Ward 3 Recreation

Various Projects at Power Center Sports Complex

3210 Power Center Parkway
Lake Charles, Louisiana 70615

Index to Drawings

1. ALL WORK SHALL COMPLY WITH FEDERAL, STATE, AND LOCAL LAWS.
2. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THE EXTENT, NATURE, AND SCOPE OF WORK DESCRIBED IN THESE DOCUMENTS AND IS RESPONSIBLE FOR PROVIDING ALL LABOR, MATERIALS, EQUIPMENT, TRANSPORTATION, DELIVERY, HANDLING, SERVICES, SUPERVISION AND QUALITY CONTROL REQUIRED TO EXECUTE ALL AS WORK AS DELINEATED IN THESE DRAWINGS UNLESS AS SPECIFICALLY NOTED AS OUTSIDE OF THE CONTRACT.
3. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL WORK WITH ALL TRADES INCLUDING THOSE OPERATING UNDER SEPARATE CONTRACTS WITH THE OWNER.
4. ALL WORK SHALL BE PERFORMED BY SKILLED AND QUALIFIED WORKERS IN ACCORDANCE WITH ESTABLISHED "BEST PRACTICES."
5. CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH ALL EXISTING CONDITIONS AND ANY DIFFICULTIES OR RESTRICTIONS AFFECTING THE EXECUTION OF THE CONTRACT PRIOR TO SUBMITTING A PROPOSAL.
6. ALL PRODUCTS, MATERIALS, AND EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFIC INSTRUCTIONS AS WELL AS IN COMPLIANCE WITH ALL APPLICABLE CODES.
7. JOB SAFETY IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
8. CONTRACTOR SHALL VERIFY LOCATIONS, LEVELS, DISTANCES, AND FEATURES THAT MAY AFFECT THE WORK. SHOULD EXISTING CONDITIONS DIFFER FROM THOSE SHOWN OR INDICATED, OR IF IT APPEARS THAT THESE PLANS, STANDARD SPECIFICATIONS, AND SPECIAL PROVISIONS DO NOT ADEQUATELY DETAIL THE WORK TO BE DONE, CONTRACTOR SHALL NOTIFY THE ARCHITECT PRIOR TO CONTINUING WITH ANY RELATED WORK. NO ALLOWANCE WILL BE MADE IN HIS BEHALF FOR ANY EXTRA EXPENSE RESULTING FROM FAILURE OR NEGLECT IN DETERMINING THE CONDITIONS UNDER WHICH WORK IS TO BE PERFORMED. NOTED DIMENSIONS TAKE PRECEDENCE OVER SCALE.
9. CONTRACTOR SHALL PROVIDE ANY AND ALL NECESSARY TEMPORARY SHORING, BRACING, AND BARRICADES NECESSARY TO INSURE SAFE EXECUTION OF CONSTRUCTION AND DEMOLITION.
10. CONTRACTOR AGREES TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR THE JOB SITE DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY; THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS; AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY, AND HOLD THE CITY REPRESENTATIVES HARMLESS FROM ANY AND ALL LIABILITY, REAL AND/OR ALLEGED, IN CONJUNCTION WITH THE PERFORMANCE OF THIS PROJECT.
11. CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SUITABLE TRASH CONTAINERS AND TIMELY TRASH REMOVAL FROM THE SITE CONSISTENT WITH THE STAGE OF CONSTRUCTION. WORK AREA SHALL BE KEPT CLEAN, SAFE, AND ORDERLY AT ALL TIMES.
12. ALL DIMENSIONS, ANGLES, ELEVATIONS, CONDITIONS, AND PHYSICAL CONFIGURATIONS RELATIVE TO EXISTING CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING WORK OR ORDERING MATERIAL. IF ANY DIFFERENCES ARE FOUND, THE ARCHITECT SHOULD BE NOTIFIED IMMEDIATELY.
13. DO NOT SCALE DRAWINGS! WRITTEN DIMENSIONS SHALL GOVERN. NOTIFY ARCHITECT FOR ANY DIMENSIONS NOT SHOWN.
14. THESE DRAWINGS ARE INTENDED TO DEFINE THE GENERAL DESIGN AND SCOPE OF THE WORK REQUIRED TO COMPLETE THE PROJECT. IT IS THE INTENT OF THESE DOCUMENTS TO PROVIDE FOR COMPLETE FINISHED WORK AND OPERATING SYSTEMS. OMISSIONS FOUND IN THESE DRAWINGS DO NOT RELIEVE THE CONTRACTOR OF SUCH RESPONSIBILITIES AS IMPLIED BY THE SCOPE OF WORK UNLESS SPECIFICALLY NOTED.
15. ALL WORK WILL BE SUBJECT TO REVIEW AND ACCEPTANCE BY THE OWNER UPON COMPLETION OF THE WORK.

General Notes

PROJECT SCOPE:

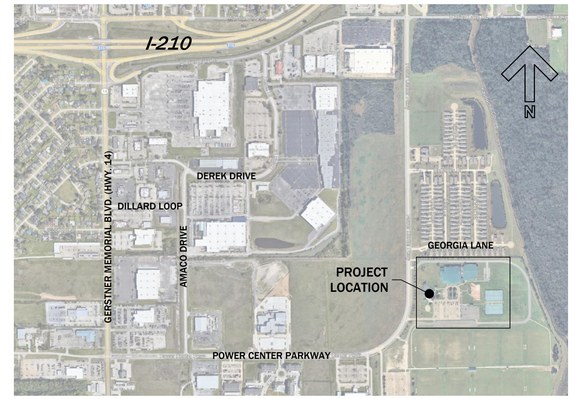
THE PROJECT CONSISTS OF VARIOUS IMPROVEMENTS THROUGHOUT THE SPORTS COMPLEX INCLUDING:

1. COVERED SEATING AREAS AT TENNIS COURTS
2. INTERIOR RENOVATION TO ACTIVITY ROOM TO PROVIDE A COFFEE AREA
3. PORT COCHERE AT FRONT ENTRANCE
4. ADVENTURE COVE PARK RESURFACING
5. A FLAT OUTDOOR SKATING RINK AND COVERED SEATING AREAS AT SKATE PARK
6. A NEW 1,200 SQ. FT. STORAGE BUILDING.
7. MAIN BUILDING DRAINAGE IMPROVEMENTS - ALTERNATE #1
8. SOUTH BALL FIELD DRAINAGE IMPROVEMENTS - ALTERNATE #2

APPLICABLE CODES AT NEW CONSTRUCTION:

INTERNATIONAL BUILDING CODE (IBC) - 2021
NFPA 101 LIFE SAFETY CODE (NFPA) - 2015
INTERNATIONAL PLUMBING CODE (IPC) - 2021
INTERNATIONAL MECHANICAL CODE (IMC) - 2021
NATIONAL ELECTRIC CODE (NEC) - 2020
AMERICAN WITH DISABILITIES ACT (ADA) - 2010

Project Overview & Code Research



Vicinity Map

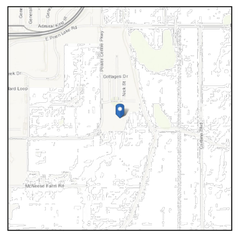
ASCE
AMERICAN SOCIETY OF CIVIL ENGINEERS

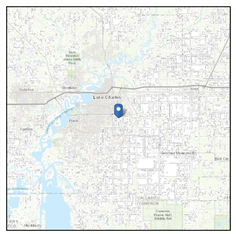
ASCE Hazards Report

Address:
No Address at This Location

Standard: ASCE/SEI 7-22
Risk Category: II
Soil Class: Default

Latitude: 30.189782
Longitude: -93.167464
Elevation: 14.814425175505159 ft (NAVD 88)





Wind

Results:

Wind Speed	130 Vmph
10-year MRI	76 Vmph
25-year MRI	88 Vmph
50-year MRI	98 Vmph
100-year MRI	108 Vmph
300-year MRI	121 Vmph
700-year MRI	130 Vmph
1,700-year MRI	138 Vmph
3,000-year MRI	144 Vmph
10,000-year MRI	155 Vmph
100,000-year MRI	175 Vmph
1,000,000-year MRI	193 Vmph

Data Source: ASCE/SEI 7-22, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2
Date Accessed: Fri Apr 11 2025

https://ascehazardtool.org/ Page 1 of 3 Fri Apr 11 2025

Windspeed

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Ward 3 Recreation
Various Projects at Power Center Sports Complex
3210 Power Center Pkwy
Lake Charles, Louisiana 70607

stamp:

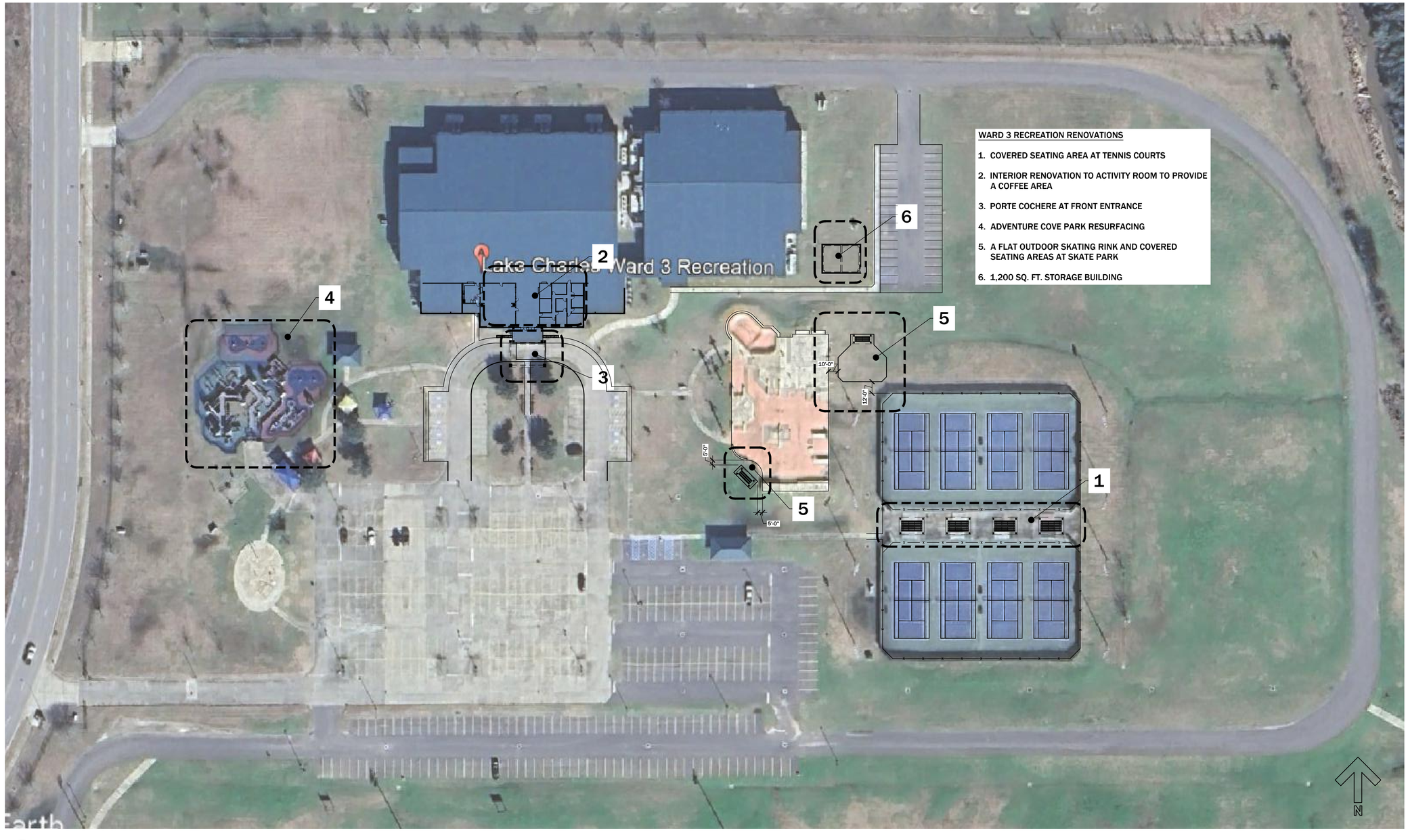
JEFFERY M. KUDLA REG. NO. 31971 STATE OF LOUISIANA REGISTERED ARCHITECT

phase:
For Construction
Construction Bid Documents

project #: 2503
date issued: 08/11/2025
drawn by: kr
checked by: jk
revisions:

Title Sheet and General Information

T 1.0



WARD 3 RECREATION RENOVATIONS

1. COVERED SEATING AREA AT TENNIS COURTS
2. INTERIOR RENOVATION TO ACTIVITY ROOM TO PROVIDE A COFFEE AREA
3. PORTE COCHERE AT FRONT ENTRANCE
4. ADVENTURE COVE PARK RESURFACING
5. A FLAT OUTDOOR SKATING RINK AND COVERED SEATING AREAS AT SKATE PARK
6. 1,200 SQ. FT. STORAGE BUILDING

1 KEY PLAN
SCALE: 1" = 50'-0"

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Ward 3 Recreation
Various Projects at Power Center
Sports Complex
3210 Power Center Pkwy
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stamp:

JEFFERY M. KUDLA
REG. NO. 9814
STATE OF LOUISIANA
REGISTERED ARCHITECT

phase:
For Construction
Construction Bid Documents

project #: 2503
date issued: 08/11/2025
drawn by: kr
checked by: jk
revisions:

Site Plan

A 1.0

1 PARTIAL SPECIFICATIONS

I. CODES AND SPECIFICATIONS

A. GENERAL BUILDING CODE...2021 IBC

B. CONCRETE CODES...318-19

C. STRUCTURAL STEEL CODES...AISC 360-16

D. LIGHT GAUGE STEEL CODES...AISI S100-16

E. WOOD CODES - N/A

F. MASONRY CODES - N/A

G. WIND & SEISMIC CODES - ASCE 7-16
All Codes and Specifications listed above shall include all amendments and addenda in force at the date of the contract documents.

H. CONFLICTS IN STRUCTURAL REQUIREMENTS
Where conflict exists between the various publications as specified herein, the strictest requirements of the various publications shall govern unless noted otherwise. Where conflict exists among the various parts of the Structural Contract Documents, Structural Drawings, General Notes, Specifications) the strictest requirements shall govern.

II. TYPICAL DETAILS

Details labeled "Typical Details" on the drawings shall apply to all situations occurring on the project that are the same or similar to those specifically detailed. Such details shall apply whether or not they are keyed in at each location. Questions regarding applicability of Typical Details shall be determined by the Engineer.

III. DESIGN CRITERIA

A. DEAD LOADS
1. Dead Loads. Dead load materials assumed in the design are shown on the Architectural and Structural Drawings. Any changes in construction materials from those shown on the architectural or structural drawings shall be reported by the General Contractor to the Structural Engineer for verification of load carrying capacity of the structure.

B. LIVE LOADS
Live loads assumed for each area of the structure are shown on the structural drawings. ive loads have been reduced on members supporting the roof based on their tributary area in accordance the building code.

C. BUILDING MOVEMENT AND DEFLECTIONS
1. Roof members. Span over 240 for total loads.
2. Floor members. span/360 for live loads.
3. Member supporting plaster or masonry. Span over 600 and less than 5/8" for total loads.
4. Drift - Mean roof height over 400 for 10 year wind loads.

IV. FOUNDATION - GENERAL

A. GEOTECHNICAL REPORT
Foundation design is based on the geotechnical investigation provided by Daniel J. Holder, P.E., Inc. (DJH File 07-059) dated August 17, 2007.

B. FOUNDATION APPROVAL AND INSPECTION BY AUTHORIZED INSPECTOR
Unless authorized otherwise by the Owner or Architect, the General Contractor shall notify the Geotechnical Engineer or other authorized inspector for review of foundation bearing surface, inspection of foundation installation, and foundation installation methods and shall not place concrete prior to inspector's approval.

C. FOUNDATION REINFORCING STEEL INSPECTION BY STRUCTURAL ENGINEER
1. The Contractor shall notify the Engineer 72 hours in advance of any major foundation pour.
2. The Contractor shall not pour any foundation concrete without inspection and approval of all reinforcing steel placement by the Structural Engineer or Owner's special inspector.

D. SLAB-ON-GRADE CONSTRUCTION: N/A

V. FOUNDATION CONSTRUCTION

A. SPECIFICATION
Pile Cap construction shall conform to the requirements of the Standard Building Code Requirements for structural concrete (ACI 318) published by the American Concrete Institute, except as modified hereafter by these General Notes, the structural drawings, or the specifications.

B. HELICAL PILE
1. Pile design shall be in accordance with project geotechnical report.
2. Helical piles shall be designed and manufactured in accordance with the current international building code (IBC) adopted by the local jurisdiction.
3. Helical pile shall be recognized by ICC and the manufacturer shallhold a current ICC-ES ESR report showing compliance with AC358and current IBC adopted by the local jurisdiction. Alternativelymanufacturer can supply steel mill certificates, welding procedure specifcations and welder certifications.
4. Helical piles shall be designed by Engineer registered in Louisiana to meet specified loads and acceptance criteria as shown on the structural drawings. The calculations and drawings required from the contractor or engineer shall bear the seal of a professional engineer licensed to practice in the state of Louisiana, and be submitted to the architect for review and acceptance.
5. Dimensions of the central shaft and the number, size, spacing, and thickness of the helical bearing plates shall be designed and fabricated to support the specified design loads.
6. Only round central pile shafts will be allowed.
7. The overall length and minimum and maximum installation torquesshall be specified by the helical pile manufacturer, the minimum installation torque shall be high enough to achieve the requiredbearing capacity, including a safety factor of 2. The maximum installation torque shall not exceed the allowable torsion capacity of the pile shafts. See plans for minimum tip depth.
8. Helical piles shall be designed and manufactured to resist allstresses induced by installation.
9. The helical pile attachment (pile cap) shall distribute the design load to the concrete foundation such that the concrete bearing stress does not exceed those in the ACI building code and the stresses in the steel plates/welds does not exceed AISC allowable stresses for steel members.
10. Each helical pile and associated coating and/or auxiliary corrosion protection system shall be designed to meet the forgoing requirements for a life of 50 years.
11. See plans for pile locations. Piles not specifically located on theplan shall be located on centerline of column above. Where no column occurs, locate on centerline of wall or beam. Locations of piles shall not be changed without written approval from the structural engineer.
12. Existing conditions and underground obstructions shall be confirmed by the pile installer. Probing or scanning may be necessary to locate underground obstructions. Report any unforeseen obstructions to the structural engineer.

