

SUBSURFACE SOIL INVESTIGATION

Promontory Phase 2 Lots: 139-142, 155-158, 172-206, 215-225, & 244-353 Greeley, Colorado

PREPARED FOR:

Journey Homes, LLC 7251 W. 20th St., L-200 Greeley, CO 80634

JOB NO. 184203-2 March 29, 2022

Respectfully Submitted, RMG – Rocky Mountain Group Reviewed by, RMG – Rocky Mountain Group

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PROJECT DESCRIPTION AND SCOPE

Project Location

The project site lies in part of the east half of Section 11 and west half of Section 12, Township 5 North, Range 67 West of the 6th P.M., City of Greeley, Colorado. The project site is generally located to the west of the City of Greeley, Colorado. The site is located east of Promontory Parkway and in between Highway 34 Bypass and 16th Street. The approximate location of the proposed subdivision is shown in the Site Vicinity Map on Figure 1.

Project Description

The project is to consist of the development of a parcel of vacant, semi-developed property for residential use. It is our understanding that this development is to be occupied by single-family residences. RMG was retained to evaluate the soil conditions for all single-family residential lots located on the north half of the subdivision. The lots included in this investigation are as follows: 139-142, 155 -158, 172-206, 215-225, and 244-353 for a total of 164 lots.

This report will reference the *Promontory Phase 1* investigation report.

Scope of Work

RMG was retained to assess the soil conditions and develop geotechnical engineering recommendations to support the proposed residential land development. Our scope of services consisted of a field investigation, laboratory testing, engineering analysis, and report preparation.

This report presents geotechnical engineering recommendations for the design of foundations for the proposed single-family residences on the lots listed above. The recommendations in this report are also contingent upon completion of an Open Excavation Observation by RMG, prior to construction of the foundations, in order to verify subsurface conditions for the specific excavated site.

The following is excluded from the scope of this report including but not limited to geologic, natural and environmental hazards such as landslides, unstable slopes, seismicity, snow avalanches, water flooding, corrosive soils, erosion, radon, wild fire protection, hazardous waste and natural resources.

Existing Site Conditions

At the time of our field exploration, overlot grading had been completed. Water and sewer connections were being installed during our investigation. Curb and gutter had not been installed, and the roads had not been paved. Topography of the site generally sloped gently down from the northwest to the south and southeast. Vegetation was not present at the site due to the over lot grading. The surrounding areas consisted of residential properties to the north,

agricultural farm land to the east, a JBS Office building to the west, and a recently-constructed fire station to the south.

FIELD INVESTIGATION AND LABORATORY TESTING

The information included in this report has been compiled from field reconnaissance, exploratory soil borings, and soil laboratory testing. Monitoring programs, which typically include instrumentation and/or observations for surface water flows, slope stability, subsidence, and similar conditions, are not known to exist and were not considered applicable for the scope of this report.

Subsurface Investigation

The subsurface conditions on the site were investigated by drilling 164 exploratory test borings, one bore per lot included in this investigation. The arrangement of the proposed lots is presented in the Lot Layout Plan, Figure 2.

The test borings were advanced with a truck-mounted, continuous-flight auger drill rig to depths of about 20 feet below the existing ground surface (bgs). Soil samples were obtained from the test borings in general accordance with ASTM D-3550 utilizing a 2½-inch OD modified California sampler. The approximate depth of groundwater was investigated in each of the test borings at the time of drilling. The static groundwater levels and elevations of the test borings were recorded following the completion of drilling. The Test Boring Logs are presented in Figures 3 through 84. An Explanation of Test Boring Logs is presented in Figure 85.

Laboratory Testing

The moisture contents for the recovered samples were obtained in the laboratory. Grain-size analysis, Atterberg Limits, and swell/consolidation tests were performed on selected samples for purposes of classification and to develop pertinent engineering properties. A Summary of Laboratory Test Results is presented in Figure 86. Soil Classification Data are presented in Figures 87 through 94. Swell/Consolidation Test Results are presented in Figures 95` through 115.

SUBSURFACE CONDITIONS

Soil and Bedrock Profile

Generally, the subsurface materials encountered within the Promontory Subdivision consisted of dry to wet, very loose to dense, silty to clayey sand and dry to wet, weathered to very hard siltstone bedrock with occasional occurrences of claystone lenses. These soils were encountered at various depths throughout the site. These soils extended down to the 20-foot termination depths of the majority of the test borings.

Groundwater

Groundwater was encountered in 15 of the test borings at the time of drilling at depths ranging from 17 and 19 feet below existing grade. When checked 1 to 7 days following the completion of drilling, static groundwater was observed in 45 test bores at depths ranging between 11 to 18 feet below existing grade. Elevation data for each test boring was collected utilizing GPS. The elevations of the top of each bore as well as the measured depth to top of groundwater can be found on the Test Boring Logs.

Fluctuations in groundwater and subsurface moisture conditions may occur due to variations in rainfall and other factors not readily apparent at this time. Development of the property and adjacent properties may also affect groundwater levels.

CONCLUSIONS

The following discussion is based on the subsurface conditions encountered in the test borings and on the project characteristics previously described. If conditions are different from those described in this report or the project characteristics change, RMG should be retained to review our recommendations and adjust them, if necessary. The results of this investigation indicate that the site is suitable for the proposed project provided the recommendations presented herein are implemented.

As previously discussed, the site consists of dry to wet, very loose to very dense, silty to clayey sand, and weathered to very hard siltstone bedrock with occasional occurrences of claystone lensing that are encountered at various depths and extended to the 20 foot termination depth of the test borings. The silty sand samples tested generally exhibited a swell potential up to 0.4% and consolidation potential of up to 2.1%. The siltstone with occasional claystone lensing samples tested exhibited consolidation potential of up to 5.1%, with a majority being less than 1% consolidation, and swell potential of up to 1.9%. Swell/consolidation tests for this investigation were performed by wetting all samples under 1,000 psf surcharge pressures.

Geotechnical recommendations based on the field investigation and laboratory testing are presented below. It must be understood that these recommendations should be verified after the excavation on each individual lot is completed.

SITE DEVELOPMENT AND EARTHWORK

Site Preparation

Prior to construction, the ground surface in proposed structure and improvement areas should be stripped of existing vegetation, debris, topsoil, undocumented fill, soft, loose, or disturbed native soils, and other deleterious material. Materials generated during clearing operations should be removed from the project site for disposal. Soft, loose, or yielding subgrade should

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be removed to a depth that exposes firm subgrade and replaced with structural fill. In areas to receive structural fill, the exposed subgrade should be scarified, moisture conditioned, and compacted per the recommendations set forth in this report.

Excavations

The on-site surface and near surface soils may generally be excavated with heavy-duty earthmoving or excavation equipment in good operating condition. Excavations into the bedrock may have variable excavation rates and the use of more aggressive excavation techniques, such as single-shank rippers or other rock breaking equipment, may be needed to achieve proposed site grades, specifically where the very hard siltstone was encountered. During wet weather, earthen berms, swales, or other methods should be used where necessary to route water away from excavations. Water that accumulates in excavations should be promptly pumped out or otherwise removed and the area allowed to dry before resuming construction.

Geotechnical Considerations

Groundwater:

Groundwater was encountered at depths anticipated to impact basement construction in 5 of the test borings (lots 343, 344, 345, 346, and 347) performed for this investigation.

A minimum 3-foot separation is generally recommended between the bottom of the foundation components/floor slabs and the estimated seasonal high-water table levels. However, if the recommended separation cannot be readily achieved, additional drainage and/or ground stabilization measures may be required. If groundwater is determined to be within 3 feet of proposed foundation components or if soil in the bottom of the excavation is soft or unstable due to elevated water conditions, a 3 foot layer of 3/4" to 1.5" diameter angular washed rock or 3/4" to 3.0" diameter crushed concrete with both of these options containing less than 5% passing the #200 sieve is recommended directly beneath the footings and slabs.

Soft/Loose Soils:

Soft/loose soils were encountered at anticipated foundation depths throughout the investigated. Soft/loose soils under foundation components should have a minimum of 2 feet of material removed, moisture-conditioned to within -1% to +3% of optimum moisture content, and compacted to a minimum of 95% maximum dry density as determined by the Standard Proctor (ASTM-698) that results in a minimum of two feet of reconditioned soil beneath the foundation components and slabs.

Isolated pockets of loose soils may be encountered in the excavations, even on lots where none are indicated on the test borings. If soft/loose soils are encountered, they may also require additional compaction and/or stabilization as previously mentioned to achieve the allowable bearing pressure indicated in this report.

It is imperative to maintain the proper moisture in the moisture-conditioned on-site soils throughout the entire construction process including prior to placing the footings, foundation walls and slabs. Once the on-site soils are properly moisturized and compacted they should be protected from the elements to prevent losing moisture. Additionally, the conditioned soils should be protected from increases or decreases in moisture content, such as might occur from exposure to the elements.

Foundation Wall Backfill

Backfill should be placed in loose lifts not exceeding 8 to 12 inches with material no greater than 4 inches in diameter, moisture conditioned to facilitate compaction (usually within 2 percent of the optimum moisture content) and compacted to 90 percent of the maximum dry density as determined by the Standard Proctor test, ASTM D-698 on exterior sides of walls in landscaped areas. In areas where backfill supports pavement and concrete flatwork, the materials should be moisture conditioned to \pm 2 percent optimum moisture content compacted to 95 percent of the maximum dry density as determined by the Standard Proctor test, ASTM D-698.

Fill placed on slopes should be benched into the slope. Maximum bench heights should not exceed 4 feet, and bench widths should be wide enough to accommodate compaction equipment.

The backfill should not be placed on frozen subgrade or allowed to freeze during moisture conditioning and placement. Backfill should be compacted by mechanical means, and foundation walls should be braced during backfilling and compaction.

Structural Fill

Areas to receive compacted granular structural fill should have topsoil, organic material, or debris removed. The upper 6 inches of the exposed surface soils should be scarified and moisture conditioned to facilitate compaction and compacted to a minimum of 95 percent of the maximum dry density as determined by the Standard Proctor test (ASTM D-698) prior to placing structural fill.

Structural fill placed on slopes should be benched into the slope. Maximum bench heights should not exceed 4 feet, and bench widths should be wide enough to accommodate compaction equipment.

Structural fill shall consist of non-expansive material. It should be placed in loose lifts not exceeding 8 to 12 inches, moisture conditioned to facilitate compaction (usually within 2 percent of the optimum moisture content) and compacted to a minimum of 95 percent of the maximum dry density as determined by the Standard Proctor test, ASTM D-698. The materials should be compacted by mechanical means.

Materials used for structural fill should be approved by RMG prior to use. Structural fill should not be placed on frozen subgrade or allowed to freeze during moisture conditioning and placement.

To verify the condition of the compacted soils, density tests should be performed during placement. The first density tests should be conducted when 24 inches of fill have been placed.

FOUNDATION OPTIONS

Anticipated Foundation Systems

A spread footing foundation system may be utilized for all lots included in this investigation bearing on the appropriate materials determined during the Open Excavation Observation (i.e. native soils or a minimum of two feet of reconditioned soils due to soft/loose conditions). The maximum allowable bearing pressure of 1,500 psf with a 500 psf minimum dead load requirement may be utilized for design purposes.

Foundation components must be below all organic material and should extend 30 inches or more below the lowest exterior finished grade for frost protection. The foundation design should be prepared by a qualified Colorado Registered Professional Engineer using the recommendations presented in this report. This foundation system should be designed to span a minimum of 10 feet under the design loads.

Open Excavation Observations

During construction, foundation excavations should be observed by RMG prior to placing structural fill, forms or concrete to verify the foundation bearing conditions for each structure.

INTERIOR FLOOR SYSTEMS

Interior Floor Slabs

Vertical slab movement of one to three inches is considered possible for all soil types. In some cases, vertical movement may exceed this range. If movement and associated damage to floors and finishes cannot be tolerated, a structural floor system should be used.

Floor slabs should be separated from structural components to allow for vertical movement. Control and construction joints should be placed in accordance with the latest guidelines and standards published by the American Concrete Institute (ACI) and applicable local Building Code requirements.

Interior Partitions

Interior non-bearing partitions and attached furnishings (e.g., cabinets, shower stalls, etc.) on concrete slabs should be constructed with a void so that they do not transmit floor slab

movement to the roof or overlying floor. A void of at least 1-1/2 inches is recommended beneath non-bearing partitions. The void may require reconstruction over the life of the structure to re-establish the void due to vertical slab movement.

LATERAL EARTH PRESSURES

Foundation walls should be designed to resist lateral earth pressures. For backfill materials that are granular with cohesive fines and non-expansive, we recommend an active fluid pressure of 50 pcf, an at-rest fluid pressure of 70 pcf, or a passive pressure of 240 pcf should be used for design. Expansive soils or bedrock should not be used as backfill against foundation walls.

The above lateral earth pressure applies to level, drained backfill conditions. Equivalent Fluid Pressures for sloping/undrained conditions should be determined on an individual basis.

SURFACE GRADING AND DRAINAGE

Grading and Irrigation

The ground surface should be sloped from the building with a minimum gradient of 10 percent for the first 10 feet. This is equivalent to 12 inches of fall across this 10-foot zone. If a 10-foot zone is not possible on the upslope side of the structure, then a well-defined swale should be created a minimum 5 feet from the foundation and sloped parallel with the wall with a minimum slope of 2 percent to intercept the surface water and transport it around and away from the structure. Roof drains should extend across backfill zones and landscaped areas to a region that is graded to direct flow away from the structure. Homeowners should maintain the surface grading and drainage recommended in this report to help prevent water from being directed toward and/or ponding near the foundations.

Landscaping should be selected to reduce irrigation requirements. Plants used close to foundation walls should be limited to those with low moisture requirements and irrigated grass should not be located within 5 feet of the foundation. To help control weed growth, geotextiles should be used below landscaped areas adjacent to foundations. Impervious plastic membranes are not recommended.

Irrigation devices should not be placed within 5 feet of the foundation. Irrigation should be limited to the amount sufficient to maintain vegetation. Application of more water will increase the likelihood of slab and foundation movements.

The recommendations listed in this report are intended to address normal surface drainage conditions, assuming the presence of groundcover (established vegetation, paved surfaces, and/or structures) throughout the regions upslope from this structure. However, groundcover may not be present due to a variety of factors (ongoing construction/development, wildfires, etc.). During periods when groundcover is not present in the "upslope" regions, higher than normal surface drainage conditions may occur, resulting in perched water tables, excess runoff,

flash floods, etc. In these cases, the surface drainage recommendations presented herein (even if properly maintained) may not mitigate all groundwater problems or moisture intrusion into the structure. We recommend that the site plan be prepared with consideration of increased runoff during periods when groundcover is not present on the upslope areas.

Perimeter Drain

A subsurface perimeter drain is recommended around portions of the structure which will have habitable or storage space located below the finished ground surface. This includes crawlspace areas but not the walkout trench, if applicable.

The perimeter drain can be installed as an interior (if a minimum of six inches of free draining aggregate is placed beneath the slab) or exterior perimeter drain system. The perforated drainage pipe should be installed so the top of the pipe is not above the top of the footing and should be surrounded by material to reduce the infiltration of silt into the drainage pipe. The pipe should be installed in one of the following manners:

1) The pipe may be installed as level as possible as long as the pipe is placed in a minimum of six inches of gravel or crushed stone and discharges into a sump pit with mechanical means to remove the water.

Or

2) The pipe may be installed as a gravity system with a minimum 1/8 inch fall per 1 foot length surrounded by a minimum six inches of gravel or crushed stone that either daylights to allow free flow drainage or discharges into a sump pit with mechanical means to remove the water.

A subsurface perimeter drain is designed to intercept some types of subsurface moisture and not others. Therefore, the drain could operate properly and not mitigate all moisture problems relating to foundation performance or moisture intrusion into the basement area.

CONCRETE

Sulfate testing was performed on selected samples based on ASTM C1580. Test results yielded 0.00% to 0.14% sulfate by weight, indicating the soils present low sulfate exposure. Based on these results and on the test results from our previous investigation (referenced above), Type V (or equivalent) cement is recommended for concrete in contact with the subsurface materials. Calcium chloride should be used with caution for soils with high sulfate contents. The concrete should not be placed on frozen ground. If placed during periods of cold temperatures, the concrete should be kept from freezing. This may require covering the concrete with insulated blankets and heating. Concrete work should be completed in accordance with the latest applicable guidelines and standards published by ACI.

Recommendations for exterior concrete slabs, such as patios, driveways, and sidewalks, are not included in this report.

CLOSING

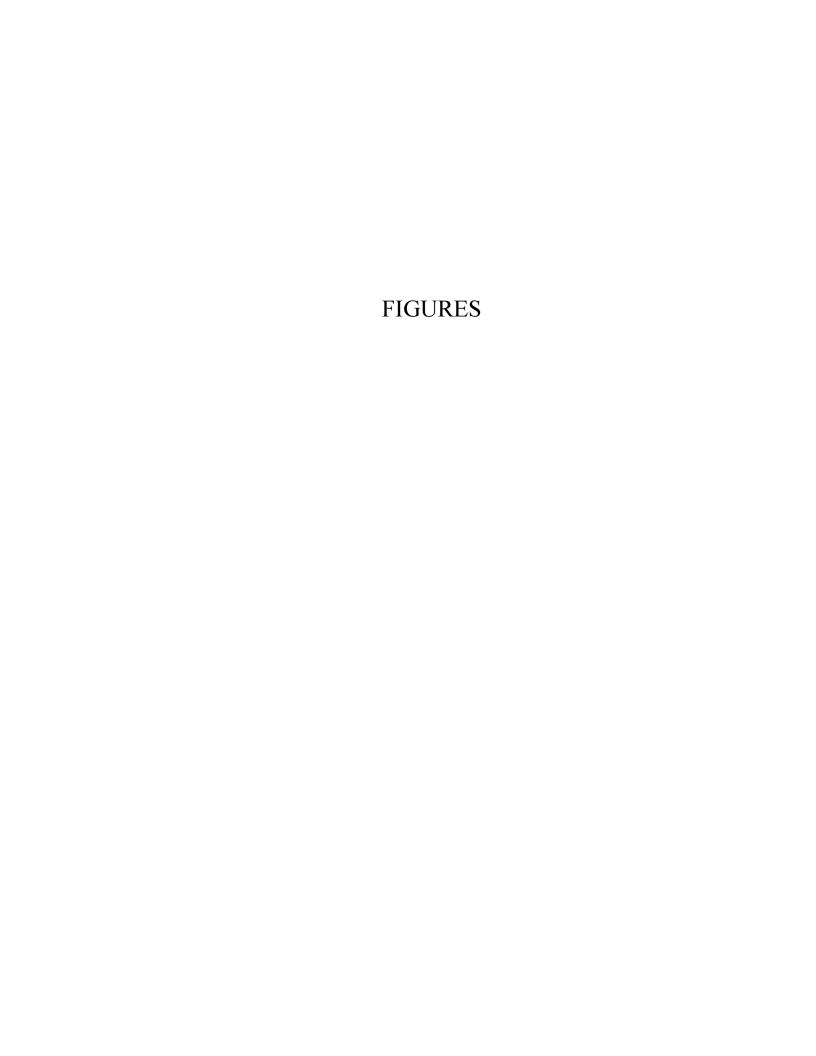
This report has been prepared for the exclusive purpose of providing geotechnical engineering information and recommendations for development described in this report. RMG should be retained to review the final construction documents prior to construction to verify our findings, conclusions and recommendations have been appropriately implemented.

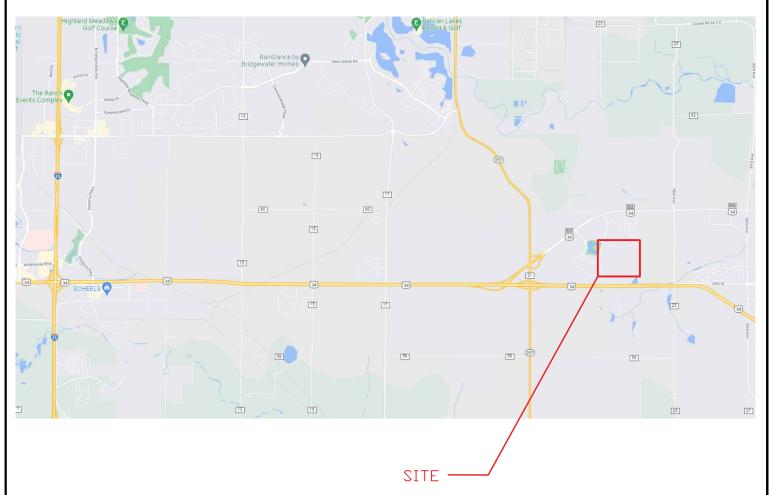
This report has been prepared for the exclusive use by **Journey Homes, LLC** for application as an aid in the design and construction of the proposed development in accordance with generally accepted geotechnical engineering practices. The analyses and recommendations in this report are based in part upon data obtained from test borings and site observations. The nature and extent of variations may not become evident until construction. If variations then become evident, RMG should be retained to review the recommendations presented in this report considering the varied condition, and either verify or modify them in writing.

Our professional services were performed using that degree of care and skill ordinarily exercised, under similar circumstances, by geotechnical engineers practicing in this or similar localities. RMG does not warrant the work of regulatory agencies or other third parties supplying information which may have been used during the preparation of this report. No warranty, express or implied is made by the preparation of this report. Third parties reviewing this report should draw their own conclusions regarding site conditions and specific construction techniques to be used on this project.

The scope of services for this project does not include, either specifically or by implication, environmental assessment of the site or identification of contaminated or hazardous materials or conditions. Development of recommendations for the mitigation of environmentally related conditions, including but not limited to biological or toxicological issues, are beyond the scope of this report. If the Client desires investigation into the potential for such contamination or conditions, other studies should be undertaken.

If we can be of further assistance in discussing the contents of this report or analysis of the proposed development, from a geotechnical engineering point-of-view, please feel free to contact us.







<u>CLIENT:</u>

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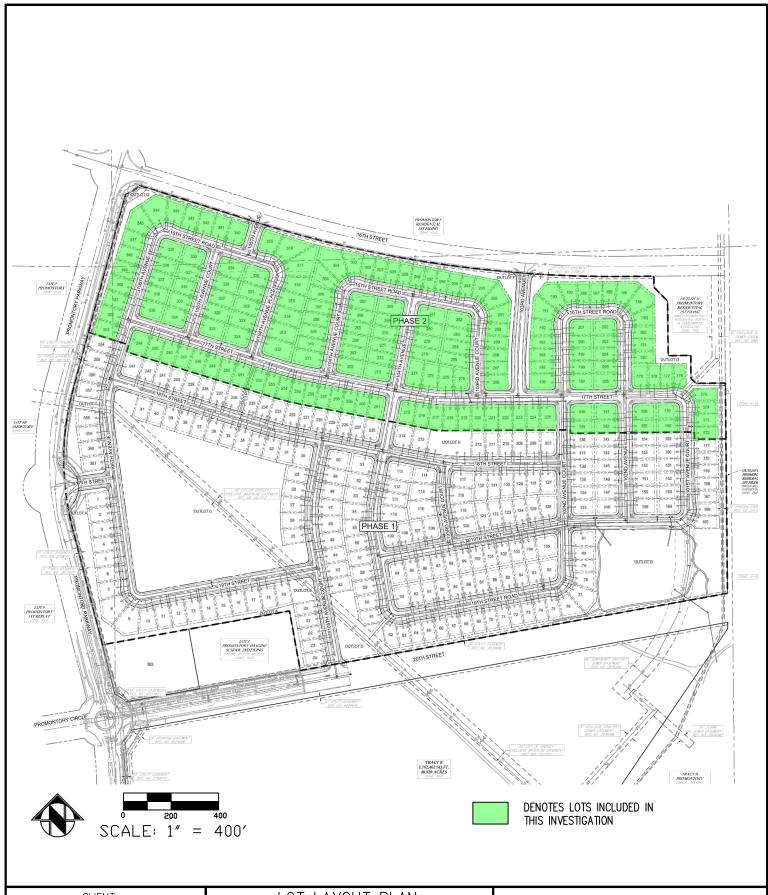
RMG PROJECT #184203 DATE: MARCH 29, 2022

SITE VICINITY MAP

LOTS 140 THROUGH 142, LOTS 156 THROUGH 158, LOTS 172, THROUGH 206, LOTS 215 THROUGH 225 AND LOTS 224 THROUGH 353, PROMONTORY SUBDIVISION PHASE 2 GREELEY, COLORADO



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CLIENT :

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> RMG PROJECT #184203 DATE: MARCH 29, 2022

LOT LAYOUT PLAN

LOTS 140 THROUGH 142, LOTS 156 THROUGH 158, LOTS 172, THROUGH 206, LOTS 215 THROUGH 225 AND LOTS 224 THROUGH 353, PROMONTORY SUBDIVISION PHASE 2 GREELEY, COLORADO



7292 GREENRIDGE RD., UNIT 108 WINDSOR, CO. 80550 PHONE: (970) 330-1071 FAX: (970) 330-1252

LOT No.: Lot 139 DATE DRILLED: 7/7/21 ELEVATION (FT): 4925.8 GROUNDWATER @ Dry' 7/9/21	ОЕРТН (FT)	SYMBOL	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 140 DATE DRILLED: 7/7/21 ELEVATION (FT): 4926.2 GROUNDWATER @ Dry ' 7/9/21	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SAND, silty brown, fine-grained, loose, moist	5		13	12.9	SAND, silty brown, fine-grained, loose, dry to moist	5 —			7	8.9
SILTSTONE brown with rust staining, weathered to hard, moist	10 —	× × × × × × × × × × × × × × × × × × ×	26	14.6	SILTSTONE, clayey brown with rust staining, firm to hard, moist	10 —	× × × × × × × × × × × × × × × × × × ×		32	17.8
	- 15 —-	**************************************	50/6"	15.6		15 —	× × × × × × × × × × × × × × × × × × ×		50/8"	12.2
ROCKY MOUNTAIN	20	× × × × × × × × × × × × × × × × × × ×	50/6"	18.9		20	× × × × × × × × × × × × × × × × × × ×		50/6"	19.2

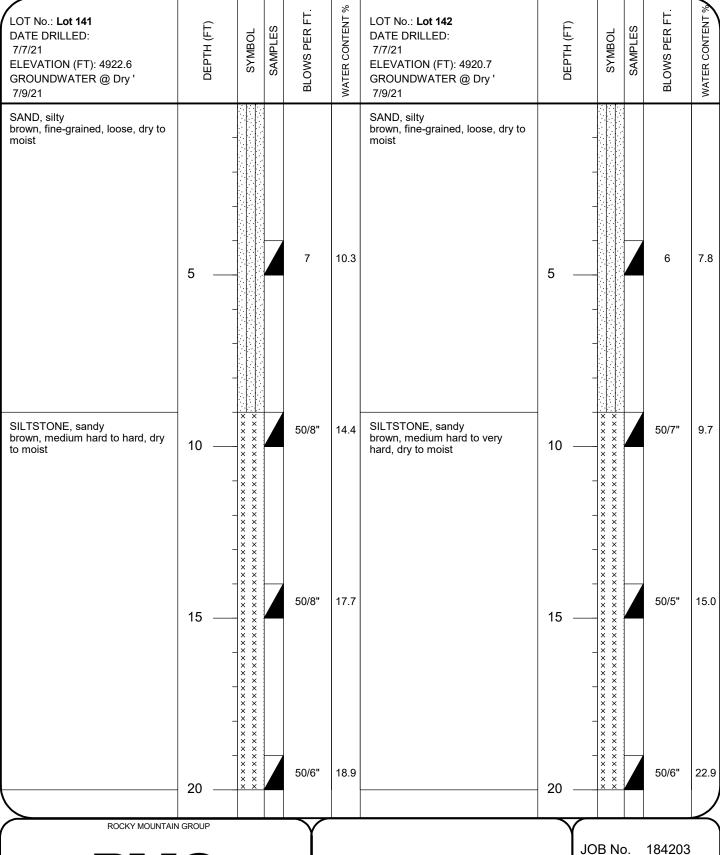


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TEST BORING LOGS

FIGURE No. 3



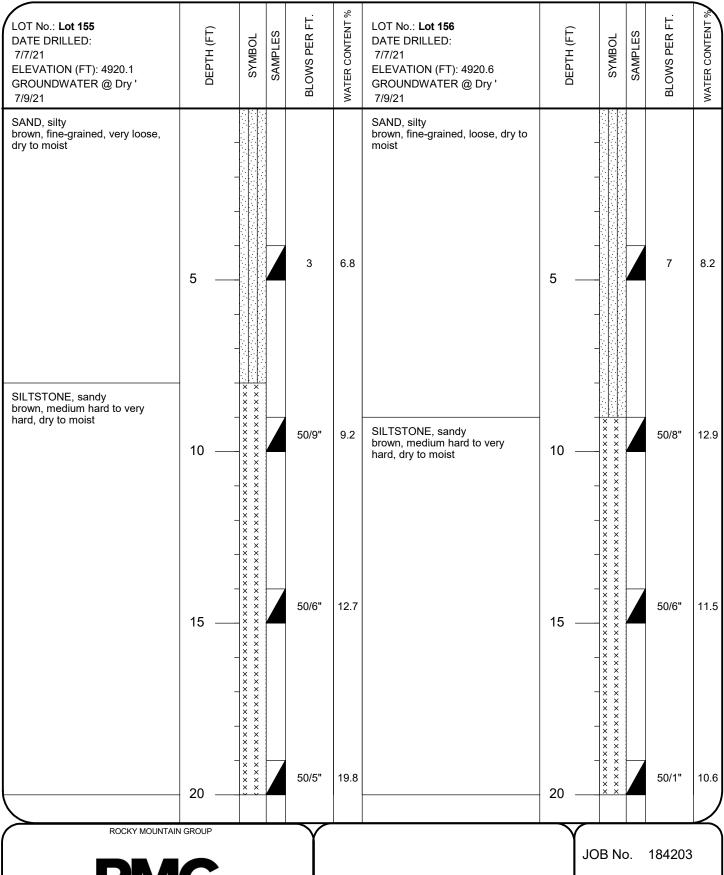


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TEST BORING LOGS

184203

FIGURE No. 4



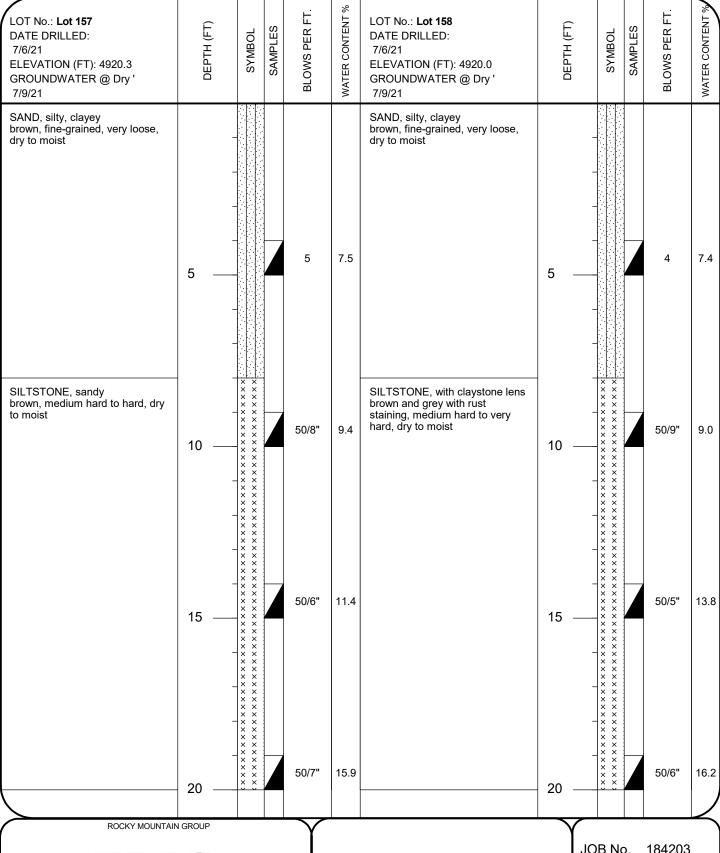


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Geotechnical Materials Testing Civil, Planning TEST BORING LOGS

FIGURE No. 5





TEST BORING LOGS

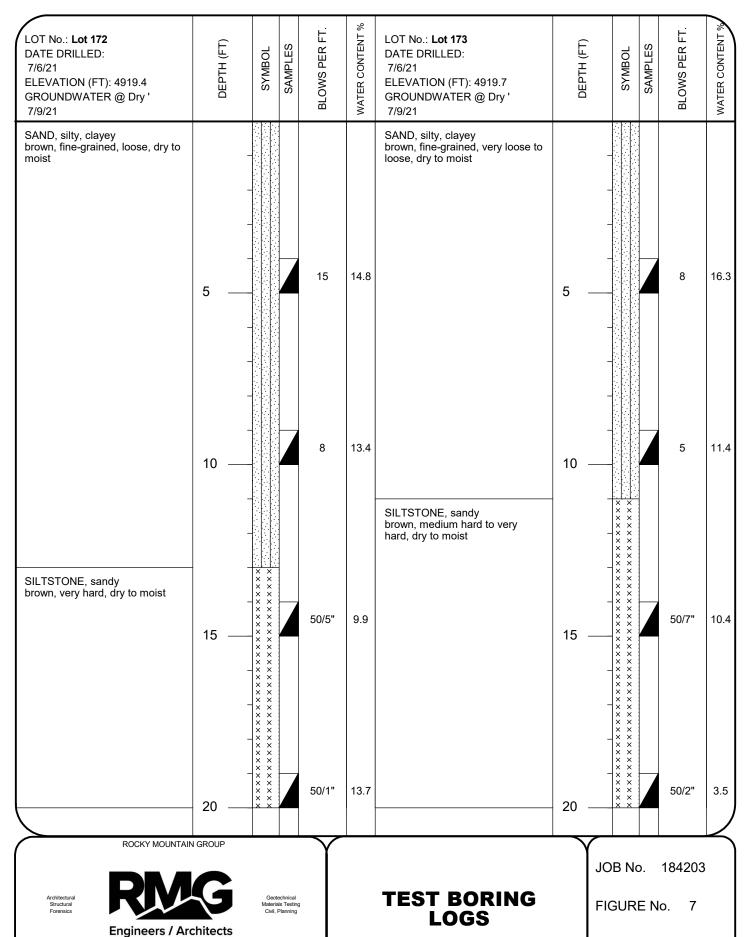
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FIGURE No. 6

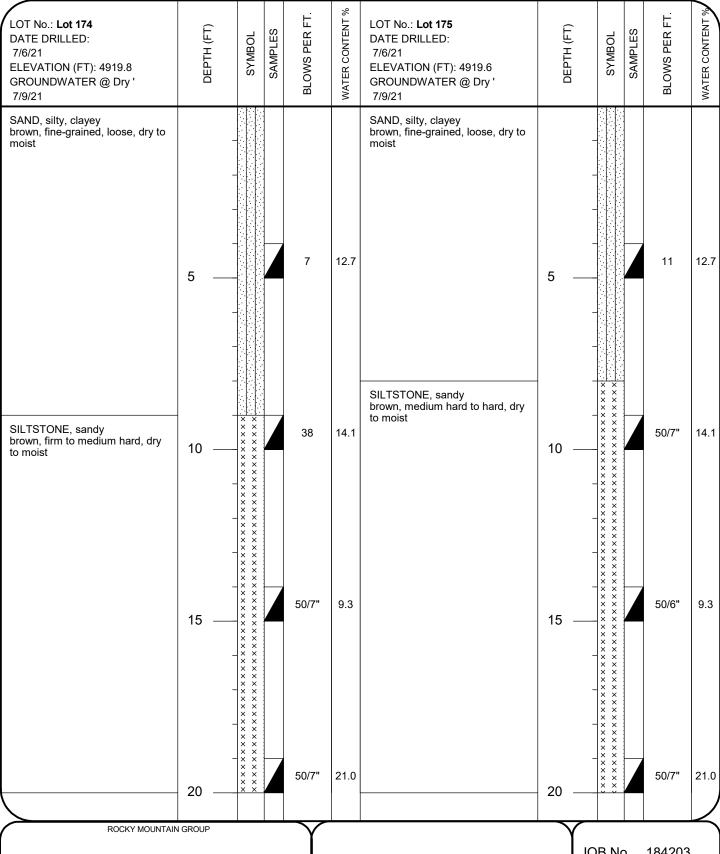
Mar/28/2022 DATE

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TEST BORING LOGS

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FIGURE No. 8

DATE Mar/28/2022

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LOT No.: Lot 176 DATE DRILLED: 2/10/22 ELEVATION (FT): 4919.0 GROUNDWATER @ Dry ' 2/11/22	ОЕРТН (FT)	SYMBOL	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 177 DATE DRILLED: 2/10/22 ELEVATION (FT): 4920.4 GROUNDWATER @ Dry ' 2/11/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE tan and brown, medium hard to very hard, dry to moist	5 —	× × × × × × × × × × × × × × × × × × ×	50/7"	7.2	SILTSTONE tan and brown, firm to very hard, dry to moist	5 —	× × × × × × × × × × × × × × × × × × ×	Z	44	10.5
	10	× × × × × × × × × × × × × × × × × × ×	50/5"	9.7		10 —	× × × × × × × × × × × × × × × × × × ×		50/5"	11.3
	15 —	× × × × × × × × × × × × × × × × × × ×	50/4"	18.4	Auger Refusal at 16 feet due to very hard Bedrock	15 —	× × × × × × × × × × × × × × × × × × ×		50/4"	12.4
ROCKY MOUNTAIL	- 20	× × × × × × × × × × × × × × × × × × ×	50/5"	20.3						



