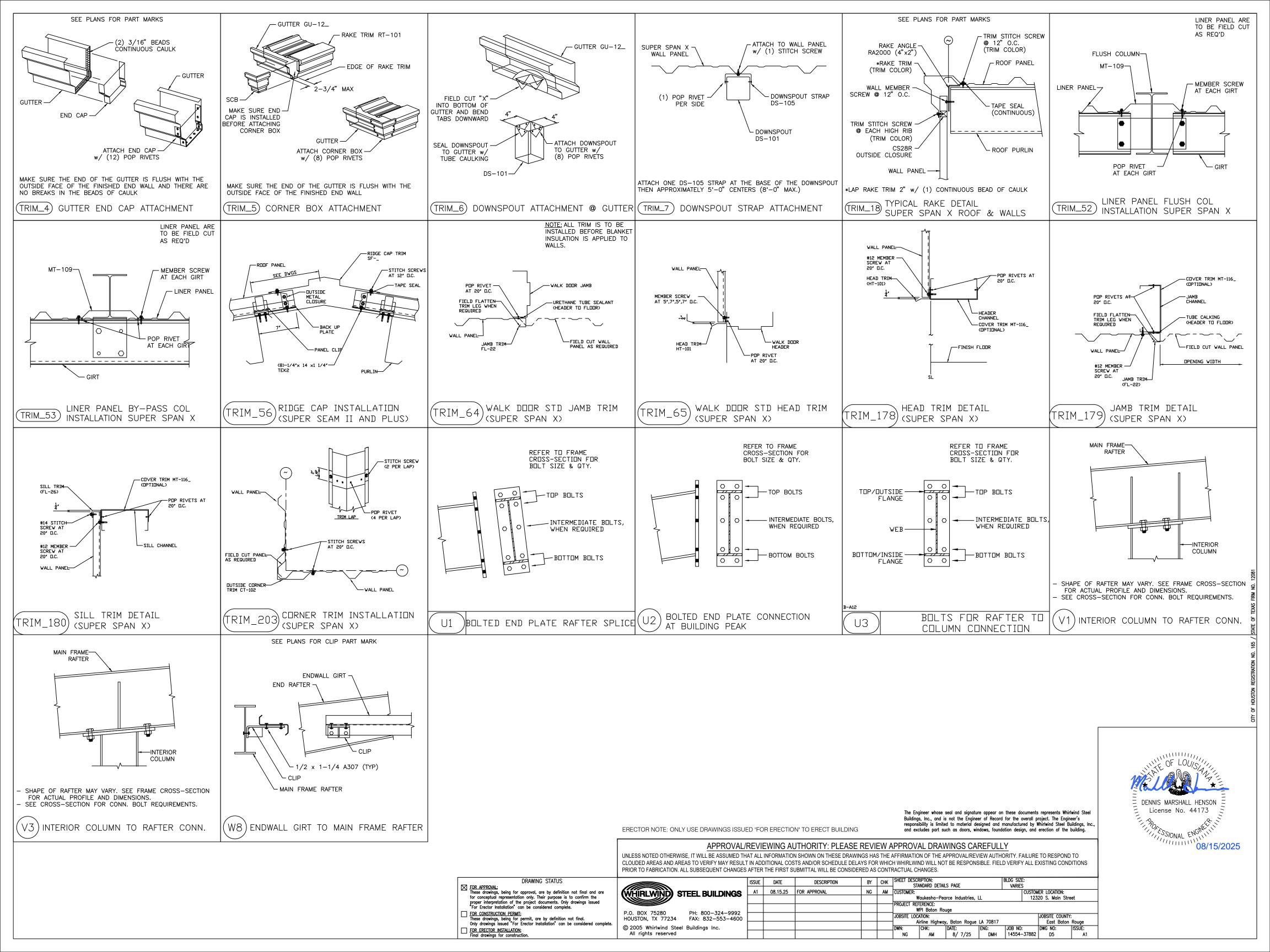
 $\odot$  2005 Whirlwind Steel Buildings Inc.