C. TEMPLATES FOR ANCHOR BOLTS
Vertical reinforcing steel and anchor bolts for column base plates shall be accurately set in footing and pedestal tops using 3/4" plywood or 1/8" steel templates.Such templates shall be detailed and submitted with concrete reinforcing steel shop drawings.

VI CONCRETE

A. CLASSES OF CONCRETE
All concrete shall conform to the requirements as specified in the table below unless noted otherwise on the drawings:

	Usage	28-day PSI	Slump	Max Agg.	Max w/c	Notes
1	Pile Caps	3500	3"	1½"	.52	

B. CONCRETE MIX DESIGNS
1. Engineer Approval. Concrete mix designs must be submitted aminimum of 15 days prior to the start of the work for Engineer andOwner's testing laboratory approval prior to placement of concrete in the plant or field. Any adjustments in approved mix designs including changes in admixtures must be submitted in writing to the Engineer and Owner's testing laboratory for approval prior to use in the field.
2. Pumped Concrete. Concrete designed to be pumped shall be so noted on the mix designs and shall have mix proportions compatible with the pumping process.
3. Calcium Chloride and/or fly ash is strictly prohibited.

VII. CONCRETE FORMWORK
A. RESPONSIBILITY
The design, construction, and safety of all formwork shall be the responsibility of the General Contractor. All forms, shores, backshores, falsework, bracing, and other temporary supports shall be designed to support all loads imposed including the wet weight of concrete, construction equipment, live loads, lateral loads due to wind and wet concrete imbalance. The Contractor shall also be responsible for determining when temporary supports, shores, backshores, and other bracing may be safely removed.

VIII. CONCRETE FINISH
Refer to the Specifications for concrete finish requirements including concrete surface tolerances (flatness and levelness).

IX. REINFORCING STEEL

A. SPECIFICATION
1. Reinforcing Steel. ASTM A 615 Grade 60 unless noted otherwise on the drawings. Welded Reinforcing Steel - ASTM A 706.
2. Deformed Bar Anchors. ASTM A 496 minimum yield strength 70,000 PSI (reinforcing bars may not be substituted for deformed bar anchorswithout Engineer approval).
3. Welded Wire Fabric. Welded smooth wire fabric, ASTM A 185, yieldstrength 65,000 PSI. Welded deformed wire fabric for, ASTM A 497, yield strength 70,000 PSI. All welded wire fabric shall be furnished inflat sheets only.
4. Post tension cables. ASTM A-416, 1/2"Ø, 270 k.s.i., low relaxation, seven strand. Additionally, all post tension cables shall be obtained from a P.T.I. certified plant.

The Contractor shall submit mill certificates of all reinforcing steel productssigned by producer and by the Contractor.

B. DETAILING AND BAR SUPPORTS
Detailing of and bar supports for reinforcing steel shall be in accordance with the ACI Standard Details and Detailing of Concrete Reinforcement as reported by ACI Committee 315.

C. MANUAL OF CONCRETE PRACTICE
Unless noted otherwise, methods of estimating, detailing, fabricating, placing and contracting for reinforcing materials shall follow the Manual of Standard Practice as published by the Concrete Reinforcing Steel Institute.

D. SHRINKAGE AND TEMPERATURE REINFORCEMENT
Provide shrinkage and temperature reinforcement at right angles to main top and bottom bars for all slabs unless detailed otherwise on the drawings.
See drawings for sizes and spacings.

E. REINFORCING STEEL COVERAGE
Reinforcing steel coverage shall conform to the requirements specified on the plans and specifications. The reinforcing steel detailer shall adjust reinforcing steel cage sizes at intersecting structural members as required to allow clearance for intersectingreinforcing bar layers maintaining minimum specified cover. Cover not indicated on the plans and specifications shall conform to the requirements of ACI 318-II Section 7.7 unless specified otherwise on the drawings.

F. Concrete cover shall be as follows, unless shown otherwise:
1. Concrete cast against earth, 3".
2. Concrete cast against cardboard void boxes, 2".
3. Concrete exposed to weather, 2".
4. All other concrete, 3/4".

X. TILT-UP PANEL CONCRETE: N/A

XI. STRUCTURAL STEEL

A. MATERIAL
1. Hot Rolled Structural Members. All hot rolled steel plates, shapes, sheet piling, and bars shall be new steel conforming to ASTM Specification A6.2. ASTM Specification and Grade
Clearly mark the grade of steel on each piece, with a distinguishing mark visible from floor surfaces, for the purpose of field inspection of proper grade of steel. Unless noted otherwise on the drawings, structural steel shall be as follows:
a. Wide Flange Sections and Channels. A992-50
b. Edge Angles and Bent Plates. A 36.
c. Tube Sections. ASTM A500, Grade C.
d. Baseplates. ASTM A36.
e. Anchor Rods: ASTM F1554-36
f. Connection Material.
(1) Beam, Column Stiffener Plates and Doubler Plates. Beam column stiffener plates and doubler plates shall be the grade of steel to which they are connected (highest grade if more than one grade is used).
(2) All connection material, except as noted otherwise herein or on the drawings, including bearing plates, gusset plates, stiffener plates, filler plates, angles, etc. shall be A36 steel unless a higher or matching grade of steel with the members connected is required by strength or stiffness calculation and provided the resulting sizes are compatible with the connected members.
g. Other Steel.
Any other steel not indicated otherwise shall conform to ASTM A36.

B. CONNECTIONS
1. Typical connection details are indicated on the drawings.
2. Design Procedure.
a. Connection types to be used are indicated on the drawings.
b. The design of all steel connections for the project shall be based on standard AISC LRFD tabulated connections.
3. Design Intent. It is the intention of the plans and specifications that shop connections be welded or bolted and that field connections be bolted, unless detailed otherwise on the drawings.
4. a. All typical beam simple connections shall be standard AISC connections.
b. Seated connections shall not be used unless approved by EOR.
c. Connections shall be designed to resist one half the total uniform load capacity from the table of Uniform Load Constants in the AISC LRFD Manual for given shape, span, and grade of steel.

XII. ANCHOR BOLTS

A. SPECIFICATION
All anchor bolts shall be made from threaded round stock conforming to ASTM F-1554 grade 36 steel unless otherwise detailed on the drawings.

B. NUTS
All nuts with anchor bolts shall be hex head conforming to ASTM Specification A563.

C. WASHERS
Where indicated, base plates shall have plate washers, 1/4" thick, extending minimum 1" from edge of base plate holes on each side with AISC standard holes. Washers shall conform to ASTM A36 steel.

D. TEMPLATES
All anchor bolts set in concrete shall utilize 3/4" thick plywood templates same size as the base plate. Templates shall be detailed on the concrete shop drawings.

XIII. LIGHT GAUGE STEEL

A. All light gauge members shall be designed in accordance with American Iron and Steel Institute.
B. All light gauge metal framing members shall be of the type, size, and gauge as shown on the plans, minimum 43 mil.
C. All posts, joists, and accessories shall be primed with rust inhibitive paint meeting the performance requirements of TT-P-636C, or shall be formed steel having a G-60 galvanized coating conforming to ASTM A924.
D. All members shall meet the requirements of ASTM A607 Class 2 Grade 55 with a minimum yield of 55,000 PSI.
E. Welding, where permitted, is to be done per manufacturer's recommendations on rod type and amperage. Minimum gauge for welding shall be 54 mil. Welds within exterior framing walls shall be touched up with zinc rich primer.
F. All light gauge metal framing shall be installed per manufacturer's recommendations regarding minimum installation standards for bearing, bridging, and bracing.
G. Light gauge member designations are per the 2012 AISI North American specifications.
H. All connections shall be welded, screwed or powder fastened as indicated on these drawings.
Welds- All welded connections shall be performed in accordance with the last edition of the AWS D1.3 specification for welding sheet steel in structures. All welding shall be performed by AWS certified welders. All welds shall be cleaned and coated with rust inhibitive zinc paint.
Screws- #10 self drilling screws manufactured by grabber or HILTI and installed per the fastener manufacturer's specifications. Minimum ½" length for light gage to light gauge connections. (Minimum 1½" length for light gage to timber connections.) Screws shall be spaced at a minimum of ½" between adjacent screws and from metal edges.
Powder Activated Fasteners (P.A.F.)- 0.138" minimum shank diameter P.A.F. manufactured by Ramset or HILTI and installed per the fastener manufacturer's specifications.
Provide minimum 1¼" long P.A.F. for light gage connections to concrete. P.A.F. in concrete shall be spaced a minimum of 4" between adjacent P.A.F. and a minimum of 3" from concrete edges. Minimum P.A.F. embedment in concrete shall be 1⅛".
Provide minimum ½" long P.A.F. with knurled shanks for light gage connections to structural steel. P.A.F. Shall be spaced a minimum1½" between adjacent P.A.F. in structural steel and a minimum of ½" from steel edges. The P.A.F. point shall be driven completely through the back side of the structural steel member.
Masonry Anchors- ¼" diameter x 2" long self-drilling screw anchors manufactured by Ramset (Tapcon) or HILTI (Kwik Con II) and installed per the fastener manufacturer's specifications for light gage connections to concrete masonry.
Drive-in Expansion Anchors (Mushroom Head)- ¼" diameter x 1¼" long Zamac Nailin by Rawl, metal by HILTI or Hammer Set by Ramset and installed per the manufacturer's specifications. Anchors in concrete shall be spaced a minimum of 4" between adjacent anchors and a minimum of 3" from concrete edges. Minimum anchor embedment in concrete shall be 1⅛".
Expansion Anchors- Provide minimum ⅜" diameter Kwik Bolt II Expansion anchors by HILTI or equal, with a minimum 2½" embedment into concrete. Minimum spacing between adjacent expansion anchors to be 5". Expansion anchors shall be located a minimum of 3" from concrete edges. Use oversize washers for attaching light gage with expansion anchors. Install per the manufacturer's specifications for light gage connections to concrete.
J. All members shall be cut squarely for attachment to perpendicular members or slope cut as required for an angular fit against abutting members.
K. Field cutting of light gage members shall be done by sawing or shearing. Torch cutting of light gage members is not permitted.
L. Do not cut or splice light gage framing members unless indicated by these drawings.
M. Do not bear or connect light gage members within twelve inches of the punched openings in the member webs unless the members are reinforced with a minimum 18" long unpunched track or stud at the punch opening. The track or stud reinforcing piece shall be the same size and gage as the punched member. Fasten the reinforcing piece to the member with a minimum of four screws.
N. The light gage framing has been designed to support the loads indicated in the calculations. Additional temporary bracing and shoring shall be provided as required to stabilize the framing and to support construction loads. Temporary bracing shall remain in place until permanent bracing is installed and/or additional construction loads are removed.

XV . NON-SHRINK GROUT FOR BASE PLATES AND BEARING PLATES

A. TYPE
Grout for base plates and bearing plates shall be a non-metallic, shrinkage resistant, premixed, non-corrosive, non-staining product containing Portland cement, silica sands, shrinkage compensating agents, and fluidity improving compounds.
B. SPECIFICATIONS
Non-shrink grout shall conform to Corps of Engineers Specification for Non-Shrink Grout, CE-CRD-C621.
C. COMPRESSIVE STRENGTH
Twenty-eight day compressive strength as determined by grout cube tests, shall be: 7,500 PSI.
D. PLACEMENT
Grout shall be placed in a fluid flowable state under baseplates that have a form built around them for grout confinement. Grout shall be cured according to manufacturer's recommendations.
E. MINIMUM THICKNESS
Minimum thickness of grout under all baseplates and bearing plates shall be 1 1/2", unless specified otherwise on the drawings.

2 POST INSTALLED ANCHOR NOTES

I. ANCHORAGE TO CONCRETE
A. Adhesive Anchors for Cracked and Uncracked Concrete Use:
1. Hilti HIT-HY 200 safe set system with the Hilti HIT-Z rod per ICC ESR-3187
2. Hilti HIT-HY 200 safe set system with Hilti hollow drill bit (te-cd or te-yd) and vc 20/40 vacuum (vc 20-u or vc 40-u) system with HAS-E threaded rod per ICC ESR-3187
3. Hilti HIT-RE 500 v3 safe set system with Hilti hollow drill bit (te-cd or te-yd) and vc 20/40 vacuum (vc 20-u or vc 40-u) with HAS-E threaded rod per ICC ESR-3814
4. Hilti HIT-RE 500 v3 safe set system with hilti roughening tool (te-yrt) with HAS-E threaded rod per ICC ESR-3814 for diamond cored holes.
B. Medium Duty Mechanical Anchors for Cracked and Uncracked Concrete Use:
1. HILTI KWIK HUS EZ and KWIK HUS EZ-I screw anchors per ICC ESR-3027
2. HILTI KWIK BOLT-TZ2 expansion anchors per ICC ESR-1917
3. HILTI KWIK BOLT 3 expansion anchors (uncracked concrete only) per ICC ESR-2302
C. Heavy Duty Mechanical Anchors for Cracked and Uncracked Concrete Use:
1. HILTI HDA undercut anchors per ICC ESR 1546
2. HILTI HSL-3 expansion anchors per ICC ESR 1545

II. REBAR DOWELING INTO CONCRETE
A. Adhesive Anchors for Cracked and Uncracked Concrete Use:
1. HILTI HIT-HY 200 safe set system with hilti hollow drill bit (te-cd or te-yd) and vc 20/40 vacuum (vc 20-u or vc 40-u) system with continuously deformed rebar per ICC ESR-3187
2. HILTI HIT-HY 500 V3 safe set system with hilti hollow drill bit (te-cd or te-yd) and vc 20/40 vacuum (vc 20-u or vc 40-u) system with continuously deformed rebar per ICC ESR-3814
3. HILTI HIT-RE 500 v3 safe set system with hilti roughening tool (te-yrt) with continuously deformed rebar per ICC ESR-3814 in diamond cored holes

III. ANCHORAGE TO SOLID GROUTED MASONRY
A. Adhesive Anchors Use:
1. Hilti HIT-HY 70 masonry adhesive anchoring system (ICC pending).
2. Steel anchor element shall be Hilti HAS-E continuously threaded rod or continuously deformed steel rebar.
B. Mechanical Anchors Use:
Hilti Kwik Bolt-3 expansion anchors per ICC ESR 1385

IV. ANCHORAGE TO HOLLOW / MULTI-WYTHE MASONRY
A. Adhesive Anchors Use:
1. HILTI HIT-HY 70 masonry adhesive anchoring system per ICC ESR-3342.
2. Steel anchor element shall be HILTI HAS-E continuously threaded rod or continuously deformed steel rebar.
3. The appropriate size screen tube shall be used per adhesive manufacturer's recommendation.

V. Anchor capacity used in design shall be based on the technical data published by Hilti or such other method as approved by the structural engineer of record. Substitution requests for alternate products must be approved in writing by the structural engineer of record prior to use. Contractor shall provide calculations demonstrating that the substituted product is capable of achieving the performance values of the specified product. Substitutions will be evaluated by their having an ICC ESR showing compliance with the relevant building code for seismic uses, load resistance, installation category, and availability of comprehensive installation instructions. adhesive anchor evaluation will also consider creep, in-service temperature and installation temperature.

VI. Install anchors per the manufacturer instructions, as included in the anchor packaging.

VII. Overhead adhesive anchors must be installed using the Hilti Profi System.

VIII. The contractor shall arrange an anchor manufacturer's representative to provide onsite installation training for all of their anchoring products specified. The structural engineer of record must receive documented confirmation that all of the contractor's personnel who install anchors are trained prior to the commencement of installing anchors.

IX. Anchor capacity is dependant upon spacing between adjacent anchors and proximity of anchors to edge of concrete. Install anchors in accordance with spacing and edge clearances indicated on the drawings.

X. Existing reinforcing bars in the concrete structure may conflict with specific anchor locations. Unless noted on the drawings that the bars can be cut, the contractor shall review the existing structural drawings and shall undertake to locate the position of the reinforcing bars at the locations of the concrete anchors, by Hilti Ferroscan, GPR, X-Ray, chipping or other means.

3 STRUCTURAL LOADS

ROOF DESIGN LOADS:

DEAD LOADS: PSF:
STANDING SEAM ROOF 1.5
FRAMING 6.5

ROOF LIVE LOADS: 20 Reducible

SNOW LOADS:
0 PSF Ground
Ce = 0.9
I = 1.0
Ct = 1.1

WIND: EXP. C, CAT III
ENCLOSED (GCpi=±0.18)
Vult = 142 MPH
Vasd = 107 MPH

SEISMIC : Ss = 0.084 S1 = 0.05
SITE CLASS D
Sds= 0.089, Sd1 = 0.079
DESIGN CAT B
BASE SHEAR: 0.3k
FORCE RESISTING SYSTEM: ORDINARY MOMENT FRAMES OF STEEL
CS = 0.032
R = 3.5
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

4 ROOF FRAMING NOTES

1. All point loads on roof purlins shall bear within 24" of support. All point loads not addressed on the drawings (i.e. piping) shall not exceed 100 pounds per joist.

2. All lines of bridging and blocking interrupted by roof penetrations shall be terminated at the penetration point(s). Substitute diagonal bridging in the 2 adjacent panels on either side of the penetration. All locations shall be noted on the shop and erection drawings.

3. All roof top units (RE: mechanical) shall be located within in the areas shown.

4. Components and cladding shall be designed for a net unfactored wind loads as tabulated herein, RE: 2/S1.1.

5. Lateral stability for this structure is provided by ordinary moment frames (RE: S3). The stability of the structure shall be the responsibility of the general contractor and the steel erector until all wind resisting elements are in place and accepted in writing by the engineer.

6. Typical roof deck shall be 22 gauge, 16" Battenlock HS by MBCI. RE: Arch for finish. Attachment shall be per manufacturer, but as a minimum: one (1) 24 ga.panel clip each purlin with two (2) #12-14x1" self-tapping hex-head screws.