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TEST BORING LOGS

JOB No. 184203

FIGURE No. 9

LOT No.: Lot 178 DATE DRILLED: 2/10/22 ELEVATION (FT): 4921.7 GROUNDWATER @ Dry' 2/11/22	DЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 179 DATE DRILLED: 2/10/22 ELEVATION (FT): 4922.8 GROUNDWATER @ Dry ' 2/11/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE, tan and brown, medium hard to very hard, dry to moist		× × × × × × × × × × × × × × × × × × ×				SILTSTONE, tan and brown, medium hard to very hard, dry to moist		× × × × × × × × × × × × × × × × × × ×			
	5 —	· · · · · · · · · · · · · · · · · · ·		50/9"	9.8		5 —	-		50/10"	10.9
	10 —	× × × × × × × × × × × × × × × × × × ×		50/6"	10.3		10 —	× × × × × × × × × × × × × × × × × × ×		50/6"	9.4
	15 —	× × × × × × × × × × × × × × × × × × ×		50/4"	12.6	Auger Refusal at 16 feet due to very hard Bedrock	15 —	× × × × × × × × × × × × × × × × × × ×		50/3"	11.0
ROCKY MOUNTAIN	20 —	× × × × × × × × × × × × × × × × × × ×		50/5"	20.3						



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TEST BORING LOGS

JOB No. 184203

FIGURE No. 10

LOT No.: Lot 180 DATE DRILLED: 2/9/22 ELEVATION (FT): 4923.1 GROUNDWATER @ Dry ' 2/11/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 181 DATE DRILLED: 2/9/22 ELEVATION (FT): 4920.7 GROUNDWATER @ Dry ' 2/11/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE, sandy brown and tan, medium hard to very hard, dry to moist	5 —	× × × × × × × × × × × × × × × × × × ×		50/8"	18.0	SILTSTONE tan and brown, very hard, moist	5	× × × × × × × × × × × × × × × × × × ×		50/5"	9.2
Auger Refusal at 12 feet due to very hard Bedrock	10 —	\(\chi \) \(\chi		50/6"	12.7		10 —	^		50/5"	10.3
						Auger Refusal at 15 feet due to very hard Bedrock	-	× × × × × × × × × × × × × × × × × × ×		50/5"	12.4
ROCKY MOUNTAIN	N GROUP			Y			Y ₁₀	DB No		184203	



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TEST BORING LOGS JOB No. 184203

FIGURE No. 11

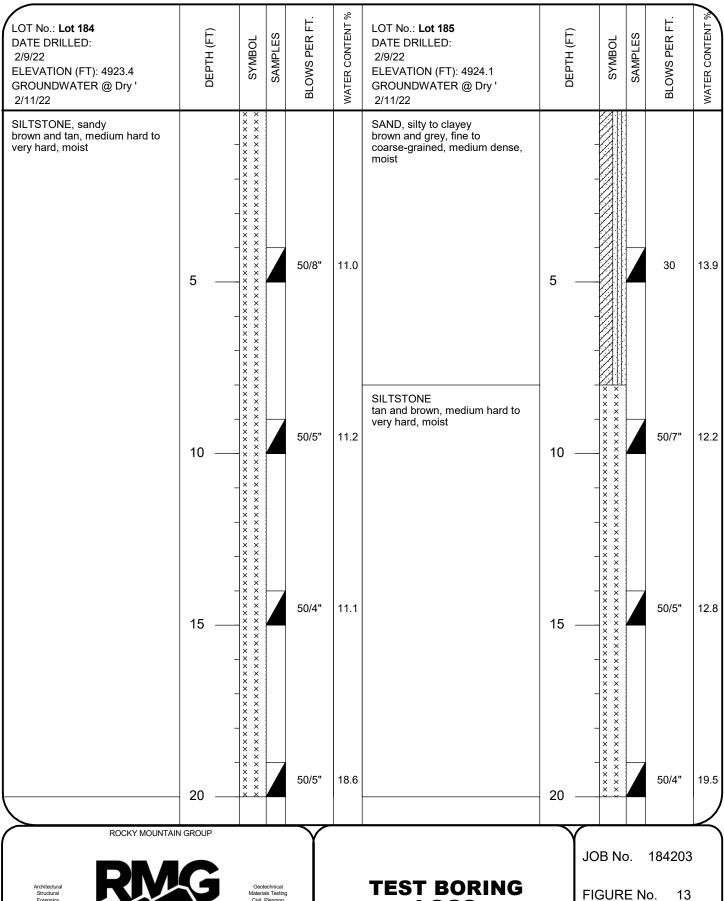
LOT No.: Lot 182 DATE DRILLED: 2/9/22 ELEVATION (FT): 4920.8 GROUNDWATER @ Dry ' 2/11/22	ОЕРТН (FT)	SYMBOL SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 183 DATE DRILLED: 2/9/22 ELEVATION (FT): 4923.6 GROUNDWATER @ Dry ' 2/11/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE, sandy tan and brown, hard to very hard, moist	5	× × × × × × × × × × × × × × × × × × ×	50/6"	8.4	SILTSTONE, tan and brown, firm to very hard, moist	5 —			39	13.4
	10 —	× × × × × × × × × × × × × × × × × × ×	50/5"	10.9		10 —	- X X X X X X X X X X X X X X X X X X X		50/7"	9.2
Auger Refusal at 16 feet due to very hard Bedrock	15 —	× × × × × × × × × × × × × × × × × × ×	50/4"	18.1		15 —	- x x x x x x x x x x x x x x x x x x x		50/5"	14.1
ROCKY MOUNTAIN	N GROUP					20 -	JOB N		50/5"	19.3



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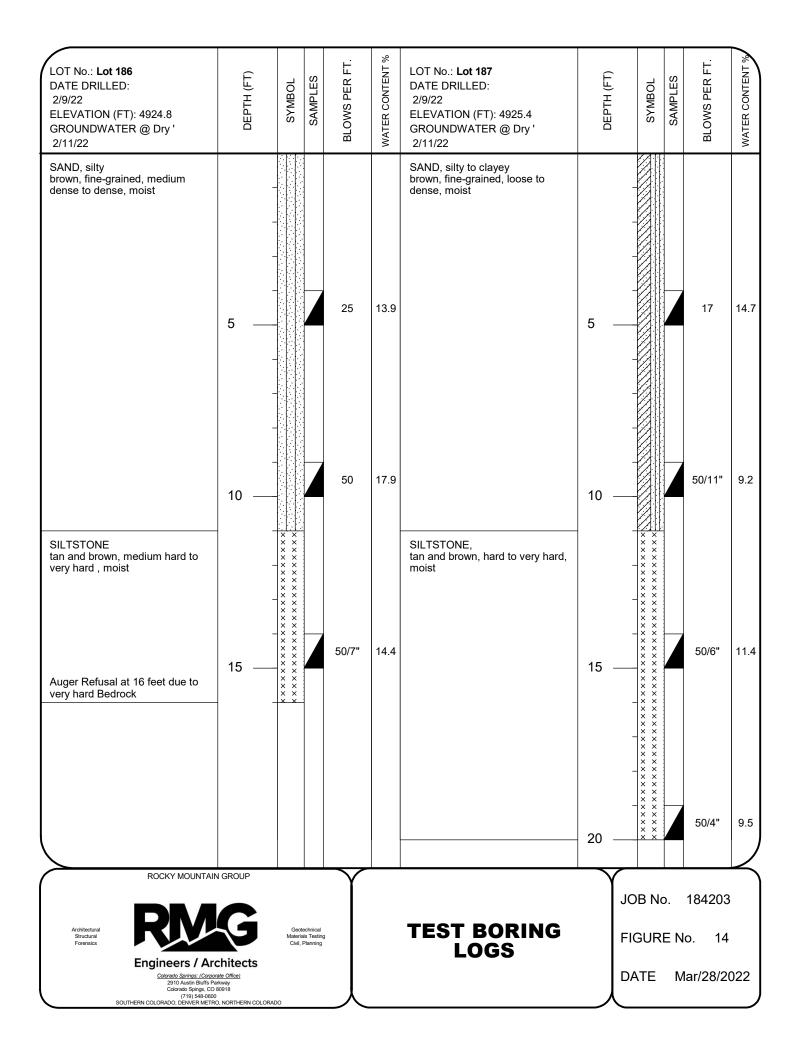
FIGURE No. 12

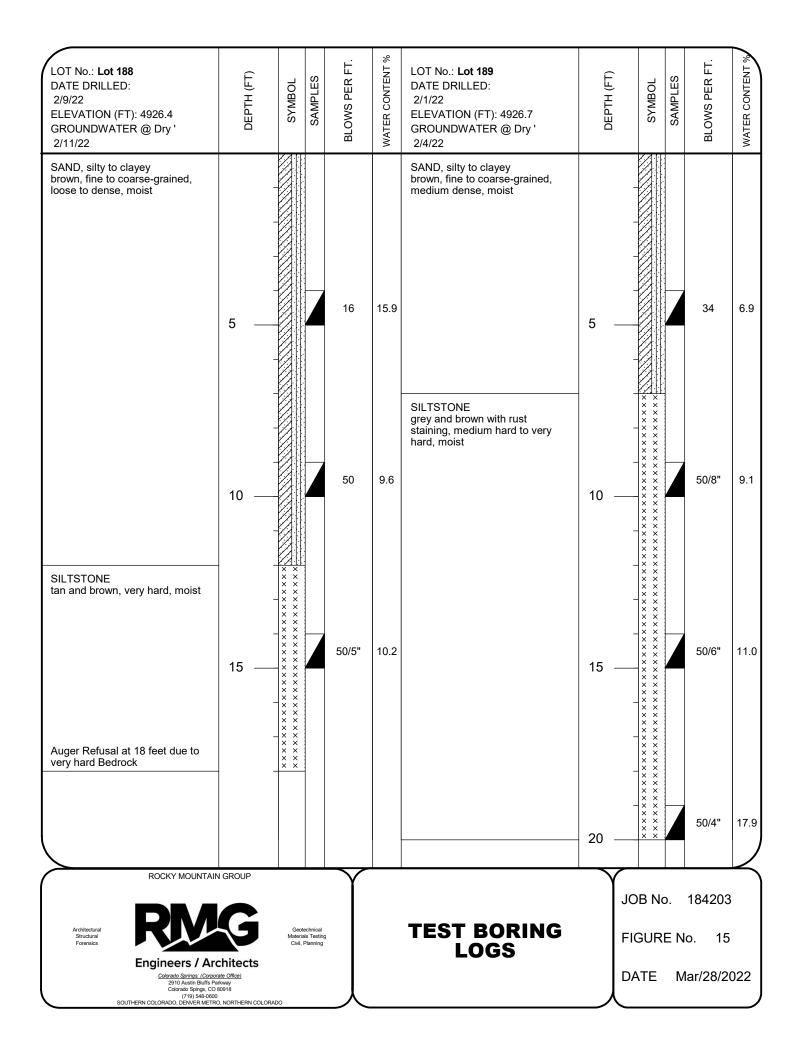


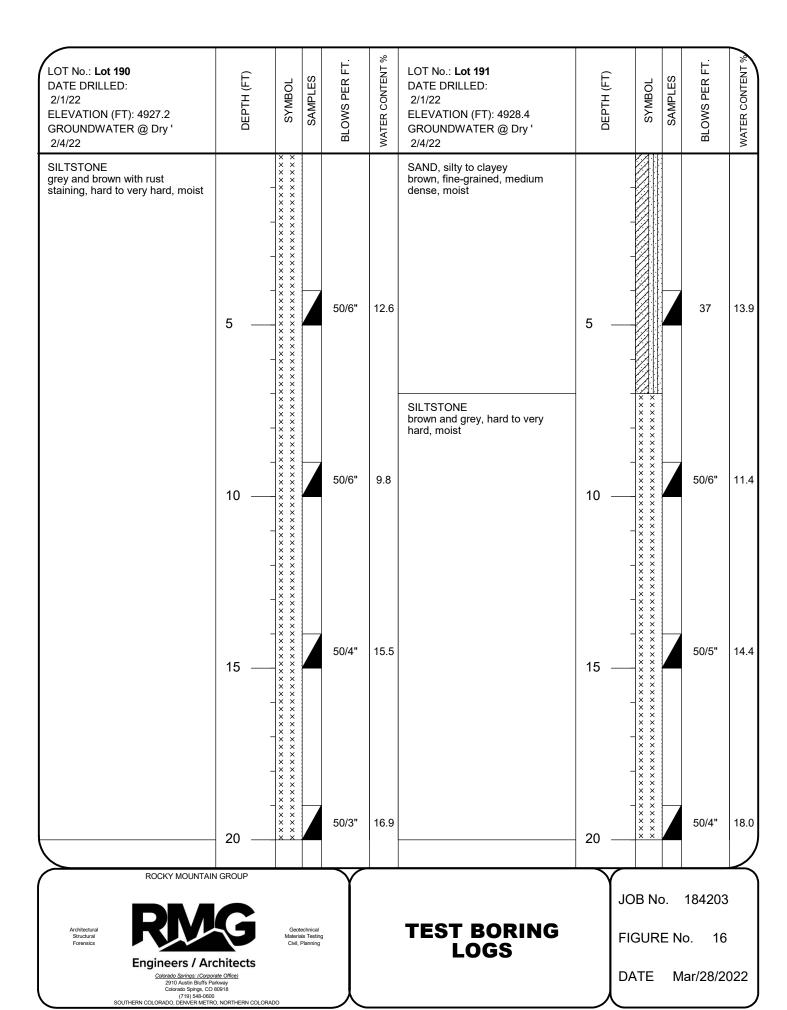
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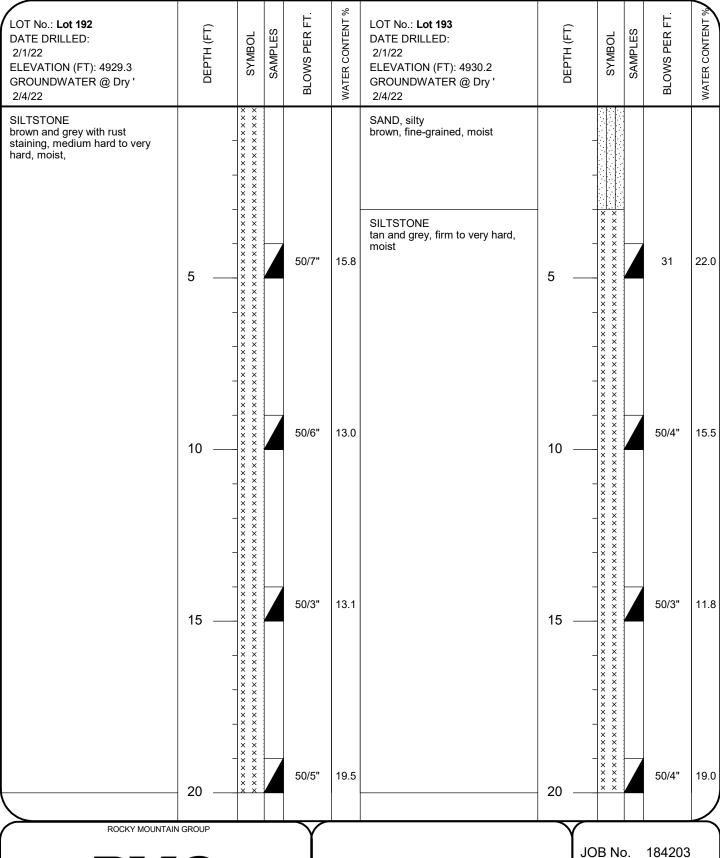
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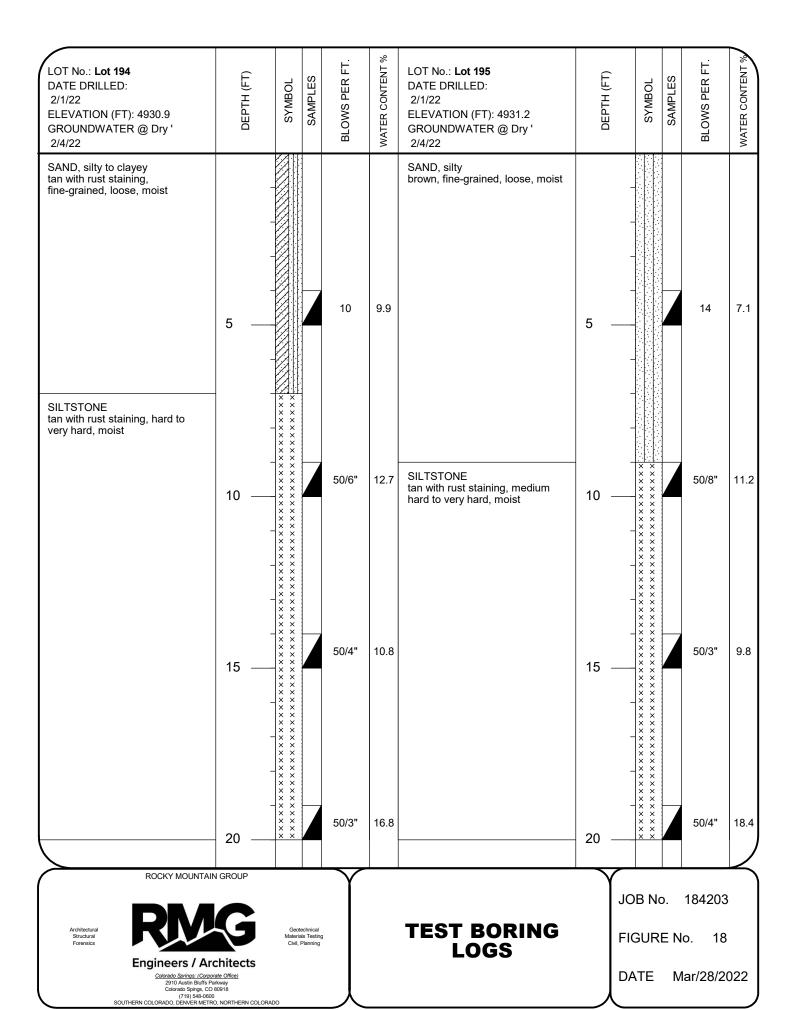


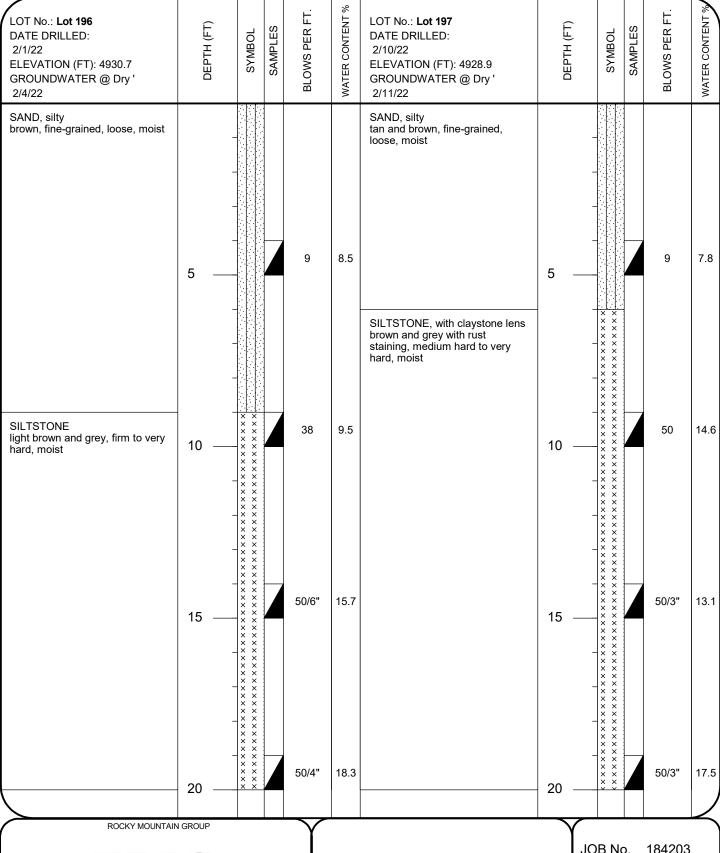


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FIGURE No. 17







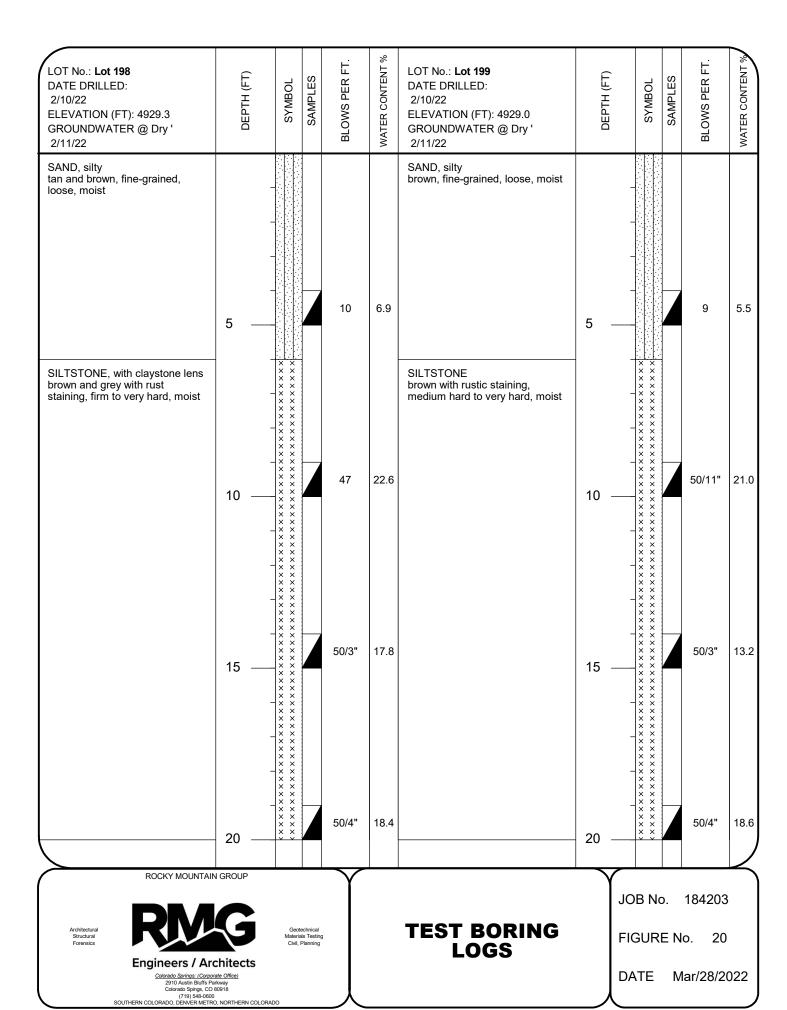
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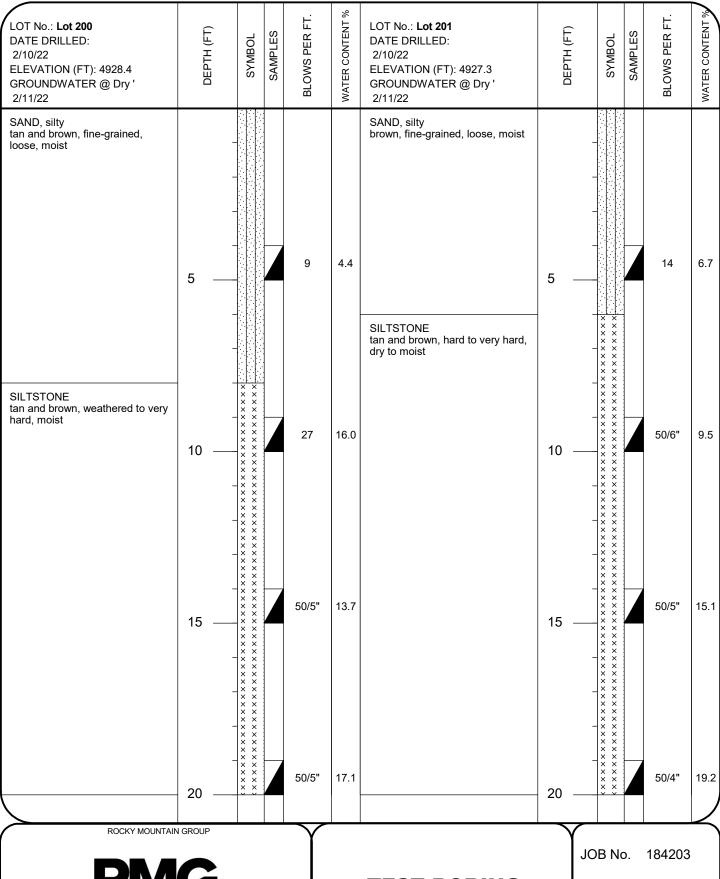
TEST BORING LOGS

JOB No. 184203

FIGURE No. 19

Mar/28/2022 DATE



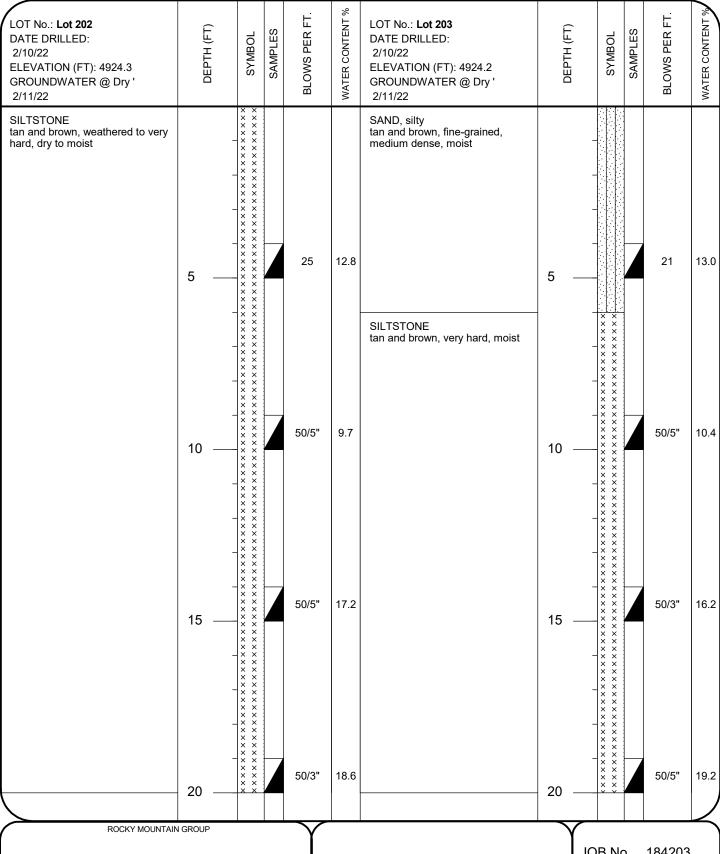




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FIGURE No. 21





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JOB No. 184203

FIGURE No. 22

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LOT No.: Lot 204 DATE DRILLED: 2/10/22 ELEVATION (FT): 4924.6 GROUNDWATER @ Dry ' 2/11/22	DЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 205 DATE DRILLED: 2/10/22 ELEVATION (FT): 4923.8 GROUNDWATER @ Dry ' 2/11/22	DEРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE, with claystone lens grey and brown, with rust staining, weathered to very hard, dry to moist	->					SILTSTONE, with claystone lens tan and brown, with rust staining, medium hard to very hard, moist		× × × × × × × × × × × × × × × × × × ×			1
	5			27	22.3		5 —	× × × × × × × × × × × × × × × × × × ×		50/9"	18.6
	10			50/6"	17.5		10 —	**************************************		50/6"	16.1
	15 —	<pre></pre>		50/4"	18.6		15 —	× × × × × × × × × × × × × × × × × × ×		50/5"	17.8
ROCKY MOUNTAIN	20			50/5"	20.0		20 —	× × × × × × × × × × × × × × × × × × ×		50/4"	23.0



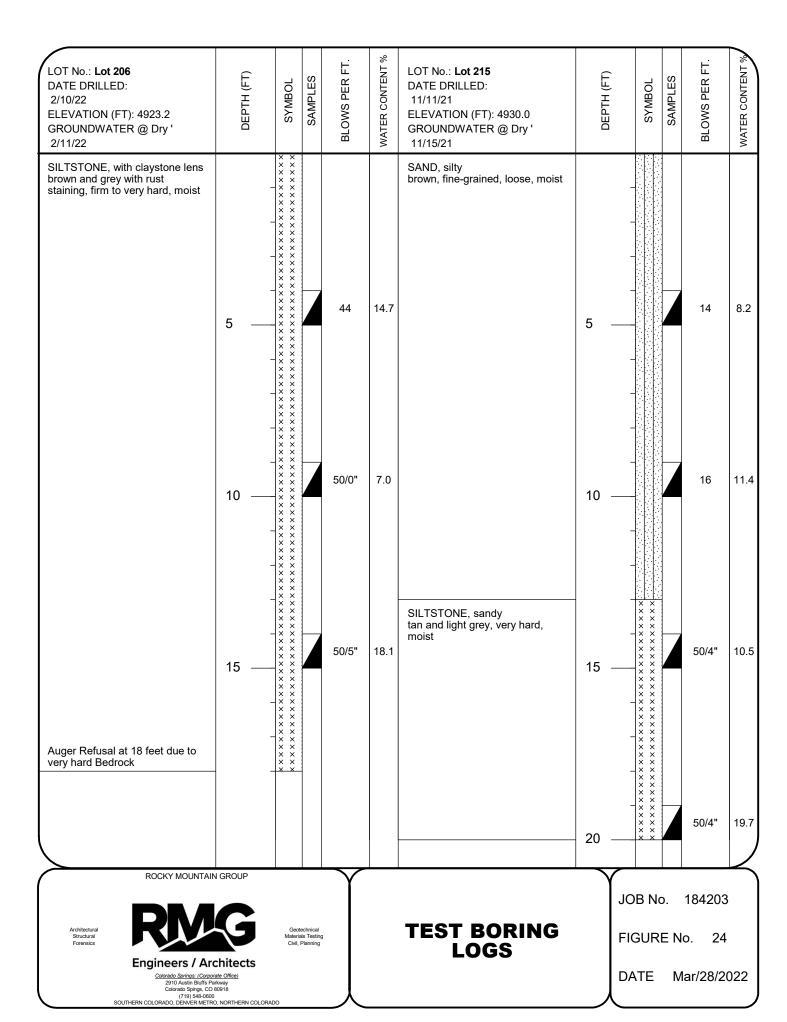
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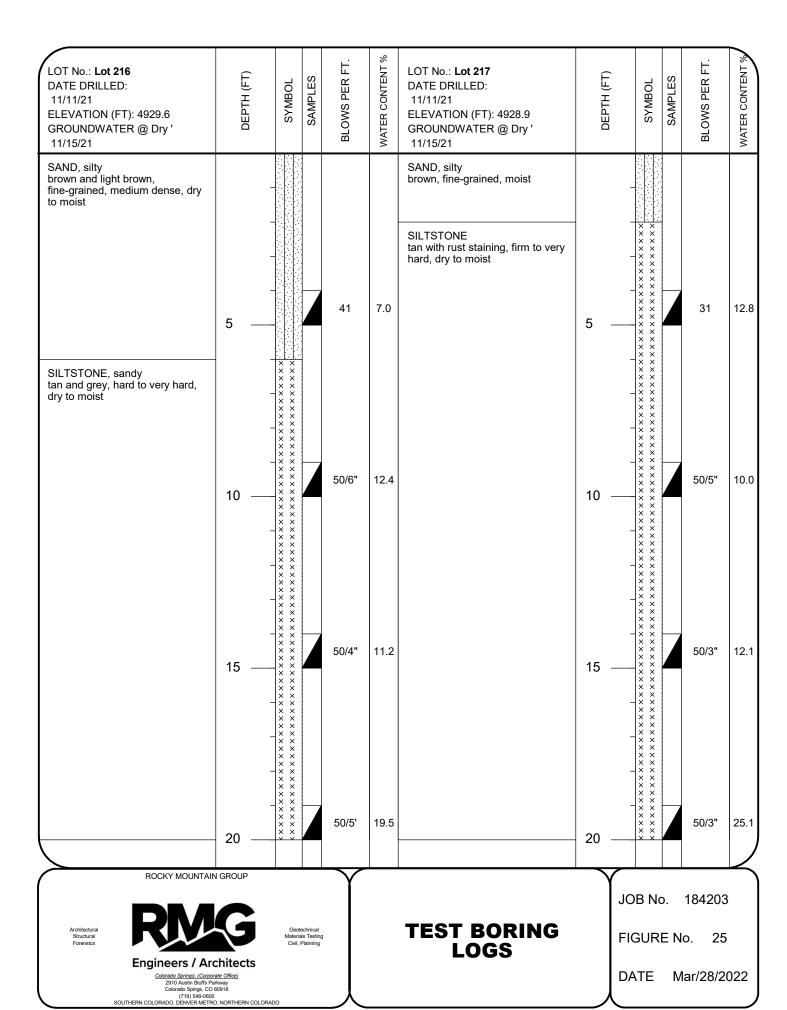
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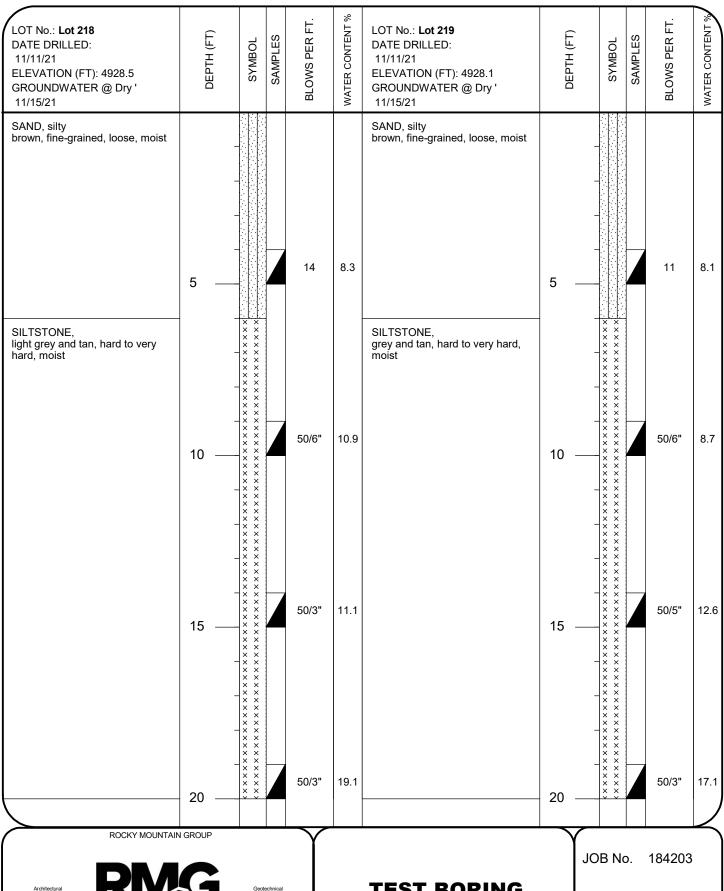
TEST BORING LOGS