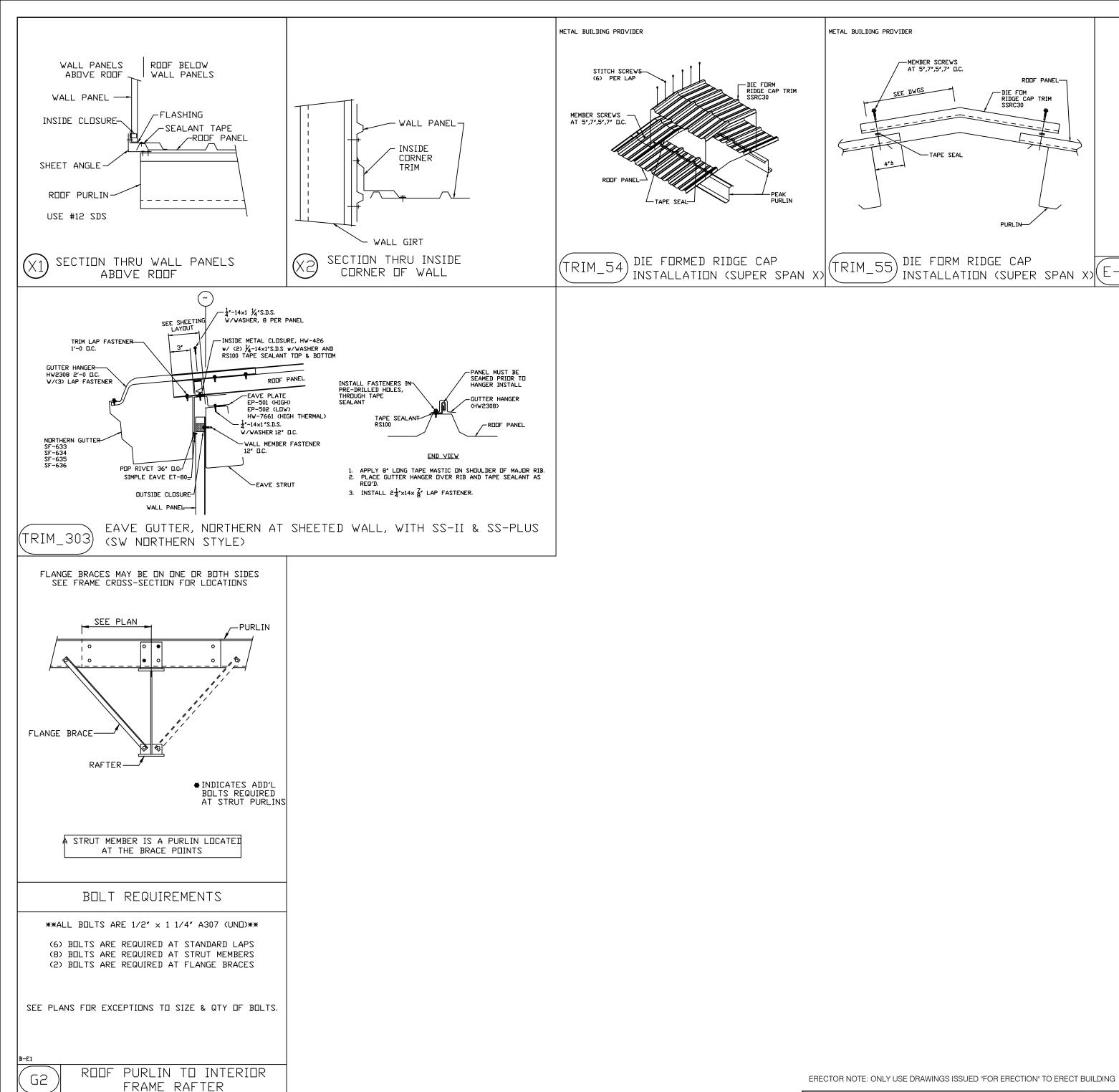
12320 S. Main Street East Baton Rouge NG AM 8/ 7/25 DMH 14554-37882 D1











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DRAWING	STATUS

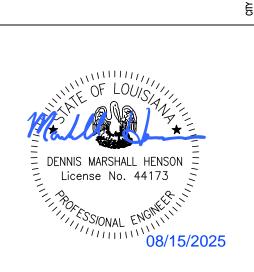
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	ISSUE	DATE	DESCRIPTION	BY	CHK	STA	iiption: NDARD DETAIL	S PAGE		VARIES			
WIND STEEL BUILDINGS		08.15.25	FOR APPROVAL	NG	AM	CUSTOMER:				CUSTOMER LOCATION:			
							Waukesha-Pearce Industries, LL			123	12320 S. Main Street		
						PROJECT REF	ERENCE:						
75280 PH: 800-324-9992						w	PI Baton Rou	ıge					
TX 77234 FAX: 832-553-4600						JOBSITE LOCA	ATION:				JOBSITE COUNTY	<u>':</u>	
1X 77234 FAX. 632-333-4000				_	1	Ai	rline Highway	, Baton Rogue	LA 70817		East Baton	Rouge	
hirlwind Steel Buildings Inc.						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:	
s reserved						NG	ΔM	8/7/25	DMH	14554-37882	D6	Δ1	



TAPE SEAL

MEMBER SCREWS— AT 5", 7", 5"

TAPE SEAL

FASTENER PATTERN FOR "SUPER SPAN X"

ROOF-/ PANEL

9 1/2"

ALL ROOF MEMBERS EXCEPT AS NOTED BELOW

AT EAVE STRUT, PANEL END LAP, AND PEAK PURLIN

2 1/2"

2 1/2"

SHEETING DIRECTION

2 1/2"

SHEETING DIRECTION

ROOF PANEL-

-DIE FOM RIDGE CAP TRIM SSRC30

PURLIN-

9 1/2"

DETAIL "A"-

DETAIL

2 1/2"

2 1/2"

ROOF PANEL

STITCH SCREW

- ROOF PANEL

—AT 20″□.C.

PANEL

— PURLIN

-BEARING LEG

SHEETING DIRECTION DETAIL "A"

SECTION THRU PANEL END LAPS