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GENERAL NOTES AND SPECIFICATIONS

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S1

1 SPECIAL INSPECTIONS

CONCRETE - SECTION 1705.3

SPECIAL INSPECTION	REFERENCED STANDARD	MARK IF REQ'D	CONT./ PERIODIC "C" OR "P"
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	ACI 318 CH. 20, 25.2, 25.3, 26.6.1-26.6.3 IBC 1908.4	[X]	P
2. REINFORCING BAR WELDING:			
a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706;	AWS D1.4; ACI 318: 26.6.4	[ ]	P
b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND	AWS D1.4; ACI 318: 26.6.4		P
c. INSPECT ALL OTHER WELDS.	AWS D1.4; ACI 318: 26.6.4	[ ]	C
3. INSPECT ANCHORS CAST IN CONCRETE	ACI 318: 17.8.2	[X]	P
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.			
a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	ACI 318:17.8.4	[ ]	C
b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 11.a	ACI 318:17.8.2	[X]	P
5. VERIFY USE OF REQUIRED DESIGN MIX	ACI 318: CH. 19, 26.4.3, 26.4.4 IBC 1904.1, 1904.2, 1908.2, 1908.3	[X]	P
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AN DETERMINE THE TEMPERATURE OF THE CONCRETE.	ASTM C172; ASTM C31; ACI 318: 26.4, 26.12	[X]	C
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	ACI 318: 26.5 IBC 1908.6, 1908.7, 1908.8	[ ]	C
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	ACI 318: 26.5.3 - 26.5.5 IBC 1908.9	[X]	P
9. INSPECT PRESTRESSED CONCRETE FOR:			
a. APPLICATION OF PRESTRESSING FORCES; AND	ACI 318: 26.10	[ ]	C
b. GROUTING OF BONDED PRESTRESSING TENDONS	ACI 318: 26.10	[ ]	C
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	ACI 318: 26.8	[ ]	P
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	ACI 318: 11.2	[ ]	P
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER	ACI 318: 11.1.2 (b)	[X]	P
13. NOTE EXCEPTIONS 1,2,3,4 & 5 IN SECTION 1705.3 DISCUSSING FOOTINGS FOR BUILDING THREE STORIES OR LESS, NONSTRUCTURAL SLABS, FOUNDATIONS AND CERTAIN EXTERIOR CONCRETE FEATURES WHEN PLACED ON GRADE.		[ ]	

STEEL-SECTION 1705.2

SPECIAL INSPECTION	REFERENCED STANDARD	MARK IF REQ'D	CONT./ PERIODIC "C" OR "P"
1. STRUCTURAL STEEL: SPECIAL INSPECTION AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES AN DPORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360	AISC 360 CHAPTER N	[X]	PER REF. STANDARD
a. MATERIAL VERIFICATION OF STRUCTURAL STEEL SHALL COMPLY WITH THE REQUIREMENTS OF SECTION 6.1 OF THE CODE OF STANDARD PRACTICE	SECTION 6.1 OF THE CODE OF STANDARD PRACTICE	[X]	
b. WELDING, HIGH STRENGTH BOLTING, AND DETAILS IN ACCORDANCE WITH SECTION N5	AISC 360 SECTION N5	[X]	PER REF. STANDARD
c. STEEL DECK AND HEADED STEEL STUD ANCHOR PLACEMENT AND ATTACHMENT IN ACCORDANCE WITH SECTION N6	AISC 360 SECTION N6	[X]	PER REF. STANDARD
d. CUT SURFACES IN ACCORDANCE WITH SECTION M2.2	AISC 360 SECTION M2.2	[X]	
e. HEATING FOR STRAIGHTENING IN ACCORDANCE WITH SECTION M2.1	AISC 360 SECTION M2.1	[ ]	
f. TOLERANCES FOR FIELD ERECTION IN ACCORDANCE WITH SECTION 7.13 OF THE CODE OF STANDARD PRACTICE	SECTION 7.13 OF THE CODE OF STANDARD PRACTICE	[X]	
2. COLD-FORMED STEEL DECK	SDI/QA/QA		
3. COLD-FORMED STEEL TRUSSES SPANNING 60 FEET OR GREATER		[ ]	

INSPECTION OF FABRICATORS

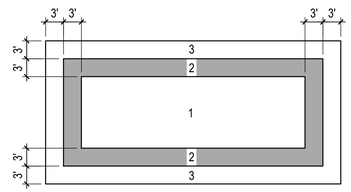
SPECIAL INSPECTION	REFERENCED STANDARD	MARK IF REQ'D	CONT./ PERIODIC "C" OR "P"
1. STRUCTURAL STEEL		[X]	
2. STEEL JOISTS & GIRDERS		[ ]	
3. PRE - CAST CONCRETE		[ ]	
4. PRESTRESSED CONCRETE		[ ]	
5. WOOD CONSTRUCTION (WOOD TRUSSES, WALLS, FLOORS, ROOF ASSEMBLIES)		[ ]	

HELICAL PILE FOUNDATIONS - SECTION 1705.9

SPECIAL INSPECTION	REFERENCED STANDARD	MARK IF REQ'D	CONT./ PERIODIC "C" OR "P"
1. RECORD INSTALLATION EQUIPMENT USED, PILE DIMENSIONS, TIP ELEVATIONS, FINAL DEPTH, FINAL INSTALLATION TORQUE AND OTHER PERTIMENT INSTALLATION INFORMATION AS REQUIRED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE		[X]	C

2 DESIGN ULTIMATE WIND PRESSURES, W FOR COMPONENTS AND CLADDING (psf) - ASCE 7-16

	ZONE*	EFFECTIVE WIND AREA (sqft)	POSITIVE PRESSURE	NEGATIVE PRESSURE
ROOF	1	9	47.8	-42.5
	1	18	47.8	-42.5
	1	36	47.8	-42.5
	2	9	71.7	-64.3
	2	18	71.7	-64.3
	2	36	47.8	-42.5
	3	9	95.7	-127.4
	3	18	71.7	-64.3
	3	36	47.8	-42.5



3 LAP SCHEDULES

TENSION SPLICES (IN.) 3,500 psi CONCRETE NORMAL REBAR				
BAR SIZE	TOP BARS		OTHER BARS	
	A	B	A	B
#3	20	26	16	20
#4	27	35	21	27
#5	33	43	26	33
#6	40	52	31	40
#7	58	75	45	58
#8	66	86	51	66
#9	75	97	58	75
#10	84	109	65	84
#11	93	121	72	93

TENSION SPLICES (IN.) 4,000 psi CONCRETE NORMAL REBAR				
BAR SIZE	TOP BARS		OTHER BARS	
	A	B	A	B
#3	19	25	15	19
#4	25	33	19	25
#5	31	41	24	31
#6	37	49	29	37
#7	54	71	42	54
#8	62	81	48	62
#9	70	91	54	70
#10	79	102	61	79
#11	87	114	67	87

4 STRUCTURAL ABBREVIATIONS

NS - NEAR SIDE OF DETAIL AS PICTURED

FS - FAR SIDE OF DETAIL AS PICTURED

T/D - TOP OF DECK

T.O.P. - TOP OF PARAPET

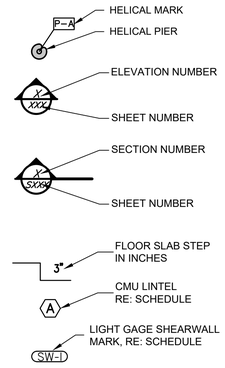
T.O.F. - TOP OF FOOTING

T.O.B. - TOP OF BEAM

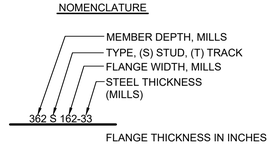
T.O.S. - TOP OF STEEL

PAF - POWDER ACTUATED FASTENERS

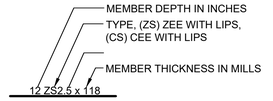
5 STRUCTURAL LEGEND



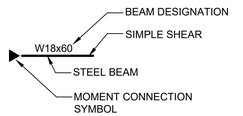
STEEL STUD NOMENCLATURE



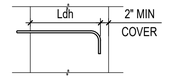
LIGHT GAUGE FRAMING NOMENCLATURE



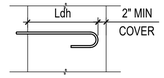
STEEL BEAM NOMENCLATURE



6 TENSION DEVELOPMENT (EMBEDMENT) LENGTH FOR STANDARD END HOOKS (GRADE 60 BARS-NORMAL WEIGHT CONCRETE-GENERAL USE)

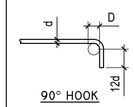


STANDARD 90 HOOK
SIDE COVER >2½"

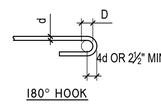


STANDARD 180 HOOK
SIDE COVER >2½"

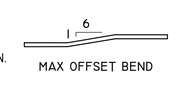
BAR SIZE	f'c=3000 psi	f'c=4000 psi	f'c=5000 psi	f'c=6000 psi
	Ldh	Ldh	Ldh	Ldh
#3	9"	7"	7"	6"
#4	11"	10"	9"	8"
#5	1'-2"	1'-0"	11"	10"
#6	1'-5"	1'-3"	1'-1"	1'-0"
#7	1'-7"	1'-5"	1'-3"	1'-2"
#8	1'-10"	1'-7"	1'-5"	1'-4"
#9	2'-1"	1'-10"	1'-7"	1'-6"
#10	2'-4"	2'-0"	2'-10"	1'-8"
#11	2'-6"	2'-2"	2'-0"	1'-10"



90° HOOK



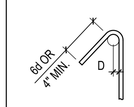
180° HOOK



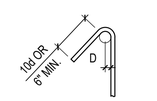
MAX OFFSET BEND

NOTE: FOR 'D' ETC SEE CRSI HANDBOOK GOVERNING EDITION.

PRINCIPAL REINFORCING



TIE OR STIRRUP



SEISMIC STIRRUP/TIE

BAR DIA	MIN "D"
#3	2½"
#4	3"
#5	5½"
#6	4½"
#7	5½"
#8	6"
#9	9"
#10	10½"
#11	11½"

DETAIL NOTES:

1. BENDS SHALL BE MADE COLD.
2. #14 AND #18 BARS SHALL BE BEND-TESTED AND APPROVED PRIOR TO BENDING.

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stamp:

phase:

project #: 2503
date issued: 05/13/2025
drawn by: MB
checked by: KLS
revisions:

SPECIAL INSPECTIONS AND SCHEDULES

S1.1

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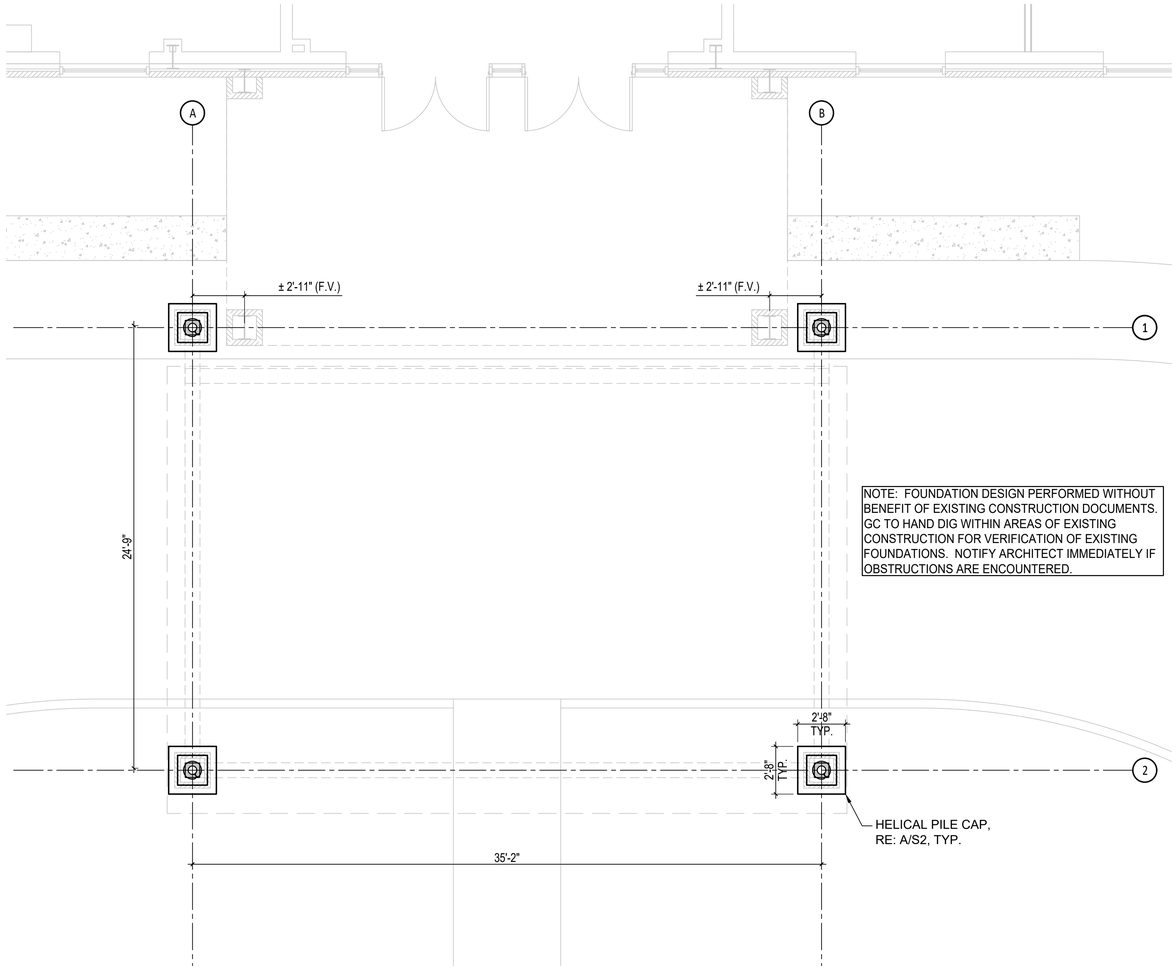
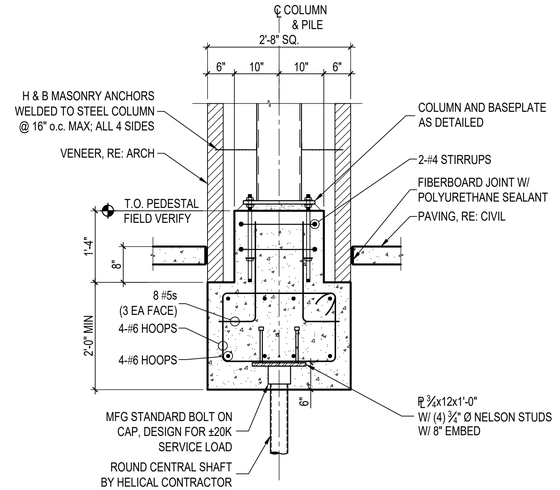


NOTE: FOUNDATION DESIGN PERFORMED WITHOUT BENEFIT OF EXISTING CONSTRUCTION DOCUMENTS. GC TO HAND DIG WITHIN AREAS OF EXISTING CONSTRUCTION FOR VERIFICATION OF EXISTING FOUNDATIONS. NOTIFY ARCHITECT IMMEDIATELY IF OBSTRUCTIONS ARE ENCOUNTERED.

FOUNDATION PLAN
SCALE: 1/4" = 1'-0"



CANOPY COLUMN HELICAL PILE CAP DETAIL

DETAIL A
SCALE:3/4" = 1'-0"

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FOUNDATION PLAN AND DETAIL

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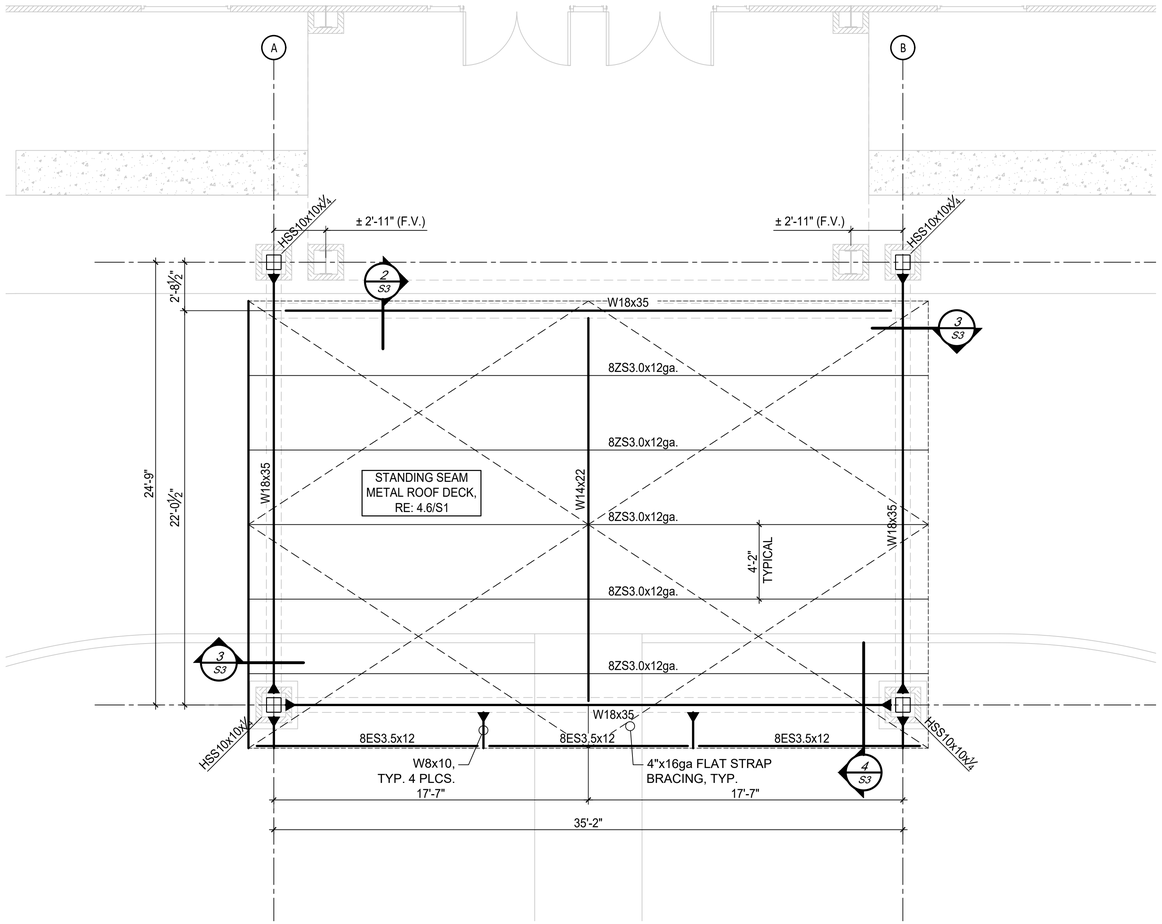
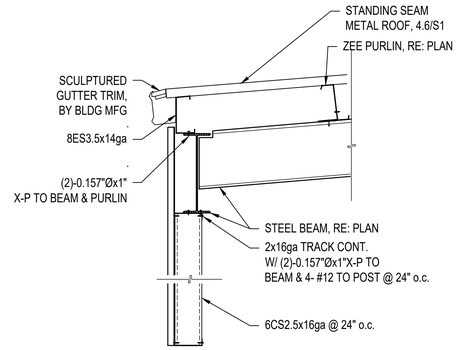
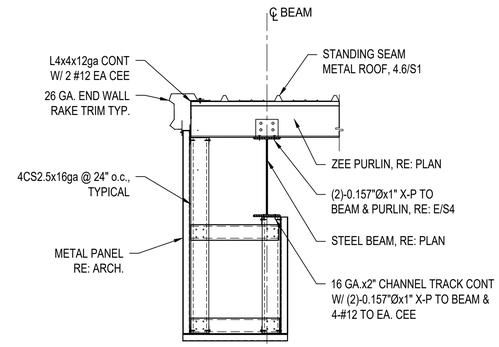
S2