JOB No. 184203

FIGURE No. 23





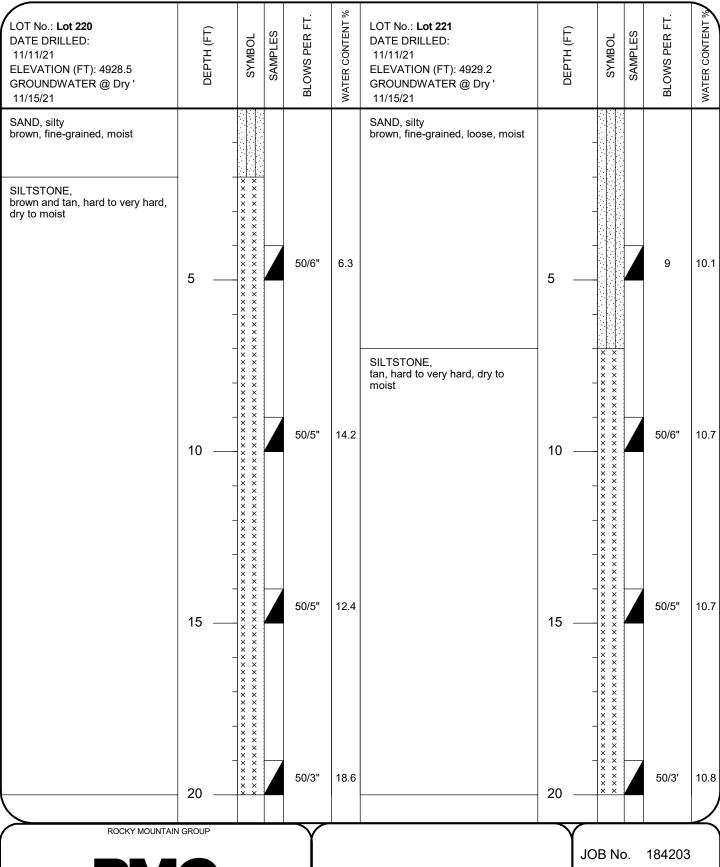




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FIGURE No. 26

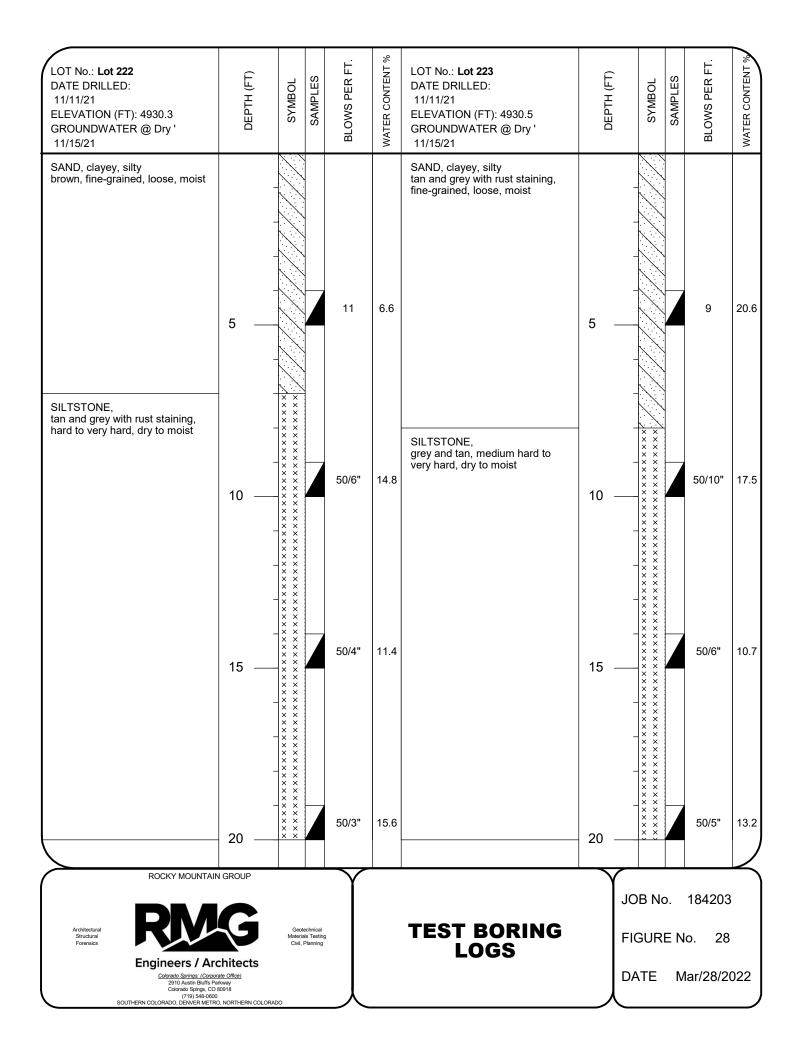


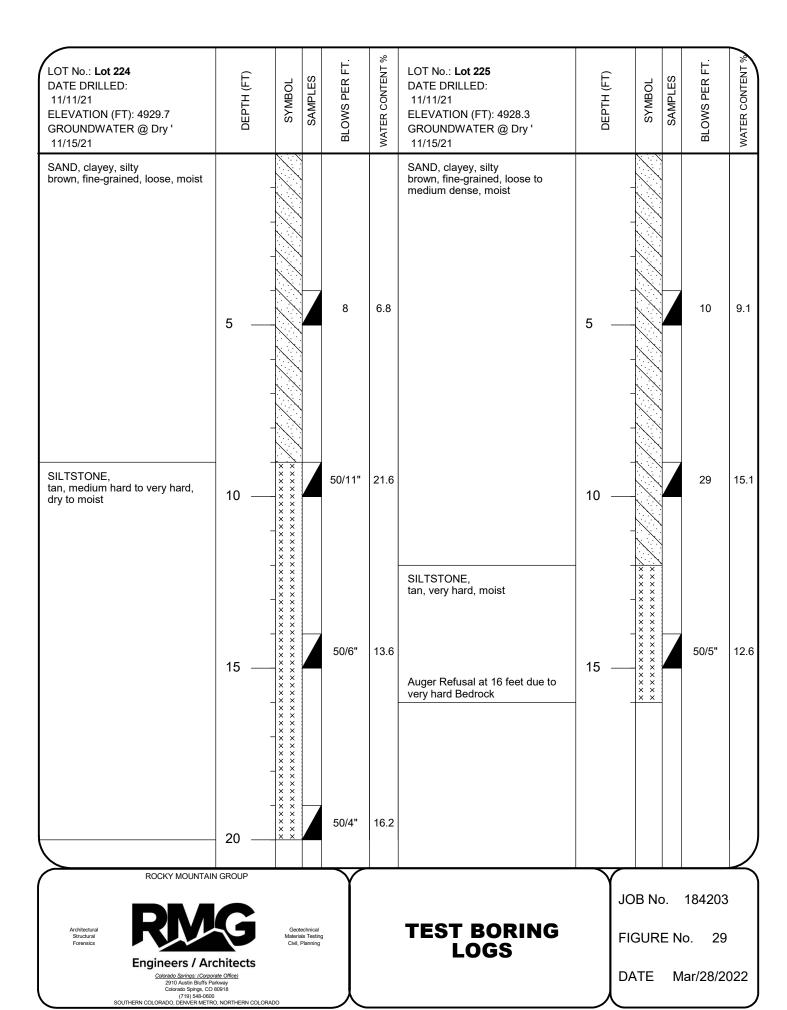


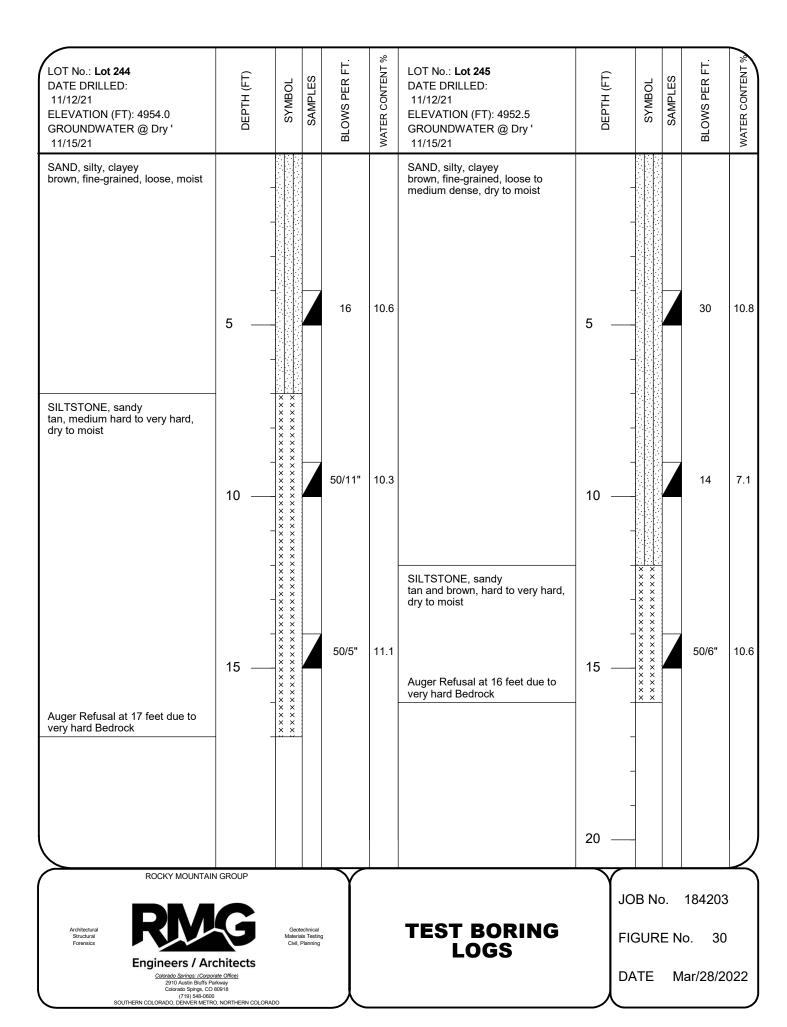
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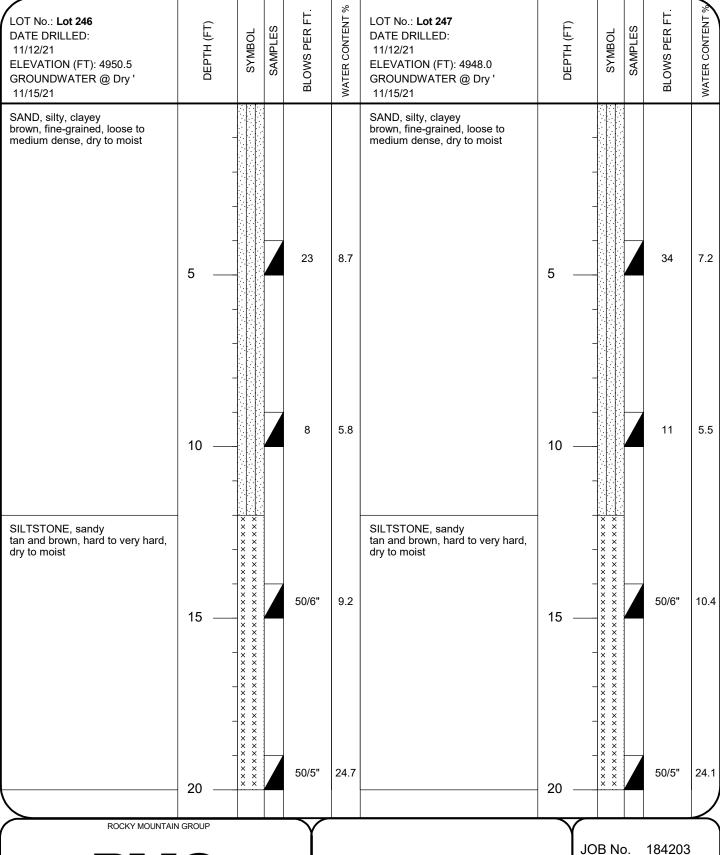
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FIGURE No. 27











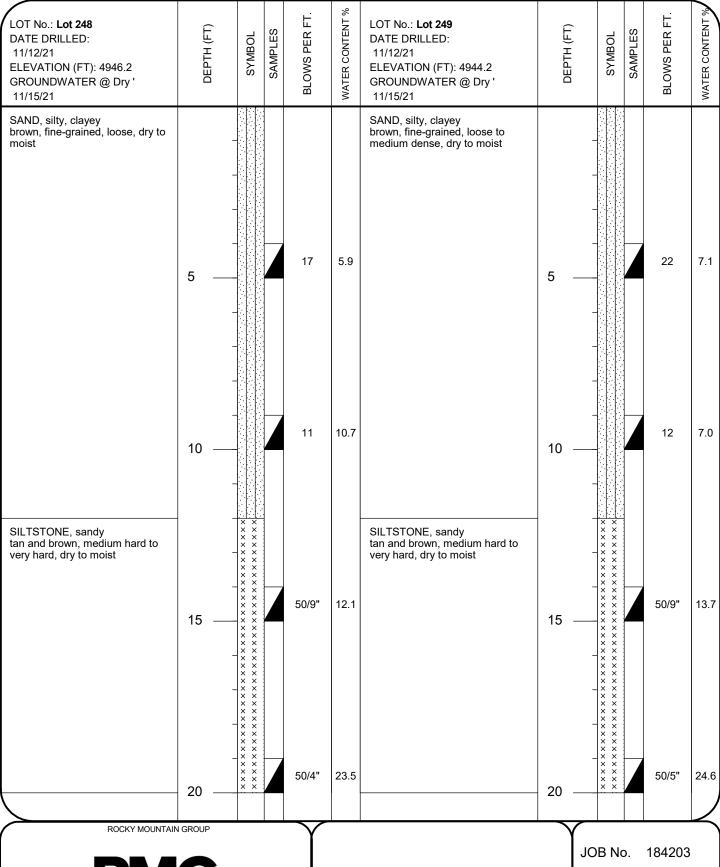
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JOB No.

FIGURE No. 31

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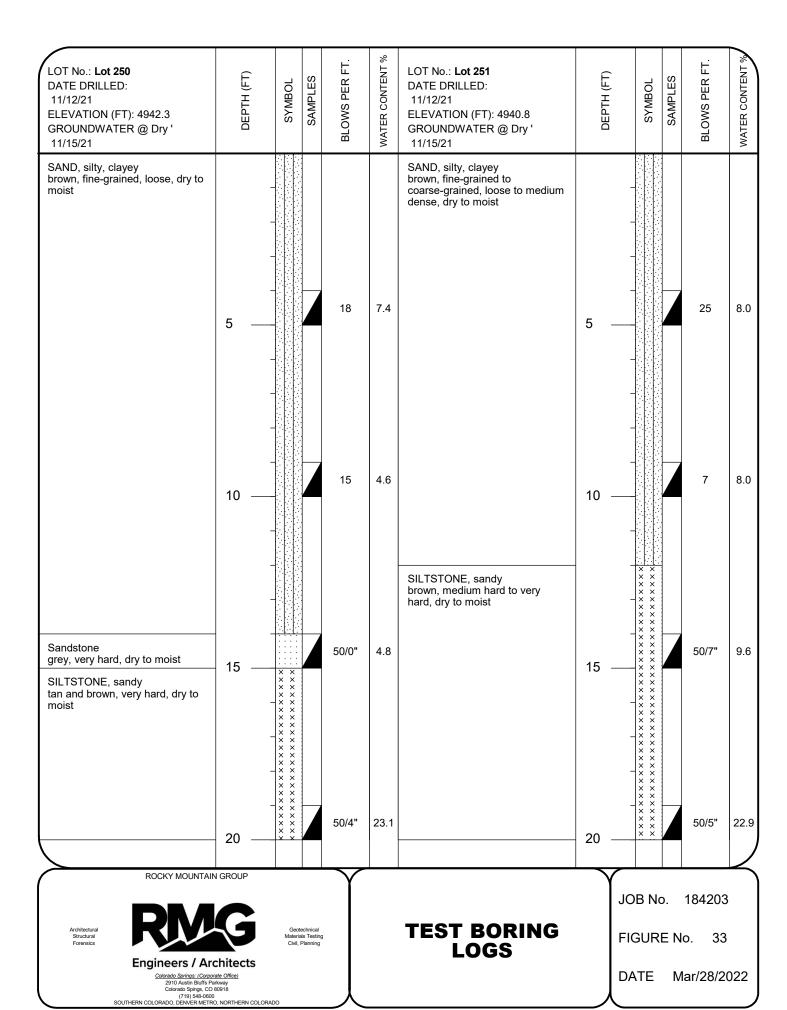


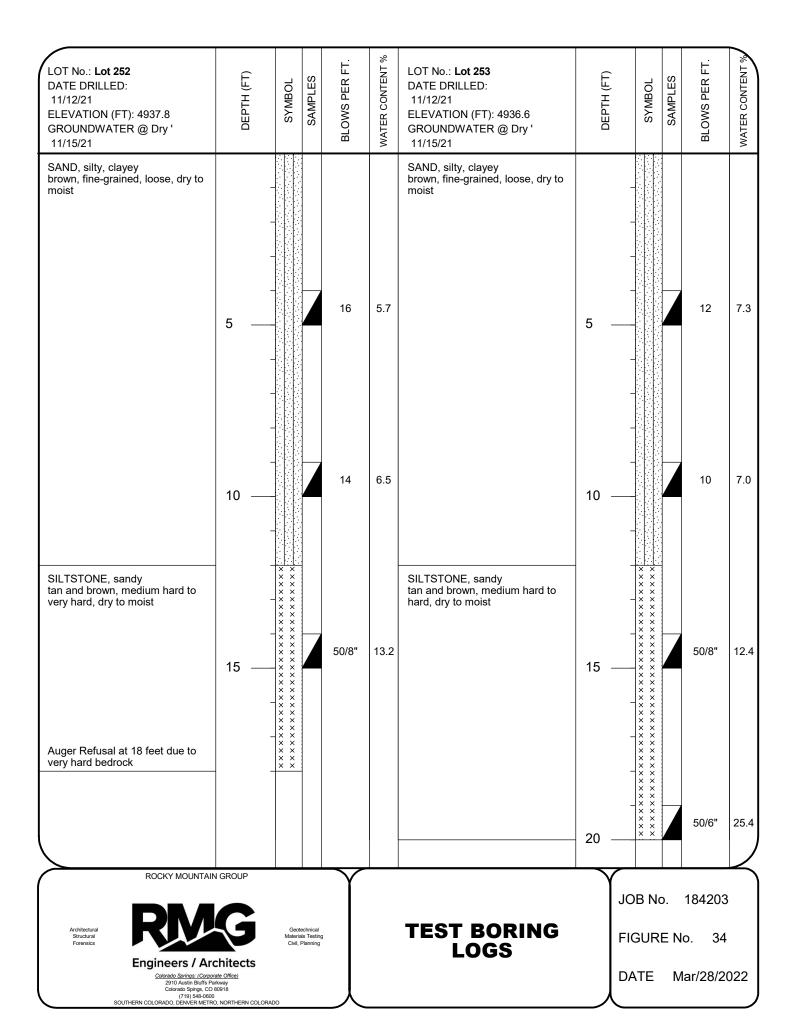
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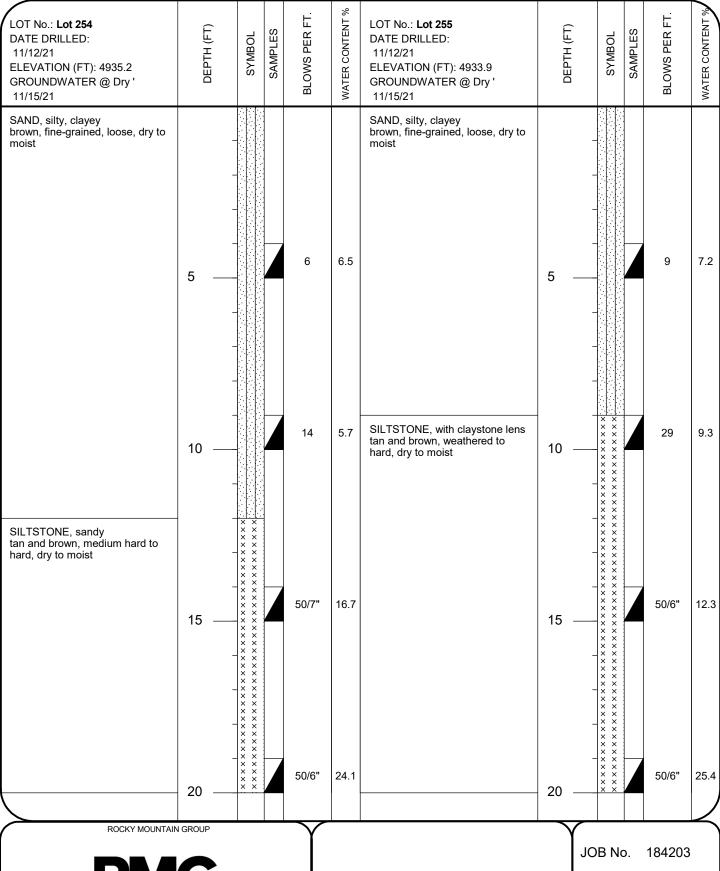
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FIGURE No. 32







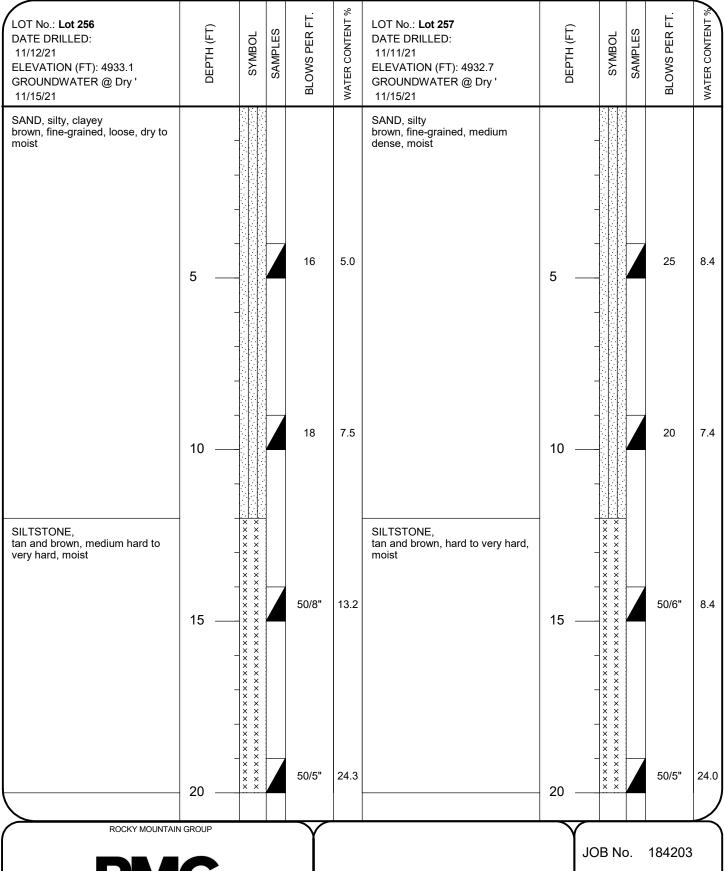


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FIGURE No. 35



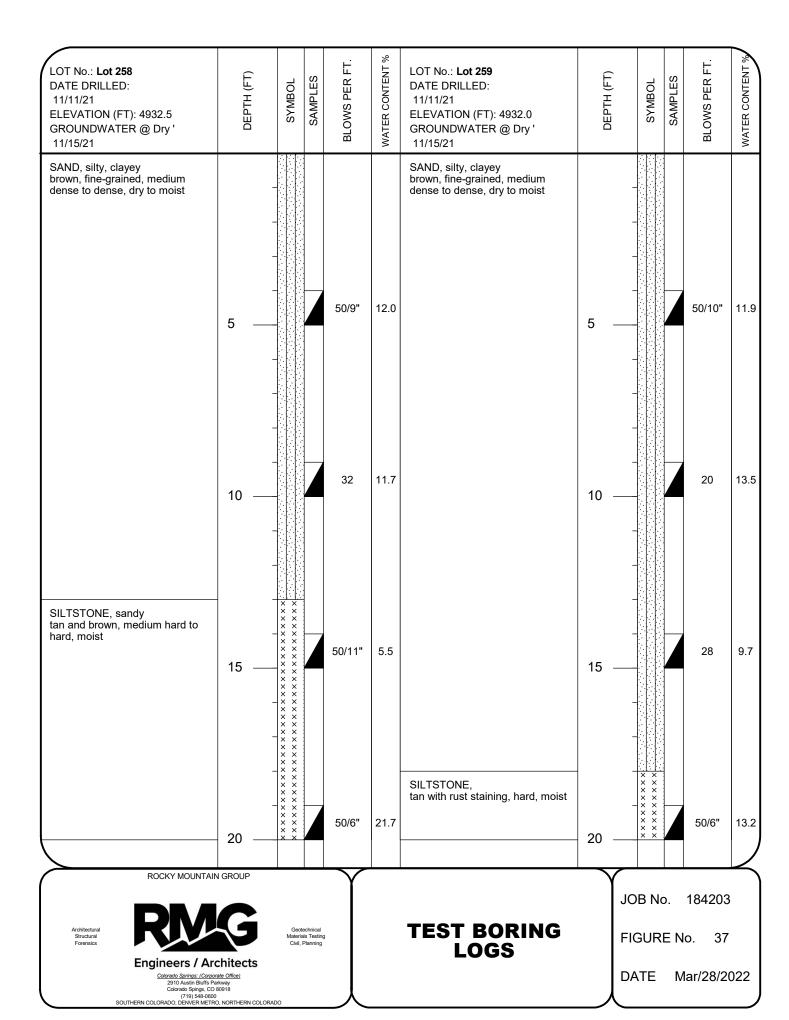


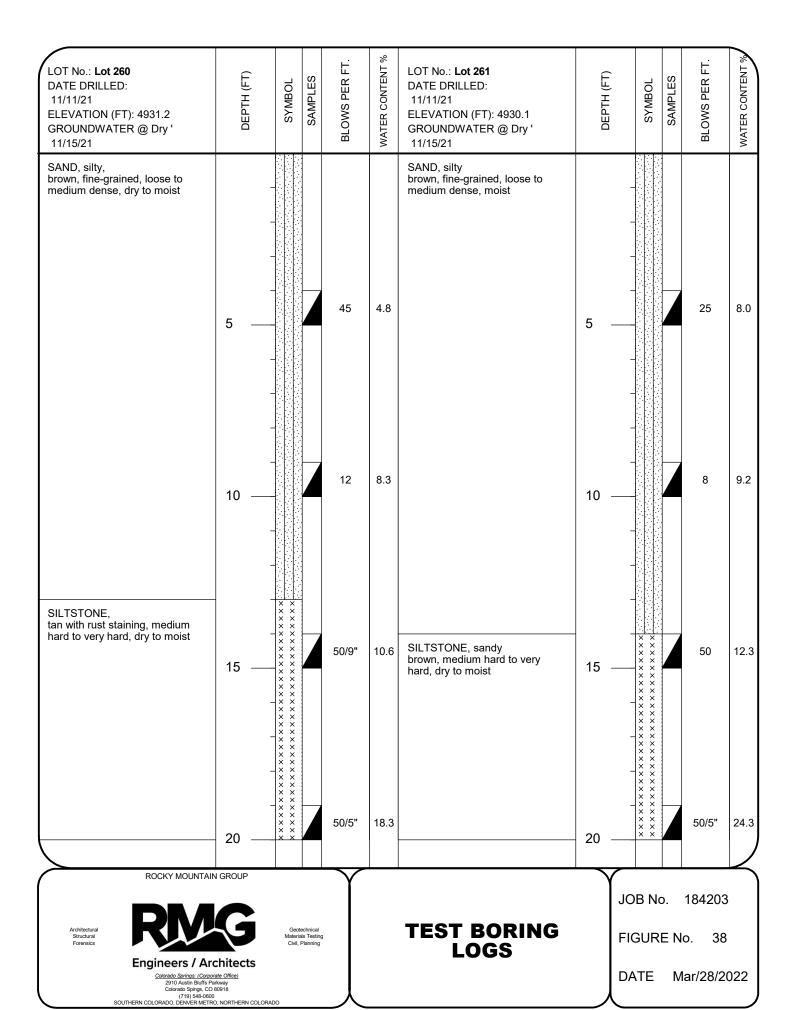
Engineers / Architects

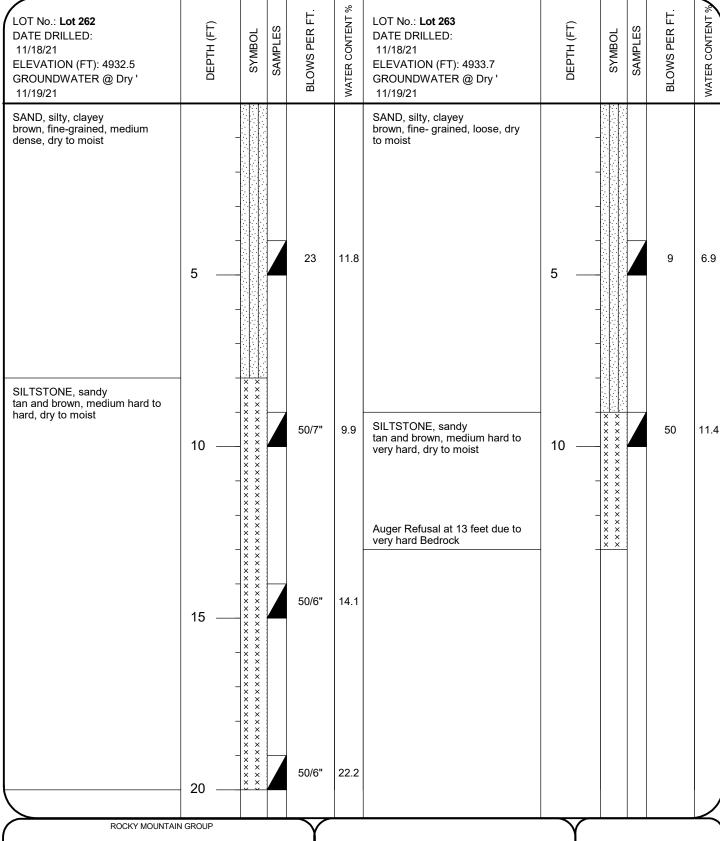
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FIGURE No. 36









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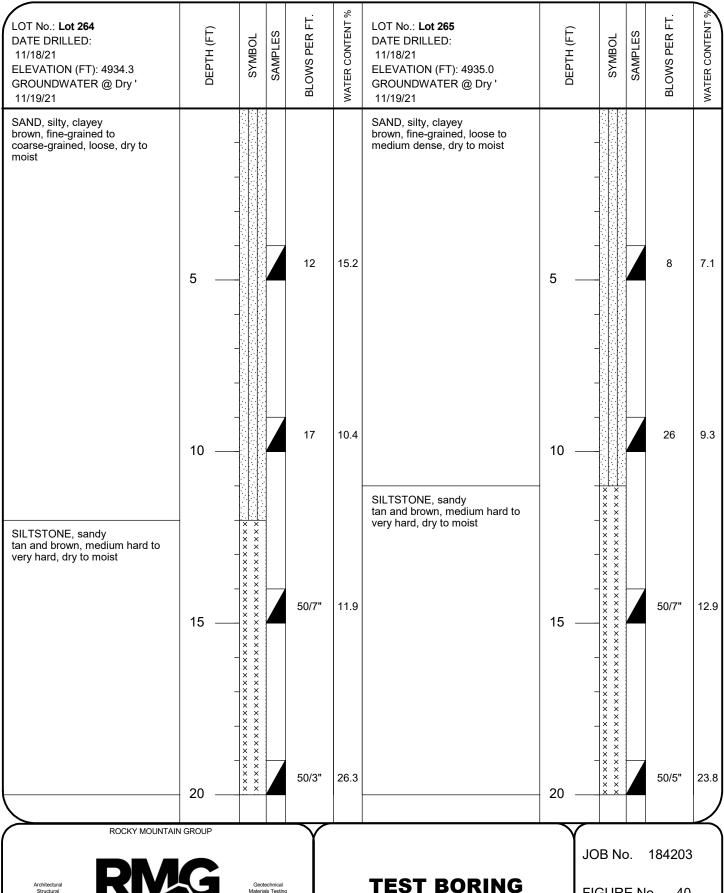
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JOB No. 184203

FIGURE No. 39

Mar/28/2022 DATE

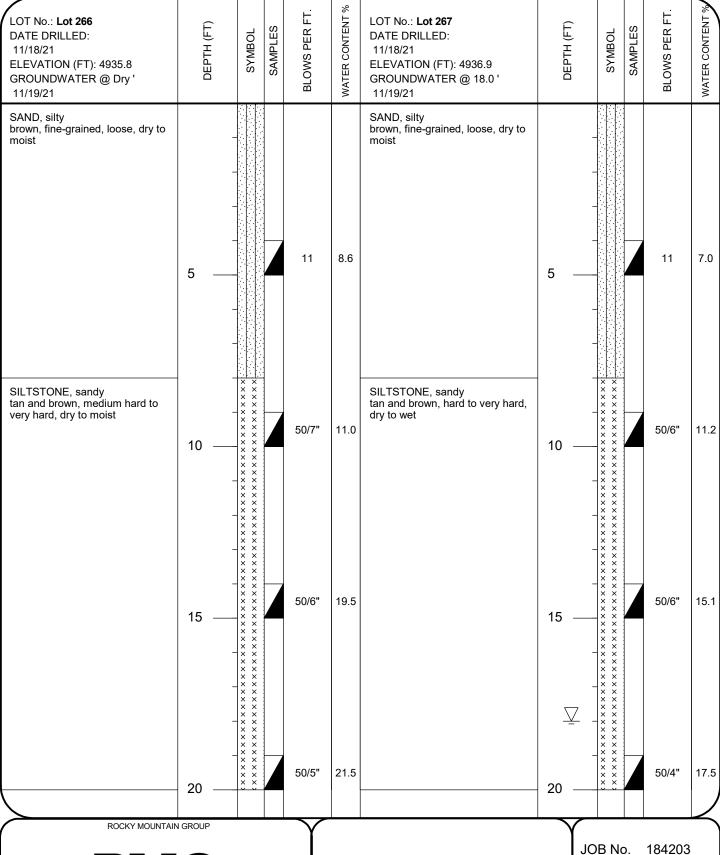


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LOGS

FIGURE No. 40



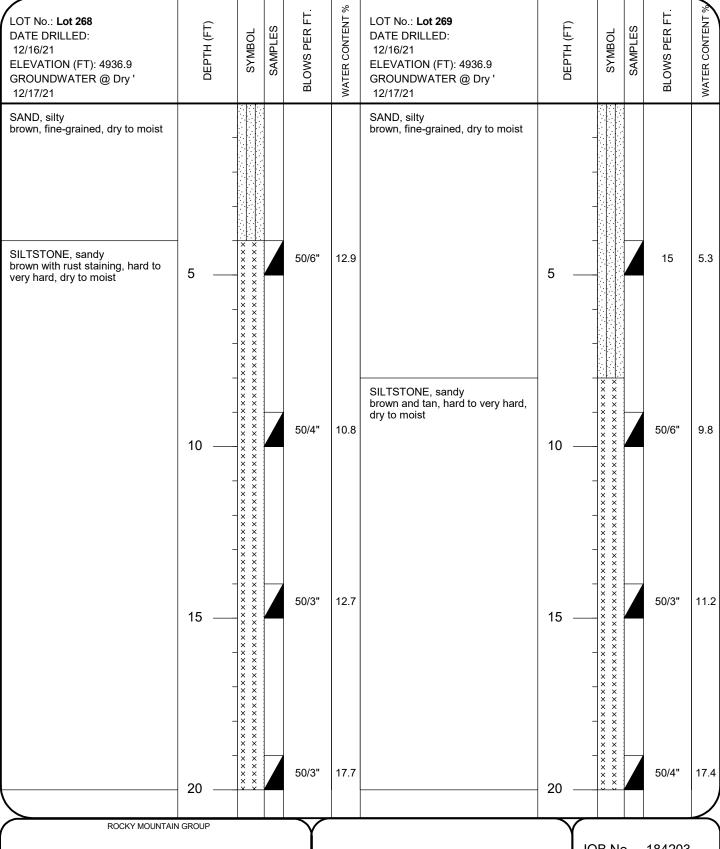


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FIGURE No. 41



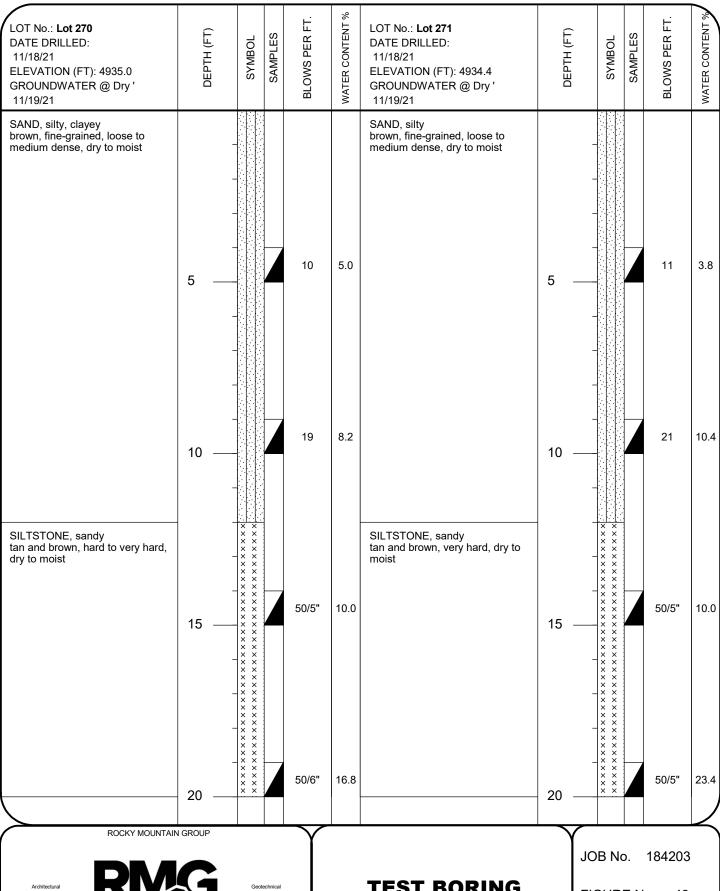


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JOB No. 184203

FIGURE No. 42





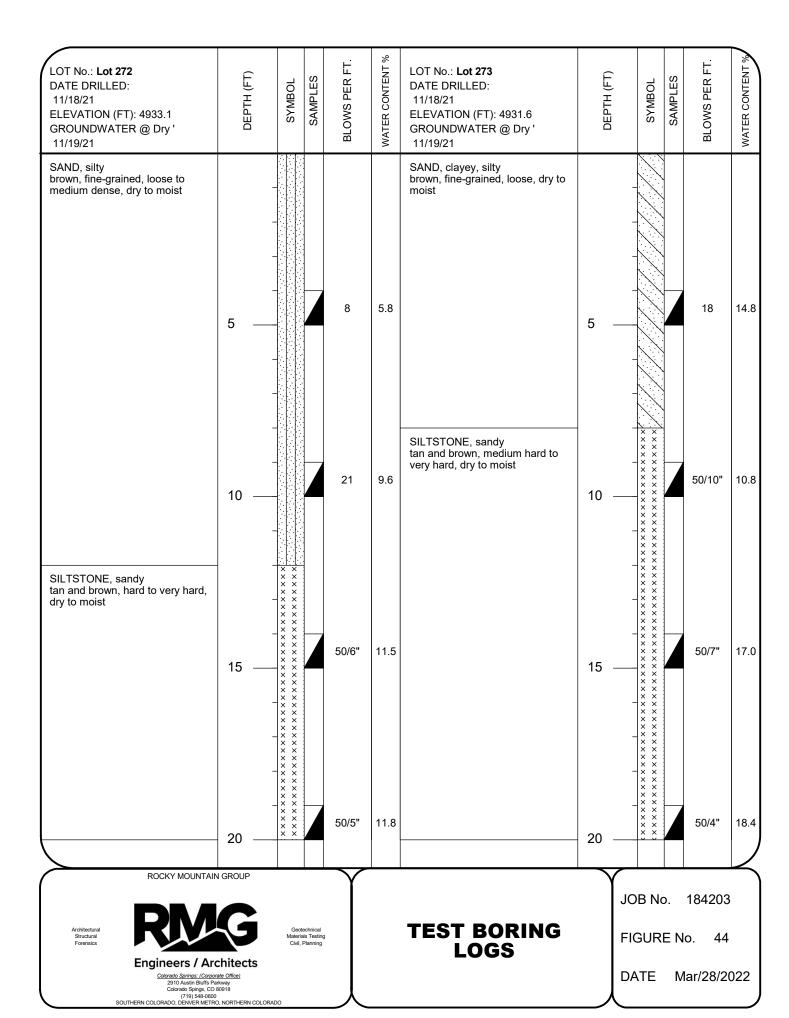
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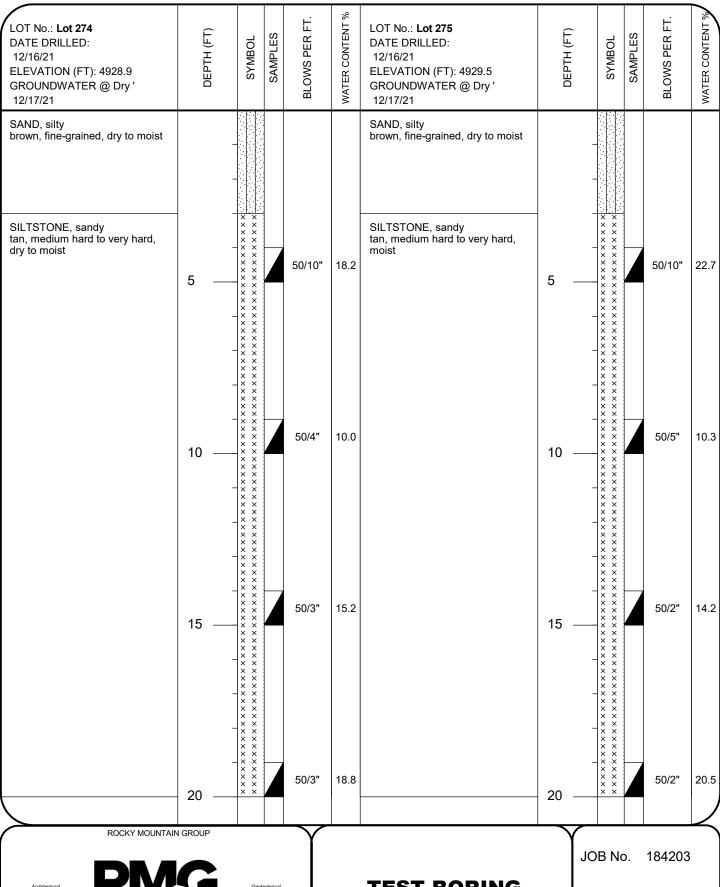
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FIGURE No. 43

Mar/28/2022 DATE







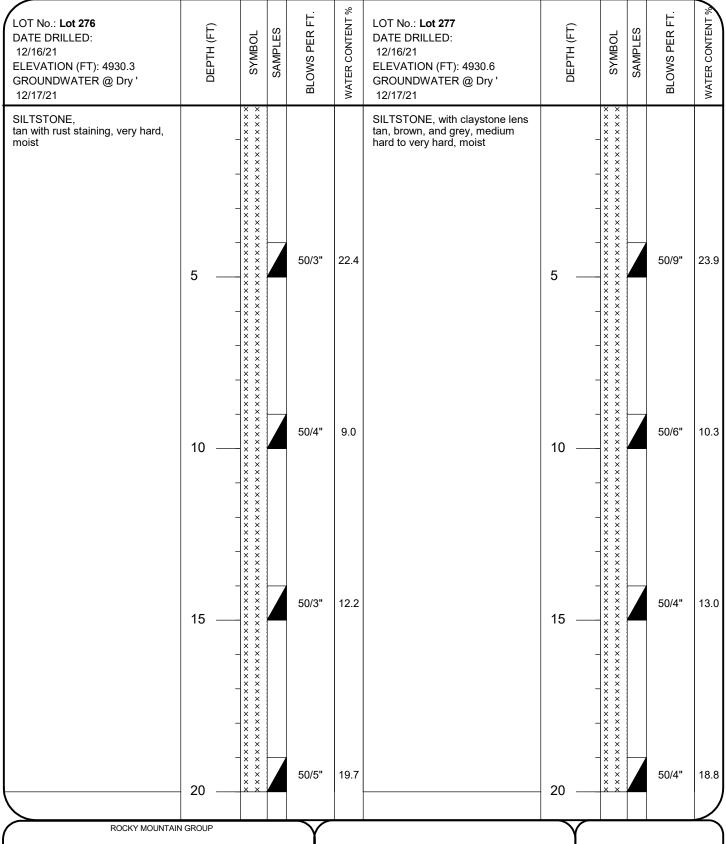
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FIGURE No. 45

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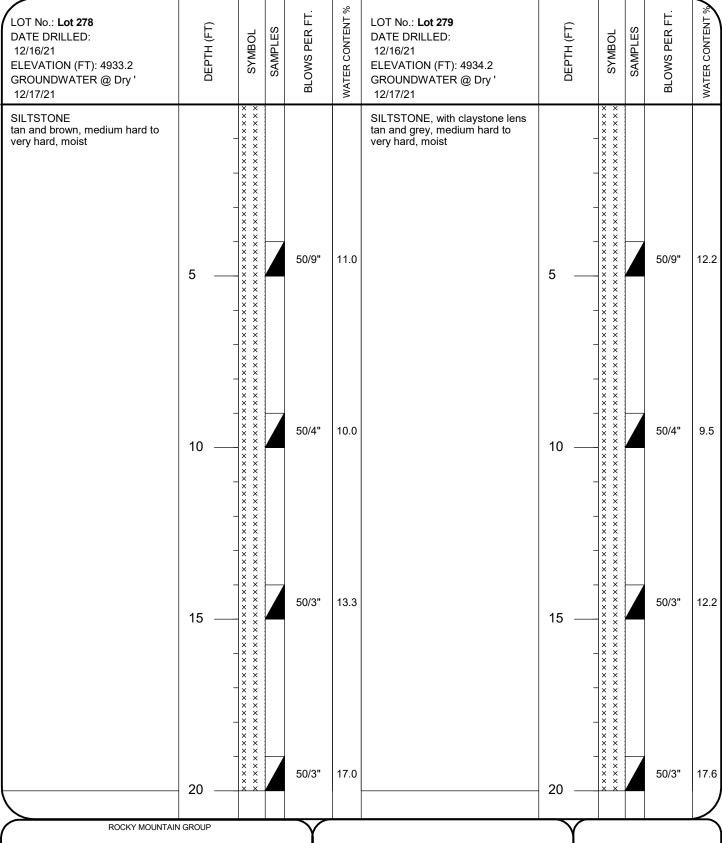




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FIGURE No. 46





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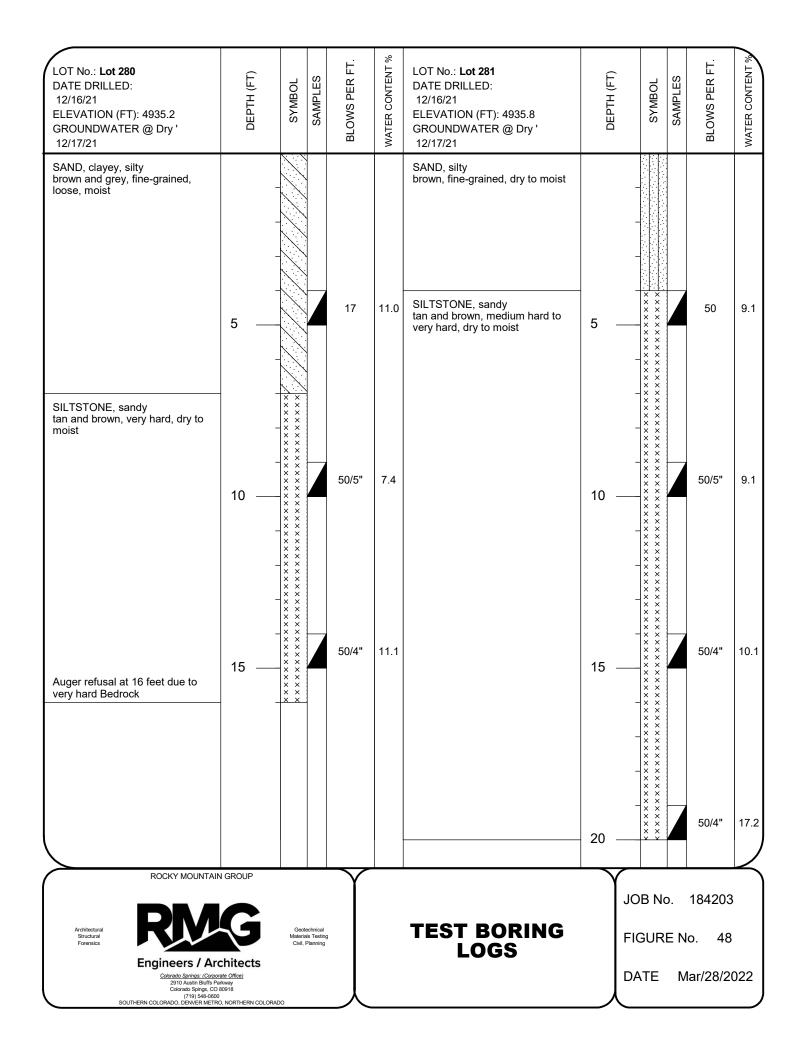
FIGURE No. 47

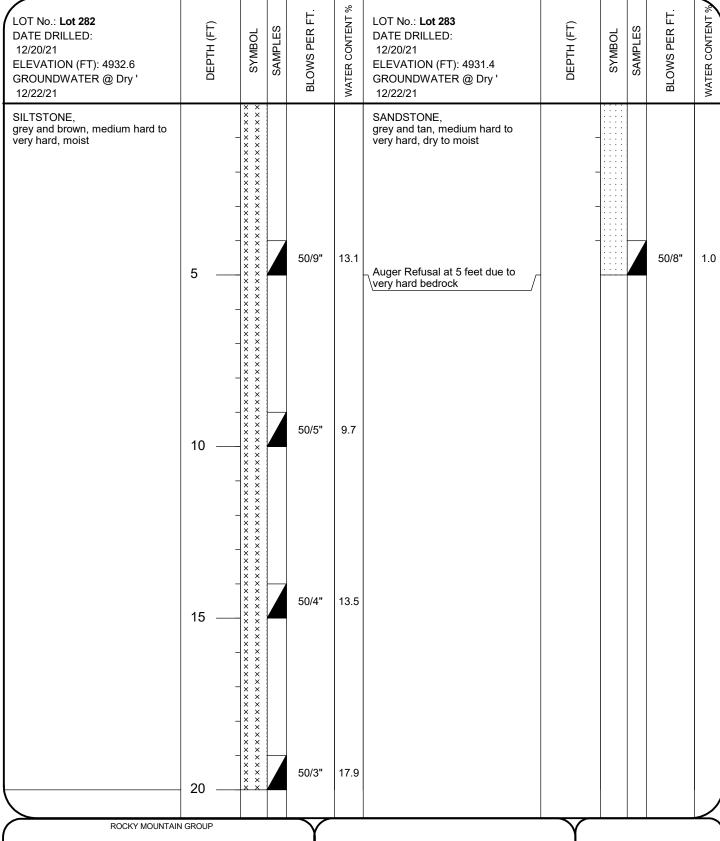
DATE Mar/28/2022

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Architectural Structural Forensics

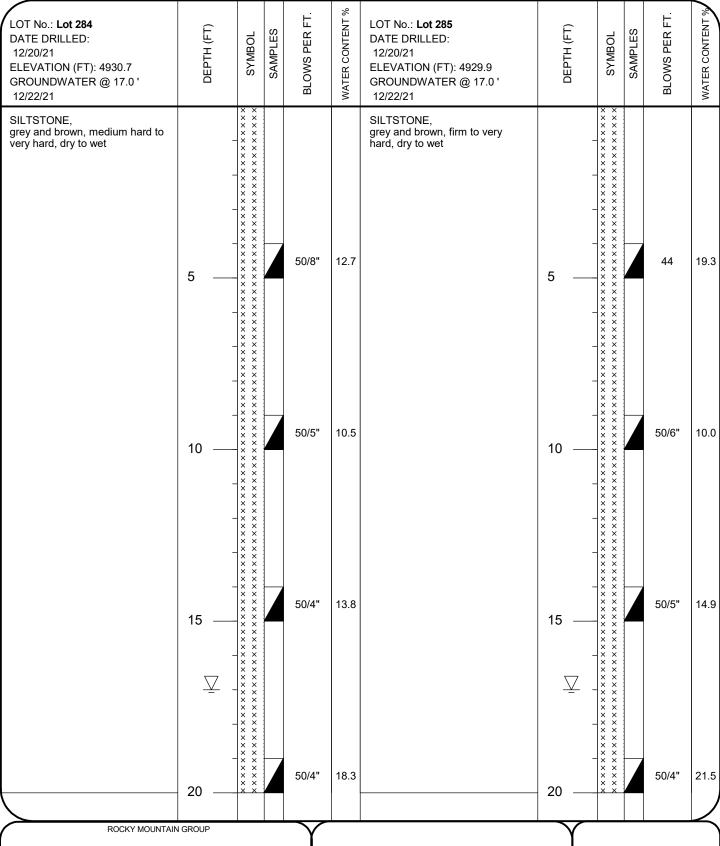


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FIGURE No. 49



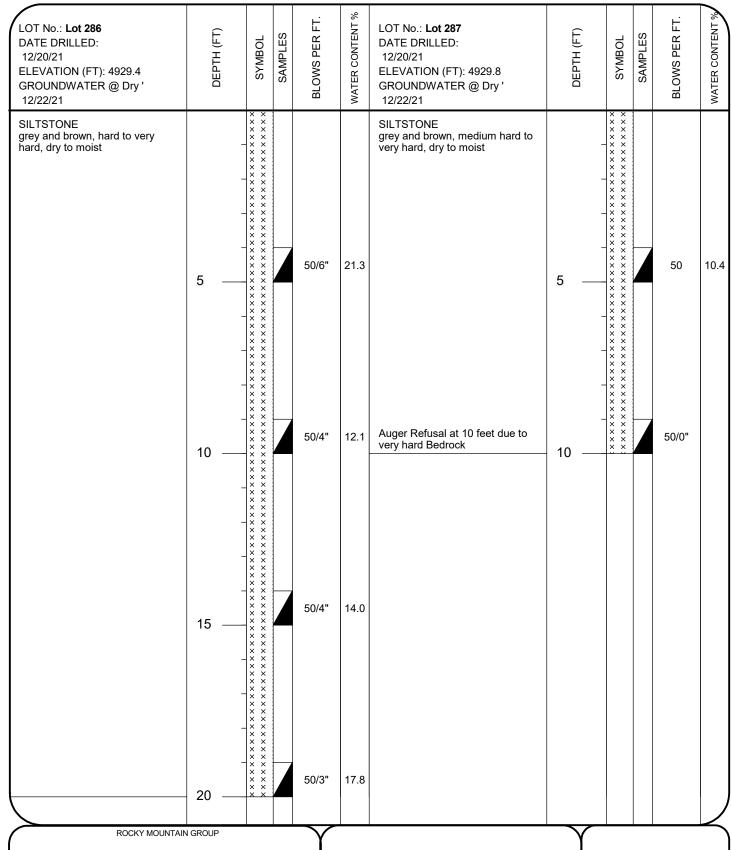


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FIGURE No. 50





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FIGURE No. 51

LOT No.: Lot 288 DATE DRILLED: 12/20/21 ELEVATION (FT): 4930.5 GROUNDWATER @ Dry' 12/22/21		SYMBOL	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 289 DATE DRILLED: 12/20/21 ELEVATION (FT): 4931.3 GROUNDWATER @ Dry ' 12/22/21	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE, with claystone lens brown, grey, and tan, firm to very hard, dry to moist	5	***************************************	42	19.8	SILTSTONE, with claystone lens brown, tan, and grey, medium hard to very hard, dry to moist	5 —	**************************************		50/9"	17.4
	10	× × × × × × × × × × × × × × × × × × ×	50/4"	11.4		10 —	× × × × × × × × × × × × × × × × × × ×		50/5"	12.8
	15	× × × × × ×	50/4"	12.9		- 15 —	× × × × × × × × × × × × × × × × × × ×		50/4"	14.6
	- 20 - ×	× × × × × × × × × × × × × × × × × × ×	50/5"	19.2	SANDSTONE, Light grey to grey, very hard, dry to moist	20 —	× × × × × × × × × × × × × × × × × × ×		50/0"	1.3

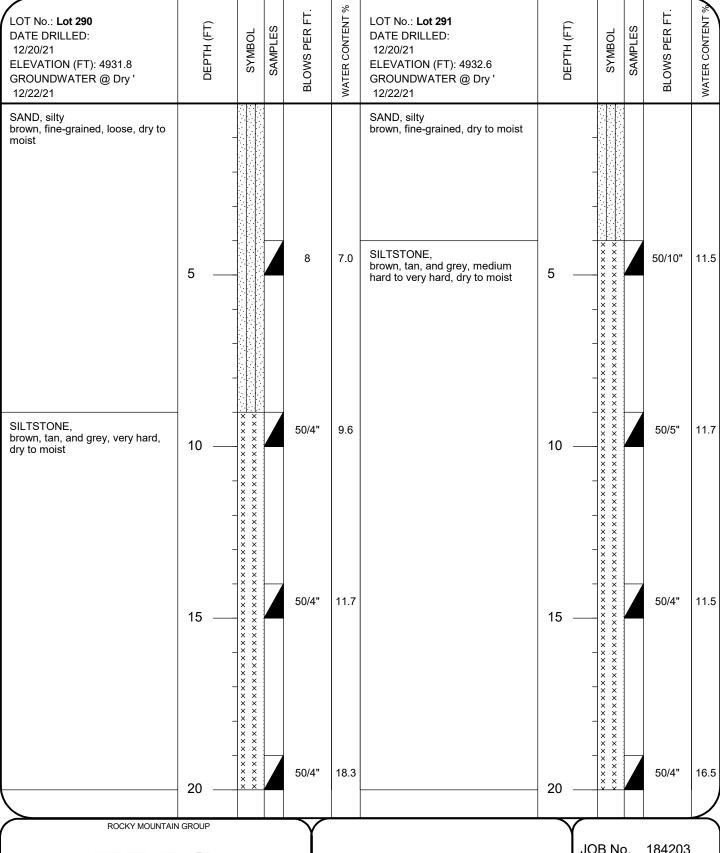


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FIGURE No. 52





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FIGURE No. 53

LOT No.: Lot 292 DATE DRILLED: 12/20/21 ELEVATION (FT): 4933.0 GROUNDWATER @ Dry ' 12/22/21	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 293 DATE DRILLED: 12/20/21 ELEVATION (FT): 4933.2 GROUNDWATER @ Dry ' 12/22/21	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SAND, silty brown, fine-grained, dry to moist	-					SAND, silty brown, fine-grained, dry to moist	-				
SILTSTONE, brown, tan, and grey, firm to very hard, dry to moist	5	× × × × × × × × × × × × × × × × × × ×		38	13.1	SILTSTONE, brown and tan, medium hard to very hard, dry to moist	5 —	× × × × × × × × × × × × × × × × × × ×		50/11"	15.8
	10	××××××××××××××××××××××××××××××××××××××		50/5"	12.6		10 —	^		50/5"	9.4
SANDSTONE, light grey, very hard, dry to moist Auger Refusal at 16 feet due to very hard bedrock	15	× ×		50/0"		SANDSTONE, light grey, very hard, dry to moist Auger Refusal at 16 feet due to very hard bedrock	15 —	× ×		50/4"	12.2
ROCKY MOUNTAIN	N GROUP			$\overline{\Upsilon}$			Y	ND NI		184203	

Architectural Structural



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FIGURE No. 54

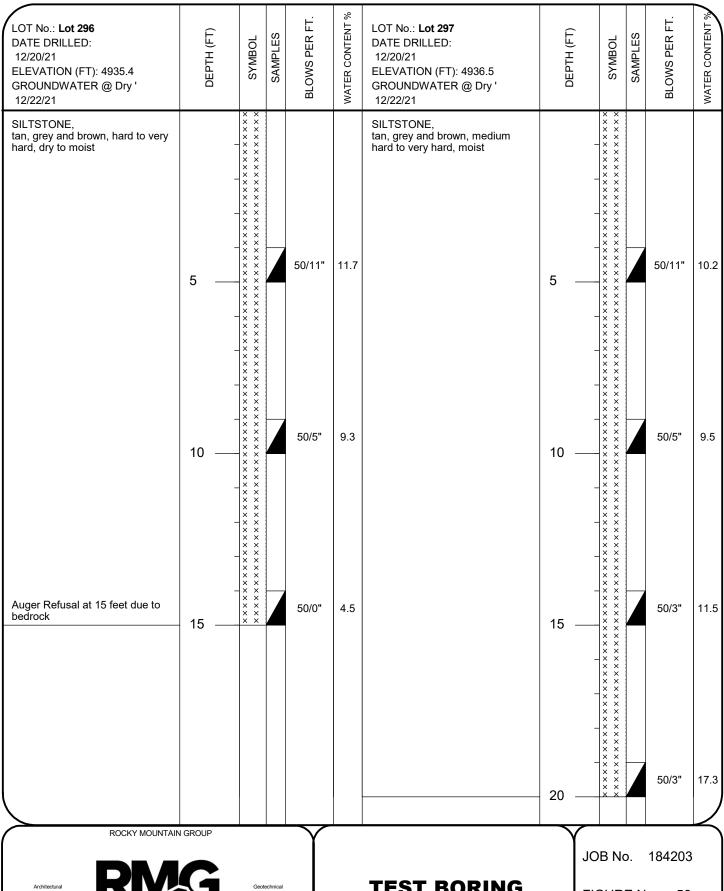
LOT No.: Lot 294 DATE DRILLED: 12/20/21 ELEVATION (FT): 4933.8 GROUNDWATER @ Dry ' 12/22/21	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 295 DATE DRILLED: 12/20/21 ELEVATION (FT): 4934.8 GROUNDWATER @ Dry ' 12/22/21	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE, tan, grey and brown, medium hard to very hard, dry to moist	5 —	××××××××××××××××××××××××××××××××××××××		50/8"	12.9	SILTSTONE, tan, grey and brown, medium hard to very hard, dry to moist	5 —	- x x x x x x x x x x x x x x x x x x x		50/8"	9.2
	10 —	× × × × × × × × × × × × × × × × × × ×		50/4"	10.8		10 —	- × × × × × × × × × × × × × × × × × × ×		50/4"	8.9
SANDSTONE, light grey, very hard, dry to moist Auger Refusal at 16 feet due to very hard bedrock	15 —	\$\hat{x} \times		50/5"	13.5	SANDSTONE, light grey, very hard, dry to moist Auger Refusal at 15 feet due to very hard bedrock	15 —	× × × × × × × × × × × × × × × × × × ×		50/5"	13.2
ROCKY MOUNTAIN	N GROUP			$\overline{\Upsilon}$			Y	IOB No		184203	



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TEST BORING LOGS

FIGURE No. 55

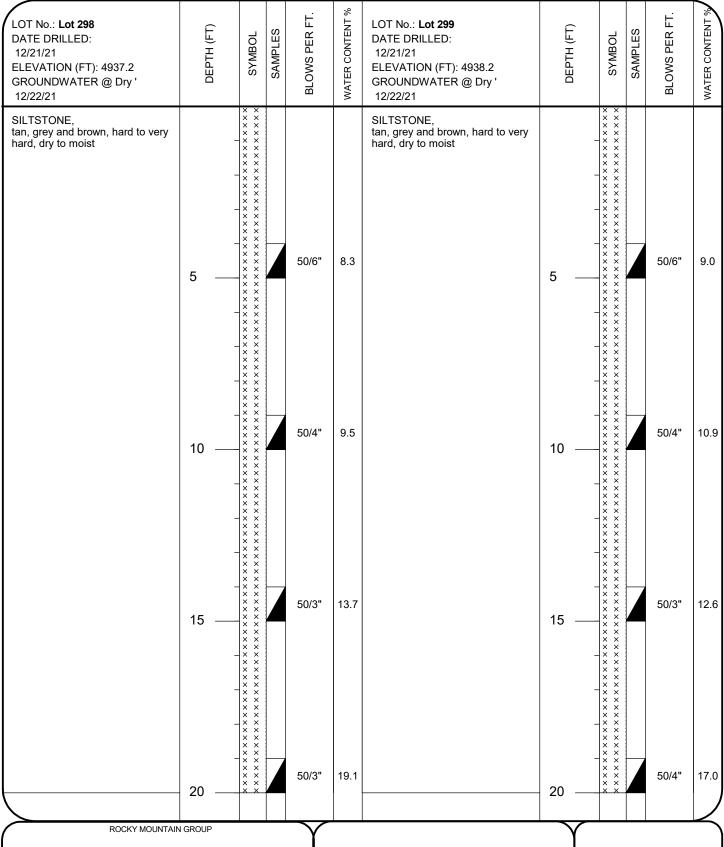




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SOUTHERN COLORADO, DENVER METRO, NORTHERN COLORADO **TEST BORING LOGS**

FIGURE No. 56

Mar/28/2022 DATE



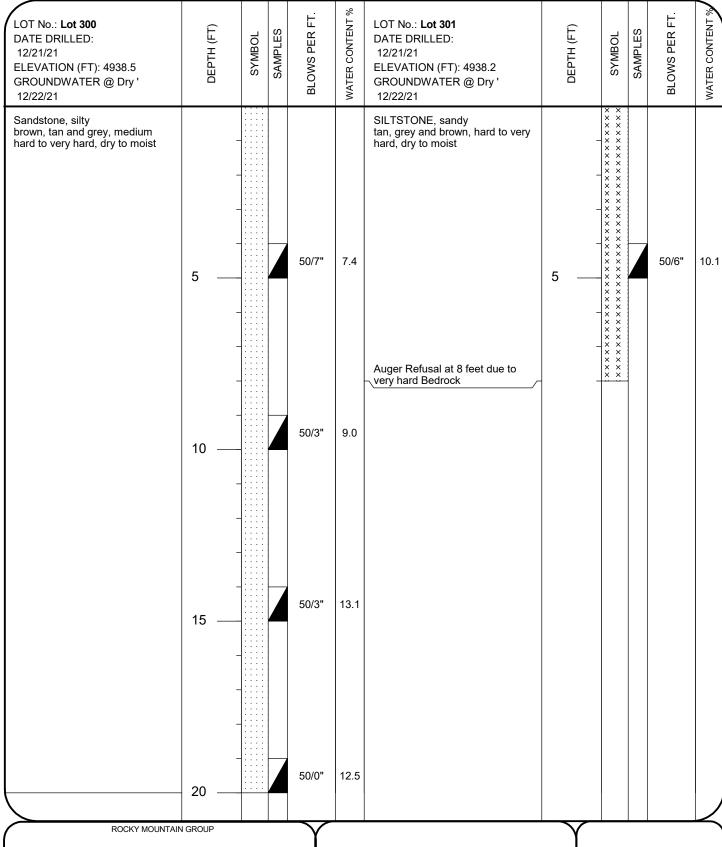


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TEST BORING LOGS JOB No. 184203

FIGURE No. 57





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FIGURE No. 58

LOT No.: Lot 302 DATE DRILLED: 12/21/21 ELEVATION (FT): 4937.6 GROUNDWATER @ 17.0 ' 12/22/21	DЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 303 DATE DRILLED: 12/21/21 ELEVATION (FT): 4937.0 GROUNDWATER @ Dry' 12/22/21	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE, sandy tan, grey and brown, hard to very hard, dry to wet	5 —	× × × × × × × × × × × × × × × × × × ×		50/6"	9.1	SILTSTONE, sandy tan, grey and brown, very hard, dry to moist	5 —	**************************************	Z	50/5"	8.8
	10 —	× × × × × × × × × × × × × × × × × × ×		50/4"	11.7		10 —	× × × × × × × × × × × × × × × × × × ×		50/5"	13.8
	15 —	××××××××××××××××××××××××××××××××××××××		50/3"	19.7	Auger Refusal at 13 feet due to very hard Bedrock		× × × × × × × × × × × × × × × × × × ×			
ROCKY MOUNTAIN	20 —	× × × × × × × × × × × × × × × × × × ×		50/4"	18.8						

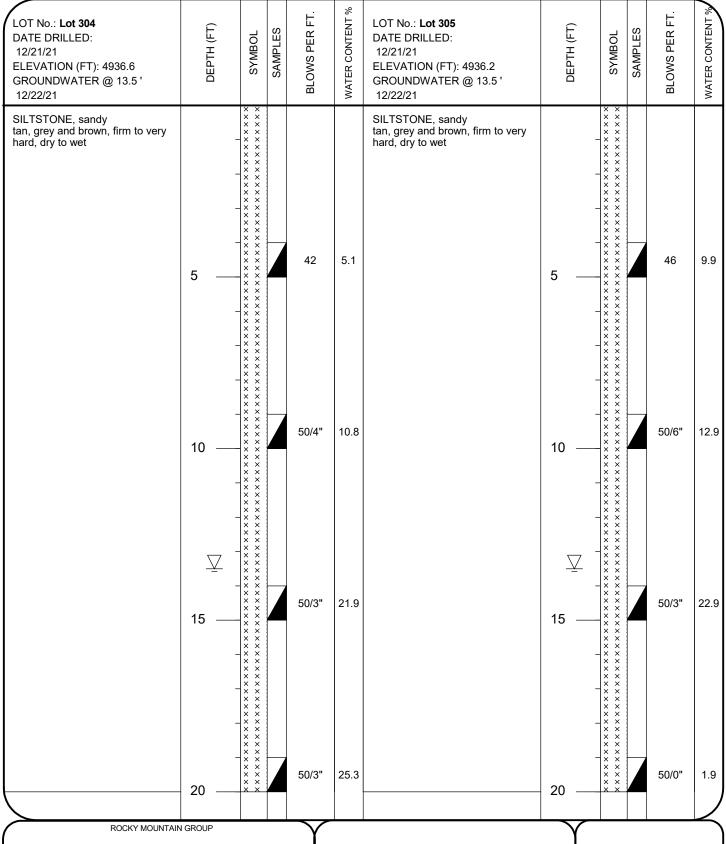


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TEST BORING LOGS JOB No. 184203

FIGURE No. 59

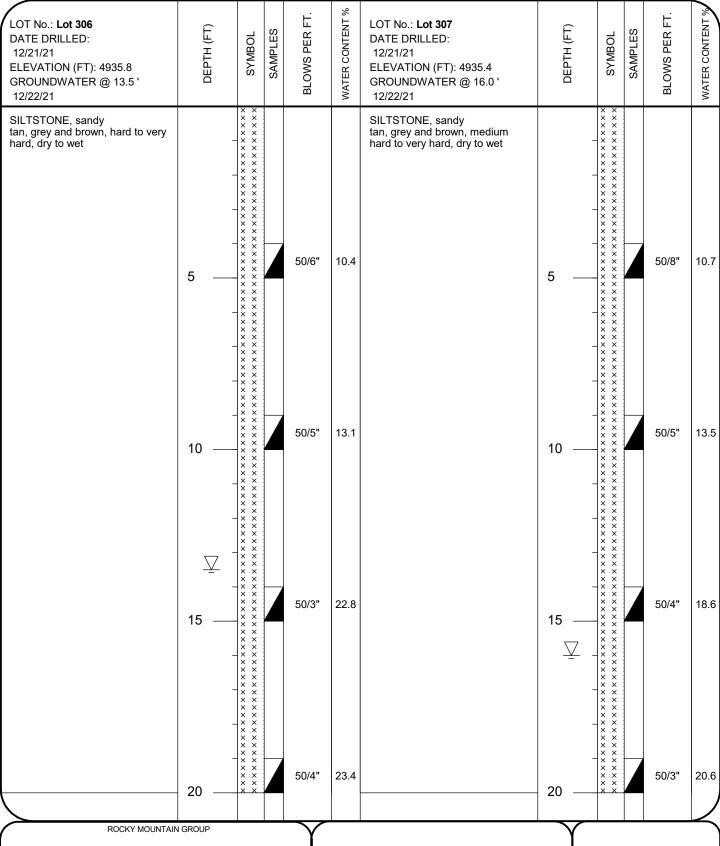




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Geotechnical Materials Testin Civil, Planning TEST BORING LOGS JOB No. 184203