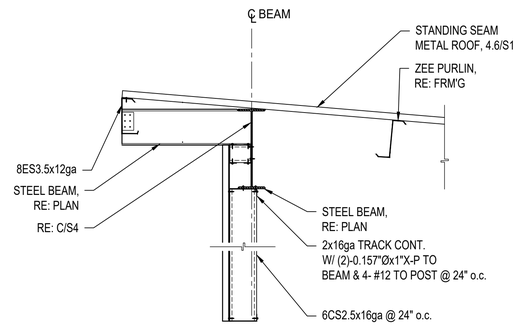
I FRAMING PLAN
SCALE: 1/4" = 1'-0"



SECTION 2
SCALE: 3/4 " = 1'-0"



SECTION 3
SCALE: 3/4 " = 1'-0"



SECTION 4
SCALE: 3/4 " = 1'-0"

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FRAMING PLAN
AND SECTIONS

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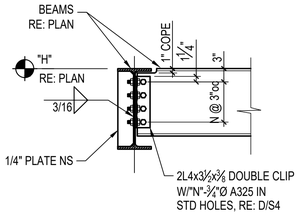
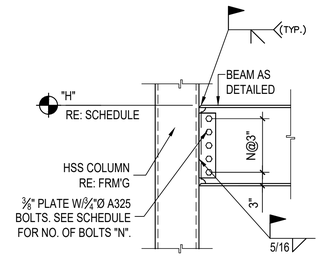
S3



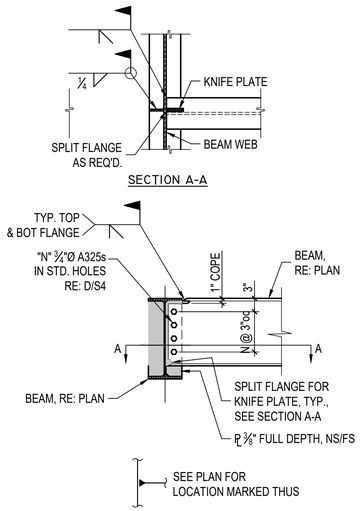
ONE SIDED BEAM
TO BEAM CONNECTION

DETAIL A
SCALE:3/4" = 1'-0"



BEAM-COLUMN
MOMENT CONNECTION

DETAIL B
SCALE: 3/4 " = 1'-0"



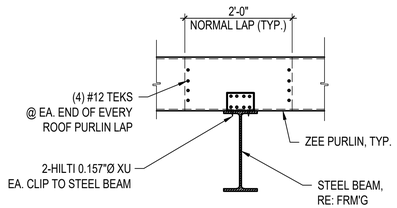
BEAM TO BEAM
MOMENT CONNECTION

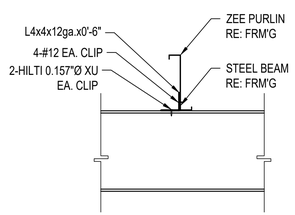
DETAIL C
SCALE: 3/4" = 1'-0"

SIZE	N
C8 & C10	2
W8	2
W10	2
W12	3
W14	4
W16	4
W18	5
W21	6
W24	7
W27	8
W30	9

CONNECTION SCHEDULE

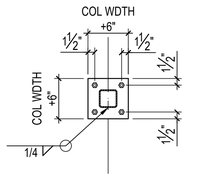
DETAIL D
SCALE:3/4" = 1'-0"



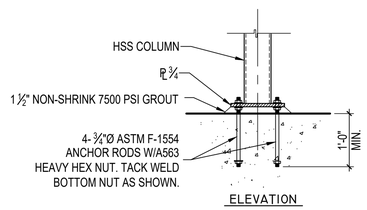


ROOF PURLIN TO
W BEAM CONNECTION

DETAIL E
SCALE: 3/4 " = 1'-0"



PLAN



ELEVATION

BP 01
SCALE:3/4" = 1'-0"

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Ward 3 Recreation
Additions and Renovations
3210 Power Center Pkwy
Lake Charles, Louisiana 70601

stamp:

phase:

project #: **2503**
date issued: 05/13/2025
drawn by: MB
checked by: KLS
revisions:

CONNECTION
DETAILS

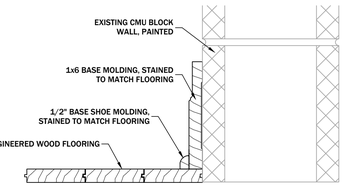
BK SMITH
ENGINEERING CO., INC.
1914 CITIZENS BANK DR.
BOSSIER CITY, LOUISIANA
318-741-0127

S4

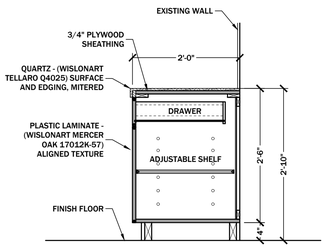
ROOM FINISH SCHEDULE

NOTE: ALL FINAL MATERIAL SELECTIONS, COLORS, PATTERNS AND INSTALLATION ARRANGEMENTS TO BE BY ARCHITECT. EQUALS AS APPROVED BY ARCHITECT 10 DAYS PRIOR TO BID DATE.

ROOM NUM.	DESIGNATION	FLOOR MAT.	BASE				WALLS		DOOR TRIM		WINDOW TRIM		DOORS	CEILING	
							MAT.	COLOR	MAT.	COLOR	MAT.	COLOR	COLOR	TYPE	TILE
		NO WORK	SHAW/CONTRACT ENGINEERED HARDWOOD - AUTHENTICITY. COLOR: ATTRIBUTE OAK-OR APPROVED EQUAL	NO WORK	STAINED OAK BASE	1/2" STAINED WOOD BASE SHOE	NO WORK	PAINTED GYP. BOARD - ALL SURFACES	NO WORK	COLOR AS SELECTED BY ARCHITECT	NO WORK	COLOR AS SELECTED BY ARCHITECT	COLOR AS SELECTED BY ARCHITECT	NO WORK	ARMSTRONG FINE FISSURED 15/16" TILES - GREY STONE
100	LOBBY	●		●			●		●		●			●	
102	**ACTIVITY ROOM**		●		●	●		●						●	●
103	MEETING ROOM	●		●			●		●		●			●	
106	EXECUTIVE ASSISTANT	●		●			●		●		●			●	
107	ADMINISTRATIVE ASSISTANT RECEIPTIONIST	●		●			●		●		●			●	
108	SPORTS ASSISTANT	●		●			●		●		●			●	
109	ASSISTANT DIRECTOR	●		●			●		●		●			●	
110	CONCESSION #1	●		●			●		●		●			●	
111	CONCESSION #2	●		●			●		●		●			●	
112	PANTRY	●		●			●		●		●			●	
132	CORRIDOR # 5	●		●			●		●		●			●	
133	CORRIDOR # 4	●		●			●		●		●			●	
134	CORRIDOR # 3	●		●			●		●		●			●	



4 BASE DETAIL
SCALE: 3" = 1'-0"



3 MILLWORK SECTION
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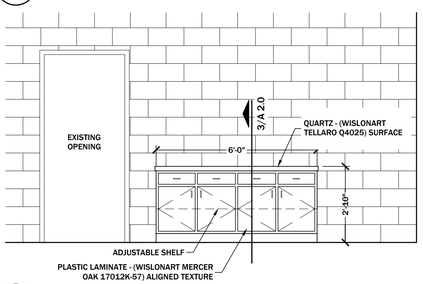
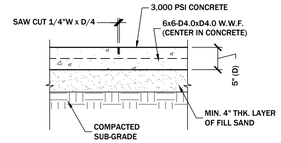
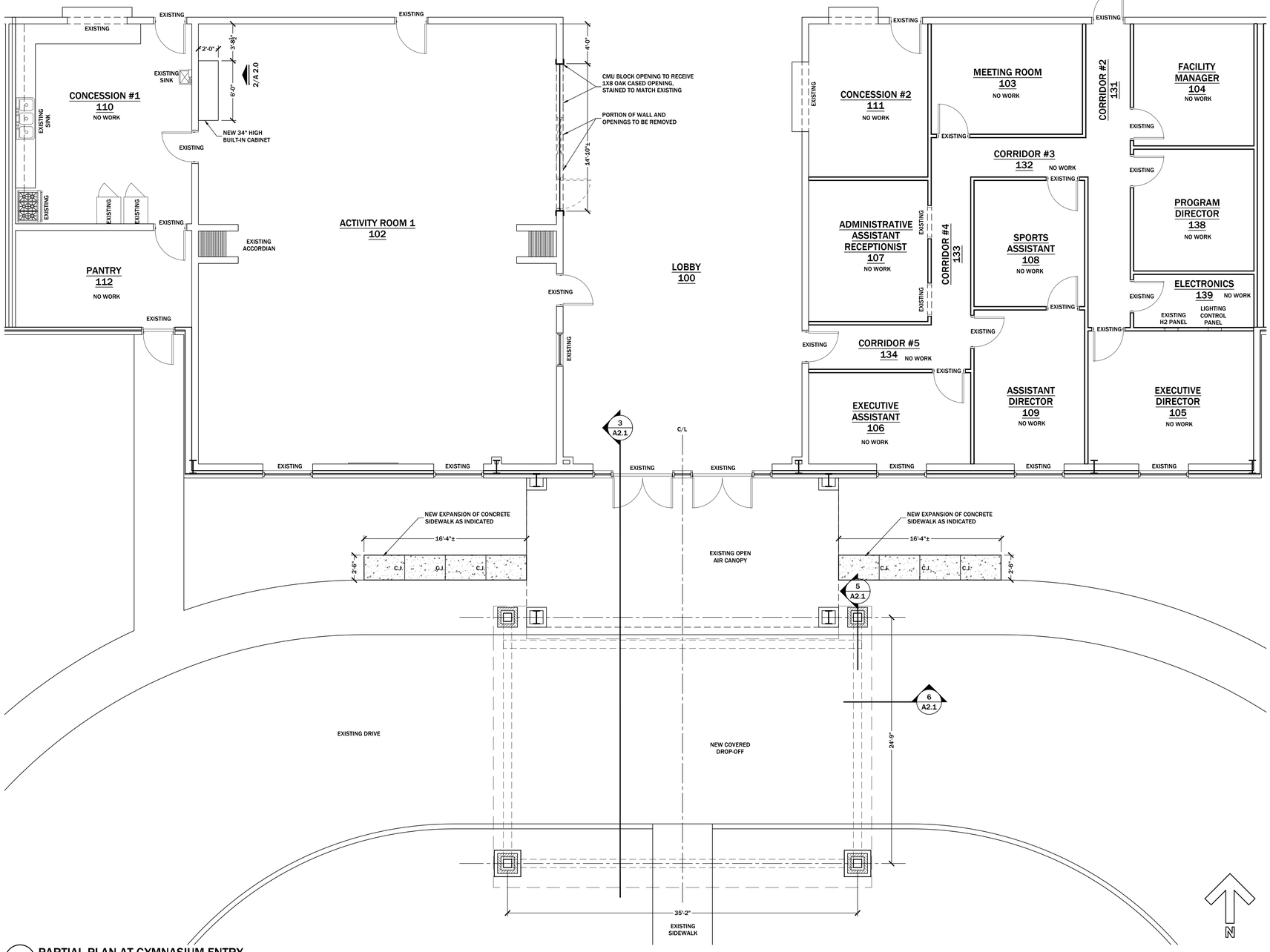


2 INTERIOR ELEVATION
SCALE: 3/8" = 1'-0"



5 CONTROL JOINT DETAIL (C.J.)
SCALE: 3/4" = 1'-0"



1 PARTIAL PLAN AT GYMNASIUM ENTRY
SCALE: 3/16" = 1'-0"

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Ward 3 Recreation
Various Projects at Power Center
Sports Complex
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Lake Charles, Louisiana 70607

stamp:

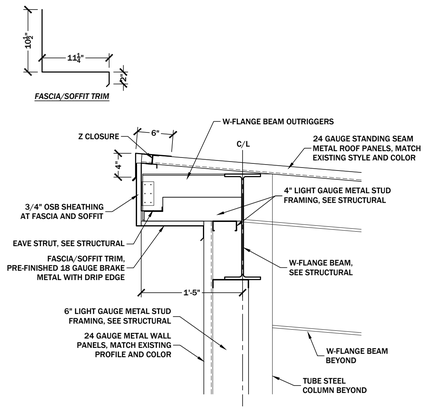
JEFFERY M. KUDLA REG. NO. 28174 STATE OF LOUISIANA REGISTERED ARCHITECT

phase:
For Construction
Construction Bid Documents

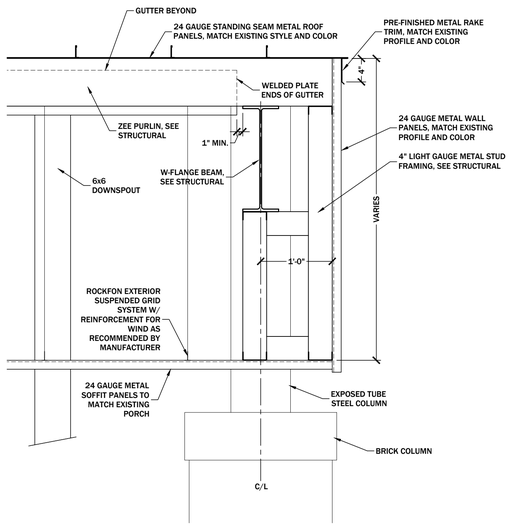
project #: 2503
date issued: 08/11/2025
drawn by: kr
checked by: jk
revisions:

Gymnasium Floor Plan

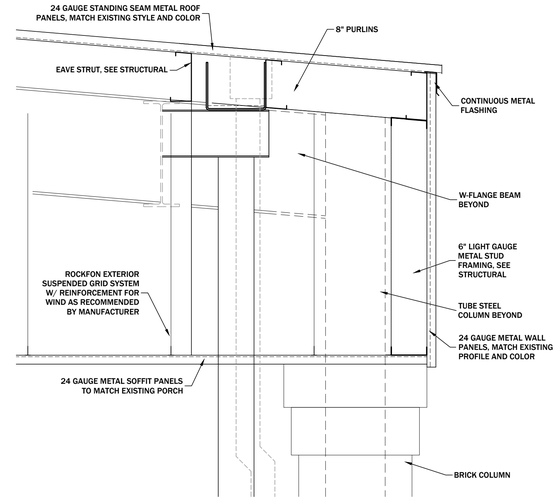
A 2.0



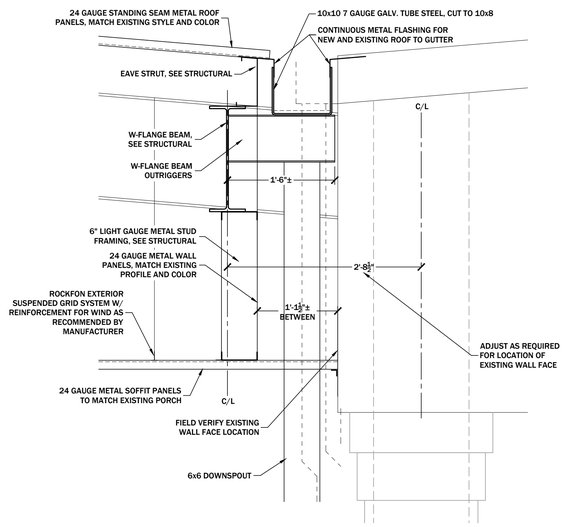
7 ENLARGED SECTION
SCALE: 1" = 1'-0"



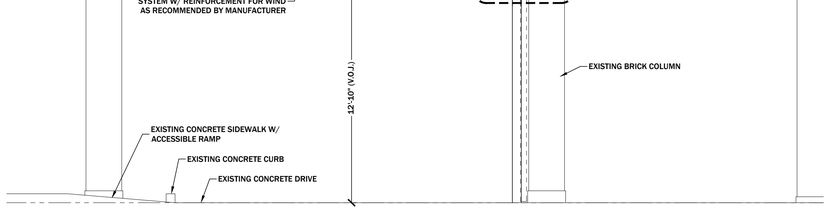
6 ENLARGED SECTION
SCALE: 1" = 1'-0"



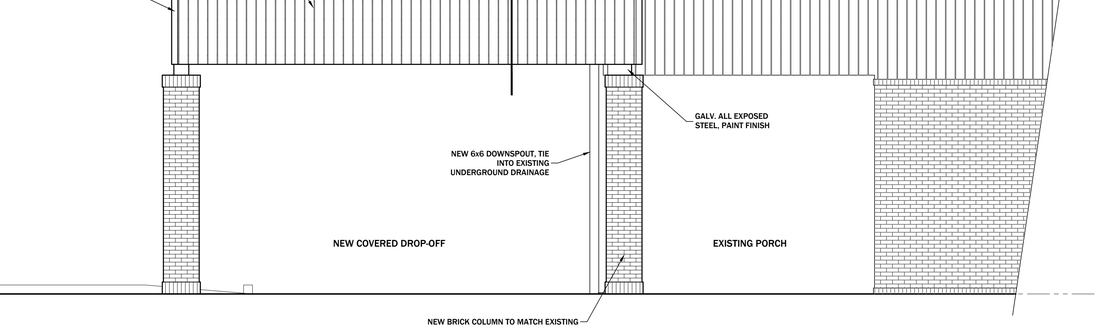
5 ENLARGED SECTION
SCALE: 1" = 1'-0"