FIGURE No. 60



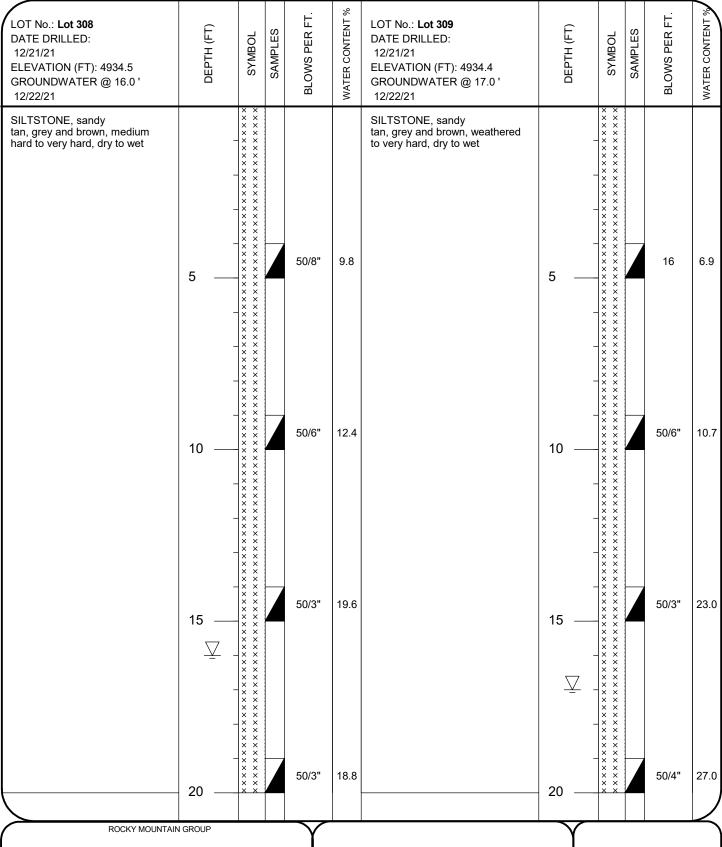


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TEST BORING LOGS JOB No. 184203

FIGURE No. 61





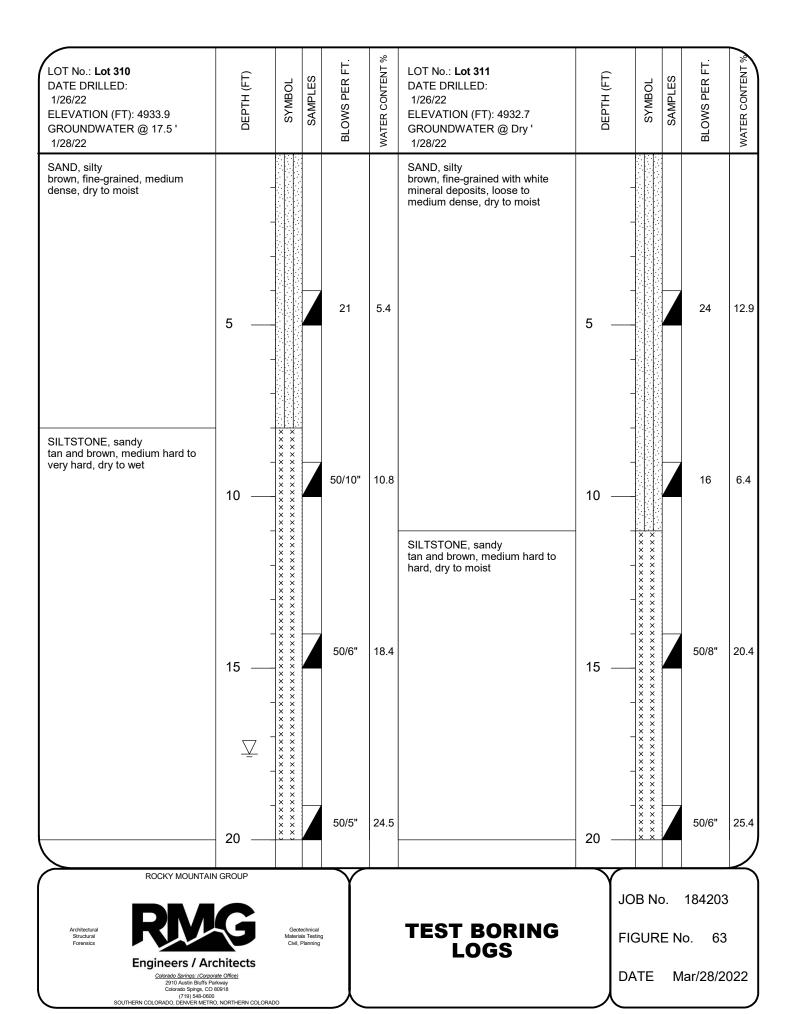
Engineers / Architects

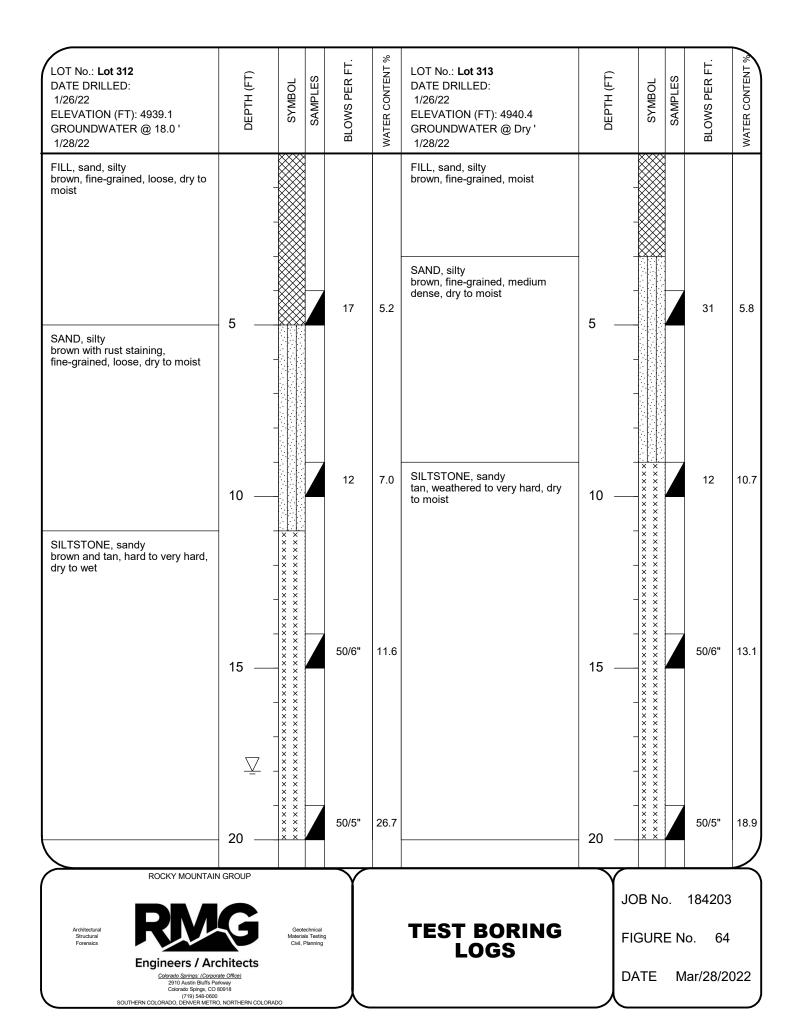
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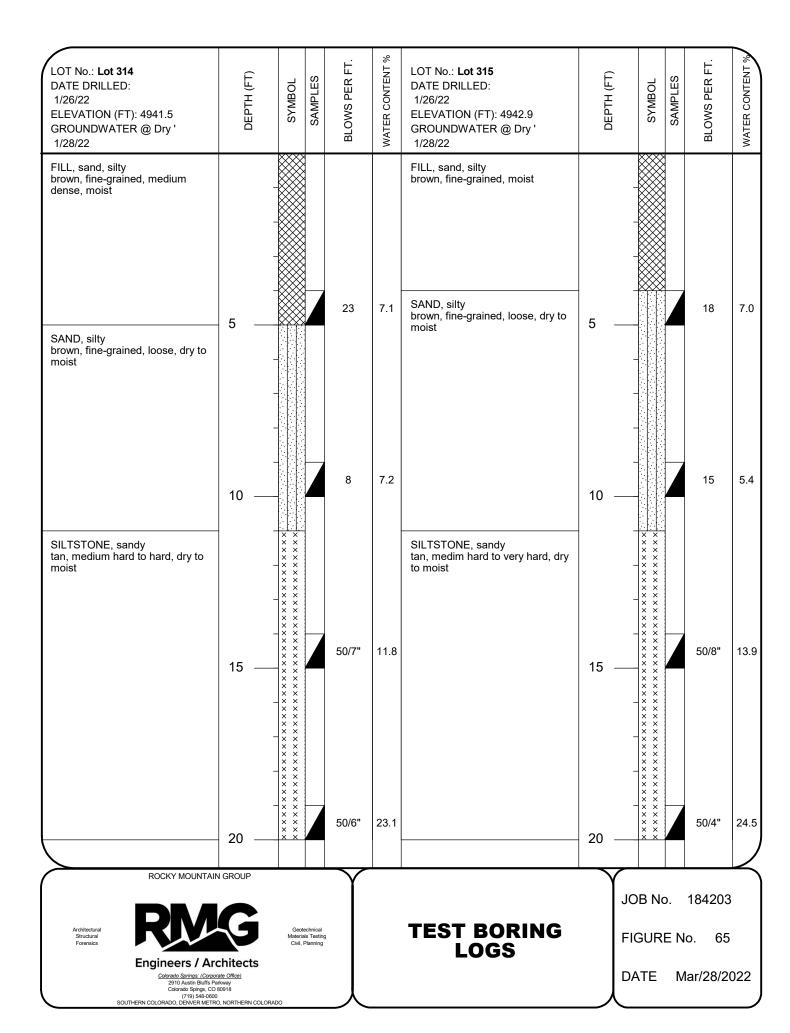
TEST BORING LOGS

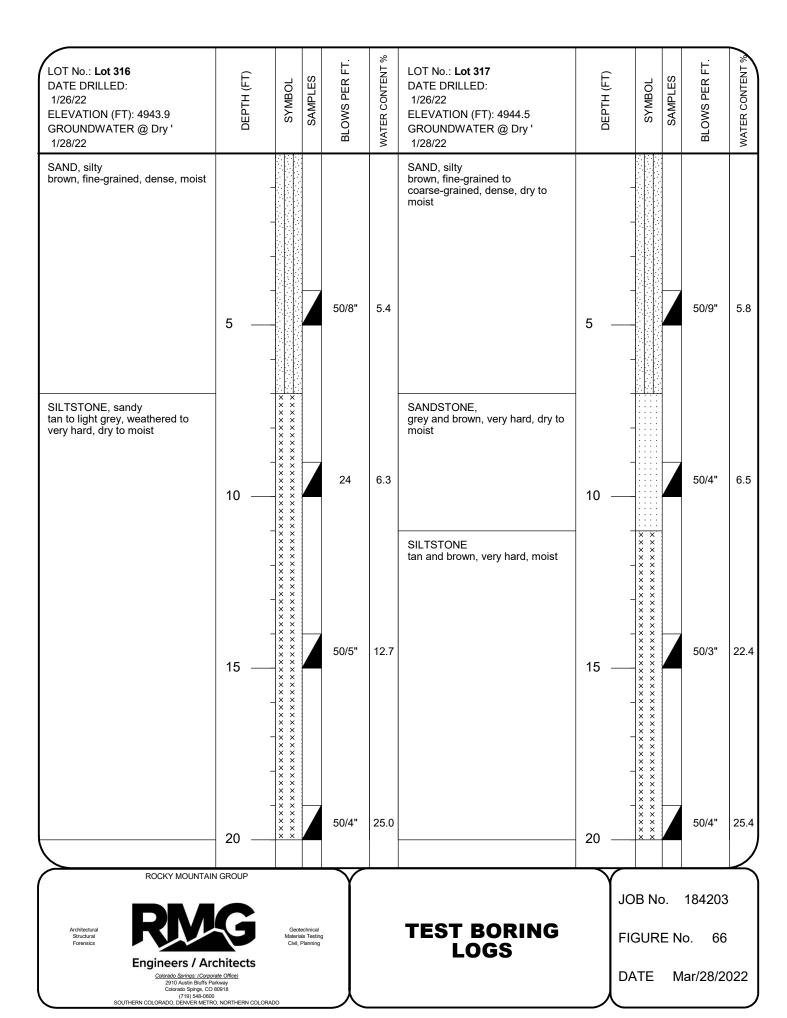
JOB No. 184203

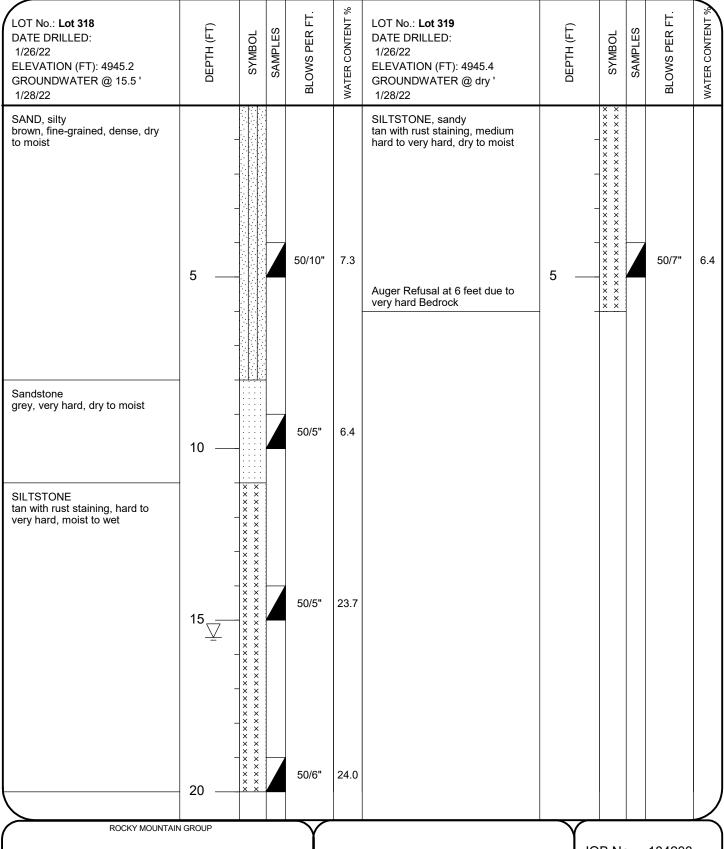
FIGURE No. 62









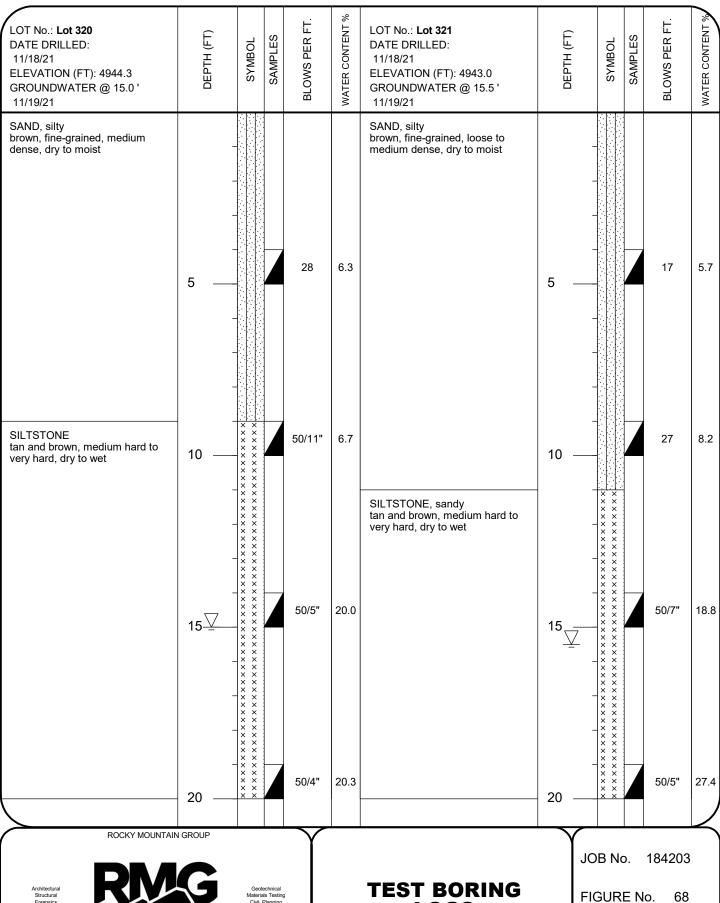




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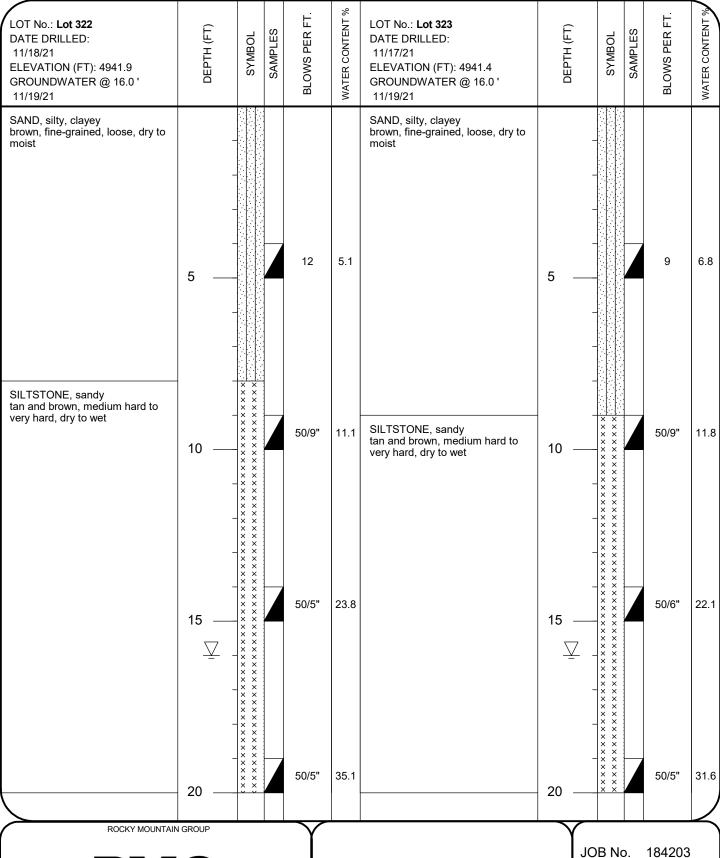
Materials Testing Civil, Planning TEST BORING LOGS JOB No. 184203

FIGURE No. 67



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Mar/28/2022 DATE



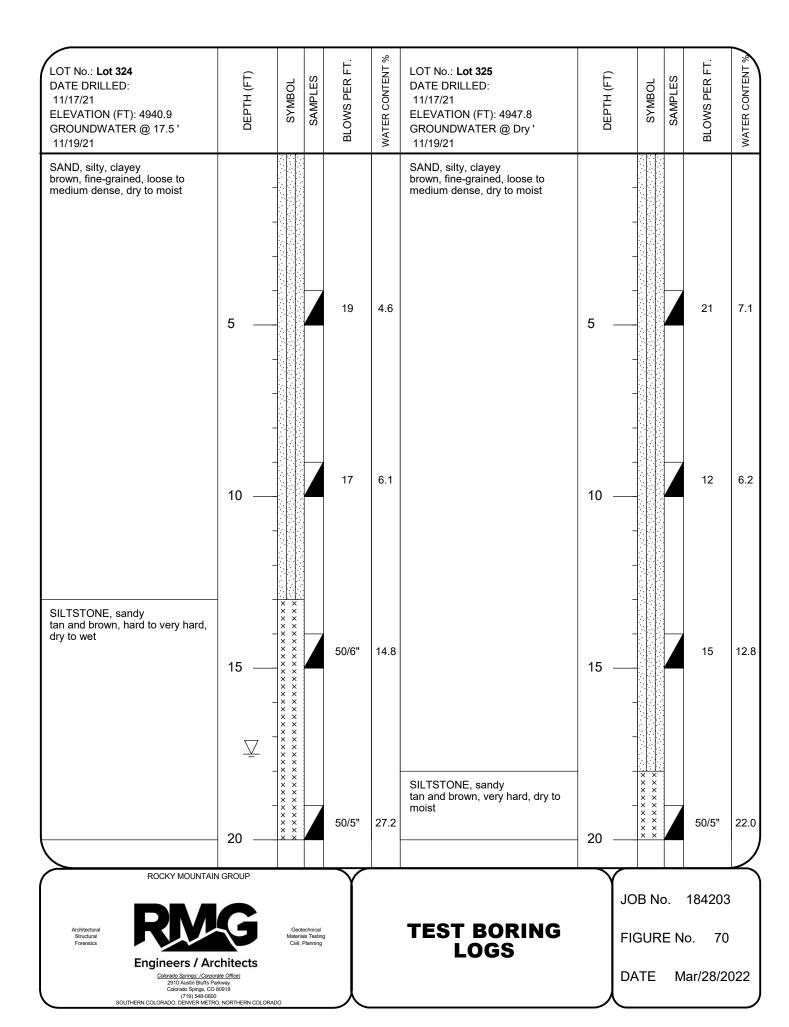


TEST BORING LOGS

FIGURE No. 69

DATE Mar/28/2022

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LOT No.: Lot 326 DATE DRILLED: 11/17/21 ELEVATION (FT): 4948.1 GROUNDWATER @ Dry ' 11/19/21	ОЕРТН (FT)	SYMBOL	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 327 DATE DRILLED: 11/17/21 ELEVATION (FT): 4948.1 GROUNDWATER @ 17.8 ' 11/19/21	БЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SAND, silty, clayey brown, fine-grained to medium-grained, loose to medium dense, dry to moist	-				SAND, silty brown, fine-grained to medium-grained, loose to medium dense, dry to wet	-				
	5 —		32	9.7		5 —			33	11.3
	10 —		15	9.3		10 —			18	14.7
	15 —		34	14.1		15 —			28	17.8
SILTSTONE, sandy tan and brown, hard, moist	_ 20	× × × × × × × × × × × × × × × × × × ×	50/6"	22.6	SILTSTONE, tan and brown, very hard, moist to wet	20 —	× × × × × × × × × × × × × × × × × × ×		50/5"	25.1

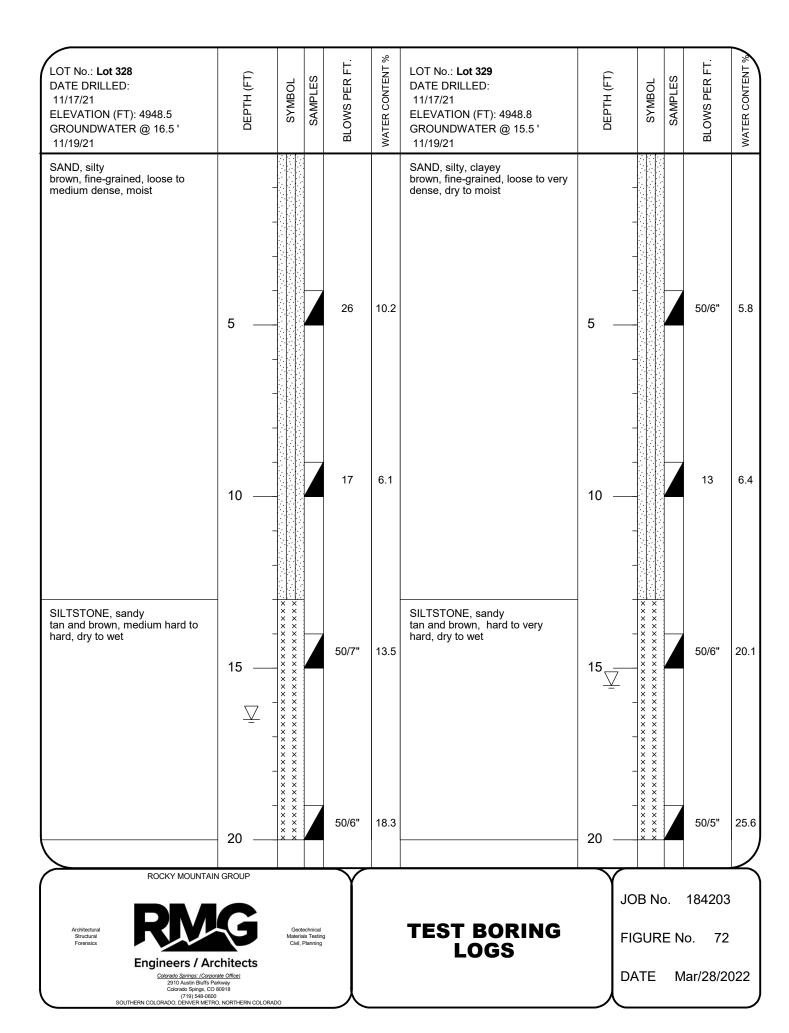


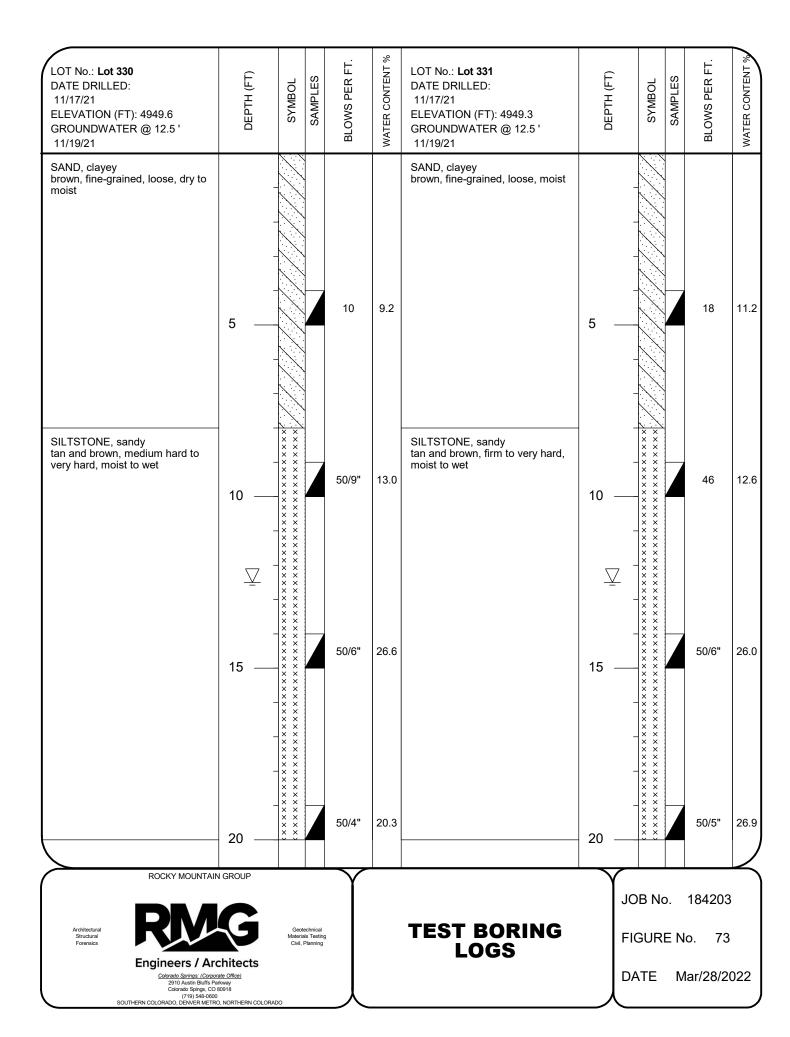
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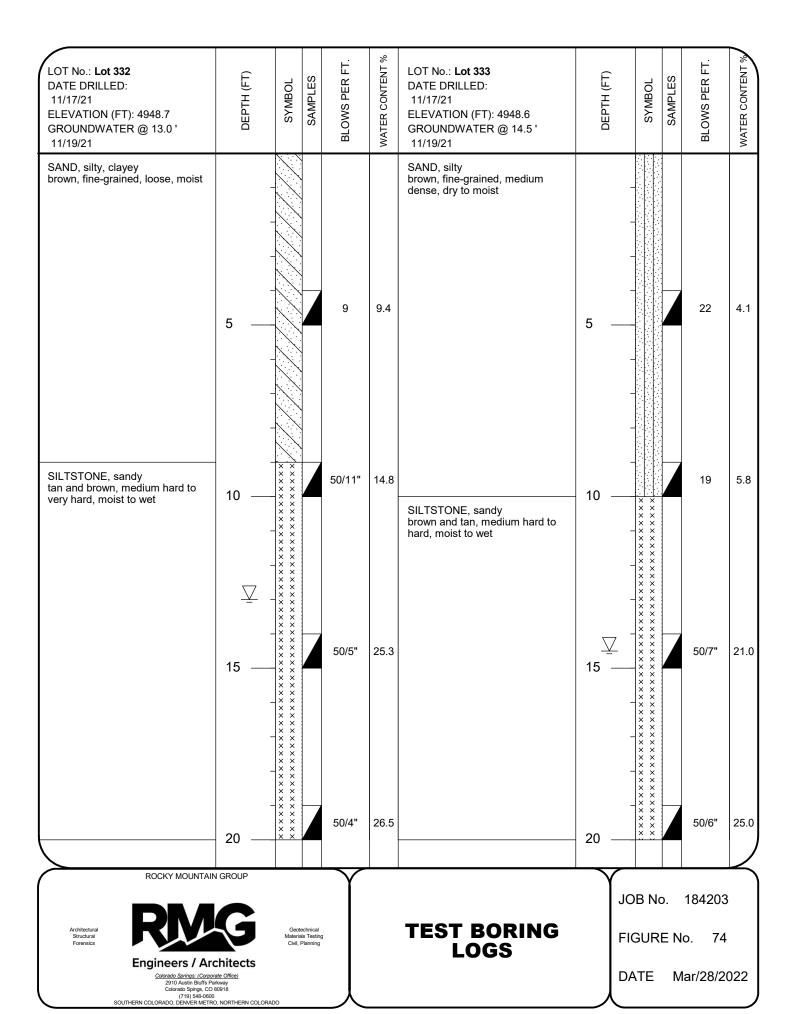
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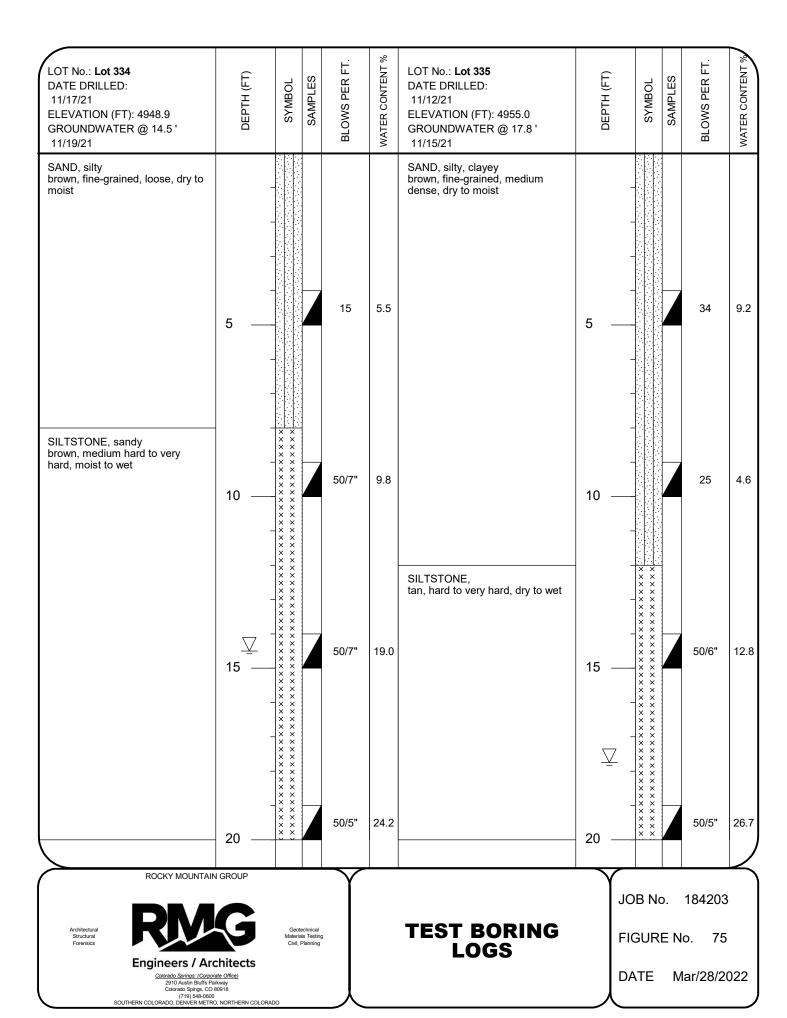
TEST BORING LOGS