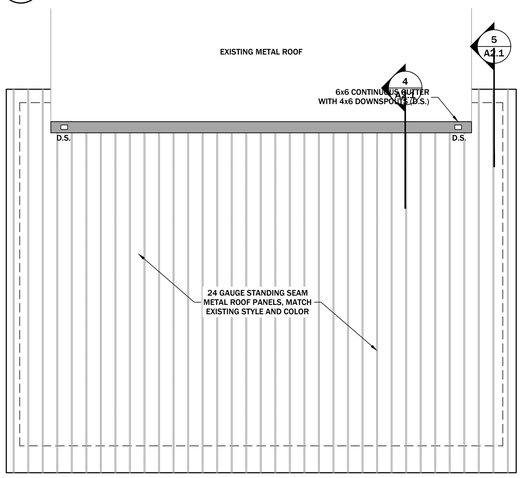
4 ENLARGED SECTION
SCALE: 1" = 1'-0"



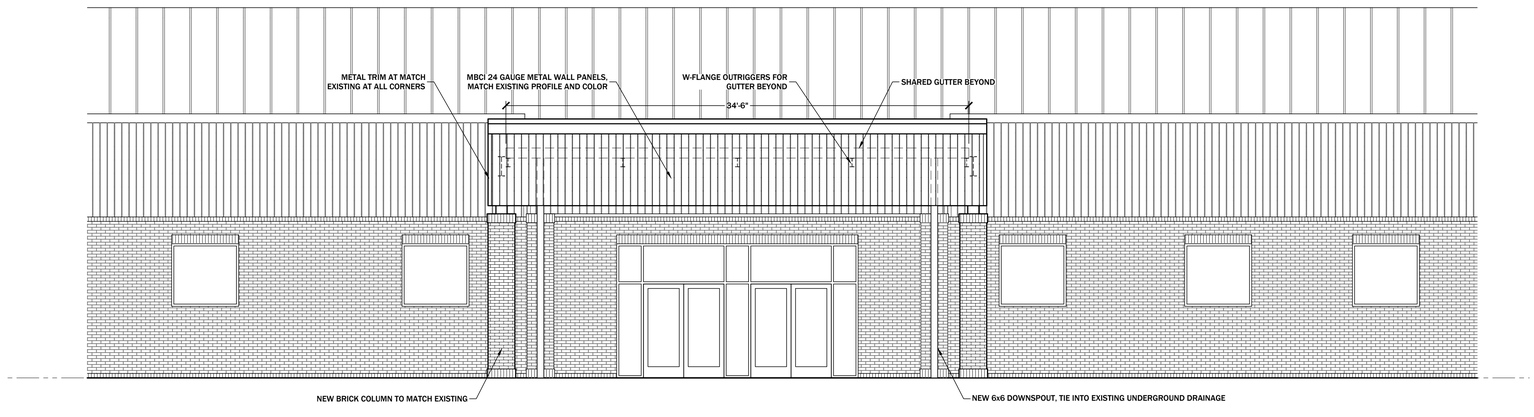
3 BUILDING SECTION
SCALE: 1/4" = 1'-0"



2 PARTIAL EAST ELEVATION
SCALE: 3/16" = 1'-0"



8 ROOF PLAN
SCALE: 3/16" = 1'-0"



1 PARTIAL SOUTH ELEVATION
SCALE: 3/16" = 1'-0"

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Ward 3 Recreation
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3210 Power Center Pkwy
Lake Charles, Louisiana 70607

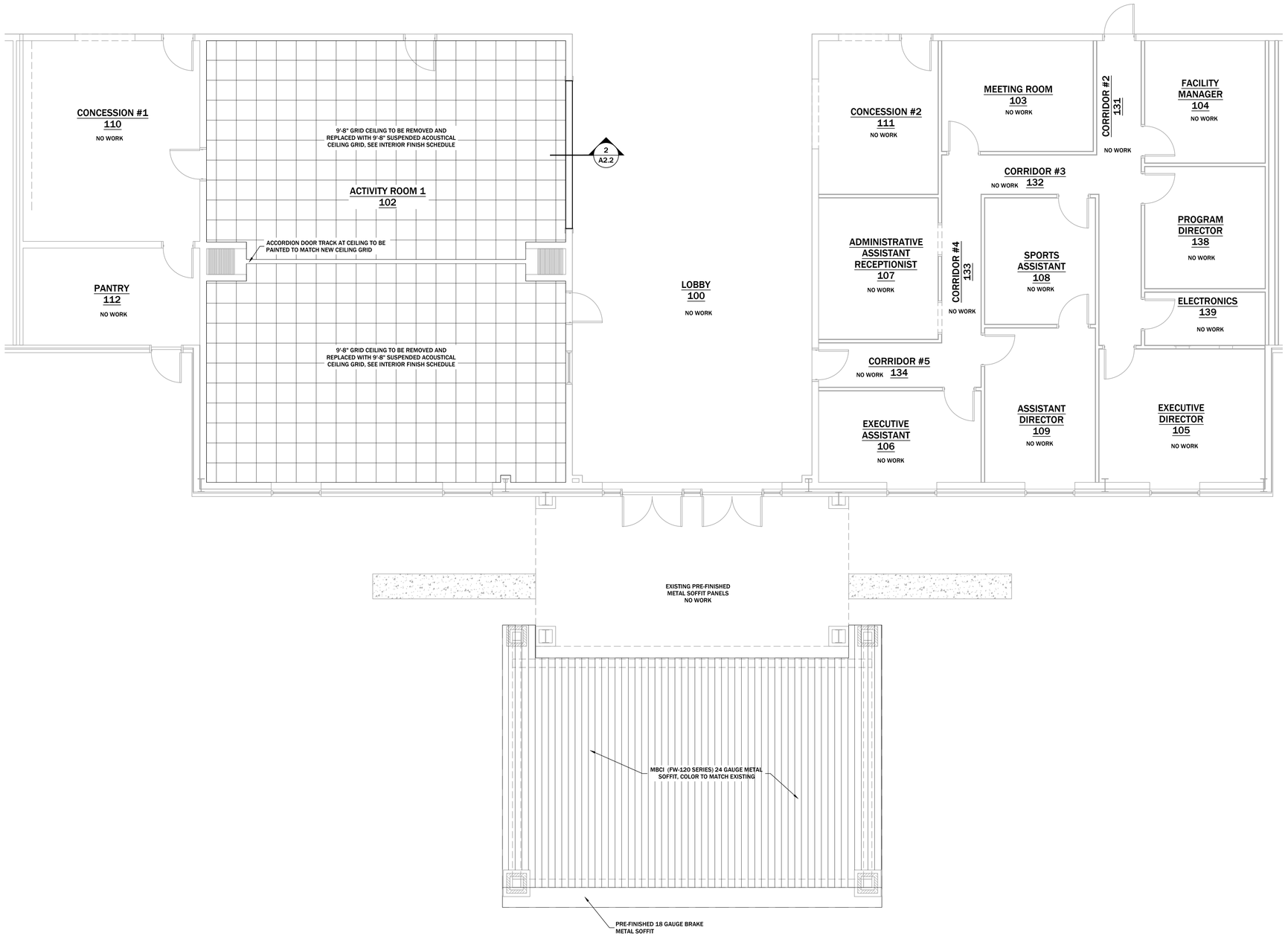
stamp:

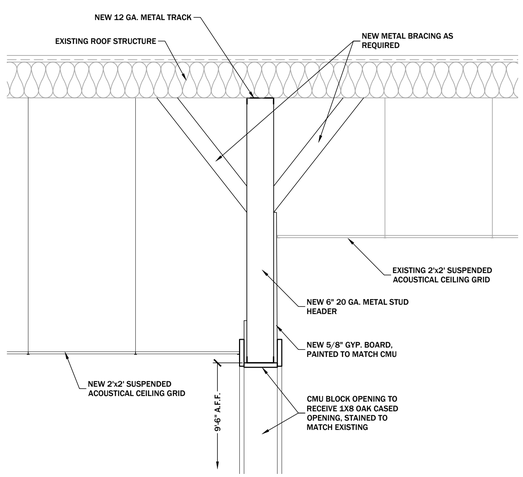
phase:
For Construction
Construction Bid Documents

project #: 2503
date issued: 08/11/2025
drawn by: kr
checked by: jk
revisions:

Gymnasium Elevations and Sections

A 2.1





2 **CEILING DETAIL**
SCALE: 3/4" = 1'-0"

1 **PARTIAL PLAN AT GYMNASIUM ENTRY**
SCALE: 3/16" = 1'-0"

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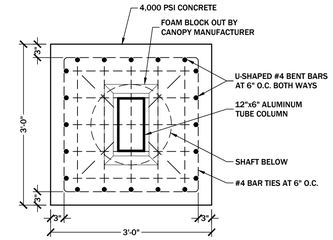
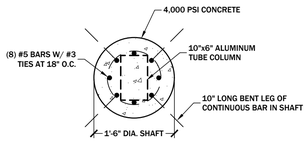
drawn by: kr
checked by: jk
revisions:

Gymnasium Reflected Ceiling Plan and Details

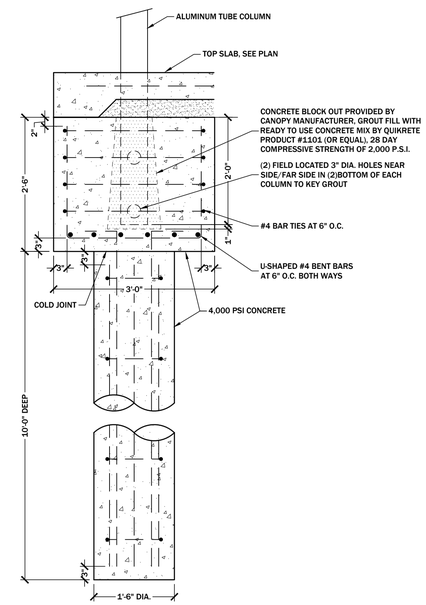
A 2.2



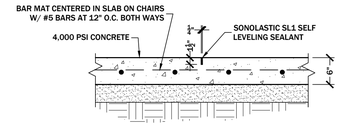
CAP PLAN VIEW



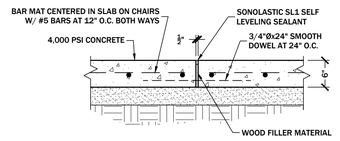
SHAFT PLAN VIEW



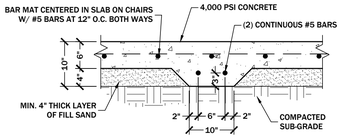
3 CANOPY COLUMN SHAFT DETAIL
SCALE: 3/4" = 1'-0"



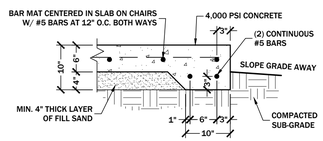
7 CONTROL JOINT DETAIL
SCALE: 3/4" = 1'-0"



6 EXPANSION JOINT DETAIL
SCALE: 3/4" = 1'-0"

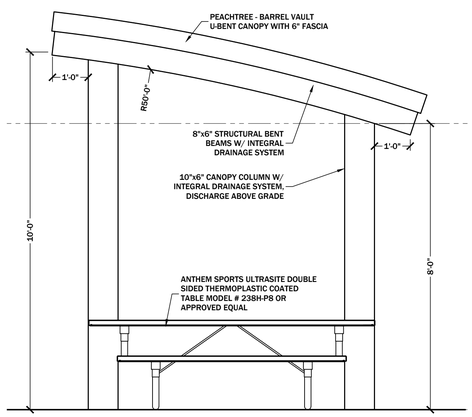


5 INTERIOR FOOTING DETAIL
SCALE: 3/4" = 1'-0"



4 PERIMETER FOOTING DETAIL
SCALE: 3/4" = 1'-0"

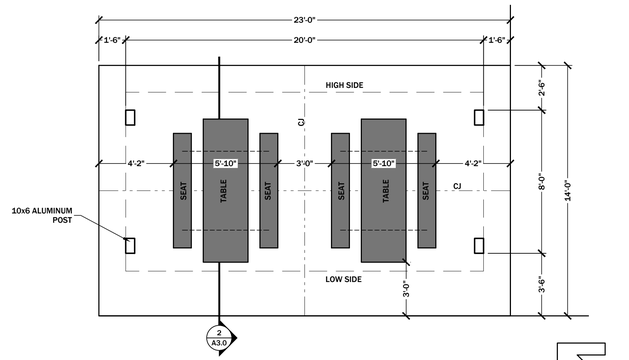
NOTE: SEE SPECIFICATIONS FOR SLOPED BARREL VAULT U-BENT CANOPY - PEACHTREE COVERS OR APPROVED EQUAL. ALL COMPONENTS TO BE PRE-FINISHED. SUBMIT SHOP DRAWINGS TO ARCHITECT FOR APPROVAL PRIOR TO FABRICATION.



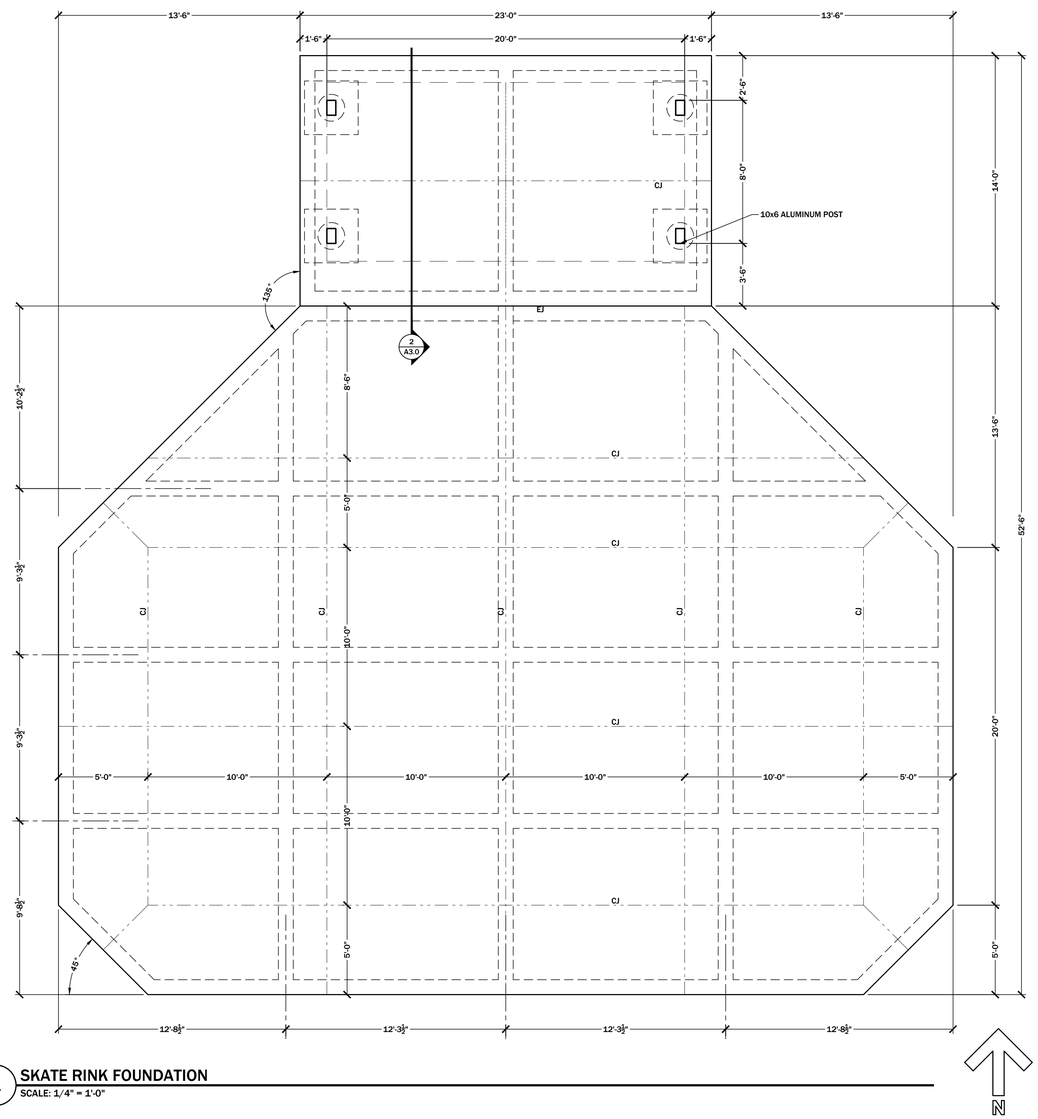
2 SEAT CANOPY SECTION
SCALE: 1/2" = 1'-0"

FOUNDATION AND PAVING NOTES:

1. A 6 MIL POLYETHYLENE BARRIER SHALL BE INSTALLED, PRIOR TO PLACING REINFORCEMENT OR CONCRETE, AFTER APPROVAL OF PREPARED EARTHWORK.
2. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI.
3. REINFORCING STEEL SHALL BE GRADE 60 DEFORMED BARS CONFORMING TO ASTM A-165.
4. WELDED STEEL MESH SHALL CONFORM TO ASTM A-185.
5. ALL REINFORCING RODS SHALL BE BENT AROUND ALL CORNERS A MINIMUM OF 24".
6. REMOVE TOP 48" OF VEGETATION, DELETERIOUS MATERIAL AND TOPSOIL FROM AREA TO RECEIVE BUILDING AND DOWN TO 18" BELOW PARKING AREAS. REPLACE EXCAVATED MATERIAL WITH SELECT FILL AND COMPACT TO 95% STANDARD PROCTOR DENSITY. BUILDING PAD FINAL ELEVATION TO BE EQUAL TO EXISTING ADJACENT BUILDING.
7. MAINTAIN UNIFORM MOISTURE CONDITIONS IN THE EXPOSED SUB-GRADE SOILS PRIOR TO CONSTRUCTION OF FLOOR SLAB. EXPOSED SUB-GRADE SOILS SHOULD NOT BE ALLOWED TO DRY OUT OR BECOME WET DURING CONSTRUCTION.
8. BACKFILL ALL UTILITY TRENCHES OR OTHER POTENTIAL CONDUITS OF WATER UNDERNEATH SLAB TO A DISTANCE WITHIN 5'-0" OF ADJACENT FLOOR SLABS.
9. INSTALL CONCRETE SEALER (EUCO DIAMOND HARD OR APPROVED EQUAL) AS RECOMMENDED BY MANUFACTURER TO FINISH SLAB.
10. CONCRETE AT THIS LOCATION SHALL BE FINISHED WITH MECHANICAL LASER SCREED