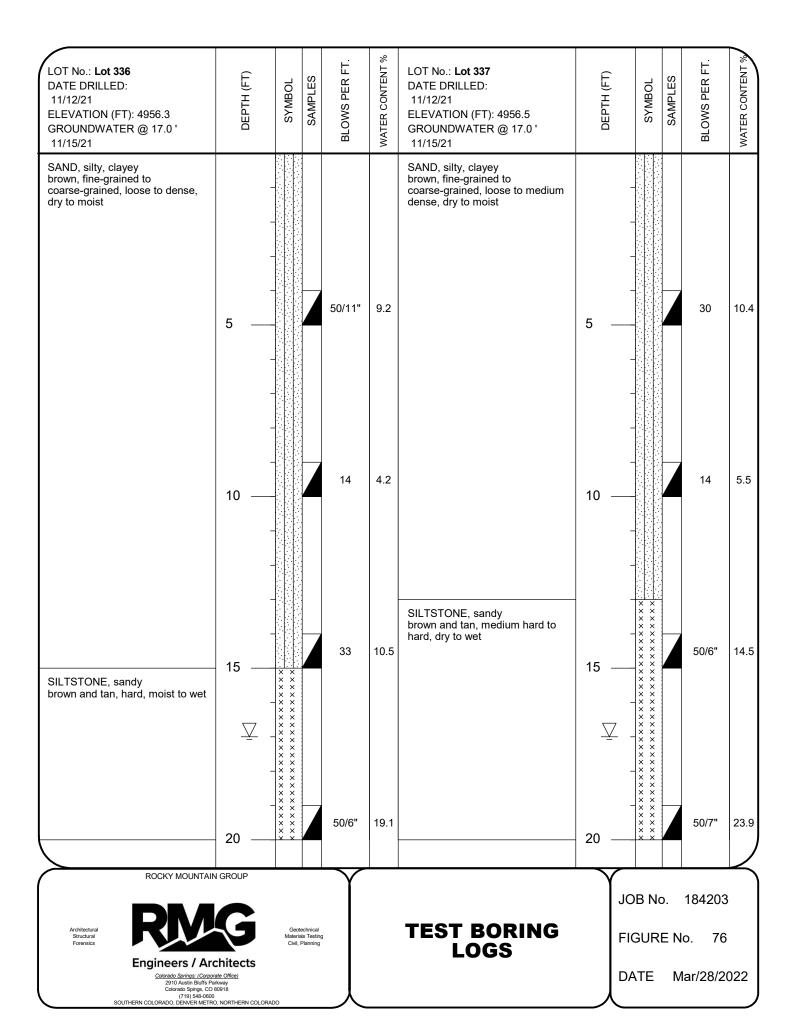
FIGURE No. 71

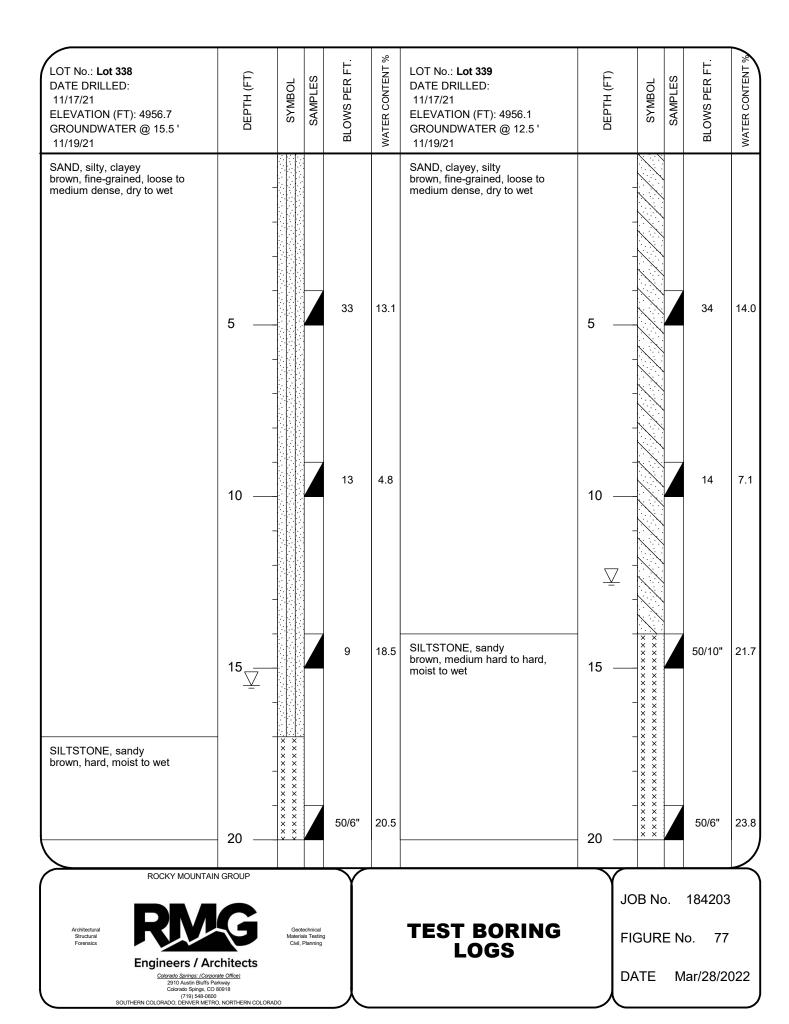


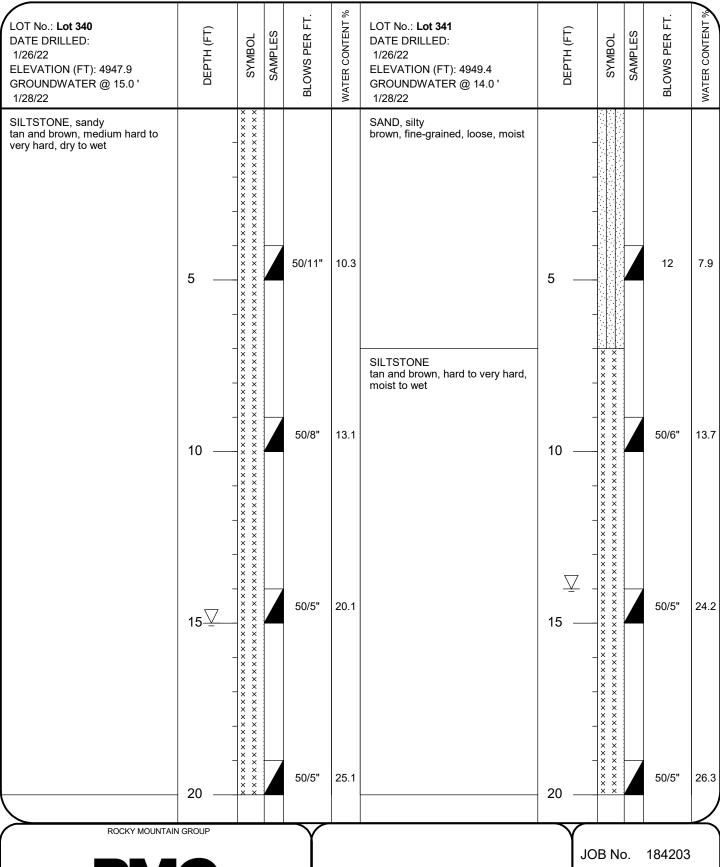














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Geotechnical Materials Testing Civil, Planning TEST BORING LOGS

FIGURE No. 78

LOT No.: Lot 342 DATE DRILLED: 1/27/22 ELEVATION (FT): 4950.8 GROUNDWATER @ 13.0 ' 1/28/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 343 DATE DRILLED: 1/27/22 ELEVATION (FT): 4952.9 GROUNDWATER @ 12.0 ' 1/28/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SILTSTONE tan and brown, medium hard to very hard, moist to wet	5	××××××××××××××××××××××××××××××××××××××		50/7"	12.1	SILTSTONE tan and brown, hard to very hard, dry to moist	5 —	**************************************		50/6"	7.0
	10 —	* * * * * * * * * * * * * * * * * * *		50/5"	20.0		10 — 	× ×		50/5"	17.4
	15 —	× × × × × × × × × × × × × × × × × × ×		50/5"	17.6	Auger Refusal at 15 feet due to very hard Bedrock	-	× × × × × × × × × × × × × × × × × × ×		50/0"	
ROCKY MOUNTAIL	- 20	× × × × × × × × × × × × × × × × × × ×		50/6"	22.3						



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TEST BORING LOGS

JOB No. 184203

FIGURE No. 79

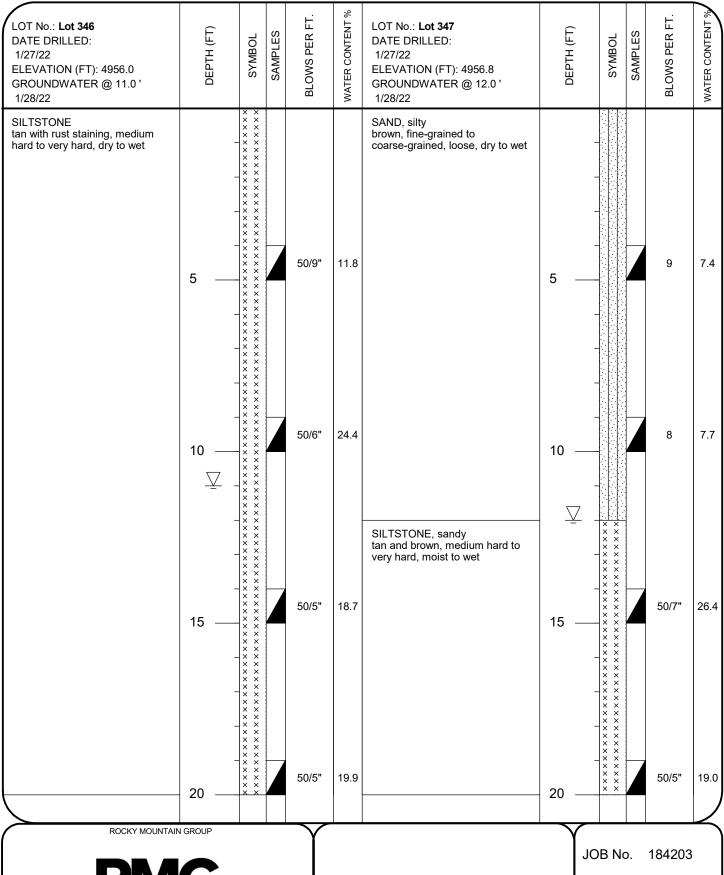
LOT No.: Lot 344 DATE DRILLED: 1/27/22 ELEVATION (FT): 4953.6 GROUNDWATER @ 11.0 ' 1/28/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %	LOT No.: Lot 345 DATE DRILLED: 1/27/22 ELEVATION (FT): 4955.0 GROUNDWATER @ 11.0 ' 1/28/22	ОЕРТН (FT)	SYMBOL	SAMPLES	BLOWS PER FT.	WATER CONTENT %
SAND, silty brown, fine-grained, loose, dry to moist	- - - - 5 —		Z	10	6.9	SILTSTONE, sandy tan and brown, hard to very hard, dry to wet	5 —	× × × × × × × × × × × × × × × × × × ×		50/8"	16.5
SILTSTONE tan and brown with mineral staining, medium hard to very hard, moist to wet	- - - 10 —- \(\sum_{-}\)	× × × × × × × × × × × × × × × × × × ×		50/8"	14.7	Auger Refusal at 12 feet due to very hard Bedrock	10 — —	× × × × × × × × × × × × × × × × × × ×		50/6"	23.7
Auger Refusal at 13 feet due to very hard Bedrock	_	× × × × × × × × × × × × × × × × × × ×				very flara Bearook	_	0 0			
ROCKY MOUNTAIN	N GROUP			Y			Y.,			184203	



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JOB No. 184203

FIGURE No. 80



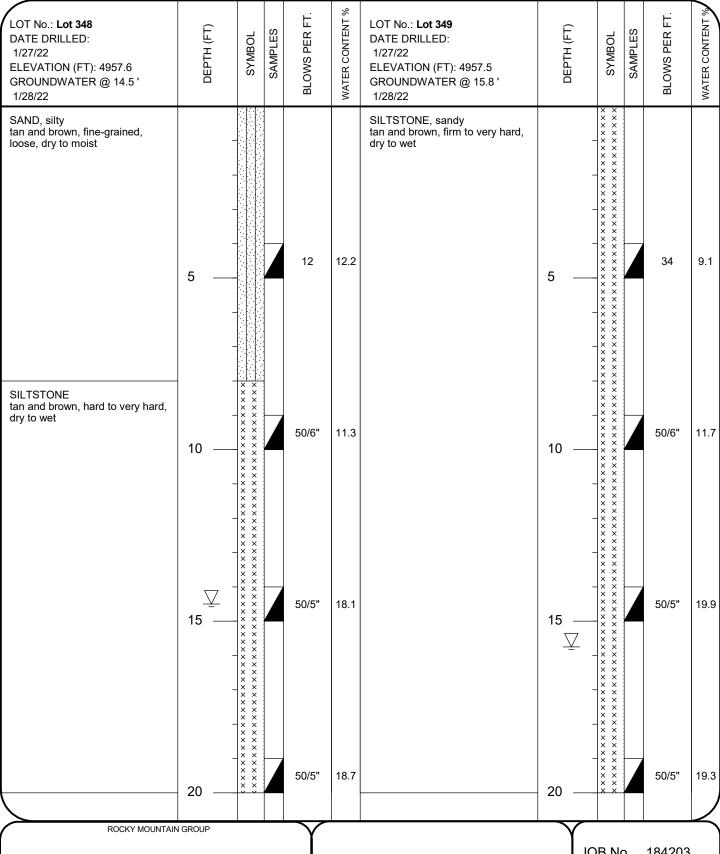


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Geotechnical Materials Testin Civil, Planning TEST BORING LOGS

FIGURE No. 81



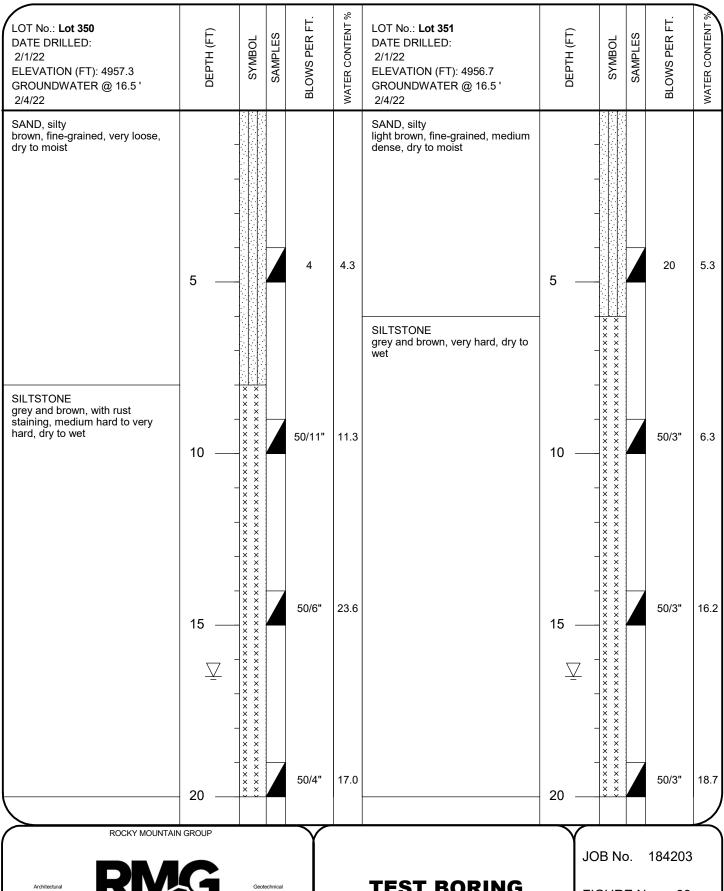


TEST BORING LOGS

JOB No. 184203

FIGURE No. 82

Mar/28/2022 DATE



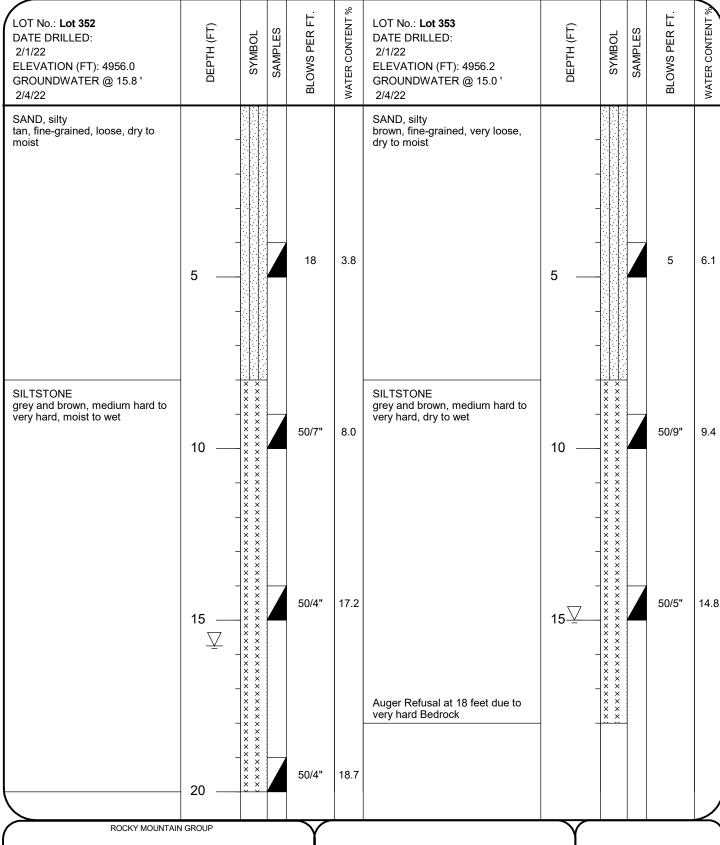
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TEST BORING LOGS

FIGURE No. 83

Mar/28/2022 DATE





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TEST BORING LOGS

JOB No. 184203

FIGURE No. 84

SOILS DESCRIPTION

CLAYEY SAND

FILL: SAND, SILTY TO CLAYEY

SANDSTONE

SILTSTONE

SILTY SAND

SILTY TO CLAYEY SAND

UNLESS NOTED OTHERWISE, ALL LABORATORY TESTS PRESENTED HEREIN WERE PERFORMED BY: RMG - ROCKY MOUNTAIN GROUP 1601 37TH ST. EVANS, COLORADO

SYMBOLS AND NOTES

STANDARD PENETRATION TEST - MADE BY DRIVING A SPLIT-BARREL SAMPLER INTO THE SOIL BY DROPPING A 140 LB. HAMMER 30", IN GENERAL ACCORDANCE WITH ASTM D-1586. NUMBER INDICATES NUMBER OF HAMMER BLOWS PER FOOT (UNLESS OTHERWISE INDICATED).

UNDISTURBED CALIFORNIA SAMPLE - MADE BY DRIVING A RING-LINED SAMPLER INTO THE SOIL BY DROPPING A 140 LB. HAMMER 30", IN GENERAL ACCORDANCE WITH ASTM D-3550. NUMBER INDICATES NUMBER OF HAMMER BLOWS PER FOOT (UNLESS OTHERWISE INDICATED).

FREE WATER TABLE

DEPTH AT WHICH BORING CAVED

BULK DISTURBED BULK SAMPLE

AUG AUGER "CUTTINGS"

4.5 WATER CONTENT (%)

ROCKY MOUNTAIN GROUP

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Geotechnical Materials Testing Civil, Planning EXPLANATION OF TEST BORING LOGS

JOB No. 184203

FIGURE No. 85

DATE Mar/28/2022

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Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 139	4.0	12.9	115.7						
Lot 139	9.0	14.6	104.5						
Lot 139	14.0	15.6	103.6						
Lot 139	19.0	18.9	95.5						
Lot 140	4.0	8.9	106.5						
Lot 140	9.0	17.8	105.9	43	26	1.9	64.7	1.9	1000
Lot 140	14.0	12.2	98.0						
Lot 140	19.0	19.2	95.3						
Lot 141	4.0	10.3	108.9						
Lot 141	9.0	14.4	96.3	28	7	0.0	21.1	- 0.1	1000
Lot 141	14.0	17.7	97.1						
Lot 141	19.0	18.9	98.5						
Lot 142	4.0	7.8	107.3						
Lot 142	9.0	9.7	107.9						
Lot 142	14.0	15.0	99.2						
Lot 142	19.0	22.9	95.5						
Lot 155	4.0	6.8	95.7						
Lot 155	9.0	9.2	101.9						
Lot 155	14.0	12.7	103.5						
Lot 155	19.0	19.8	103.4						
Lot 156	4.0	8.2	105.0						
Lot 156	9.0	12.9	103.0						
Lot 156	14.0	11.5	112.1						
Lot 156	19.0	10.6	110.0						
Lot 157	4.0	7.5	101.4						
Lot 157	9.0	9.4	101.4						
Lot 157	14.0	11.4	99.6						
Lot 157	19.0	15.9	101.4						
Lot 158	4.0	7.4	101.8						
Lot 158	9.0	9.0	102.3	NP	NP	0.0	16.7	- 0.1	1000
Lot 158	14.0	13.8	104.4						
Lot 158	19.0	16.2	101.9						
Lot 172	4.0	14.8	111.4						
Lot 172	9.0	13.4	94.1						

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Geotechnical Materials Testing

SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 1 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 172	14.0	9.9	92.5						
Lot 172	19.0	13.7	99.0						
Lot 173	4.0	16.3	108.5						
Lot 173	9.0	11.4	96.1						
Lot 173	14.0	10.4	97.1						
Lot 173	19.0	3.5							
Lot 174	4.0	12.7	115.7						
Lot 174	9.0	14.1	102.2	NP	NP	0.0	24.7	- 0.9	1000
Lot 174	14.0	9.3	96.5						
Lot 174	19.0	21.0	95.1						
Lot 175	4.0	12.7	115.7						
Lot 175	9.0	14.1	102.2						
Lot 175	14.0	9.3	96.5						
Lot 175	19.0	21.0	95.1						
Lot 176	4.0	7.2	102.5						
Lot 176	9.0	9.7	96.6						
Lot 176	14.0	18.4	96.8						
Lot 176	19.0	20.3	86.9						
Lot 177	4.0	10.5	102.9	NP	NP	0.0	21.1	- 0.2	1000
Lot 177	9.0	11.3	91.7						
Lot 177	14.0	12.4	88.3						
Lot 178	4.0	9.8	101.7						
Lot 178	9.0	10.3	99.5						
Lot 178	14.0	12.6	81.9						
Lot 178	19.0	20.3	87.6						
Lot 179	4.0	10.9	98.9						
Lot 179	9.0	9.4	93.7						
Lot 179	14.0	11.0	93.8						
Lot 180	4.0	18.0	100.8						
Lot 180	9.0	12.7	98.5						
Lot 181	4.0	9.2	94.7						
Lot 181	9.0	10.3	91.9	NP	NP	0.0	36.1	- 0.9	1000
Lot 181	14.0	12.4	98.0						
Lot 182	4.0	8.4	97.5						

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Geotechnical Materials Testing

SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 2 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 182	9.0	10.9	95.5						
Lot 182	14.0	18.1	99.6						
Lot 183	4.0	13.4	113.0						
Lot 183	9.0	9.2	93.2						
Lot 183	14.0	14.1	88.6						
Lot 183	19.0	19.3	82.8						
Lot 184	4.0	11.0	96.6						
Lot 184	9.0	11.2	81.9						
Lot 184	14.0	11.1	89.6						
Lot 184	19.0	18.6	79.8						
Lot 185	4.0	13.9	115.9	28	8	1.5	38.7	- 0.1	1000
Lot 185	9.0	12.2	101.9						
Lot 185	14.0	12.8	91.3						
Lot 185	19.0	19.5	88.9						
Lot 186	4.0	13.9	109.3						
Lot 186	9.0	17.9	100.7						
Lot 186	14.0	14.4	101.8						
Lot 187	4.0	14.7	112.9						
Lot 187	9.0	9.2	107.3						
Lot 187	14.0	11.4	92.1						
Lot 187	19.0	9.5	104.6						
Lot 188	4.0	15.9	110.4						
Lot 188	9.0	9.6	107.8						
Lot 188	14.0	10.2	90.0						
Lot 189	4.0	6.9	112.8						
Lot 189	9.0	9.1	104.6						
Lot 189	14.0	11.0	96.1						
Lot 189	19.0	17.9							
Lot 190	4.0	12.6	109.1						
Lot 190	9.0	9.8	93.0	31	8	0.0	10.1	- 0.8	1000
Lot 190	14.0	15.5	90.4						
Lot 190	19.0	16.9							
Lot 191	4.0	13.9	101.3						
Lot 191	9.0	11.4	95.8						

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Geotechnical Materials Testing

SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 3 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 191	14.0	14.4	92.9						
Lot 191	19.0	18.0	99.8						
Lot 192	4.0	15.8	96.6						
Lot 192	9.0	13.0	98.5						
Lot 192	14.0	13.1	80.5						
Lot 192	19.0	19.5	98.9						
Lot 193	4.0	22.0	101.7						
Lot 193	9.0	15.5	96.0						
Lot 193	14.0	11.8	97.9						
Lot 193	19.0	19.0	97.7						
Lot 194	4.0	9.9	100.9						
Lot 194	9.0	12.7	99.7						
Lot 194	14.0	10.8	96.9						
Lot 194	19.0	16.8	106.2						
Lot 195	4.0	7.1	107.3	NP	NP	0.0	46.7	- 0.8	1000
Lot 195	9.0	11.2	103.2						
Lot 195	14.0	9.8							
Lot 195	19.0	18.4	98.2						
Lot 196	4.0	8.5	100.8						
Lot 196	9.0	9.5	105.9						
Lot 196	14.0	15.7	95.2						
Lot 196	19.0	18.3							
Lot 197	4.0	7.8	104.0						
Lot 197	9.0	14.6	102.5						
Lot 197	14.0	13.1	98.0						
Lot 197	19.0	17.5	100.4						
Lot 198	4.0	6.9	102.8						
Lot 198	9.0	22.6	99.9	35	10	0.0	74.3	0.4	1000
Lot 198	14.0	17.8	103.9						
Lot 198	19.0	18.4	87.9						
Lot 199	4.0	5.5	101.3						
Lot 199	9.0	21.0	104.6						
Lot 199	14.0	13.2	95.2						
Lot 199	19.0	18.6	99.5						

Architectural Structural Forensics



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SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 4 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 200	4.0	4.4	99.3						
Lot 200	9.0	16.0	107.7						
Lot 200	14.0	13.7							
Lot 200	19.0	17.1	96.5						
Lot 201	4.0	6.7	110.1	27	9	4.5	36.0	- 0.9	1000
Lot 201	9.0	9.5	94.1						
Lot 201	14.0	15.1	87.4						
Lot 201	19.0	19.2	98.1						
Lot 202	4.0	12.8	97.6						
Lot 202	9.0	9.7	95.9						
Lot 202	14.0	17.2							
Lot 202	19.0	18.6	94.3						
Lot 203	4.0	13.0	116.8						
Lot 203	9.0	10.4	100.3						
Lot 203	14.0	16.2	94.5						
Lot 203	19.0	19.2	79.0						
Lot 204	4.0	22.3	103.1						
Lot 204	9.0	17.5	98.2						
Lot 204	14.0	18.6	87.8						
Lot 204	19.0	20.0	94.9						
Lot 205	4.0	18.6	95.6						
Lot 205	9.0	16.1	108.2	35	18	0.0	47.7	- 0.2	1000
Lot 205	14.0	17.8	85.2						
Lot 205	19.0	23.0	92.2						
Lot 206	4.0	14.7	108.8						
Lot 206	9.0	7.0							
Lot 206	14.0	18.1	93.3						
Lot 215	4.0	8.2	112.7						
Lot 215	9.0	11.4	108.1						
Lot 215	14.0	10.5	101.9						
Lot 215	19.0	19.7	98.9						
Lot 216	4.0	7.0	106.6	22	3	0.0	38.1	- 2.9	1000
Lot 216	9.0	12.4	107.0						
Lot 216	14.0	11.2	96.2						

Architectural Structural Forensics



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Geotechnical Materials Testing

SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 5 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 216	19.0	19.5	99.0						
Lot 217	4.0	12.8	114.3						
Lot 217	9.0	10.0	103.4						
Lot 217	14.0	12.1	105.6						
Lot 217	19.0	25.1	95.1						
Lot 218	4.0	8.3	102.3						
Lot 218	9.0	10.9	115.2						
Lot 218	14.0	11.1	99.3						
Lot 218	19.0	19.1	87.1						
Lot 219	4.0	8.1	106.7						
Lot 219	9.0	8.7							
Lot 219	14.0	12.6	102.3						
Lot 219	19.0	17.1	101.5						
Lot 220	4.0	6.3	118.9						
Lot 220	9.0	14.2	104.7						
Lot 220	14.0	12.4	100.5						
Lot 220	19.0	18.6	101.7						
Lot 221	4.0	10.1	93.6						
Lot 221	9.0	10.7	109.4	30	6	0.0	51.1	- 0.4	1000
Lot 221	14.0	10.7	102.2						
Lot 221	19.0	10.8	104.4						
Lot 222	4.0	6.6	109.7						
Lot 222	9.0	14.8	107.2						
Lot 222	14.0	11.4	101.4						
Lot 222	19.0	15.6	93.2						
Lot 223	4.0	20.6	98.4						
Lot 223	9.0	17.5	101.4						
Lot 223	14.0	10.7	104.8						
Lot 223	19.0	13.2	110.2						
Lot 224	4.0	6.8	107.9						
Lot 224	9.0	21.6	87.4						
Lot 224	14.0	13.6	102.1						
Lot 224	19.0	16.2	109.3						
Lot 225	4.0	9.1	105.0	25	5	0.0	38.5	- 0.6	1000

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Geotechnical Materials Testing SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 6 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 225	9.0	15.1	109.8						
Lot 225	14.0	12.6	108.8						
Lot 244	4.0	10.6	109.6						
Lot 244	9.0	10.3	112.4	33	12	0.0	19.1	- 0.2	1000
Lot 244	14.0	11.1	103.7						
Lot 245	4.0	10.8	113.3						
Lot 245	9.0	7.1	105.6						
Lot 245	14.0	10.6	100.6						
Lot 246	4.0	8.7	111.9						
Lot 246	9.0	5.8	104.2						
Lot 246	14.0	9.2	103.0						
Lot 246	19.0	24.7	81.6						
Lot 247	4.0	7.2	117.5	22	3	0.0	25.9	- 1.2	1000
Lot 247	9.0	5.5	107.9						
Lot 247	14.0	10.4	97.1						
Lot 247	19.0	24.1	97.4						
Lot 248	4.0	5.9	108.2						
Lot 248	9.0	10.7	111.7						
Lot 248	14.0	12.1							
Lot 248	19.0	23.5	94.7						
Lot 249	4.0	7.1	114.2						
Lot 249	9.0	7.0	108.6						
Lot 249	14.0	13.7	111.0						
Lot 249	19.0	24.6	95.8						
Lot 250	4.0	7.4	110.9						
Lot 250	9.0	4.6	112.9						
Lot 250	14.0	4.8							
Lot 250	19.0	23.1	91.4						
Lot 251	4.0	8.0	113.8						
Lot 251	9.0	8.0	102.1						
Lot 251	14.0	9.6	98.8						
Lot 251	19.0	22.9	96.0						
Lot 252	4.0	5.7	109.3						
Lot 252	9.0	6.5	112.1						

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Geotechnical Materials Testing SUMMARY OF LABORATORY TEST RESULTS JOB No. 184203 FIGURE No. 86 PAGE 7 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 252	14.0	13.2	96.8						
Lot 253	4.0	7.3	111.7	25	8	0.0	37.7	- 1.5	1000
Lot 253	9.0	7.0	97.3						
Lot 253	14.0	12.4	99.9						
Lot 253	19.0	25.4	95.9						
Lot 254	4.0	6.5	107.3						
Lot 254	9.0	5.7	109.4						
Lot 254	14.0	16.7	101.2						
Lot 254	19.0	24.1	93.8						
Lot 255	4.0	7.2	112.0						
Lot 255	9.0	9.3	104.5						
Lot 255	14.0	12.3	103.4						
Lot 255	19.0	25.4	95.7						
Lot 256	4.0	5.0	111.3						
Lot 256	9.0	7.5	93.5						
Lot 256	14.0	13.2	105.4						
Lot 256	19.0	24.3	95.5						
Lot 257	4.0	8.4	117.9						
Lot 257	9.0	7.4	102.8						
Lot 257	14.0	8.4	102.8						
Lot 257	19.0	24.0	97.2						
Lot 258	4.0	12.0	110.3						
Lot 258	9.0	11.7	109.2						
Lot 258	14.0	5.5							
Lot 258	19.0	21.7	96.7						
Lot 259	4.0	11.9	116.0						
Lot 259	9.0	13.5	113.6						
Lot 259	14.0	9.7	105.2						
Lot 259	19.0	13.2	104.5						
Lot 260	4.0	4.8	110.3						
Lot 260	9.0	8.3	93.5						
Lot 260	14.0	10.6	109.4						
Lot 260	19.0	18.3	93.5						
Lot 261	4.0	8.0	112.4						

Architectural Structural Forensics



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Geotechnical Materials Testing

SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 8 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 261	9.0	9.2	108.4	26	7	0.0	49.6	- 0.4	1000
Lot 261	14.0	12.3	100.2						
Lot 261	19.0	24.3	97.4						
Lot 262	4.0	11.8	109.3						
Lot 262	9.0	9.9	101.7						
Lot 262	14.0	14.1	105.7						
Lot 262	19.0	22.2							
Lot 263	4.0	6.9	111.6						
Lot 263	9.0	11.4	100.7						
Lot 264	4.0	15.2	112.0						
Lot 264	9.0	10.4	108.6						
Lot 264	14.0	11.9	103.4						
Lot 264	19.0	26.3	92.8						
Lot 265	4.0	7.1	108.6						
Lot 265	9.0	9.3	105.5						
Lot 265	14.0	12.9	103.0						
Lot 265	19.0	23.8	84.3						
Lot 266	4.0	8.6	105.3						
Lot 266	9.0	11.0	101.4	NP	NP	0.0	23.2	- 0.1	1000
Lot 266	14.0	19.5	100.2						
Lot 266	19.0	21.5	88.4						
Lot 267	4.0	7.0	105.2						
Lot 267	9.0	11.2	92.0						
Lot 267	14.0	15.1	89.8						
Lot 267	19.0	17.5	88.4						
Lot 268	4.0	12.9	100.8						
Lot 268	9.0	10.8	93.3						
Lot 268	14.0	12.7	92.3						
Lot 268	19.0	17.7	84.2						
Lot 269	4.0	5.3	102.9						
Lot 269	9.0	9.8	99.7						
Lot 269	14.0	11.2	101.4						
Lot 269	19.0	17.4	87.5						
Lot 270	4.0	5.0	106.5						

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Geotechnical Materials Testing

SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 9 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 270	9.0	8.2	107.3					- 0.5	1000
Lot 270	14.0	10.0	92.2						
Lot 270	19.0	16.8	89.2						
Lot 271	4.0	3.8	109.4						
Lot 271	9.0	10.4	103.0						
Lot 271	14.0	10.0	87.8						
Lot 271	19.0	23.4	94.2						
Lot 272	4.0	5.8	103.8						
Lot 272	9.0	9.6	102.4						
Lot 272	14.0	11.5	100.8						
Lot 272	19.0	11.8							
Lot 273	4.0	14.8	95.3	28	8	0.0	62.1	0.0	1000
Lot 273	9.0	10.8	98.4						
Lot 273	14.0	17.0	96.9						
Lot 273	19.0	18.4							
Lot 274	4.0	18.2	97.2						
Lot 274	9.0	10.0	102.4						
Lot 274	14.0	15.2	92.9						
Lot 274	19.0	18.8							
Lot 275	4.0	22.7	99.2						
Lot 275	9.0	10.3	92.1	NP	NP	0.0	16.3	- 0.8	1000
Lot 275	14.0	14.2	93.5						
Lot 275	19.0	20.5	96.7						
Lot 276	4.0	22.4	96.7						
Lot 276	9.0	9.0	95.8						
Lot 276	14.0	12.2	108.3						
Lot 276	19.0	19.7	91.1						
Lot 277	4.0	23.9	96.5						
Lot 277	9.0	10.3	90.0						
Lot 277	14.0	13.0	95.2						
Lot 277	19.0	18.8							
Lot 278	4.0	11.0	98.6						
Lot 278	9.0	10.0	100.9						
Lot 278	14.0	13.3							

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Geotechnical Materials Testing

SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 10 OF 19 DATE Mar/28/2022

Engineers / Architects

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Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 278	19.0	17.0	106.9						
Lot 279	4.0	12.2	106.9	48	26	0.0	53.9	- 1.1	1000
Lot 279	9.0	9.5	96.2						
Lot 279	14.0	12.2	99.0						
Lot 279	19.0	17.6	96.4						
Lot 280	4.0	11.0	99.6						
Lot 280	9.0	7.4	93.4						
Lot 280	14.0	11.1	94.8						
Lot 281	4.0	9.1	102.9						
Lot 281	9.0	9.1	99.7						
Lot 281	14.0	10.1	94.1						
Lot 281	19.0	17.2							
Lot 282	4.0	13.1	100.7	33	5	0.0	29.0	- 0.7	1000
Lot 282	9.0	9.7	94.9						
Lot 282	14.0	13.5	89.0						
Lot 282	19.0	17.9	94.7						
Lot 283	4.0	1.0							
Lot 284	4.0	12.7	109.7						
Lot 284	9.0	10.5	88.6						
Lot 284	14.0	13.8	86.4						
Lot 284	19.0	18.3	89.2						
Lot 285	4.0	19.3							
Lot 285	9.0	10.0	91.3						
Lot 285	14.0	14.9	96.7						
Lot 285	19.0	21.5	62.8						
Lot 286	4.0	21.3	90.7						
Lot 286	9.0	12.1	96.2						
Lot 286	14.0	14.0	92.4						
Lot 286	19.0	17.8	85.8						
Lot 287	4.0	10.4	114.4						
Lot 288	4.0	19.8	105.9	49	28	0.0	61.5	0.1	1000
Lot 288	9.0	11.4	95.2						
Lot 288	14.0	12.9	85.8						
Lot 288	19.0	19.2	96.8						