8 SKATE PARK BLEACHER FOUNDATION
SCALE: 1/4" = 1'-0"



1 SKATE RINK FOUNDATION
SCALE: 1/4" = 1'-0"

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Ward 3 Recreation
Various Projects at Power Center
Sports Complex

3210 Power Center Pkwy
Lake Charles, Louisiana 70607

stamp:

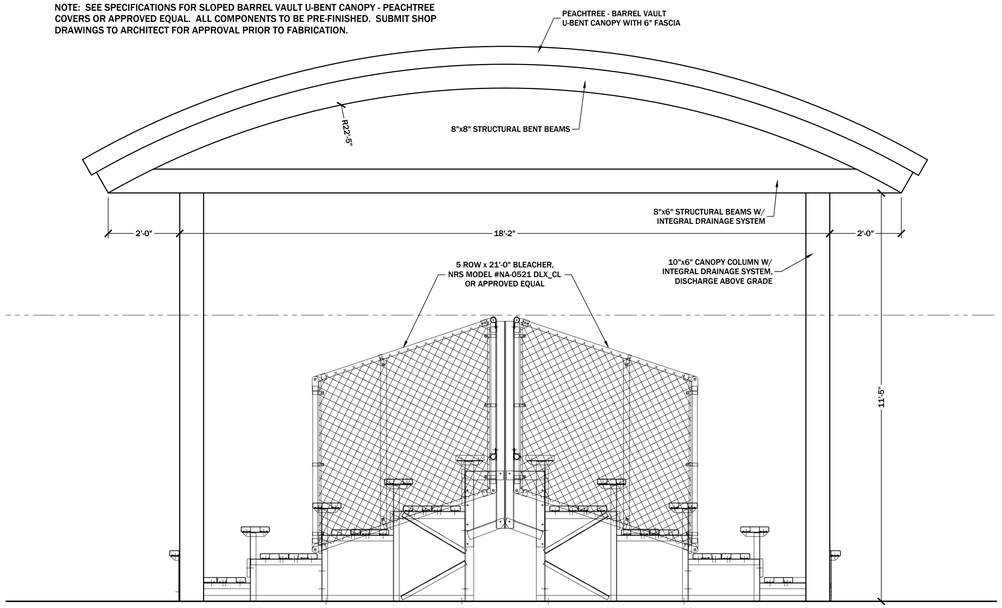
phase:
For Construction
Construction Bid Documents

project #: 2503
date issued: 08/11/2025
drawn by: kr
checked by: jk
revisions:

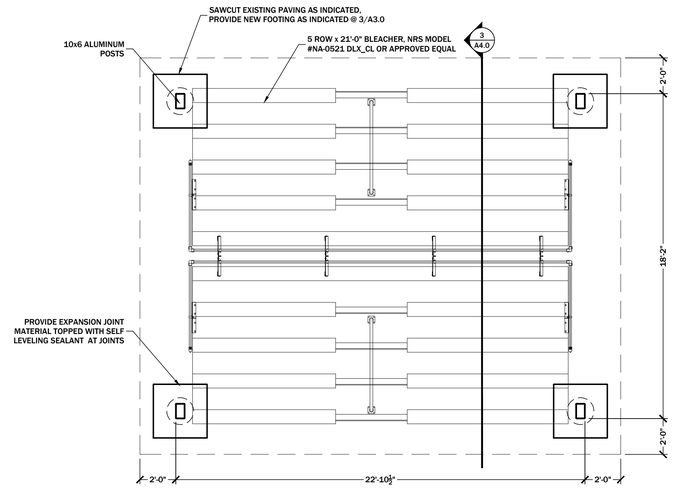
Skate Rink Plan and Elevations

A 3.0

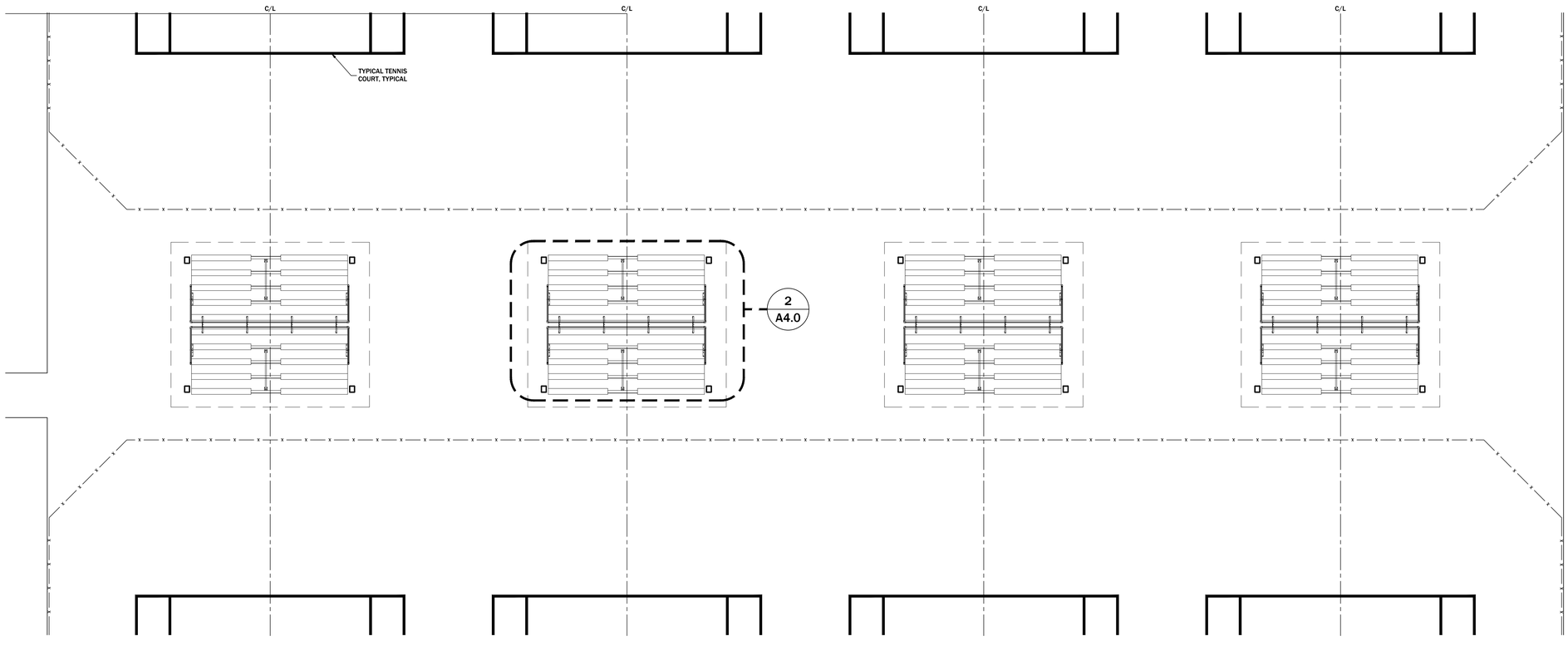
NOTE: SEE SPECIFICATIONS FOR SLOPED BARREL VAULT U-BENT CANOPY - PEACHTREE COVERS OR APPROVED EQUAL. ALL COMPONENTS TO BE PRE-FINISHED. SUBMIT SHOP DRAWINGS TO ARCHITECT FOR APPROVAL PRIOR TO FABRICATION.



3 COVERED CANOPY PLAN
SCALE: 1/2" = 1'-0"



2 COVERED CANOPY PLAN
SCALE: 1/4" = 1'-0"



1 ENLARGED SITE PLAN
SCALE: 1/8" = 1'-0"

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Various Projects at Power Center
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3210 Power Center Pkwy
Lake Charles, Louisiana 70607

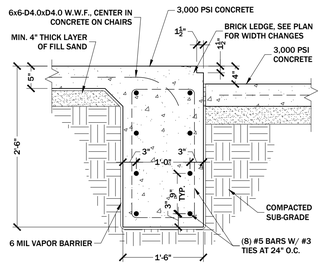
stamp:
JEFFERY M. KUDLA
REG. NO. 31814
STATE OF LOUISIANA
REGISTERED ARCHITECT

phase:
For Construction
Construction Bid Documents

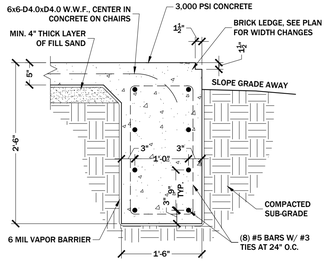
project #: 2503
date issued: 08/11/2025
drawn by: kr
checked by: jk
revisions:

Tennis Court Covered Seating

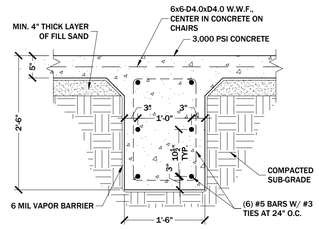
A 4.0



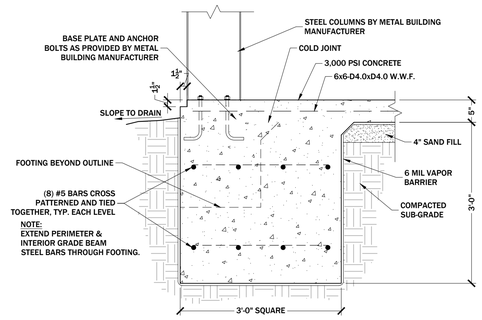
6 EXTERIOR FOOTING AT SIDEWALK
SCALE: 3/4" = 1'-0"



5 EXTERIOR FOOTING
SCALE: 3/4" = 1'-0"



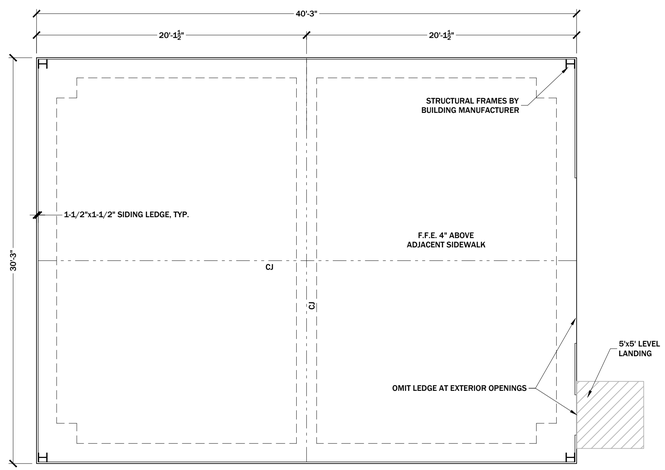
4 INTERIOR FOOTING
SCALE: 3/4" = 1'-0"



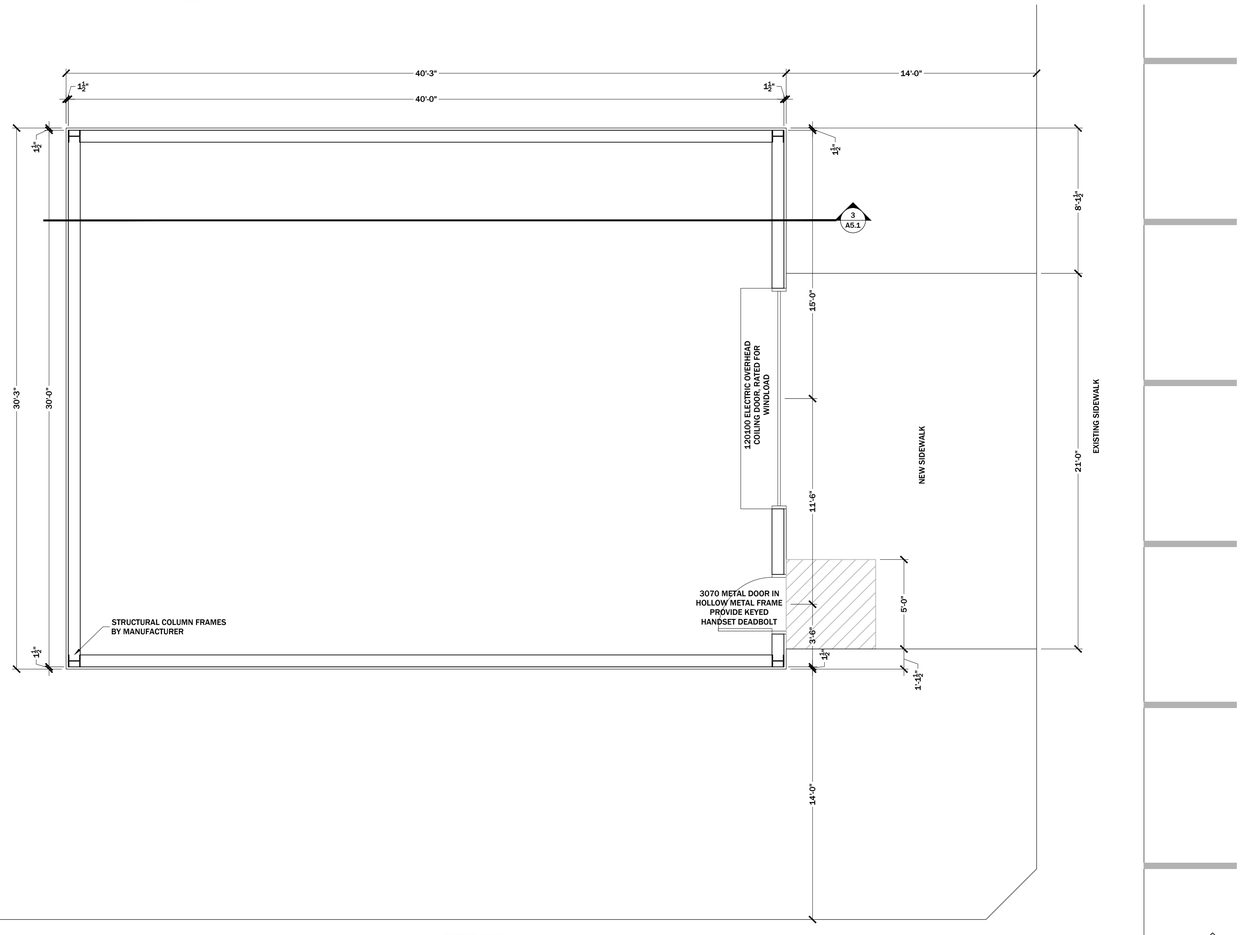
3 FOOTING AT STEEL COLUMNS
SCALE: 3/4" = 1'-0"

FOUNDATION AND PAVING NOTES:

1. A 6 MIL POLYETHYLENE BARRIER SHALL BE INSTALLED, PRIOR TO PLACING REINFORCEMENT OR CONCRETE, AFTER APPROVAL OF PREPARED EARTHWORK.
2. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
3. REINFORCING STEEL SHALL BE GRADE 60 DEFORMED BARS CONFORMING TO ASTM A-165.
4. WELDED STEEL MESH SHALL CONFORM TO ASTM A-185.
5. ALL REINFORCING RODS SHALL BE BENT AROUND ALL CORNERS A MINIMUM OF 24".
6. REMOVE TOP 48" OF VEGETATION, DELETERIOUS MATERIAL AND TOPSOIL FROM AREA TO RECEIVE BUILDING AND DOWN TO 18" BELOW PARKING AREAS. REPLACE EXCAVATED MATERIAL WITH SELECT FILL AND COMPACT TO 95% STANDARD PROCTOR DENSITY. BUILDING PAD FINAL ELEVATION TO BE EQUAL TO EXISTING ADJACENT BUILDING.
7. MAINTAIN UNIFORM MOISTURE CONDITIONS IN THE EXPOSED SUB-GRADE SOILS PRIOR TO CONSTRUCTION OF FLOOR SLAB. EXPOSED SUB-GRADE SOILS SHOULD NOT BE ALLOWED TO DRY OUT OR BECOME WET DURING CONSTRUCTION.
8. BACKFILL ALL UTILITY TRENCHES OR OTHER POTENTIAL CONDUITS OF WATER UNDERNEATH SLAB TO A DISTANCE WITHIN 5'-0" OF ADJACENT FLOOR SLABS.