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SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 11 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 289	4.0	17.4	98.8						
Lot 289	9.0	12.8	91.9						
Lot 289	14.0	14.6	85.3						
Lot 289	19.0	1.3							
Lot 290	4.0	7.0	102.3						
Lot 290	9.0	9.6							
Lot 290	14.0	11.7	90.1						
Lot 290	19.0	18.3	78.2						
Lot 291	4.0	11.5	103.3						
Lot 291	9.0	11.7	89.7						
Lot 291	14.0	11.5	73.6						
Lot 291	19.0	16.5	60.1						
Lot 292	4.0	13.1	98.8						
Lot 292	9.0	12.6	93.1						
Lot 293	4.0	15.8	97.7						
Lot 293	9.0	9.4	107.6	37	13	0.0	19.7	- 0.8	1000
Lot 293	14.0	12.2							
Lot 294	4.0	12.9	113.6						
Lot 294	9.0	10.8	92.8						
Lot 294	14.0	13.5	83.6						
Lot 295	4.0	9.2	106.5						
Lot 295	9.0	8.9	88.3						
Lot 295	14.0	13.2	88.4						
Lot 296	4.0	11.7	99.4						
Lot 296	9.0	9.3	89.1						
Lot 296	14.0	4.5							
Lot 297	4.0	10.2	111.4						
Lot 297	9.0	9.5	91.7						
Lot 297	14.0	11.5	92.0						
Lot 297	19.0	17.3	86.7						
Lot 298	4.0	8.3	103.1						
Lot 298	9.0	9.5	77.9						
Lot 298	14.0	13.7	86.1						
Lot 298	19.0	19.1	88.3						

Architectural Structural Forensics



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Materials Te Civil, Plan SUMMARY OF LABORATORY TEST RESULTS JOB No. 184203 FIGURE No. 86 PAGE 12 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 299	4.0	9.0	99.2	32	7	0.0	30.6	- 5.1	1000
Lot 299	9.0	10.9	90.6						
Lot 299	14.0	12.6	80.8						
Lot 299	19.0	17.0	88.6						
Lot 300	4.0	7.4	103.7						
Lot 300	9.0	9.0							
Lot 300	14.0	13.1	95.9						
Lot 300	19.0	12.5	91.8						
Lot 301	4.0	10.1	98.4						
Lot 301	9.0	5.0							
Lot 302	4.0	9.1	99.5						
Lot 302	9.0	11.7	95.2						
Lot 302	14.0	19.7	104.6						
Lot 302	19.0	18.8	97.7						
Lot 303	4.0	8.8	94.2						
Lot 303	9.0	13.8	91.9						
Lot 304	4.0	5.1	103.4						
Lot 304	9.0	10.8	91.7	32	7	0.0	15.4	- 1.3	1000
Lot 304	14.0	21.9	94.8						
Lot 304	19.0	25.3	92.6						
Lot 305	4.0	9.9	111.0						
Lot 305	9.0	12.9	94.4						
Lot 305	14.0	22.9	82.3						
Lot 305	19.0	1.9							
Lot 306	4.0	10.4	91.6						
Lot 306	9.0	13.1	90.6						
Lot 306	14.0	22.8	91.7						
Lot 306	19.0	23.4	88.2						
Lot 307	4.0	10.7	103.2						
Lot 307	9.0	13.5	100.6						
Lot 307	14.0	18.6	86.4						
Lot 307	19.0	20.6	90.8						
Lot 308	4.0	9.8	94.2						
Lot 308	9.0	12.4	101.5						

Architectural Structural Forensics



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Geotechnical Materials Testing SUMMARY OF LABORATORY TEST RESULTS JOB No. 184203 FIGURE No. 86 PAGE 13 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 308	14.0	19.6	97.6						
Lot 308	19.0	18.8	76.9						
Lot 309	4.0	6.9	102.7	NP	NP	0.5	28.5	- 0.9	1000
Lot 309	9.0	10.7	95.1						
Lot 309	14.0	23.0	78.0						
Lot 309	19.0	27.0	84.9						
Lot 310	4.0	5.4	108.3						
Lot 310	9.0	10.8	105.4						
Lot 310	14.0	18.4	100.7						
Lot 310	19.0	24.5	97.6						
Lot 311	4.0	12.9	106.3						
Lot 311	9.0	6.4	108.3						
Lot 311	14.0	20.4	97.7						
Lot 311	19.0	25.4	95.9						
Lot 312	4.0	5.2	106.8	29	14	0.0	38.9	- 2.0	1000
Lot 312	9.0	7.0	107.5						
Lot 312	14.0	11.6	108.5						
Lot 312	19.0	26.7	95.4						
Lot 313	4.0	5.8	103.9						
Lot 313	9.0	10.7	97.9						
Lot 313	14.0	13.1	101.6						
Lot 313	19.0	18.9	96.5						
Lot 314	4.0	7.1	110.0						
Lot 314	9.0	7.2	103.4						
Lot 314	14.0	11.8	105.0						
Lot 314	19.0	23.1	100.9						
Lot 315	4.0	7.0	113.4						
Lot 315	9.0	5.4	100.0						
Lot 315	14.0	13.9	108.8						
Lot 315	19.0	24.5	98.6						
Lot 316	4.0	5.4	119.4						
Lot 316	9.0	6.3	106.6						
Lot 316	14.0	12.7	106.6						
Lot 316	19.0	25.0	95.3						

Architectural Structural Forensics



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SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 14 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 317	4.0	5.8	108.6						
Lot 317	9.0	6.5	94.4	28	9	0.0	14.4	- 2.3	1000
Lot 317	14.0	22.4	101.9						
Lot 317	19.0	25.4	93.4						
Lot 318	4.0	7.3	105.8						
Lot 318	9.0	6.4							
Lot 318	14.0	23.7	97.3						
Lot 318	19.0	24.0	98.4						
Lot 319	4.0	6.4	98.4						
Lot 320	4.0	6.3	110.3						
Lot 320	9.0	6.7	114.2	26	5	0.0	32.5	0.3	1000
Lot 320	14.0	20.0	99.5						
Lot 320	19.0	20.3							
Lot 321	4.0	5.7	111.1						
Lot 321	9.0	8.2	113.4						
Lot 321	14.0	18.8	102.5						
Lot 321	19.0	27.4	94.4						
Lot 322	4.0	5.1	106.7						
Lot 322	9.0	11.1	103.8						
Lot 322	14.0	23.8	97.9						
Lot 322	19.0	35.1	81.0						
Lot 323	4.0	6.8	108.8						
Lot 323	9.0	11.8	113.9						
Lot 323	14.0	22.1	101.3						
Lot 323	19.0	31.6							
Lot 324	4.0	4.6	109.9						
Lot 324	9.0	6.1	105.6						
Lot 324	14.0	14.8	95.4						
Lot 324	19.0	27.2	91.0						
Lot 325	4.0	7.1	112.4						
Lot 325	9.0	6.2	108.5					- 0.8	1000
Lot 325	14.0	12.8	101.3						
Lot 325	19.0	22.0	88.3						
Lot 326	4.0	9.7	115.8	NP	NP		30.7	- 0.3	1000

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Geotechnical Materials Testing Civil, Planning SUMMARY OF LABORATORY TEST RESULTS JOB No. 184203 FIGURE No. 86 PAGE 15 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 326	9.0	9.3	111.3						
Lot 326	14.0	14.1	110.9						
Lot 326	19.0	22.6	90.6						
Lot 327	4.0	11.3	120.4						
Lot 327	9.0	14.7	78.6						
Lot 327	14.0	17.8	107.1						
Lot 327	19.0	25.1	96.6						
Lot 328	4.0	10.2	117.5						
Lot 328	9.0	6.1	107.7						
Lot 328	14.0	13.5	103.2						
Lot 328	19.0	18.3	91.8						
Lot 329	4.0	5.8	111.3						
Lot 329	9.0	6.4	108.4						
Lot 329	14.0	20.1	101.0						
Lot 329	19.0	25.6	95.6						
Lot 330	4.0	9.2	109.6						
Lot 330	9.0	13.0	96.0						
Lot 330	14.0	26.6	94.9						
Lot 330	19.0	20.3	95.2						
Lot 331	4.0	11.2	116.1						
Lot 331	9.0	12.6	108.2						
Lot 331	14.0	26.0	94.4						
Lot 331	19.0	26.9	95.0						
Lot 332	4.0	9.4	105.7						
Lot 332	9.0	14.8	97.3	28	1	0.0	23.3	- 0.4	1000
Lot 332	14.0	25.3	95.3						
Lot 332	19.0	26.5	94.5						
Lot 333	4.0	4.1	105.2						
Lot 333	9.0	5.8	107.6						
Lot 333	14.0	21.0	117.4						
Lot 333	19.0	25.0	86.8						
Lot 334	4.0	5.5	106.1						
Lot 334	9.0	9.8	102.8						
Lot 334	14.0	19.0	104.1						

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SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 16 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 334	19.0	24.2	86.7						
Lot 335	4.0	9.2	110.7						
Lot 335	9.0	4.6	110.9						
Lot 335	14.0	12.8	103.3						
Lot 335	19.0	26.7	90.4						
Lot 336	4.0	9.2	110.7						
Lot 336	9.0	4.2	108.0						
Lot 336	14.0	10.5	113.7						
Lot 336	19.0	19.1	106.9						
Lot 337	4.0	10.4	118.1	23	5		32.1	- 0.2	1000
Lot 337	9.0	5.5	107.2						
Lot 337	14.0	14.5	98.3						
Lot 337	19.0	23.9	98.5						
Lot 338	4.0	13.1	117.2						
Lot 338	9.0	4.8	106.6						
Lot 338	14.0	18.5	107.8						
Lot 338	19.0	20.5	97.8						
Lot 339	4.0	14.0	113.5						
Lot 339	9.0	7.1	108.3						
Lot 339	14.0	21.7	101.8						
Lot 339	19.0	23.8	101.3						
Lot 340	4.0	10.3	97.8	NP	NP	0.0	23.6	- 2.1	1000
Lot 340	9.0	13.1	98.8						
Lot 340	14.0	20.1	94.9						
Lot 340	19.0	25.1	75.7						
Lot 341	4.0	7.9	98.4						
Lot 341	9.0	13.7	92.2						
Lot 341	14.0	24.2	85.9						
Lot 341	19.0	26.3	82.7						
Lot 342	4.0	12.1							
Lot 342	9.0	20.0	90.7						
Lot 342	14.0	17.6	92.3						
Lot 342	19.0	22.3	99.2						
Lot 343	4.0	7.0	100.9						

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SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 17 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 343	9.0	17.4	97.2						
Lot 344	4.0	6.9	109.6						
Lot 344	9.0	14.7	100.5						
Lot 345	4.0	16.5	98.2						
Lot 345	9.0	23.7	96.4	29	3	0.0	14.4	0.0	1000
Lot 346	4.0	11.8	104.2						
Lot 346	9.0	24.4	94.3						
Lot 346	14.0	18.7	98.7						
Lot 346	19.0	19.9	96.2						
Lot 347	4.0	7.4	108.9						
Lot 347	9.0	7.7	106.1						
Lot 347	14.0	26.4	92.4						
Lot 347	19.0	19.0	99.9						
Lot 348	4.0	12.2	108.6	27	5	0.0	31.6	- 0.2	1000
Lot 348	9.0	11.3	104.4						
Lot 348	14.0	18.1	98.2						
Lot 348	19.0	18.7	94.8						
Lot 349	4.0	9.1	98.9						
Lot 349	9.0	11.7	100.5						
Lot 349	14.0	19.9	101.7						
Lot 349	19.0	19.3	85.9						
Lot 350	4.0	4.3	98.6						
Lot 350	9.0	11.3	102.0						
Lot 350	14.0	23.6	96.2						
Lot 350	19.0	17.0	99.1						
Lot 351	4.0	5.3	103.8						
Lot 351	9.0	6.3							
Lot 351	14.0	16.2	83.8						
Lot 351	19.0	18.7	91.6						
Lot 352	4.0	3.8	103.2						
Lot 352	9.0	8.0	94.5						
Lot 352	14.0	17.2	93.4						
Lot 352	19.0	18.7	88.2						
Lot 353	4.0	6.1	105.8						

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Geotechnical Materials Testing

SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 18 OF 19 DATE Mar/28/2022

Test Boring No.	Depth	Water Content (%)	Dry Density (pcf)	Liquid Limit	Plasticity Index	% Retained No.4 Sieve	% Passing No. 200 Sieve	% Swell/ Collapse	Load (psf)
Lot 353	9.0	9.4	105.0	NP	NP	0.0	16.0	- 0.6	1000
Lot 353	14.0	14 8	97.3						

Architectural Structural Forensics

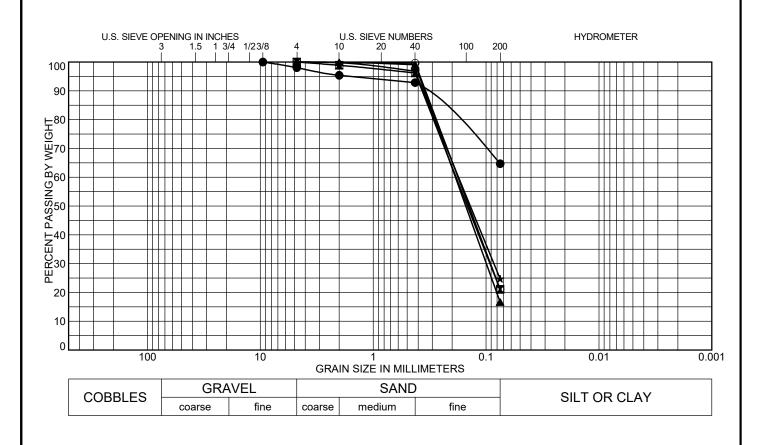


Geotechnical Materials Testing SUMMARY OF LABORATORY TEST RESULTS

JOB No. 184203 FIGURE No. 86 PAGE 19 OF 19 DATE Mar/28/2022

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-	Test Boring	Depth (ft)			Classific	ation		LL	PL	PI
•	Lot 140	9.0		SANDY LEAN CLAY(CL)						26
	Lot 141	9.0		SILTY, CLAYEY SAND(SC-SM)						7
A	Lot 158	9.0		SILTY SAND(SM)						NP
*	Lot 174	9.0			SILTY SAN	ND(SM)		NP	NP	NP
•	Lot 177	4.0			SILTY SAN	ND(SM)		NP	NP	NP
-	Test Boring	Depth (ft)	%Gravel	%Sand	%Silt	%Clay				
•	Lot 140	9.0	1 0 33.3 64.7							

	est Boring	Deptn (π)	%Gravei	%Sand	%5III	%Clay
•	Lot 140	9.0	1.9	33.3	64	.7
X	Lot 141	9.0	0.0	78.9	21	.1
A	Lot 158	9.0	0.0	83.3	16	5.7
*	Lot 174	9.0	0.0	75.3	24	.7
•	Lot 177	4.0	0.0	78.9	21	.1

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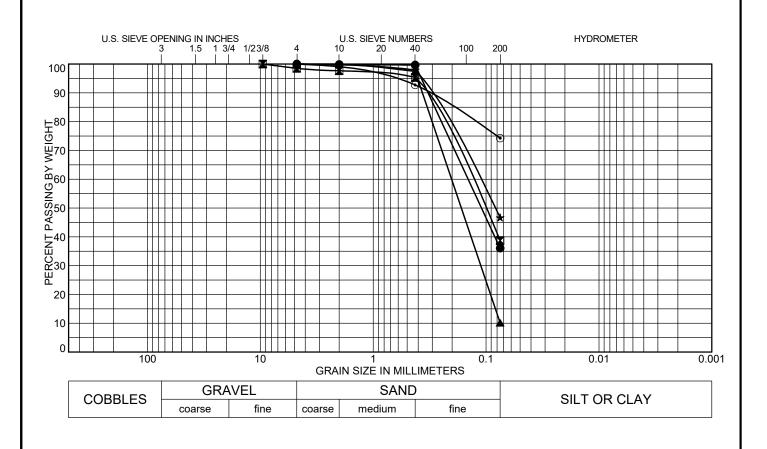
Geotechnical Materials Testing Civil, Planning

SOIL CLASSIFICATION DATA

JOB No. 184203

FIGURE No. 87

DATE Mar/28/2022



		0.0	0.0 63.9 36.1							
•	Lot 181	9.0	იი 63.9 36.1							
1	Test Boring	Depth (ft)	%Gravel	%Sand	%Silt	%Clay				
•	Lot 198	9.0		(SILT with S	AND(ML)		35	25	10
*	Lot 195	4.0		SILTY SAND(SM)						NP
▲	Lot 190	9.0		POORLY GRADED SAND with SILT(SP-SM)						8
X	Lot 185	4.0		CLAYEY SAND(SC)						8
•	Lot 181	9.0		SILTY SAND(SM)						NP
٦	est Boring	Depth (ft)		Classification						PI

-		p · (· · ·)		700ana		
•	Lot 181	9.0	0.0	63.9	36	5.1
\blacksquare	Lot 185	4.0	1.5	59.7	38	3.7
lack	Lot 190	9.0	0.0	89.9	10).1
*	Lot 195	4.0	0.0	53.3	46	5.7
•	Lot 198	9.0	0.0	25.7	74	.3

Architectural Structural Forensics



Geotechnical Materials Testing Civil, Planning SOIL CLASSIFICATION DATA

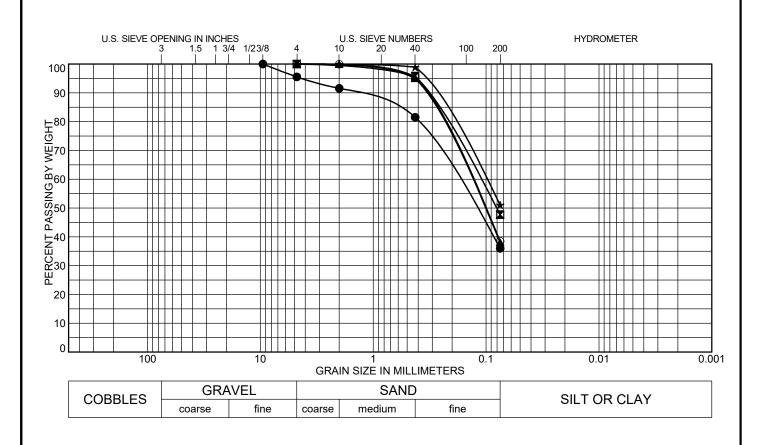
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FIGURE No. 88

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•	Lot 201	4.0	4.5	4.5 59.5 36.0						
T	Γest Boring	Depth (ft)	%Gravel %Sand %Silt %Clay							
•	Lot 225	4.0		SILTY	, CLAYEY S	SAND(SC-SM)	25	20	5
*	Lot 221	9.0			SANDY SI	LT(ML)		30	24	6
A	Lot 216	4.0		SILTY SAND(SM)					19	3
X	Lot 205	9.0		CLAYEY SAND(SC)					17	18
•	Lot 201	4.0			CLAYEY SA	AND(SC)		27	18	9
7	Test Boring	Depth (ft)			Classific	ation		LL	PL	PI

	i est Boring	Deptn (π)	%Gravei	%Sand	%5III	%Clay
•	Lot 201	4.0	4.5	59.5	36	.0
X	Lot 205	9.0	0.0	52.3	47	.7
A	Lot 216	4.0	0.0	61.9	38	.1
*	Lot 221	9.0	0.0	48.9	51	.1
•	Lot 225	4.0	0.0	61.5	38	.5

Architectural Structural Forensics



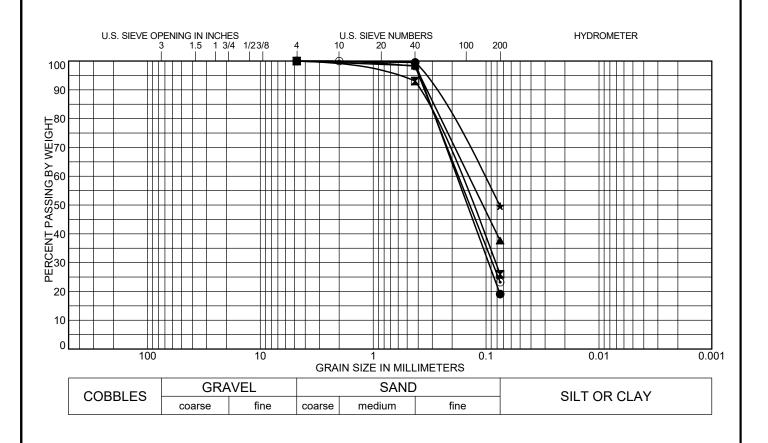
Geotechnical Materials Testing Civil, Planning

SOIL CLASSIFICATION DATA

JOB No. 184203

FIGURE No. 89

DATE Mar/28/2022



1	Test Boring	Boring Depth (ft) Classification				ation		LL	PL	PI
•	Lot 244	9.0			CLAYEY SA	AND(SC)		33	21	12
	Lot 247	4.0			SILTY SAN		22	19	3	
▲	Lot 253	4.0				25	17	8		
*	Lot 261	9.0		SILTY	, CLAYEY S	SAND(SC-SM	1)	26	19	7
•	Lot 266	9.0			SILTY SAN	ND(SM)		NP	NP	NP
	Test Boring Depth (ft)		%Gravel %Sand %Silt %Clay					·		
•	Lot 244	9.0	0.0	0.0 80.9 19.1						
					1					

	r cot bornig	Dopui (it)	70 Ola VCI	70Sanu	70	70 Oldy
•	Lot 244	9.0	0.0	80.9	19).1
X	Lot 247	4.0	0.0	74.1	25	5.9
▲	Lot 253	4.0	0.0	62.3	37	'.7
*	Lot 261	9.0	0.0	50.4	49).6
•	Lot 266	9.0	0.0	76.8	23	3.2

Architectural Structural Forensics

Geotechnical Materials Testing Civil, Planning

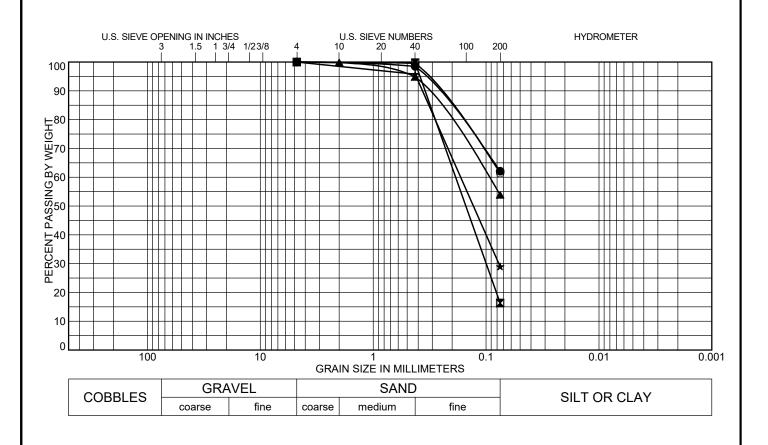
SOIL CLASSIFICATION DATA

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FIGURE No. 90

DATE Mar/28/2022

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-	Test Boring	Depth (ft)		Classification				PL	PI
•	Lot 273	4.0		SA	NDY LEAN	I CLAY(CL)	28	20	8
×	Lot 275	9.0			NP	NP	NP		
A	Lot 279	4.0		SANDY LEAN CLAY(CL)				22	26
*	Lot 282	4.0			SILTY SA	ND(SM)	33	28	5
•	Lot 288	4.0		SA	NDY LEAN	CLAY(CL)	49	21	28
-	Test Boring Depth (ft)		%Gravel	%Sand	%Silt	%Clay			•
•	Lot 273	4.0	0.0	37.9	6	2.1			

	l est Boring	Deptn (π)	%Gravei	%Sand	%SIIT	%Clay
•	Lot 273	4.0	0.0	37.9	62	1
X	Lot 275	9.0	0.0	83.7	16	3.3
	Lot 279	4.0	0.0	46.1	53	.9
*	Lot 282	4.0	0.0	71.0	29	.0
•	Lot 288	4.0	0.0	38.5	61	.5

Architectural Structural Forensics

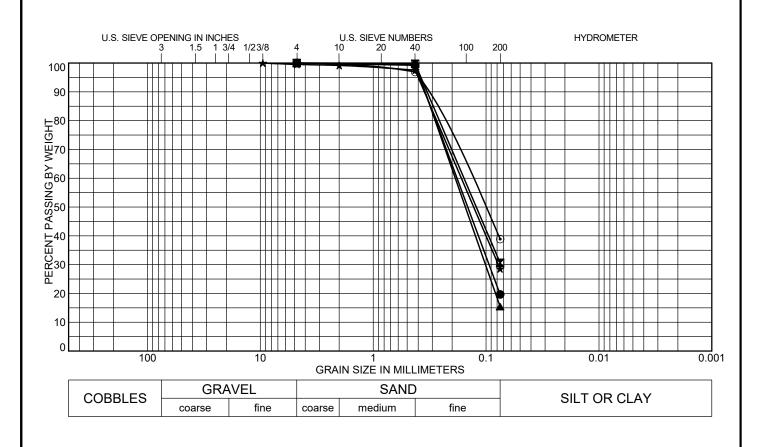
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Geotechnical Materials Testing Civil, Planning SOIL CLASSIFICATION DATA

JOB No. 184203

FIGURE No. 91

DATE Mar/28/2022



Test Boring	Depth (ft)		Classification						PI
● Lot 293	9.0			CLAYEY SA	AND(SC)		37	24	13
▼ Lot 299	4.0		SILTY SAND(SM)					25	7
▲ Lot 304	9.0		SILTY SAND(SM)					25	7
★ Lot 309	4.0			SILTY SAI	ND(SM)		NP	NP	NP
⊙ Lot 312	4.0		CLAYEY SAND(SC)				29	15	14
Test Boring	Test Boring Depth (ft)		%Gravel %Sand %Silt %Clay					,	
■ Lot 293 9.0		0.0	80.3	19	9.7				

	l est Boring	Deptn (π)	%Gravei	%Sand	%SIIT	%Clay
•	Lot 293	9.0	0.0	80.3	19	.7
X	Lot 299	4.0	0.0	69.4	30	.6
	Lot 304	9.0	0.0	84.6	15	5.4
*	Lot 309	4.0	0.5	71.0	28	3.5
•	Lot 312	4.0	0.0	61.1	38	3.9

Architectural Structural Forensics

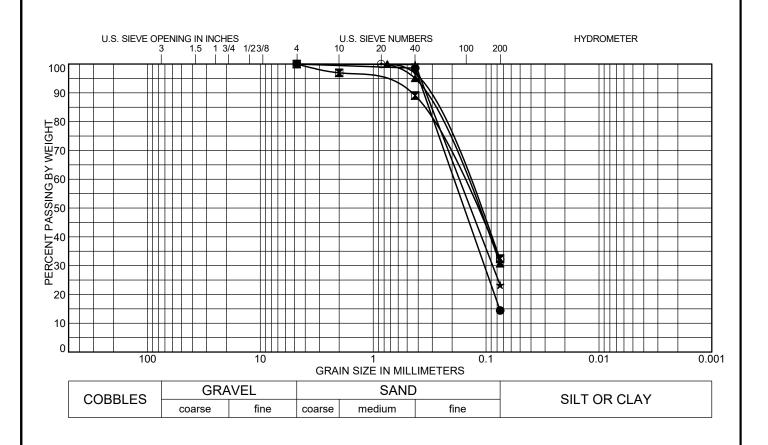
Geotechnical Materials Testing Civil, Planning SOIL CLASSIFICATION DATA

JOB No. 184203

FIGURE No. 92

DATE Mar/28/2022

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T	Test Boring	Depth (ft)			Classific		LL	PL	PI	
•	Lot 317	9.0			CLAYEY SA	AND(SC)		28	19	9
X	Lot 320	9.0		SILTY	, CLAYEY S	l)	26	21	5	
▲	Lot 326	4.0				NP	NP	NP		
*	Lot 332	9.0			SILTY SAN	ND(SM)		28	27	1
•	Lot 337	4.0		SILTY	, CLAYEY S	SAND(SC-SM	l)	23	18	5
٦	Test Boring Depth (ft)		%Gravel	%Gravel %Sand %Silt %Clay						
•	Lot 317	9.0	0.0	85.6	14	1.4				

	est Boring	Deptn (π)	%Gravei	%Sand	%SIIT	%Clay
•	Lot 317	9.0	0.0	85.6	14	.4
X	Lot 320	9.0	0.0	67.5	32	5
A	Lot 326	4.0	0.0	69.3	30	.7
*	Lot 332	9.0	0.0	76.7	23	.3
•	Lot 337	4.0	0.0	67.9	32	1

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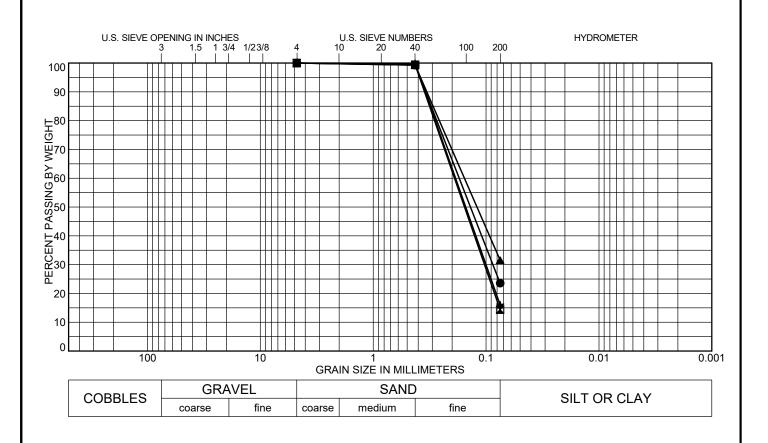
Geotechnical Materials Testing Civil, Planning SOIL CLASSIFICATION DATA

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FIGURE No. 93

DATE Mar/28/2022

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	Test Boring	Depth (ft)	Classification					LL	PL	PI
	Lot 340	4.0		SILTY SAND(SM)					NP	NP
X	Lot 345	9.0			SILTY SAN	ID(SM)		29	26	3
4	Lot 348	4.0		SILTY SAND(SM)				27	22	5
*	Lot 353	9.0		SILTY SAND(SM)				NP	NP	NP
	Test Boring	Depth (ft)	%Gravel	0/ Cand	%Silt	%Clay				

	Lest Boring	Depth (ft)	%Gravel	%Sand	%Silt	%Clay
•	Lot 340	4.0	0.0	76.4	23	3.6
	Lot 345	9.0	0.0	85.6	14	.4
A	Lot 348	4.0	0.0	68.4	31	.6
*	Lot 353	9.0	0.0	84.0	16	6.0

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Geotechnical Materials Testing Civil, Planning SOIL CLASSIFICATION DATA

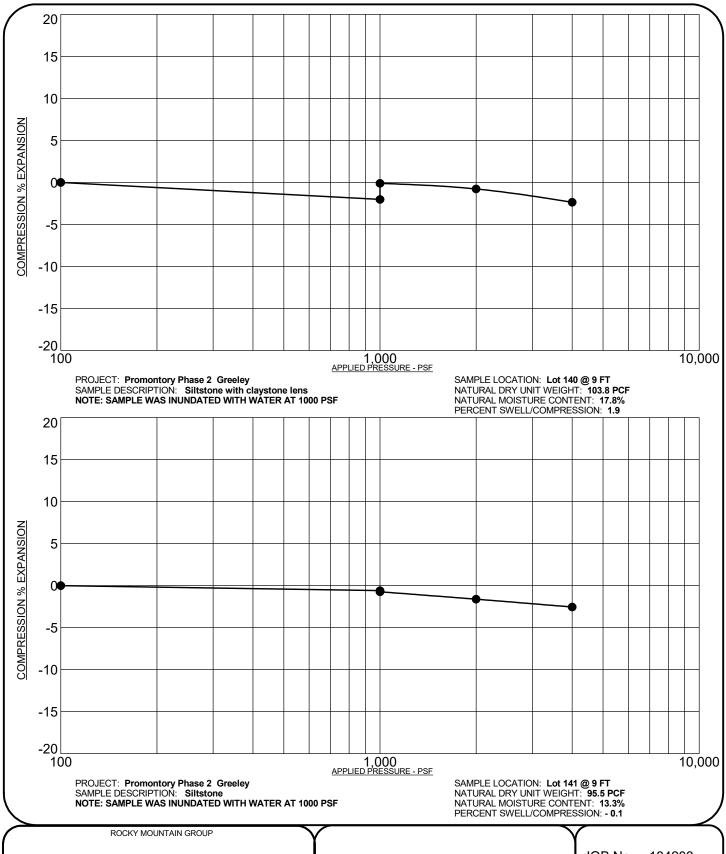
JOB No. 184203

FIGURE No. 94

DATE Mar/28/2022

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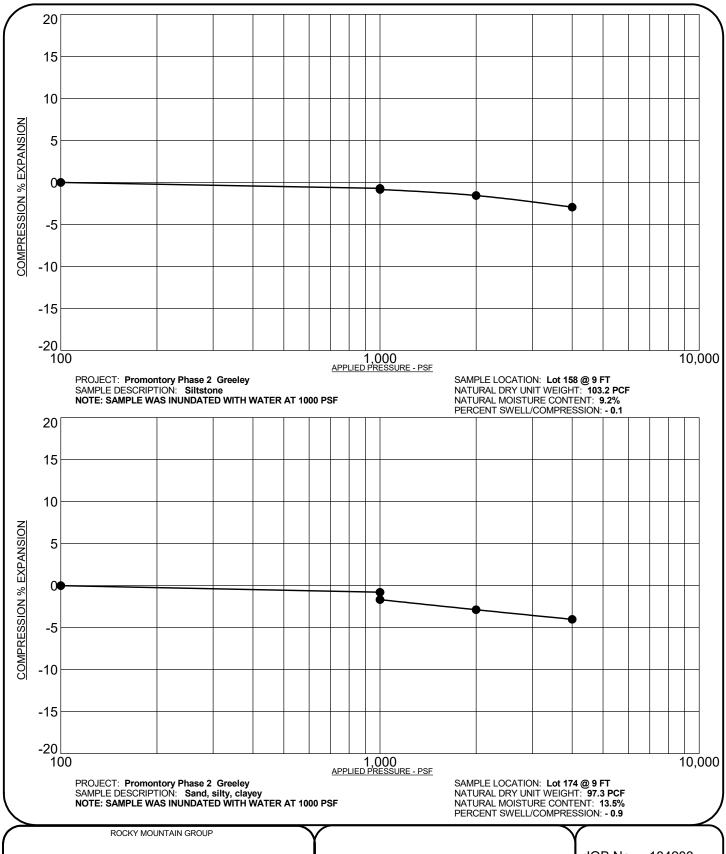
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SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 95





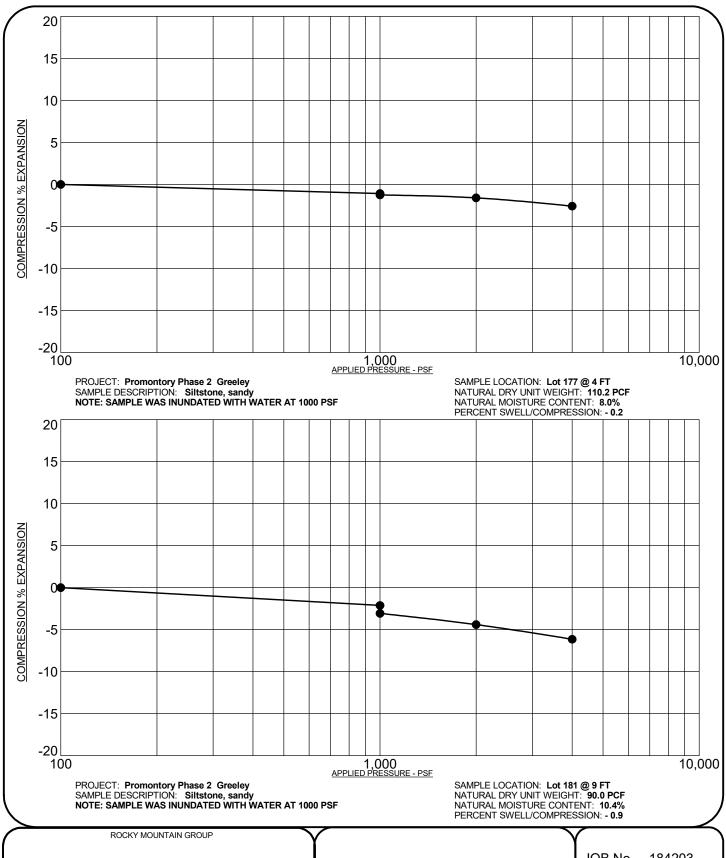
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SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 96





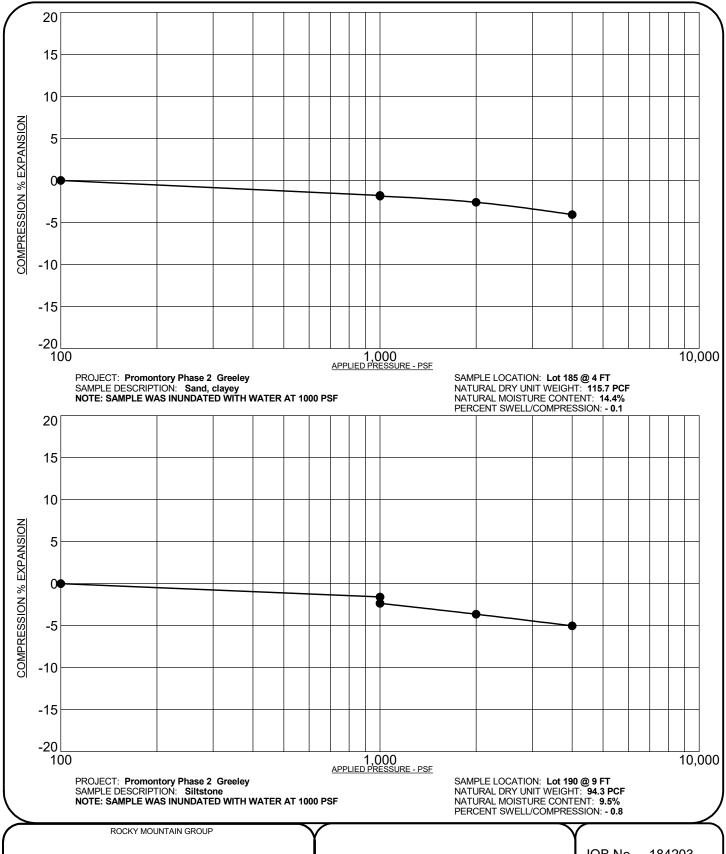
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SWELL/CONSOLIDATION **TEST RESULTS**

JOB No. 184203

FIGURE No. 97





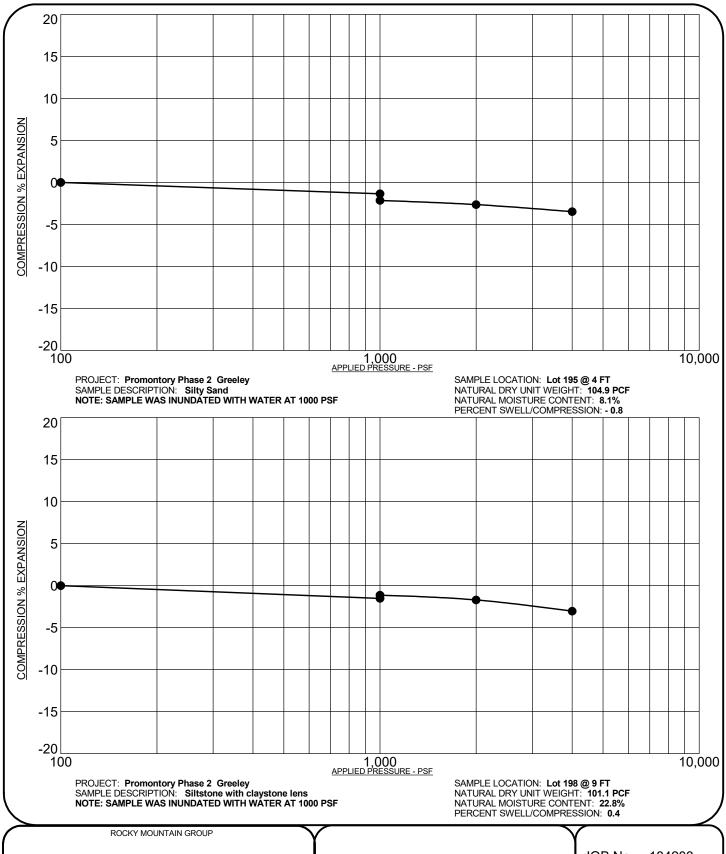
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SWELL/CONSOLIDATION **TEST RESULTS**

JOB No. 184203

FIGURE No. 98





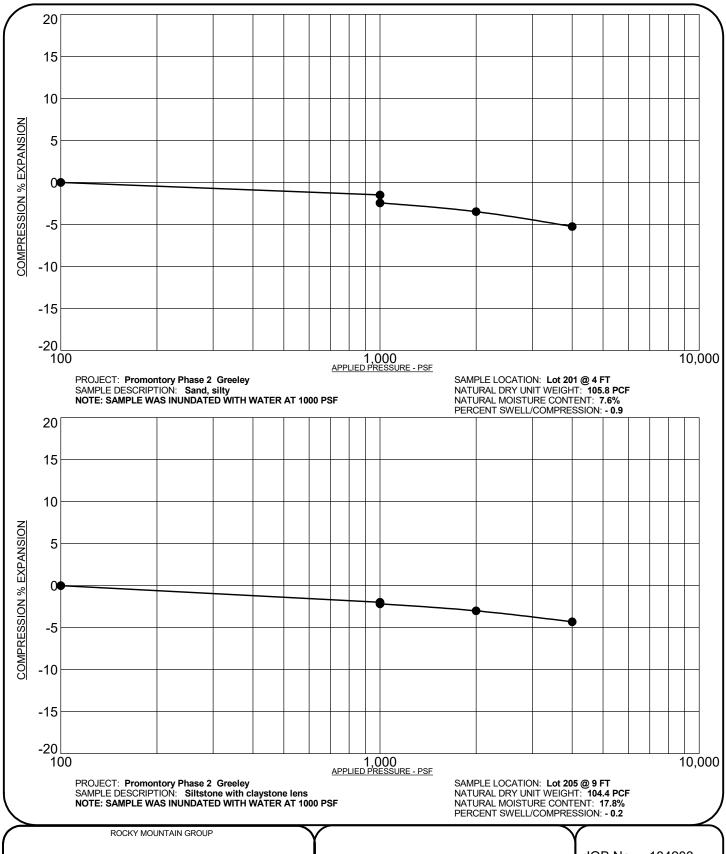
Materials Testin Civil, Planning SWELL/CONSOLIDATION TEST RESULTS

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FIGURE No. 99

DATE Mar/28/2022

Engineers / Architects





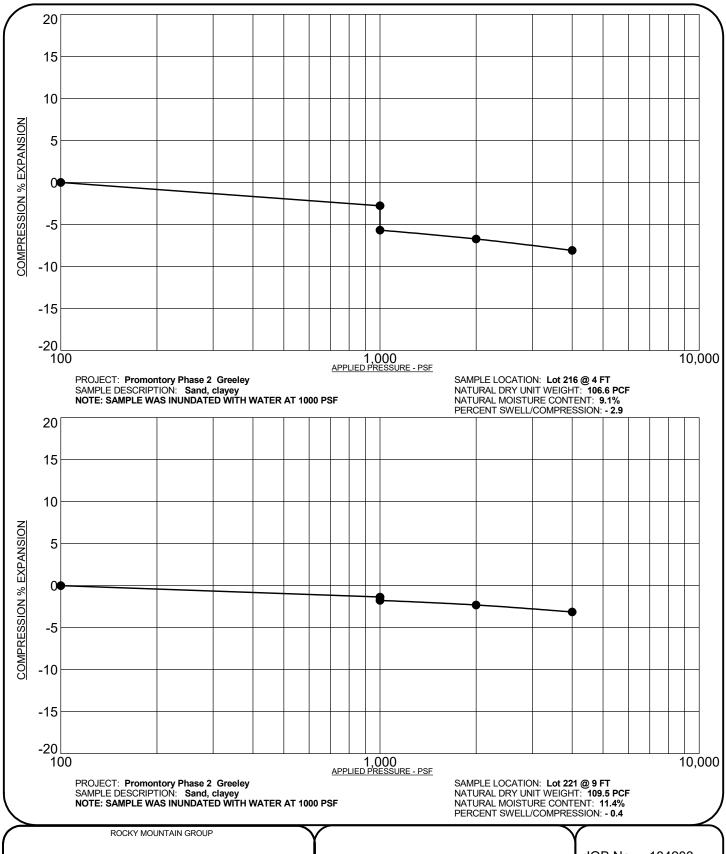
Engineers / Architects

Colorado Springs: (Corporate Office)
2910 Austin Bluffs Parkway
Colorado Spings, CO 80918
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SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 100





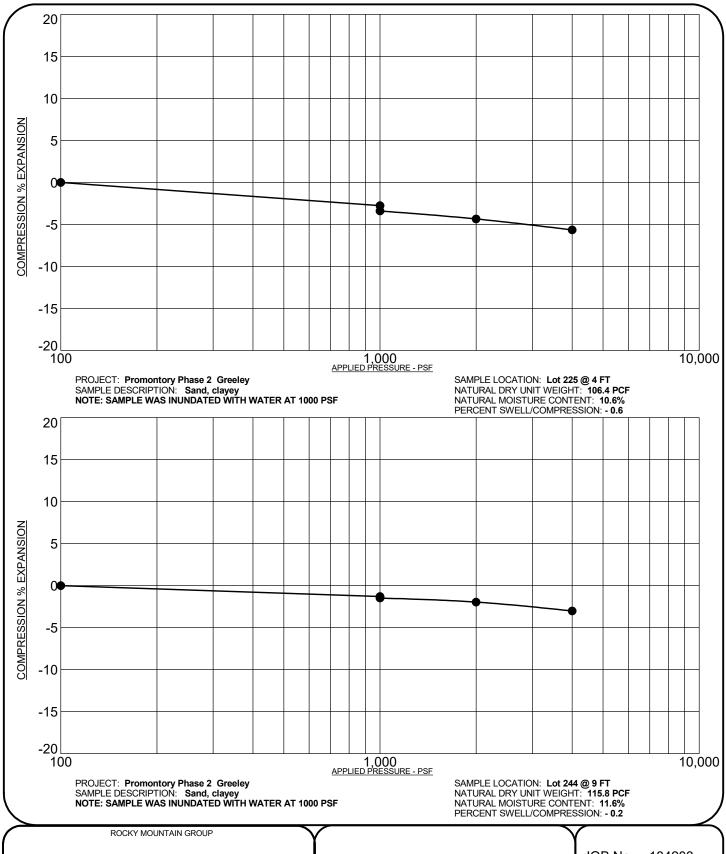
Engineers / Architects

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SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 101





Engineers / Architects

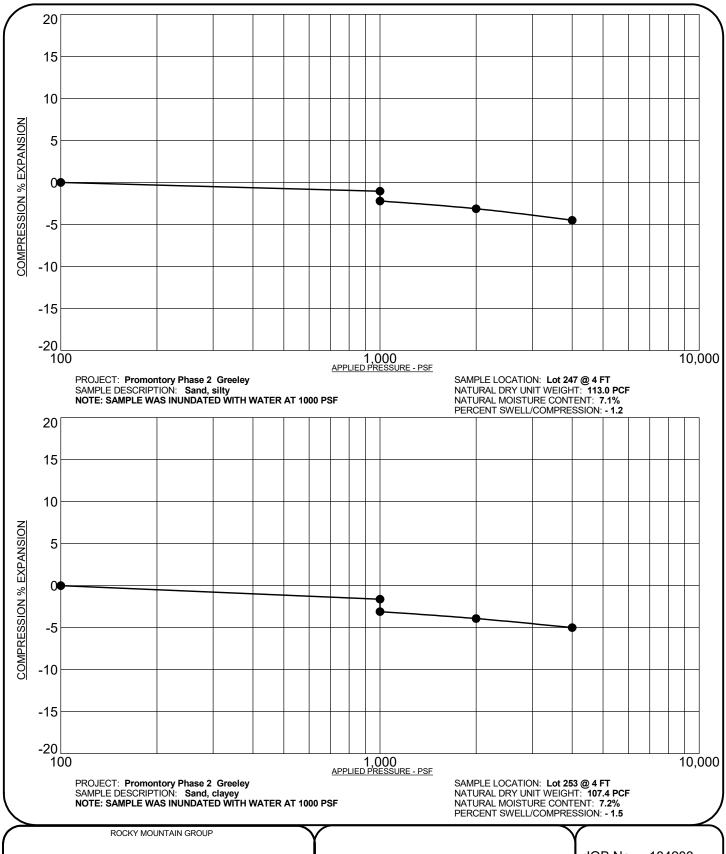
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SWELL/CONSOLIDATION TEST RESULTS

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FIGURE No. 102





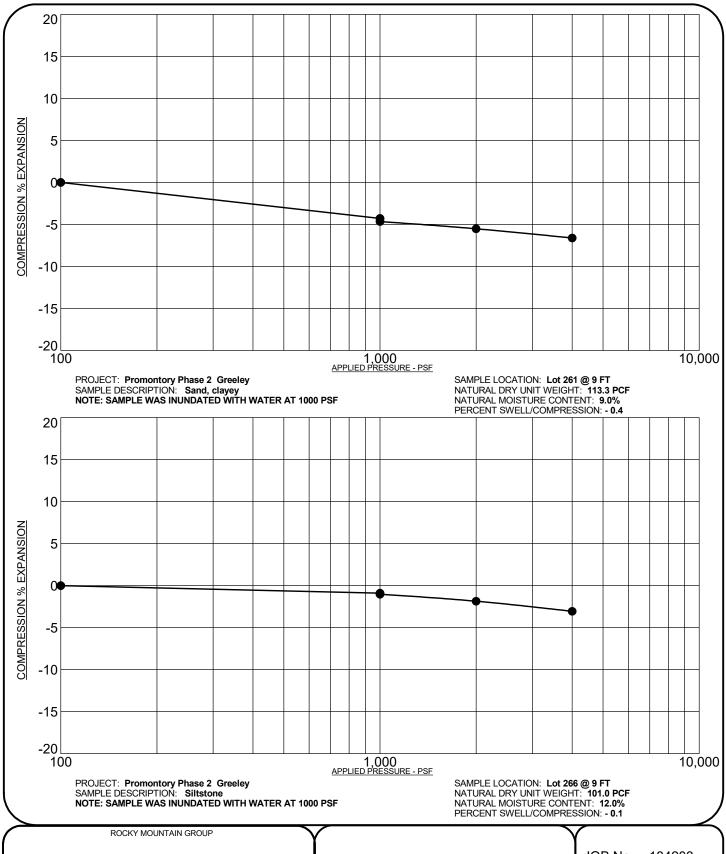
Engineers / Architects

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SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 103

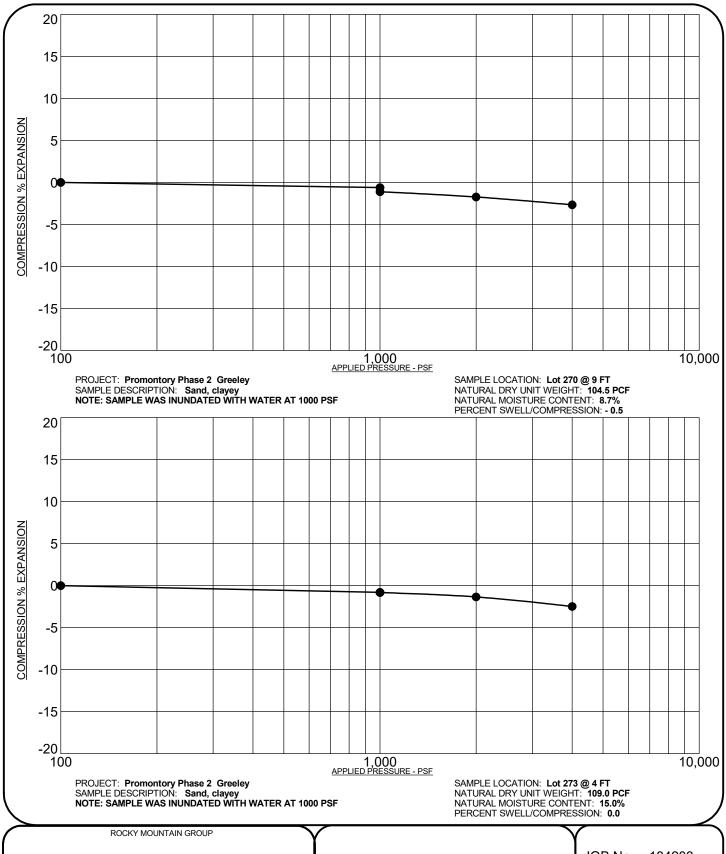




SWELL/CONSOLIDATION **TEST RESULTS**

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FIGURE No. 104

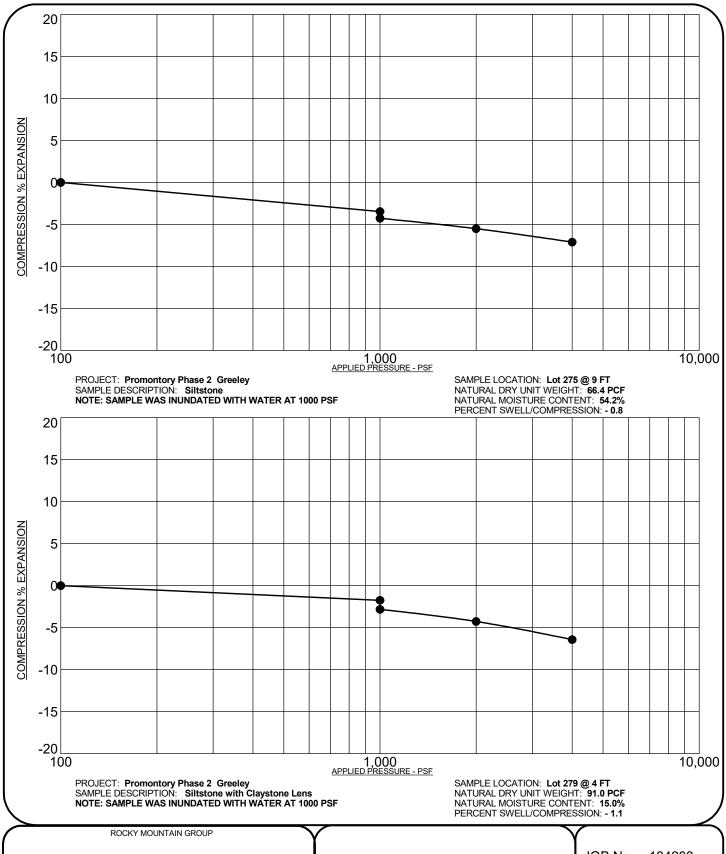




SWELL/CONSOLIDATION **TEST RESULTS**

JOB No. 184203

FIGURE No. 105

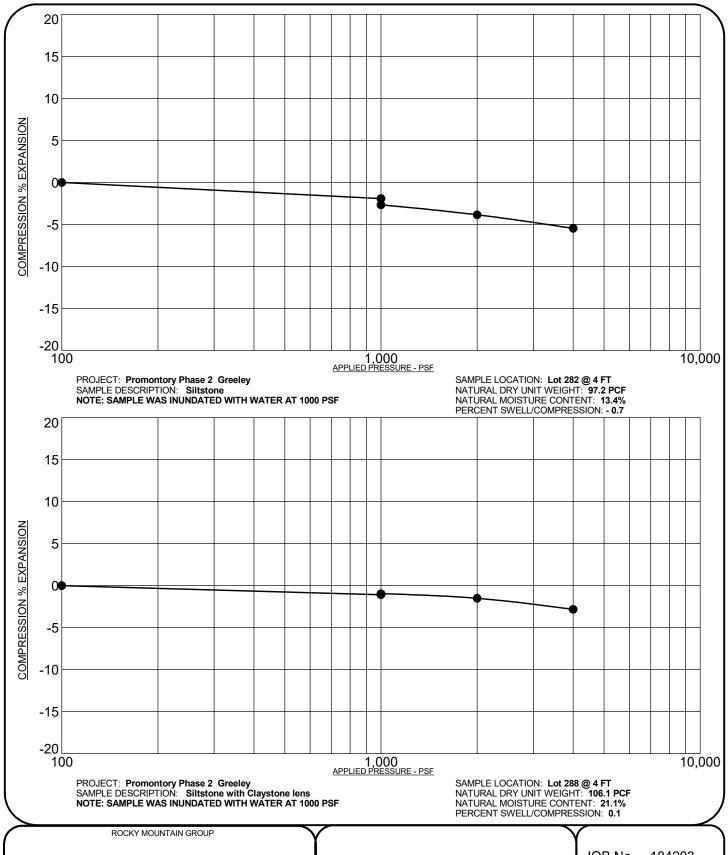




SWELL/CONSOLIDATION **TEST RESULTS**

JOB No. 184203

FIGURE No. 106





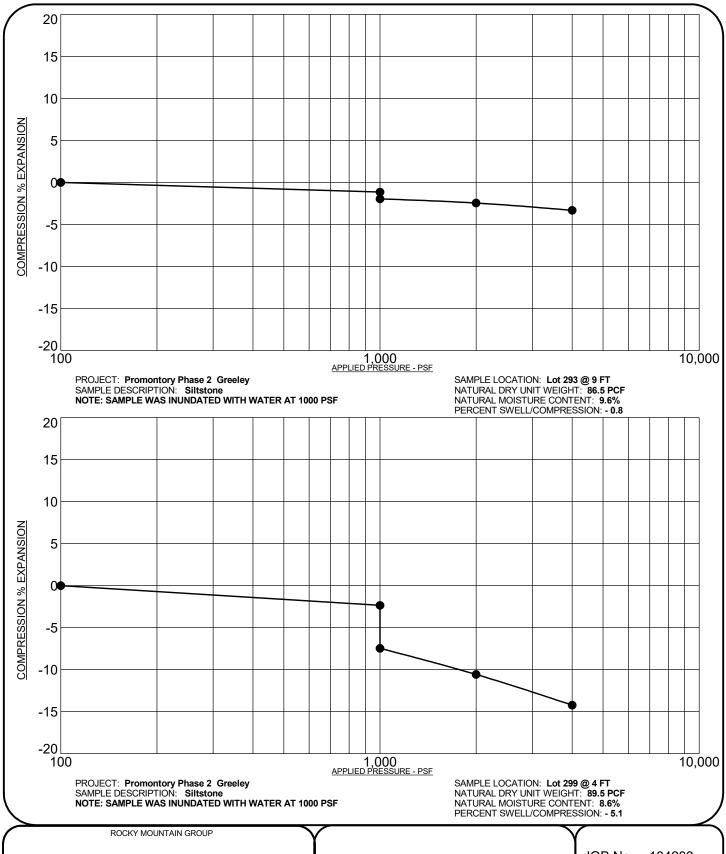
Engineers / Architects

Colorado Springs: (Corporate Office)
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SWELL/CONSOLIDATION TEST RESULTS

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FIGURE No. 107





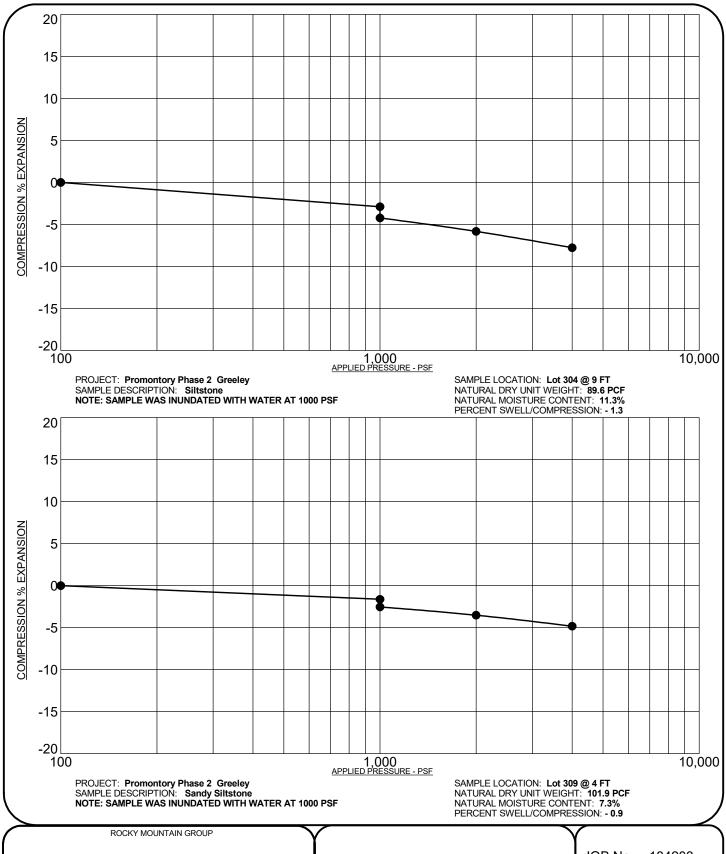
Engineers / Architects

Colorado Springs: (Corporate Office)
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FIGURE No. 108





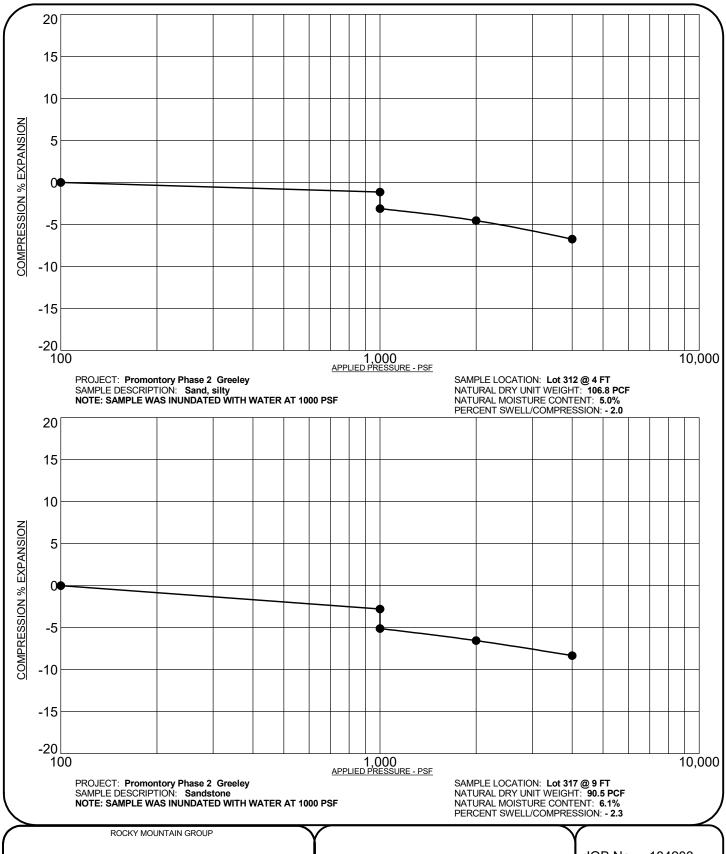
Engineers / Architects

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SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 109





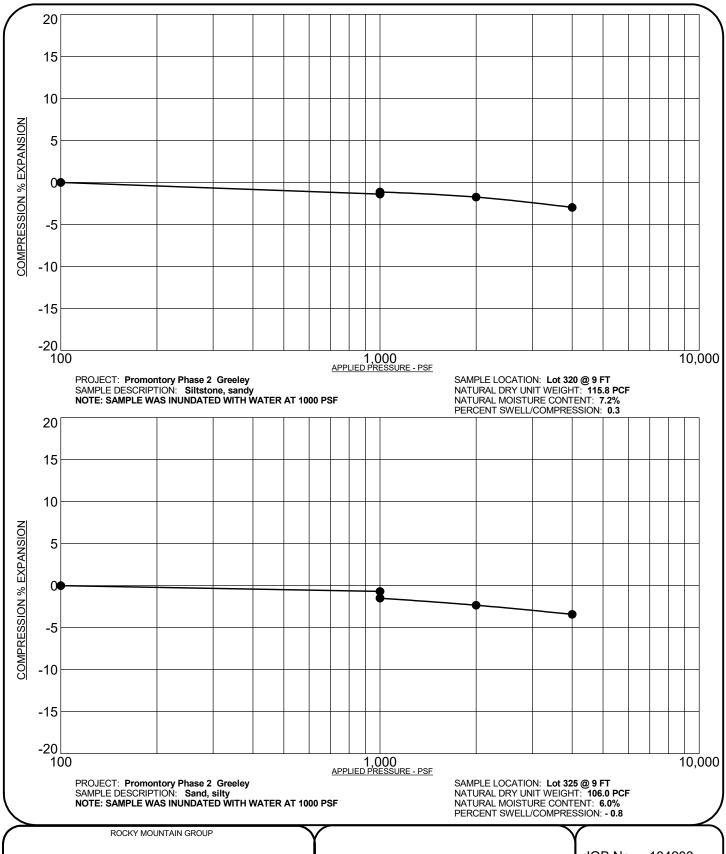
Engineers / Architects

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SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 110





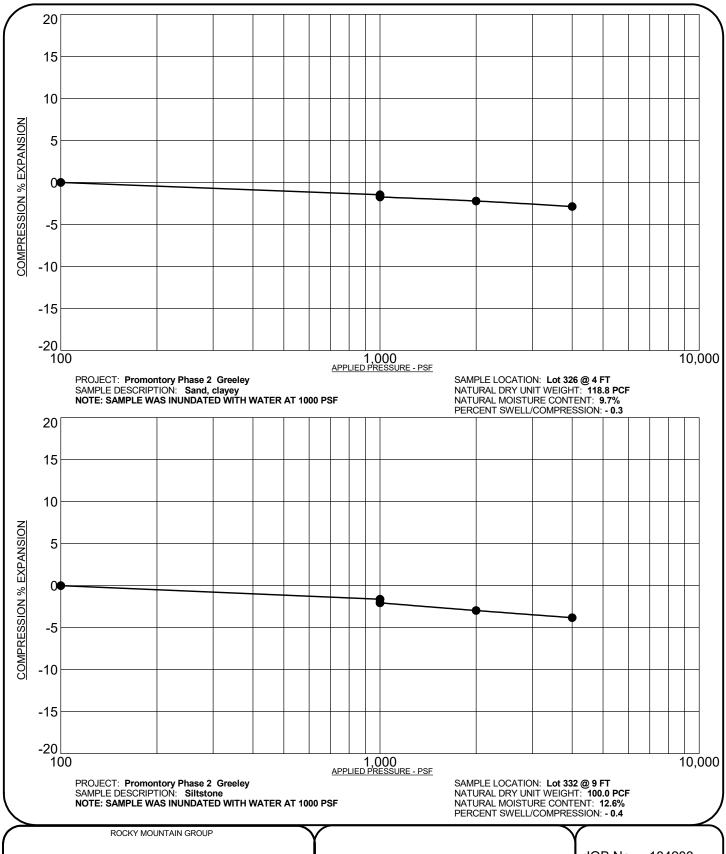
Engineers / Architects

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SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 111

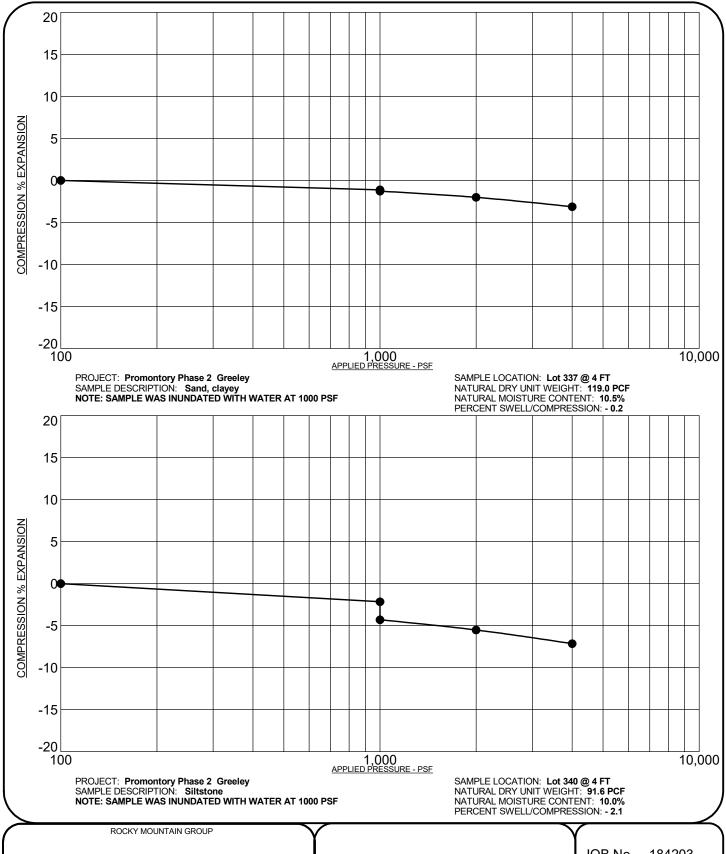




SWELL/CONSOLIDATION **TEST RESULTS**

JOB No. 184203

FIGURE No. 112





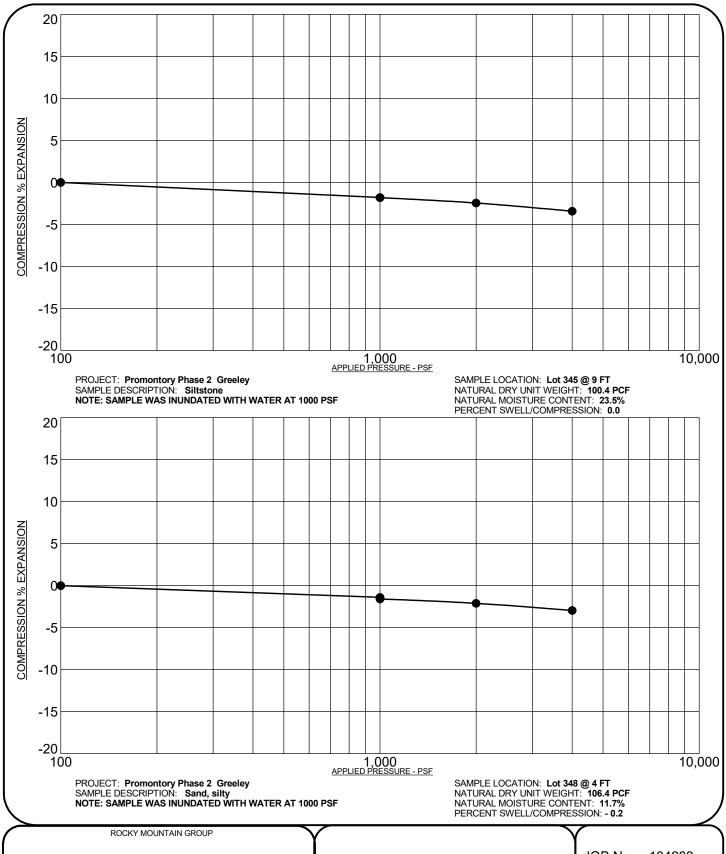
Engineers / Architects

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SWELL/CONSOLIDATION **TEST RESULTS**

JOB No. 184203

FIGURE No. 113





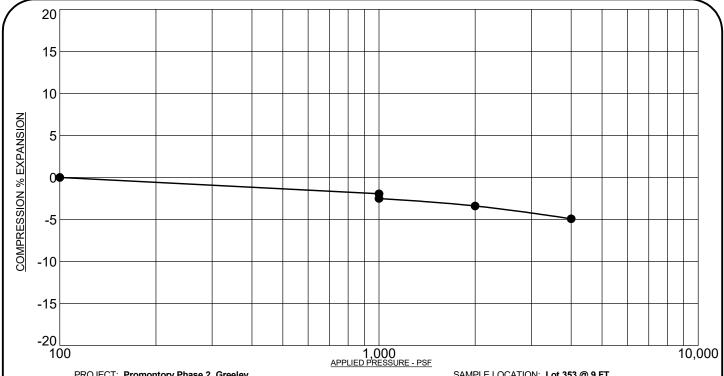
Materials Testir Civil, Planning SWELL/CONSOLIDATION TEST RESULTS

JOB No. 184203

FIGURE No. 114

DATE Mar/28/2022

Engineers / Architects



PROJECT: Promontory Phase 2 Greeley SAMPLE DESCRIPTION: Siltstone

NOTE: SAMPLE WAS INUNDATED WITH WATER AT 1000 PSF

SAMPLE LOCATION: Lot 353 @ 9 FT NATURAL DRY UNIT WEIGHT: 108.2 PCF NATURAL MOISTURE CONTENT: 9.1% PERCENT SWELL/COMPRESSION: - 0.6

ROCKY MOUNTAIN GROUP

Architectural Structural Forensics



Geotechnical Materials Testin Civil, Planning

Engineers / Architects

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SWELL/CONSOLIDATION TEST RESULTS

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FIGURE No. 115