2 FOUNDATION PLAN
SCALE: 3/16" = 1'-0"



1 FLOOR PLAN
SCALE: 1/4" = 1'-0"

PRE-ENGINEERED METAL BUILDING NOTES:

1. METAL BUILDING STRUCTURE SHALL BE DESIGNED TO MEET NOT LESS THAN 130 MPH WINDLOAD.
2. PRE-ENGINEERED METAL BUILDING PROVIDER IS TO INTEGRATE ADDITIONAL BUILDING STRUCTURE AS REQUIRED TO FRAME EXTERIOR CANOPIES AND PARAPET WALLS.
3. PROVIDE BLOCKING AS REQUIRED TO MOUNT EXTERIOR ELEMENTS, SUCH AS BUT NOT LIMITED TO, EXTERIOR CANOPIES, BUILDING SIGNAGE, BUILDING LIGHTING, ETC. ALL EXTERIOR ELEMENTS MOUNT BACK TO BUILDING STRUCTURE, DO NOT MOUNT TO BRICK.
4. GENERAL CONTRACTOR SHALL SUBMIT METAL BUILDING SHOP DRAWINGS TO ARCHITECT FOR APPROVAL PRIOR TO ORDERING.
5. LAPPED METAL ROOF PANELS AT A SLOPE LESS THAN 3/12 (25% SLOPE) SHALL HAVE LAP SEALANT IN ACCORDANCE WITH MANUFACTURERS INSTALLATION INSTRUCTIONS.
6. MAIN STRUCTURAL STEEL FRAMES TO UTILIZE STRAIGHT COLUMNS ONLY
7. WALL FRAMING OF PEMB SHALL MAINTAIN A MINIMUM DEFLECTION OF L/360 WHERE STUCCO IS TO BE APPLIED
8. GIRT SPACING OF 24" O.C. TO BE USED AT ALL LOCATIONS SHOWN

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Ward 3 Recreation
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stamp:

phase:
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Construction Bid Documents

project #: 2503
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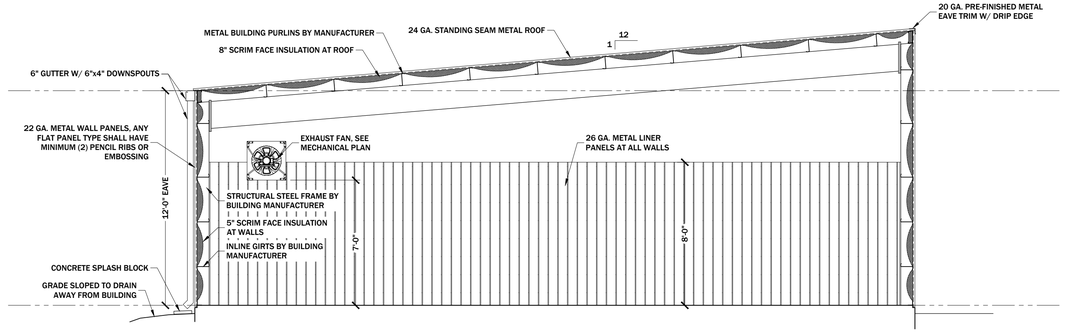
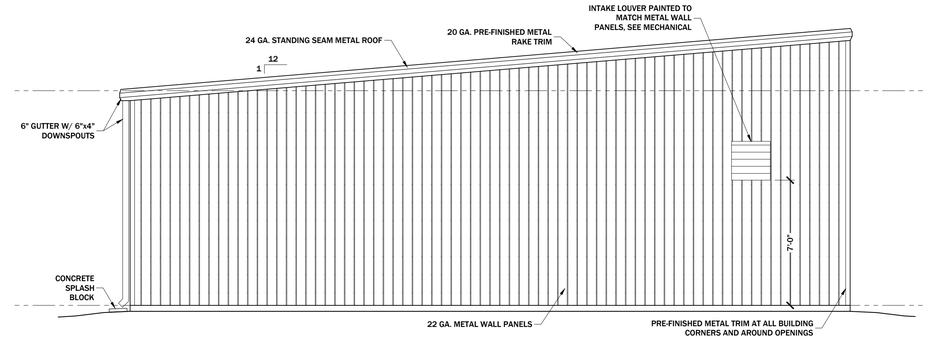
drawn by: kr
checked by: jk
revisions:

Storage Building Plans and Details

A 5.0



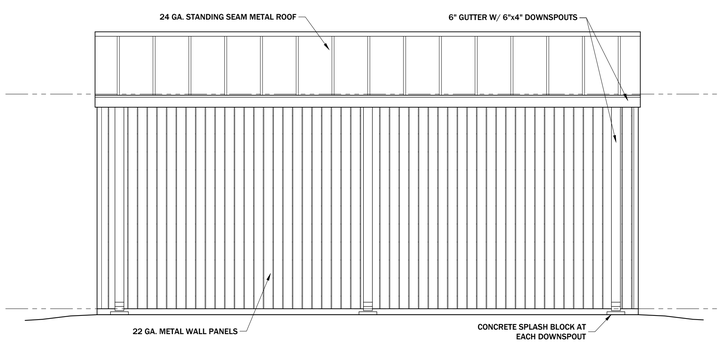
4 BUILDING SECTION
SCALE: 1/4" = 1'-0"



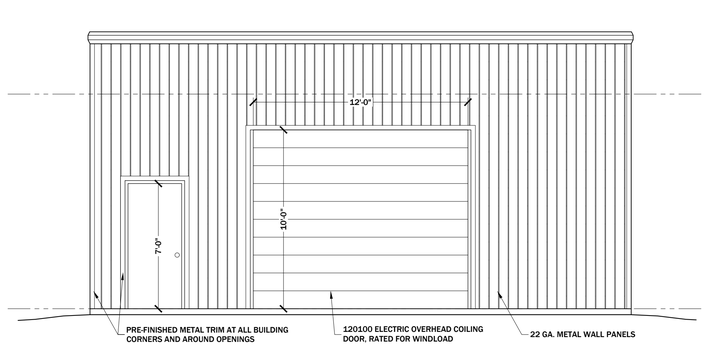
3 SOUTH ELEVATION - NORTH ELEVATION SIMILAR
SCALE: 1/4" = 1'-0"

PRE-ENGINEERED METAL BUILDING NOTES:

1. ALL PEMB STRUCTURAL PRIMARY & SECONDARY COMPONENTS SHALL BY HOT DIPPED GALVANIZED.
2. METAL BUILDING STRUCTURE SHALL BE DESIGNED TO MEET NOT LESS THAN 130 MPH WINDLOAD.
3. PRE-ENGINEERED METAL BUILDING PROVIDER IS TO INTEGRATE ADDITIONAL BUILDING STRUCTURE AS REQUIRED TO FRAME EXTERIOR CANOPIES AND PARAPET WALLS.
4. PROVIDE BLOCKING AS REQUIRED TO MOUNT EXTERIOR ELEMENTS, SUCH AS BUT NOT LIMITED TO, EXTERIOR CANOPIES, BUILDING SIGNAGE, BUILDING LIGHTING, ETC. ALL EXTERIOR ELEMENTS MOUNT BACK TO BUILDING STRUCTURE, DO NOT MOUNT TO BRICK.
5. GENERAL CONTRACTOR SHALL SUBMIT METAL BUILDING SHOP DRAWINGS TO ARCHITECT FOR APPROVAL PRIOR TO ORDERING.
6. LAPPED METAL ROOF PANELS AT A SLOPE LESS THAN 3/12 (25% SLOPE) SHALL HAVE LAP SEALANT IN ACCORDANCE WITH MANUFACTURERS INSTALLATION INSTRUCTIONS.
7. PROVIDE A COMPLETE, INTEGRATED SET OF MUTUALLY DEPENDENT COMPONENTS AND ASSEMBLIES THAT FORM A METAL BUILDING SYSTEM CAPABLE OF WITHSTANDING STRUCTURAL AND OTHER LOADS, THERMALLY INDUCED MOVEMENT, AND EXPOSURE TO WEATHER WITHOUT FAILURE OR INFILTRATION OF WATER INTO BUILDING INTERIOR. INCLUDE PRIMARY AND SECONDARY FRAMING, METAL ROOF PANELS, METAL WALL PANELS, AND ACCESSORIES COMPLYING WITH REQUIREMENTS INDICATED. PROVIDE METAL BUILDING SYSTEM OF SIZE AND WITH SPACING, SLOPES, AND SPANS INDICATED.
8. MAIN STRUCTURAL STEEL FRAMES TO UTILIZE STRAIGHT COLUMNS ONLY, NO TAPERED COLUMNS WILL BE ACCEPTED.
9. WALL FRAMING OF PEMB SHALL MAINTAIN A MINIMUM DEFLECTION OF L/360 WHERE STUCCO IS TO BE APPLIED
10. GIRT SPACING OF 24" O.C. TO BE USED AT ALL LOCATIONS SHOWN



2 WEST ELEVATION
SCALE: 1/4" = 1'-0"



1 EAST ELEVATION
SCALE: 1/4" = 1'-0"

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checked by: jk
revisions:

Storage Building Elevations and Sections

A 5.1

KEYNOTES:

A. REMOVE AND REPLACE TRUNKLINE "B" IN IT'S ENTIRETY - INSTALL NEW 24"Ø ADS HP STORM PIPE FOR TRUNKLINE REPLACEMENT AND PROVIDE NEW CATCH BASIN "CB-04" WHERE INDICATED, SEE DETAIL PROCEDURE 4&5/A8.0

B. MOVE NEW CATCH BASIN "CB-04" LOCATION IN LINE WITH "T" JUNCTION WHERE INDICATED

C. LOCATIONS FOR "NDS" DRAIN INLET RESTORATION, AT LOCATIONS WHERE GRATE DAMAGE OCCURS - REMOVE AND REPLACE GRATE AS INDICATED. SEE PROCEDURE DETAIL 3/A8.0

D. LOCATIONS FOR PIPE JOINT RESTORATION, SEE PROCEDURE DETAIL 2/A8.0

E. ADD NEW "NDS" STYLE DRAIN CATCH BASIN AT LOCATIONS INDICATED

SYMBOL KEY:

E EXISTING "NDS" STYLE CATCH BASIN

E EXISTING "CB - 04" STYLE CATCH BASIN

N E EXISTING "NDS" STYLE CATCH BASIN TO BE REMOVED AND REPLACED WITH NEW "CB-04" CATCH BASIN

N NEW "NDS" STYLE CATCH BASIN

N NEW "CB - 04" STYLE CATCH BASIN

1 PARTIAL SITE PLAN - SOUTH FIELDS
SCALE: 1" = 150'-0"

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checked by: jk
revisions:

Partial Site Plan
South Fields

N

A 6.0

SCOPE OF WORK:

1. EXCAVATE AROUND EXISTING CB-01 INLET AND POUR WITH 3000 PSI CONCRETE COLLAR AROUND EXISTING UNDERGROUND PIPE CONNECTION AS SHOWN IN DETAIL 1 / A8.0 . TEMPORARILY COVER EXISTING OPENINGS WITH PLYWOOD AND 2X BRACING AS REQUIRED - FOR ALL EXISTING INLETS SHOWN IN THIS DIAGRAM.

2. PROVIDE NEW 24" Ø ADS STORM DUAL WALL DRAIN PIPE IN EXISTING DITCH ON EAST SIDE OF PROPERTY. VERIFY PIPE LENGTH IN THE FIELD - BACKFILL AS PER PIPE BEDDING SPECIFICATIONS

SYMBOL KEY:

E — EXISTING "CB - 01" STYLE DRAIN INLET TO BE REPAIRED

NEW 24" Ø ADS STORM DUAL WALL DRAIN PIPE

1 PARTIAL SITE PLAN - MAIN BUILDING AREA
SCALE: 1" = 60'-0"

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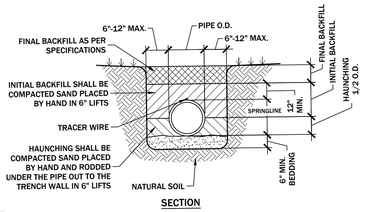
revisions:

Partial Site Plan
Main Buildings

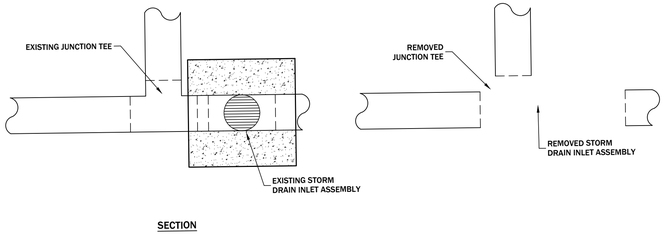
A 7.0



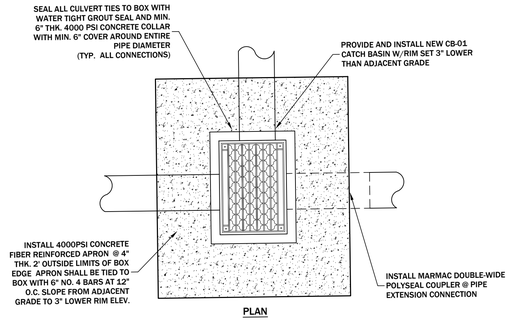
SECTION

6 STANDARD BACKFILL WITH BEDDING

SCALE: NTS



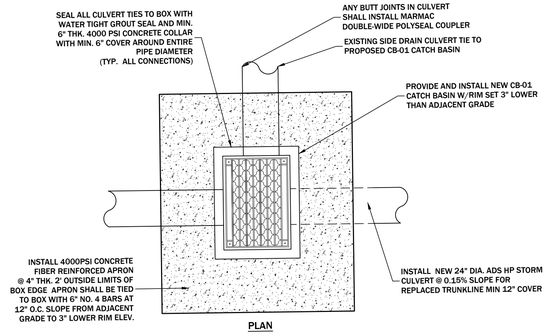
SECTION



PLAN

5 TRUNKLINE INLET RESTORATION PROCEDURE

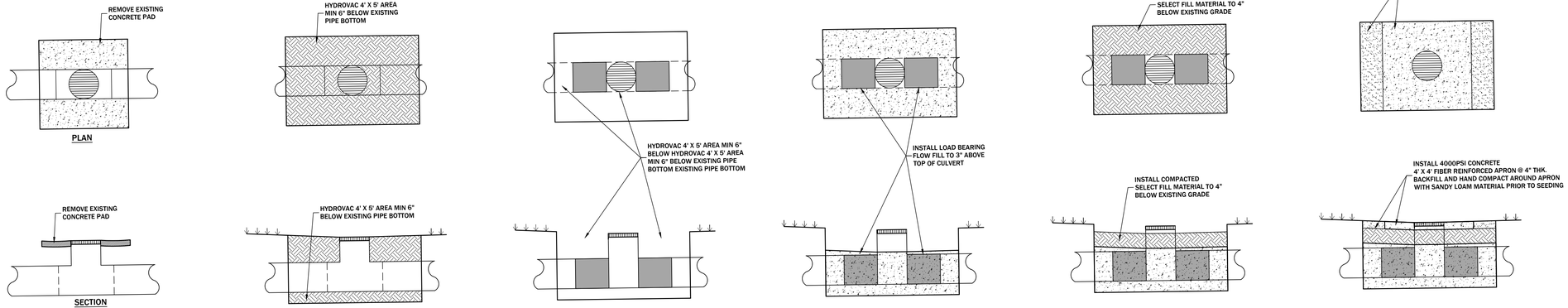
SCALE: NTS



PLAN

4 TRUNKILINE INLET REPLACEMENT

SCALE: NTS

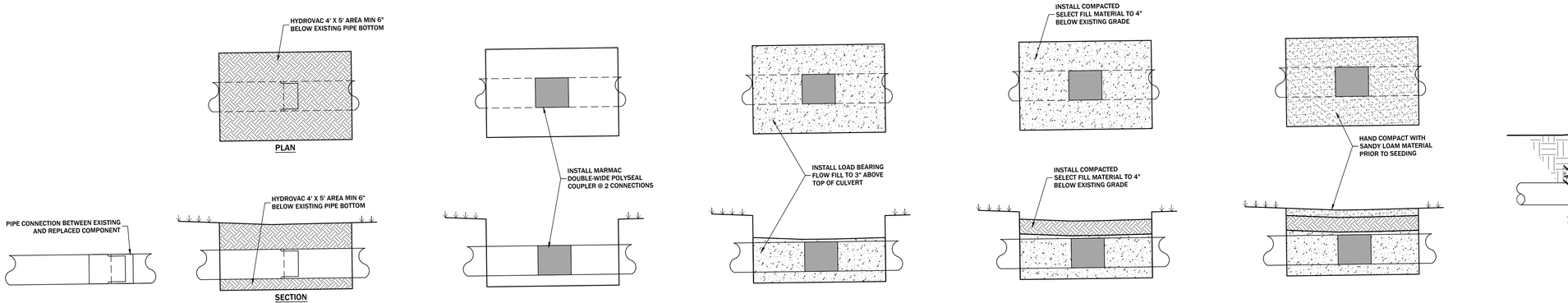


PLAN

SECTION

3 NDS INLET RESTORATION PROCEDURE

SCALE: NTS

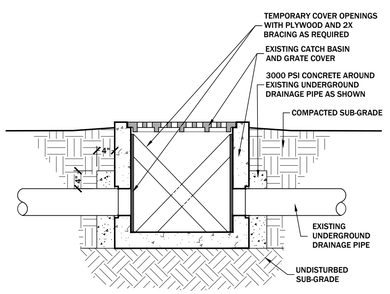


PLAN

SECTION

2 PIPE JOINT RESTORATION PROCEDURE

SCALE: NTS



SECTION

1 CB-01 STYLE DRAIN INLET REPAIR

SCALE: NTS

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revisions:

Details

A 8.0

HVAC LEGEND:

	EXISTING SUPPLY TO BE RELOCATED OR REMOVED
	EXISTING SUPPLY TO REMAIN
	NEW SUPPLY

SPRINKLER LEGEND:

	EXISTING SPRINKLER TO BE RELOCATED
	EXISTING SPRINKLER TO REMAIN
	NEW SPRINKLER

Electrical Key

	OUTLET TO BE DEMOLISHED
EX	DUPLEX OUTLET TO REMAIN
	DUPLEX OUTLET 110V - 1 20A CIRCUIT
FO	DUPLEX FLOOR OUTLET 110V - 1 20A CIRCUIT
CT	DUPLEX OUTLET 110V - 1 20A CIRCUIT MOUNTED AT COUNTER TOP HEIGHT
UC	DUPLEX OUTLET 110V - 1 20A CIRCUIT MOUNTED UNDER COUNTER
USB	DUPLEX USB RECEPTACLE
GFI	DUPLEX OUTLET 110V - 1 20A CIRCUIT PROVIDE GFI PROTECTED OUTLETS FOR ALL DUPLEX OUTLETS LOCATED WITHIN 3'-0" OF SINKS OR 6'-0" OF TOILET ROOM FIXTURES.
WP	DUPLEX OUTLET 110V - 1 20A CIRCUIT- WET LOCTATION OUTLET WITH COVER
	QUADPLEX OUTLET 110V - 1 20A CIRCUIT
EX	EXISTING DATA OUTLET
	DATA OUTLET - CAT5
T	THERMOSTAT
OS	OCCUPANCY SENSOR
$	1-POLE LIGHT SWITCH
J	JUNCTION BOX WITH HOMERUN
PP	OCCUPANCY SENSOR POWER PACK

Luminaire Schedule

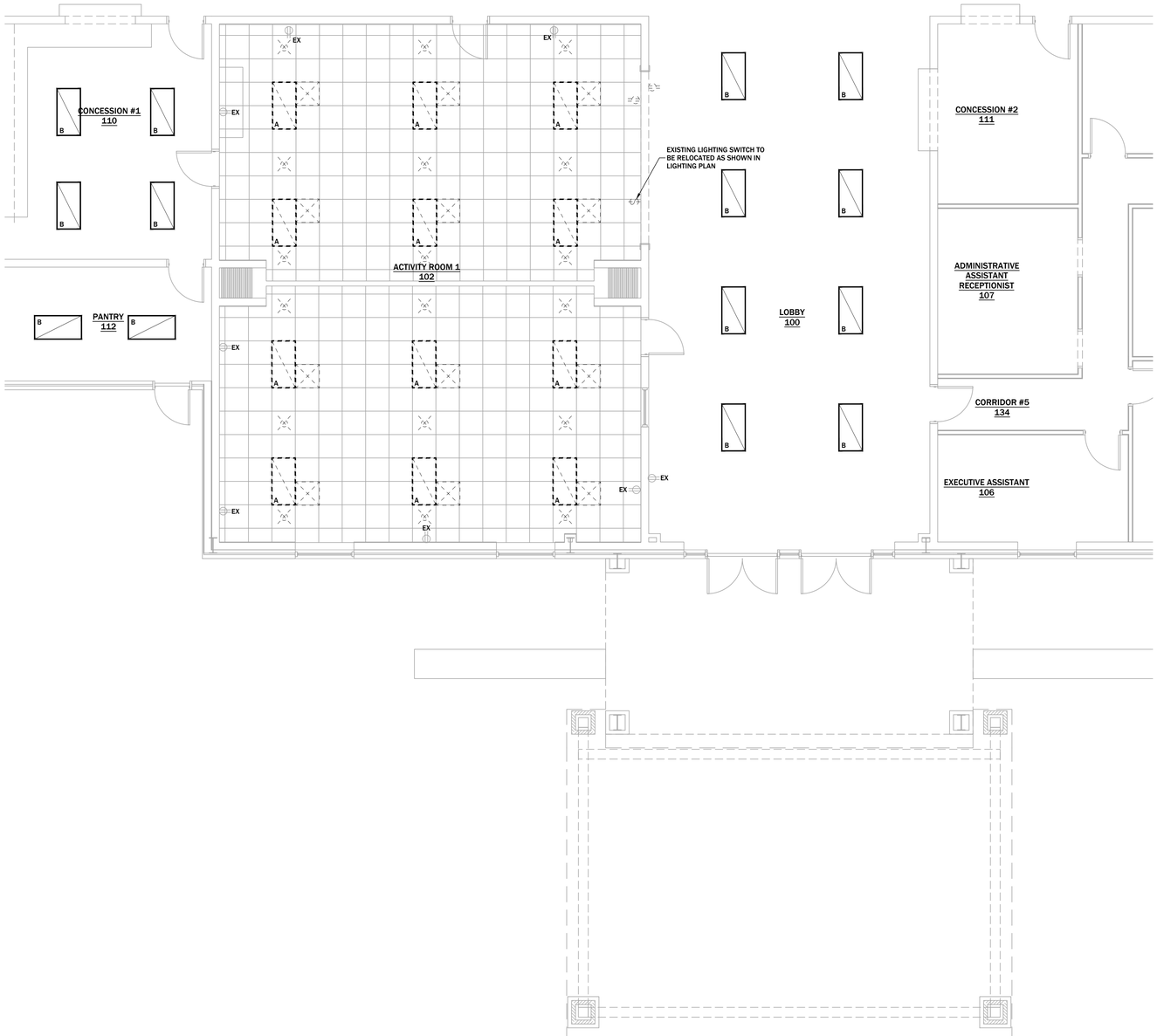
SYMBOL	LABEL	DESCRIPTION
	A	2x4 LAY-IN LED LIGHT TO BE DEMOLISHED
	B	EXISTING TO REMAIN
	C	NEW 2x4 LAY-IN LED LIGHT
	D	6" EXTERIOR RECESSED LIGHT
	E	4" INTERIOR RECESSED LIGHT
EXIT		EXIT SIGN / EMERGENCY LIGHT COMBO FIXTURE - PROVIDE MATCHING REMOTE HEAD AT EXTERIOR DOOR LOCATIONS
		EMERGENCY LIGHTING - WALL MOUNT - HARD WIRE TO ADJACENT LIGHTING CIRCUIT AS SHOWN

PANEL DESCRIPTION: **H2** (EXISTING)

VOLTAGE: 480Y/277V,304W
MOUNTING: SURFACE
NEMA: 1
ULSE: YES __ NO X

MAIN: 125A MLO
AIC: 14,000
LOAD: 53.1 KVA

SERVING	CKT #	BKR AMP.	WIRE		WIRE	BKR AMP.	CKT #	SERVING
LIGHTING	1	20/1	#12		#12	20/1	2	LIGHTING
	3	20/1	#12		#12	20/1	4	
	5	20/1	#12		#12	20/1	6	
SPARE	7	20/1	#12		#12	20/1	8	
	9	20/1	#12		#12	20/1	10	
LIGHTING COURT	11	20/1	#12		#10	20/1	12	EXTERIOR LIGHTING
	13	20/1	#12		#8	20/1	14	SITE LIGHTING
	15						16	
	17						18	
	19						20	
SPACE	21	20/1	#12		#12	20/1	22	SPARE
SPACE	23	20/1	#12		#12	20/1	24	SPACE
SPACE	25	20/1	#12		#12	20/1	26	SPACE
SPACE	27	20/1	#12		#12	20/1	28	SPACE
SPACE	29	20/1	#12		#8	35/2	30	SPACE
SPACE	31	20/1	#12		–	–	32	SPACE
SPACE	33	20/1	#12		#12	20/1	34	SPACE
SPACE	35				#12	20/1	36	SPACE
SPACE	37				#12	20/1	38	XFMR T-L3
SPACE	39				#12	20/1	40	SPACE
SPACE	41				#12	20/1	42	SPACE



1 EXISTING/DEMOLITION POWER AND LIGHTING PLAN
SCALE: 3/16" = 1'-0"

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checked by: jk
revisions:

Gymnasium Existing/ Demolition Plan

MEP 1.0

ELECTRICAL NOTES:

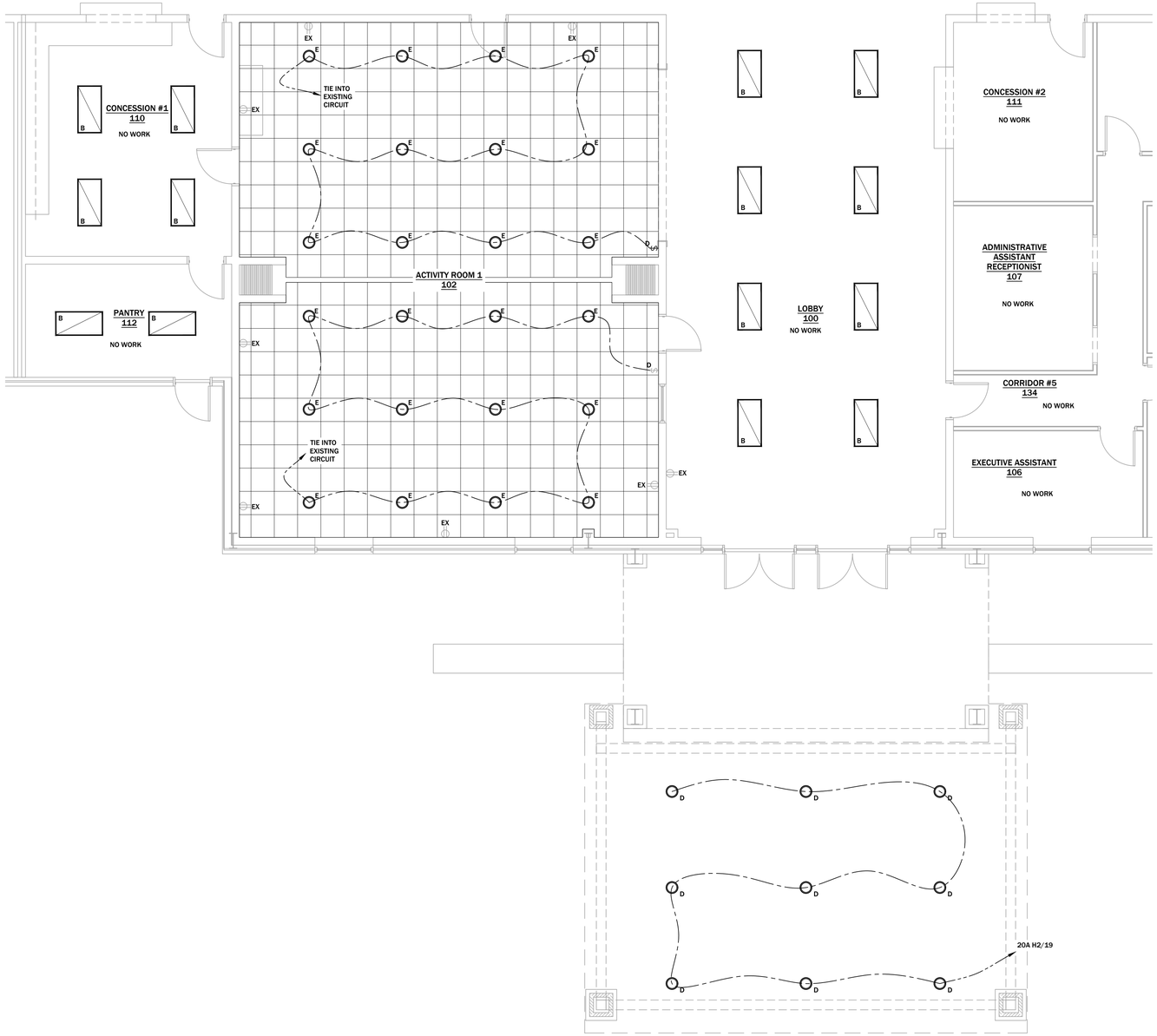
1. ALL ELECTRICAL WORK TO COMPLY WITH NEC 2020 AND NFPA 70E (CURRENT EDITION).
2. ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL LOAD DESIGN REQUIRED FOR ALL LIGHTING POWER, MECHANICAL EQUIPMENT AND ANY OTHER MECHANICAL EQUIPMENT AND ANY OTHER EQUIPMENT REQUIRED AS PART OF THIS PROJECT.
3. FURNISH ALL LABOR AND MATERIALS NECESSARY TO PROVIDE THE COMPLETE PROJECT.
4. CONTRACTOR SHALL COORDINATE CONNECTION OF NEW DEVICES SUCH THAT THE MAXIMUM LOAD ON ANY ONE 20 AMP, 1 POLE CIRCUIT SHALL NOT EXCEED EIGHT (8) 2X4, 4 LAMP FLUORESCENT FIXTURES (OR EQUIVALENT NUMBER OF LAMPS) OR SEVEN (7) GENERAL PURPOSE CONVENIENCE OUTLETS.
5. ALL LIGHT FIXTURES SHALL BE SELECTED BY OWNER AND PROVIDED/INSTALLED BY ELECTRICAL CONTRACTOR.
6. PROVIDE TYPE WRITTEN DIRECTORY FOR PANELS.
7. CONDUCTORS SHALL BE M/C OR IN CONDUIT.

Electrical Key

EX	⇒	EXISTING RECEPTACLE
	⇒	DUPLEX OUTLET 110V - 1 20A CIRCUIT
FO	⇒	DUPLEX FLOOR OUTLET 110V - 1 20A CIRCUIT
CT	⇒	DUPLEX OUTLET 110V - 1 20A CIRCUIT MOUNTED AT COUNTER TOP HEIGHT
UC	⇒	DUPLEX OUTLET 110V - 1 20A CIRCUIT MOUNTED UNDER COUNTER
USB	⇒	DUPLEX USB RECEPTACLE
GFI	⇒	DUPLEX OUTLET 110V - 1 20A CIRCUIT PROVIDE GFI PROTECTED OUTLETS FOR ALL DUPLEX OUTLETS LOCATED WITHIN 3'-0" OF SINKS OR 6'-0" OF TOILET ROOM FIXTURES.
WP	⇒	DUPLEX OUTLET 110V - 1 20A CIRCUIT- WET LOCTATION OUTLET WITH COVER
	⊕	QUADPLEX OUTLET 110V - 1 20A CIRCUIT
EX	◀	EXISTING DATA OUTLET
	◀	DATA OUTLET - CAT5
	T	THERMOSTAT
	OS	OCCUPANCY SENSOR
	$	1-POLE LIGHT SWITCH
	J	JUNCTION BOX WITH HOMERUN
	PP	OCCUPANCY SENSOR POWER PACK

Luminaire Schedule

SYMBOL	LABEL	DESCRIPTION
	A	2x4 LAY-IN LED LIGHT TO BE DEMOLISHED
	B	EXISTING TO REMAIN
	C	NEW 2x4 LAY-IN LED LIGHT
	D	6" EXTERIOR RECESSED LIGHT
	E	6" INTERIOR RECESSED LIGHT
EXIT		EXIT SIGN / EMERGENCY LIGHT COMBO FIXTURE - PROVIDE MATCHING REMOTE HEAD AT EXTERIOR DOOR LOCATIONS
		EMERGENCY LIGHTING - WALL MOUNT - HARD WIRE TO ADJACENT LIGHTING CIRCUIT AS SHOWN



1 POWER AND LIGHTING PLAN
SCALE: 3/16" = 1'-0"

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Gymnasium Power and Lighting Plan

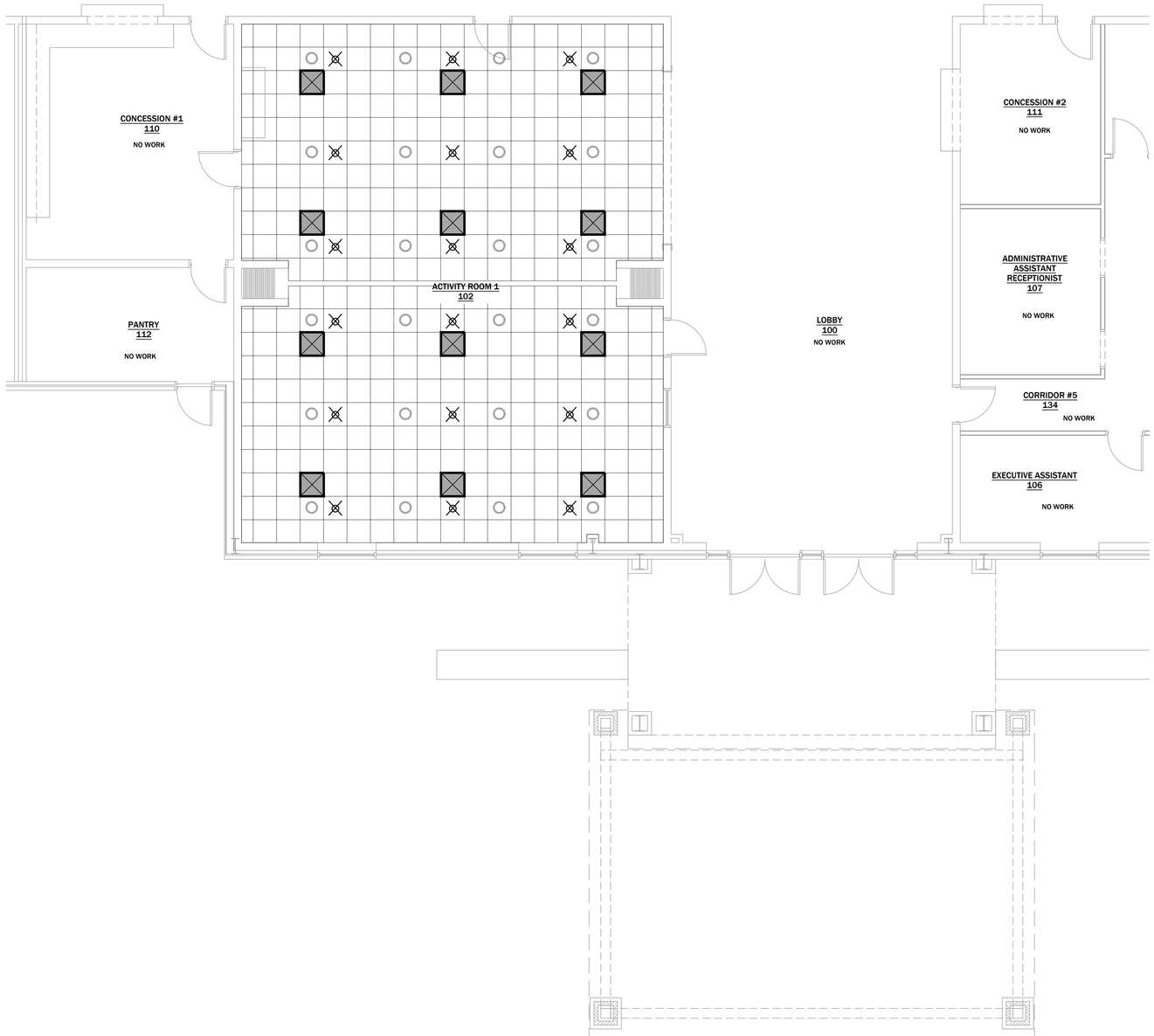
MEP 1.1

HVAC LEGEND:

	EXISTING SUPPLY TO BE RELOCATED OR REMOVED
	EXISTING SUPPLY TO REMAIN
	NEW SUPPLY

SPRINKLER LEGEND:

	EXISTING SPRINKLER TO BE RELOCATED
	EXISTING SPRINKLER TO REMAIN
	NEW SPRINKLER



1 **POWER AND LIGHTING PLAN**
SCALE: 3/16" = 1'-0"

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revisions:

Gymnasium Mechanical Plan

MEP 1.2

PANEL DESCRIPTION:	S	MOUNTING:	RECESSED	VOLT:	120/240	NEUTRAL BUS:	COPPER
		MAIN:	100A	PHASE:	SINGLE ∅	GROUND BUS:	COPPER
				WIRE:	3	MANUFACTURER:	

SERVING	CKT #	BKR AMP.	WIRE		WIRE	BKR AMP.	CKT #	SERVING
LIGHTING	1	20/1	#12		#12	20/1	2	POWER
SPACE	3						4	SPACE
SPACE	5						6	SPACE
SPACE	7						8	SPACE
SPACE	9						10	SPACE
SPACE	11						12	SPACE
SPACE	13						14	SPACE
SPACE	15						16	SPACE
SPACE	17						18	SPACE
SPACE	19						20	SPACE

NOTE:

1. SHUNT TRIP BREAKER TO FEED BUS FOR ENTIRE PANEL AND CONTROLLED BY E-STOP FROM PANEL A.
2. (1) FUEL PUMP BREAKER PER EACH FUEL TANK STORAGE UNIT. COORDINATE TYPE OF STORAGE TANK AND IF MORE THAN 1 IS INSTALLED.
3. ALL DISPENSER BREAKERS TO BE SHUNT TRIP W/ NEUTRAL.

1 **POWER AND LIGHTING PLAN**
SCALE: 1/4" = 1'-0"

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Storage Building Power, Lighting, and Mechanical Plan

MEP 2.0